3'' Radius

Top of Beam

3 - ½'' φ 270 ksi strands

Fan ±6"

LIFTING LOOP DETAIL

1 4" \$ Sch. 40 Steel

Pipe or equivalent



Contract #70258 ‡ 22VBR-1 and 144SBR-2

NOTES

Inserts for $^34''$ 4 threaded dowel rods, when specified, are to be two strut, coil type for interior beams and single coil, flared loop type for exterior beams.

Prestressing steel shall be uncoated high strength, low relaxation 7-wire strand, Grade 270. The nominal diameter shall be $^{\rm l}_2$ '' and the nominal cross-sectional area shall be 0.153 sq. in.

Non-prestressing steel shall conform to AASHTO designation M-31 or M 322, Grade 60.

A minimum $2^l z'' \phi$ lifting pin shall be used to engage the lifting loops during handling.

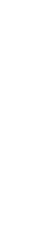
Reinforcement bars designated (E) shall be epoxy coated.

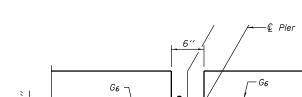
Cut G_6 bars when necessary to maintain $l_2^{\prime\prime}$ clearance.

The bottom plates and studs shall be galvanized according to AASHTO MIII and ASTM A385.

Threaded rods shall be ASTM F 1554 Grade 55.

The cut strands at each beam end shall be given two coats of zinc dust spray or paint meeting the requirements of ASTM A 780. The zinc dust spray or paint shall be applied before corrosion appears and allowed to dry according to the manufacturer's specifications prior to another coat of zinc. A concrete sealer meeting the requirements of Section 587 of the Standard Specifications shall be applied to all portions of the I-beam or Bulb-T beam, except the top surface of the top flange and the bottom surface of the bottom flange, starting at each beam end and extending out a distance of 42 inches. The sealer shall be applied after visible crack growth has subsided. This work shall be performed by the producer and included with the cost of the beam.





ELEVATION OF BEAM AT PIER

To outside face

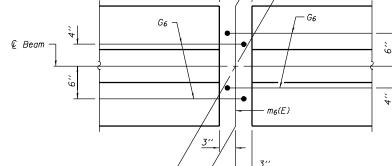
 $m_6(E)$

of bar, typ.

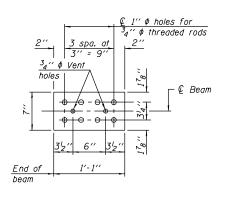
Bottom of beam

End of beams

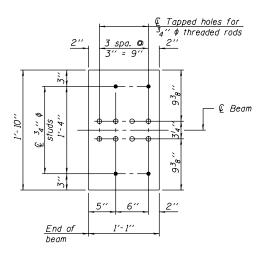
No. 9 wire ties (See sheet 13 of 46).



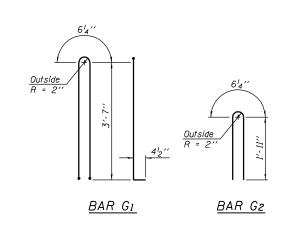
PLAN OF BEAM AT PIER

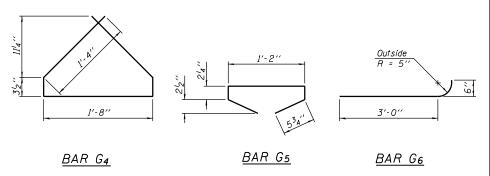


TOP PLATE



BOTTOM PLATE





BILL OF MATERIAL

Item	Unit	Total
Furnishing and Erecting Precast Prestressed Concrete I-Beams, 42"	Ft.	1195

