STRUCTURE GEOTECHNICAL REPORT US 52 / IL 64 / IL 84 IL 84 RETAINING WALL SN 008-7001, SECTION 104B-2 PTB 158/018 IDOT JOB D-92-001-11 CARROLL COUNTY, ILLINOIS

for Parsons Transportation Group, Inc. 10 South Riverside Plaza, Suite 400 Chicago, IL 60606 (312) 930-5100

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> > Original: August 14, 2014 Revised: November 25, 2014



Technical Report Documentation Page

| 1. Title and Subtitle Structure Geotechnical Report | | 2. Report Date November 25, 2014 |
|--|--|---|
| IL 84 Retaining Wall | | 3. Report Type ⊠ SGR □ RGR □ Draft ⊠ Final ⊠ Revised |
| 4. Route / Section / County IL 84/104B-2/ Carroll | | 5. IDOT Job / Contract No. Job D-92-001-11 |
| 6. PTB / Item No. 158/18 | 5. Existing Structure Number(s) N/A | 6. Proposed Structure Number(s) S.N. 008-7001 |
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| A new retaining wall will be roadway. This report provides proposed retaining wall. The p completed 27 structure borings At some locations, below the g sandy loam, clay loam, and/or colluvium over up to 10 feet of to 641 feet elevation (10 to 34 f Due to soil and rock condition wall. The report provides soil p | constructed to accommodate widening s geotechnical recommendations for th proposed wall is a cut wall with maxin along the proposed retaining wall align ground surface, the borings revealed up silty clay. Beneath the fill or surface, th highly weathered shale bedrock. The sh feet bgs). and constructability, we recommend dri arameters for the design of the wall. | g of IL 84 on the east side of the he design and construction of the num total height of 30 feet. Wang nent. to 10 feet of fill consisting of sand, he borings revealed up to 34 feet of ale bedrock was encountered at 599 lled soldier pile wall or tangent pile |
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STRUCTURE GEOTECHNICAL REPORT US 52 / IL 64 / IL 84 IL 84 RETAINING WALL SN 008-7001, SECTION 104B-2 PTB 158/018 IDOT JOB D-92-001-11 FOR PARSONS TRANSPORTATION GROUP, INC.

1.0 INTRODUCTION

This report presents the results of Wang Engineering, Inc. (Wang) subsurface investigation, laboratory testing, and geotechnical evaluation for a new retaining wall proposed along the east side of IL Route 84 (IL 84), on the east flank of the Mississippi River, just north of City of Savanna, Illinois. The purpose of our investigation was to characterize the site subsurface soil and groundwater conditions and provide geotechnical analyses and recommendations for the design and construction of the new wall. A *Site Location Map* is presented as Exhibit 1.

1.1 Proposed Structure

The new retaining wall (SN 008-7001) is basically a cut wall retaining existing sloping ground immediately east of IL 84 roadway. The proposed retaining wall be 1565'-0" long measured along wall's front face from Station 709+65.66 to Station 725+08.27 and its maximum total height of 30'-0". TSL plan dated August 13, 2014 is included in Appendix C.

2.0 SITE CONDITIONS AND GEOLOGICAL SETTING

The project area is located in the northwestern section of Carroll County just north of Savanna, Illinois. On the USGS *Savanna Quadrangle 7.5 Minute Series* map, the proposed Retaining Wall is located in the NE ¹/₄ of Section 4, Tier 24 North, and Range 3 East of the 4th Principal Meridian.



The following review of published geologic data, with emphasis on factors that might influence the design and construction of the proposed engineering works, is meant to place the project area within a geological framework and, thus, to confirm the dependability and consistency of the present subsurface investigation results. For the study of the regional geologic framework, Wang considered northwestern Illinois area in general and Carroll County in particular. Exhibit 2 illustrates the *Site and Regional Geology*.

2.1 Physiography

Northwestern Carroll County and a slice of northeastern Jackson County are part of the Wisconsin Driftless Section. It is a low plateau area bordering the outwash-filled valley of the upper Mississippi River (Leighton et al. 1948).

The project is located along the Mississippi River Valley in an unglaciated area of northwestern Illinois. The project lies on the southwestern flank of the gently sloping Wisconsin Arch, and just northwest of the Illinois Basin. Although the Savanna area was not directly impacted by glacial ice, early pre-Illinois glaciers encircled the Savanna area and had a dramatic effect on the region geomorphology. Meltwater from over 400,000 years of glacial retreats and advances established the Mississippi River Valley to its maximum depth by erosion (Richmond et al 1991). After this time, the Mississippi River Valley began to be aggraded by outwash and stratified sand and gravel deposits (Henry Formation); with intermitted periods of erosion during subsequent glacial and interglacial intervals. Modern river deposits are identified as Cahokia Alluvium and occur as stratified silt, clay, and redeposited loess within the river channel, floodplains and backwaters of the Mississippi River. At the time of our investigation the Mississippi River water surface elevation measured 582.5 feet. At the south end of the proposed wall near the bridge, the abrupt ground surface on the Illinois side steps into the valley from 631.2 feet elevation to river level.

2.2 Surficial Cover

The surficial cover is represented by up to 34-foot thick colluvium made up of silty clay to silty loam with little to some angular clasts that rests over the bedrock (Grimley 1997). Along the wall alignment, the surficial cover varies in thickness from 10 to 34 feet.



2.3 Bedrock

The bedrock outcrops along the edge of the Mississippi River Valley on the east (Illinois) side of the project and forms an extended line of rock cliffs known as the Mississippi Palisades (Frankie 2001). Within the river valley, the elevation of the bedrock surface becomes gradually deeper toward the west, such that rock beneath the Iowa side of the main river channel was found at approximately 120 feet below the water surface. The general lithological profile includes Silurian dolostones over Ordovician shale and dolostones.

The Plum River fault zone is mapped approximately 1,000 feet south of the existing bridge. The east-west Plum River fault zone width may measure as much as 4,000 feet. The fault is considered inactive, but the near vertical fault lines with a strike-slip component of movement have resulted in a structure with horsts and grabens and zones of breccia with rotated blocks.

There was no coal mining activity in the bridge area; the nearest active coal mines and coal resources are identified by the Illinois State Geological Survey at approximately 7 miles east from the bridge, just west from the village of Mt. Carroll.

Our subsurface investigation results fit into the local geologic context. The borings drilled in the project area revealed the native sediments consists of silty clay loam to silty loam with trace to some gravel to cobble size angular clasts (Colluvium) resting over weathered to solid shale bedrock. Borings referenced in this report encountered the bedrock at depths ranging from 10.0 to 34.5 feet bgs (elevations of 599.1 to 640.8 feet).

3.0 METHODS OF INVESTIGATION

The following sections outline the subsurface soil and groundwater investigation and laboratory testing performed by Wang.

3.1 Subsurface Investigation

For the proposed retaining wall along IL 84, Wang drilled a total of 27 borings between Station 709+50 and Station 724+50. During Phase 1, Wang drilled Boring RWB-01 in November 2011. During Phase 2, Wang drilled 26 borings designated as 84-RWB-01 through 84-RWB-22 including the A and B borings, from both the proposed wall alignment and from the IL 84 roadway where the access was problematic. Drilling and sampling for Phase 2 was



performed between December 2013 and July 2014. Borings 84-RWB-01, 84-RWB-04, and 84-RWB-05 were performed on the pavement along IL 84. The borings were drilled to depths ranging from 17.5 to 72.0 feet below ground surface (bgs). However, due to the sloped and wooded area with scattered debris from demolished cabins, it was difficult to gain access along the new retaining wall alignment to the proposed boring location. In order to do so, we had to cut trees and use a small bulldozer to perform the borings. Also because of these difficulties we performed some of the borings at an offset location, from the IL 84 roadway.

Truck- and ATV-mounted drilling rigs were used to complete the structure borings. Drilling was conducted with hollow stem augers to advance and maintain an open borehole. Soil sampling was performed according to AASHTO T 206, "*Penetration Test and Split Barrel Sampling of Soils*." The soil was sampled at 2.5-foot intervals to a depth of 30 feet bgs and at 5.0-foot intervals to the bedrock. The bedrock was cored in 5- and 10-foot runs up to 40 feet. Samples collected from each sampling interval were placed in sealed glass jars. As-drilled northing, easting, and elevations were surveyed by Wang using a mapping grade Trimble GPS survey system, capable of +/- 4 inches accuracy for vertical and lateral directions. Boring locations and elevations data are shown in the *Boring Logs* (Appendix A). The as-drilled locations are shown in the *Boring Location Plan* (Exhibits 4).

Field boring logs prepared and maintained by a Wang engineer or geologist included lithological descriptions, visual-manual soil classifications, results of Rimac or pocket penetrometer unconfined compression tests, and Standard Penetration Tests (SPT) recorded as blows per 6 inches of penetration.

Groundwater levels were measured while drilling and at the completion of drilling operations. Upon completion, the boreholes were backfilled with bentonite chips mixed with soil cuttings. The pavement surface was restored using cold asphalt patch.

3.2 Laboratory Testing

Samples obtained in the field were transported to our in-house laboratory in Lombard, Illinois. The testing program included moisture content (AASHTO T 265) on all soil samples. Particlesize gradation (AASHTO T 88) and Atterberg limits (T 89 and T 90) were performed on



selected soil samples. Tested samples were classified according to the IDH and AASHTO classification systems. Density-unit weight (ASTM D 7263) and uniaxial compressive strength tests were performed on selected rock cores. Field visual-manual classifications were also verified in the laboratory. The results of the laboratory testing program are shown in the attached *Boring Logs* (Appendix A) and in *Subgrade Data Profile* (Exhibit 4).

The soil samples will be retained in our laboratory for 60 days following the final report submittal. After that, the samples will be discarded unless a specific written request is received as to their disposition.

4.0 RESULTS OF FIELD AND LABORATORY INVESTIGATIONS

Detailed descriptions of the soil conditions encountered during the subsurface investigation are presented in the attached *Boring Logs* (Appendix A) and in the *Soil Profile* (Exhibit 3). Please note that strata contact lines represent approximate boundaries between soil types. The actual transition between soil types in the field may be gradual in horizontal and vertical directions.

4.1 Soil and Bedrock Conditions

Up to 12-inch thick black silty loam topsoil was measured in the borings drilled within the wooded area or outside of the roadway pavement. Borings 84-RWB-01, 84-RWB-04, and 84-RWB-05 were drilled through IL 84 roadway pavement and recorded 12-inch thick asphalt pavement. Borings 84-RWB-16, 84-RWB-17, and 84-RWB-18 were drilled through a driveway and encountered 3- to 4- inch thick asphalt. The subsurface soils encountered below the pavement and/or topsoil are described in descending order as follows:

- a) Man-made ground consisting of up to 10.5-foot thick granular and/or cohesive fill was encountered in Borings 84-RWB-01, 84-RWB-03, 84-RWB-11, 84-RWB-13, 84-RWB-22, and 84-RWB-22A. Granular fill consists of loose to very dense silty loam, sand to gravelly sand characterized by SPT N-values of 5 to 62 blows/foot and moisture content (MC) values of 8 to 28%. The cohesive fill is made up of soft to hard clay loam to gravelly silty clay characterized by unconfined compressive strength (Qu) values of 0.3 to 4.5 tsf and MC values of 11to 22%.
- b) Up to 34-foot thick colluvium deposits overlie the bedrock. The colluvium is made up of 20-foot thick stiff to hard silty clay with little gravel characterized by Qu values of 1.8 to



more than 4.5 tsf, MC values of 9 to 21%; and 14-foot thick of medium dense to very dense sand to gravelly sandy loam with SPT N-value of 15 blow/foot to spoon refusal.

