STATE OF ILLINOIS

DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

PROPOSED HIGHWAY PLANS

F.A.P. ROUTE 326 (IL 47)

SECTION E-8-4-B

OVER I-88 (E-W TOLLWAY)

SUPERSTRUCTURE REPLACEMENT & SUBSTRUCTURE REPAIRS

KANE COUNTY

C-91-321-02

IMPROVEMENT IS LOCATED IN UNINCORPORATED BLACKBERRY TOWNSHIP, KANE COUNTY

FOR INDEX OF SHEETS, SEE SHEET NO. 2

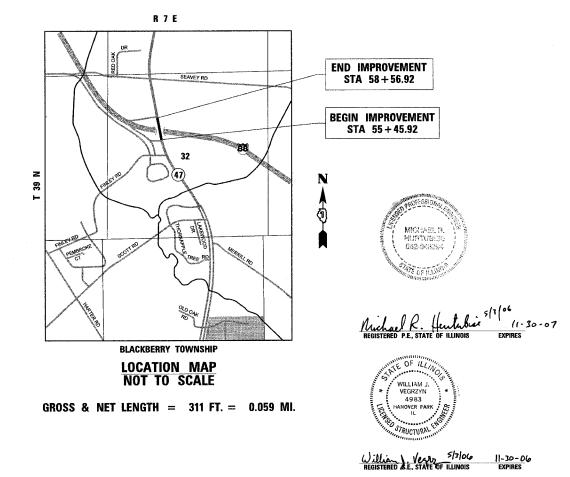
FUNCTIONAL CLASSIFICATION: OTHER PRINCIPAL ARTERIAL

1999 ADT = 10,400

2020 ADT = 11,500

IL 47 DESIGN SPEED LIMIT = 55 MPH IL 47 POSTED SPEED LIMIT = 55 MPH

IL 47 OVER I-88
SN: 045-0082 (EXIST.)
REMOVE AND REPLACE EXISTING
2-SPAN SUPERSTRUCTURE WITH
48" P.P.C. I-BEAMS AND NEW DECK.
REMOVE AND REPLACE EXISTING
VAULTED ABUTMENT SLABS,
APPROACH SLABS AND ALL JOINTS
UTILIZING STAGED CONSTRUCTION.



MICROFILMED		_	
REEL NUMBER			÷,
AWARDED			
RESIDENT ENGINEER			
AS BUILT CHANGES	WERE MADE		
ON THE FOLLOWING	SHEETS		
0 50'	100°		
	1"= 50'		
0	50'	100'	
		— 1" ==	20'

FULL SIZE PLANS HAVE BEEN PREPARED USING STANDARD ENGINEERING SCALES. REDUCED SIZED PLANS WILL NOT CONFORM TO STANDARD SCALES. IN MAKING MEASUREMENTS ON REDUCED PLANS, THE ABOVE SCALES MAY BE USED.

J.U.L.I.E.
JOINT UTILITY LOCATION INFORMATION FOR EXCAVATION
1-800-892-0123

CONTRACT NO. 62531

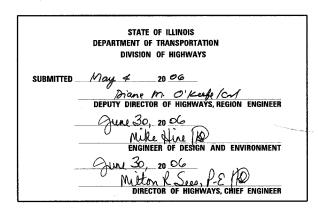
RTE, SECTION COUNTY TOTAL SHEET NO.

326 E-8-4-B KANE 65 1

CONTRACT NO. 62531 D-91-321-02 ISTHA CONTRACT RR-02-5129 ISTHA BRIDGE NO. 1101







PRINTED BY THE AUTHORITY OF THE STATE OF ILLINOIS

DISTRICT ONE - BUREAU OF DESIGN PROJECT MANAGER: RUSS SINHA (847) 705-42

F.A.P. RTE.	SECTION			COUNT	Υ	TOTAL SHEETS	SHEET NO.
326	E-8-4-E	KANE			62	. 2	
FFD. RO	DAD DIST, NO. 1	THE	ois	FFD.	AID	PROJECT	

CONTRACT NO. 62531 ISTHA CONTRACT RR-02-512 ISTHA BRIDGE NO. 1101

INDEX OF SHEETS

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INDEX OF SHEETS

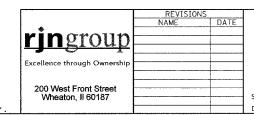
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5-6. TYPICAL SECTIONS
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8. ALIGNMENT, TIES & BENCHMARKS
9. TRAFFIC CONTROL CONSTRUCTION STAGING NOTES AND DETAILS
10. TRAFFIC CONTROL CONSTRUCTION STAGING - STAGE I
11. TRAFFIC CONTROL CONSTRUCTION STAGING - STAGE II
12. TRAFFIC CONTROL CONSTRUCTION STAGING - STAGE II
13. DEMOLITION PLAN
14. PLAN AND PROFILE
15. PAVEMENT MARKING PLAN
16. EROSION CONTRUCT ON ELANDSCAPING PLAN
17-18. APPROACH PAVEMENT DETAILS
19. GENERAL PLAN AND ELEVATION
20. GENERAL PLAN AND ELEVATION
21. STAGE CONSTRUCTION - BRIDGE
22. STAGE CONSTRUCTION - BRIDGE
22. STAGE CONSTRUCTION - BRIDGE
23. TOP OF SLAB BLEVATIONS - II
24. TOP OF SLAB BLEVATIONS - II
25. TOP OF SLAB BLEVATIONS - II
26. TOP OF SLAB BLEVATIONS - IV
27. TOP OF SLAB BLEVATIONS - V
28. NORTH VAULTED SPAN
29. SOUTH VAULTED SPAN
29. SOUTH VAULTED SPAN
30. VAULTED SPAN DETAILS
31. SUPERSTRUCTURE DETAILS
31. SUPERSTRUCTURE DETAILS
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34. SUPERSTRUCTURE DETAILS
35. DRAINAGE SCUPPER, DS-33
36. DRAINAGE SCUPPER, DS-33
36. DRAINAGE SCUPPER, DS-33
36. DRAINAGE SCUPPER, DS-33
36. DRAINAGE SCUPPER, DS-34
37. CONTINUOUS SEAL TYPE NEOPRENE EXPANSION JOINTS
38. FRAMING PLAN
39. 48 IN PPC 1-BEAM
40. 48 IN PPC 1-BEAM
40. 48 IN PPC 1-BEAM
40. 48 IN PPC 1-BEAM
41. BEAPING DETAILS
42. ANCHOR BOLT DETAILS
43. NORTH ABUTMENT DETAILS
44. SOUTH ABUTMENT DETAILS
45. NORTH ABUTMENT DETAILS
46. SOUTH ABUTMENT DETAILS
47. PIER DETAILS
48. PIER REPAIRS
49. BAR SPLICER ASSEMBLY DETAILS
50-ECC. ISATA STANDARDS
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HIGHWAY STANDARDS

000001- 04	STANDARD SYMBOLS, ABBREVIATIONS AND PATTERNS
280001- <i>0</i> 2	TEMPORARY EROSION CONTROL SYSTEMS
515001- 02	NAME PLATE FOR BRIDGES
630001- 06	STEEL PLATE BEAM GUARDRAIL
630301- 03	SHOULDER WIDENING FOR TYPE 1 (SPECIAL) GUARDRAIL TERMINALS
631031 - <i>0</i> 5	TRAFFIC BARRIER TERMINAL, TYPE 6
635006~ 02	REFLECTOR AND TERMINAL MARKER PLACEMENT
635011- <i>01</i>	REFLECTOR MARKER AND MOUNTING DETAILS
701422- 0 1	LANE CLOSURE, MULTILANE, FOR SPEEDS ≥ 45 MPH TO 55 MPH
701426- <i>0</i> 2	LAND CLOSURE, MULITLANE, INTERMITTENT OR MOVING OPERATIONS, FOR SPEEDS > 45 MPH
702001 <i>-06</i>	TRAFFIC CONTROL DEVICES
704001- 02	TEMPORARY CONCRETE BARRIER
720011	METAL POSTS FOR SIGNS, MARKERS & DELINEATORS
780001-01	TYPICAL PAVEMENT MARKINGS

ISHTA STANDARDS (INCLUDED AS PLAN SHEETS 62A- 62C.)

STANDARD PROTECTIVE DEVICES LANE CLOSURE DETAILS LANE CLOSURE DETAILS ADVANCE SIGNAGE



ILLINOIS DEPARTMENT OF TRANSPORTATION F.A.P. ROUTE 326 (IL 47)

> INDEX OF SHEETS & STANDARDS

SCALE: VERT. HORIZ. DATE: MARCH 21, 2006

_	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	326	E-8-4-B	KANE	62	3
	FFD, RO	AD DIST. NO. 1 TILLIN	DIS FED. AID	PROJECT	

CONTRACT NO. 62531 ISTHA CONTRACT RR-02-512 ISTHA BRIDGE NO. 1101

GENERAL NOTES - ROADWAY

- SAW CUTTING OF PAVEMENTS, SHOULDERS, ETC. SHALL BE FULL DEPTH AND SHALL RESULT IN A CLEAN, STRAIGHT EDGE ON THE PORTION REMAINING. ALL SAW CUTTING SHALL BE CONSIDERED INCLUDED IN THE COST OF THE ITEM REMOVED.
- 2. USE NO. 8 EPOXY-COATED TIE BARS (OR DOWEL BARS) CONFORMING TO ARTICLE 1003.10(B)(2) OF THE STANDARD SPECIFICATIONS FOR LONGITUDINAL CONSTRUCTION JOINT GROUTED-IN-PLACE TIE BAR AS SHOWN ON STATE STANDARD 420001 AND FOR TYING PORTLAND CEMENT CONCRETE PAVEMENT TO EXISTING CONCRETE PAVEMENT AS SHOWN ON THE PLANS. THE TIE BARS WILL NOT BE PAID FOR SEPARATELY BUT SHALL BE INCLUDED IN THE COST OF THE PAVEMENT ITEMS BEING CONSTRUCTED.
- 3. THE THICKNESS OF BITUMINOUS MIXTURES SHOWN ON THE PLANS IS THE NOMINAL THICKNESS. DEVIATIONS FROM THE NOMINAL THICKNESS WILL BE PERMITTED WHEN SUCH DEVIATIONS OCCUR DUE TO IRREGULARITIES IN THE BASE ON WHICH THE BITUMINOUS MIXTURE IS PLACED.
- 4. EXCEPT AS NOTED ON THE PLANS, PAVEMENT GRADES SHOWN ARE AT THE TOP OF PAVEMENT SURFACES.
- 5. THE ENGINEER SHALL BE THE SOLE JUDGE CONCERNING CURING TIME FOR THE VARIOUS PAVEMENT LIFTS.

GENERAL NOTES - TRAFFIC CONTROL & PROTECTION

- STAGING PROCEDURES PRESENTED ARE THE SUGGESTED SEQUENCE OF OPERATIONS. AT HIS OPTION, THE CONTRACTOR MAY SUBMIT AN ALTERNATIVE STAGING PROPOSAL TO THE ENGINEER FOR HIS APPROVAL.
- 2. TRAFFIC CONDITIONS, ACCIDENTS AND OTHER UNFORESEEN EMERGENCY CONDITIONS MAY REQUIRE THE ENGINEER TO RESTRICT, MODIFY OR REMOVE LANE CLOSURES OF CHANNELIZATION SHOWN IN THE PLANS. THE CONTRACTOR SHALL MAKE THE NECESSARY ADJUSTMENTS AS DIRECTED BY THE ENGINEER WITHOUT DELAY. THE CONTRACTOR SHALL RESPOND TO ANY REQUEST MADE BY THE ENGINEER FOR CORRECTION WITHIN TWO HOURS FROM THE TIME OF NOTIFICATION.
- 3. ALL TEMPORARY PAVEMENT MARKINGS PROPOSED WITHIN THE WORK AREA SHALL BE COMPLETED PRIOR TO THE CONSTRUCTION PHASE CHANGE.
- THE RESIDENT ENGINEER SHALL CONTACT DON CHIARUG; AREA TRAFFIC ENGINEER AT (847) 741-9857, A
 MINIMUM OF TWO WEEKS PRIOR TO PLACEMENT OF PERMANENT PAVEMENT MARKINGS.

GENERAL NOTES – DRAINAGE & UTILITIES

- BEFORE STARTING ANY EXCAVATION, THE CONTRACTOR SHALL CALL J.U.L.I.E. AT 1-800-892-0123 FOR FIELD LOCATIONS OF BURIED UTILITIES. (48 HOUR NOTIFICATION REQUIRED).
- 2. THE CONTRACTOR SHALL COORDINATE CONSTRUCTION ACTIVITIES WITH UTILITY COMPANIES AND THE DEPARTMENT OF TRANSPORTATION

GENERAL NOTES - MISCELLANEOUS

- 1. ALL ELEVATIONS SHOWN ON THESE PLANS ARE ON THE U.S.G.S DATUM
- 2. THE CONTRACTOR WILL NOT BE ALLOWED TO SET UP A YARD OR FIELD OFFICE ON STATE PROPERTY WITHOUT THE WRITTEN PERMISSION FROM THE DEPARTMENT
- 3. ANY REFERENCE TO A STANDARD IN THESE PLANS SHALL BE INTERPRETED TO MEAN THE EDITION AS INDICATED BY THE SUBNUMBER LISTED ON THE INDEX OF SHEETS OR THE COPY OF THE STANDARD INCLUDED IN THESE PLANS.

GENERAL NOTES - EROSION CONTROL & LANDSCPING

- . ALL EROSION CONTROL MEASURES SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF THE STANDARD SPECIFICATIONS AND OF THE APPLICABLE STATE STANDARDS FOR THE ENTIRE DURATION OF THE CONTRACT, OR UNTIL SUCH TIME AS DIRECTED BY THE ENGINEER.
- ALL AREAS DISTURBED AND RESTORED WITH SEEDING SHALL BE COVERED WITH AN EROSION CONTROL BLANKET IN ACCORDANCE WITH SECTION 251 OF THE STANDARD SPECIFICATIONS.
- 3. SEEDING WILL NOT BE PERMITTED AT ANY TIME WHEN THE GROUND IS FROZEN, WET, OR IN AN UNTILLABLE CONDITION. LOCATIONS TO BE SEEDED ARE AS OUTLINED IN THESE PLANS. ANY ADDITIONAL AREAS SHALL BE DETERMINED BY THE ENGINEER.
- 3. BEFORE STARTING CONSTRUCTION ON STAGE 2 OF THE PROJECT, EROSION CONTROL BLANKET AND SEEDING, CLASS 3 SHALL BE PLACED AFTER FINAL GRADING HAS BEEN COMPLETED ON THE FIRST PHASE OF CONSTRUCTION.
- 4. THE CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS FOR THE PROTECTION OF EXISTING PLANT MATERIAL FOR WHICH THE CONTRACT DOES NOT PROVIDE REMOVAL. THE PROTECTION OF EXISTING PLANT MATERIAL AND THE REPAIR OR REPLACEMENT OF EXISTING PLANT MATERIAL DAMAGED BY THE CONTRACTOR SHALL BE DONE IN ACCORDANCE WITH THE REQUIREMENTS OF SECTION 201 OF THE STANDARD SPECIFICATIONS.
- 5. LANDSCAPE CONTRACTOR SHALL STAKE THE LOCATION OF ALL SEEDING AND HAVE ALL PLANTING LAYOUTS APPROVED BY THE ENGINEER PRIOR TO INSTALLATION.
- APPROVED WATERING EQUIPMENT SHALL BE AT THE SITE OF THE WORK AND IN OPERATING CONDITION PRIOR TO STARTING THE SEEDING OPERATION AND DURING ALL SEEDING OPERATIONS OR SEEDING WILL NOT BE ALLOWED.
- 7. THE CONTRACTOR SHALL REPAIR IN KIND ANY AREAS DAMAGED AS A RESULT OF LANDSCAPING OPERATIONS
- . THE FINISHED EARTHWORK SHALL HAVE A VEGETATION SUSTAINING SOIL COVERING THE TOP FOUR (4) INCHES IN AREAS TO BE SEEDED OR SODDED.

REVISIONS
NAME DATE

NAME DATE

Excellence through Ownership

200 West Front Street
Wheaton, II 60187

ILLINOIS DEPARTMENT OF TRANSPORTATION
F.A.P. ROUTE 326 (IL 47)

GENERAL NOTES

SCALE: VERT. HORIZ.

DRAWN BY: NEC CHECKED BY: SPF

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_	F.A.P.	SECTION		,	COUNT	Υ	TOTAL	SHEET
	326	E-8-4-E	3		KANE		62	4
	FED. RO	DAD DIST. NO. 1	TELETRA	210	EED.	ΔID	PROJECT	

Γ		CHRARADY OF CHARITITIES			/00% S7		
		SUMMARY OF QUANTITIES		URBAN TOTAL	J000-2A		81-2A 045-0082
	CODE # 63200310	PAY ITEM GUARDRAIL REMOVAL	FOOT	QUANTITY 235	ROADWAY 235	IDOT	ISTHA
_	67000400	ENGINEER'S FIELD OFFICE, TYPE A	CAL MO	8	8		
		MOBILIZATION	L SUM	1	0.5	0.5	
	67100100	TRAFFIC CONTROL AND PROTECTION, STANDARD 701422	L SUM	1	1	0.3	
_	70100320	TRAFFIC CONTROL AND PROTECTION, STANDARD 101422	L SUM	1	1		
	70101800	TRAFFIC CONTROL SURVEILLANCE	CAL DA	20	20		
	70103815			364	364		
L	70300100	SHORT-TERM PAVEMENT MARKING	FOOT				
L	70300510	PAVEMENT MARKING TAPE, TYPE III - LETTERS AND SYMBOLS	SQ FT	218	218		
	70300540	PAVEMENT MARKING TAPE. TYPE III 6"	FOOT	863	863		
E	70300560	PAVEMENT MARKING TAPE, TYPE III 12"	FOOT	219	219		
E	70400100	TEMPORARY CONCRETE BARRIER	FOOT	540	540		
	70400200	RELOCATE TEMPORARY CONCRETE BARRIER	FOOT	480	480		
_	78008200	POLYUREA PAVEMENT MARKING TYPE I - LETTERS AND SYMBOLS	SQ FT	73	73		
	78008210	POLYUREA PAVEMENT MARKING TYPE I - LINE 4"	FOOT	9,057	9,057		
H	78008230	POLYUREA PAVEMENT MARKING TYPE I - 6"	FOOT	305	305		
	78100100	RAISED REFLECTIVE PAVEMENT MARKER	EACH	2	2		
_	78100105	RAISED REFLECTIVE PAVEMENT MARKER (BRIDGE)	EACH	12	12		
F	78200410	GUARDRAIL MARKERS - TYPE A	EACH	8	8		
F	78200520	BARRIER WALL MARKERS, TYPE B	EACH	6	6		
F	78201000	TERMINAL MARKER - DIRECT APPLIED	EACH	2	2		
F	78300100	PAVEMENT MARKING REMOVAL	SQ FT	2,717	2,717		
F	X0322256	TEMPORARY INFORMATION SIGNING	SQ FT	51.4	51.4		
F	X0325305	STRUCTURAL REPAIR OF CONCRETE (DEPTH EQUAL TO OR LESS THAN 5")	SQ FT	168			168
F	X0323082	DRAINAGE SCUPPERS. DS-33	EACH	2		2	
	X0751105	REMOVAL OF EXISTING CONCRETE DECK NO. 1	EACH	1		1	
_	X0751205	REMOVAL OF EXISTING CONCRETE DECK NO. 2	EACH	11		1	
_	X3550405	BITUMINOUS BASE COURSE SUPERPAVE 7 1/4"	SQ YD	2,765	2,765		
	X4066424	BITUMINOUS CONCRETE SURFACE COURSE, SUPERPAVE MIX "D", N50	TON	271	271		
_	X6050700	REMOVE INLET BOX	EACH	2	2		
	X7011015	TRAFFIC CONTROL AND PROTECTION (EXPRESSWAYS)	L SUM	1	1		
	X7030100	WET TEMPORARY PAVEMENT MARKING TAPE TYPE III	FOOT	23,216	23,216		
	Z0002600	BAR SPLICERS	EACH	807		803	4
_	Z0018800	DRAINAGE SYSTEM	L SUM	1			1
	Z0030250	IMPACT ATTENUATORS, TEMPORARY (NON-REDIRECTIVE), TEST LEVEL 3	EACH	2	2		
L	Z0030350	IMPACT ATTENUATORS, RELOCATE (NON-REDIRECTIVE), TEST LEVEL 3	EACH	2	2		
E	Z0047300	PROTECTIVE SHIELD	SQ YD	3,138		3,138	
L	Z0013798	CONSTRUCTION LAYOUT	L SUM	1 1	1		1
	70076600	-TRAINEES	-HOUR-				
-	X0325490 FTY-3N	FURNISH AND INSTALL DOOR	EACH	. 2			2

PECIALTY ITEM

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QUANTITY ROADWAY IDOT ISTHA

235

0.5

39

2,075

617

536

73

147

485

815

2,448

34

350

2,610 2,610

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734.9

1,800

1,984

2,075 2,075

CONSTRUCTION CODE

100% ISHTA

SN. # 045-0082

* SPECIALTY ITEM

SUMMARY OF QUANTITIES

EARTH EXCAVATION

20201006 GRADING AND SHAPING SHOULDERS

21101615 TOPSOIL FURNISH AND PLACE, 4"

25000400 NITROGEN FERTILIZER NUTRIENT

25000500 PHOSPHORUS FERTILIZER NUTRIENT

28000250 TEMPORARY EROSION CONTROL SEEDING

31101100 SUB-BASE GRANULAR MATERIAL, TYPE B

42001400 BRIDGE APPROACH PAVEMENT (SPECIAL)

44000500 COMBINATION CURB AND GUTTER REMOVAL

50101500 REMOVAL OF EXISTING SUPERSTRUCTURES

50300310 ELASTOMERIC BEARING ASSEMBLY, TYPE I

60607400 COMBINATION CONCRETE CURB AND GUTTER, TYPE B-9.24

* 63100167 TRAFFIC BARRIER TERMINAL TYPE 1. SPECIAL (TANGENT)

50800205 REINFORCEMENT BARS, EPOXY COATED

60618300 CONCRETE MEDIAN SURFACE, 4 INCH

63100085 TRAFFIC BARRIER TERMINAL, TYPE 6

63000000 STEEL PLATE BEAM GUARDRAIL, TYPE A

50401005 FURNISHING AND ERECTING PRECAST PRESTRESSED CONCRETE I-BEAMS, 48 IN.

42001420 BRIDGE APPROACH PAVEMENT CONNECTOR (PCC)

31101400 SUB-BASE GRANULAR MATERIAL, TYPE B 6"

25000600 POTASSIUM FERTILIZER NUTRIENT

25100630 EROSION CONTROL BLANKET

28000400 PERIMETER EROSION BARRIER 28100107 STONE RIPRAP, CLASS A4

28200200 FILTER FABRIC

42001300 PROTECTIVE COAT

44000100 PAVEMENT REMOVAL

44003100 MEDIAN REMOVAL

44000700 APPROACH SLAB REMOVAL

44004250 PAVED SHOULDER REMOVAL

50102400 CONCRETE REMOVAL 50104650 SLOPE WALL REMOVAL

50300225 CONCRETE STRUCTURES

50300255 CONCRETE SUPERSTRUCTURE

50300260 BRIDGE DECK GROOVING

51100100 SLOPE WALL 4 INCH

51500100 NAME PLATES 58700200 BRIDGE SEAT SEALER 59000100 EPOXY CRACK SEALING

48101200 AGGREGATE SHOULDERS, TYPE B 48202600 BITUMINOUS SHOULDERS SUPERPAVE 8"

50300150 NEOPRENE EXPANSION JOINT 2"

25000210 SEEDING, CLASS 2A

20201200 REMOVAL AND DISPOSAL OF UNSUITABLE MATERIAL

20200100

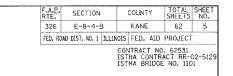
PAY ITEM

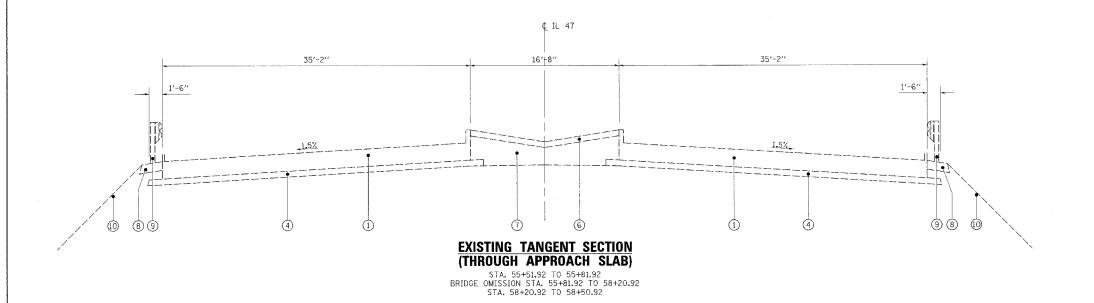
<u>rjngroup</u> 200 West Front Street Wheaton, II 60187

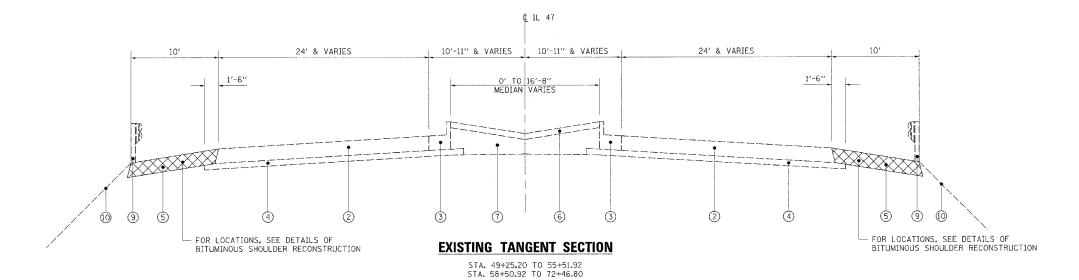
ILLINOIS DEPARTMENT OF TRANSPORTATION F.A.P. ROUTE 326 (IL 47)

> SUMMARY OF QUANTITIES

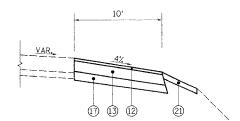
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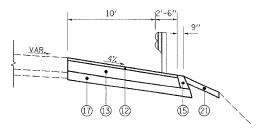


NOTE: ROADWAY IS SUPERELEVATED STA. 49+25.20 TO 54+16.50



DETAIL OF BITUMINOUS SHOULDER RECONSTRUCTION

STA. 49+33.03 TO 52+44.06, LT STA. 60+48.77 TO 72+46.80, RT STA. 62+99.61 TO 65+85.88, LT



DETAIL OF BITUMINOUS SHOULDER RECONSTRUCTION WITH BITUMINOUS STABILIZATION AT GUARDRAIL

STA. 58+06.11 TO 60+48.77, RT STA. 58+35.73 TO 61+95.11, LT

LEGEND

- 1 EXISTING 12" APPROACH PAVEMENT
- ② EXISTING 10" REINFORCED P.C.C. PAVEMENT
- 3 EXISTING COMBINATION CONCRETE CURB AND GUTTER B-9.24
- 4 EXISTING 4" STABILIZED BASE COURSE
- (5) EXISTING BITUMINOUS SHOULDER (DEPTH VARIES 7" TO 9")
- 6 EXISTING CONCRETE BARRIER MEDIAN
- 7 EXISTING GRANULAR SUB-BASE
- 8 EXISTING AGGRETATE SHOULDER
- 9 EXISTING GUARDRAIL
- 10 EXISTING EMBANKMENT
- 11) PROPOSED 15" APPROACH PAVEMENT
- $\ensuremath{\textcircled{\sc 12}}$ Proposed bituminous concrete surface course, superpave, MIX "D", N50, $1\ensuremath{\sc 134}\ensuremath{\sc 134}\ensuremath{\sc 134}$ "
- (3) PROPOSED BITUMINOUS BASE COURSE, SUPERPAVE, 71/4"
- (4) PROPOSED BITUMINOUS SHOULDERS, SUPERPAVE, 8"
- (15) PROPOSED AGGREGATE SHOULDER
- (6) PROPOSED SUB-BASE GRANULAR MATERIAL, TYPE B, 4"
- 17) PROPOSED SUB-BASE GRANULAR MATERIAL TYPE B, 6"
- (18) PROPOSED SUB-BASE GRANULAR MATERIAL, TYPE B
- 19 PROPOSED CONCRETE MEDIAN SURFACE, 4"
- PROPOSED STEEL PLATE BEAM GUARDRAIL
- $\fill \ensuremath{\text{(21)}}$ Proposed topsoil, furnish and place, $4^{\prime\prime}$

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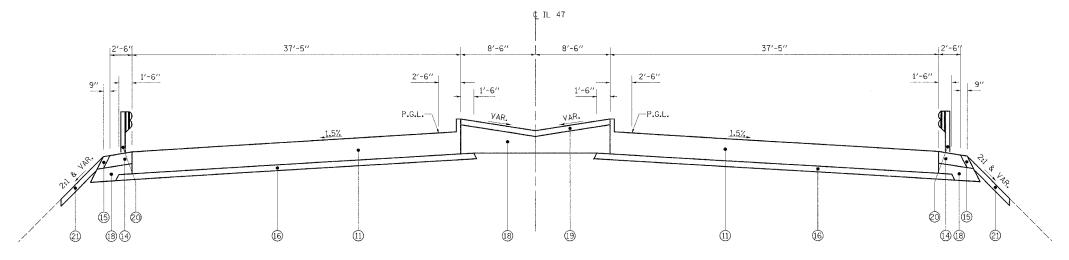
ILLINOIS DEPARTMENT OF TRANSPORTATION
F.A.P. ROUTE 326 (IL 47)

TYPICAL SECTIONS

SCALE: VERT. HORIZ. DATE: MARCH 21, 2006

F.A.P. RTE.	SECT	ION			COUNT	Υ	TOTAL SHEETS	SHEET NO.
326	E-8-4-B			KANE			62	6
FED. RC	AD DIST. N	0. 1	ILLIN	ois	FED.	AID	PROJECT	

CONTRACT NO. 62531 ISTHA CONTRACT RR-02-5129 ISTHA BRIDGE NO. 1101



PROPOSED TANGENT SECTION

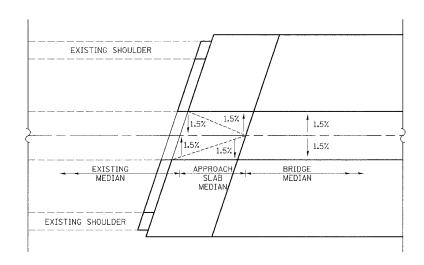
STA. 55+45.92 TO 55+81.92 BRIDGE OMISSION STA. 55+81.92 TO 58+50.92 STA. 58+20.92 TO 58+56.92

THE FOLLOWING MIXTURE REQUIREMENTS ARE APPLICABLE FOR THIS PROJECT:

	BITUMINOUS CONCRETE	BITUMINOUS CONCRETE BASE	BITUMINOUS SHOULDER
	SURFACE COURSE SUPERPAVE	COURSE SUPERPAVE	SUPERPAVE
PG GRADE	PG 64-22	PG 58-22	PG 58-22
MAX % RAP ALLOWABLE *	15%	50%	50%
DESIGN AIR VOIDS	4.0% @ N50	2.0% @ N50	2.0% @ 30 GYR
MIXTURE COMPOSTION	IL 12.5 OR IL 9.5	BAM	BAM
FRICTION AGGREGATE	MIXTURE D		

* IF RAP OPTION IS SELECTED, THE ASPHALT CEMENT GRADE MAY NEED TO BE ADJUSTED. THIS WILL BE DETERMINED BY THE ENGINEER.

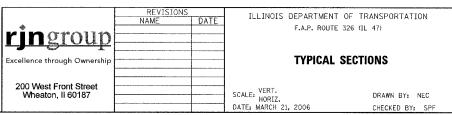
THE UNIT WEIGHT USED TO CALCULATE ALL BITUMINOUS SURFACE MIXTURE QUANTITIES IS 112 POUNDS PER SQUARE YARD PER INCH.