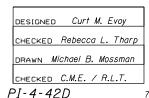
42" PPC I-BEAM DETAILS

F.A.S. RT. 1671 - SEC. 22VBR-1

DOUGLAS COUNTY

STATION 1151+65.86

STRUCTURE NO. 021-0061



August 4, 2006

EXAMINED Thomas Lomas Like

PASSED Galph E. M. Carson

ENGINEER OF BRIDGES AND STRUCTURES

7-15-05

item which is the property of the Illinois Department of Transportation. Use, reproduction or disclosure without express written permission is prohibited and protected under Federal copyright laws. The production and 'd'' 🖣 Holes with zerk the fabrication of this bolt for use on highway projects for epoxy grout in the State of Illinois shall be permitted and there shall be no incurred charges or fees to the manufacturer or the fabricator for producing or fabricating this bolt. ′′D′′ Ø 1316 " 134" 1³8′′ 1/16'' Anchor Bolt (See Bearing Details 12" 1⁵8′′ 1⁵16′′ 218'' \equiv for number, size and length.) $\pm \mathbb{F}$ 2" 28" 1¹³16 '' 278" =258" 2516" 338" 垂 ▆▋ # Top of base plate == End of End of groove coil lock $^{5}_{32}$ " wide x $^{3}_{32}$ " deep groove in anchor bolt with 18" \$ of coil PLAN-COIL WIRE

'R" Notch

ILLINOIS COIL-LOCK ANCHOR BOLT

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY		TOTAL SHEETS	9-667 NO.	SHEET NO. <i>18</i>
F.A.S. 1671	†	DOUG	LAS	181	102	46 SHEETS
FED. ROAD DIST	. NO. 7	ILL INDIS	OIS PED. AID PROJECT-			

Contract #70258 ‡ 22VBR-1 and 144SBR-2

MATERIALS FOR ILLINOIS COIL-LOCK ANCHOR BOLT

The anchor bolt shall be fabricated from cold drawn or hot finished seamless carbon steel mechanical tubing conforming to ASTM A 519, Grade 1026, CW and supplied with hexagonal nuts and cut washers.

The coil wire shall be made of any suitable soft steel wire.

The finished anchor bolt shall be cleaned of rust and other foreign materials and wrapped or packaged to prevent contamination until they are installed. The epoxy grout shall be a two-component, epoxy resin bonding system conforming to ASTM C 881, Type I, Grade 1 and of a Class suitable for the temperature at installation.

INSTALLATION PROCEDURE for the ILLINOIS COIL-LOCK ANCHOR BOLT

- 1. With the coil wire in place, the bolt shall be inserted into the hole and turned clockwise to a snug fit in the hole. Nut and washer shall be placed on the bolt. The nut shall be tensioned until the steel base plates are held securely to the concrete bearing seat.
- 2. Epoxy grout shall be pumped through the zerk fitting with a pressure gun. Pumping shall continue until the epoxy overflows the hole around the bolt shank. After pumping is discontinued, excess epoxy shall be immediately wiped off.

ALTERNATE ANCHOR BOLTS

The Contractor may use, at his option, the capsule or the adhesive cartridge type anchor rods that have been previously tested and given a prior approval by the Department. The Contractor shall install these anchor rods in pre-drilled holes according to the manufacturer's recommendations and procedures.

The capsule or the adhesive cartridge type anchor rods shall be a two part system composed of:

- 1. A threaded rod stud with nut and washer of the type specified.
- 2. A sealed glass capsule or a sealed glass adhesive cartridge containing premeasured amounts of the adhesive chemical.

Location	Туре
· ·	4 70 7
Piers	A 307

ASTM F 1554 Grade 105, ASTM A 449 and AASHTO M 314 Grade 105 anchor bolts may be substituted for the anchor bolts shown above.

GENERAL NOTES

Holes in the masonry for anchor bolts shall be drilled through the base plates to the diameter and depth shown or according to the manufacturer's recommendation after beams or girders have been erected and adjusted. Prior to setting the bolts, the holes shall be dry and all dust and loose particles shall be removed by the use of compressed air or vacuuming.

The anchor bolts, furnished and installed and including the epoxy grout or capsules shall not be paid for separately but shall be included in the unit bid price for Concrete Structures.