- c) Up to 10.0 feet of weathered shale bedrock, described as hard silty clay to silty loam with shale and mudstone fragments characterized by unconfined compressive strength (Qu) values of more than 4.0 tsf, SPT N-values of 35 blows/foot to sampler refusal, and MC of 8 to 28%;
- d) At 15 to 35 feet bgs, the borings advanced through shale-mudstone interbedded bedrock, of Ordovician Maquoketa Group, characterized by sampler refusal and MC values of 9 to 14%; In our analysis we distinguish two layers as follows
 - 1) Weak shale bedrock was encountered below the weathered bedrock, with thicknesses ranging from 2 to 22 feet characterized by Qu value of 1030 psi, dry unit weight values of 116.7 to 138.1 pcf., weak strength, with very poor to fair rock mass quality, and RQD values of 0 to 72%.
 - 2) Competent shale bedrock was encountered at elevations ranging from 567.2 to 630.6 feet (22.0 to 60.0 feet bgs.) characterized by Qu values of 1130 to 9420 psi, dry unit weight values ranging from 141.7 to 160.0 pcf., moderately strong, poor to excellent rock mass quality with RQD values of 44 to 100%.

4.2 Groundwater Conditions

Groundwater was encountered in Borings 84-RWB-01A and 84-RWB-22 at 8.5 and 13.8 feet bgs and more likely is perched water within the granular layers. In general, borings have not encountered groundwater during our drilling operations. Mississippi River runs approximately 450 feet west from boring location and its water level was recorded at elevations of 582.5 feet.

4.3 Results of laboratory testing

We perform unconfined compressive strength, moisture content, and unit weight tests on selected rock cores samples and the results are shown in Appendix A - *Boring Logs* and are included in Appendix B – *Laboratory Test Results*.

4.4 Seismic Considerations

Seismic design data is not required for wall design. As per 2012 IDOT Bridge Manual, specific need for Seismic Design was not identified.



5.0 ANALYSIS AND RECOMMENDATIONS

5.1 Retaining Wall Type Evaluation

Wang has evaluated possible wall types that can be considered for the support of the proposed cut associated with the roadway widening. The selection of type of wall system should be based on the construction and cost considerations. The proposed retaining wall is basically a cut wall. The boring log data is shown on cross sections at the boring station/off set locations in Appendix D. It should be noted that the borings could not be performed exactly along the wall alignment due to existing ground condition. Since existing ground surface was regraded for providing access to boring locations for drilling, the grade elevation shown on the boring logs may not match with the cross sections.

It is our opinion that drilled soldier-pile wall or a tangent pile wall would be more appropriate considering existing site and soil conditions and constructability. Our evaluation for the selection of wall type at this location is presented below.

Based on cross section drawings provided by Parsons, the proposed retaining wall will retain the existing high slope on the east side of IL 84. The possible wall types that could be considered are cast-in-place concrete cantilever, Mechanically Stabilized Earth (MSE), soldier-pile and steel sheet pile walls with concrete facing.

Since the cast-in-place reinforced concrete and MSE walls are fill wall types, additional open cut excavations into the existing slope or a temporary soil retention system will be required to construct them. This would also require backfilling and longer construction time.

Thus, cantilevered pile wall type such as sheet and soldier pile would be more appropriate for this cut situation. Since the borings show dense to very dense granular soils and very stiff to hard cohesive soils over shale bedrock at 15 to 30 feet bgs it will be difficult to drive steel sheeting through very dense granular and hard cohesive soils. Therefore, it is our opinion that the cantilever steel sheet pile wall is not an appropriate wall system at this site.

Soldier pile and lagging type of retaining wall (S-P Wall) can be considered as a wall installed with a top-down constructed method. Depending on the embedment length required, it may not be difficult to drive soldier piles in existing soils at locations where the wall is not



very high. However, driving H-piles in very dense granular and hard cohesive soils will be very difficult and the required pile embedment depth may not be achieved. Piles will be limited to H-pile section for the driven. Thus, we recommend D*rilled Soldier Pile Retaining Wall*. The drilled shafts will provide more passive resistance and wider section can be used such as wide flange beam (W) section.

Another wall type option could be the tangent pile wall consisting of a single row of tangentially touching drilled reinforced-concrete piles. The reinforcement of each pile may consist of a steel beam or reinforcing bar cage. Lateral deflections can be relatively less compared to S-P Wall. Design parameters and construction recommendations for the selected wall types are presented in the following sections.

For the higher portion of the wall, larger soldier pile section and/or a less spacing, of the piles, or permanent ground anchors (tie backs) will be necessary for additional lateral support and control of deflection. Design of permanent ground anchors should consider corrosion protection. Design and construction should be in accordance with AASHTO LRFD Bridge Design Specifications latest edition. US Department of Transportation, Federal Highway Administration publications FHWA-IF-99-015 and FHWA/Rd-82/047 may also be referred for the design and construction of ground anchors. The soldier piles should also be designed to resist the total vertical component of the ground anchor load. Vertical Soldier pile capacity below the bottom of the excavation can be evaluated from the skin friction considering soil and rock properties given in Tables 1A through 1C.

5.2 Drilled Soldier Pile Wall

Soldier pile and lagging type of retaining wall (S-P Wall) can be considered as a wall installed with a top-down construction method. As discussed earlier, we recommend drilled soldier pile wall.

Based on the soil conditions encountered in borings the geotechnical design parameters shown in Tables 1A through 1C are recommended to be used for the design.

The design of the soldier-pile wall should ignore 3 feet of soil in front of the wall measured from the finished ground surface elevation in providing passive pressure due to excavation required for installation of concrete facing, drainage system and for frost-heave conditions. In



developing the design lateral pressure, the lateral pressure due to construction equipment surcharge loads should be added to the lateral earth pressure. The earth pressure distributions shown in 2012 AASHTO LRFD Bridge Design Specifications should be used. Design considerations should include deflection control at the top of the wall. The recommendations pertaining to site preparation and earthwork are presented in subsequent sections of this report. The plan should show minimum timber lagging thickness of 3 inches.

5.3 **Tangent Pile Wall**

A tangent pile wall consisting of a single row of tangentially touching drilled reinforced concrete shafts can be considered. Lateral movement of this type of wall is relatively small compared to more flexible wall systems. The recommendations for the design of the drilled soldier pile wall including the geotechnical parameters shown in Tables 1A through 1C are also applicable to the tangent pile wall.

| | Tabl | e IA: Recon | nmended Soil | Parameters | | |
|--|-----------------|---|----------------------------------|-----------------------------------|-----------------------------------|------------------------------|
| | | Shear | Strength Prope | rties | Estimated | |
| Soil | Moist Unit | Short T | Serm | Long Term | Lateral Soil Modulus | Estimated Soil Strain |
| Description | Weight (pcf) | Cohesion Cu (psf) 0 1000 1000 4000 | Friction Angle, φ (Degree) | Friction Angle, φ' (Degree) | Parameter (Static), k (pci) | Parameter, _{E50} |
| Existing Fill- Granular | 120 | 0 | 30 | 30 | 20 | - |
| Existing Fill- Cohesive | 120 | 1000 | 0 | 28 | 300 | 0.0085 |
| Medium Stiff to Stiff Cohesive (Qu 0.5 to 1.5 tsf) | 120 | 1000 | 0 | 28 | 300 | 0.0085 |
| Stiff to Hard Cohesive (Qu 1.5 to > 4.0 tsf) | 125 | 4000 | 0 | 31 | 1400 | 0.0045 |
| Loose Granular (N < 10) | 110 | 0 | 28 | 28 | 10 | - |
| Medium Dense Granular (N 10 to 29) | 120 | 0 | 30 | 30 | 40 | - |
| Dense Granular (N 30 to 50) | 130 | | 37 | 37 | 185 | - |
| Very Dense Granular $(N > 50)$ | 135 | | 39 | 39 | 220 | - |

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| | | Shear | Strength Prope | Estimated | | | |
|-----------------------------|-----------------|----------------------|----------------------------------|-----------------------------------|-----------------------------------|----------------------------|--|
| Soil Description | Moist Unit | Short T | erm | Long Term | Lateral Soil Modulus | Estimated Soil Strain | |
| | Weight (pcf) | Cohesion Cu (psf) | Friction Angle, φ (Degree) | Friction Angle, φ' (Degree) | Parameter (Static), k (pci) | Parameter, ϵ_{50} | |
| Weathered Bedrock, SHALE | 140 | | 41 | 41 | 260 | | |

• Granular soils are classified as Sand, Sandy Gravel, Silty Loam, Sandy Loam and Gravelly Sand on the boring logs.

• Cohesive soils are classified as Silty Clay, Silty Clay Loam and Clay Loam on the boring logs.

- Unconfined Compressive Strength values of the cohesive soils are shown as Qu on the boring logs.
- Boring logs show SPT values number for three consecutive 6-inch penetration. N value is the sum of the second and the third numbers.
- Moist Unit Weight and Friction Angle estimated from SPT numbers.

Table 1B: Recommended Rock Parameters

| Rock Type | Total Unit Weight, γ (pcf) | Modulus of Rock Mass (ksi) | Uniaxial Compressive Strength (psi) | RQD (%) | Strain Factor (k _{r m}) |
|-----------------------------------|----------------------------------|----------------------------------|--|------------|--------------------------------------|
| Weak Shale Bedrock Note 1 | 135 | 170 | 700 | 30 | 0.0005 |
| Competent Shale Bedrock Note 2 | 140 | 250 | 5000 | 70 | 0.0005 |

South of Station 714+40

Table 1C: Recommended Rock Parameters

| North of Station 714+40 | | | | | | | | | | |
|-----------------------------------|----------------------------------|----------------------------------|--|------------|--------------------------------------|--|--|--|--|--|
| Rock Type | Total Unit Weight, γ (pcf) | Modulus of Rock Mass (ksi) | Uniaxial Compressive Strength, Qu (psi) | RQD (%) | Strain Factor (k _{r m}) | | | | | |
| Weak Shale Bedrock Note 1 | 135 | 230 | 1500 | 35 | 0.0005 | | | | | |
| Competent Shale Bedrock Note 2 | 140 | 280 | 5500 | 80 | 0.0005 | | | | | |

Notes: 1. Below weathered shale bedrock, 2. Below weak shale bedrock



5.4 Drainage Behind Wall

Drainage behind the wall and underdrain should be as per 2012 IDOT Bridge Manual. The water pressure should be added to the earth pressure if drainage is not provided. A Geocomposite Wall Drain should be placed between the timber lagging and CIP concrete facing and connected to the 4 inch diameter perforated drain pipe. It is understood that ground surface water runoff behind the wall will be drain by providing a concrete gutter and inlet system behind and at top of the wall. The wall inlets will be connected to IL 84 roadway storm sewer inlets.

5.5 Global Slope Stability

The retaining wall will be drilled shaft soldier pile wall. The slope stability failure circle will go below tip of the drilled shafts which are expected to be in hard/dense soils or bedrock giving very high factor of safety for a permanent condition. Therefore, we do not anticipate any global instability for a permanent condition.

To install ground anchors, excavation to a level few feet below ground anchor locations will be required having a temporary cantilever wall condition. We will perform global stability analysis after knowing elevation of ground anchors during the design phase.

6.0 CONSTRUCTION CONSIDERATIONS

6.1 Excavation

Any required excavations should be performed in accordance with local, state, and federal regulations including current OSHA regulations. The potential effect of ground movements upon nearby rail tracks and utilities should also be taken into consideration.