PLAN VIEW DETAIL OF MEDIAN PAVEMENT TRANSITION APPROACH SLAB

<u>LEGEND</u>

- ① EXISTING 12" APPROACH PAVEMENT
- ② EXISTING 10" REINFORCED P.C.C. PAVEMENT
- 3 EXISTING COMBINATION CONCRETE CURB AND GUTTER B-9.24
- 4 EXISTING 4" STABILIZED BASE COURSE
- (5) EXISTING BITUMINOUS SHOULDER (DEPTH VARIES 7" TO 9")
- 6 EXISTING CONCRETE BARRIER MEDIAN
- 7 EXISTING GRANULAR SUB-BASE
- 8 EXISTING AGGRETATE SHOULDER
- (10) EXISTING EMBANKMENT
- 11) PROPOSED 15" APPROACH PAVEMENT
- 2 PROPOSED BITUMINOUS CONCRETE SURFACE COURSE, SUPERPAVE, MIX "D", N50, $1\frac{N}{2}\!\!\!\!/_4$ "
- (3) PROPOSED BITUMINOUS BASE COURSE, SUPERPAVE, 71/4"
- (14) PROPOSED BITUMINOUS SHOULDERS, SUPERPAVE, 8"
- (15) PROPOSED AGGREGATE SHOULDER
- (6) PROPOSED SUB-BASE GRANULAR MATERIAL, TYPE B, 4"
- (7) PROPOSED SUB-BASE GRANULAR MATERIAL TYPE B, 6"
- (18) PROPOSED SUB-BASE GRANULAR MATERIAL, TYPE B
- 19 PROPOSED CONCRETE MEDIAN SURFACE, 4"
- 20 PROPOSED STEEL PLATE BEAM GUARDRAIL
- 21) PROPOSED TOPSOIL, FURNISH AND PLACE, 4"



PAVEMENT SCHEDULE											
LOCATION	SUB-BASE GRANULAR MATERIAL TYPE B (CU YD)	SUB-BASE GRANULAR MATERIAL TYPE B 6" (SQ YD)	PROTECTIVE COAT (SQ YD)	BRIDGE APPROACH PAVEMENT (SPECIAL) (SQ YD)	BRIDGE APPROACH PAVEMENT CONNECTOR (PCC) (SQ. YD)	BITUMINOUS SHOULDERS SUPERPAVE 8" (SQ YD)	BITUMINOUS BASE COURSE SUPERPAVE, 7 1/4" (SQ YD)	CONCRETE MEDIAN SURFACE, 4" (SQ FT)	BITUMINOUS CONCRETE SURFACE COURSE, SUPERPAVE, MIX "D", N50 (TON)		
NORTHBOUND											
STA. 52+24.85 TO 53+55.00											
STA, 53+55.00 TO 55+81.92		77	141	125	16	8.5			<u>l</u>		
STA, 58+20.92 TO 61+58.00		1634	186	161	25	8.5	306		30		
STA. 61+58.00 TO 72+46.80							1330	-	130		
MEDIAN											
STA. 55+45.92 TO 55+81.92	42							606			
STA. 58+20.92 TO 58+56.92	20							202			
SOUTHBOUND											
STA. 49+25,20 TO 52+44.10		316					315		31		
STA. 53+55.00 TO 55+81.92		7	141	125	16	8.5	7		1		
STA. 58+20.92 TO 61+58.00		478	141	125	16	8.5	480		47		
STA, 61+58.00 TO 61+92.63											
STA, 62+99.78 TO 65+85.87		318					320		31		
TOTAL	62	2760	609	536	73	34.0	2765	808	271		

F.A.P. RTE.	SECTION			-	COUN	Υ	TOTAL	SHEET NO.		
326		E-8	3-4	- E	3		KANE	E	62	7
FED. RO	DAD	DIST.	NO.	1	ILLIN	OIS	FED.	AID	PROJECT	

STHA CONTRACT RR-02-512

CURB & GUTTER / MEDIAN							
LOCATION	COMBINATION CONCRETE CURB AND GUTTER TYPE B-9.24 (FOOT)	CONCRETE MEDIAN SURFACE, 4 INCH (SQ FT)	PROTECTIVE COAT (SQ YD)				
MEDIAN							
RT, STA, 55+43,10 TO 55+49,10	- 6		2				
RT, STA, 58+52.12 TO 58+58.12	6		2				
STA, 55+45,92 TO 55+81,92		606					
LT. STA. 55+48.74 TO 55+54.74	6		2				
LT. STA. 58+53.74 TO 58+59.74	6		2				
STA, 58+20.92 TO 58+56.92		201.5					
TOTAL	24	808	8				

				PAVEME	NT MARKING SCHED	ULE					
LOCATION	PAVEMENT MARKING MARKING		POLYUREA PAVEMENT MARKING TYPE I – LINE 4"(FOOT)		RAISED REFLECTIVE PAVEMENT MARKER (EACH)		PAVEMENT MARKING TAPE, TYPE III – LETTERS & SYMBOLS (SQ. FT)	PAVMENT MARKING TAPE, TYPE III 6" (FOOT)	PAVEMENT MARKING TAPE, TYPE III 12" (FOOT)	GUARDRAIL MARKERS – TYPE A (EACH)	BARRIER WALL MARKERS TYPE B (EACH)
STAGE I (NORTHBOUND)											
STA, 46+22.85 TO 67+58.18	4,838										
STAGE I (MEDIAN & TURN LANE) STA. 47+23.28 TO 61+90.41 STA. 63+82.74 TO 71+00.00	2,024 1,685						72.6	284	38 122		
STAGE I (SOUTHBOUND)											
STA. 49+39.00 TO 52+28,43	294										
STA. 53+54,92 TO 61+94.86 STA. 62+98.37 TO 65+85.73	294 642 288										
STAGE II (NORTHBOUND) STA, 51+20.44 TO 72+46.66	5,254										
STAGE II (MEDIAN & TURN LANE)											
STA. 45+00.00 TO 53+55.00	1135						72.6	232	29		
STA. 53+55.00 TO 61+58.00 STA. 61+58.00 TO 72+73.68	1,135 1,516								30		
STAGE II (SOUTHBOUND) STA, 49+48,97 TO 53+55.00	527										
STA. 53+55.00 TO 61+83.81 STA. 61+83.81 TO 67+77.15	879 1,173										
NORTHBOUND STA. 44+96.86 TO 72+73.67			1,916								
STA. 49+47.77 TO 61+58.00 STA. 53+55.00 TO 61+58.00			340		2	6				4	3
CENTERLINE, MEDIAN & TURN LANE											
STA, 44+99.78 TO 53+55.00	444	73	1,552 1,587	305			72.6	347			
STA. 53+55.00 TO 61+58.00 STA. 61+58.00 TO 72+73.67	1,622 895	73	2,182	503			0.47	571			
SOUTHBOUND											
STA, 44+99,70 TO 47+99,35 STA, 49+25,15 TO 52+45,72			326								
STA. 53+55.00 TO 61+90.41 STA. 63+53.62 TO 72+73.67			865 289			6				4	3
TOTAL	23,216	73	9,057	305	2	12	218	863	219	8	6

REMOVAL SCHEDULE								
	PAVEMENT REMOVAL (SQ YD)	COMBINATION CURB & GUTTER REMOVAL (FOOT)	APPROACH SLAB REMOVAL (SQ YD)	MEDIAN REMOVAL (SQ FT)	PAVED SHOULDER REMOVAL (SQ YD)	GUARDRAIL REMOVAL (FOOT)	PAVEMENT MARKING REMOVAL (SQ. FT)	INLET BOX REMOVAL (EACH)
NORTHBOUND								
STA. 51+82.38 TO 55+81.92								
STA. 53+55.00 TO 55+81.92	16		113		8	25	60	1 1
STA. 58+20.92 TO 61+58.00	25		148	ļ	1,432	89	476	
STA. 61+58.00 TO 72+46.80		·						<u> </u>
MEDIAN								
STA. 55+45.92 TO 55+81.92		72		608			687	
STA. 58+20.92 TO 58+56.92		75		207			1,076	
SOUTHBOUND		, , , , , , , , , , , , , , , , , , , ,						
STA. 49+25.20 TO 52+44.10					299		110	
STA. 53+55.00 TO 55+81.92	16		112		8	25		1
STA. 58+20.92 TO 61+58.00	16		112		382	96	212	
STA. 61+58.00 TO 61+92.63							-	
STA. 62+99.78 TO 65+85.87				-	319		96	
TOTAL	73	147	485	815	2,448	235	2,717	2

EARTHWORK SCHEDULE								
LOCATION	EARTH EXCAVATION (CU YD)	REMOVAL AND DISPOSAL OF UNSUITABLE MATERIAL (CU YD)	EMBANKMENT (CU YD)	EARTH EXCAVATION ADJUSTED FOR SHRINKAGE (CU YD)	EARTHWORK BALANCE (+) WASTE (-) SHORTAGE (CU YD)	FURNISHED EXVACATION (CU YD)		
STA. 50+00.00 TO STA. 55+81.92	130	25	8	98	(+) 90	0		
STA. 58+20.92 TO STA. 71+00.00	420	210	33	315	(+) 282	0		

	REVISIONS		
	NAME	DATE	
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Excellence through Ownership			
200 West Front Street	-		
Wheaton, II 60187			

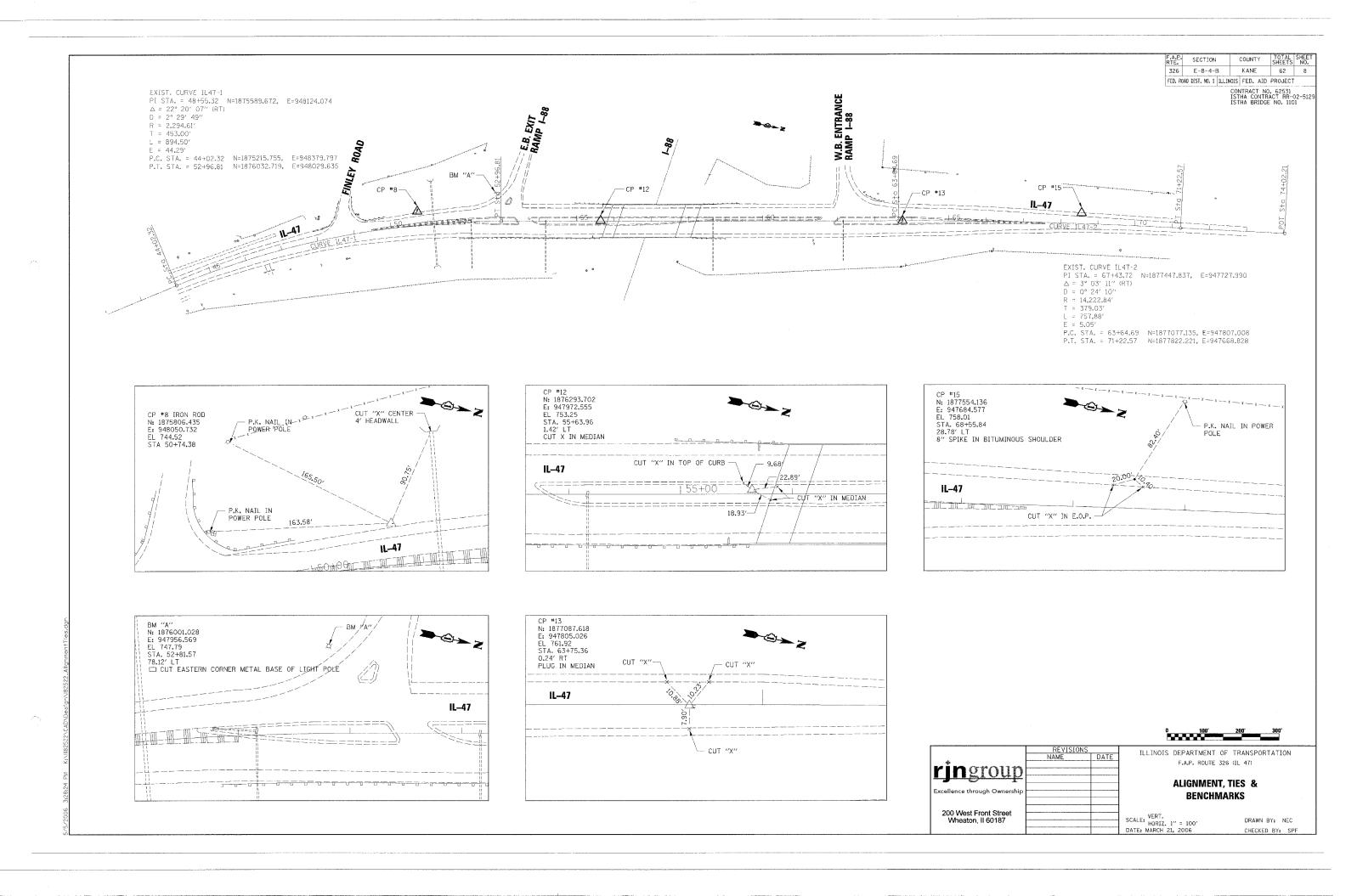
ILLINOIS DEPARTMENT OF TRANSPORTATION F.A.P. ROUTE 326 (IL 47)

SCHEDULE OF QUANTITIES

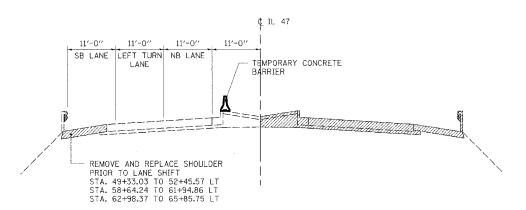
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CONTRACT NO. 62531 ISTHA CONTRACT RR-02-51



STAGE I REMOVAL

TEMPORARY CONCRETE

TEMPORARY CONCRETE

BARRIER

11'-0"

SB LANE

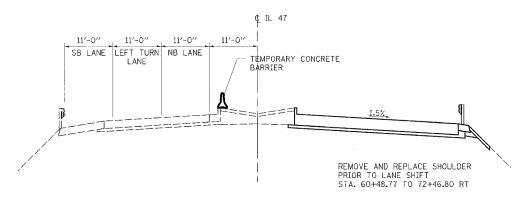
LEFT TURN

NB LANE

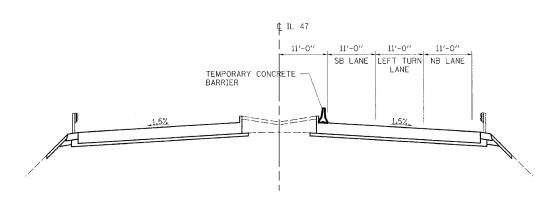
LANE

1.5%

STAGE II REMOVAL



STAGE I CONSTRUCTION



STAGE II CONSTRUCTION

STAGE I CONSTRUCTION

- 1. REMOVE AND REPLACE SOUTHBOUND SHOULDERS AS SHOWN IN THESE PLANS UNDER TRAFFIC CONTROL STD. 701422
- 2. INSTALL PROTECTIVE SHIELD UNDER TRAFFIC CONTROL AND PROTECTION (EXPRESSWAY) FOLLOWING REQUIREMENTS OF ISTHA TRAFFIC CONTROL STANDARD E1, E10 & E10A.
- 3. INSTALL TRAFFIC CONTROL AND PROTECTION (SPECIAL) AS DETAILED IN THESE PLANS
- 4. REMOVE NORTHBOUND PORTION OF EXISTING STRUCTURE
- 5, REMOVE NORTHBOUND PORTION OF EXISTING APPROACH SLAB AND SHOULDER
- 6. CONSTRUCT NORTHBOUND PORTION OF STRUCTURE
- 7. CONSTRUCT NORTHBOUND PORTION OF PROPOSED APPROACH SLAB
- 8. CONSTRUCT NORTHBOUND SHOULDER
- 9. INSTALL GUARDRAIL
- 10. INSTALL SEEDING AND RESTORATION

STAGE II CONSTRUCTION

- RELOCATE TRAFFIC CONTROL AND PRODECTION (SPECIAL) TO NORTH BOUND LANES AS DETAILED IN THESE PLANS
- 2. REMOVE SOUTHBOUND PORTION OF EXISTING STRUCTURE
- 3. REMOVE SOUTHBOUND PORTION OF EXISTING APPROACH SLAB AND SHOULDER
- 4. CONSTRUCT SOUTHBOUND PORTION OF STRUCTURE
- 5. CONSTRUCT SOUTHBOUND SHOULDER
- 6. CONSTRUCT SOUTHBOUND PORTION OF PROPOSED APPROACH SLAB
- 7. INSTALL GUARDRAIL
- 8. INSTALL SEEDING AND RESTORATION

STAGE III CONSTRUCTION

- 1. RELOCATE TRAFFIC CONTROL AND PROTECTION (SPECIAL) TO MEDIAN AS DETAILED IN PLANS PROVIDING ONE THROUGH LANE IN EACH DIRECTION
- 2. CONSTRUCT MEDIAN

FINAL PAVEMENT MARKING AND CLEANUP

- 1. REMOVE TRAFFIC CONTROL AND PROTECTION (SPECIAL).
- 2. INSTALL FINAL PAVEMENT MARKING UNDER TRAFFIC CONTROL STD. 701426.

	REVISIONS	
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Wheaton, II 60187		

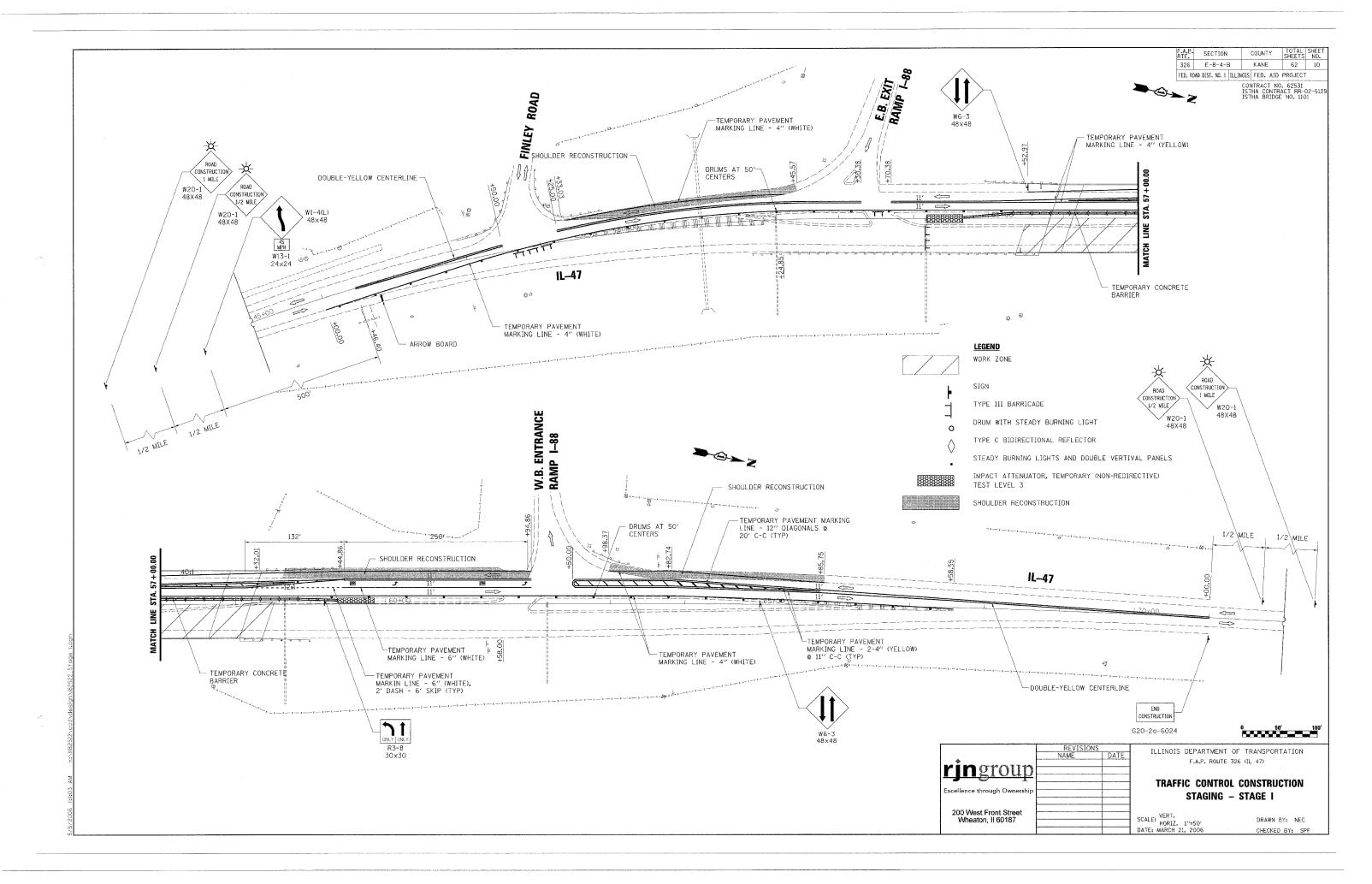
ILLINOIS DEPARTMENT OF TRANSPORTATION F.A.P. ROUTE 326 (IL 47)

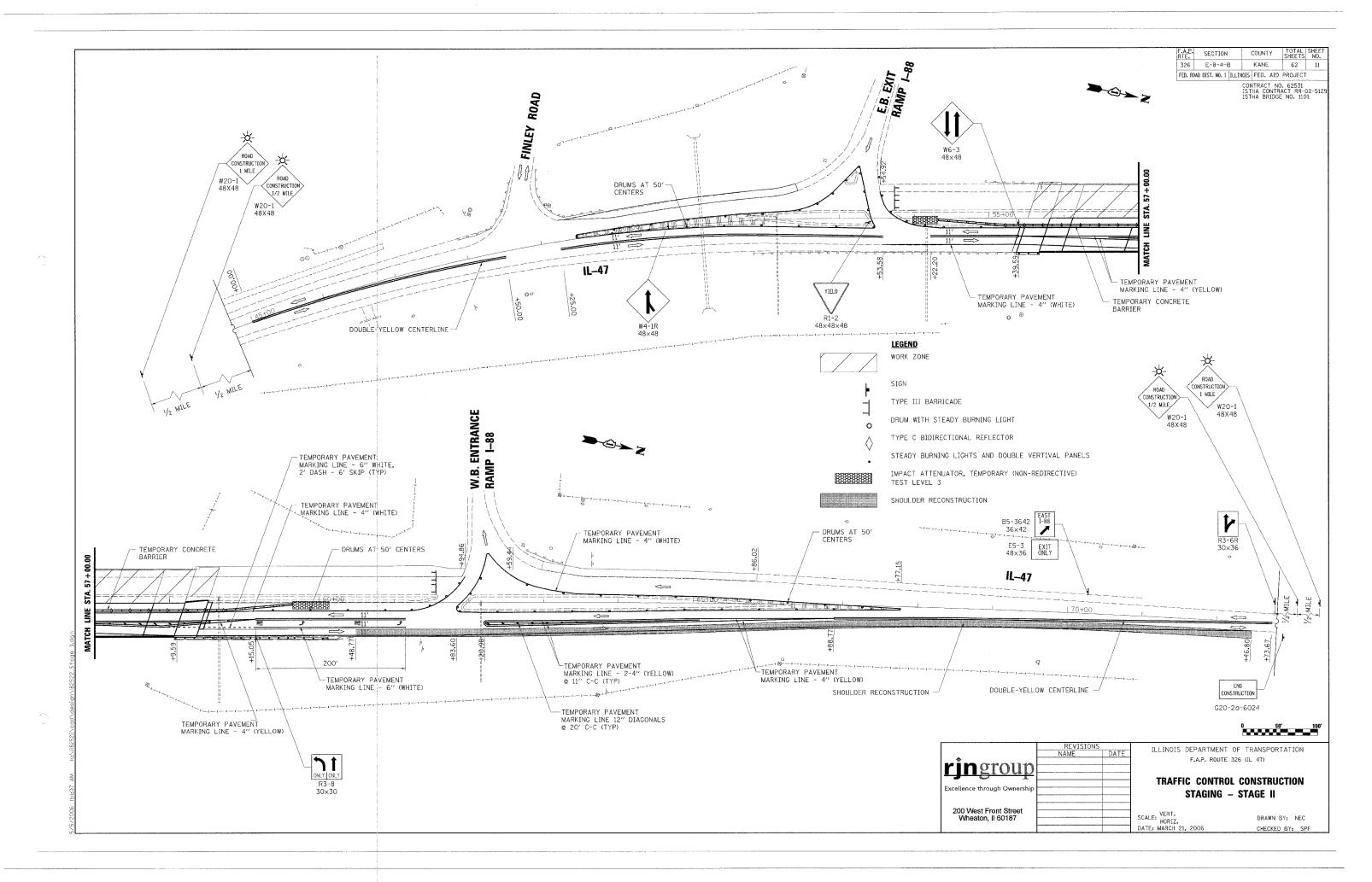
TRAFFIC CONTROL
CONSTRUCTION STAGING
NOTES AND DETAILS

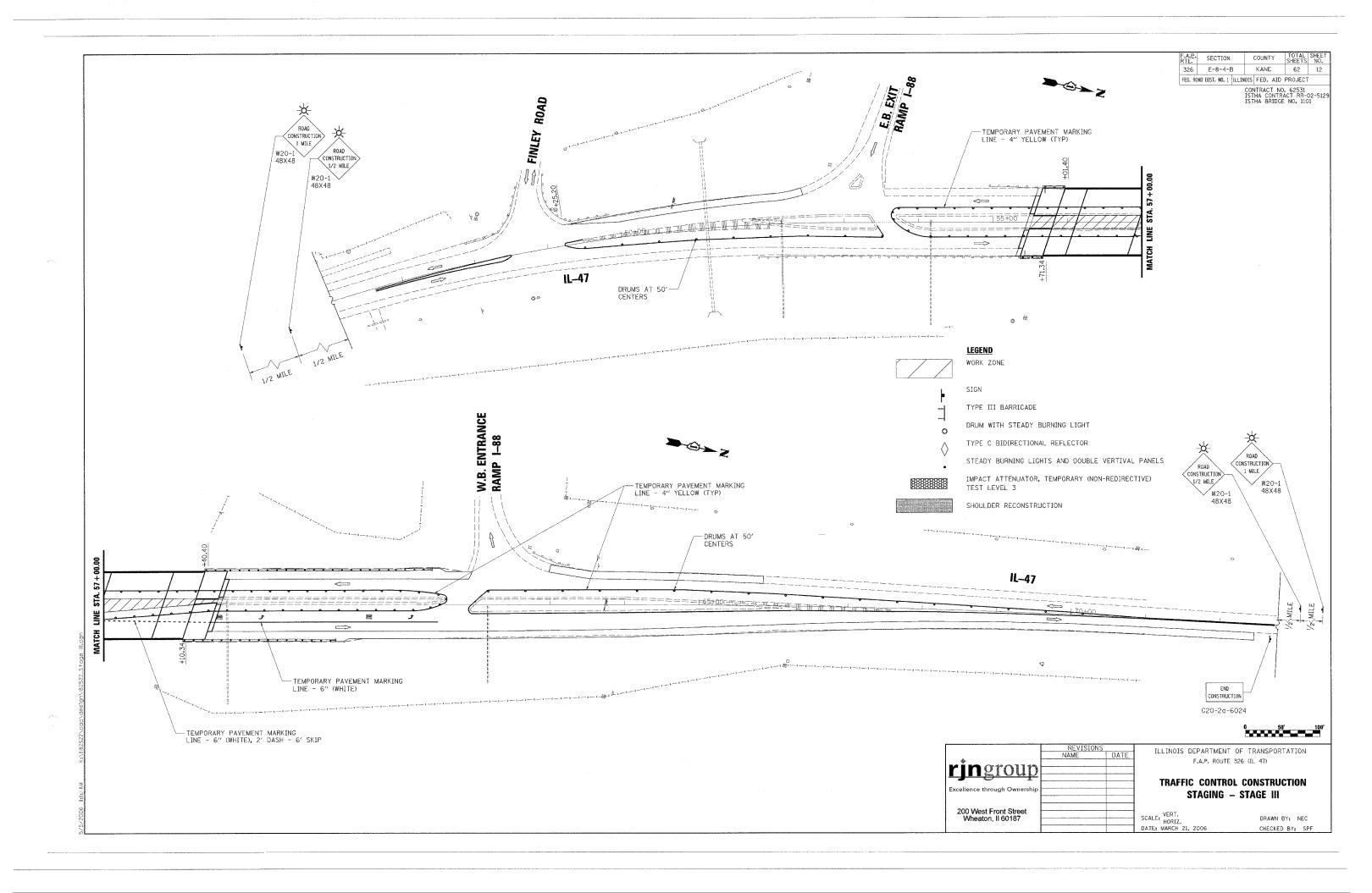
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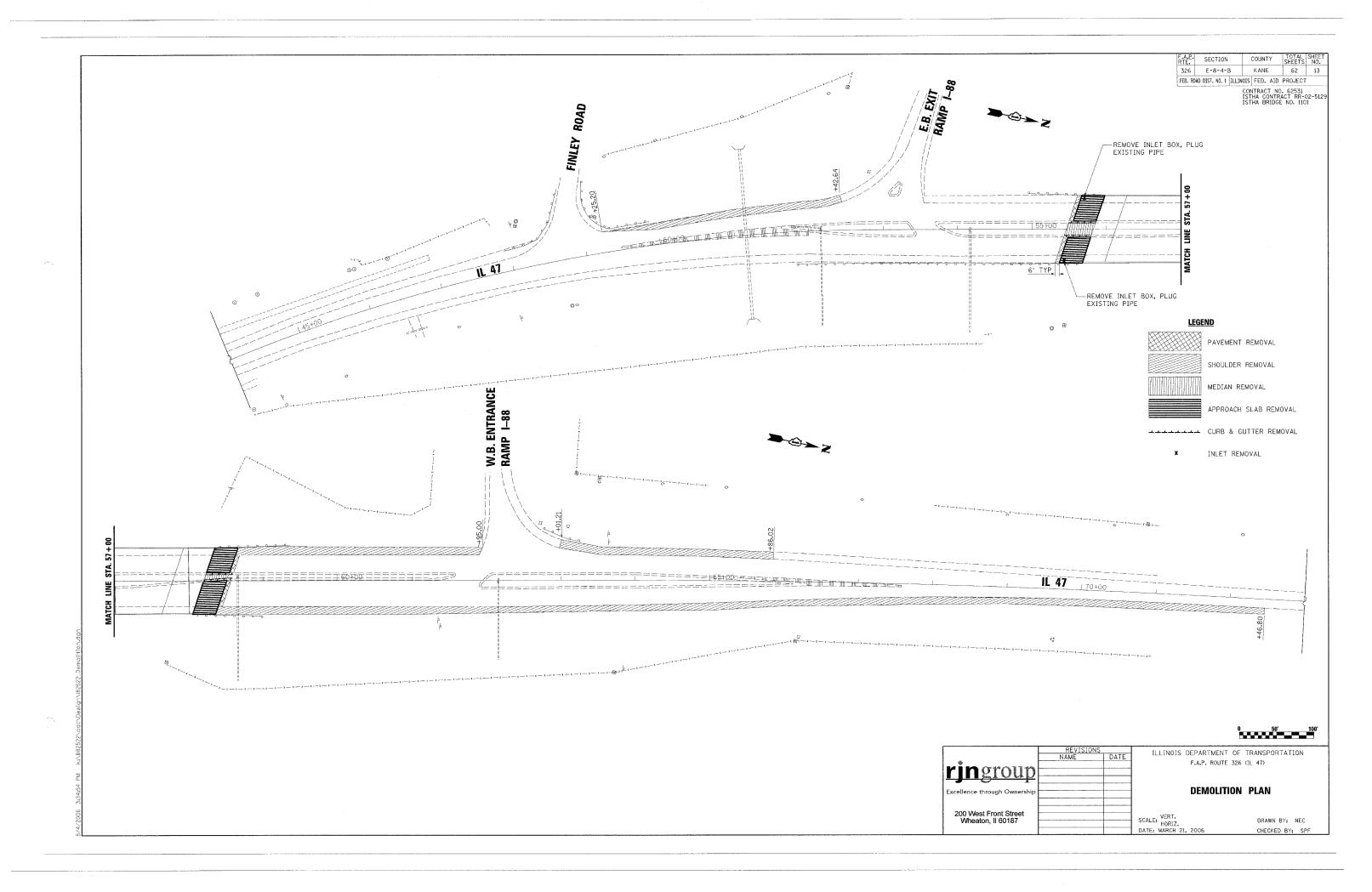
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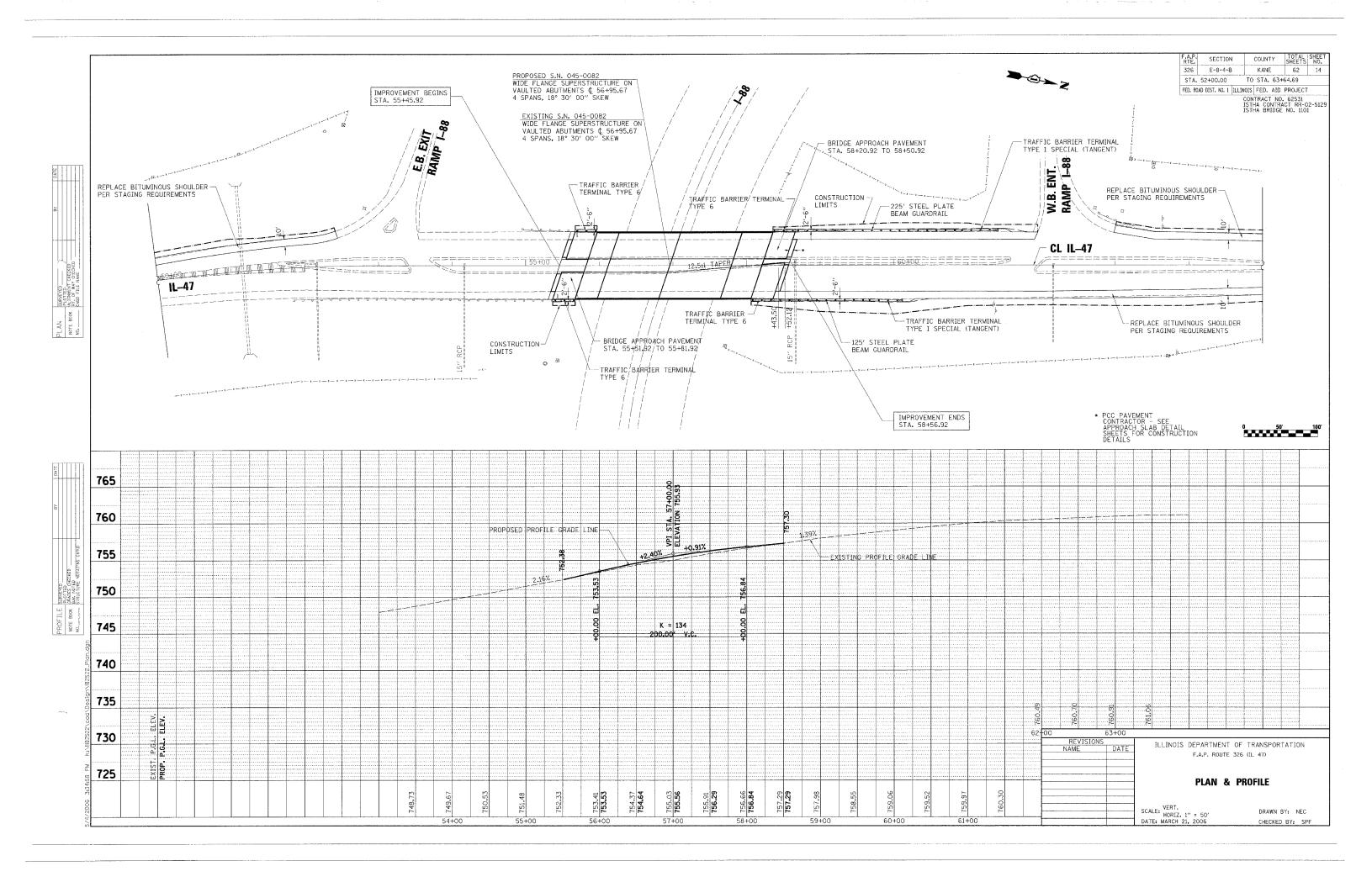
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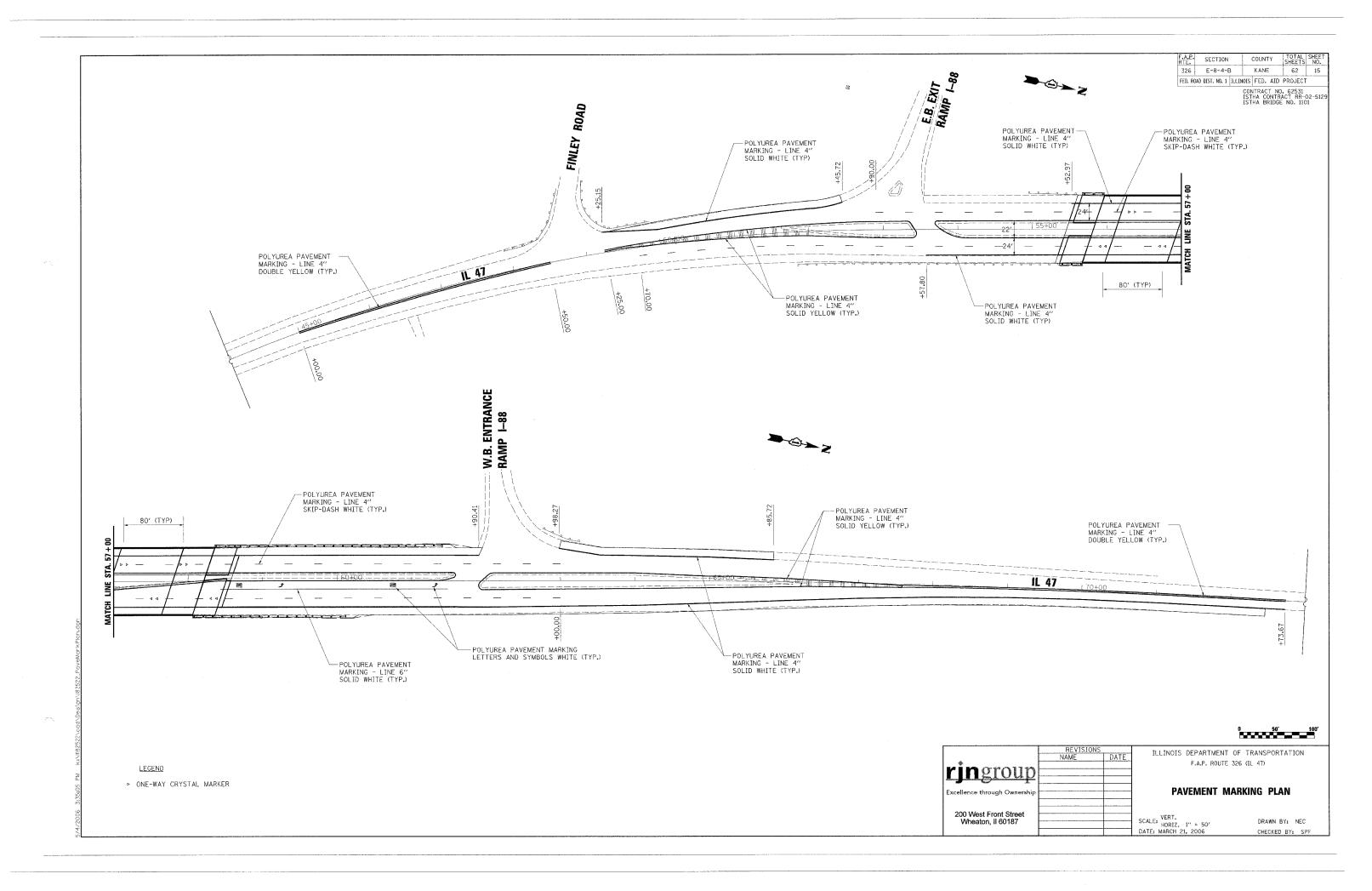