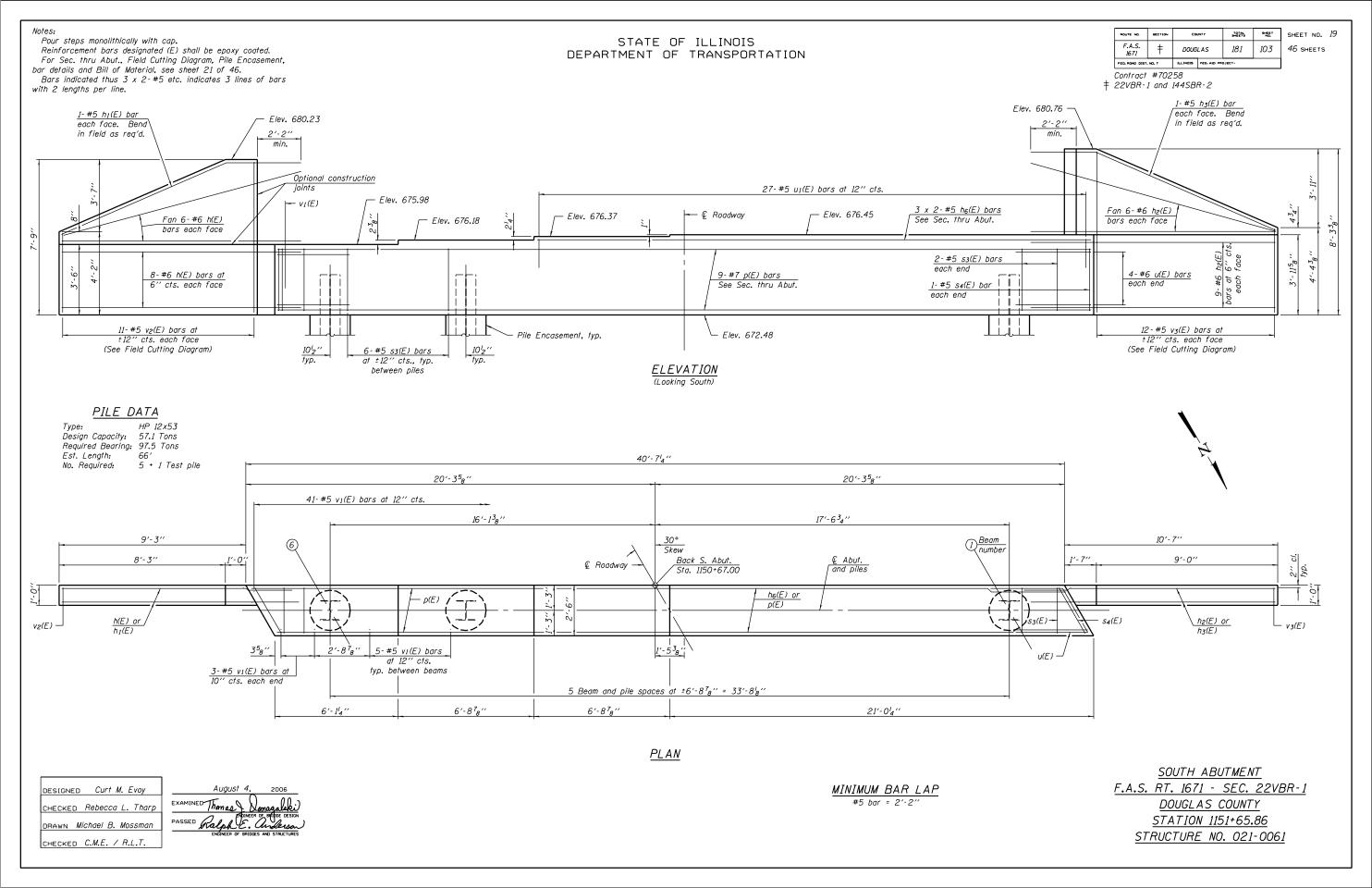
> ANCHOR BOLT DETAILS F.A.S. RT. 1671 - SEC. 22VBR-1 DOUGLAS COUNTY STATION 1151+65.86 STRUCTURE NO. 021-0061