6.2 Dewatering

Since groundwater was encountered deeper than expected excavation depths, we do not anticipate any significant groundwater problems during the construction. Perched water existing in the granular soils will seep into the excavation in relatively small quantity which can be handled by the sump pump method.



6.3 Filling and Backfilling

All fill and backfill materials should be pre-approved by the site engineer. The backfill material should be porous granular material free of organic materials and debris. Backfill material should be compacted in lifts no greater than 8 inches in loose thickness. Each layer should be compacted to a minimum 95 percent of the maximum dry density as determined by AASHTO T 99, Standard Proctor Method. The slope of the ground surface behind the wall due to cut and fill should match with the existing slope for proper drainage preferably not steeper than 1:2(V:H). The excavated soil material can be reused for the roadway embankment.

6.4 Wall Construction

The wall should be constructed as per IDOT Standard Specifications and the current special provision developed by IDOT for construction of drilled soldier-pile wall, available at the IDOT web-site (http://www.dot.state.il.us/bridges/gbsp.html). A special provision *Permanent Ground Anchors* is available from IDOT BBS. Policies, Standards & Specifications Unit. A temporary casing in the granular soils will be required. It is understood that a storm sewer for IL 84 roadway will be constructed in front of the retaining wall. The construction of this roadway storm sewer is discussed in a separate report (Roadway Geotechnical Report for IL 84 Roadway Reconstruction.) If the roadway storm sewer is to be constructed after construction of the wall, the design of retaining wall should consider temporary excavation performed for construction of roadway storm sewer in front of the wall. It would be preferable to construct roadway storm sewer including backfilling to the proposed roadway subgrade before excavation in front of the wall.

6.5 Construction Monitoring

There is no need for a special construction monitoring for the retaining wall except normally required by the IDOT Standard Specifications.



7.0 QUALIFICATIONS

The analysis and recommendations submitted in this report are based upon the data obtained from the borings performed by Wang at the locations shown on the boring logs and Exhibit 2. This report does not reflect any variations that may occur between the borings or elsewhere on the site, variations whose nature and extent may not become evident until the course of construction. In the event that any changes in the design and/or location of the retaining wall are planned, we should be timely informed so that foundation recommendations can be rereviewed, and revised if necessary.

It has been a pleasure to assist Parsons Transportation Group and IDOT on this project. Please call if there are any questions, or if we can be of further service.

Respectfully Submitted,

WANG ENGINEERING, INC.

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Jerny WH Wang ICTF

Jerry W.H. Wang, Ph.D., P.E. Principal





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EXHIBITS





THIS EXHIBIT ISTO BE USED FOR BORING LOCATION ONLY

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| | | | R NA | 578 |
| | | | | 574 |
| N 008-7001 | | ·····/ | | |
| KED BY: M. Kothawala 45 N. Main Street | | | | 566 |
| ombard, IL 60148 ww.wangeng.com | R NA | 7 | | |
| 42-06-01 | | | | 002 |

APPENDIX A

| W 11 La Ta Fa | angeng@wangeng.com 145 N Main Street ombard, IL 60148 elephone: 630 953-9928 ax: 630 953-9938 | Client Proje ktS 5 2 Location | 2 ov | BC er Mis | DRI WE | NG Job P/ iippi Carrc | No. ARS Rive | OG : 342 ONS er and ounty | RW-01 -06-02 d IL 84 Retaining Wa , IL | Datum: N Elevation: North: 197 East: 2299 Station: 7 Offset: 36 | AVD 88 626.25 79857.7 9120.56 10+26.7 .96 RT | ft 6 ft 6 ft 71 | Page | 1 of 1 |
|---------------------------|---|--|------------------------|---------------------------|-------------|-----------------------------------|--------------------|---------------------------------------|--|--|---|--------------------------|-------------|-------------------------|
| Profile | SOIL AND ROCK DESCRIPTION | Depth (ft) Sample Type | recovery Sample No. | SPT Values (blw/6 in) | Qu (tsf) | Moisture Content (%) | Profile | Elevation (ft) | SOIL AND ROC DESCRIPTION | Depth X | Sample Type | SPT Values (blw/6 in) | Qu (tsf) | Moisture Content (%) |
| | 625.86-inch thick, black SILTY CL LOAM TOPS Very dense, brown weathere dolostone fragments | AY OIL/ - ed | 1 | 6 5 <u>0</u> /2 | 1.50 P | 15 | | | | | | | | |
| | HARD DRILLING | 5 | 2 | 30 25 32 | NP | 4 | | | | | | | | |
| | HARD DRILLING | | 3 | 20 23 11 | NP | 8 | | | | | | | | |
| | C45 0 | 10 | 4 | 13 13 17 | NP | 10 | | | | | | | | |
| | Hard, gray SILTY CLAY to S LOAM, some shale and mudstone fragments WEATHERED SI BEDRC | HALE | 5 | 10 19 20 | > 4.50 P | 13 | | | | | | | | |
| | | 15 | 6 | 21 35 40 | > 4.50 P | 18 | | | | | | | | |
| | | | 7 | 30 42 42 | NP | 12 | | | | | | | | |
| | ^{606.8} Very dense, dark gray interbedded MUDSTONE ar | 20 | 8 | 24 38 5 <u>0/</u> 3 | NP | 14 | | | | | | | | |
| //1//14 | SHALEBEDRO |)СК 🔀 |] 9 | 5 <u>0</u> /5 | NP | 16 | | | | | | | | |
| WANGENG.GU | Boring terminated at 23.75 f | 25_ | 10 | 5 <u>0/</u> 3 | NP | 15 | | | | | | | | |
| <u>ر</u> ان | GENE | RAL NO | FES | 5 | • | | | • | WATE | R LEVE | LDA | TA | | |
| Be | gin Drilling 11-21-2011 | Comple | te Dri | illing | 1 | 1-21 | -20 | 11 | While Drilling | $\underline{\nabla}_{\underline{x}}$ | | DRY | | |
| ∯ Dri | Iling Contractor Wang Testir | ng Service | S | Drill Rig | 9 | B-57 | 7 TN | /IR | At Completion of Drilling | ↓ <u>↓</u> | | DRY | | |
| E Dri | iller R&N Logger | F. Boz | ga | Ch | ecked | by . | C. N | iarin | Time After Drilling | | | | | |
| | ining wethoa 3.5 HSA | | | | | | | | The stratification lines rep between soil types: the ac | resent the app tual transition | proximat may be | e boundai oradual | у | |

3420602.GPJ WANGENG.GDT 8/14/14 NANGENGINC

| W 1 L T F | vangeng@wangeng.com 145 N Main Street ombard, IL 60148 elephone: 630 953-9928 ax: 630 953-9938 | BORING LOG 84- WEI Job No.: 342-0 Client Parsons Transportation Project US 52 / IL 64 / IL Location Carrol County, Illing | | | | | | 3 84- : 342-0 tation 64 / IL nty, Illi | -RWB-02 Datum: NAVD 88 06-02 Elevation: 627.17 ft Group, Inc. North: 1979864.82 ft 84 Station: 710+18.76 Offset: 62.17 RT Offset: 62.17 RT | | | | Page 2 | 2 of 2 |
|-----------------------|---|---|------------------------|--------------------------|-------------|-------------------------|---------|--|--|------------|-------------------------------------|--------------------------|-------------|-------------------------|
| Profile | SOIL AND ROCK | Depth (ft) Sample Type | recovery Sample No. | SPT Values (blw/6 in) | Qu (tsf) | Moisture Content (%) | Profile | Elevation (ft) | SOIL AND ROC DESCRIPTION | Cepth (ft) | Sample Type | SPT Values (blw/6 in) | Qu (tsf) | Moisture Content (%) |
| | RUN 3: 50.0 to 60. RECOVERY = 7 RQD = 4 | 0 feet | 3 | C O R E | | 7 | | | | | | | | |
| | 567.2 Competent rock, moderate strength, gray, fair rock mas quality, slightly weathered to fresh, laminated to massive, interbedded SHALE and MUDSTONE RUN 4: 60.0 to 65. RECOVERY = 9 RQD = 7 562.2 Boring terminated at 65.00 f | $\begin{array}{c} & & & & \\ & & & & \\ & & & & \\ & & & & $ | 4 | CORE | | 7 | | | | | | | | |
| | GENE | | TES | \$ | | | | | WATE | RLEVE | L D | | | |
| Be Dr Dr Dr Dr | GENERAL NOTES Begin Drilling 07-08-2014 Complete Drilling 07-08-2014 Drilling Contractor Wang Testing Services Drill Rig D-50 Turbo ATV Driller K&P Logger M. de los Reyes Checked by C. Marin Drilling Method 3.25" HSA, boring backfilled upon completion Complete Drilling Complete Drilling | | | | | | | While Drilling At Completion of Drilling Time After Drilling Depth to Water | | roximat | DRY DRY e boundar gradual. | У | | |

WANG

Boring 84-RWB-02

Run #1, 35.0' to 45.0', RECOVERY = 16%, RQD = 0% Run #2, 45.0' to 50.0', RECOVERY = 88%, RQD = 72% Run #3, 50.0' to 60.0', RECOVERY = 76%, RQD = 47% Run #4, 60.0' to 65.0', RECOVERY = 92%, RQD = 72%

| BEDROCK CORE SAVANNA, ILLINO | : IL 84 RETAINING WALL, NS | |
|---------------------------------|-------------------------------|---|
| SCALE: GRAPHIC | 84-RWB-02 | DRAWN BY: M.DLR CHECKED BY: C.LM |
| | Wang Engineering | 1145 N. Main Street Lombard, IL 60148 www.wangeng.com |
| FOR PARSONS | S CORPORATION | 342-06-02 |

Project

BORING LOG 84-RWB-03A

WEI Job No.: 342-06-02

wangeng@wangeng.com 1145 N Main Street Lombard, IL 60148 Telephone: 630 953-9928 Fax: 630 953-9938

Profile

WANGENGINC 3420602.GPJ WANGENG.GDT 8/14/14

Client Parsons Transportation Group, Inc.

US 52 / IL 64 / IL 84

Datum: NAVD 88 Elevation: 634.19 ft North: 1979928.56 ft East: 2299108.70 ft Station: Offset

| lephone: 630 953-9928 x: 630 953-9938 | Location | | | | Ca | rrol C | Cour | nty, | , Illinois Offset: | |
|--|---------------|-------------|------------|--------------------------|-------------|-------------------------|---------|-------------------|---|-------|
| SOIL AND ROCK | Depth (ft) | Sample Type | Sample No. | SPT Values (blw/6 in) | Qu (tsf) | Moisture Content (%) | Profile | Elevation (ft) | Depth | (tst) |
| Drilled without sampling | | | | | | | | <u>600.2</u> | 2 Competent rock, moderate strength, gray and brown, fair 35 rock mass quality, highly to moderately weathered, interbedded SHALE and MUDSTONE RUN 1: 34.0-44.0 ft RECOVERY= 86% RQD= 66% 1 40 40 40 40 40 40 40 40 40 40 | |

50 25 **GENERAL NOTES** WATER LEVEL DATA 07-08-2014 07-08-2014 NA Complete Drilling ∇ **Begin Drilling** While Drilling NA Wang Testing Services Drill Rig D-50 Turbo ATV **Drilling Contractor** At Completion of Drilling Ţ Driller K&P Logger M. de los Reyes Checked by Time After Drilling NA NA V **Drilling Method** Depth to Water The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.