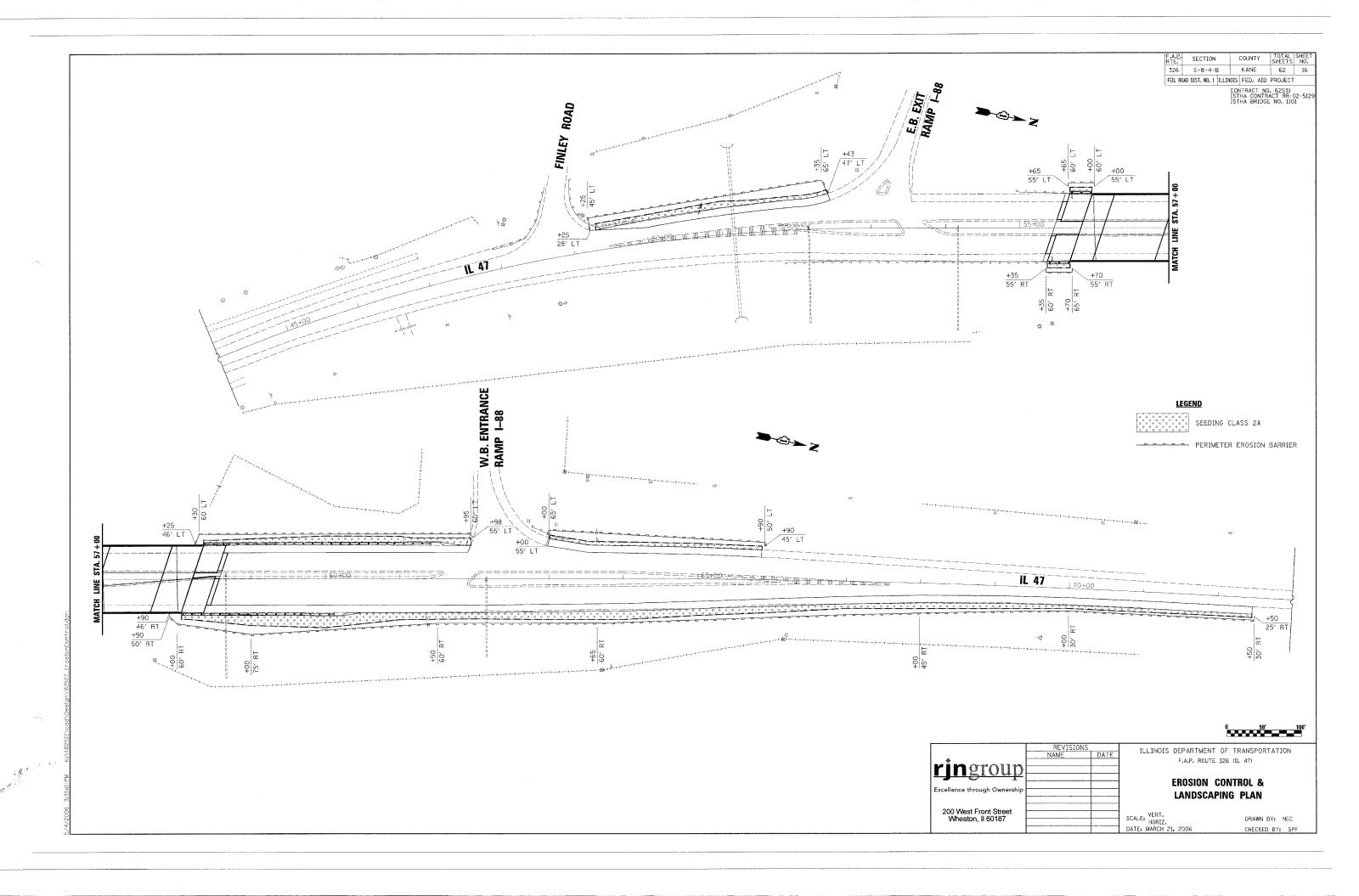


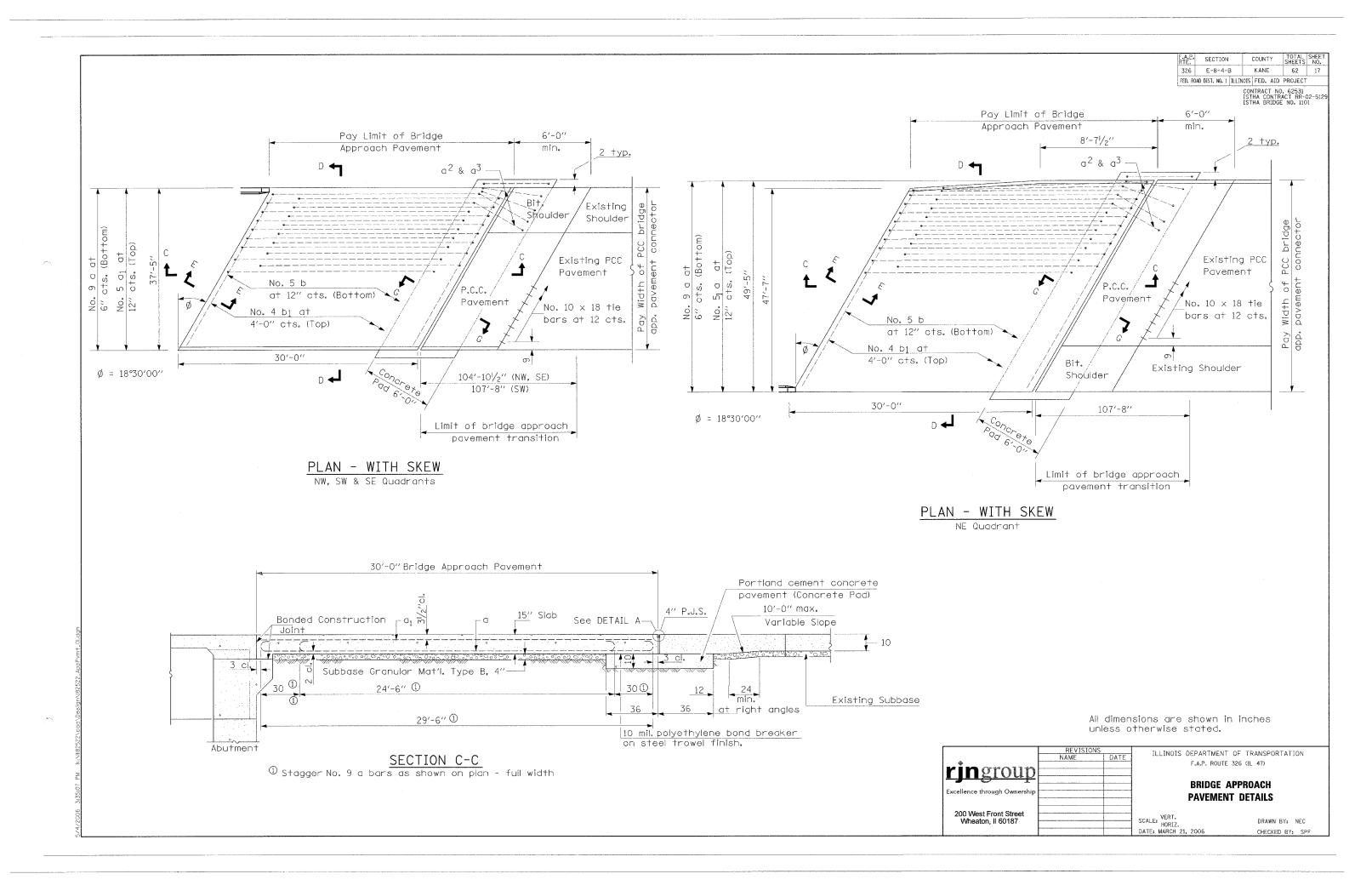


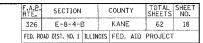




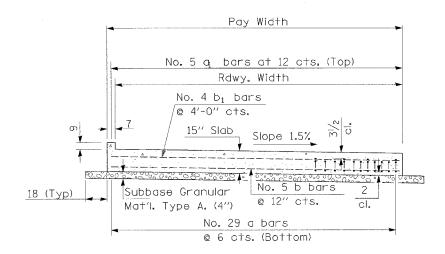






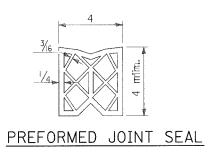


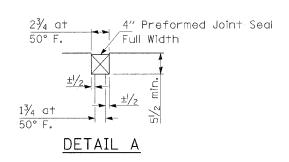
ISTHA CONTRACT RR-02-51 ISTHA BRIDGE NO. 1101

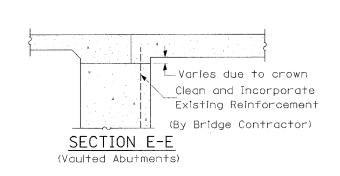


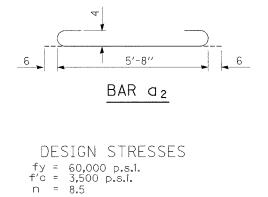
SECTION D-D

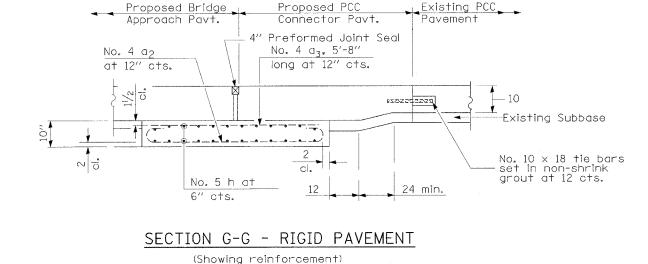
(See Plan for Dimensions not shown)
All reinforcement bars shall be epoxy coated.

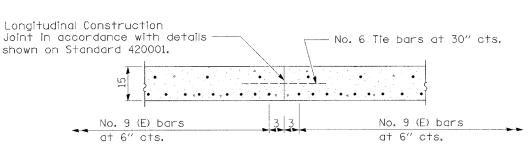






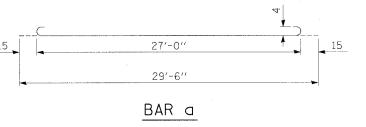






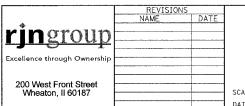
OPTIONAL LONGITUDINAL CONSTRUCTION JOINT

As approved by the Engineer, the Contractor may elect to reduce the widths of pour by use of the Optional Longitudinal Construction Joint shown. Joints shall be located at the edge of a traffic lane.



GENERAL NOTES

All dimensions are shown in inches unless otherwise stated.

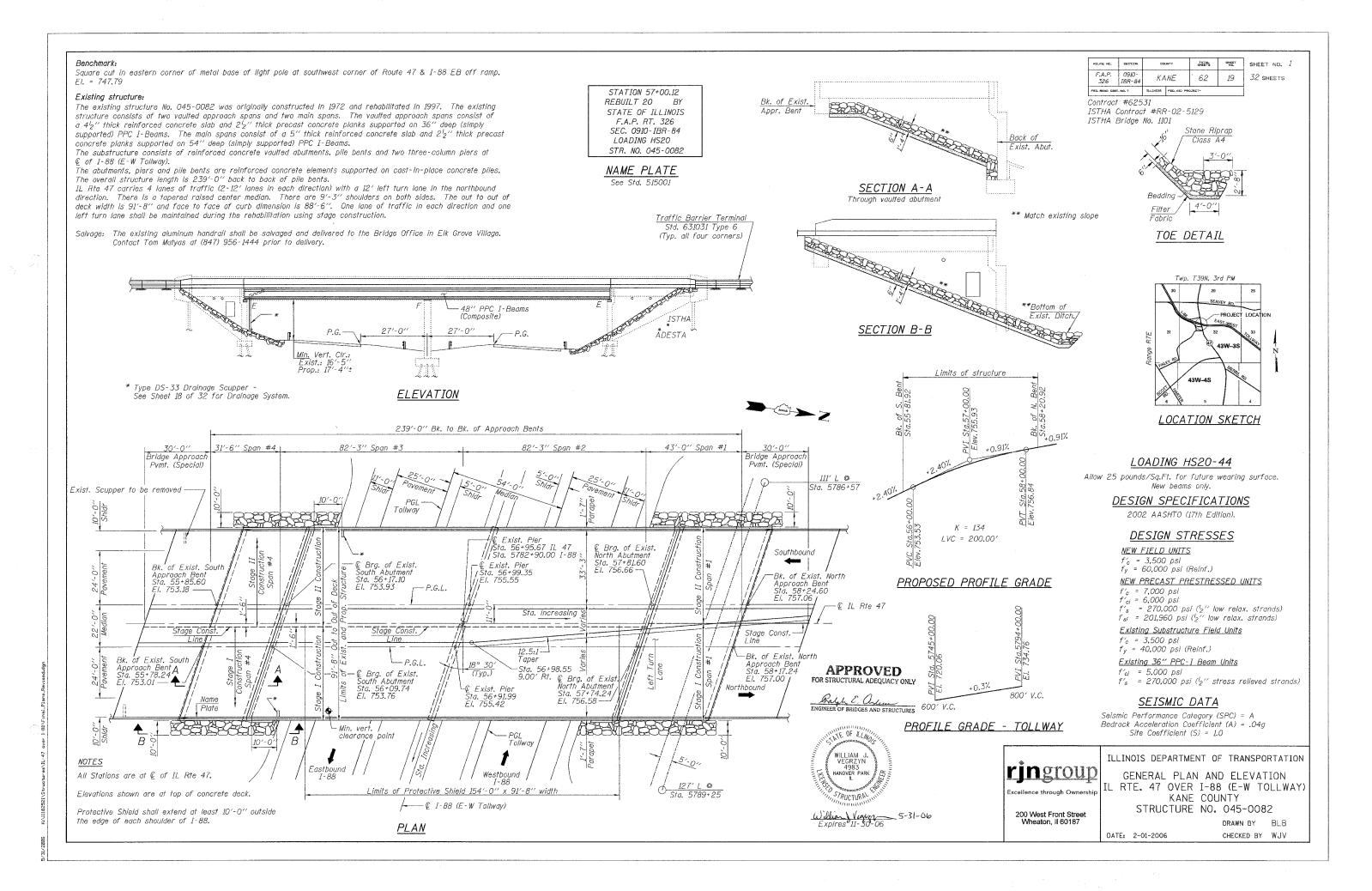


ILLINOIS DEPARTMENT OF TRANSPORTATION

F.A.P. ROUTE 326 (IL 47)

BRIDGE APPROACH PAVEMENT DETAILS

SCALE: VERT. HORIZ. DATE: MARCH 21, 2006



20 32 SHEETS

Contract #62531 ISTHA Contract #RR-02-5129 ISTHA Bridge No. 1101

GENERAL NOTES

The structural steel bearing plates of the Elastomeric Bearing Assembly shall conform to the requirements of AASHTO M 270 Grade 50.

Reinforcement bars shall conform to the requirements of AASHTO M31 or M322 Grade 60.

Plan dimensions and details relative to existing structure have been taken from existing plans and are subject to nominal construction variations. It shall be the Contractor's responsibility to verify such dimensions and details in the field and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in the scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price for the work.

Bridge Seat Sealer shall be applied to the seat area of the abutments.

All Construction joints shall be bonded.

All new structural steel shall be shop painted with an inorganic zinc rich primer per AASHTO M 300, Type 1.

Slope wall shall be reinforced with welded wire fabric, 6" x 6" - W4.0 x W4.0, weighing 58 lbs. per 100 sq. ft.

TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Protective Coat	Sq. Yd.	2551	300	2551
Concrete Removal	Cu. Yd.	2001	18.6	18.6
Removal of Existing Concrete Deck No. 1	Each	1	10.0	10.0
Removal of Existing Concrete Deck No. 2	Each	1		1
Removal of Existing Superstructures	Each	1		1
Neoprene Expansion Joint 2"	Foot	190		190
Concrete Structures	Cu. Yd.	100	34.5	34.5
Concrete Superstructure	Cu. Yd.	734.9	37.0	734.9
Bridge Deck Grooving	Sq. Yd.	1800		1800
Elastomeric Bearing Assembly Type I	Each	24		24
Structural Repair of Concrete (Depth Equal To or Less Than 5")	Sq. Ft.	tan 1	168	168
Epoxy Crack Sealing	Foot		263	263
Furnishing and Erecting Precast Prestressed Concrete I-Beams, 48''	Foot	1984		1984
Bridge Seat Sealer	Sq. Ft.		430	430
Reinforcement Bars, Epoxy Coated	Pound	163,330	2410	165,740
Stone Riprap, Class A4	Sq. Yd.		915	915
Filter Fabric	Sq. Yd.		915	915
Name Plates	Éach	1		1
Bar Splicers	Each	803	4	807
Drainage Scuppers, DS-33	Each	2		2
Protective Shield	Sq. Yd.	3138		3138
Drainage System	L. Sum		1	1
Slope Wall Removal	Sq. Yd.			2
Slope Wall 4 Inch	Sq. Yd.			2
Furnish and Install Door	Each		2	2

*1569 Sq. Yd. of Protective Shield will be furnished and installed for the removal of the concrete deck, then the Protective Shield will be removed and stored while the existing beams are being removed and replaced. Then the 1569 Sq. Yd. of Protective Shield will be reinstalled for the placement of the Concrete Superstructure. The Contractor will be paid for a total of 3138 Sq. Yd. of Protective Shield. This total quantity includes providing 1569 Sq. Yd. of Protective Shield and installing and removing 3138 Sq. Yd. of Protective Shield.

INDEX OF SHEETS

Sheet No.	Description
1 2 3 4 5	General Plan and Elevation General Notes and Total Bill of Material Stage Construction - Bridge Stage Construction - Approach
5	Top of Slab Elevations - I
	Top of Slab Elevations - II
7	Top of Slab Elevations - III
8	Top of Slab Elevations - IV
9	Top of Slab Elevations - V
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12	Vaulted Span Detalls
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15 16	Superstructure Details - II
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21	48" P.P.C. I-Beam
22	48" P.P.C. I-Beam Details
23	Bearing Details
24	Anchor Bolt Details
25	North Abutment Details
26	South Abutment Details
27	North Abutment Repairs
28	South Abutment Repairs
29	Pier Details
30	Pier Repairs
31	Bar Splicer Assembly Details
32	Temporary Concrete Barrier For Stage Construction



200 West Front Street Wheaton, II 60187

ILLINOIS DEPARTMENT OF TRANSPORTATION GENERAL NOTES AND TOTAL BILL OF MATERIAL Excellence through Ownership IL RTE. 47 OVER I-88 (E-W TOLLWAY) KANE COUNTY STRUCTURE NO. 045-0082

DRAWN BY BLB

CHECKED BY WJV

DATE: 2-01-2006

Contract #62531 ISTHA Contract #RR-02-5129 ISTHA Bridge No. 1101

9!'-8" out to out of parapet

1'-7"

44'-3" face of parapet to © of bridge

44'-3" face of parapet to © of bridge

1'-7"

3' II'-0" II'-0" II'-0" II'-0"

SB traffic left turn lane NB traffic

Toe of Temp. Conc.
Std.704001 Barrier
Slope 1.5%

Slope 1.5%

5 spaces © 8'-0" = 40'-0"

5 spaces © 8'-0" = 40'-0"

3'-0" 54" PPC-1 Beams

CROSS SECTION SPAN #2 & 3, STAGE I - REMOVAL

91'-8" out to out of parapet

1'-7"

44'-3" face of parapet to © of bridge

Temp. Concrete
Barrier Std.704001

Stage I - Construction

44'-4"

Slope 1.5%

Slope 1.5%

Slope 1.5%

1'-6"

48" PPC-I

Beams (Typ.)

CROSS SECTION SPAN #2 & 3, STAGE I - CONSTRUCTION
(Looking North)

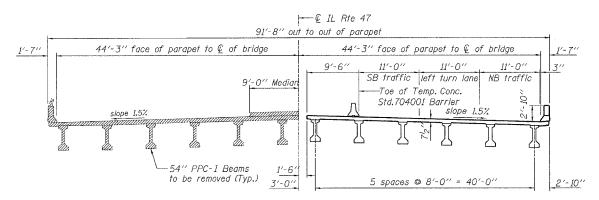
LEGEND:

Superstructure Removal

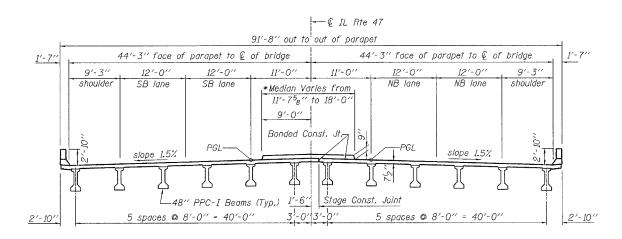
* Median to be constructed after completion of Stage II - Construction.

NOTE:

Removal of the existing bridge rail is included with Removal of Existing Superstructures.



CROSS SECTION SPAN #2 & 3, STAGE II - REMOVAL



CROSS SECTION SPAN #2 & 3, STAGE II - CONSTRUCTION
(Looking North)



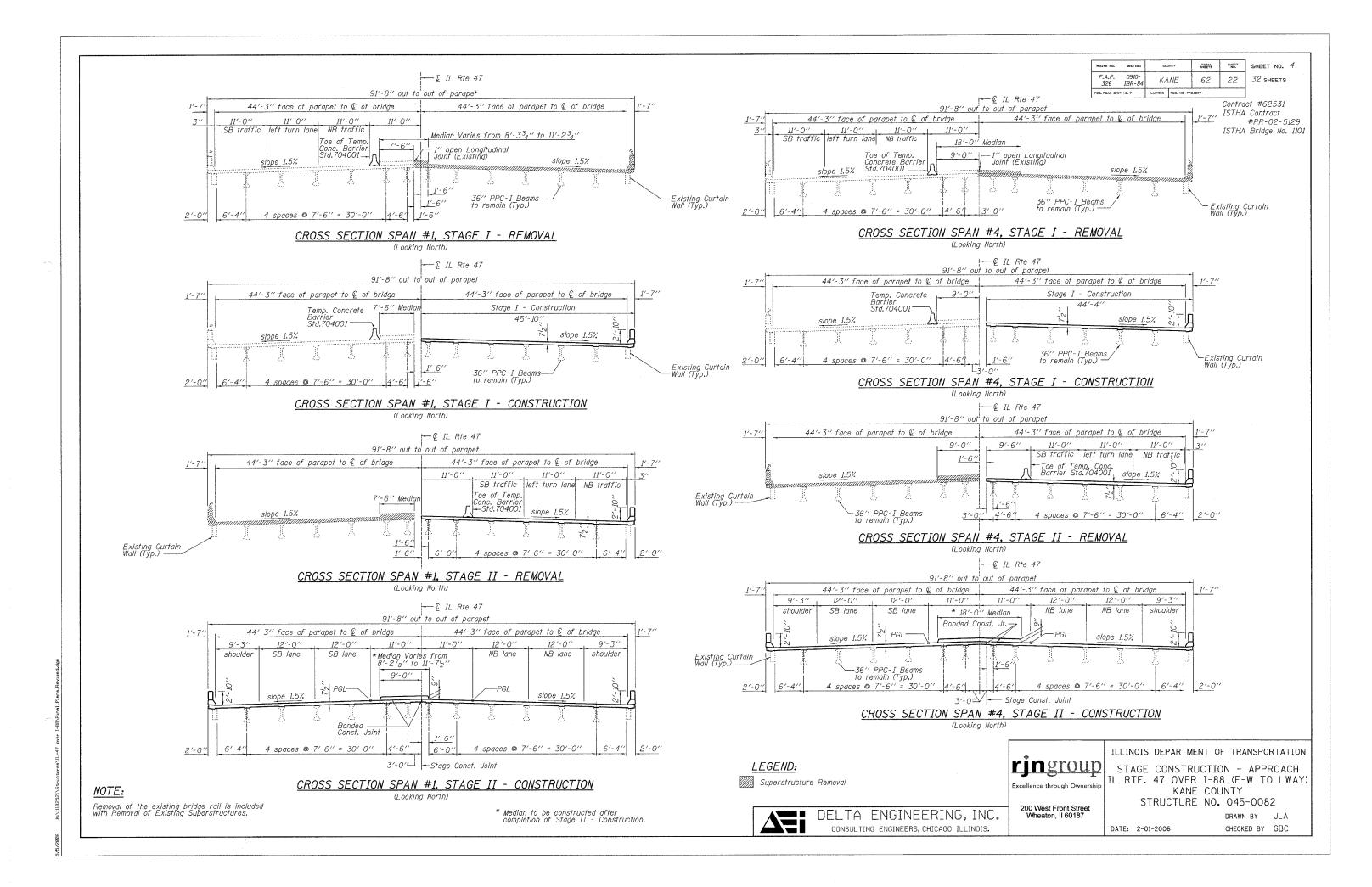
200 West Front Street Wheaton, II 60187

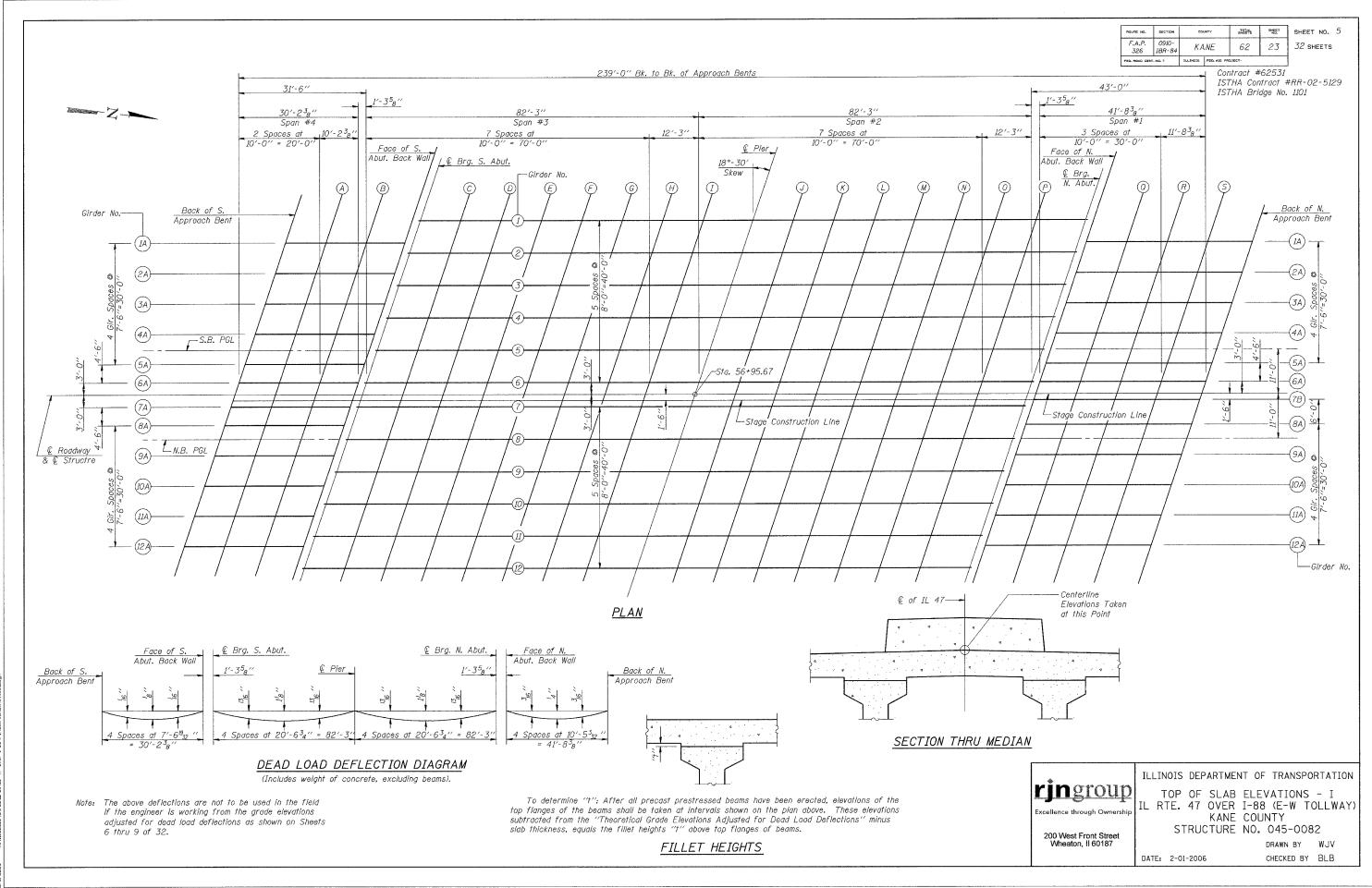
ILLINOIS DEPARTMENT OF TRANSPORTATION

STAGE CONSTRUCTION - BRIDGE
IL RTE. 47 OVER I-88 (E-W TOLLWAY)
KANE COUNTY
STRUCTURE NO. 045-0082

DRAWN BY JLA
DATE: 2-01-2006 CHECKED BY GBC

DELTA ENGINEERING, INC. consulting engineers, chicago illinois.