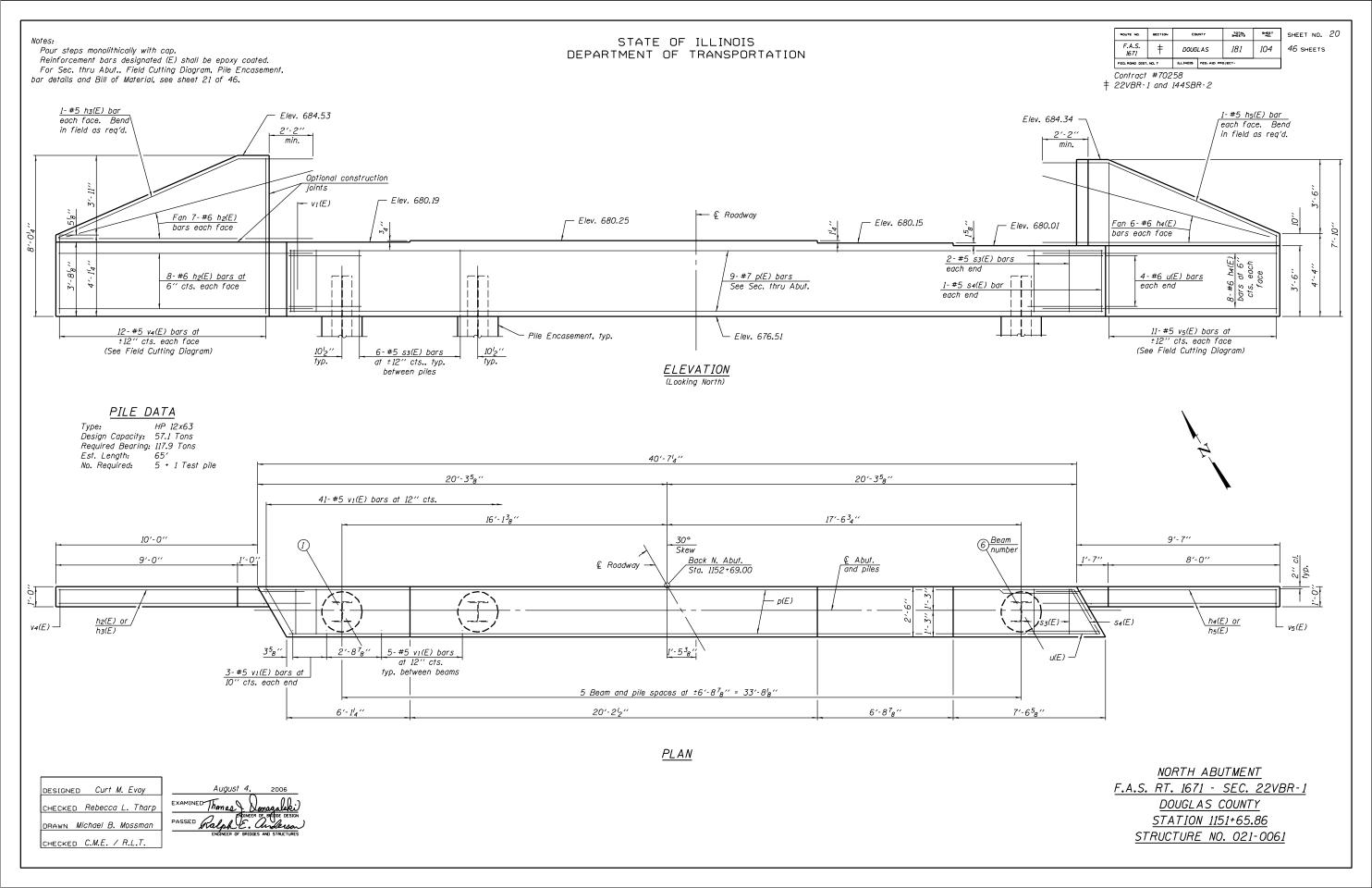
DESIGNED Curt M. Evoy CHECKED Rebecca L Tharp DRAWN Michael B. Mossman

CHECKED C.M.E. / R.L.T.