Moisture Content (%)

6

84-RWB-03

84-RWB-03A

Boring 84-RWB-03 Run #1, 25.0' to 30.0', RECOVERY = 8%, RQD = 0% Run #2, 30.0' to 37.5', RECOVERY = 60%, RQD = 29% Run #3, 37.5' to 42.0', RECOVERY = 22%, RQD = 9%

Boring 84-RWB-03A Run #1, 34.0' to 44.0', RECOVERY = 86%, RQD = 66%







Boring 84-RWB-04A Run #1, 20.0' to 30.0', RECOVERY = 60%, RQD = 37% Run #2, 30.0' to 40.0', RECOVERY = 95%, RQD = 93% Run #3, 40.0' to 50.0', RECOVERY = 100%, RQD = 100%

| BEDROCK CORE: IL 84 RETAINING WALL, SAVANNA, ILLINOIS / SABULA, IOWA | | | | | | | | | | |
|---|---------------------|---|--|--|--|--|--|--|--|--|
| SCALE: GRAPHIC | 84-RWB-04A | DRAWN BY: C.L.M. CHECKED BY: M.A.K. | | | | | | | | |
| | Wang Engineering | 1145 N. Main Street Lombard, IL 60148 www.wangeng.com | | | | | | | | |
| FOR PARSONS | 342-06-01 | | | | | | | | | |



3420602.GPJ WANGENG.GDT 8/15/14





Boring 84-RWB-05A Run #1, 15.0' to 25.0', RECOVERY = 81%, RQD = 73% Run #2, 25.0' to 35.0', RECOVERY = 93%, RQD = 92%







Boring 84-RWB-06 Run #1, 20.0' to 29.0', RECOVERY = 72%, RQD = 47% Run #2, 29.0' to 39.0', RECOVERY = 93%, RQD = 62%







Boring 84-RWB-07 Run #1, 20.0' to 30.0', RECOVERY = 85%, RQD = 40% Run #2, 30.0' to 40.0', RECOVERY = 73%, RQD = 40%







FOR PARSONS CORPORATION

342-06-02



NANGENGINC



Boring 84-RWB-09 Run #1, 20.0' to 30.0', RECOVERY = 87%, RQD = 5% Run #2, 30.0' to 40.0', RECOVERY = 97%, RQD = 83% Run #3, 40.0' to 50.0', RECOVERY = 99%, RQD = 82%





3420602.GPJ WANGENG.GDT NANGENGINC



Boring 84-RWB-10 Run #1, 25.0' to 35.0', RECOVERY = 100%, RQD = 86% Run #2, 35.0' to 45.0', RECOVERY = 96%, RQD = 84%

| BEDROCK CORE: IL 84 RETAINING WALL, SAVANNA, ILLINOIS / SABULA, IOWA | | | | | | | | | | |
|---|--------------------------|---|--|--|--|--|--|--|--|--|
| | | | | | | | | | | |
| SCALE: CRAPHIC | SCALE: GRAPHIC 84-RWB-10 | | | | | | | | | |
| SCALL. GIVAPTIC | | | | | | | | | | |
| | Wang Engineering | 1145 N. Main Street Lombard, IL 60148 www.wangeng.com | | | | | | | | |
| FOR PARSONS | S CORPORATION | 342-06-02 | | | | | | | | |

BOTTOM



3420602.GPJ WANGENG.GDT NANGENGINC



Boring 84-RWB-11 Run #1, 20.0' to 26.0', RECOVERY = 66%, RQD = 27% Run #2, 26.0' to 31.0', RECOVERY = 92%, RQD = 43% Run #3, 31.0' to 34.0'. RECOVERY = 96%, RQD = 13% Run #4, 34.0' to 40.0', RECOVERY = 80%, RQD = 51% Run #5, 40.0' to 50.0', RECOVERY = 95%, RQD = 94%

| BEDROCK CORE SAVANNA, ILLINC | BEDROCK CORE: IL 84 RETAINING WALL, SAVANNA, ILLINOIS / SABULA, IOWA | | | | | | | | | |
|---------------------------------|---|---|--|--|--|--|--|--|--|--|
| SCALE: GRAPHIC | 84-RWB-11 | DRAWN BY: A. Happel. CHECKED BY: C. Marin | | | | | | | | |
| | Wang Engineering | 1145 N. Main Street Lombard, IL 60148 www.wangeng.com | | | | | | | | |
| FOR PARSONS | S CORPORATION | 342-06-02 | | | | | | | | |



WANGENGINC 3420602.GPJ WANGENG.GDT 8/14/14

| Wangeng@wangeng.comBORING LOG 84-RWB-12wangeng@wangeng.comWEI Job No.: 342-06-021145 N Main StreetClientLombard, IL 60148ProjectTelephone: 630 953-9928ProjectFax: 630 953-9938Carrol County, Illinois | | | | | | | | | Page 2 | 2 of 2 | | | | | |
|--|---|---|-----------------------|--------------------------|-------------|-------------------------|--------------|---|---|-----------|-------------|------------|--------------------------|-------------|-------------------------|
| Profile | SOIL AND ROCK | Depth (ft) Sample Type | s Sample No. | SPT Values (blw/6 in) | Qu (tsf) | Moisture Content (%) | Profile | Elevation (ft) | SOIL AND ROC DESCRIPTION | Depth (#) | Sample Type | Sample No. | SPT Values (blw/6 in) | Qu (tsf) | Moisture Content (%) |
| | 606.6 Competent rock, moderate strength, gray, good rock ma quality, slightly weathered to fresh, laminated to massive SHALE and MUDSTONE 56.5 ft-Qu=2610 ps Run 4: 55.0-6 Run 4: 55.0-6 RU 4: 55.0-6 RU 4: 55.0-6 RU 2010 ps 61.5 ft-Qu=2610 ps 61.5 ft-Qu=2610 ps 61.5 ft-Qu=2610 ps 61.5 ft-Qu=1130 ps 61.5 ft-Qu=1130 ps 593.6 Boring terminated at 65.00 ft | si> 5.0 ft 60 55 60 60 60 61 60 65 60 65 60 65 60 65 65 65 65 70 65 70 70 70 70 70 70 75 75 75 75 75 75 75 75 75 75 | 4 | CORE | | | | | | | | | | | |
| | GENE | RAL NO | ΓES | ; ; | · | | | | WATE | | LD | AT/ | A | | |
| | gin Drilling U5-13-2014 Illing Contractor Wang Testir | Comple Comple | te Dril s 「 | lling Drill Ric | כ | D-2 | -20' 5 AT | 14 V | While Drilling At Completion of Drilling | <u>¥</u> | | DF | KY RY | | |
| | iller N&J Logaer | D. Kolpa | acki | Ch | ecked | by (| C. M | arin | Time After Drilling | NA | | | N.I | | |
| Dri | Drilling Method 2.25" HSA to 10'. mud rotary thereafter. boring | | | | | | | Depth to Water Y NA | | | | | | | |
| MAIN | backfilled upon completion | | | | | | | The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual. | | | | | | | |



Boring 84-RWB-12 Run #1, 30.0' to 35.0', RECOVERY = 83%, RQD = 37% Run #2, 35.0' to 45.0', RECOVERY = 50%, RQD = 39% Run #3, 45.0' to 55.0'. RECOVERY = 70%, RQD = 53% Run #4, 55.0' to 65.0', RECOVERY = 100%, RQD = 87%

| | | - |
|----------------------------------|--|---|
| BEDROCK CORE: SAVANNA, ILLINO | : IL 84 RETAINING WALL, NS / SABULA, IOWA | |
| SCALE: GRAPHIC | 84-RWB-12 | DRAWN BY: A. Happel. CHECKED BY: C. Marin |
| | Wang Engineering | 1145 N. Main Street Lombard, IL 60148 www.wangeng.com |
| FOR PARSONS | S CORPORATION | 342-06-02 |



| w 1º Lo Fa | angeng@wangeng.com 145 N Main Street ombard, IL 60148 elephone: 630 953-9928 ax: 630 953-9938 | Client Project | BOR Parse | WEI Jo Ons Tran US 5 Carrol | LOG b No.: asporta 2 / IL 6 Count | 84-1 342-06 ation G 64 / IL 8 nty, Illin | RWB-13 6-02 Group, Inc. 34 ois | Datum: N/ Elevation: North: 198 East: 2299 Station: 71 Offset: 46. | VD 88 655.44 ft 0702.79 007.77 ft 8+63.42 74 RT | ft | Page 2 | ? of 2 |
|-------------------------|--|---|---|--|---|---|--|---|--|--------------------------|-------------|-------------------------|
| Profile | SOIL AND ROCK DESCRIPTION | Depth (ft) Sample Type _{recovery} | Sample No. SPT Values (blw/6 in) | Qu (tsf) Moisture | Profile | Elevation (ft) | SOIL AND ROCH DESCRIPTION | Depth (ft) | Sample Type recovery Sample No. | SPT Values (blw/6 in) | Qu (tsf) | Moisture Content (%) |
| | Run 3: 49.0-5 RECOVERY= RQD= 601.4 Boring terminated at 54.00 ft | - 4.0 ft 97% _ 97% _ - - - | 3 3 | | | | | | | | | |
| | | | | | | | | | | | | |
| | | | | | | | | | | | | |
| | | 65 | | | | | | | | | | |
| | | 70 | | | | | | | | | | |
| | | | | | | | | | | | | |
| Be Dri Dri Dri | gin Drilling 05-15-2014 illing Contractor Wang Testin iller N&J Logger illing Method 2.25" HSA to 10 backfilled upon completio | Complete Ig Services D. Kolpac ', mud rotai n | _⊃ Drilling Drill Ri :ki Cr 'y there | 05-1 g D- lecked by after, bo | 6-201 25 AT\ C. Ma ring | 4 V arin | While Drilling At Completion of Drilling Time After Drilling Depth to Water | K LEVE ↓ NA NA esent the app transition of | L DAI D D roximate b nay be gra | RY RY | | |



า #5

1 TOP



WANGENGINC 3420602.GPJ WANGENG.GDT 8/14/14

| | Page 2 of 2 Page 2 of 2 Page 2 of 2 BORING LOG 84-RWB-14 Weil Job No.: 342-06-02 Wangeng@wangeng.com 1145 N Main Street Lombard, IL 60148 Telephone: 630 953-9928 Fax: 630 953-9938 Description Datum: NAVD 88 Elevation: 643.68 ft North: 1980769.05 ft East: 2298973.05 ft Station: 719+34.94 Offset: 25.61 RT | | | | | | | | | | | | |
|----------------------|--|---|--|---------------------------|--------------------------------|------------------------------|---------------------------|--|---|---------------------------------------|--------------------------|-------------|-------------------------|
| Profile | SOIL AND ROCK DESCRIPTION | Depth (ft) Sample Type | Sample No. SPT Values | (blw/6 in) Qu (tsf) | Moisture Content (%) | Profile | Elevation (ft) | SOIL AND ROC DESCRIPTION | L Depth X | Sample Type recovery Sample No. | SPT Values (blw/6 in) | Qu (tsf) | Moisture Content (%) |
| | RUN 2: 46.0-{ RECOVERY= RQD= | - 56.0 ft - 100% _ = 89% - - - - 55_ - | 2 | | | | | | | | | | |
| | RUN 3: 56.0-(RECOVERY= RQD= | | 3 | | | | | | | | | | |
| | 577.7 Boring terminated at 66.00 f | - 65_ - 1 - - - - - - - - - - - - - - - - - | | | | | | | | | | | |
| איזאטרואטיטרא איזאיז | GENF | | ES | | | | | WATF | RLEVF | | ΓΑ | | |
| | regin Drilling 06-05-2014 prilling Contractor Wang Testin priller N&K Logger prilling Method 2.25" HSA. 2.5' | Complet ng Services A. Happ interval to | e Drilling Drill Drill Del 36', Re | Rig Checkec DCk CO | 06-05 D-2 I by re the | 5-20 5 A1 C. N erea | 14 TV larin fter | While Drilling At Completion of Drilling Time After Drilling Depth to Water | ✓ ✓ NA ✓ Index resent the approximation | roximate | NA NA | у | |