X-VIISOESINStructures VII -47 over I-88/Erral Plans

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ROUTE NO.	SECTION	COUNTY		TOTAL	SHEET NO.	S
F.A.P. 326	0910- IBR-84	KANE		62	24	٠.,
FEG. ROAD DIST	NO. 7	ILLBOIS	FED. AID PRO	DIECT-		

SHEET NO. 6 32 sheets

Contract #62531 ISTHA Contract #RR-02-5129 ISTHA Bridge No. 1101 GIRDER 4A

	GIRDER	1A
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Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
55+94.47	- 37.50	753.000	753.000
56+04.47 56+14.47	- 37.50 - 37.50	753.239 753.472	753.247 753.480
56+24.67	- 37.50	753.702	753.702
57+91.77	- 37.50	756.365	756.365
58+01.77 58+11.77 58+21.77	- 37.50 - 37.50 - 37.50	756.458 756.549 756.640	756.474 756.570 756.657
58+33.47	- 37.50	756.747	756.747
	55+94.47 56+04.47 56+14.47 56+24.67 57+91.77 58+01.77 58+11.77 58+21.77	55+94.47 -37.50 56+04.47 -37.50 56+14.47 -37.50 56+24.67 -37.50 57+91.77 -37.50 58+01.77 -37.50 58+11.77 -37.50 58+21.77 -37.50	Station Offset Grade Elevations 55+94.47 -37.50 753.000 56+04.47 -37.50 753.239 56+14.47 -37.50 753.472 56+24.67 -37.50 753.702 57+91.77 -37.50 756.365 58+01.77 -37.50 756.458 58+11.77 -37.50 756.549 58+21.77 -37.50 756.640

GIRDER 2A

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection			
Back of South Approach Bent	55+91.96	- 30.00	753,052	753.052			
A B Face of South	56+01.96 56+11 . 96	- 30.00 - 30.00	753.292 753.526	753.300 753.535			
Abut. Back Wall	56+22.16	-30.00	753.758	753 . 758			
Face of North Abut, Back Wall	57+89,26	-30.00	756.453	756.453			
Q R S Back of North	57+99.26 58+09.26 58+19.26	- 30.00 - 30.00 - 30.00	756.548 756.639 756.730	756.563 756.660 756.747			
Approach Bent	58+30.96	- 30.00	756.837	756.837			

GIRDER 3A

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection	
Back of South Approach Bent	55+89.45	-22.50	753.104	753.104	
A B Face of South	55+99.45 56+09.45	-22.50 -22.50	753.344 753.581	753.352 753.589	
Abut. Back Wall	56+19.65	-22.50	753.815	753.815	
Face of North Abut. Back Wall	57+86.75	-22.50	756.540	756.540	
Q R S Back of North Approach Bent	57+96.75 58+06.75 58+16.75 58+28.45	-22.50 -22.50 -22.50 -22.50	756.637 756.729 756.820 756.926	756.653 756.750 756.837 756.926	
	l	L	L	l	

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Back of South Approach Bent	55+86.94	- 15.00	753 . 156	753.156
A	55+96.94	- 15.00	753 . 396	753.404
В	56+06.94	- 15.00	753.635	753.643
Face of South				
Abut, Back Wall	56+17.14	- 15.00	753.870	753.870
Face of North Abut, Back Wall	57+84.24	- 15.00	756.627	756.627
0	57+94.24	- 15.00	756,726	756,742
R	58+04.24	- 15.00	756.819	756.839
S	58+14.24	- 15.00	756.910	756.926
Back of North	ļ			
Approach Bent	58+25.94	- 15.00	757.016	757.0 <i>1</i> 6
	l	L		

S.B. PGL

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Back of South Approach Bent	55+85.60	- 11.00	753.184	753.184
A B	55+95.60 56+05.60	- 11.00 - 11.00	753.424 753.663	753.432 753.671
Face of South Abut. Back Wall	56+15.80	- 11.00	753.900	753.900
Face of North Abut, Back Wall	57+82.90	-11.00	756,673	756.673
Q R S Back of North	57+92.90 58+02.90 58+12.90	- 11.00 - 11.00 - 11.00	756.773 756.866 756.957	756,789 756.887 756.974
Approach Bent	58+24.60	- 11.00	757.064	757.064

GIRDER 5A

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Back of South Approach Bent	55+84.43	- 7.50	753.209	753.209
A B	55+94.43 56+04.43	- 7.50 - 7.50	753 . 449 753.688	753.457 753.696
Face of South Abut. Back Wall	56+14.63	- 7.50	753.926	753.926
Face of North Abut, Back Wall	57+81.73	-7.50	756.714	756.714
Q R S Back of North	57+91.73 58+01.73 58+11.73	- 7.50 - 7.50 - 7.50	756.815 756.908 756.999	756,830 756,929 757,016
Approach Bent	58+23.43	-7.50	757.106	757.106

GIRDER 6A

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Back of South Approach Bent	55+82.92	- 3.00	753.240	753.240
A	55+92 . 92 56+02 . 92	- 3.00 - 3.00	753,480 753,720	753.488 753.728
Face of South	30,02,32	-3,00	755,720	133.120
Abut. Back Wall	56+13.12	- 3.00	753 . 958	753.958
Face of North Abut, Back Wall	57+80.23	- 3.00	756.765	756.765
Q	57+90.23	- 3.00	756.867	756.883
R	58+00.23	- 3.00	756.962	756.983
S	58+10.23	- 3.00	757.053	757.070
Back of North Approach Bent	58+21.92	- 3.00	757 . 159	757 . 159

② ROADWAY & STRUCTURE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Back of South Approach Bent	55+81.92	0.00	753.261	753.261
Α	55+91.92	0.00	753.501	753.509
B Face of South	56+01.92	0.00	753,741	753.749
Abut. Back Wall	56+12.12	0.00	753,980	753.980
Face of North Abut, Back Wall	57+79.22	0.00	756.800	756.800
Q	57+89,22	0.00	756.903	756.918
R	57+99.22	0.00	756.998	757.019
S	58+09.22	0.00	757.089	757.106
Back of North				
Approach Bent	58+20.92	0.00	757.195	757.195

SPANS #1 AND #4



TOP OF SLAB ELEVATIONS - II
IL RTE. 47 OVER I-88 (E-W TOLLWAY)
KANE COUNTY STRUCTURE NO. 045-0082

ILLINOIS DEPARTMENT OF TRANSPORTATION

DRAWN BY WJV DATE: 2-01-2006 CHECKED BY BLB

ROUTE NO.	SECTION	COUNTY		TOTAL SHEETS	SHEET NO.	s
F.A.P. 326	0910- IBR-84	KANE		62	25	17.)
FED. ROAD DIST. NO. 7		1LL194018	FED. ALD PRO	OJECT-		

SHEET NO. 7 32 sheets

Contract #62531 ISTHA Contract #RR-02-5129 ISTHA Bridge No. 1101 N.B. PGL

STAGE CONSTRUCTION LINE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Back of South Approach Bent	55+81.42	1.50	753,227	753.227
A	55+91.42	1.50	753,467	753.474
В	56+01.42	1.50	753 . 706	753.714
Face of South Abut. Back Wall	56+11.62	1.50	753.946	753.946
Face of North Abut. Back Wall	57+79.22	0.00	756.800	756.800
Q	57+89.22	0.00	756.903	756.918
R	57+99.22	0.00	756.998	757 . 019
S	58+09.22	0.00	757.089	757.106
Back of North Approach Bent	58+20.92	0.00	757.195	757.195

GIRDER 7A & 7B

Location	Station	Offset	Theoretical Grade Elevations	Theorefical Grade Elevations Adjusted For Dead Load Deflection
Back of South Approach Bent	55+80,92	3,00	753.192	753 . 192
A	55+90.92	3.00	753.432	753,440
В	56+00.92	3.00	753,672	753.680
Face of South				
Abut. Back Wall	56+11.11	3.00	753.912	753.912
Face of North	57+78.72	1.50	756.772	756.772
Abut. Back Wall				
a	57+88.72	1.50	756 . 875	756.890
R	57+98.72	1.50	756,971	756,992
S	58+08.72	1.50	757.062	757.079
Back of North				
Approach Bent	58+20.42	1.50	757 . 168	757.168

GIRDER 8A						
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection		
Back of South Approach Bent	55+79.41	7.50	753.088	753.088		
A B Face of South	55+89.41 55+99.41	7.50 7.50	753 . 328 753,568	753.336 753.576		
Abut. Back Wall	56+09.61	7.50	753.810	753.810		
Face of North Abut, Back Wall	57+76.71	7.50	756 . 660	756.660		
Q R S Back of North	57+86.71 57+96.71 58+06.71	7.50 7.50 7.50	756.765 756.862 756.954	756.780 756.883 756.970		
Approach Bent	58+18.41	7.50	757.060	757.060		

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Back of South Approach Bent	55+78.24	11.00	753.008	753.008
Α	55+88.24	11.00	753.248	753.256
В	55+98.24	11.00	753.488	753.496
Face of South				
Abut. Back Wall	56+08.44	11,00	753.730	753.730
Face of North Abut. Back Wall	57+75.54	11.00	756.595	756.595
Q	57+85,54	11,00	756,701	756.7 <i>1</i> 6
R	57+95,54	11.00	756.799	756.8 <i>1</i> 9
S	58+05.54	11.00	756.890	756.907
Back of North			1	
Approach Bent	58+17.24	11.00	756.997	756 . 997

GIRDER 9A

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Back of South Approach Bent	55+76.90	15.00	752.916	752.916
Approach Bolli A B	55+86.90 55+96.90	15.00 15.00	753.156 753.396	753.163 753.404
Face of South Abut, Back Wall	56+07.10	15.00	753 . 638	753.638
Face of North Abut, Back Wall	57+74.20	15.00	756.520	756.520
Q R	57+84.20 57+94.20	15.00 15.00	756.627 756.726	756.642 756.747
S Back of North	58+04.20	15.00	756 . 818	756.835
Approach Bent	58+15.90	15.00	756.925	756.925

GIRDER 10A

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Back of South Approach Bent	55+74.39	22.50	752.743	752.743
A B Face of South	55+84.39 55+94.39	22.50 22.50	752.983 753.223	752.991 753.231
Abut. Back Wall	56+04.59	22.50	753,467	753.467
Face of North Abut, Back Wall	57+71.69	22.50	756.380	756.380
Q R S Back of North	57+81.69 57+91.69 58+01.69	22.50 22.50 22.50	756.488 756.589 756.683	756.504 756.610 756.700
Approach Bent	58+13.39	22.50	756.789	756.789

GIRDER 11A

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Back of South Approach Bent	55+71.88	30.00	752.570	752.570
A B Face of South	55+81.88 55+91.88	30.00 30.00	752.810 753.050	752.818 753.058
Abut. Back Wall	56+02.08	30.00	753.295	753.295
Face of North Abut, Back Wall	57+69 .18	30.00	756 . 239	756.2 <i>3</i> 9
Q R	57+79.18 57+89.18 57+99.18	30.00 30.00 30.00	756.349 756.452 756.547	756.365 756.473 756.564
Back of North Approach Bent	58+10.88	30.00	756.654	756.654

GIRDER 12A

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Back of South Approach Bent	55+69.37	37.50	752.397	752.397
A	55+79.37	37.50	752.637	752,645
В	55+89.37	37.50	752.877	752,885
Face of South			'	
Abut. Back Wall	55+99.57	37.50	753,122	753.122
	1			
Face of North	57+66.68	37.50	756.098	756.098
Abut, Back Wall	1			
a	57+76.68	37.50	756,210	756.225
R	57+86.68	37.50	756.314	756.335
S	57+96.68	37.50	756,412	756.428
Back of North				
Approach Bent	58+08.37	37.50	756,518	756.518
, ,				

SPANS #1 AND #4



ILLINOIS DEPARTMENT OF TRANSPORTATION TOP OF SLAB ELEVATIONS - III IL RTE. 47 OVER I-88 (E-W TOLLWAY) KANE COUNTY STRUCTURE NO. 045-0082

DRAWN BY WJV DATE: 2-01-2006 CHECKED BY BLB

ROUTE NO.	SECTION	COUNTY		TOTAL SHEETS	SHEET NO.	s
F.A.P. 326	0910- IBR-84	KANE		62	26	. 17.)
FED. ROAD DIST	D. ROAD DIST. NO. 7 ILLINOIS FED. AID PROJECT-					

SHEET NO. 8 32 sheets

Contrac	+ #62531	
ISTHA	Contract #RR-02-5129	
ISTHA	Bridge No. 1101	
	0 * C C C C	

 Bridge No. 110.	1
	- 7

GI	RDER 1	!			
)	Offset	Theoretical Grade	Theoretical Grade Elevations Adjusted	Location	

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
© Bearing S. Abutment C D E F G H I © Pier J K L M N	56+27.81 56+37.81 56+47.81 56+57.81 56+67.81 56+97.81 57+10.06 57+30.06 57+40.06 57+50.06 57+50.06 57+70.06	-43.00 -43.00 -43.00 -43.00 -43.00 -43.00 -43.00 -43.00 -43.00 -43.00 -43.00 -43.00 -43.00	753.688 753.904 754.112 754.313 754.506 754.692 754.870 755.041 755.240 755.394 755.5812 755.812 755.937 756.054	Deflection 753.688 753.939 754.177 754.390 754.600 754.780 755.082 755.240 755.428 755.5605 755.766 755.907 756.026 756.125
P © Bearing	57+80.06	- 43.00	756 . 163	756.206
N. Abutment	57+92.31	-43.00	756,288	756.288

GIRDER 2					
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection	
© Bearing S. Abutment	56+25.13	- 35.00	753.749	753.749	
C D E F G H I V Pier J K L M N O P	56+35.13 56+45.13 56+55.13 56+65.13 56+85.13 56+95.13 57+07.38 57+27.38 57+47.38 57+57.38 57+57.38 57+57.38	-35.00 -35.00 -35.00 -35.00 -35.00 -35.00 -35.00 -35.00 -35.00 -35.00 -35.00 -35.00 -35.00	753.967 754.177 754.380 754.575 754.763 754.943 755.116 755.317 755.474 755.623 755.764 755.898 756.024 756.143 756.255	754.002 754.242 754.466 754.669 754.851 755.013 755.157 755.507 755.507 755.686 755.849 755.992 756.114 756.214	
© Bearing N. Abutment	57+89.63	- 35.00	756.381	756.381	

GIRDER 3					
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection	
© Bearing S. Abutment C D E F G H I © Pier J K	56+22.45 56+32.45 56+42.45 56+52.45 56+62.45 56+82.45 56+82.45 56+92.45 57+04.70 57+24.70	-27.00 -27.00 -27.00 -27.00 -27.00 -27.00 -27.00 -27.00 -27.00 -27.00	753.810 754.030 754.242 754.446 754.643 754.833 755.015 755.190 755.394 755.553 755.703	753.810 754.064 754.307 754.532 754.738 754.922 755.085 755.231 755.394 755.586 755.767	
L M N O P @ Bearing N. Abutment	57+34.70 57+44.70 57+54.70 57+64.70 57+74.70	-27.00 -27.00 -27.00 -27.00 -27.00 -27.00	755.847 755.983 756.111 756.232 756.346	755.932 756.077 756.200 756.303 756.388	

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
© Bearing S. Abutment	56+19.78	- 19.00	753.870	753.870
C	56+29.78	- 19.00	754,092	754,126
D	56+39,78	- 19.00	754,306	754,371
Ē	56+49.78	- 19.00	754.512	754.598
F	56+59.78	- 19 . 00	754,711	754.806
G	56+69.78	- 19.00	754.903	754.992
Н	56+79.78	- 19.00	755.087	755.157
I	56+89.78	- 19.00	755.264	755.305
€ Pier	57+02.03	- 19.00	755.471	755,471
J	57+12.03	- 19.00	755 . 631	755.665
K	57+22.03	- 19.00	755.784	755.848
L	57+32.03	- <i>19.00</i>	755.929	756.015
M	57+42.03	- <i>1</i> 9.00	756.067	756.161
N	57+52.03	- 19.00	756,198	756.287
0	57+62.03	- 19.00	756.321	756.392
P	57+72.03	- 19.00	756.436	756.479
N. Abutment	57+84.28	- 19.00	756.568	756.568
	I			

GIRDER 5 & S.B. PGL

	Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
	© Bearing S. Abutment	56+17.10	- 11.00	753.929	753.929
	C . ADG////O///	56+27.10	- 11.00	754.153	754.188
ļ	D	56+37.10	- 11.00	754.369	754.434
	Ε	56+47.10	- 11.00	754.578	754.664
į	F	56+57.10	- 11.00	754.779	754.873
	G	56+67.10	- 11.00	754.973	755.061
	Н	56+77.10	- 11.00	755 . 159	755.229
	I	56+87.10	- 11.00	755.338	755.379
	ℚ Pier	56+99.35	- 11.00	755.547	755.547
	J	57+09.35	- 11.00	755.709	755.743
	K	<i>57+19.35</i>	- 11.00	755.864	755.928
-	L	57+29,35	- 11.00	756.011	756.096
	M	57+39.35	- 11.00	756.151	756.245
	N	57+49.35	- 11.00	756.283	756.373
	0	57+59.35	- 11.00	756.408	756.480
	© Pogring	57+69.35	- 11.00	756.526	756.568
	© Bearing N. Abutment	57+81.60	- 11,00	756,660	756,660
	N. ADUIMENI	3/ +01,00	- 11.00	750,000	/30.000
	l	Į.	I	I	I

GIRDER 6

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
	56+14.42	- 3.00	753.988	753.988
С	56+24.42	- 3.00	754.214	754.249
D	56+34.42	- 3.00	75 4.4 32	754.497
Ε	56+44.42	- 3.00	754.643	754.729
F	56+54.42	- 3.00	754 . 846	75 4. 940
G	56+64.42	- 3.00	755,042	755.130
Н	56+74.42	-3.00	755 . 230	755.300
I	56+84.42	-3.00	755 . 411	755.452
Pier	56+96.67	- 3.00	755.622	755.622
J	57+06.67	- 3.00	755.786	755.820
K	<i>57+16.67</i>	- 3.00	755,943	756.007
L	57+26.67	- 3.00	756.092	756.178
M	57+36.67	- 3.00	756.234	756.329
N	57+46.67	- 3.00	756.369	756,458
0	57+56.67	- 3.00	756,496	756.567
P	57+66.67	- 3.00	756.615	756.658
€ Bearing				
N. Abutment	57+78.92	- 3.00	756 . 752	756.752

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
© Bearing S. Abutment	56+13.42	0.00	754.010	754.010
C	56+23,42	0.00	754.237	754,272
D	56+33.42	0.00	754 . 455	754.521
E	56+43.42	0.00	754.667	754,753
F	56+53.42	0.00	754.871	754 . 965
G	56+63.42	0,00	755.067	755 . 156
Н	56+73.42	0.00	755.256	755.326
I	56+83.42	0.00	755 . 438	755.479
© Pier	56+95.67	0.00	755.650	755.650
J	57+05.67	0.00	755.815	755.849
K	<i>57+15.67</i>	0.00	755.973	756.037
L	57+25.67	0.00	756.123	756.208
М	57+35.67	0.00	756.265	756,360
Ν	57+45.67	0.00	756.401	756.490
0	57+55.67	0.00	756.528	756.599
P	57+65.67	0.00	756.649	756.691
© Bearing				
N. Abutment	57+77.92	0.00	756.786	756.786

STAGE CONSTRUCTION LINE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
€ Bearing S. Abutment C D	56+12.92 56+22.92 56+32.92	1.50 1.50 1.50	753.976 754.203 754.422	753.976 754.238 754.488
E F G H	56+42.92 56+52.92 56+62.92 56+72.92	1.50 1.50 1.50 1.50	754.634 754.838 755.035 755.224	754.720 754.933 755.124 755.294
I © Pier J K	56+82.92 56+95.17 57+05.17 57+15.17	1.50 1.50 1.50 1.50	755.406 755.619 755.785 755.942	755.447 755.619 755.818 756,006
L M N O	57+25.17 57+35.17 57+45.17 57+55.17	1.50 1.50 1.50 1.50	756.093 756.236 756.372 756.500	756.178 756.330 756.461 756.571
P © Bearing N. Abutment	57+65.17 57+77.42	1.50 1.50	756.620 756.758	756.663 756.758

SPANS #2 AND #3



ILLINOIS DEPARTMENT OF TRANSPORTATION

TOP OF SLAB ELEVATIONS - IV
IL RTE. 47 OVER I-88 (E-W TOLLWAY)
KANE COUNTY STRUCTURE NO. 045-0082

200 West Front Street Wheaton, II 60187

DATE: 2-01-2006

DRAWN BY WJV CHECKED BY BLB Offset

3.00

3.00

3.00

3.00

3.00

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Station

56+12.42

56+22.42

56+32.42

56+42.42

56+52.42

56+62,42

56+72.42

56+82.42

56+94.67

57+04.67

57+14.67

57+24.67

57+34.67

57+44,67

57+54.67

57+64.67

57+76.92

Location

Bearing

S. Abutment

₽ Pier

N. Abutment

Theoretical

Grade

Elevations

753.942

754.169 754.389

754.601 754.806 755.003 755.193 755.375 755.588 755.754

755.912

756.063 756.206 756.342

756.471

756.592

756.730

heoretical Grade

Location

© Bearing

S. Abutment

D

G

⊈ Pier

Ν

levations Adjusted

For Dead Load

Deflection

753.942

754.204

754.454

754.687

754.900

755,091

755.262

755.416

755.588

755.788

755.976

756.148

756,301

756.432

756.542

756.634

756.730

GIRDER	8	ጴ	N.R.	PGI
ULINDLIN	0	O.	14.0.	1 01

Offset

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11.00 11.00 11.00

11.00 11.00

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11.00

11.00

11.00

Station

56+09.74

56+19.74

56+29.74 56+39.74 56+49.74

56+59.74

56+69.74 56+79.74

56+91,99

57+01.99

57+11.99

57+21.99

57+31.99

57+41.99

57+51.99 57+61.99

57+74.24

Theoretical

Grade

Elevations

753.760

753,989

754.211

754.425

754.632

754.831

755.023

755.207 755.422

755.590

755.751

755.903 756.049

756.187

756,317

756,440

756,581

Theoretical Grade

Elevations Adjusted

For Dead Load

Deflection

753.760

754.024 754.276

754,511

754.726 754.919

755.092

755.248 755.422

755.624

755.814

755.989

756.143

756.276 756.388

756.483

756.581

G	TR	ŊΕ	R	C

	GIRDER 9						
	Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection		
	€ Bearing S. Abutment	56+07.06	19,00	753,578	753.578		
	С	56+17.06	19.00	753.809	753.843		
	D	56+27.06	19.00	754.032	754.097		
	E	56+37.06	19.00	754.248	754.334		
ĺ	F	56+47.06	19.00	754,457	754.551		
	G	56+57.06	19.00	754,658	754.747		
	Н	56+67.06	19.00	754.852	754.922		
	I	56+77.06	19.00	755 . 038	755,079		
	ℚ Pier	56+89.31	19.00	755.256	755,256		
	J	56+99.31	19.00	755.426	755.460		
-	K	57+09.31	19.00	755,588	755.652		
1	L	57+19.31	19.00	755.743	755.828		
	М	57+29.31	19.00	755.891	755.985		
	N	57+39.31	19.00	756.030	756.120		
	0	57+49.31	19.00	756.163	756.234		
	P	57+59.31	19.00	756.288	756.330		
	© Bearing N. Abutment	57+71.56	19.00	756 . 431	756.431		
		<u> </u>	L				

<i>GIRDER</i>	10

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
<pre> Bearing S. Abutment C D E F G H I</pre>	56+04.39 56+24.39 56+24.39 56+34.39 56+54.39 56+54.39 56+64.39 56+86.64 56+96.64 57+06.64 57+16.64	27.00 27.00 27.00 27.00 27.00 27.00 27.00 27.00 27.00 27.00 27.00 27.00	753.394 753.627 753.853 754.071 754.282 754.485 754.681 754.869 755.090 755.261 755.426 755.582	753.394 753.662 753.918 754.157 754.376 754.573 754.750 754.910 755.090 755.295 755.489 755.668
M N O P © Bearing N. Abutment	57+26.64 57+36.64 57+46.64 57+56.64 57+68.89	27.00 27.00 27.00 27.00 27.00	755.732 755.874 756.008 756.135 756.281	755.826 755.963 756.079 756.178

GIRDER 11

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
€ Bearing S. Abutment C D	56+01.71 56+11.71 56+21.71	35.00 35.00 35.00	753.211 753.446 753.673	753.211 753.481 753.739
E F G H	56+31.71 56+41.71 56+51.71 56+61.71 56+71.71	35.00 35.00 35.00 35.00 35.00	753.893 754.106 754.311 754.509 754.699	753.980 754.200 754.400 754.579 754.740
© Pier J K I	56+83.96 56+93.96 57+03.96 57+13.96	35.00 35.00 35.00 35.00	754.922 755.096 755.262 755.421	754.922 755.130 755.326 755.506
M N O	57+23.96 57+33.96 57+43.96 57+53.96	35.00 35.00 35.00 35.00	755.572 755.716 755.853 755.982	755.667 755.806 755.924 756.024
€ Bearing N. Abutment	57+66.21	35.00	756.130	756.130

GIRDER 12

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
© Bearing S. Abutment	55+99.03	43.00	753.027	753.027
С	56+09,03	43,00	753,263	753.298
D	56+19.03	43.00	753 . 493	753.558
E	56+29.03	43.00	753.715	753.801
F	56+39.03	43.00	753.930	754.024
G	56+49.03	43.00	754.137	754.225
Н	56+59.03	43.00	754.337	754.406
I	56+69.03	43.00	754.529	754,570
ℚ Pier	56+81.28	43.00	754.754	754.754
J	56+91.28	43.00	754.930	754.964
K	57+01.28	43.00	755,098	755 . 162
L	57+11.28	43.00	755,259	755 . 345
М	57+21.28	43.00	755.413	755.507
N	57+31.28	43.00	755.559	755 . 648
0	57+41.28	43.00	755.697	755.768
P	57+51.28	43. 00	755.828	755.870
N. Abutment	57+63.53	43.00	755.978	755 . 978

SPANS #2 AND #3

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200 West Front Street Wheaton, II 60187

ILLINOIS DEPARTMENT OF TRANSPORTATION

TOP OF SLAB ELEVATIONS - V IL RTE. 47 OVER I-88 (E-W TOLLWAY) KANE COUNTY STRUCTURE NO. 045-0082

DATE: 2-01-2006

DRAWN BY WJV CHECKED BY BLB

SHEET NO. 9

27 32 SHEETS

ROUTE NO. SECTION

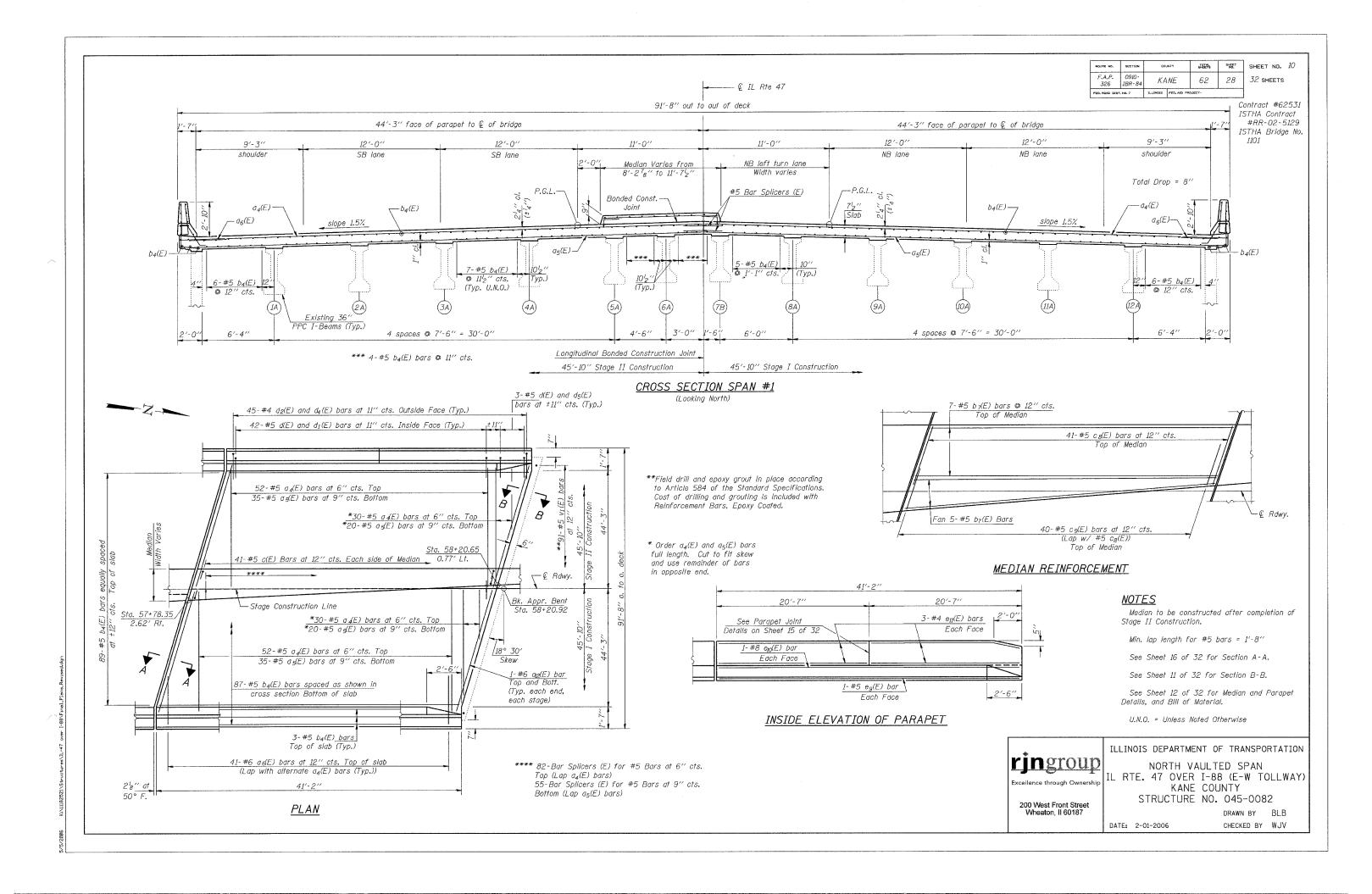
Contract #62531

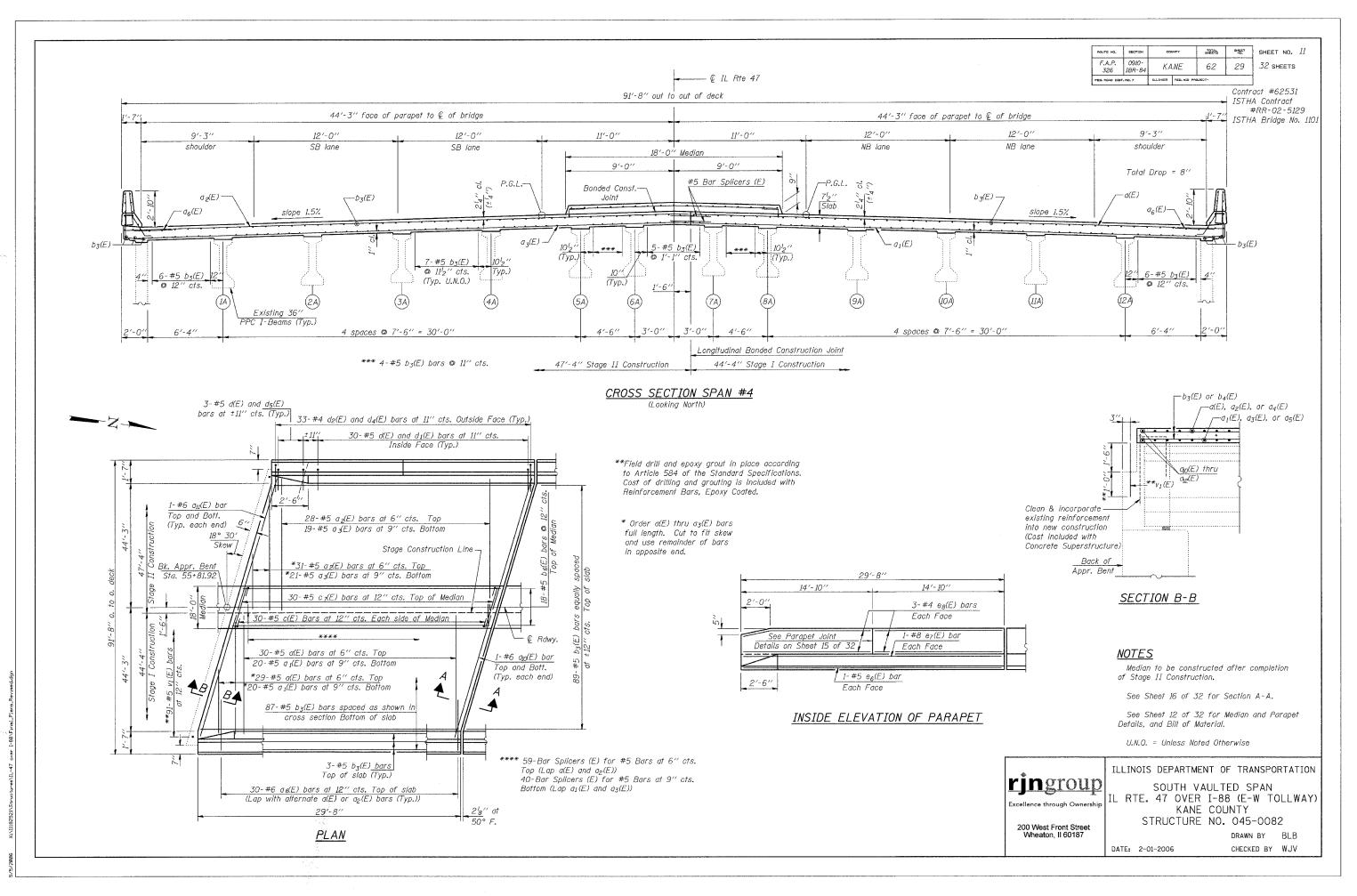
ISTHA Contract #RR-02-5129 ISTHA Bridge No. 1101

