

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
F.A.J.8966	SEC. 97-00208-01-05	MADISON	34	12
FED. ROAD DIST. NO. 7		ILLINOIS	PROJECT	

Note:  
The Bridge Slab shall be placed in one continuous operation, or sequentially as described below.

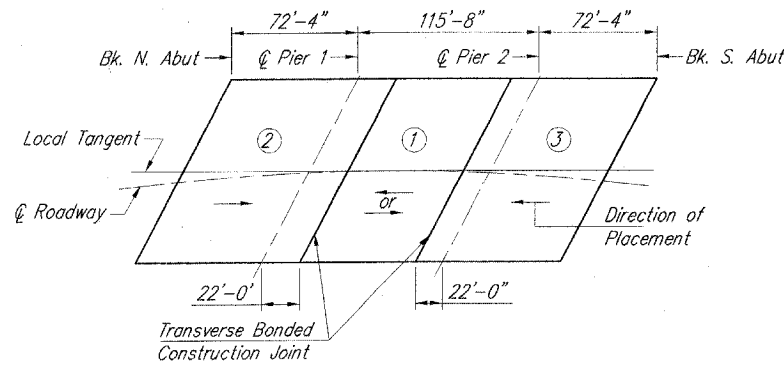
**TOTAL BILL OF MATERIAL**

ITEM	UNIT	SUPER.	SUB.	TOTAL
Structure Excavation	Cu. Yd.	-	377	377
Bar Splicers	Each	122	-	122
Floor Drains	Each	16	-	16
Concrete Superstructure	Cu. Yd.	546.2	-	546.2
* Protective Coat	Sq. Yd.	2000	-	2000
Elastomeric Expansion Bearings, Type I	Each	14	-	14
Concrete Structures	Cu. Yd.	-	399.0	399.0
Furnishing & Erecting Structural Steel	L. Sum	1	-	1
Stud Shear Connectors	Each	3549	-	3549
Reinforcement Bars, Epoxy Coated	Pound	147130	45250	192380
Furnishing Steel Piles HP12x74	Foot	-	3551	3551
Furnishing Steel Piles HP14x102	Foot	-	2412	2412
Driving Steel Piles	Foot	-	5963	5963
Test Pile Steel HP12x74	Each	-	1	1
Test Pile Steel HP14x102	Each	-	1	1
Metal Shoes	Each	-	48	48
Name Plates	Each	1	-	1
Slopewall 4 inch	Sq. Yd.	-	997	997
Bridge Deck Grooving	Sq. Yd.	1773	-	1773
Porous Granular Embankment	Cu. Yd.	-	272	272
Temporary Sheet Piling	Sq. Ft.	-	2987	2987

\* Quantity for Protective Coat includes bridge deck, and top and inside faces of parapets.