Boring 84-RWB-14 Run #1, 36.0' to 46.0', RECOVERY = 18%, RQD = 0% Run #2, 46.0' to 56.0', RECOVERY = 100%, RQD = 89% Run #3, 56.0' to 66.0', RECOVERY = 99%, RQD = 95%

| BEDROCK CORE: IL 84 RETAINING WALL, SAVANNA, ILLINOIS / SABULA, IOWA | | | | | | | | | | |
|---|---------------------|---|--|--|--|--|--|--|--|--|
| SCALE: GRAPHIC | 84-RWB-14 | DRAWN BY: C.L.M. CHECKED BY: M.A.K. | | | | | | | | |
| | Wang Engineering | 1145 N. Main Street Lombard, IL 60148 www.wangeng.com | | | | | | | | |
| FOR PARSONS | 342-06-01 | | | | | | | | | |



| VV. | Vang Engineering |
|-----|----------------------------|
|-----|----------------------------|

BORING LOG 84-RWB-15

WEI Job No.: 342-06-02

wangeng@wangeng.com 1145 N Main Street Lombard, IL 60148 Telephone: 630 953-9928

Project

Parsons Transportation Group, Inc. Client

US 52 / IL 64 / IL 84

Datum: NAVD 88 Elevation: 650.10 ft North: 1980843.99 ft East: 2298970.76 ft Station: 720+08.27

| F | Fax: 630 953-9938 | cation | | | Car | rol C | Cour | nty, Illiı | nois | Offset: 39 | .07 R | Т | | | |
|-----------------|--|---|------------|--------------------------|-------------|-------------------------|---------|-------------------|---|---------------------|-------------|------------|--------------------------|-------------|-------------------------|
| Profile | SOIL AND ROCK | Depth (ft) Sample Type | Sample No. | SPT Values (blw/6 in) | Qu (tsf) | Moisture Content (%) | Profile | Elevation (ft) | SOIL AND ROC DESCRIPTION | Depth (1) | Sample Type | Sample No. | SPT Values (blw/6 in) | Qu (tsf) | Moisture Content (%) |
| | | | | | | | | | | | | | | | |
| | Competent rock, moderate strength, gray and brown, good to excellent rock mass quality, slightly weathered to fresh, laminated to massive SHALE and MUDSTONE RUN 3: 52.0-62.0 1 RECOVERY= 99% RQD= 98% | - - - d 55_ ft - % - - - - | 3 | CORE | | | | | | | | | | | |
| | | - - 60_ - - - - | | | | | | | | | | | | | |
| | RUN 4: 62.0-72.01 RECOVERY= 809 RQD= 809 | π % 65 | | CORE | | | | | | | | | | | |
| | | - - - 70_ - - | 4 | | | | | | | | | | | | |
| | ^{578.1} Boring terminated at 72.00 ft | | | | | | | | | | | | | | |
| WAW | | 75_ | | | | | | | | | | | | | |
| 2.6FJ | GENERA | | ËS | • | | | | | WATE | R LEVE | LD | AT | Α | | |
| nonzi Be | Begin Drilling 06-03-2014 Complete Drilling 06-03-2014 | | | | | | 4 | While Drilling | <u> </u> | | 12.5 | 50 ft | | | |
| | illing Contractor Wang Testing S | Services | ્ [ન | Drill Rig | J | D-2 | 5 AT | V | At Completion of Drilling | ¥ | 12 | 2.5 (| (MUE |)) | |
| | Driller N&K Logger A. Happel Checked by C. Marin | | | | | | | arin | Time After Drilling | NA | | | | | |
| ום אופנ אופנ | "IIIng Method 2.25" HSA. 2.5' inte | erval to | 40', | Roci | < cor | e the | reat | ter | Depth to Water The stratification lines repr | NA esent the app | proxim | ate bo | oundary | / | |
| ŝ | | | | | | | | | between soil types; the act | ual transition | may b | e grad | dual. ໌ | | |



Boring 84-RWB-15 Run #1, 40.0' to 45.0', RECOVERY = 82%, RQD = 58% Run #2, 45.0' to 52.0', RECOVERY = 20%, RQD = 0% Run #3, 52.0' to 62.0', RECOVERY = 99%, RQD = 98% Run #4, 62.0' to 72.0', RECOVERY = 80%, RQD = 80%

| BEDROCK CORE: IL 84 RETAINING WALL, SAVANNA, ILLINOIS / SABULA, IOWA | | | | | | | | | | |
|---|---------------------|---|--|--|--|--|--|--|--|--|
| SCALE: GRAPHIC | 84-RWB-15 | DRAWN BY: C.L.M. CHECKED BY: M.A.K. | | | | | | | | |
| | Wang Engineering | 1145 N. Main Street Lombard, IL 60148 www.wangeng.com | | | | | | | | |
| FOR PARSONS | S CORPORATION | 342-06-01 | | | | | | | | |



3420602.GPJ WANGENG.GDT

| wa 11 Lo Fa | Wangeng@wangeng.com Main Street Client Parsons Transportation Group, Inc. Datum: NAVD 88 Elevation: 643.41 ft Lombard, IL 60148 Project US 52 / IL 64 / IL 84 Datum: 721+11.15 Station: 721+11.15 Fax: 630 953-9938 Corrol County, Illinois Offset: 35.99 RT | | | | | | | | | | |
|----------------------|---|--|--|--|------------------------------|-----------------------------|---|--|--|--|--|
| Profile | SOIL AND ROCK DESCRIPTION | Depth (ft) Sample Type | Sample No. SPT Values (blw/6 in) | Qu (tsf) Moisture Content (%) | Profile Elevation (ft) | SOIL AND ROC DESCRIPTION | Depth (ft) Sample Type recovery Samole No | SPT Values (blw/6 in) Qu (tsf) Moisture Content (%) | | | |
| | massive, interbedded SHAL and MUDSTONE 50.0 ft-Qu=5180 ps Run 3: 48.5-5 | E si> i8.5 ft | E | | | | | | | | |
| | RECOVERY= RQD= | 92% _ 91% _ 55_ - - - - - - - | 3 | | | | | | | | |
| | ^{584.9} Boring terminated at 58.50 fl | | | _ | | | | | | | |
| | | 60 | | | | | | | | | |
| | | - | | | | | | | | | |
| | | 65 | | | | | | | | | |
| | | - | | | | | | | | | |
| | | - - 70 | | | | | | | | | |
| | | - | | | | | | | | | |
| | | - - - 75_ | | | | | | | | | |
| | GENE | RAL NOT | ES ES | | 1 1 | WATE | | | | | |
| Be | gin Drilling 05-07-2014 | Complete | 7-2014 | While Drilling | Ţ | DRY | | | | | |
| 5 Dri | Iling Contractor Wang Testir | ng Services | Drill Ri | g D-5 | 0 TMR | At Completion of Drilling | ₹. | NA | | | |
| Dri Dri | Driller R&J Logger A. Tomaras Checked by C. Marin Time After Drilling NA Drilling Method 3.25" HSA, boring backfilled upon completion Depth to Water V NA The stratification lines represent the approximate boundary between soil types; the actual transition may be oradual. The stratification lines represent the approximate boundary between the appr | | | | | | | | | | |





WANGENGINC 3420602.GPJ WANGENG.GDT 8/14/14

| wa 11 Lo Fa | angeng@ 145 N M ombard, elephone ax: 630 S | BORING LOG 84- WEI Job No.: 342-0 Client Parsons Transportation Project US 52 / IL 64 / IL Location Carrol County, Illin | | | | | | | | Page 2 of 2 •RWB-17 Datum: NAVD 88 06-02 Elevation: 643.61 ft Group, Inc. North: 1981014.52 ft 84 Station: 721+82.61 nois Offset: 39.95 RT | | | | | | |
|---|--|--|---|--|------------------|--------------------------|-------------|-------------------------|---------|--|---|--------------|-------------|--|-------------|-------------------------|
| Profile | Elevation (ft) | SOIL AND ROCK DESCRIPTION | Depth (ft) | Sample Type | Sample No. | SPT Values (blw/6 in) | Qu (tsf) | Moisture Content (%) | Profile | Elevation (ft) | SOIL AND ROC DESCRIPTION | L Depth X | Sample Type | Sample No. SPT Values (blw/6 in) | Qu (tsf) | Moisture Content (%) |
| | 588.6 Bc | RUN 3 - 50.0-5 RECOVERY= 9 RQD= 8 | 5.0 ft- | | 3 | CORE | | | | | | | | | | |
| | | | 75_ | | | | | | | | | | | | | |
| GENERAL NOTES Begin Drilling 05-06-2014 Complete Drilling 05-06-2014 Drilling Contractor Wang Testing Services Drill Rig D-50 TMR | | | | | | | | | | | While Drilling At Completion of Drilling | ER LEVE ♀ | LD | ATA DRY NA | | |
| Dri Dri | ller lling Met | R&J Logger thod 3.25" HSA, bori | Time After Drilling Depth to Water The stratification lines rep between soil types: the ac | NA L NA present the app tual transition | roxima may be | te boundar | у | | | | | | | | | |




WANGENGINC 3420602.GPJ WANGENG.GDT 8/14/14

| Wangeng@wangeng.comBORING LOG 84-RV1145 N Main StreetWEI Job No.: 342-06-02Lombard, IL 60148ClientTelephone: 630 953-9928ProjectUS 52 / IL 64 / IL 84Fax: 630 953-9938LocationCarrol County, Illinois | | | | | | | •RWB-18 06-02 Group, Inc. 84 nois | Datum: N/ Elevation: North: 198 East: 2298 Station: 72 Offset: 35 | AVD 8 639.9 31080 3911.8 22+51 .43 R | 38 96 ft .42 f 88 ft .37 T | ït | Page 2 | 2 of 2 | | | |
|---|--|---|------------------------------|------------|--------------------------|-------------|---|--|---|---|-------------------|------------------------------|------------|--------------------------|-------------|-------------------------|
| Profile | Elevation (ft) | SOIL AND ROCK DESCRIPTION | Depth (ft) Sample Type | Sample No. | SPT Values (blw/6 in) | Qu (tsf) | Moisture Content (%) | Profile | Elevation (ft) | SOIL AND ROCI DESCRIPTION | Depth | Sample Type | Sample No. | SPT Values (blw/6 in) | Qu (tsf) | Moisture Content (%) |
| | 586.0 | RUN 3 - 39.0-54 RECOVERY= 10 RQD= 8 | - | 3 | O R E | | | | | | | | | | | |
| | Bc | oring terminated at 54.00 fi | 55 | | | | | | | | | | | | | |
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| 2 | | GENE | 75_ RAL NOT | ES | | | | | | WATE | R LEVE | L D | AT | A | | |
| Be Dri | GENEKAL NOTES Begin Drilling 05-05-2014 Complete Drilling 05-05-2014 Drilling Contractor Wang Testing Services Drill Rig D-50 TMR Driller R&J Logger A. Tomaras Checked by C. Marin Drilling Method 3.25" HSA, boring backfilled upon completion | | | | | | | While Drilling At Completion of Drilling Time After Drilling Depth to Water | ↓ NA NA esent the app ual transition | roxima | DI N ate bo | RY IA oundary dual. | | | | |

WAN







Boring 84-RWB-19 Run #1, 24.0' to 32.0', RECOVERY = 100%, RQD = 20% Run #2, 32.0' to 42.0', RECOVERY = 98%, RQD = 64% Run #3, 42.0' to 47.0', RECOVERY = 95%, RQD = 77%

| BEDROCK CORE: IL 84 RETAINING WALL, SAVANNA, ILLINOIS | | | | | | | |
|--|--|--|--|--|--|--|--|
| SCALE: GRAPHIC | DRAWN BY: C.L.M. CHECKED BY: M.A.K. | | | | | | |
| | Wang Engineering | | | | | | |
| FOR PARSONS | 342-06-01 | | | | | | |





BORING LOG 84-RWB-20

WEI Job No.: 342-06-02

wangeng@wangeng.com 1145 N Main Street Lombard, IL 60148 Telephone: 630 953-9928 Fax: 630 953-9938

Client Parsons Transportation Group, Inc.