The Illinois Coil-Lock Anchor Bolt is a proprietary

ABB-1 10-22-04





ROUTE NO.	SECTION	cox	UNTY	TOTAL SHEETS	9-667 NO.	SHEET NO. 21
F.A.S. 1671	‡	DOUGLAS		181	105	46 SHEETS
FED. ROAD DIST	NO. 7	ILL INDIS	FED. AID FR	DJECT-		

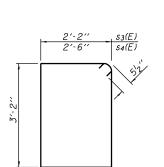
Contract #70258 ‡ 22VBR-1 and 144SBR-2

<u>SOUTH ABUTMENT</u> BILL OF MATERIAL

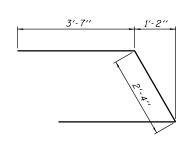
Bar	No.	Size	Length	Shape
h(E)	28	#6	12'-4''	
hį(E)	2	#5	12′-6′′	
h2(E)	30	#6	13'-1''	
h3(E)	2	#5	13'-4''	
h6(E)	6	#5	14′-10′′	
p(E)	9	#7	40'-3''	
s3(E)	34	#5	11'-7''	
s4(E)	2	#5	12′-3′′	
u(E)	8	#6	9'-6'	
u1(E)	27	#5	4'-8''	
v1(E)	72	#5	4'-4''	
v2(E)	11	#5	11'-3''	
ν3(E)	12	#5	11'-11''	
	te Struc		Cu. Yd.	19.4
Reinfor Epoxy	cement Coated	Bars,	Pound	3280
Furnishing Steel Piles HP 12x53			Foot	330
Driving	Steel F	Piles	Foot	330
Test Pile Steel HP 12x53			Each	1
Structu	ire Exc	avation	Cu. Yd.	95

<u>NORTH ABUTMENT</u> BILL OF MATERIAL

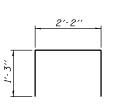
				_
Bar	No.	Size	Length	Shape
h2(E)	30	#6	13'-1''	
h3(E)	2	#5	13'-4''	
h4(E)	28	#6	12'-0''	
h5(E)	2	#5	12'-4''	
ρ(Ε)	9	#7	40'-3''	
53(E)	34	#5	11'-7''	
54(E)	2	#5	12'-3''	
u(E)	8	#6	9'-6''	
				_
v _I (E)	72	#5	4'-4''	
v4(E)	12	#5	11'-5''	
v5(E)	11 #5		11'-6''	
	te Struc		Cu. Yd.	18.5
	cement Coated	Bars,	Pound	3040
		ol Piloc		
Furnishing Steel Piles HP 12x63			Foot	325
Driving Steel Piles			Foot	325
Test Pile Steel HP 12x63			Each	1
	ire Exco	vation	Cu. Yd.	95



BARS 53(E) & 54(E)



BAR u(E)



<u>BAR uı(E)</u>

SECTION THRU SOUTH ABUTMENT

2'-6"

€ Abut.

1'-3''

and piles

 $v_I(E)$

typ.

 $u_I(E)$

2" cl. typ. s3(E) or s4(E) h6(E)

<u>SECTION THRU</u> NORTH ABUTMENT

2'-6"

v1(E)

typ.

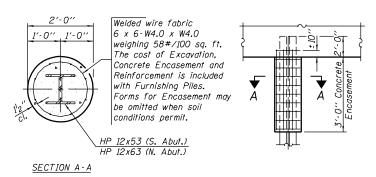
€ Abut.

1'-3''

and piles

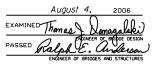
2" cl. typ. s3(E) or s4(E)

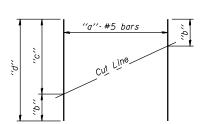
p(E) -



PILE ENCASEMENT DETAIL

DESIGNED	Curt M. Evoy
CHECKED	Rebecca L. Tharp
DRAWN Mid	chael B. Mossman
CHECKED (C.M.E. / R.L.T.





FIELD CUTTING DIAGRAM

Order $v_2(E)$ thru $v_5(E)$ full length. Cut as shown and use remainder of bars in opposite face.