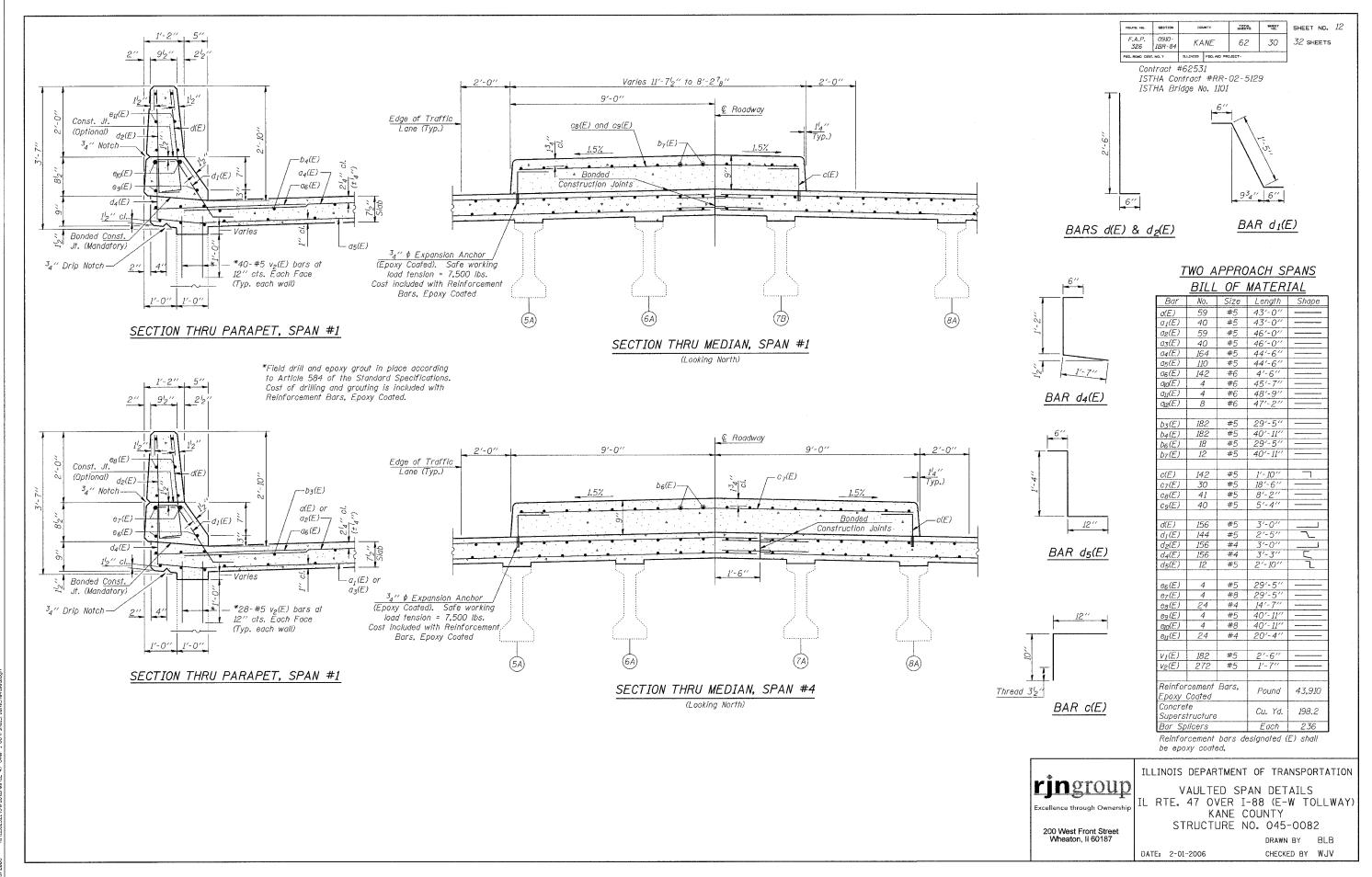
CGUNTY

KANE

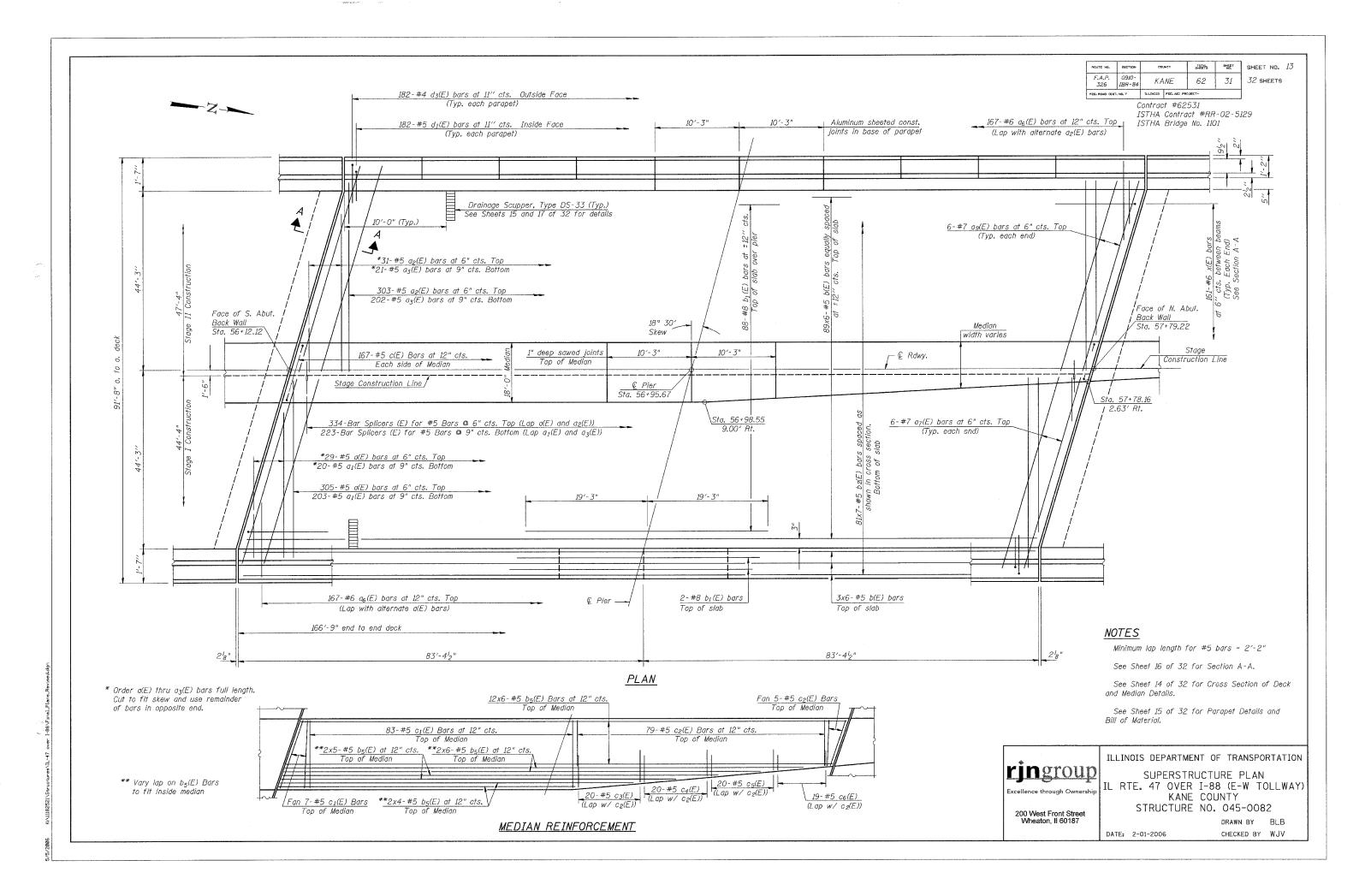
62







7 1100F(2) (CTC) (CTC) (CTC)

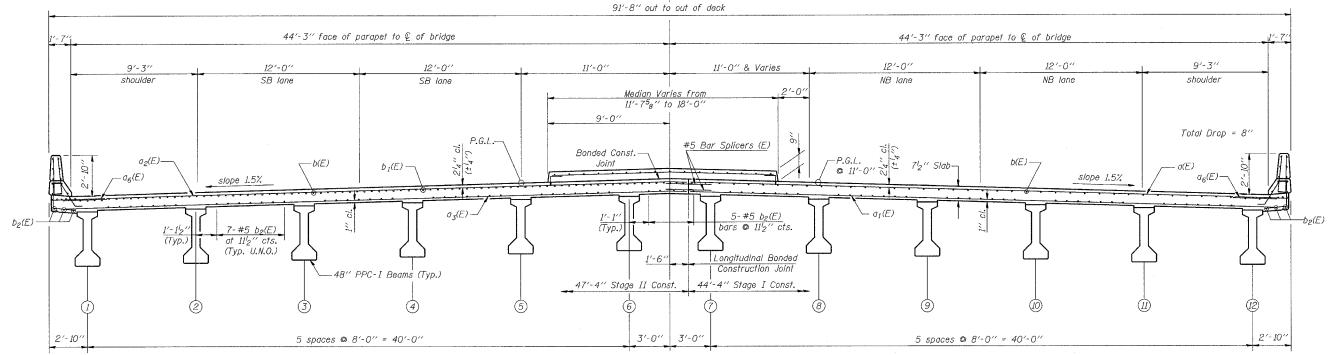


SHEET NO. 14 ROUTE NO. SECTION TOTAL SHEETS F.A.P. 0910-326 IBR-84 KANE 62 32 FED. ROAD DIST. NO. 7 ILLINGIS PED. ALD PROJE

32 sheets

Contract #62531

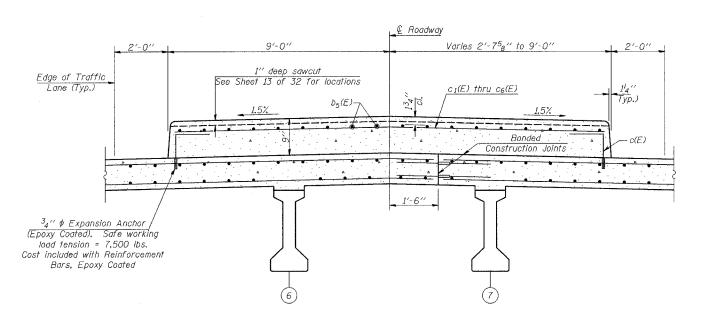
ISTHA Contract #RR-02-5129 ISTHA Bridge No. 1101



----- € IL Rte 47

CROSS SECTION SPAN #2 & #3

(Looking North)



SECTION THRU MEDIAN, SPANS #2 & #3

AT PIER

<u>NOTES</u>

AT MIDSPAN

Median to be constructed after completion of Stage II Construction.

See Sheet 15 of 32 for Parapet Details.

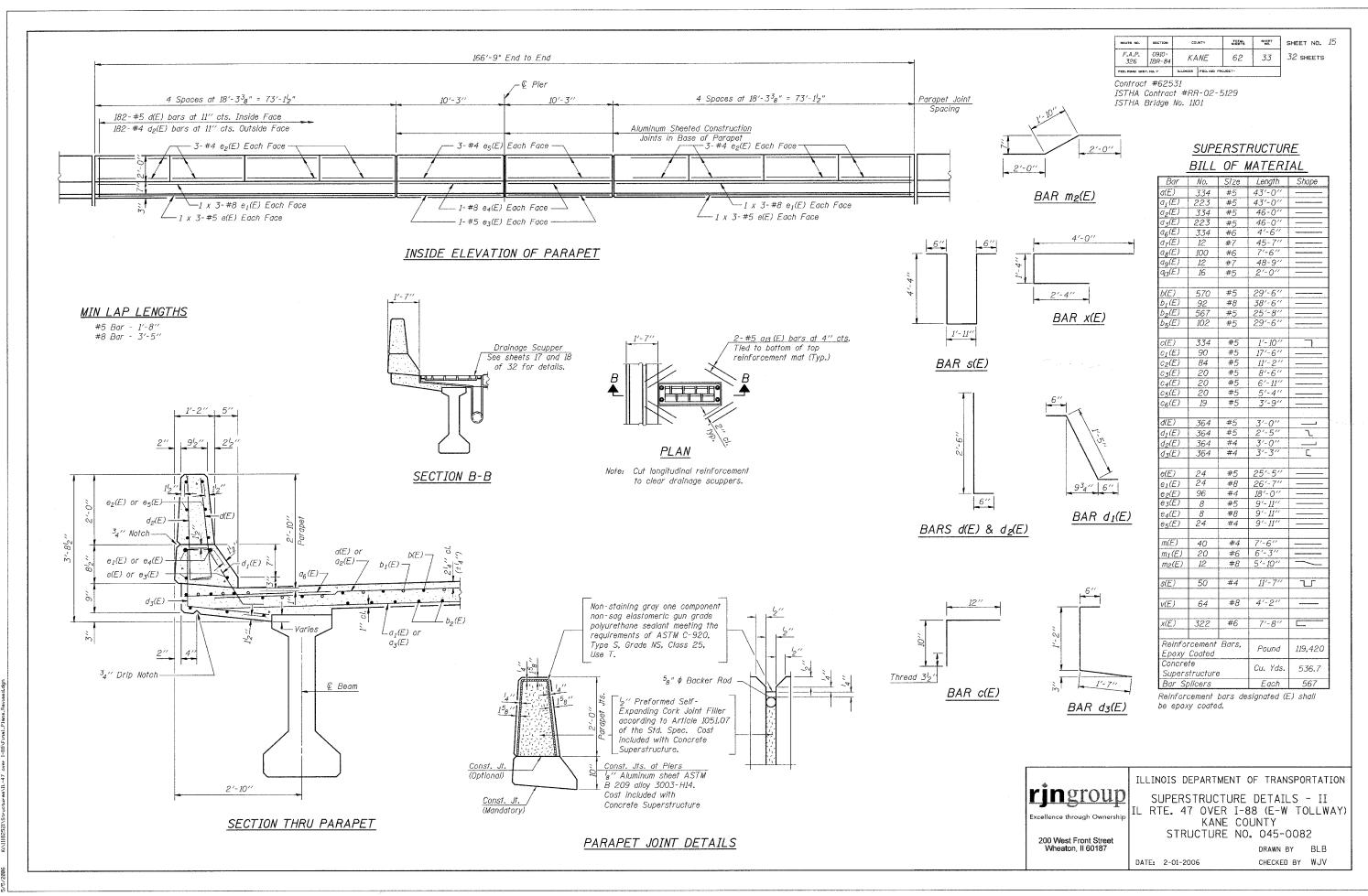
U.N.O. = Unless Noted Otherwise

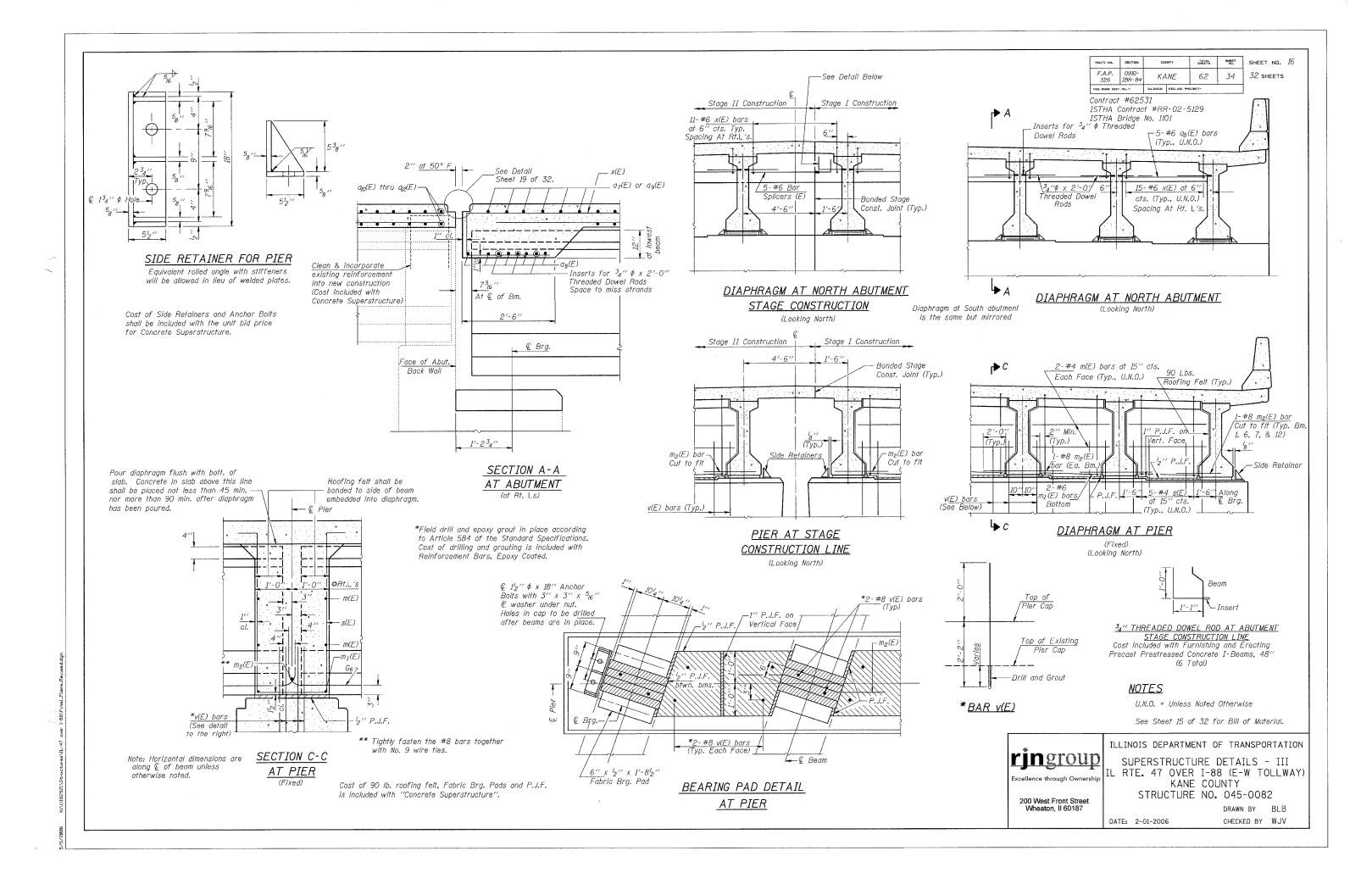


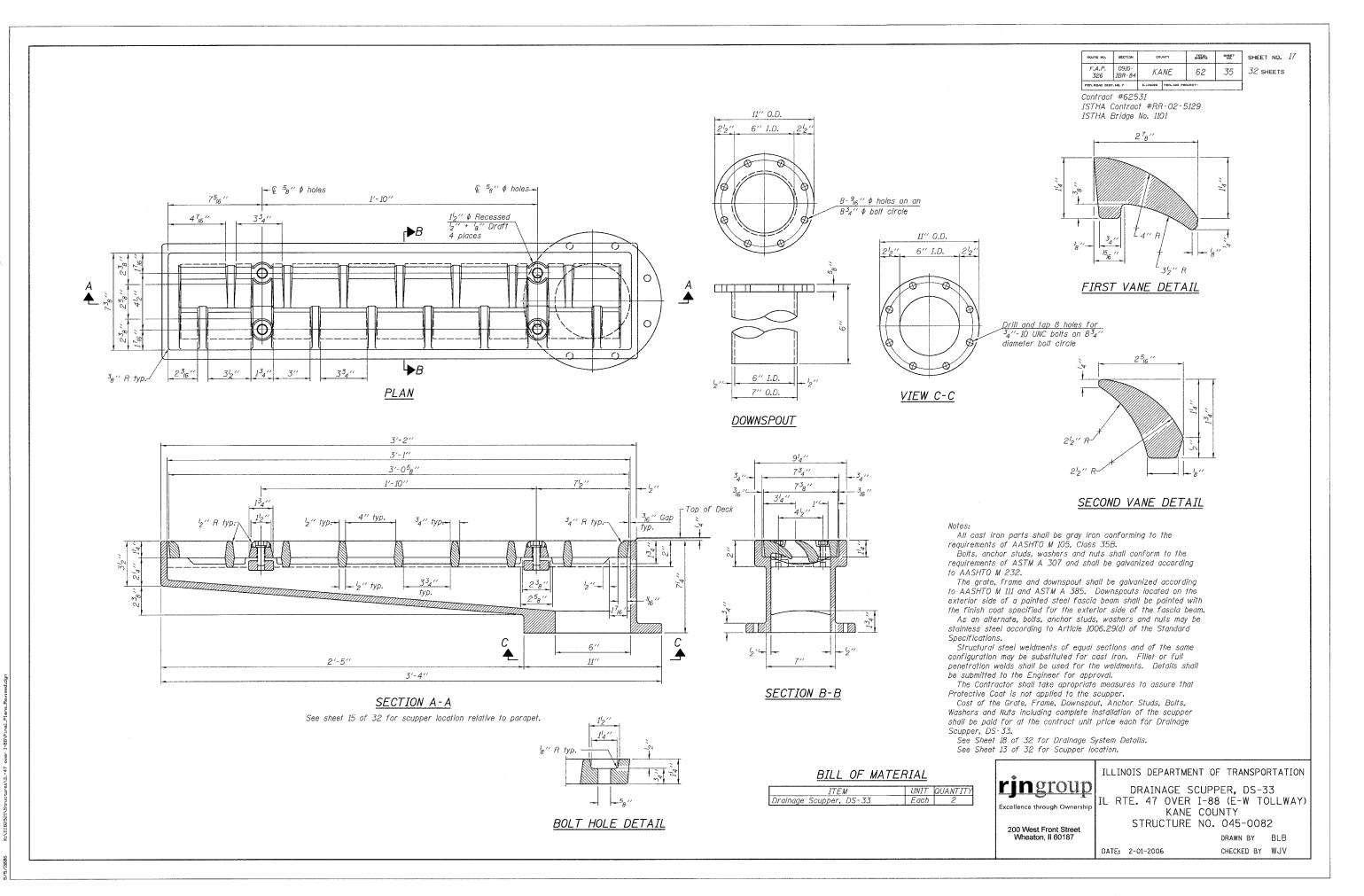
ILLINOIS DEPARTMENT OF TRANSPORTATION SUPERSTRUCTURE DETAILS - I IL RTE. 47 OVER I-88 (E-W TOLLWAY) KANE COUNTY STRUCTURE NO. 045-0082

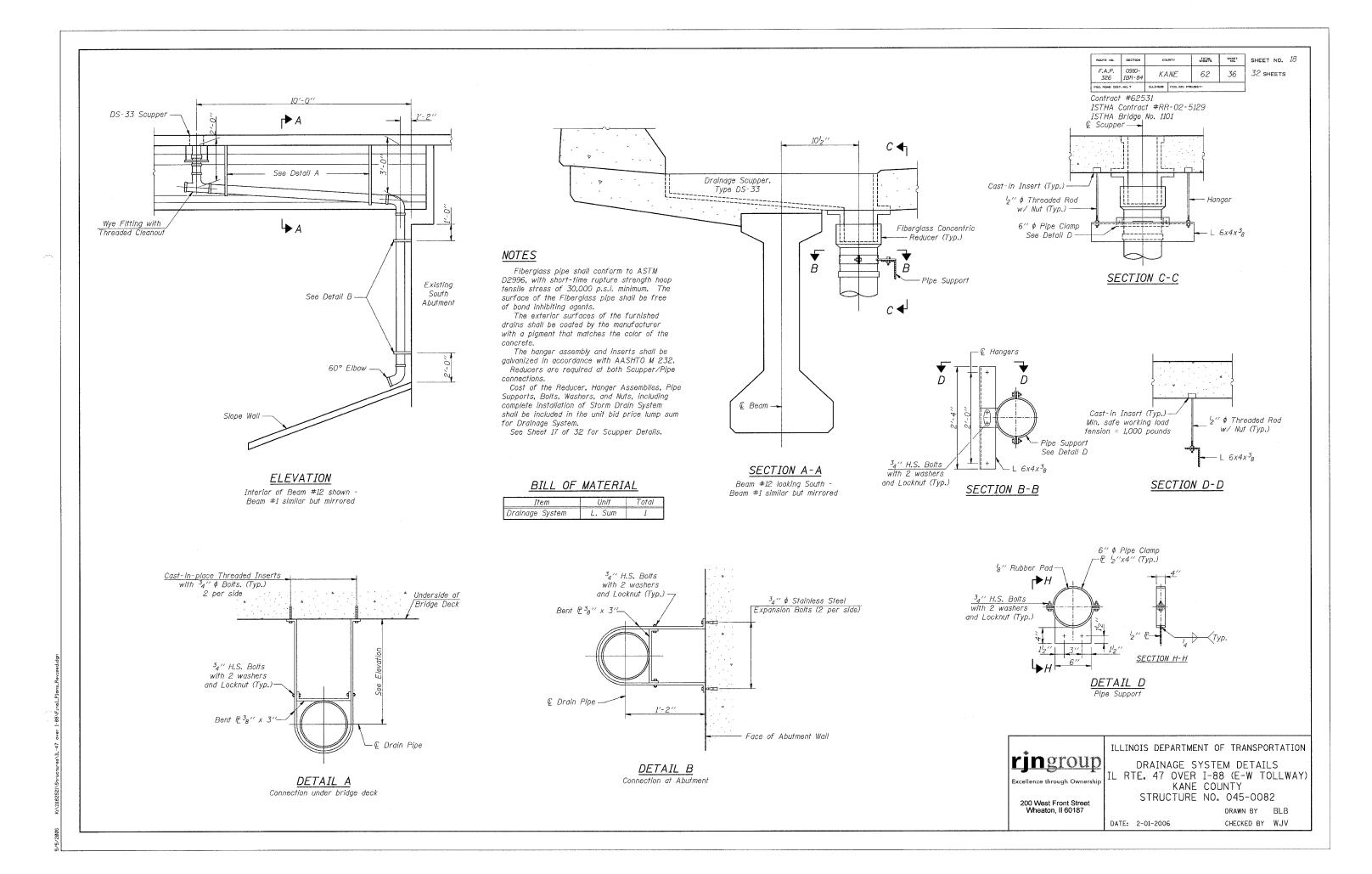
200 West Front Street Wheaton, II 60187

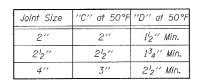
DRAWN BY BLB DATE: 2-01-2006 CHECKED BY WJV











INSTALLATION NOTES

- Install continuous seal in roadway, parapet, curb, and sidewalk.
- Install anchor blocks as indicated.

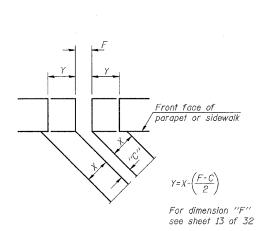
Note A.

Threaded Anchor Studs with Washers

Maximum spacing of anchor bolts shall be 12" centers.

SKEW LIMITATIONS

The details of the anchor blocks and the elastomeric membrane in the parapet, as shown, are for up to 50° skews. For skews greater than 50°, the anchor blocks and the elastomeric membrane, installed according to dimension "D", might require modifications to insure a minimum clearance of 1'2" from centerline of anchor studs to edge of parapet opening. The anchor blocks and the elastomeric membrane shall also be installed to the top of the parapet with the anchor studs spaced at ±12" cts.



FORMING BLOCKOUT **SKETCH**

AT PARAPET

Threaded anchor

studs with washers

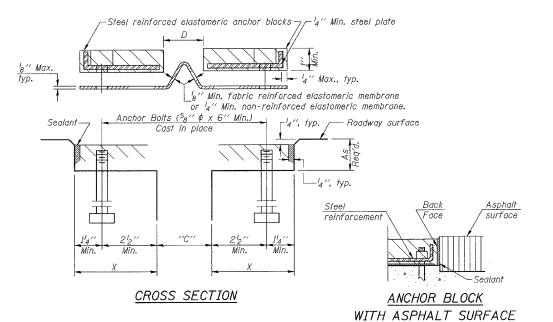
For skews greater than 50°

Std. anchor bolts

Cast in place

AT PARAPET

surface



SHEET NO. 19 0910-IBR-84 37 32 sheets KANE 62

Contract #62531 ISTHA Contract #RR-02-5129 ISTHA Bridge No. 1101

GENERAL NOTES

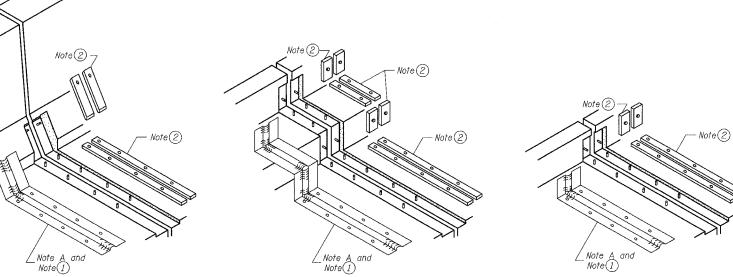
Continuous Seal Neoprene Expansion Joint shall consist of molded anchor blocks of elastomer and steel, field assembled over continuous lengths of elastomeric membrane.

The elastomeric membrane shall be premoided with a single or a double upward convolution that will have a "memory" to return to its molded position upon joint closure.

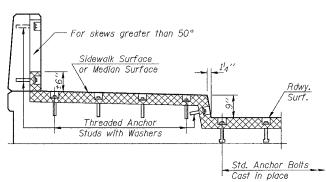
The convolution length shall be such that the extended length will not be greater than the manufactured length when the joint is fully expanded in its design range and will not protrude above the anchor blocks when the joint is fully compressed.

Joint openings shall be adjusted according to Article 503.10(c) of the Standard Specifications when the deck is poured at an ambient temperature other than 50° F.

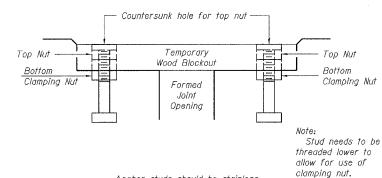
The parapet and roadway membrane shall be made continuous by an approved vulcanizing process. Lapping will not be permitted.