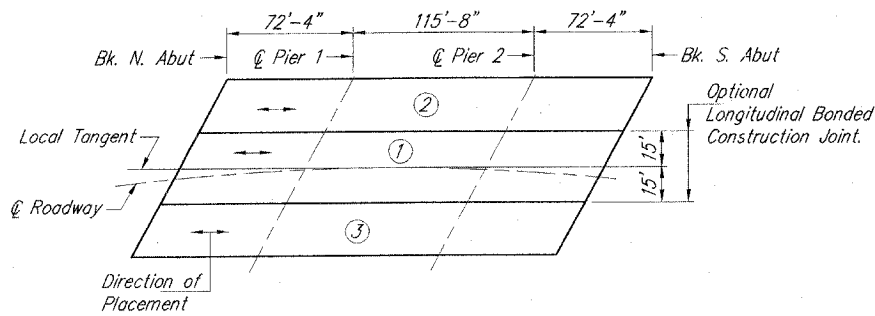
**GENERAL NOTES**

- See Sheet 18 of 18 for Borings 5A & 6A. Data for other project borings are included in the Proposal Booklet.
- Fasteners shall be high strength bolts. Bolts 3/4"  $\phi$ , open holes 13/16"  $\phi$ , unless otherwise noted.
- Calculated weight of Structural Steel = 167,171 lbs. (M270 Gr50) and 132,794 (M270 GR. 36)
- The inorganic zinc rich primer / Acrylic / Acrylic Point System shall be used for shop and field painting of new structural steel except where otherwise noted. The color of the final Acrylic finish coat shall be Gray, Munsel No. 5B 7/1. See Special Provision "Cleaning and Painting New Metal Structures".
- Field welding of construction accessories will not be permitted girders.
- Anchor bolts shall be set before bolting diaphragms over supports.
- The structural steel bearing plates of the Elastomeric Bearing Assembly shall conform to the requirements of AASHTO M 270 Grade 50.
- The main load carrying member components subject to tensile stress shall conform to the Supplemental Requirements for Notch Toughness Zone 2. These components are the tension flanges, webs and all splice plate material except fill plates.
- Reinforcement bars shall conform to the requirements of AASHTO M31 or M53 Grade 60.
- Slope wall shall be reinforced with welded wire fabric, 6" x 6" - W4.0 x W4.0, weighing 58 lbs. per 100 sq. ft.
- The embankment configuration shown shall be the minimum embankment that must be constructed prior to construction of the abutments.
- Bearing seat surfaces shall be constructed or adjusted to the designated elevations within a tolerance of 1/8 inch. Adjustment shall be made either by grinding the surface or by shimming the bearing. Two 1/8" adjusting shims, of the dimensions of the top plate at the pier locations and of the dimensions of the bottom bearing plate at the abutment locations, shall be provided for each bearing in addition to all other plates or shims.
- The contractor shall drive one HP14X102 test pile at the north abutment and one HP12X74 test pile at Pier 2 as directed by the Engineer before ordering the remainder of the piles.
- The contractor shall provide the following minimum temporary clearances at any necessary falsework, bracings or forms required to construct the highway overpass structure:  
Vertical: 22'-0" above Top of High Rail  
Horizontal: 13'-0" from centerline of track  
The only exceptions will be the safety railings for the temporary sheet piling at Piers 1 & 2. See Special Provision for Protection of Railroad Interest.
- All construction joints shall be bonded.



**SLAB PLACING PLAN**

(Without Longitudinal Bonded Construction Joints)