US 52 / IL 64 / IL 84

Carrol County, Illinois

Project

Location

Datum: NAVD 88 Elevation: 640.57 ft North: 1981206.53 ft East: 2298887.04 ft Station: 723+79.81 Offset: 40.16 RT

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| Profile | | SOIL AND ROCK DESCRIPTION | Depth (ft) Sample Type recoverv | Sample No. | SPT Values (blw/6 in) | Qu (tsf) | Moisture Content (%) | Profile | Elevation (ft) | SOIL AND ROCK DESCRIPTION | Depth (ft) | Sample Type | Sample No. | SPT Values (blw/6 in) | Qu (tsf) | Moisture Content (%) |
| | | 50.5 ft-Qu=7660 psi - | > | | | | | | | | | | | | | |
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| 2.GPJ | | GENER | AL NOT | ES |) | | | | | WATER | R LEVE | LD | AT | Ά | | |
| 2090 | Beg | gin Drilling 05-01-2014 | Complete | e Dri | lling | C |)5-01 | -201 | 4 | While Drilling | <u> </u> | | N | A | | |
| 342 | Dril | Iling Contractor Wang Testing | Services | <u> </u> | Drill Ric |] | D-50 |) TN | IR | At Completion of Drilling | Y | 8 | 3 (1 | NUD) | | |
| | Z Driller R&J Loαger F. Bozαa Checked by C. Marin | | | | | Time After Drillina | NA | | | | | | | | | |
| Drilling Method 225" SSA 4" casing boring backfilled upon | | | | | Depth to Water V | NA | | | | | | | | | | |
| ANG | | | | | | The stratification lines represent the approximate boundary | | | | | | | | | | |
| <1 | | | | | | between soil types; the actual | I transition | mav b | e ara | adual. | | | | | | |



Boring 84-RWB-20 Run #1, 34.0' to 39.0', RECOVERY = 85%, RQD = 0% Run #2, 39.0' to 44.0', RECOVERY = 83%, RQD = 24% Run #3, 44.0' to 54.0', RECOVERY = 99%, RQD = 63%

| BEDROCK CORE | LIL 84 RETAINING WALL, | |
|-------------------|---|--|
| SAVAININA, ILLING | | |
| SCALE: GRAPHIC | 84-RWB-20 | DRAWN BY: C.L.M. CHECKED BY: M.A.K. |
| | 1145 N. Main Street Lombard, IL 60148 www.wangeng.com | |
| FOR PARSONS | S CORPORATION | 342-06-01 |







Boring 84-RWB-21 Run #1, 35.0' to 45.0', RECOVERY = 89%, RQD = 48% Run #2, 45.0' to 50.0', RECOVERY = 100%, RQD = 78% Run #3, 50.0' to 55.0', RECOVERY = 100%, RQD = 36% Run #4, 55.0' to 65.0', RECOVERY = 53%, RQD = 18%

| BEDROCK CORE: IL 84 RETAINING WALL, SAVANNA, ILLINOIS | | | | | | | | |
|--|--|-----------|--|--|--|--|--|--|
| SCALE: GRAPHIC | DRAWN BY: C.L.M. CHECKED BY: M.A.K. | | | | | | | |
| Wang Engineering | | | | | | | | |
| FOR PARSONS | S CORPORATION | 342-06-01 | | | | | | |







Project

BORING LOG 84-RWB-22B

WEI Job No.: 342-06-02

wangeng@wangeng.com 1145 N Main Street Lombard, IL 60148 Telephone: 630 953-9928 Fax: 630 953-9938

Parsons Transportation Group, Inc. Client

US 52 / IL 64 / IL 84

Datum: NAVD 88 Elevation: 625.62 ft North: 1981337.87 ft East: 2298862.26 ft Station: 725+18.12 Offset: 34.84 RT

NA

6.5 (MUD)

Carrol County, Illinois Location SPT Values (blw/6 in) SPT Values (blw/6 in) Moisture Content (%) Sample Type Sample No Sample No Elevation (ft) Elevation (ft) Profile Profile SOIL AND ROCK Depth (ft) SOIL AND ROCK Depth (ft) Qu (tsf) Qu (tsf) Sample DESCRIPTION DESCRIPTION Hard, greenish gray SILTY CLAY, trace shale fragments Drilled 25 feet without sampling --WEATHERED SHALE 32 NP BEDROCK--35 --L₁(%)=42, P₁(%)=21--43 --%Gravel=8.2----%Sand=0.6----%Silt=49.1--44 NP 2 34 --%Clay=42.0----A7-6 (20)--³⁰ 45 С Very weak strength, gray, poor Ó rock mass quality, highly Ř weathered, laminated to massive, F interbedded SHALE and MUDSTONE --RUN 1 - 30.0-40.0 ft----RECOVERY= 81%-----RQD= 28%--10 35 1 589 6 36.0 ft-Qu=4920 psi ---> Competent rock, moderate strength, gray, good rock mass quality, slightly weathered to fresh, laminated to massive, interbedded SHALE and MUDSTONE 15 39.0 ft-Qu=5930 psi --->⁴⁰_ 40.0 ft-Qu=5960 psi ---> С Ō R F --RUN 2 - 40.0-50.0 ft----RECOVERY= 99%-----RQD= 81%--20 45 2 46.0 ft-Qu=6140 psi ---> 575.6 600.6 Boring terminated at 50.00 ft

Page 1 of 1

Moisture Content (%)



Boring 84-RWB-22B Run #1, 30.0' to 40.0', RECOVERY = 81%, RQD = 28% Run #2, 40.0' to 50.0', RECOVERY = 99%, RQD = 81%

| BEDROCK CORE: IL 84 RETAINING WALL, | | | | | | | |
|---|---------------|--------------------|--|--|--|--|--|
| | | | | | | | |
| SCALE: GRAPHIC | 91_D\M/B_22B | DRAWN BY: C.L.M. | | | | | |
| 00/122/01/01/01 | 04-NWD-22D | CHECKED BY: M.A.K. | | | | | |
| Wang Engineering 1145 N. Main Stree Lombard, IL 60148 www.wangeng.com | | | | | | | |
| FOR PARSONS | S CORPORATION | 342-06-01 | | | | | |



APPENDIX B



L L U U U AD ŝ d C 2120602 НО SIZE GRAIN μ



L L L U U U AB. ŝ d C 2120602 НО SIZE GRAIN



L L L L L AB. ŝ d C 2120602 НО SIZE GRAIN ЧE



L L U U U AD ŝ d C 2120602 НО SIZE GRAIN



LAB.GDT ŝ d C 3420602 НО SIZE GRAIN

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| | | | | | | | | LIQUID LIMIT | | | | | |
| | Specime | en Identif | ication | LL | PL | PI | Fines | IDH Classification | | | | | |
| l | 84-RWB-07 | 1#4 | 8.5 ft | 36 | 17 | 19 | 82 | Silty Clay | | | | | |
| × | 84-RWB-07 | 1#6 | 13.5 ft | NP | NP | NP | 19 | Gravelly Sandy Loam | | | | | |
| | 84-RWB-02 | 2#10 | 23.5 ft | 36 | 17 | 19 | 73 | Silty Clay | | | | | |
| * | 84-RWB-02 | 2#1 | 35.0 ft | 34 | 17 | 17 | 91 | Silty Clay | | | | | |
| 0 | 84-RWB-03 | 3#7 | 16.0 ft | 33 | 17 | 16 | 71 | Gravelly Silty Clay Loam | | | | | |
| o | 84-RWB-04 | 4#6 | 13.5 ft | 37 | 17 | 20 | 96 | Silty Clay | | | | | |
| 0 | 84-RWB-07 | 7#4 | 8.5 ft | 39 | 18 | 21 | 95 | Clav | | | | | |
| | 84-RWB-08 | 8#7 | 16 0 ft | 35 | 18 | 17 | 99 | Silty Clay | | | | | |
| \otimes | 84-RWB-10 | 0#7 | 16.0 ft | 35 | 17 | 18 | 90 | Silty Clay | | | | | |
| | | 0#1 | 6.0 ft | 25 | 10 | 7 | 55 | Grovelly Silty Learn | | | | | |
| | 04 DWD 44 | <u>-</u> #0 | 0.0 IL | 20 | 10 | 1 | 07 | | | | | | |
| | 04-RVVB-12 | 2#0 | 18.5 Tt | 32 | 15 | 17 | 93 | | | | | | |
| ľ | 84-RWB-13 | S#1 | 16.0 ft | NP | NP | NP | 68 | | | | | | |
| Ľ | 84-RWB-1 | 5# | 8.5 ft | NP | NP | NP | 30 | Gravelly Sandy Loam | | | | | |
| ☆ | 84-RWB-1 | 5#8 | 18.5 ft | NP | NP | NP | 35 | Gravelly Sandy Loam | | | | | |
| ස | 84-RWB-1 | 5#14 | 38.5 ft | 41 | 21 | 20 | 100 | Silty Clay | | | | | |
| | 84-RWB-18 | 8#6 | 13.5 ft | NP | NP | NP | 27 | Sandy Loam | | | | | |
| • | 84-RWB-19 | 9#3 | 6.0 ft | NP | NP | NP | 41 | Sandy Loam | | | | | |
| \diamond | 84-RWB-19 | 9#7 | 16.0 ft | NP | NP | NP | 31 | Gravelly Silty Loam | | | | | |
| \times | 84-RWB-20 | 0#12 | 28.5 ft | 35 | 18 | 17 | 99 | Silty Clay | | | | | |
| * | 84-RWB-2 | 1#5 | 11.0 ft | NP | NP | NP | 33 | Sandy Loam | | | | | |
| ſ | 4 | Wan | a Enainee | rina | | | | ATTERBERG LIMITS' RESULTS | | | | | |
| | WF | | 5 N Main S | treet | | | | Project: US 52 / IL 64 / IL 84 | | | | | |
| | | Lom | pard, IL 60 phone: 63 | 0148 0 953-9 | 9928 | | | Location: Carrol County, Illinois | | | | | |
| ļ | 51101 1902 | Fax: | 630 953-9 | 9938 | | | | Number: 342-06-02 | | | | | |



ATTERBERG LIMITS IDH 3420602.GPJ US LAB.GDT





Fax: 630 953-9938



WANGENG.GDT 8/14/14 GPJ HO

Number: 342-06-02



DENSITY-UNIT WEIGHT DETERMINATION ASTM D 7263

Client: Parsons Transportation Group, Inc. Project: US 52 / IL 64 / IL 84 WEI Job No: 342-06-02