AT SIDEWALK OR MEDIAN



AT SIDEWALK OR MEDIAN TYPICAL END TREATMENTS



Anchor studs should be stainless

RECOMMENDED BLOCKOUT DETAIL

BILL OF MATERIAL

Item	Unit	Total
Neoprene Expansion Joint 2"	Foot	190

Studs with Washers For skews greater than 50° Std. Anchor Bolts Cast in place

AT WALL

AT WALL

ringroup

200 West Front Street Wheaton, II 60187

ILLINOIS DEPARTMENT OF TRANSPORTATION CONTINUOUS SEAL TYPE NEOPRENE EXPANSION JOINTS RTE. 47 OVER I-88 (E-W TOLLWAY) KANE COUNTY STRUCTURE NO. 045-0082

DATE: 2-01-2006

DRAWN BY CHECKED BY WJV

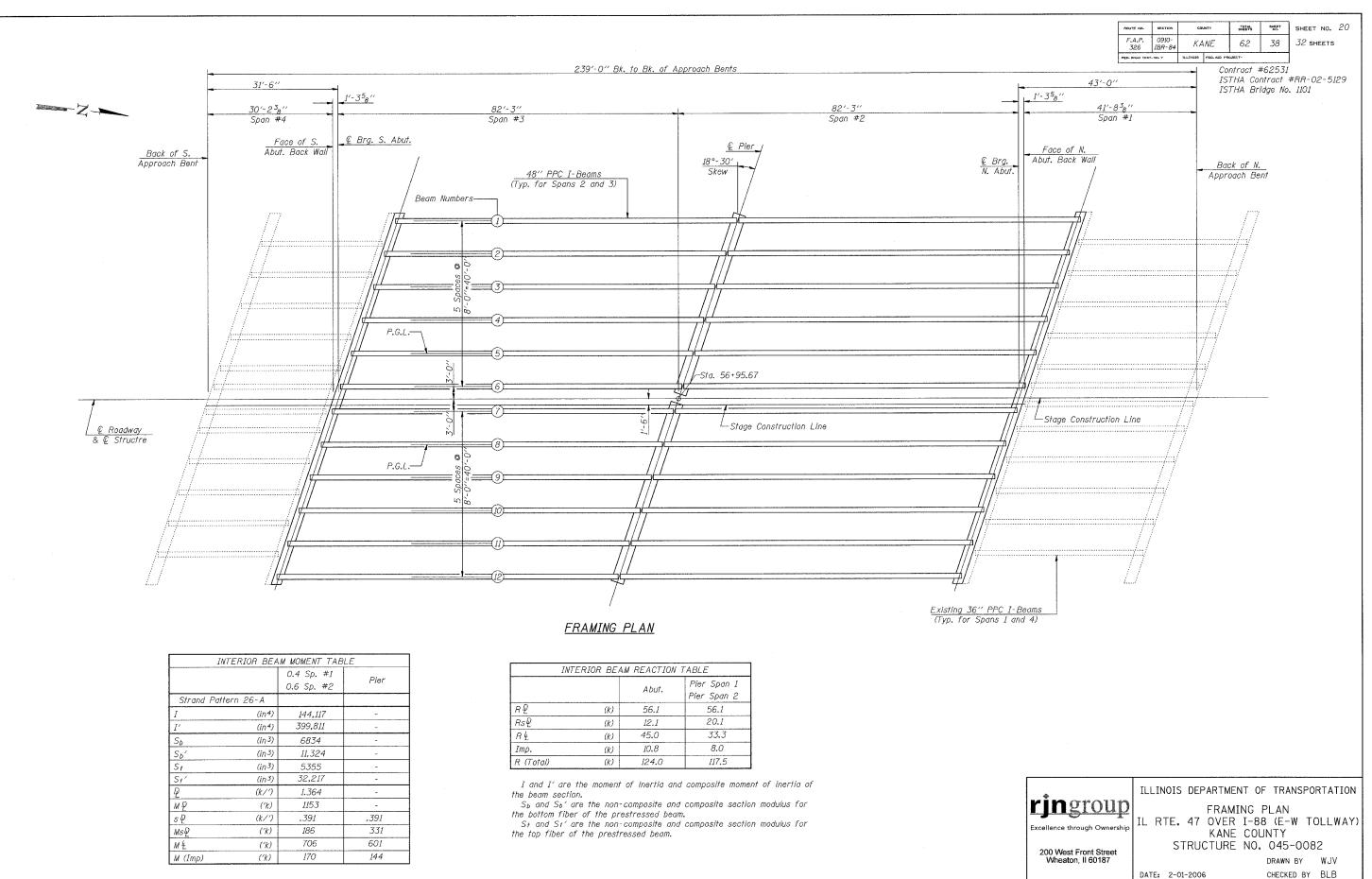
EJ-CS

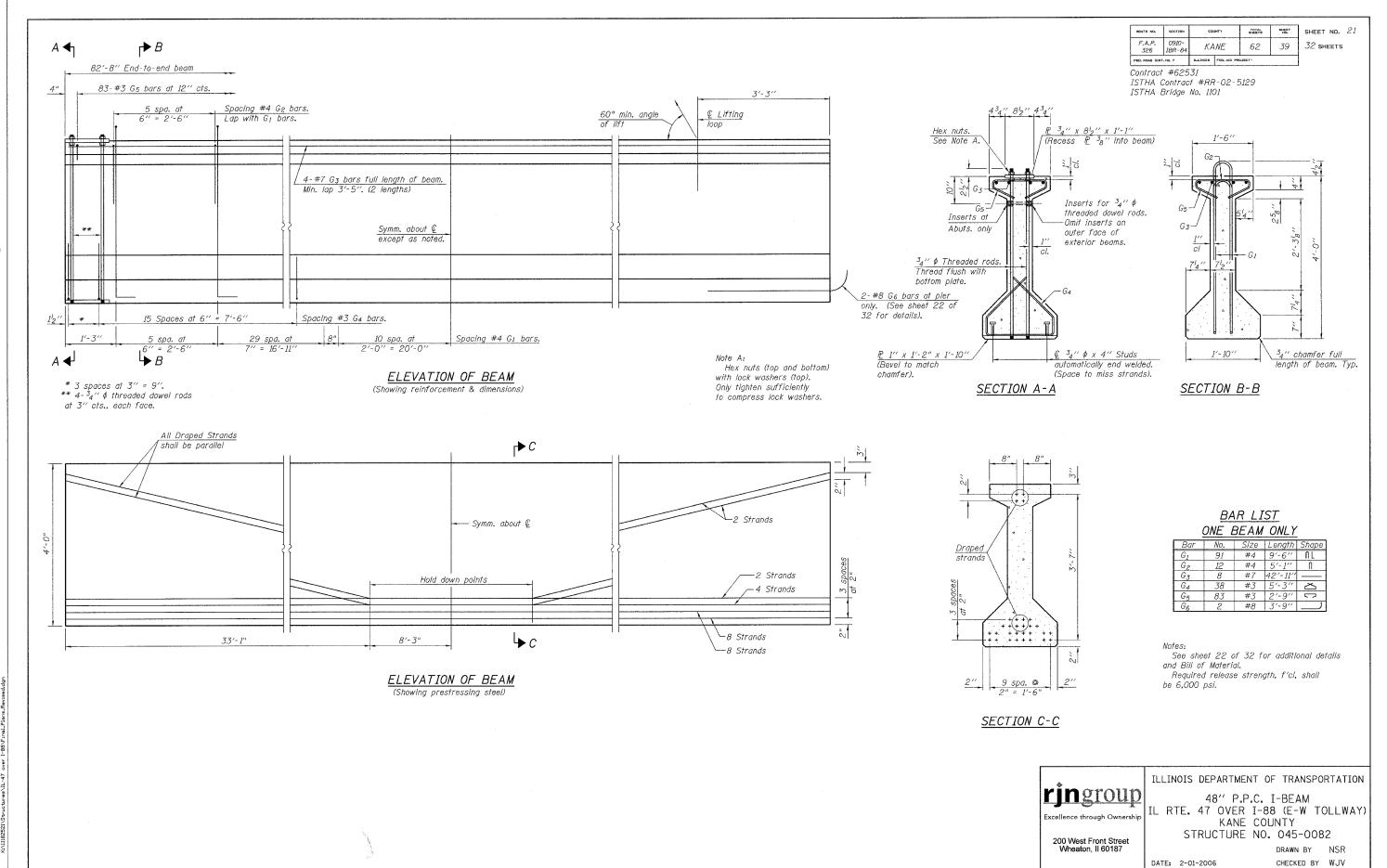
10-22-04

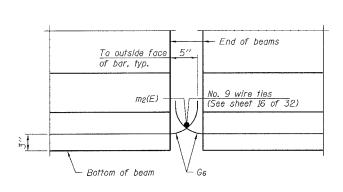
Std. Anchor Bolts

Cast in place

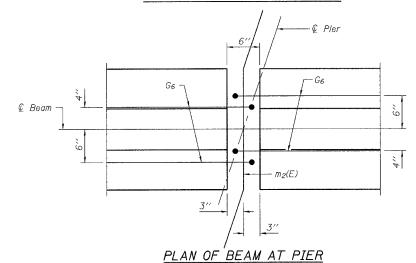
AT CURB

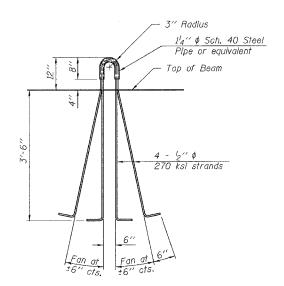




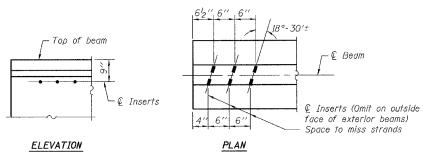


ELEVATION OF BEAM AT PIER

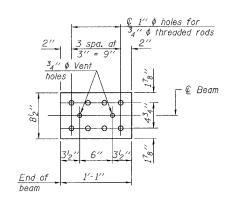




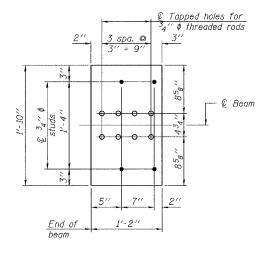
LIFTING LOOP DETAIL



INSERT DETAIL AT ABUTMENT END OF BEAM

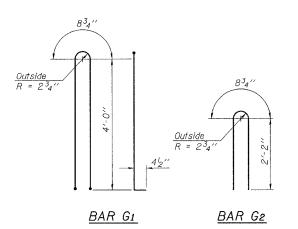


TOP PLATE



BOTTOM PLATE

See bearing details for pintle hole locations when required.



NOUTE NO. SECTION SHEET NO. 22 TOYAL F.A.P. 0910-326 IBR-84 KANE 62 40 FED. ROAD DIST. NO. 7 ILLINOIS FED. AID PRO

32 sheets

Contract #62531 ISTHA Contract #RR-02-5129 ISTHA Bridge No. 1101

NOTES

Inserts for ${}^3\!4''$ ϕ threaded dowel rods, when specified, are to be two strut, coil type for interior beams and single coil, flared loop type for exterior beams.

Prestressing steel shall be uncoated high strength, low relaxation 7-wire strand, Grade 270.

The nominal diameter shall be $\frac{1}{2}$ " and the nominal cross-sectional area shall be 0.153 sq. in.

Non-prestressing steel shall conform to AASHTO designation M-31 or M 322, Grade 60.

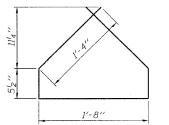
A minimum $2^{l}_{2}{}^{\prime\prime}$ ϕ lifting pin shall be used to engage the lifting loops during handling.

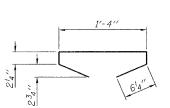
Reinforcement bars designated (E) shall be epoxy coated. Cut G_6 bars when necessary to maintain $1\frac{1}{2}$ " clearance.

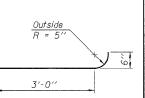
The bottom plates and study shall be galvanized according to AASHTO MIII and ASTM A385.

Threaded rods shall be ASTM F 1554 Grade 55.

The cut strands at each beam end shall be given two coats of zinc dust spray or paint meeting the requirements of ASTM A 780. The zinc dust spray or paint shall be applied before corrosion appears and allowed to dry according to the manufacturer's specifications prior to another coat of zinc. A concrete sealer meeting the requirements of Section 587 of the Standard Specifications shall be applied to all portions of the I-beam or Bulb-T beam, except the top surface of the top flange and the bottom surface of the bottom flange, starting at each beam end and extending out a distance of 48 inches. The sealer shall be applied after visible crack growth has subsided. This work shall be performed by the producer and included with the cost of the beam.







BAR G4

BAR G5

BAR G6

BILL OF MATERIAL

Item	Unit	Total
Furnishing and Erecting Precast Prestressed Concrete I-Beams, 48"	Ft.	1984



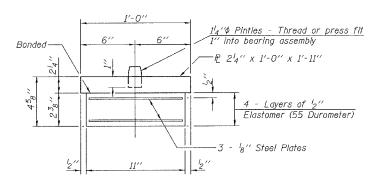
ILLINOIS DEPARTMENT OF TRANSPORTATION

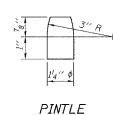
48" P.P.C. I-BEAM DETAILS IL RTE. 47 OVER I-88 (E-W TOLLWAY) KANE COUNTY STRUCTURE NO. 045-0082

200 West Front Street Wheaton, II 60187

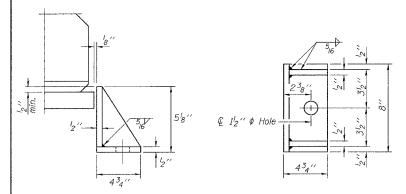
DRAWN BY NSR DATE: 2-01-2006 CHECKED BY WJV

TYPE I ELASTOMERIC EXP. BRG.





BEARING ASSEMBLY



SIDE RETAINER

Cost included with Elastomeric Bearing Assembly, Type I. Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates. Notes:

Holes at expansion bearings shall be drilled and anchor bolts grouted in place after beams have been erected. See sheet 24 of 32 for anchor bolt installation.

See sheet 22 of 32 for additional details of plate cast with beam.

Cost of anchor bolts included with Elastomeric Bearing Assembly, Type I.

Contract #62531 ISTHA Contract #RR-02-5129 ISTHA Bridge No. 1101

BILL OF MATERIAL

Item	Unit	Total
Elastomeric Bearing Assembly, Type I	Each	24

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200 West Front Street Wheaton, II 60187 ILLINOIS DEPARTMENT OF TRANSPORTATION

BEARING DETAILS
IL RTE. 47 OVER I-88 (E-W TOLLWAY)
KANE COUNTY
STRUCTURE NO. 045-0082

DRAWN BY NSR
DATE: 2-01-2006 CHECKED BY WJV

ILLINOIS COIL-LOCK ANCHOR BOLT

SHEET NO. 24 F.A.P. 0910-326 IBR-84 KANE 62 42 32 SHEETS

Contract #62531

ISTHA Contract #RR-02-5129 ISTHA Bridge No. 1101

GENERAL NOTES

Holes in the masonry for anchor bolts shall be drilled through the base plates to the diameter and depth shown or according to the manufacturer's recommendation after beams or girders have been erected and adjusted. Prior to setting the bolts, the holes shall be dry and all dust and loose

particles shall be removed by the use of compressed air or vacuuming. The anchor bolts, furnished and installed and including the epoxy grout or capsules shall not be paid for separately but shall be included in the unit bid price for Elastomeric Bearing Assembly, Type I (Abutments) or Concrete Superstructure (Pier).

MATERIALS FOR ILLINOIS COIL-LOCK ANCHOR BOLT

The anchor bolt shall be fabricated from cold drawn or hot finished seamless carbon steel mechanical tubing conforming to ASTM A 519, Grade 1026, CW and supplied with hexagonal nuts and cut washers.

The coil wire shall be made of any suitable soft steel wire.

The finished anchor bolt shall be cleaned of rust and other foreign materials and wrapped or packaged to prevent contamination until they are installed. The epoxy grout shall be a two-component, epoxy resin bonding system conforming to ASTM C 881, Type I, Grade 1 and of a Class sultable for the temperature at installation.

INSTALLATION PROCEDURE for the ILLINOIS COIL-LOCK ANCHOR BOLT

- 1. With the coil wire in place, the bolt shall be inserted into the hole and turned clockwise to a snug fit in the hole. Nut and washer shall be placed on the bolt. The nut shall be tensioned until the steel base plates are held securely to the concrete bearing seat.
- 2. Epoxy grout shall be pumped through the zerk fitting with a pressure gun. Pumping shall continue until the epoxy overflows the hole around the bolt shank. After pumping is discontinued, excess epoxy shall be immediately wiped off.

ALTERNATE ANCHOR BOLTS

The Contractor may use, at his option, the capsule or the adhesive cartridge type anchor rods that have been previously tested and given a prior approval by the Department. The Contractor shall install these anchor rods in pre-drilled holes according to the manufacturer's recommendations and procedures.

The capsule or the adhesive cartridge type anchor rods shall be a two part system composed of:

- 1. A threaded rod stud with nut and washer of the type specified.
- 2. A sealed glass capsule or a sealed glass adhesive cartridge containing premeasured amounts of the adhesive chemical.

Location	Туре
N. Abut.	A 307
S. Abut.	A 307
Pier	A 307

ASTM F 1554 Grade 105, ASTM A 449 and AASHTO M 314 Grade 105 anchor bolts may be substituted for the anchor bolts shown above.

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ILLINOIS DEPARTMENT OF TRANSPORTATION

ANCHOR BOLT DETAILS IL RTE. 47 OVER I-88 (E-W TOLLWAY) KANE COUNTY STRUCTURE NO. 045-0082

DATE: 2-01-2006

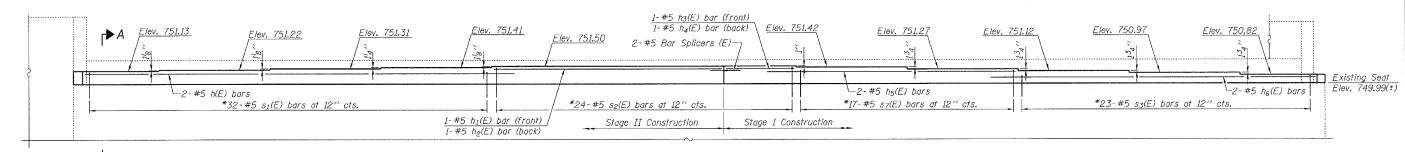
DRAWN BY BLB CHECKED BY WJV

200 West Front Street Wheaton, II 60187

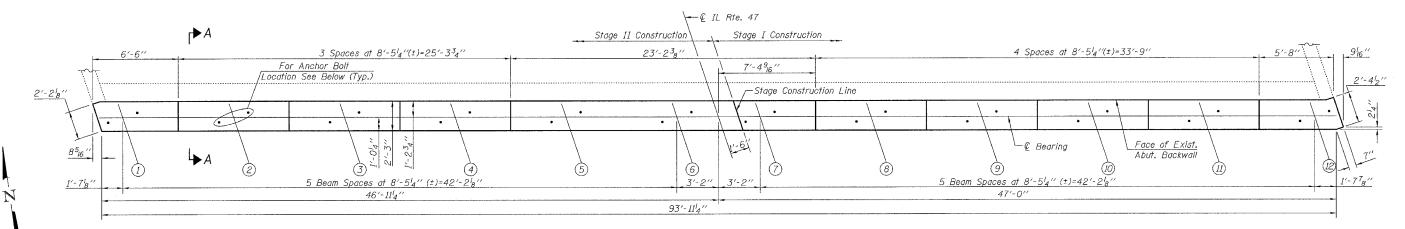
AGUTE NO. SECTION TOTAL SHEET NO. 25 F.A.P. 0910-326 IBR-84 KANE 62 43

32 sheets

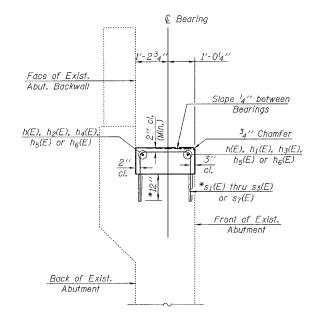
Contract #62531 ISTHA Contract #RR-02-5129 ISTHA Bridge No. 1101



ELEVATION (Looking North)

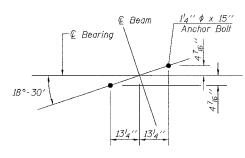


<u>PLAN</u>



SECTION A-A

*Field drill and epoxy grout in place according to Article 584 of the Standard Specifications. Cost of drilling and grouting is included with Reinforcement Bars, Epoxy Coated.



ANCHOR BOLT LOCATION DETAIL (Typical for all beams)

<u>NOTES</u>

See Sheet 24 of 32 for Anchor Bolt installation.

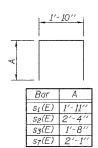
Space reinforcement in cap to miss anchor bolts.

Reinforcement bars designated (E) shall be epoxy coated.

Pour steps monolithically with cap.

All edges shall have standard ${}^3\!4''$ chamfers.

All surfaces where new concrete is cast against existing concrete shall be Bonded Construction Joints, in accordance with Article 503.09(b)(2) of the Standard Specifications.



BARS s1(E) THRU s3(E), s7(E)

Bar	No.	Size	Length	Shape
h(E)	2	#5	33'-0"	
$h_1(E)$	1	#5	17'-4''	
h ₂ (E)	1	#5	16′-9′′	
h3(E)	1	#5	5'-3"	
h4(E)	1	#5	5'-10''	
h5(E)	2	#5	18'-9''	-
h ₆ (E)	2	#5	24'-6"	-
$s_1(E)$	32	#5	5′-8′′	П
s2(E)	24	#5	6'-6''	
s3(E)	23	#5	5'-2''	
57(E)	17	#5	6′-0′′	П
Concre	te Struc	Cu. Yd.	10.1	
Reinforcement Bars,			Pound	790
Epoxy Coated			i ouno	730
Bridge	Bridge Seat Sealer			215
Bar Sp	licers		Each	2

BILL OF MATERIAL

Reinforcement bars designated (E) shall be epoxy coated.

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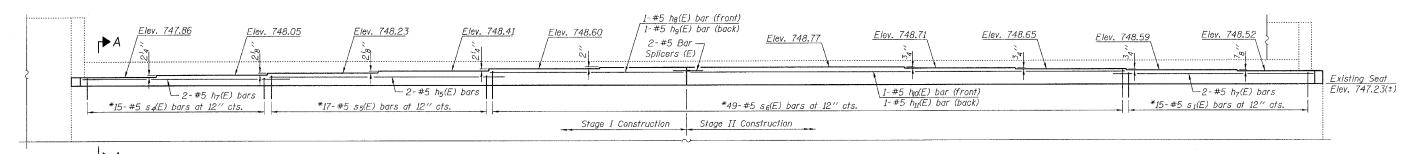
ILLINOIS DEPARTMENT OF TRANSPORTATION

NORTH ABUTMENT DETAILS IL RTE. 47 OVER I-88 (E-W TOLLWAY) KANE COUNTY STRUCTURE NO. 045-0082

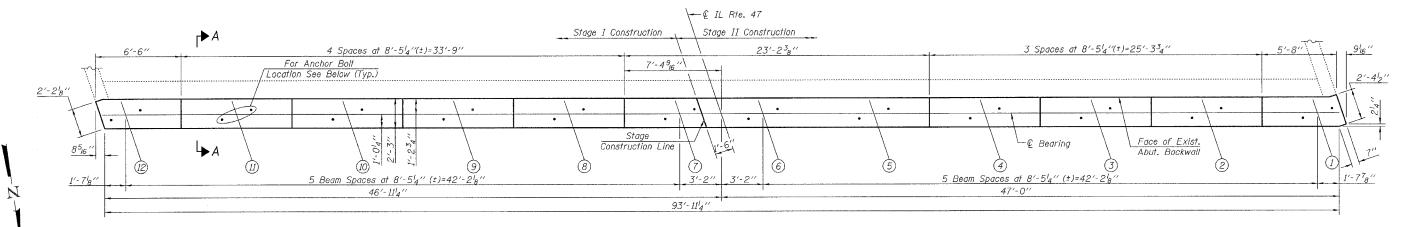
DRAWN BY BLB DATE: 2-01-2006 CHECKED BY WJV

200 West Front Street Wheaton, II 60187

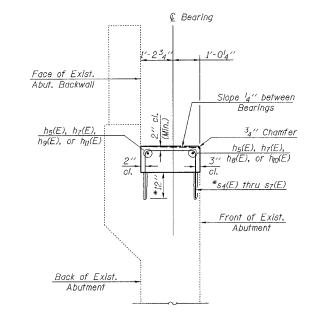
Contract #62531 ISTHA Contract #RR-02-5129 ISTHA Bridge No. 1101



ELEVATION (Looking South)

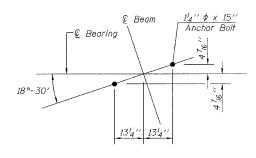


PLAN



SECTION A-A

*Field drill and epoxy grout in place according to Article 584 of the Standard Specifications. Cost of drilling and grouting is included with Reinforcement Bars, Epoxy Coated.



ANCHOR BOLT LOCATION DETAIL
(Typical for all beams)

<u>NOTES</u>

See Sheet 24 of 32 for Anchor Bolt Installation.

Space reinforcement in cap to miss anchor bolts.

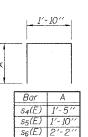
Reinforcement bars designated (E) shall be epoxy coated.

Pour steps monolithically with cap.

All edges shall have standard ${}^3\!4''$ chamfers.

All surfaces where new concrete is cast against existing concrete shall be Bonded Construction Joints, in accordance with Article 503.09(b)(2) of the Standard Specifications.

Lap Length for #5 Bar = 1'-8''.



BARS s1(E), s4(E) THRU s6(E)

<u>BILL OF MATERIAL</u>

Bar	No.	Size	Length	Shape
h ₅ (E)	2	#5	18'-9"	
h ₇ (E)	4	#5	16'-10''	
h ₈ (E)	1	#5	14-2"	
h ₉ (E)	1	#5	13'-7''	
h ₁₀ (E)	1	#5	33′-9″	-
$h_{II}(E)$	1	#5	34'-3"	
54(E)	<i>1</i> 5	#5	4'-8''	
s5(E)	17	#5	5'-6"	
s ₆ (E)	49	#5	6'-2"	
s7(E)	15	#5	6'-0''	
Concre	Concrete Structures		Cu. Yd.	10.0
Reinforcement Bars,		Pound	790	
Epoxy Coated			i odila	130
Bridge Seat Sealer			Sq. Ft.	215
Bar Sp	licers		Each	2

Reinforcement bars designated (E) shall be enoxy coated

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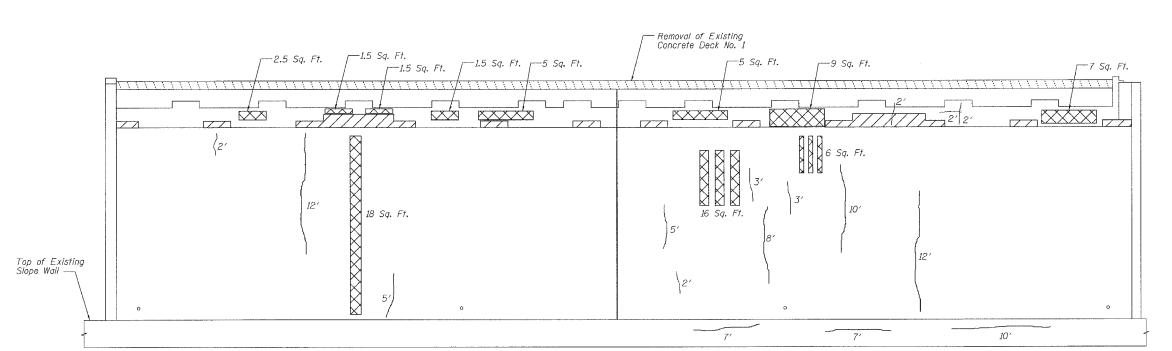
200 West Front Street Wheaton, II 60187 ILLINOIS DEPARTMENT OF TRANSPORTATION

SOUTH ABUTMENT DETAILS
IL RTE. 47 OVER I-88 (E-W TOLLWAY)
KANE COUNTY
STRUCTURE NO. 045-0082

DRAWN BY BLB

DATE: 2-01-2006 CHECKED BY WJV

Kiviii8252ivStructuresviL-47 over I-88vFinal_Plans_Rev



| ROJER NO. | GRETISH | COUNTY | STATES | STATES

| FED. NOAG CLST. NO. 7 | 11.LINGUS | FED. AID PROJECT-| Contract #62531 | ISTHA Contract #RR-02-5129

LEGEND:

ISTHA Bridge No. 1101

Epoxy Crack Sealing

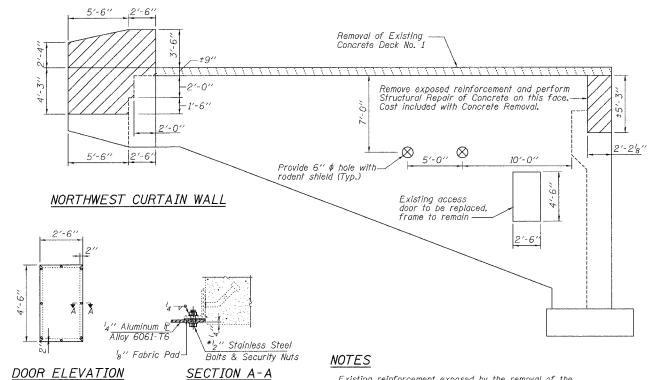
Structural Repair of Concrete (Depth Equal To or Less Than 5")

Concrete Removal

NNNN

Removal of Existing Concrete Deck

NORTH ABUTMENT ELEVATION



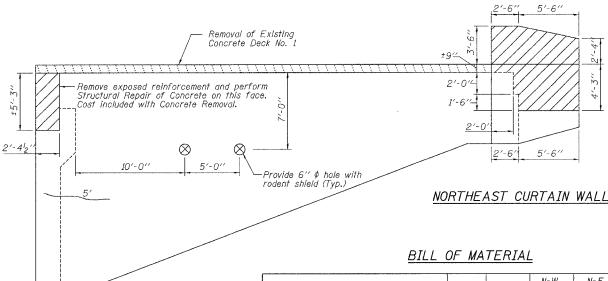
ACCESS DOOR DETAIL

*Provide Fasteners to match the existing

bolt hole locations

Existing reinforcement exposed by the removal of the abutment seats shall be removed. Cost included with Concrete Removal.

See Sheet 28 of 32 for Rodent Shield Detail.



PAY ITEM	UNIT	N. ABUT.	N-W CURTAIN WALL	N-E CURTAIN WALL	TOTAL
Structural Repair of Concrete (Depth Equal To or Less Than 5")	Sq. Ft.	73	0	0	73
Epoxy Crack Sealing	Foot	92	0	5	97
Concrete Removal	Cu. Yd.	3.1	2.7	2.7	8.5
Removal of Existing Concrete Deck No. 1	Each				1
Furnish and Install Door	Each	1			1

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200 West Front Street Wheaton, II 60187 ILLINOIS DEPARTMENT OF TRANSPORTATION

NORTH ABUTMENT REPAIRS

NORTH ABDIMENT REPAIRS

IL RTE. 47 OVER I-88 (E-W TOLLWAY)

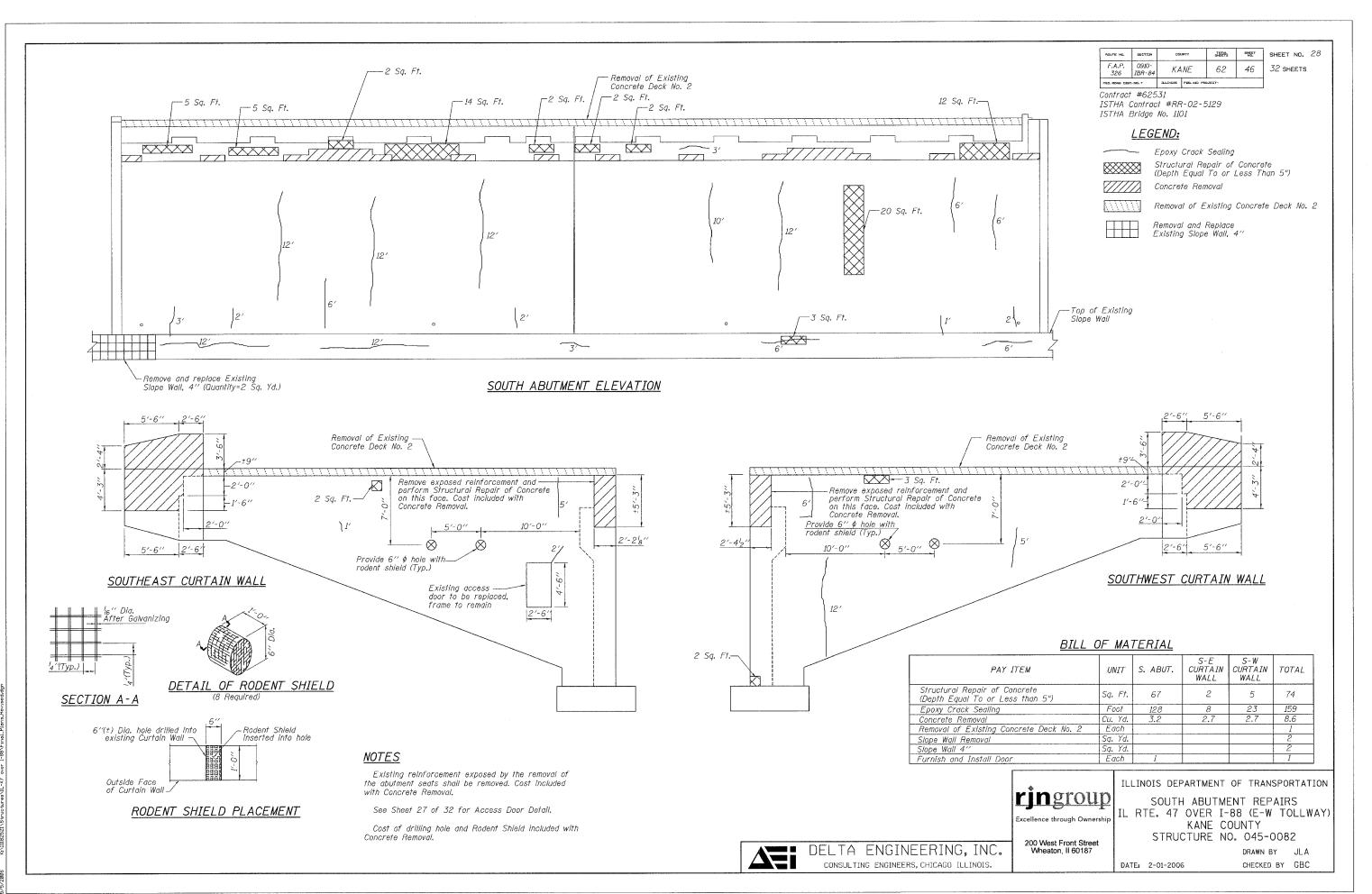
KANE COUNTY

STRUCTURE NO. 045-0082

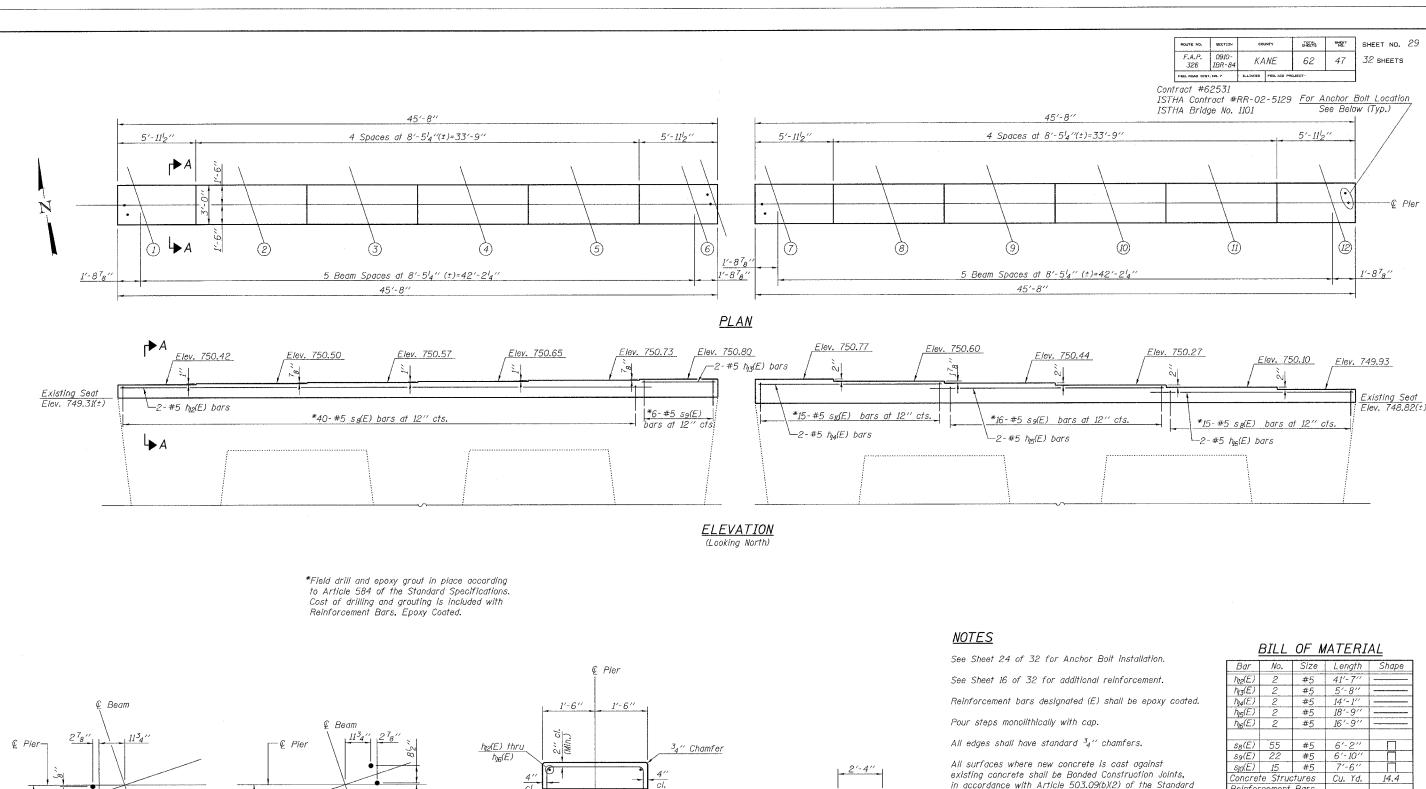
DATE: 2-01-2006 DRAWN BY JLA GBC

DELTA ENGINEERING, INC. consulting engineers, chicago illinois.