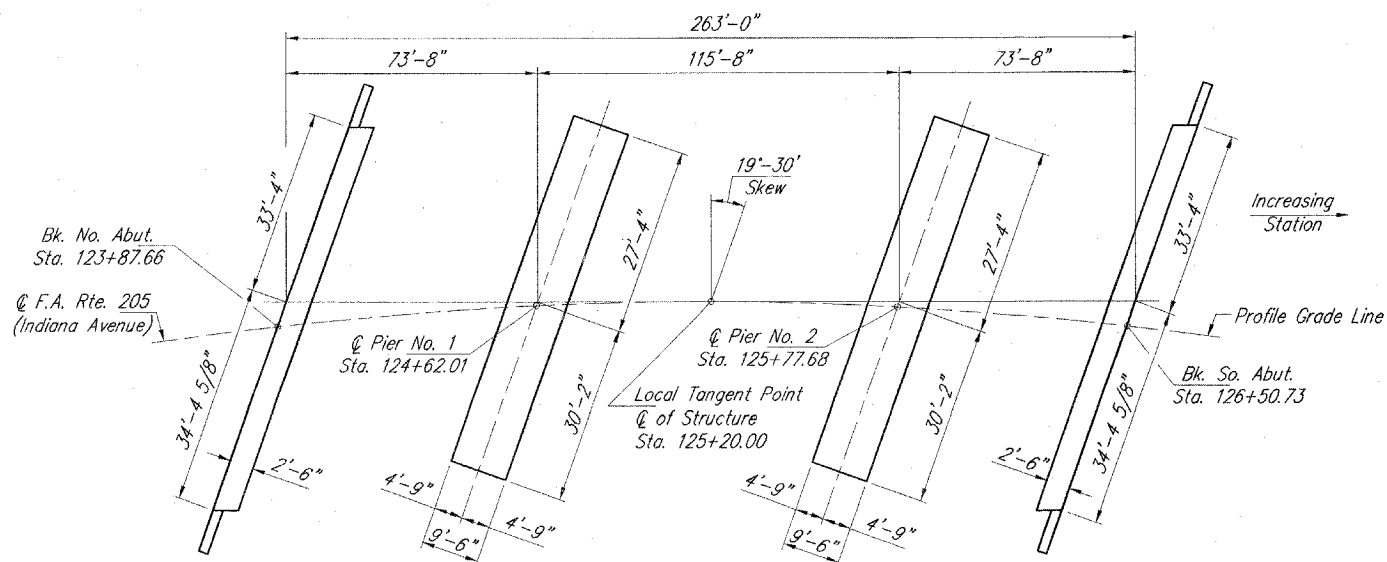


**SLAB PLACING PLAN**

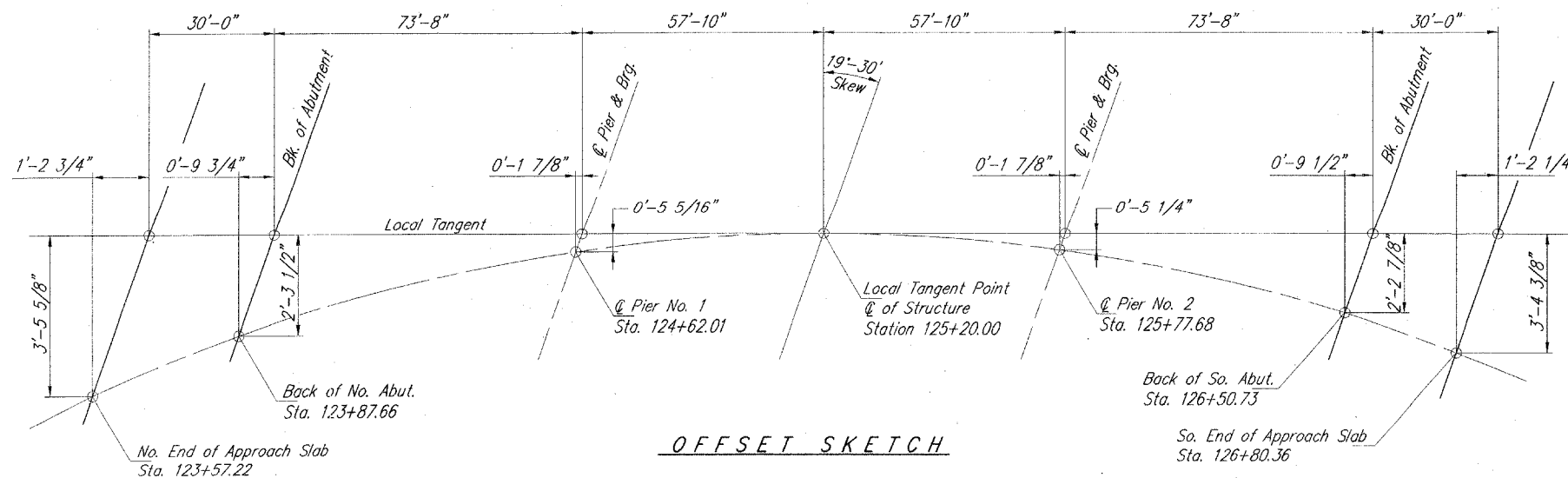
(With Longitudinal Bonded Construction Joints)

**SLAB PLACING SEQUENCE**

- Areas 1 thru 3 shall be placed consecutively and in the direction shown. If the deck pour is stopped for the day or more of the transverse joints as shown, the next pour shall not be made until both of the following requirements are met:  
a.) At least 72 hours shall have elapsed from the end of the previous pour.  
b.) The concrete shall have attained a minimum modulus of rupture of 650 psi or a minimum compressive strength of 3500 psi.
- The contractor may propose an alternative casting sequence, subject to approval by the engineer.



**FOOTING LAYOUT**



**OFFSET SKETCH**

Corporate License Number 184-001-084

**GENERAL NOTES & MISC. DETAILS**  
INDIANA AVENUE over NORFOLK SOUTHERN RAILWAY  
SEC. 97-00208-01-GS  
STATION 125+20.00  
ALTON, ILLINOIS  
STR. NO. 060-6110



JOB NO.  
9551008  
DATE  
05/12/05

10/16/96  
 3/10/98  
 3/31/05  
 11/20/05  
 7/11/04 DAB