Analyst name: A. Mohammed Test date: June 10, 2014

| Sample #: Water content determination | 84-RWB-05A Run 1, 18.5ft | 84-RWB-08 Run 1, 25.0ft | |
|--|-----------------------------|-----------------------------------|--|
| Mass of tare and wet soil (g) $W_w =$ | 169.92 | 182.94 | |
| Mass of tare and dry soil (g) $W_d =$ | 158.23 | 172.66 | |
| Mass of tare (g) $W_t =$ | 31.12 | 31.24 | |
| Water content w = | 9% | 7% | |

Density--Unit Weight

| Diameter measurements (in) $D_1 =$ | 1.993 | 2.000 | |
|------------------------------------|---------|---------|--|
| D ₂ = | 2.001 | 2.010 | |
| D ₃ = | 2.001 | 2.000 | |
| Average diameter (in) D = | 1.998 | 2.003 | |
| Height measurements (in) $H_1 =$ | 1.123 | 1.200 | |
| H ₂ = | 1.102 | 1.210 | |
| H ₃ = | 1.130 | 1.210 | |
| Average height (in) H = | 1.118 | 1.207 | |
| Total weight (g) W = | 138.800 | 151.700 | |
| Bulk Unit Weight (pcf) g = | 150.78 | 151.97 | |
| Dry Unit Weight (pcf) $g_d =$ | 138.08 | 141.67 | |

an Prepared by: Checked by:

Date: 6/10/14 Date: 6/10/14





DENSITY-UNIT WEIGHT DETERMINATION ASTM D 7263

Analyst name: A. Mohammed

| WEI Job No: 34 | 2-06-02 | Test date: | June 10, 2014 |
|---------------------------------------|----------------------------|-----------------------------------|----------------------------|
| Sample #: | 84-RWB-11 Run 1, 20.5ft | 84-RWB-13 Run 1, 39.0ft | 84-RWB-17 Run 1, 32.2ft |
| Mass of tare and wet soil (g) $W_w =$ | 119.55 | 156.62 | 179.35 |
| Mass of tare and dry soil (g) $W_d =$ | 110.66 | 146.38 | 160.13 |
| Mass of tare (g) $W_t =$ | 31.17 | 31.30 | 31.24 |
| Water content w = | 11% | 9% | 15% |
| DensityUnit Weight | | | |
| Diameter measurements (in) $D_1 =$ | 1.733 | 1.984 | 2.036 |
| D ₂ = | 1.738 | 2.000 | 2.042 |
| D ₃ = | 1.744 | 1.950 | 2.059 |
| Average diameter (in) D = | 1.738 | 1.978 | 2.046 |
| Height measurements (in) $H_1 =$ | 1.007 | 1.103 | 1.260 |
| $H_2 =$ | 1.077 | 1.066 | 1.219 |
| $H_3 =$ | 1.025 | 1.096 | 1.263 |
| Average height (in) H = | 1.036 | 1.088 | 1.247 |
| Total weight (g) W = | 88.380 | 125.320 | 148.110 |
| Bulk Unit Weight (pcf) g = | 136.92 | 142.78 | 137.66 |
| Dry Unit Weight (pcf) $g_d =$ | 123.14 | 131.11 | 119.79 |

Client: Parsons Transportation Group, Inc.

Project: US 52 / IL 64 / IL 84

Prepared by: Checked by:

Date: 6/10/14 Date: 6/16/14





DENSITY-UNIT WEIGHT DETERMINATION ASTM D 7263

Client: Parsons Transportation Group, Inc. Project: US 52 / IL 64 / IL 84 WEI Job No: 342-06-02

Analyst name: A. Mohammed Test date: June 10, 2014

| Sample #: | 84-RWB-20 Run 1, 35.0ft | 84-RWB-20 Run 2, 40.0ft | |
|---------------------------------------|----------------------------|-----------------------------------|--|
| Mass of tare and wet soil (g) $W_w =$ | 151.93 | 198.12 | |
| Mass of tare and dry soil (g) $W_d =$ | 137.16 | 182.10 | |
| Mass of tare (g) $W_t =$ | 31.57 | 31.13 | |
| Water content w = | 14% | 11% | |

Density--Unit Weight

| Diameter measurements (in) $D_1 =$ | 2.049 | 2.054 | |
|------------------------------------|---------|---------|--|
| D ₂ = | 2.058 | 2.049 | |
| D ₃ = | 2.048 | 2.051 | |
| Average diameter (in) D = | 2.052 | 2.051 | |
| Height measurements (in) $H_1 =$ | 1.044 | 1.302 | |
| H ₂ = | 1.037 | 1.293 | |
| H ₃ = | 1.047 | 1.312 | |
| Average height (in) H = | 1.043 | 1.302 | |
| Total weight (g) W = | 120.360 | 166.990 | |
| Bulk Unit Weight (pcf) g = | 133.04 | 147.83 | |
| Dry Unit Weight (pcf) $g_d =$ | 116.72 | 133.65 | |

Prepared by: 1: Checked by:

Date: 6/10/14 Date: 6/10/14





DENSITY-UNIT WEIGHT DETERMINATION ASTM D 7263

Client: Parsons Transportation Group, Inc. Project: US 52 / IL 64 / IL 84 WEI Job No: 342-06-02

Analyst name: A. Mohammed Test date: June 11, 2014

| Sample #: Water content determination | 84-RWB-22B Run 1, 35.0ft | |
|--|-----------------------------|--|
| Mass of tare and wet soil (g) $W_w =$ | 152.44 | |
| Mass of tare and dry soil (g) $W_d =$ | 148.71 | |
| Mass of tare (g) $W_t =$ | 22.49 | |
| Water content w = | 3% | |

Density--Unit Weight

| Diameter measurements (in) $D_1 =$ | 2.045 | |
|------------------------------------|---------|--|
| D ₂ = | 2.044 | |
| D ₃ = | 2.046 | |
| Average diameter (in) D = | 2.045 | |
| Height measurements (in) H_1 = | 0.916 | |
| H ₂ = | 0.906 | |
| H ₃ = | 0.908 | |
| Average height (in) H = | 0.910 | |
| Total weight (g) W = | 129.950 | |
| Bulk Unit Weight (pcf) g = | 165.66 | |
| Dry Unit Weight (pcf) $g_d =$ | 160.90 | |

Prepared by: Checked by:

_____ Date: <u>6 //0/14</u> _____ Date: <u>4/16/14</u>





DENSITY-UNIT WEIGHT DETERMINATION ASTM D 7263

Client: Parsons Transportation Group, Inc.

Project: US 52 / IL 64 / IL 84 WEI Job No: 342-06-02 Analyst name: A. Mohammed Test date: June 19, 2014

| Sample #: Water content determination | 84-RWB-03A Run 2, 32.0ft |
|--|------------------------------------|
| Mass of tare and wet soil (g) $W_w =$ | 571.10 |
| Mass of tare and dry soil (g) $W_d =$ | 544.00 |
| Mass of tare (g) $W_t =$ | 22.06 |
| Water content w = | 5% |
| DensityUnit Weight | |
| Diameter measurements (in) $D_1 =$ | 2.050 |
| D ₂ = | 2.040 |
| D ₃ = | 2.050 |
| Average diameter (in) D = | 2.047 |
| Height measurements (in) $H_1 =$ | 4.074 |
| H ₂ = | 4.094 |
| H ₃ = | 4.102 |
| Average height (in) H = | 4.090 |
| Total weight (g) W = | 549.040 |
| Bulk Unit Weight (pcf) g = | 155.47 |
| Dry Unit Weight (pcf) $g_d =$ | 147.80 |

C Prepared by: _ Checked by:

_____Date:_____6/10/14 _____Date:_____6/19/14







Project: US 52 / IL 64 / IL 84

Client: Parsons Transportation Group, Inc.

WEI Job No.: 342-06-02

Note: The specimens were sulphur capped for a more uniform break

| Field Sample ID | Lab Specimen ID | Depth (ft) | Location | Sample Description | Leng Before Capping | th(in) After Capping | Diameter (in) | Total Load (lbs) | Total Pressure (psi) | Fracture Type* | Break Date | Tested By | Area (in ²) |
|---------------------|--------------------|------------|------------|-----------------------|---------------------------|----------------------------|------------------|------------------------|----------------------------|-------------------|---------------|-----------|-------------------------|
| 84-RWB-01B RUN 3 | 9656 | 34.6 | IL-84 Wall | Shale | 3.99 | 4.19 | 2.05 | 9250 | 2800 | 3 | 5/31/14 | АМ | 3.30 |
| 84-RWB-01B RUN 3 | 9657 | 38.5 | IL-84 Wall | Shale | 4.26 | 4.42 | 2.05 | 24970 | 7570 | 3 | 5/31/14 | АМ | 3.30 |

* Fracture Types:

Type 1 - Reasonably well-formed cones on both ends, less than 1 in. [25 mm] of cracking through caps;

Type 2 - Well-formed cone on one end, vertical cracks running through caps, no well defined cone on other end;

Type 3 - Columnar vertical cracking through both ends, no well-formed cones;

Type 4 - Diagonal fracture with no cracking through ends; tap with hammer to distinguish from Type 1;

Type 5 - Side fractures at top or bottom (occur commonly with unbonded caps);

1. L. Cholig Prepared by:_ Checked by:





Project: US 52 / IL 64 / IL 84

Client: Parsons Transportation Group, Inc.

WEI Job No.: 342-06-02

Note: The specimens were sulphur capped for a more uniform break

| | | | | | | | 1 | | | | | | |
|--------------------|-------------|------------|------------|-------------|---------|---------|----------|-------|----------|----------|---------|-----------|-------------------------|
| | | | | | Leng | th (in) | | Total | Total | | | | |
| Field | Lab | | | Sample | Before | After | Diameter | Load | Pressure | Fracture | Break | | |
| Sample ID | Specimen ID | Depth (ft) | Location | Description | Capping | Capping | (in) | (lbs) | (psi) | Type* | Date | Tested By | Area (in ²) |
| 84-RWB-20 RUN 2 | 9522 | 41.5 | IL-84 Wall | Shale | 3.78 | 3.93 | 2.05 | 20970 | 6420 | 3 | 5/14/14 | AM | 3.27 |
| 84-RWB-20 RUN 3 | 9523 | 44.5 | IL-84 Wall | Shale | 4.02 | 4.17 | 2.05 | 17170 | 5250 | 3 | 5/14/14 | АМ | 3.27 |
| 84-RWB-20 RUN 3 | 9524 | 50.5 | IL-84 Wall | Shale | 3.98 | 4.11 | 2.05 | 25050 | 7660 | 3 | 5/14/14 | АМ | 3.27 |
| | | | | | | | | | | | | | |
| | | | | | | | | | | | | | |
| | | | | | | | | | | | | | |

* Fracture Types:

Type 1 - Reasonably well-formed cones on both ends, less than 1 in. [25 mm] of cracking through caps;

Type 2 - Well-formed cone on one end, vertical cracks running through caps, no well defined cone on other end;

Type 3 - Columnar vertical cracking through both ends, no well-formed cones;

Type 4 - Diagonal fracture with no cracking through ends; tap with hammer to distinguish from Type 1;

Type 5 - Side fractures at top or bottom (occur commonly with unbonded caps);

Prepared by:_____

Checked by: A: F 410/14





Project: US 52 / IL 64 / IL 84

Client: Parsons Transportation Group, Inc.