11/Structures\1L-47 over I-88\



/s



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200 West Front Street Wheaton, II 60187

Space reinforcement in cap to miss anchor bolts.

ILLINOIS DEPARTMENT OF TRANSPORTATION

Reinforcement bars designated (E) shall

Pound

830

PIER DETAILS IL RTE. 47 OVER I-88 (E-W TOLLWAY) KANE COUNTY

STRUCTURE NO. 045-0082 DRAWN BY BLB

Reinforcement Bars,

Epoxy Coated

be epoxy coated.

DATE: 2-01-2006 CHECKED BY WJV

ANCHOR BOLT LOCATION DETAIL

18°-30′

*s₈(E) thru s₁₀(E) Face of Exist. s₈(E) 1'-11'

Face of Exist.
Pier

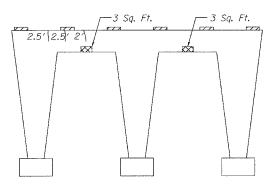
SECTION A-A

BARS S8(E) THRU S10(E)

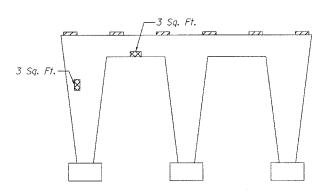
Specifications,

ROUTE NO.	SECTION	cou	INTY	TOTAL SHEETS	SHEET NO.	SHEET NO. 30
F.A.P. 326	0910- IBR-84	KA	NE	62	48	32 SHEETS
PED, ROAD DIST	NQ. 7	ILLINOIS	FED. ALD PROJECT-			

Contract #62531 ISTHA Contract #RR-02-5129 ISTHA Bridge No. 1101



EAST PIER ELEVATION NORTH FACE



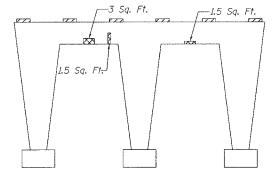
<u>WEST PIER ELEVATION</u> <u>NORTH FACE</u>



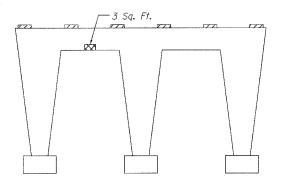
Epoxy Crack Sealing

Structural Repair of Concrete (Depth Equal To or Less Than 5")

Concrete Removal



EAST PIER ELEVATION SOUTH FACE



WEST PIER ELEVATION SOUTH FACE

BILL OF MATERIAL

PAY ITEM	UNIT	EAST PIER	WEST PIER	TOTAL
Structural Repair of Concrete (Depth Equal To or Less Than 5")	Sq. Ft.	12	9	21
Epoxy Crack Sealing	Foot	7	0	7
Concrete Removal	Cu. Yd.	0.9	0.6	1.5

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200 West Front Street Wheaton, II 60187

ILLINOIS DEPARTMENT OF TRANSPORTATION

PIER REPAIRS IL RTE. 47 OVER I-88 (E-W TOLLWAY) KANE COUNTY STRUCTURE NO. 045-0082

DATE: 2-01-2006

DRAWN BY JLA CHECKED BY GBC

ROUTE NO.	SECTION	COUNTY		TOTAL	SPARET NO.	5
F.A.P. 326	0910- IBR-84	KA	NE	62	49	
EEO 0010 8107	440.77	To a Passage	een am no	TECT.		

SHEET NO. 3132 sheets

Contract #62531 ISTHA Contract #RR-02-5129 ISTHA Bridge No. 1101

--The diameter of this part is equal or larger than the diameter of bar spliced.

of the bar spliced. ROLLED THREAD DOWEL BAR

The diameter of this part

is the same as the diameter

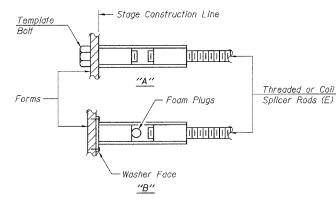
** ONE PIECE

Wire Connector

WELDED SECTIONS

BAR SPLICER ASSEMBLY ALTERNATIVES

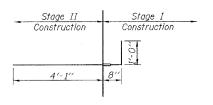
** Heavy Hex Nuts conforming to ASTM A 563, Grade C, D or DH may be used.



INSTALLATION AND SETTING METHODS

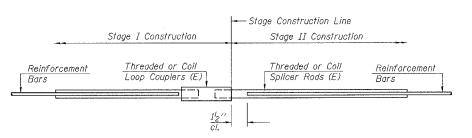
"A" :Set bar splicer assembly by means of a template bolt. "B" :Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E): Indicates epoxy coating.



#6 BAR SPLICER (E) AT ABUTMENTS

SPECIAL SPLICER DETAIL



STANDARD

	Bar Size	No. Assemblies Required	Location
	#5	4	Abutments
**	#6	10	Diaphragms
	#5	557	Deck, Span #2 & #3
	#5	137	Deck, Span #1
	#5	99	Deck, Span #4

*** See special splicer details

NOTES

Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars. Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length.

All reinforcement bars shall be lapped and tied to the splicer rods or dowel bars. Bar splicer assemblies shall be epoxy coated according to the requirements for

reinforcement bars. Other systems of similar design may be submitted to the Engineer for approval. Approval shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

Minimum Capacity (Tension in kips) = 1.25 x fy x A_t

(Tension in Kipo)
Minimum *Pull-out Strength = 1.25 x fs_{allow} x A_t

(Tension in kips) Where fy = Yield strength of lapped reinforcement bars in ksi.

fs_{allow}= Allowable tensile stress in lapped reinforcement bars in ksi (Service Load)

A_i = Tensile stress area of lapped reinforcement bars.

* = 28 day concrete

	BAR SPLICER ASSEMBLIES						
		Strength Requirements					
Bar Size to be Spliced	Splicer Rod or Dowel Bar Length		Min. Pull-Out Strength kips - tension				
#4	1′-8′′	14.7	5.9				
#5	2'-0''	23.0	9,2				
#6	2′-7′′	33.1	13.3				
#7	3′-5″	45.1	18.0				
#8	4′-6′′	58.9	23.6				
#9	5′-9′′	75.0	30.0				
#10	7′-3′′	95.0	38.0				
#11	9'-0''	117.4	46.8				

Bar splicer assemblies shall be according to Section 508 of the Standard Specifications, except as noted. The furnishing and installation of bar splicer assemblies will be measured and paid for at the contract unit price each for "BAR SPLICERS."

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ILLINOIS DEPARTMENT OF TRANSPORTATION

BAR SPLICER ASSEMBLY DETAILS IL RTE. 47 OVER I-88 (E-W TOLLWAY) KANE COUNTY STRUCTURE NO. 045-0082

200 West Front Street Wheaton, II 60187 DATE: 2-01-2006

DRAWN BY BLB CHECKED BY WJV

AQUTE NO.	SECTION	COL	PATA	TOYAL SHEETS	SHEET NO.	SHEET NO. 32
F.A.P. 326	0910 - IBR - 84	KA	NE	62	50	32 SHEETS
FED. 8040 DIST. NO. 7		ILLINOIS	PED. AID PR	BJECT-		

Contract #62531 ISTHA Contract #RR-02-5129 ISTHA Bridge No. 1101

Temporary Concrete Barrier See Standard 704001 Stage Construction Line — Stage Removal Line NJ shape | F shape NJ shape 1'-1012 shape When "A" is 3'-6" or less, the temporary concrete barrier shall be anchored to the new slab according to Detall I or Detail II. No anchorage is required / See Detail I or Detail II. - Drill 1 4" \phi Holes in existing slab for $1'' \neq x 10''$ dowel bars. Styrofoam Pads (NJ Shape

NEW SLAB

only) See Standard 704001

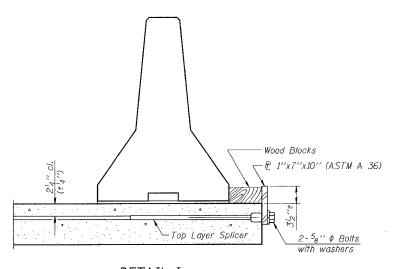
EXISTING SLAB

Traffic side only. Cost included

with Temporary Concrete Barrier.

SECTIONS THRU SLAB

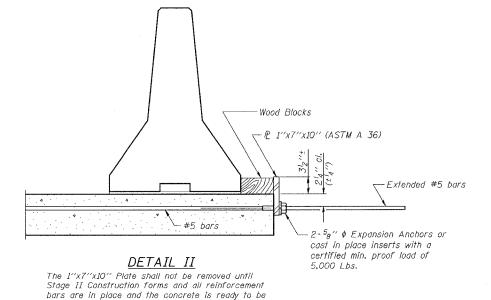
placed.



DETAIL I

when "A" is greater than 3'-6".

The I"x7"x10" Plate shall not be removed until Stage II Construction forms and reinforcement bars are in place.



Top bars spacing Detail I Detail II

NOTES

top layer of couplers with $2^{-5}8'' \phi$ bolts screwed to coupler at approximate Q of

Connect one (1) 1''x7''x10'' steel f_c^p to the concrete slab with $2^{-5}g''$ ϕ Expansion Anchors or cast in place inserts spaced between the

top layer of reinforcement at approximate & of

Detail I - With Bar Splicer or Couplers:
Connect one (1) 1''x7''x10'' steel 12 to the

each barrier panel. Detail II - With Extended Reinforcement Bars:

each barrier panel.

Cost of anchorage is included with Temporary Concrete Barrier.

尼 1"x7"x10"

* Required only with Detail II

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200 West Front Street Wheaton, II 60187

* @ 1"x12" Notch

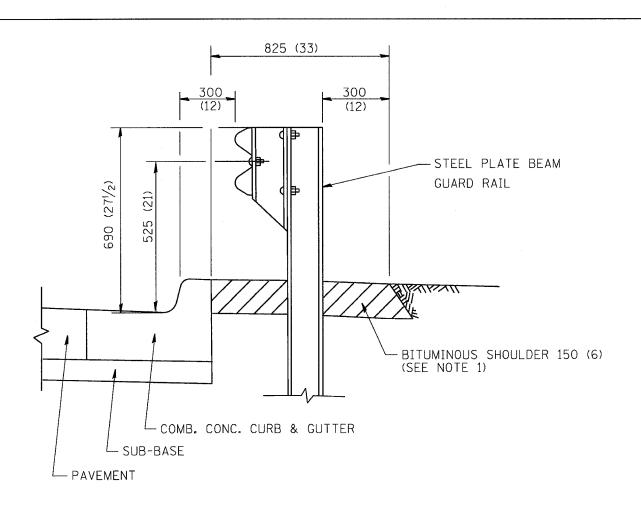
ILLINOIS DEPARTMENT OF TRANSPORTATION TEMPORARY CONCRETE BARRIER FOR STAGE CONSTRUCTION IL RTE. 47 OVER I-88 (E-W TOLLWAY) KANE COUNTY

STRUCTURE NO. 045-0082 DRAWN BY BLB

DATE: 2-01-2006 CHECKED BY WJV

R-27

10-22-04



NOTES: 1. THE BITUMINOUS SHOULDER SHALL EXTEND UNDER THE TRAFFIC BARRIER TERMINAL

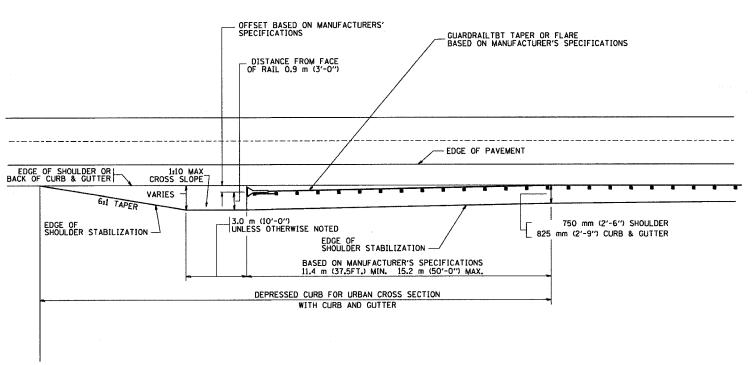
> 2. GUARD RAIL MAY BE PLACED AT THE BACK OF CURB WHEN DIRECTED BY THE ENGINEER.

BASIS OF PAYMENT: BITUMINOUS SHOULDER 150 (6) WILL BE PAID FOR AT THE CONTRACT UNIT PRICE PER m² (sq. yd.) AS "BITUMINOUS SHOULDER 150 (6)."

> STEEL PLATE BEAM GUARD RAIL AND TRAFFIC BARRIER TERMINAL, OF THE TYPE SPECIFIED WILL BE PAID FOR SEPARATELY.

DETAILS FOR STEEL PLATE BEAM GUARD RAIL ADJACENT TO CURB AND GUTTER [FOR ROADWAY SPEED 60 kmh (35 MPH) TO 70 kmh (45 MPH)]

SECTION COUNTY 326 E-8-4-B KAME TO STA. FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT



STABILIZATION AT TBT TY. 1 SPL.

ALL DIMENSIONS ARE IN MILLIMETERS (INCHES) UNLESS OTHERWISE SHOWN.

ILLINOIS DEPARTMENT OF TRANSPORTATION

DETAILS FOR STEEL PLATE BEAM GUARD RAIL ADJACENT TO CURB AND GUTTER STABILIZATION AT TBT TY 1 SPL.

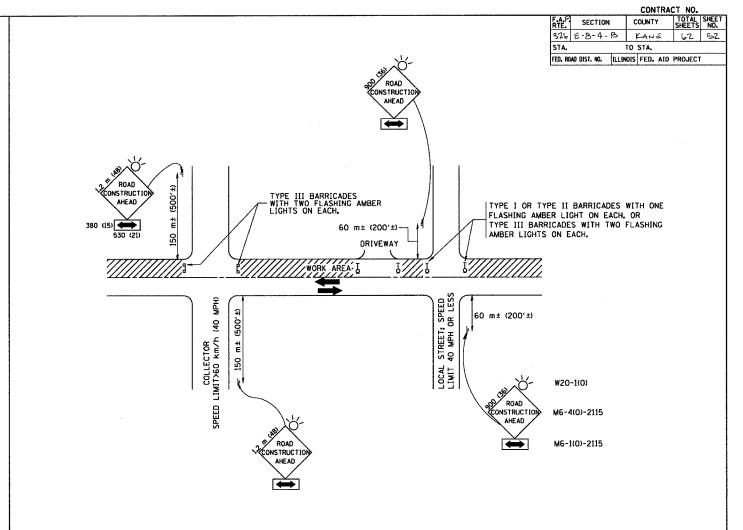
SCALE: VERT. HORIZ. DATE: 2/15/2006

DRAWN BY 11s CHECKED BY

CONTRACT NO.

BD600-10 (BD 34) REVISION DATE: 08/28/00

| DATE = 2/15/2886 | NAME = Whichstatabed34.dgn | SCALE = 49/1832.235 'r / IN, | NAME = goglionobt



TRAFFIC CONTROL AND PROTECTION FOR SIDE ROADS, INTERSECTIONS, AND DRIVEWAYS

NOTES:

- A. FOR NO LANE RESTRICTION ON THE SIDE ROAD OR DRIVEWAYS
- 1. SIDE ROAD WITH A SPEED LIMIT OF 60 km/h (40 MPH) OR LESS AS SHOWN ON THE DRAWING AND AS DIRECTED BY THE ENGINEER:
- O) ONE ROAD CONSTRUCTION AHEAD SIGN 900×900 (36×36) WITH A FLASHER AND FLAG MOUNTED ON IT APPROXIMATELY 60 m (200") IN ADVANCE OF THE MAIN ROUTE.
- b) THE CLOSED PORTION OF THE MAIN ROUTE SHALL BE PROTECTED BY BLOCKING WITH TYPE I, TYPE II OR TYPE III BARRICADES, 1/3 OF THE CROSS SECTION OF THE CLOSED PORTION.
- 2. SIDE ROAD WITH A SPEED LIMIT GREATER THAN 60 km/h (40 MPH) AS SHOWN ON THE DRAWING AND AS DIRECTED BY THE ENGINEER:
- Q) ONE ROAD CONSTRUCTION AHEAD SIGN 1.2 m x 1.2 m (48x48) WITH A FLASHER MOUNTED ON 1T APPROXIMATELY 150 m (500°) IN ADVANCE OF THE MAIN ROUTE.
- b) THE CLOSED PORTION OF THE MAIN ROUTE SHALL BE PROTECTED BY BLOCKING WITH TYPE III BARRICADES, 1/2 OF THE CROSS SECTION OF THE CLOSED PORTION.
- 3. WHEN THE SIDE ROAD LIES BETWEEN THE BEGINNING OF THE MAINLINE SIGNING AND THE WORK ZONE, A SINGLE HEADED ARROW (MG-1) SHALL BE USED IN LIEU OF THE DOUBLE HEADED ARROW (MG-4).

B. FOR A LANE CLOSURE ON A SIDE ROAD OR DRIVEWAY:

- USE APPLICABLE PORTIONS OF THE TYPICAL APPLICATION OF TRAFFIC CONTROL DEVICES (STD. 701501, STD. 701506 OR THE APPROPRIATE STANDARD). THE SPACING OF SIGNS AND BARRICADES SHALL BE ADJUSTED FOR FIELD CONDITIONS AS DIRECTED BY THE ENGINEER. THE DIRECTIONAL ARROW SHALL BE COVERED OR REMOVED WHEN NO LONGER CONSISTENT WITH THE SIDE ROAD LANE CLOSURE.
- C. ADVANCE WARNING SIGNS ARE TO BE OMITTED ON DRIVEWAY UNLESS OTHERWISE NOTED.
- D. THE TRAFFIC CONTROL AND PROTECTION FOR SIDE ROADS, INTERSECTIONS, AND DRIVEWAYS SHALL BE INCIDENTAL TO THE COST OF SPECIFIED TRAFFIC CONTROL STANDARDS OR ITEMS.

REVISIONS				
NAME		DATE		
LHA		6/89	1	
T. RAMMACHER	09	/08/94	١.	
J. OBERLE	10	/18/95		
A. HOUSEH	03	/06/96		
A. HOUSEH	10	/15/96		
T. RAMMACHER	01	/06/00		
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ILLINOIS DEPARTMENT OF TRANSPORTATION
TRAFFIC CONTROL AND PROTECTION
FOR

SIDE ROADS, INTERSECTIONS, AND DRIVEWAYS

SCALE: DATE: 2/15/2006

DRAWN BY CHECKED BY

TC-10

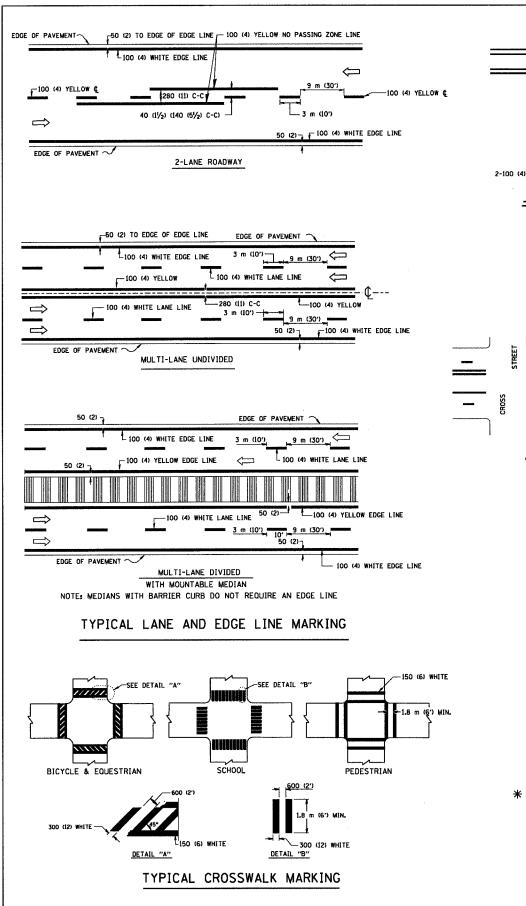
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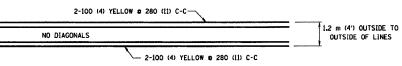
PLOT DATE = FILE NAME = PLOT SCALE = USER NAME =

REVISION DATE: 01/06/00

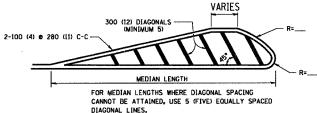
CONTRACT NO. SECTION COUNTY 326 E-8-4-B KANE 62 53 STA. TO STA. FEO. ROAD DIST. NO. ILLINOIS FED. AID PROJECT 24 m (80°) O.C. 3 e 12 m (40°) O.C. 24 m (80') O.C. SEE NOTE B \Leftrightarrow D € *** REDUCE TO 12 m (40') O.C. ON CURVES WITH POSTED OR ADVISORY SPEED 70 km/h (45 M.P.H.) OR LESS. 12 m (40') O.C. TWO-LANE/TWO-WAY SEE NOTE A-LANE REDUCTION TRANSITION TWO-WAY LEFT TURN 24 m (80') O.C. 24 m (80') O.C. SEE NOTE B SEE NOTE B GENERAL NOTES SYMBOLS 1. MARKERS USED WITH DASHED LINES SHALL BE CENTERED IN THE GAP BETWEEN SEGMENTS. ---- YELLOW STRIPE WHITE STRIPE 2. MARKERS USED ADJACENT TO SOLID LINES SHALL BE OFFSET 50 TO 75 (2 TO 3) TOWARD TRAFFIC AS SHOWN. ONE-WAY AMBER MARKER 3. MARKERS THROUGH TANGENTS LESS THAN 150 m (500') IN LENGTH BETWEEN CURVES SHALL BE INSTALLED AT THE LESSER OF THE TWO CURVE SPACINGS. ONE-WAY CRYSTAL MARKER (W/O) ⇒ TWO-WAY AMBER MARKER SEE NOTE A MULTI-LANE/UNDIVIDED LANE MARKER NOTES SEE NOTE A B. REDUCE TO 12 m (40') O.C. ON CURVES WHERE ADVISORY SPEEDS ARE 20 km/h (10 M.P.H.) LOWER THAN POSTED SPEEDS. MULTI-LANE/DIVIDED A. USE DOUBLE LANE LINE MARKERS SPACED AS SHOWN. DESIGN NOTES 1. DOUBLE LANE LINE MARKERS SHALL BE USED UNLESS SPECIFIED OTHERWISE. 2. EXCEPT AS SHOWN ON THE LANE REDUCTION TRANSITION AND FREEWAY EXIT RAMP DETAIL, MARKERS ARE NOT TO BE SPECIFIED ON RIGHT EDGE LINES. 3. THE EXACT MARKER LIMITS, SPACING, AND COLOR SHOULD BE INCLUDED IN THE PLANS. MARKERS SHOULD NOT BE USED ALONGSIDE CURBS EXCEPT FOR EXTREMELY SHORT SECTIONS OF CURBS WHERE NOT MORE THAN TWO MARKERS WOULD BE INVOLVED. MINIMUM OF 3 W
EQUALLY SPACED 3 @ 24 m (80') O.C. ___ 3 **a** 24 m (80°) 0.C. 3 e 12 m (40°) 3 e 12 m (40') 12 m (40°) O.C. \Rightarrow All dimensions are in millimeters (inches) unless otherwise shown. ILLINOIS DEPARTMENT OF TRANSPORTATION * SEE TWO-LANE/TWO-WAY WHERE MARKERS CONTINUE * * WHERE THE MEDIAN WIDTH IS 2 m (6') OR LESS USE TWO-WAY MARKERS. TYPICAL APPLICATIONS RAISED REFLECTIVE PAVEMENT MARKERS (SNOW-PLOW RESISTANT) LEFT TURN DRAWN BY CADD DATE: 2/15/2006 TC-11

REVISION DATE: 01/06/00



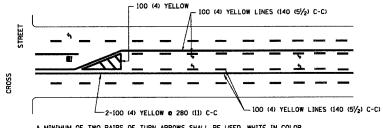


1.2 m (4') WIDE MEDIANS ONLY

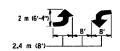


DIAGONAL LINE SPACING: 15 m (50") C-C (LESS THAN 50 km/h (30 MPH))
25 m (75") C-C (50 km/h (30 MPH) TO 70 km/h (45 MPH)) 45 m (150') C-C (MORE THAN 70 km/h (45 MPH))

MEDIANS OVER 1.2 m (4') WIDE

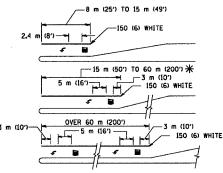


A MINIMUM OF TWO PAIRS OF TURN ARROWS SHALL BE USED, WHITE IN COLOR. ADDITIONAL PAIRS SHALL BE PLACED AT 60 m (200') TO 90 m (300') INTERVALS.



MEDIAN WITH TWO-WAY LEFT TURN LANE

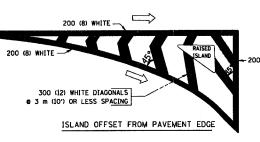
TYPICAL PAINTED MEDIAN MARKING

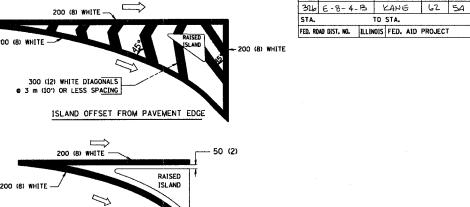


* TURN LANES IN EXCESS OF 120 m (400') IN LENGTH MAY HAVE AN ADDITIONAL SET OF ARROW - "ONLY" INSTALLED MIDWAY BETWEEN THE OTHER TWO SETS OF

TYPICAL LEFT (OR RIGHT) TURN LANE

TYPICAL TURN LANE MARKING





TYPICAL ISLAND MARKING

ISLAND AT PAVEMENT EDGE

				CDAGNIG / DEMARKS
TYPE OF MARKING	WIDTH OF LINE	PATTERN	COLOR	SPACING / REMARKS
CENTERLINE ON 2 LANE PAVEMENT	100 (4)	SKIP-DASH	YELLOW	3 m (10') LINE WITH 9 m (30') SPACE
CENTERLINE ON MULTI-LANE UNDIVEDED PAVEMENT	2 @ 100 (4)	SOLID	YELLOW	280 (II) C-C
NO PASSING ZONE LINES: FOR ONE DIRECTION FOR BOTH DIRECTIONS	100 (4) 2 c 100 (4)	SOLID SOLID	YELLOW YELLOW	140 (5)/2) C-C FROM SKIP-DASH CENTERLINE 280 (11) C-C OMIT SKIP-DASH CENTERLINE BETWEEN
LANE LINES	100 (4) 125 (5) ON FREEWAYS	SKIP-DASH SKIP-DASH	WHITE WHITE	3 m (10") LINE WITH 9 m (30") SPACE
DOTTED LINES (EXTENSIONS OF CENTER, LANE OR TURN LANE MARKINGS)	SAME AS LINE BEING EXTENDED	SKIP-DASH	SAME AS LINE BEING EXTENDED	600 (2') LINE WITH 1.8 m (6') SPACE
EDGE LINES	100 (4)	SOLID	YELLOW-LEFT WHITE-RIGHT	OUTLINE MOUNTABLE MEDIANS IN YELLOW; EDGE LINES ARE NOT USED NEXT TO BARRIER CURB
TURN LANE MARKINGS	150 (6) LINE; FULL SIZE LETTERS & SYMBOLS (2.4 m (8'))	SOLID	WHITE	SEE TYPICAL TURN LANE MARKING DETAIL
TWO WAY LEFT TURN MARKING	2 @ 100 (4) EACH DIRECTION	SKIP-DASH AND SOLID	YELLOW	3 m (10") LINE WITH 9 m (30") SPACE FOR SKIP-DASH: 140 (5½) C-C BETWEEN SOLID LINE AND SKIP-DASH LINE
	2.4 m (8') LEFT ARROW	IN PAIRS	WHITE	SEE TYPICAL TWO-WAY LEFT TURN MARKING DETAIL
CROSSWALK LINES (PEDESTRIAN) A. DIAGONALS (BIKE & EQUESTRIAN) B. LONGITUDINAL BARS (SCHOOL)	2 m 150 (6) 300 (12) m 45° 300 (12) m 90°	SOLID SOLID SOLID	WHITE WHITE WHITE	NOT LESS THAN 1.8 m (6') APART 600 (2') APART 600 (2') APART SEE TYPICAL CROSSWALK MARKING DETAILS.
STOP LINES	600 (24)	SOLID	WHITE	PLACE 1.2 m (4') IN ADVANCE OF AND PARALLEL TO CROSSWALK, IF PRESENT. OTHERWISE, PLACE AT DESIRED STOPPING POINT. PARALLEL TO CROSSROAD CENTERLINE, WHERE POSSIBLE
PAINTED MEDIANS	2 @ 100 (4) WITH 300 (12) DIAGONALS @ 45° NO DIAGONALS USED FOR 1.2 m (4') WIDE MEDIANS	SOLID	YELLOW: TWO WAY TRAFFIC WHITE: ONE WAY TRAFFIC	280 (II) C-C FOR THE DOUBLE LINE SEE TYPICAL PAINTED WEDIAN WARKING.
GORE MARKING AND CHANNELIZING LINES	200 (8) WITH 300 (12) DIAGONALS @ 45°	SOLID	WHITE	DIAGONALS: 4.5 m (15°) C-C (LESS THAN 50 km/h (30 MPH)) 6 m (20°) C-C (50 km/h (30 MPH) TO 70 km/h (45 MPH)) 9 m (30°) C-C (OVER 70 km/h (45 MPH))
RAILROAD CROSSING	600 (24) TRANSVERSE LINES: "RR" IS 1.8 m (6') LETTERS: 400 (16) LINE FOR "X"	SOLID	WHITE	SEE STATE STANDARD 780001 AREA 0F: "R"=0.33m2 (3.6 SQ. FT.) EACH "X"=5.0 m2 (54.0 SQ. FT.)
SHOULDER DIAGONALS	300 (12) e 45°	SOLID	WHITE - RIGHT YELLOW - LEFT	15 m (50") C-C (LESS THAN 50 km/h (30 MPH)) 25 m (75") C-C (50 km/h (30 MPH) TO 70 km/h (45 MPH)) 45 m (150") C-C (OVER 70 km/h (45 MPH))

FOR FURTHER DETAILS ON PAVEMENT MARKING REFER TO STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION AND STATE STANDARD 780001.

All dimensions are in millimeters (inches) unless otherwise shown.

REVISIONS		ILLINOIS DEPARTMENT OF TRANSPORTATION		
NAME	DATE	ILLINO	IS DEPARTMENT OF TRANSPORTATION	
	03-19-90			
T, RAMMACHER	10-27-94		DISTRICT ONE	
ALEX HOUSEH	10-09-96			
ALEX HOUSEH	10-17-96		TYPICAL PAVEMENT	
T. RAMMACHER 01-06-00		MARKINGS		
			MALVINGS	
		SCALE: NONE	DRAWN BY CADD	

ALE: NONE DATE: 2/15/2006

DRAWN BY CADD TC-13

CONTRACT NO.

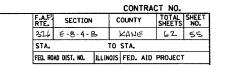
COUNTY

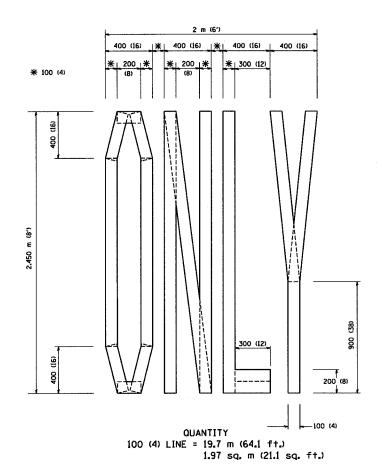
SECTION

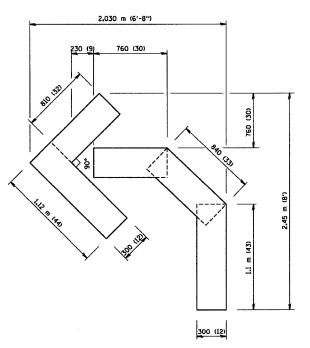
TOTAL SHEET SHEETS NO.

DATE NAME SCALE NAME

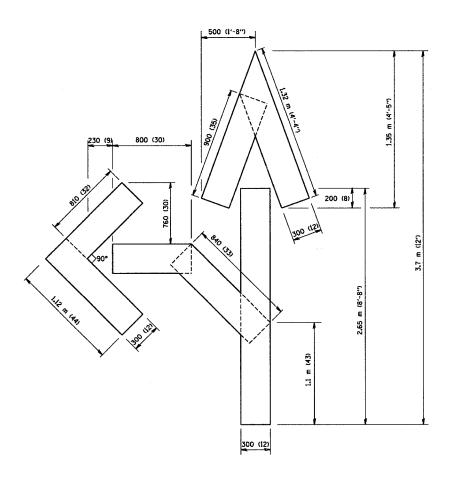
REVISION DATE: 01/06/00







QUANTITY 100 (4) LINE = 13.9 m (45.5 ft.) 1.39 sq. m (15.2 sq. ft.)



OUANTITY 100 (4) LINE = 25.3 m (82.5 ft.) 2.53 sq. m (27.5 sq. ft.)

All dimensions are in millimeters (inches) unless otherwise shown.

REVISIONS		
NAME	DATE	
T. RAMMACHER	09/18/94	
J. OBERLE	06/01/96	
T. RAMMACHER	06/05/96	
T. RAMMACHER	11/04/97	
T. RAMMACHER	03/02/98	
E. GOMEZ	08/28/00	
<u> </u>		

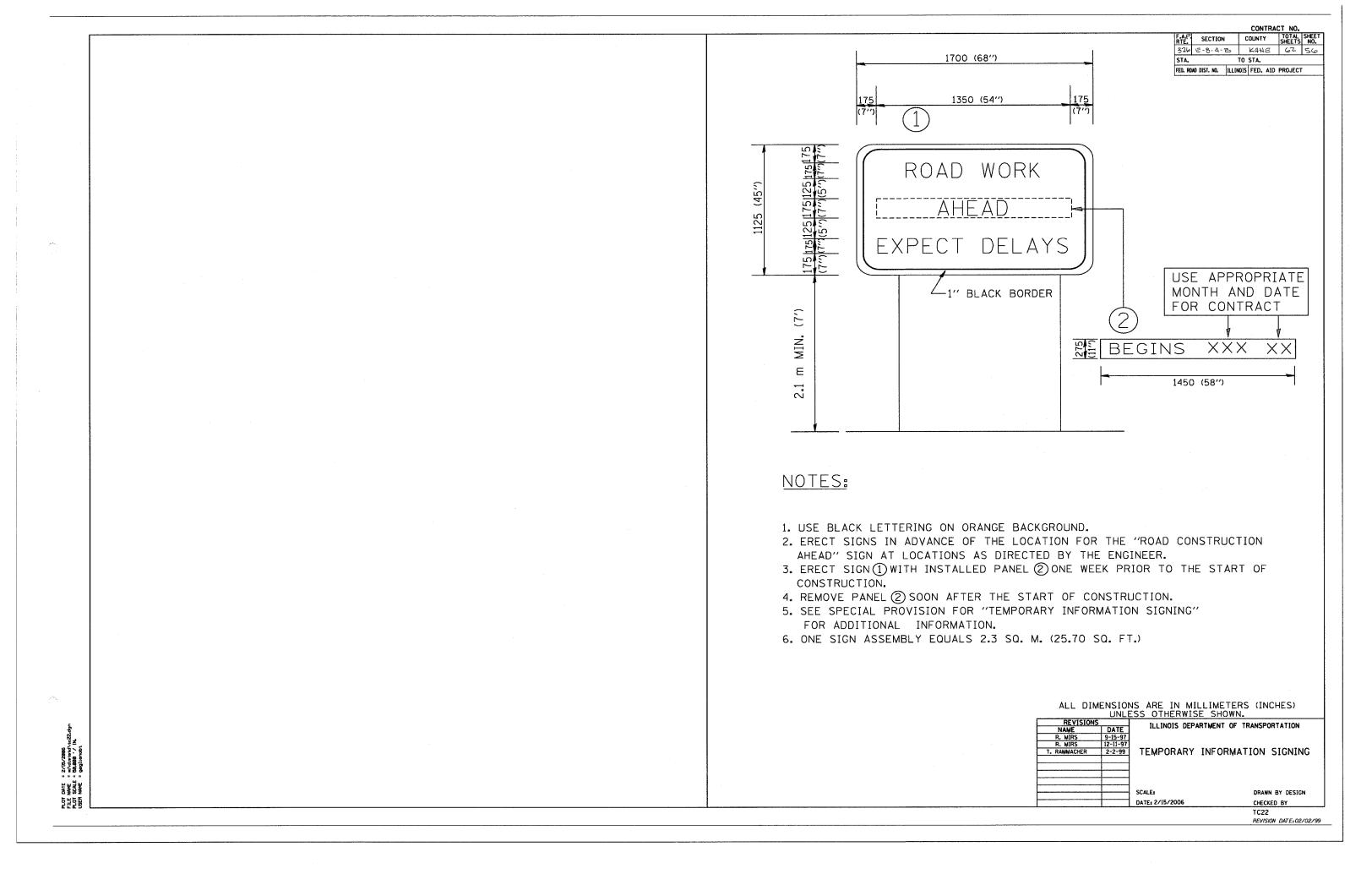
ILLINOIS DEPARTMENT OF TRANSPORTATION

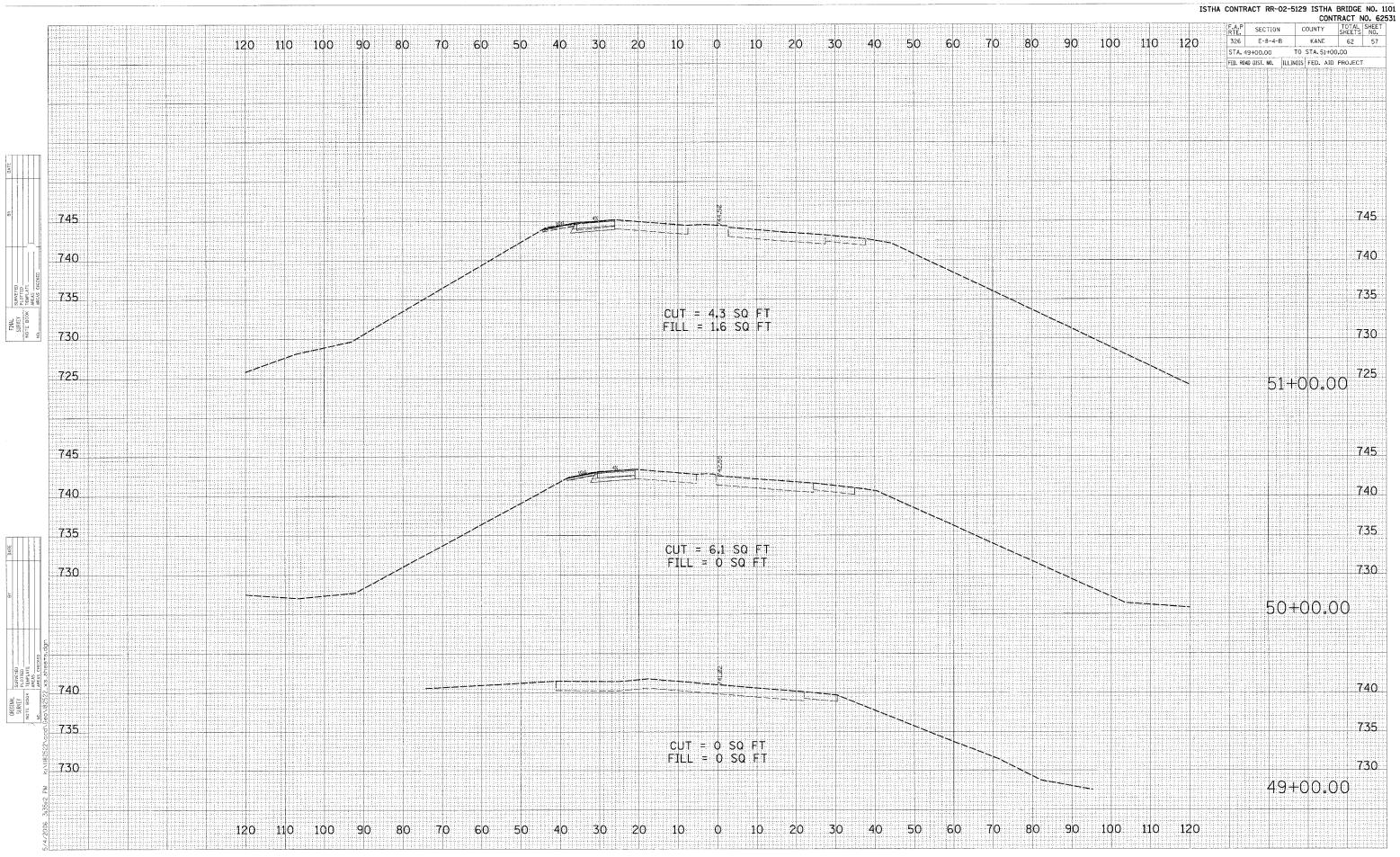
PAVEMENT MARKING LETTERS AND SYMBOLS FOR TRAFFIC STAGING

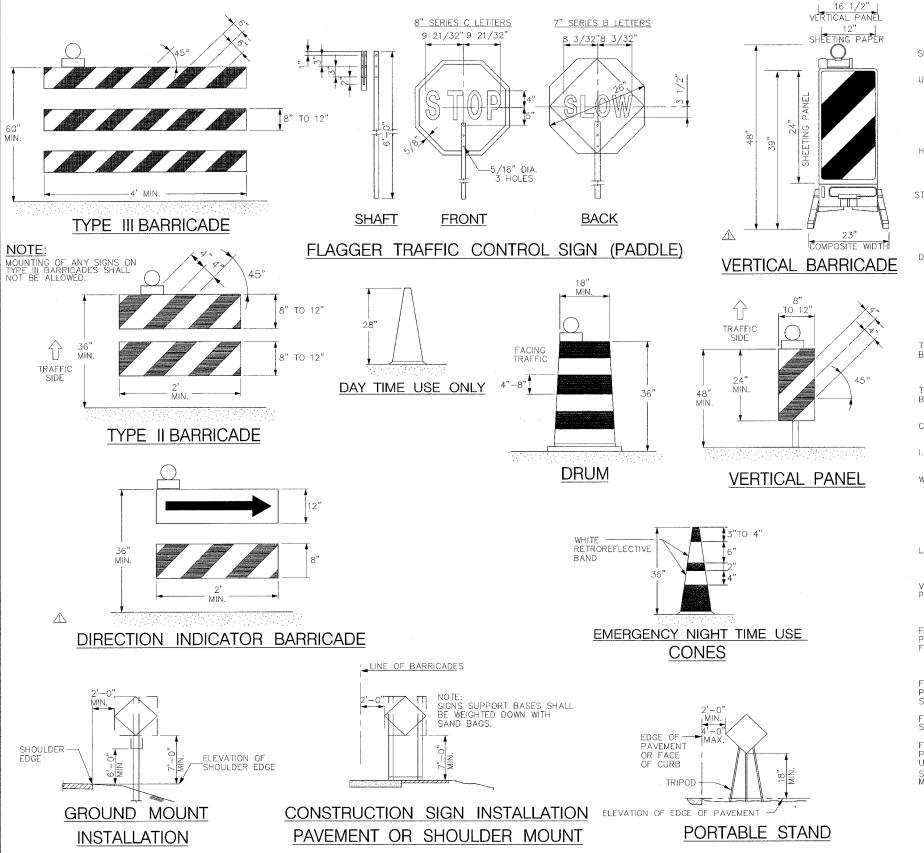
SCALE: NONE DATE: 2/15/2006 DRAWN BY CAD CHECKED BY TC-16

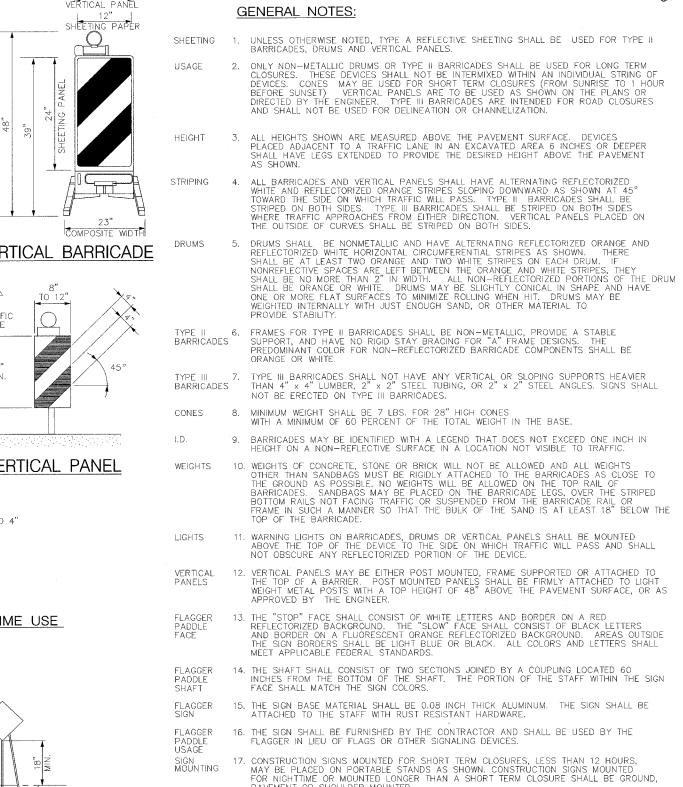
REVISION DATE: 08/28/00

PLOT DATE = 2/15/2006 FILE WAME = #14/1stsxd/tc[6.dgn PLOT SCALE = 49,9999 '/ [N.









APPROVED OHIEF ENGINEER DATE 5-28-2004





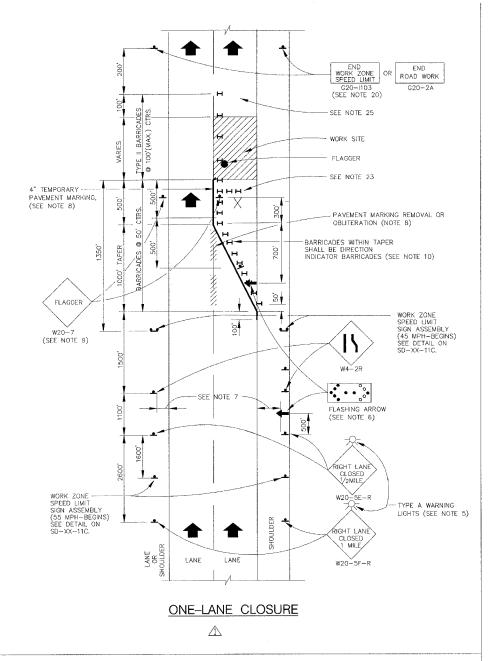


REVISIONS			STANDARD SD 04-10A	
NO.	DATE	DESCRIPTION		DRAWING NO.
1	6/1/04	CORRECTIONS		E1
		A A A A A A A A A A A A A A A A A A A	STANDARD PROTECTIVE DEVICES	OF

19. ALL MOT DEVICES AND SIGN SUPORTS SHALL BE CERTIFIED AS MEETING THE REQUIRED

18. MOUNTING OF ANY SIGNS ON TYPE III BARRICADES SHALL NOT BE ALLOWED.

TEST LEVEL OF NCHRP REPORT 350.



GENERAL NOTES

1 FOR DETAILS OF PROTECTIVE DEVICES AND SIGNS SEE APPLICABLE TOLLWAY STANDARDS DRAWINGS.

IF CLOSURES ARE EXPECTED TO PRODUCE TRAFFIC BACKUPS EXTENDING BEYOND THE FIRST WARNING SIGN SHOWN ON THE DETAILS, ADDITIONAL UPSTREAM SIGNS SHALL BE PLACED SO THAT THE TRAFFIC CONTROL ZONE ENCOMPASSES THE ANTICIPATED BACKUP ZONE

ANTICIPATED BACKUP ZONE.

ANTICIPATED BACKUP ZONE.

3. LONGITUDINAL DIMENSIONS MAY BE ADJUSTED SLIGHTLY TO FIT FIELD CONDITIONS.

4. THESE DETAILS ALSO APPLY TO OPPOSITE HAND LANE CLOSURES BY CHANGING SIGN LEGENDS AND ARROW DIRECTIONS TO INDICATE THE APPROPRIATE CLOSURE.

5. FOR NIGHT TIME CLOSURES, ONE TYPE A WARNING LIGHT SHALL BE INSTALLED ABOVE EACH OF THE 1 MILE AND 1/2 MILE ADVANCE WARNING SIGNS. FOR DAYLIGHT—ONLY CLOSURES, THE LIGHTS MAY BE OMITTED AND A MINIMUM OF 18" X 18" ORANGE WARNING FLAG AFFIXED TO THE FIRST SIGN ONLY.

6. FOR ANY LANE CLOSURE, FLASHING ARROW BOARDS SHALL BE REQUIRED AND IN OPERATION AT ALL TIMES. THE FLASHING ARROW BOARD IN ADVANCE OF THE TAPER SHALL BE PROTECTED WITH THREE TYPE II BARRICADES AT 50' O.C.

7. CONSTRUCTION SIGNS SHALL GENERALLY BE POST—MOUNTED OR ATTACHED TO PORTABLE SUPPORTS AND SHALL BE INSTALLED 8" TO 12' FROM ADJACENT TRAVEL LANE WHEREVER POSSIBLE, IN NO CASE SHALL SIGNS BE LOCATED TO PROVIDE LESS THAN 2' CLEARANCE BETWEEN EDGE OF SIGN AND ADJACENT TRAVEL LANE.

PROVIDE LESS THAN 2' CLEARANCE BETWEEN EDGE OF SIGN AND ADJACENT TRAVEL LANE.

8. PAVEMENT MARKING TAPE AND REMOVAL OR OBLITERATION OF EXISTING MARKINGS SHALL BE REQUIRED WHEN THE CLOSURE TIME EXCEEDS FOUR DAYS. THIS WORK SHALL BE MEASURED AND PAID FOR SEPARATELY.

9. WHEN A FLAGGER IS NOT ON STATION, THE FLAGGER AHEAD SIGN SHALL BE PROMPTLY REMOVED, COVERED OR TURNED TO FACE AWAY FROM TRAFFIC. SEE NOTE 14 REGARDING MOVING OPERATIONS

10. DIRECTION INDICATOR BARRICADES SHALL BE USED IN LANE TAPERS.

11. THE CONTRACTOR SHALL PROVIDE SUITABLE FRAMING TO SUPPORT THE GUIDE SIGN. THE ADEQUACY OF THE SUPPORT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.

12. FOR CLOSURES OTHER THAN SHORT TERM (SUNRISE TO ONE HOUR BEFORE SUNSET). THE MINIMUM HEIGHT OF THE SIGN FROM SHOULDER ELEVATION SHALL BE 7'-0'.

13. CONES MAY BE USED IN LIEU OF BARRICADES IN THE BUFFER AND WORK AREAS, WHEN THE CLOSURE IS FOR MAINTENANCE OPERATIONS.

END WORK ZONE SPEED LIMIT OR ROAD WORK G20-2A SEE NOTE 25 WORK SITE FLAGGER SEE NOTE 23 4" TEMPORARY PAVEMENT MARKING (SEE NOTE 8) FLAGGER (SEE NOTE 9) PAVEMENT MARKING REMOVAL OR OBLITERATION (SEE NOTE 8) FLASHING ARROW (SEE NOTE 6) ... - WORK ZONE - WORK ZONE SPEED LIMIT SIGN ASSEMBLY (45 MPH-BEGINS) SEE DETAIL ON SD-XX-11C. SEE -₩ ® NOTE BARRICADES WITHIN TAPERS SHALL BE DIRECTION INDICATOR BARRICADES (SEE NOTE 10) FLASHING ARROW (SEE NOTE 6) -PAVEMENT MARKING REMOVAL OR OBLITERATION (SEE NOTE 8) EFT LANES **→** 1/2MILE LIGHTS (SEE NOTE 5) WORK ZONE
SPEED LIMIT
SIGN ASSEMBLY
(55 MPH-BEGINS)
SEE DETAIL ON
SD-XX-11C. LEFT LANES LANE 3 TWO-LANE CLOSURE A

14. WHENEVER WORKERS ARE PRESENT, ONE WORK ZONE SPEED LIMIT SIGN ASSEMBLY WITH A 45 MPH POSTED SPEED SHALL BE PLACED ADJACENT TO THE OPEN LANE AT A DISTANCE OF 1000' MINIMUM TO 2,500' MAXIMUM IN ADVANCE OF WORKERS THROUGHOUT THE LANE CLOSURE. MOVING OPERATIONS WILL REQUIRE CONTINUOUS ADJUSTMENT OF THE SIGN ASSEMBLY LOCATION TO MAINTAIN THE ABOVE INTERVAL.

15. AN ADDITIONAL SIGN ASSEMBLY SHALL BE PLACED 500' BEYOND THE LAST ENTRANCE RAMP FOR EACH INTERCHANCE THAT FALLS WITHIN THE 2,500'.

16. THE SIGN ASSEMBLY SHALL BE PLACED NO CLOSER THAN 500' TO ANY OTHER SIGN.

17. THE SIGN ASSEMBLY SHALL NOT BE UTILIZED WHEN WORKERS ARE BEHIND A TEMPORARY (MOVABLE BARRIER) WALL.

18. THE SIGN ASSEMBLY SHALL BE PROMPTLY REMOVED OR COVERED WHEN WORKERS ARE NOT PRESENT.

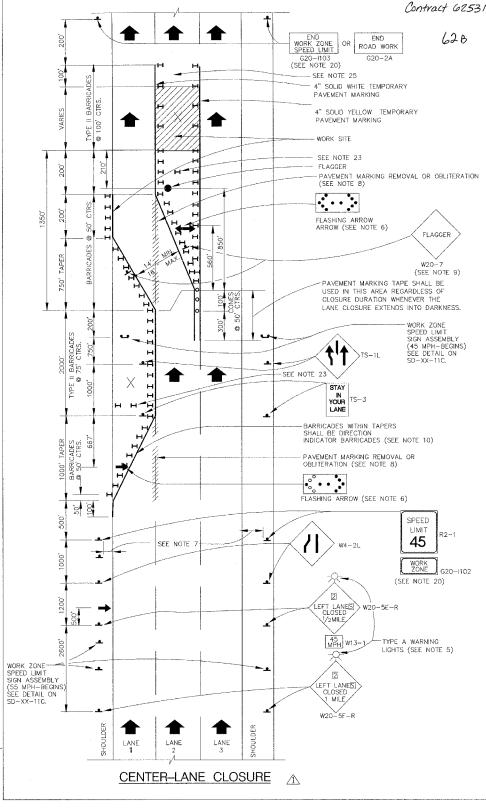
BARKHER) WALL.

18. THE SIGN ASSEMBLY SHALL BE PROMPTLY REMOVED OR COVERED WHEN WORKERS ARE NOT PRESENT.

19. ALL CONFLICTING SPEED LIMIT SIGNS SHALL BE COVERED OR REMOVED.

20. SIGNS WITH G20-2A, OR AND G20-103 SHALL BE IN PLACE WHEN THE SIGN ASSEMBLY (WORK ZONE SPEED LIMIT SIGN) IS IN THESE GROUPS SHALL BE IN PLACE WHEN THE SIGN ASSEMBLY (WORK ZONE SPEED LIMIT SIGN) IS IN THESE GROUPS SHALL BE IN PLACE WHEN THE SIGN ASSEMBLY (WORK ZONE SPEED LIMIT SIGN) IS IN THESE GROUPS SHALL BE IN PLACE WHEN THE SIGN ASSEMBLY (WORK ZONE SPEED LIMIT SIGN) IS IN THE SIGN ASSEMBLY (WORK ZONE SPEED LIMIT SIGN) IS IN THE SIGN ASSEMBLY (WORK ZONE SPEED LIMIT SIGN) IS IN THE SIGN ASSEMBLY (WORK ZONE SPEED LIMIT SIGN) IS IN THE SIGN ASSEMBLY (WORK ZONE SPEED LIMIT SIGN) IS IN THE SIGN ASSEMBLY (WORK ZONE SPEED LIMIT SIGN) IS IN THE SIGN ASSEMBLY (WORK ZONE SPEED LIMIT SIGN) IS IN THE SIGN ASSEMBLY (WORK ZONE SPEED LIMIT SIGN) IS IN THE SIGN ASSEMBLY (WORK ZONE SPEED LIMIT SIGN) IS IN THE SIGN ASSEMBLY (WORK ZONE SPEED LIMIT SIGN) IS IN THE SIGN ASSEMBLY (WORK ZONE SPEED LIMIT SIGN) IS IN THE SIGN ASSEMBLY (WORK ZONE SPEED LIMIT SIGN) IS IN THE SIGN ASSEMBLY (WORK ZONE SPEED LIMIT SIGN) IS IN THE SIGN ASSEMBLY (WORK ZONE SPEED LIMIT SIGN) IS IN THE SIGN ASSEMBLY (WORK ZONE SPEED LIMIT SIGN) IS IN THE SIGN ASSEMBLY (WORK ZONE SPEED LIMIT SIGN) IS IN THE SIGN ASSEMBLY (WORK ZONE SPEED LIMIT SPEED LIMIT SIGN ASSEMBLY (WORK ZONE SPEED LIMIT SIGN) IS UP. THESE SIGNS SHALL ALSO BE REMOVED OR COVERED WHEN THE SIGN ASSEMBLY IS REMOVED OR COVERED, UNLESS STILL REQUIRED BY THE MAINTENANCE OF TRAFFIC PLAN.

21. BARRICADES ARE TO BE LOCATED AT JOINT LINE WHEN WORK AREA EXTENDS UP TO JOINT UNLESS OTHERWISE



22. SEE MAINTENANCE OF TRAFFIC DRAWINGS FOR ADDITIONAL SIGNING IN THIS AREA.

23. PLACE CHECK BARRICADES IN ACCORDANCE WITH SECTION 1001.6.2 OF THE STANDARD SPECIFICATIONS.

24. REFER TO SHEET 2 OF 2 OF THIS STANDARD FOR SIGNAGE IN ADVANCE OF 1 MILE. 25. WHEN THE CLOSURE EXTENDS A MINIMUM 2000 FEET PAST THE LAST WORKER, AND THE WIDTH OF THE OPEN LANES HAVE NOT BEEN ALTERED, A WORK ZONE SPEED LIMIT SIGN ASSEMBLY (55-RESUMES) SHALL BE PLACED AT 1/2 MILE INTERVALS UNTIL THE END OF THE CLOSURE, OR THE NEXT WORK OPERATION.

SHEET 1 OF 2

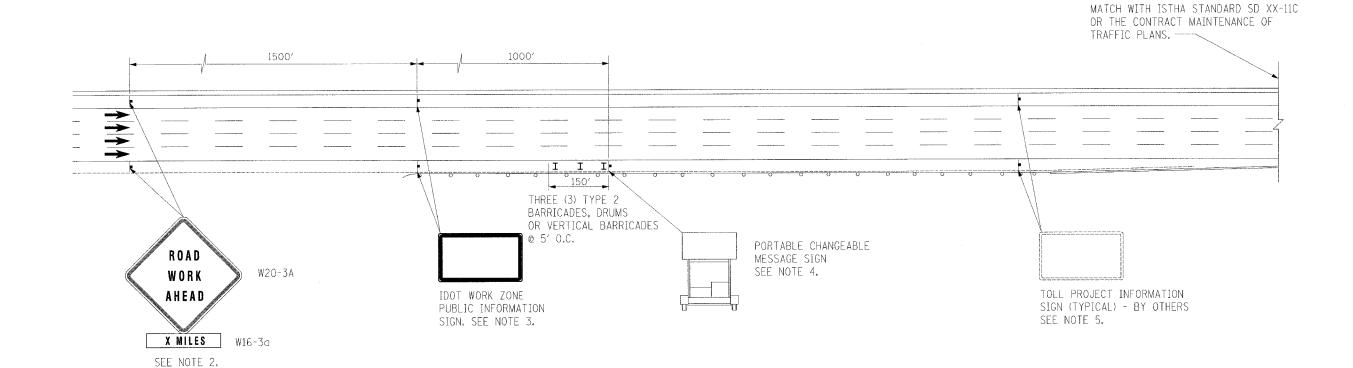
leff Roler DATE 5-21-2004 APPROVED





THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY 2700 OGDEN AVENUE DOWNERS GROVE, ILLINOIS 60515

REVISIONS			STANDARD SD 04-20A	
NO.	DATE	DESCRIPTION	STANDARD 3D 04-20A	DRAWING NO.
\triangle	6/1/04	CORRECTIONS		E10
			LANE CLOSURE DETAILS	OF.



NOTES

- 1. THE ADVANCE SIGNAGE SHOWN ON THIS STANDARD SHALL APPLY ANY TIME THE CONTRACTOR CLOSES ONE OR MORE LANES, OR IS REQUIRED TO SHIFT THE LANE ALIGNMENT. THE 'ROAD WORK AHEAD'SIGNS, WORK ZONE PUBLIC INFORMATION SIGNS AND PORTABLE CHANGEABLE MESSAGE ARE STATIONARY.
- 2. THE ROAD CONSTRUCTION AHEAD SIGN (W20-1A, WITH W16-3a SUPPLEMENTAL PLATE) SHALL BE LOCATED UP TO 5 MILES IN ADVANCE OF THE PROJECT LIMITS, WITH THE LOCATION BEING DETERMINED BY THE ENGINEER.
- 3. THE WORK ZONE INFORMATION SIGN IS 60" WIDE BY 48" HIGH. THE CONTRACTOR SHALL OBTAIN THE CAMERA-READY ARTWORK REQUIRED FOR THE SIGN MESSAGE BY CONTACTING IDOT'S CENTRAL BUREAU OF OPERATIONS (217-782-2076).
- 4. THE PORTABLE CHANGEABLE MESSAGE SIGN SHALL BE USED TO DISPLAY THE STATUS OF LANE WITHIN THE CONTRACT LIMITS. THE PRIMARY MESSAGES SHALL BE:

 "RIGHT LANE(S) CLOSED" / "x MILES AHEAD"

 "LEFT LANE (S) CLOSED" / "x MILES AHEAD"

 "LANE(S) SHIFT" / "x MILES AHEAD"

 "ALL LANES OPEN".

 THE PORTABLE CHANGEABLE MESSAGE SIGN MAY BE MOVED TO THE MEDIAN SHOULDER WHEN THE

LANE CLOSURES ARE ON THE LEFT, PROVIDED THE EXISTING SHOULDER WIDTH IS ADEQUATE.

5. THE TOLLWAY WILL FURNISH AND INSTALL STATIC PROJECT INFORMATION SIGNS IN ADVANCE, THROUGH AND AT THE END OF THE WORK ZONE. THESE SIGNS WILL BE INSTALLED ALONG THE OUTSIDE SHOULDER WITH THE ADVANCE SIGNS LOCATED BETWEEN THE PORTABLE CHANGEABLE AGE SIGN AND THE "ROAD WORK - 1 MILE AHEAD" SIGN. THE ENGINEER AND CONTRACTOR SHALL COORDINATE WITH THE AUTHORITY REGARDING THE LOCATION OF THESE SIGNS, AND NOTIFY THE AUTHORITY OF ANY DAMAGE TO THE SIGNS OR SUPPORTS.

SHEET 2 OF 2

Jeff Wally

APPROVED CHIEF ENGINEER DATE 5-28-2004





THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY

2700 OGDEN AVENUE

DOWNERS GROVE ILLINOIS 60515

REVISIONS			STANDARD SD 04-20A	
v O.	DATE	DESCRIPTION	31ANDARD 3D 04-20A	
			LANE CLOSURE DETAILS ADVANCE SIGNAGE	

E10A OF