WEI Job No.: 342-06-02

Note: The specimens were sulphur capped for a more uniform break

| Field Sample ID | Lab Specimen ID | Depth (ft) | Location | Sample Description | Leng Before Capping | th(in) After Capping | Diameter (in) | Total Load (lbs) | Total Pressure (psi) | Fracture Type* | Break Date | Tested By | Area (in ²) |
|--------------------|--------------------|------------|------------|-----------------------|---------------------------|----------------------------|------------------|------------------------|----------------------------|-------------------|---------------|-----------|-------------------------|
| 84-RWB-19 RUN 2 | 9518 | 33.0 | IL-84 Wall | Shale | 4.10 | 4.21 | 2.03 | 11700 | 3630 | 3 | 5/14/14 | АМ | 3.22 |
| 84-RWB-19 RUN 2 | 9519 | 37.5 | IL-84 Wall | Shale | 3.89 | 4.04 | 2.03 | 23390 | 7260 | 3 | 5/14/14 | АМ | 3.22 |
| 84-RWB-19 RUN 3 | 9520 | 42.0 | IL-84 Wall | Shale | 3.90 | 4.09 | 2.05 | 26700 | 8170 | 3 | 5/14/14 | АМ | 3.27 |
| 84-RWB-19 RUN 3 | 9521 | 46.5 | IL-84 Wall | Shale | 3.94 | 4.13 | 2.05 | 30780 | 9420 | 3 | 5/14/14 | AM | 3.27 |
| | | | | | | | | | | | | | |
| | | | | | | | | | | | | | |

* Fracture Types:

Type 1 - Reasonably well-formed cones on both ends, less than 1 in. [25 mm] of cracking through caps;

Type 2 - Well-formed cone on one end, vertical cracks running through caps, no well defined cone on other end;

Type 3 - Columnar vertical cracking through both ends, no well-formed cones;

Type 4 - Diagonal fracture with no cracking through ends; tap with hammer to distinguish from Type 1;

Type 5 - Side fractures at top or bottom (occur commonly with unbonded caps);

ifdd 6 f 6/10/14 Prepared by:_ Checked by:





Project: US 52 / IL 64 / IL 84

Client: Parsons Transportation Group, Inc.

WEI Job No.: 342-06-02

Note: The specimens were sulphur capped for a more uniform break

| Field Sample ID | Lab Specimen ID | Depth (ft) | Location | Sample Description | Leng Before Capping | th (in) After Capping | Diameter (in) | Total Load (lbs) | Total Pressure (psi) | Fracture Type* | Break Date | Tested By | Area (in ²) |
|---------------------|--------------------|------------|------------|-----------------------|---------------------------|-----------------------------|------------------|------------------------|----------------------------|-------------------|---------------|-----------|-------------------------|
| 84-RWB-22B RUN 1 | 9530 | 36.0 | IL-84 Wall | Shale | 3.79 | 3.93 | 2.05 | 16090 | 4920 | 3 | 5/14/14 | АМ | 3.27 |
| 84-RWB-22B RUN 1 | 9531 | 39.0 | IL-84 Wall | Shale | 4.26 | 4.39 | 2.05 | 19400 | 5930 | 3 | 5/14/14 | АМ | 3.27 |
| 84-RWB-22B RUN 2 | 9532 | 40.0 | IL-84 Wali | Shale | 3.90 | 4.07 | 2.05 | 19490 | 5960 | 3 | 5/14/14 | АМ | 3.27 |
| 84-RWB-22B RUN 2 | 9533 | 46.0 | IL-84 Wall | Shale | 3.91 | 4.09 | 2.05 | 20060 | 6140 | 3 | 5/14/14 | АМ | 3.27 |
| | | | | | | | | | | | | | |
| | | | | | | | | | | | | | |

* Fracture Types:

Type 1 - Reasonably well-formed cones on both ends, less than 1 in. [25 mm] of cracking through caps;

Type 2 - Well-formed cone on one end, vertical cracks running through caps, no well defined cone on other end;

Type 3 - Columnar vertical cracking through both ends, no well-formed cones;

Type 4 - Diagonal fracture with no cracking through ends; tap with hammer to distinguish from Type 1;

Type 5 - Side fractures at top or bottom (occur commonly with unbonded caps);

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Project: US 52 / IL 64 / IL 84

Client: Parsons Transportation Group, Inc.

WEI Job No.: 342-06-02

Note: The specimens were sulphur capped for a more uniform break

| Field | Lab Specimen | Depth (ft) | Location | Sample | Leng Before Capping | th (in) After | Diameter | Total Load | Total Pressure | Fracture | Break | Tested By | Area (in ²) |
|---------------------|-----------------|------------|------------|--------|---------------------------|------------------|----------|---------------|-------------------|----------|---------|-----------|-------------------------|
| 84-RWB-03A RUN 2 | 9871 | 32.0 | IL-84 Wall | Shale | 4.09 | 4.12 | 2.05 | 3400 | 1030 | 3 | 6/19/14 | AM | 3.30 |

* Fracture Types:

Type 1 - Reasonably well-formed cones on both ends, less than 1 in. [25 mm] of cracking through caps;

Type 2 - Well-formed cone on one end, vertical cracks running through caps, no well defined cone on other end;

Type 3 - Columnar vertical cracking through both ends, no well-formed cones;

Type 4 - Diagonal fracture with no cracking through ends; tap with hammer to distinguish from Type 1;

Type 5 - Side fractures at top or bottom (occur commonly with unbonded caps);

Prepared by: _______ Kife 6/10/14




Unconfined Compressive Strength of Intact Rock Core Specimens

Project: US 52 / IL 64 / IL 84

Client: Parsons Transportation Group, Inc.

WEI Job No.: 342-06-02

Note: The specimens were sulphur capped for a more uniform break

| Field Sample ID | Lab Specimen ID | Depth (ft) | Location | Sample Description | Leng Before Capping | th (in) After Capping | Diameter (in) | Total Load (lbs) | Total Pressure (psi) | Fracture Type* | Break Date | Tested By | Area (in ²) |
|--------------------|--------------------|------------|------------|-----------------------|---------------------------|-----------------------------|------------------|------------------------|----------------------------|-------------------|---------------|-----------|-------------------------|
| 84-RWB-21 RUN 1 | 9525 | 40.5 | IL-84 Wall | Shale | 3.90 | 4.10 | 2.05 | 13570 | 4150 | 3 | 5/14/14 | АМ | 3.27 |
| 84-RWB-21 RUN 1 | 9526 | 44.5 | IL-84 Wall | Shale | 3.94 | 4.18 | 2.05 | 11390 | 3490 | 3 | 5/14/14 | AM | 3.27 |
| 84-RWB-21 RUN 2 | 9527 | 45.5 | IL-84 Wall | Shale | 3.91 | 4.05 | 2.05 | 16000 | 4890 | 3 | 5/14/14 | АМ | 3.27 |
| 84-RWB-21 RUN 3 | 9528 | 54.5 | IL-84 Wall | Shale | 3.79 | 3.92 | 2.05 | 11750 | 3590 | 3 | 5/14/14 | AM | 3.27 |
| 84-RWB-21 RUN 4 | 9529 | 55.5 | IL-84 Wall | Shale | 3.86 | 3.95 | 2.05 | 9570 | 2930 | 3 | 5/14/14 | АМ | 3.27 |
| | | | | | | | | | | | | | |

* Fracture Types:

Type 1 - Reasonably well-formed cones on both ends, less than 1 in. [25 mm] of cracking through caps;

Type 2 - Well-formed cone on one end, vertical cracks running through caps, no well defined cone on other end;

Type 3 - Columnar vertical cracking through both ends, no well-formed cones;

Type 4 - Diagonal fracture with no cracking through ends; tap with hammer to distinguish from Type 1;

Type 5 - Side fractures at top or bottom (occur commonly with unbonded caps);

Type 6 - Similar to Type 5 but end of cylinder is pointed.

sifden 1. f. 6/10/19 Prepared by: (1 Checked by:





Unconfined Compressive Strength of Intact Rock Core Specimens

Project: US 52 / IL 64 / IL 84

Client: Parsons Transportation Group, Inc.

WEI Job No.: 342-06-02

Note: The specimens were sulphur capped for a more uniform break

| Field Sample ID | Lab Specimen ID | Depth (ft) | Location | Sample Description | Leng Before Capping | th (in) After Capping | Diameter (in) | Total Load (lbs) | Total Pressure (psi) | Fracture Type* | Break Date | Tested By | Area (in ²) |
|--------------------|-----------------------|------------|------------|-----------------------|---------------------------|-----------------------------|------------------|------------------------|----------------------------|-------------------|---------------|-----------|-------------------------|
| 84-RWB-12 RUN 4 | 9667 | 56.5 | IL-84 Wall | Shale | 3.85 | 3.98 | 2.05 | 8610 | 2610 | 3 | 5/31/14 | AM | 3.30 |
| 84-RWB-12 RUN 4 | 9663 | 61.5 | IL-84 Wall | Shale | 3.99 | 4.20 | 2.05 | 3720 | 1130 | 3 | 5/31/14 | AM | 3.30 |
| 84-RWB-16 RUN 3 | 9659 | 50.0 | IL-84 Wall | Shale | 3.89 | 4.22 | 2.05 | 17110 | 5180 | 3 | 5/31/14 | AM | 3.30 |
| 84-RWB-17 RUN 3 | 9664 | 50.0 | IL-84 Wall | Shale | 4.01 | 4.27 | 2.05 | 28630 | 8670 | 3 | 5/31/14 | АМ | 3.30 |

* Fracture Types:

Type 1 - Reasonably well-formed cones on both ends, less than 1 in. [25 mm] of cracking through caps;

Type 2 - Well-formed cone on one end, vertical cracks running through caps, no well defined cone on other end;

Type 3 - Columnar vertical cracking through both ends, no well-formed cones;

Type 4 - Diagonal fracture with no cracking through ends; tap with hammer to distinguish from Type 1;

Type 5 - Side fractures at top or bottom (occur commonly with unbonded caps);

Type 6 - Similar to Type 5 but end of cylinder is pointed.

hif Gliolia Prepared by: Checked by:



APPENDIX C







| | FLUI DHIE - 8/13/2014 | CHECKED - GH | REVISED - | | SHEET NO. 5 UP 5 SHEETS | | ILLINOIS FED. AI | ID PROJECT | _ |
|---------|-----------------------|---------------|-----------|------------------------------|-------------------------|-------|------------------|--------------------|---|
| | BLOT DATE = 8/12/2014 | | PEVICED | | | | | | - |
| PARSONS | PLOT SCALE = | DRAWN - SQ | REVISED - | DEPARTMENT OF TRANSPORTATION | | | | CONTRACT NO. 64G59 | 1 |
| | DATE 1730/14 | CHECKED - GH | REVISED - | STATE OF ILLINUIS | | 17 | 104B-2 | CARROLL | |
| | DATE - 7/30/14 | CHECKED OU | DEVICED | | | RIE. | | SHEETS NU. | 4 |
| | USER NAME = p005313A | DESIGNED - JC | REVISED - | | | F.A.P | SECTION | COUNTY TOTAL SHEE | 7 |



<u>SECTION THRU</u> DRILLED SOLDIER PILE WALL

- * Where necessary, to be determined during final design. No anchors will be used from Sta. 709+65 to Sta. 711+30 due to right-of-way restrictions.
- ** Pile section, spacing, tip elevation and shaft diameter to be determined during final design.
- *** Pipe underdrain to outlet to nearest proposed inlet.





APPENDIX D





