Bar	''a''	′′b′′	′′c′′	′′d′′
v2(E)	11	3'-10''	7′-5′′	11'-3''
ν3(E)	12	4'-0''	7'-11''	11'-11''
v4(E)	12	3'-9"	7′-8′′	11'-5''
vs(F)	11	4'-0"	7′-6′′	11'-6''

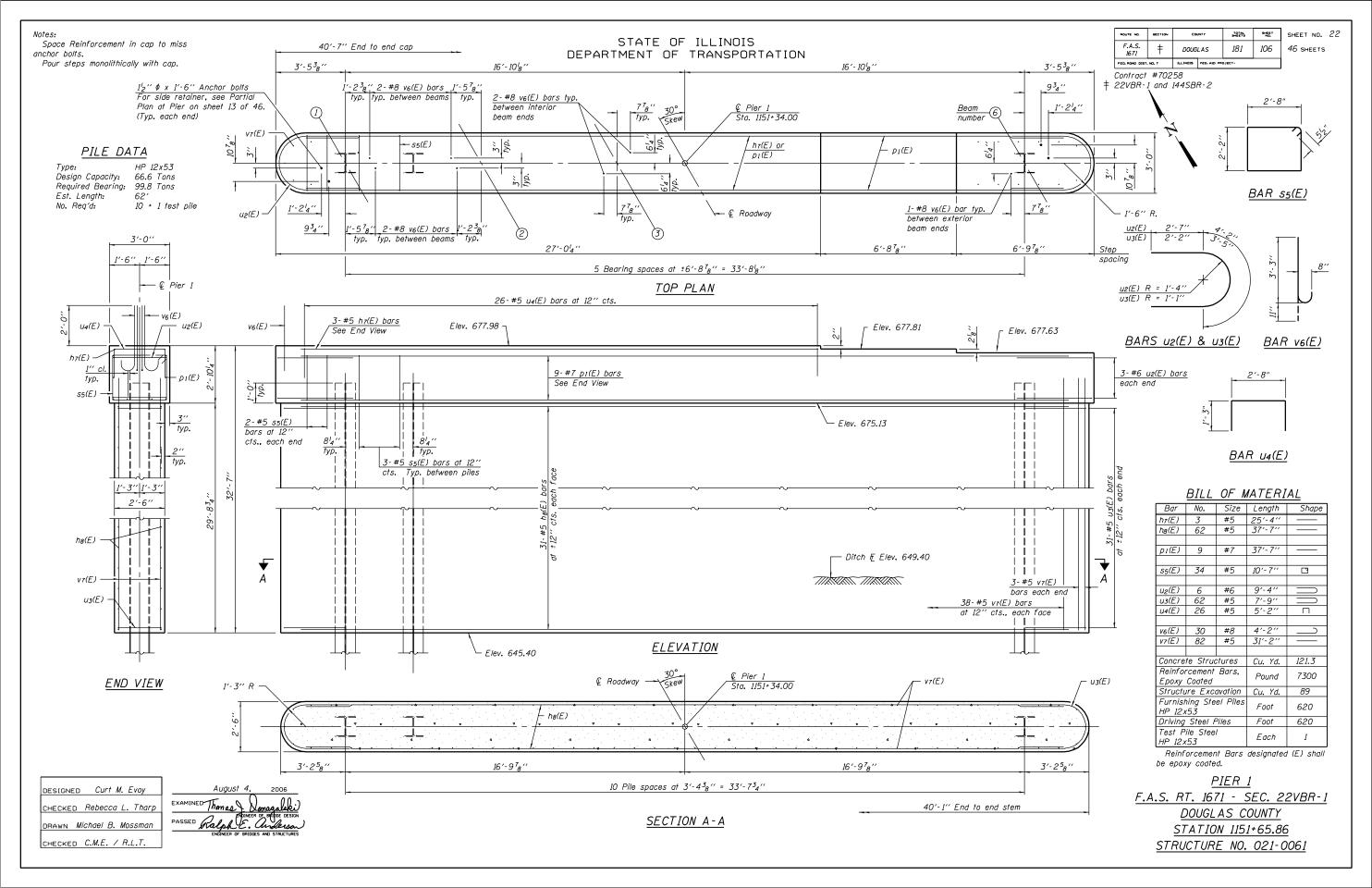
ABUTMENT DETAILS

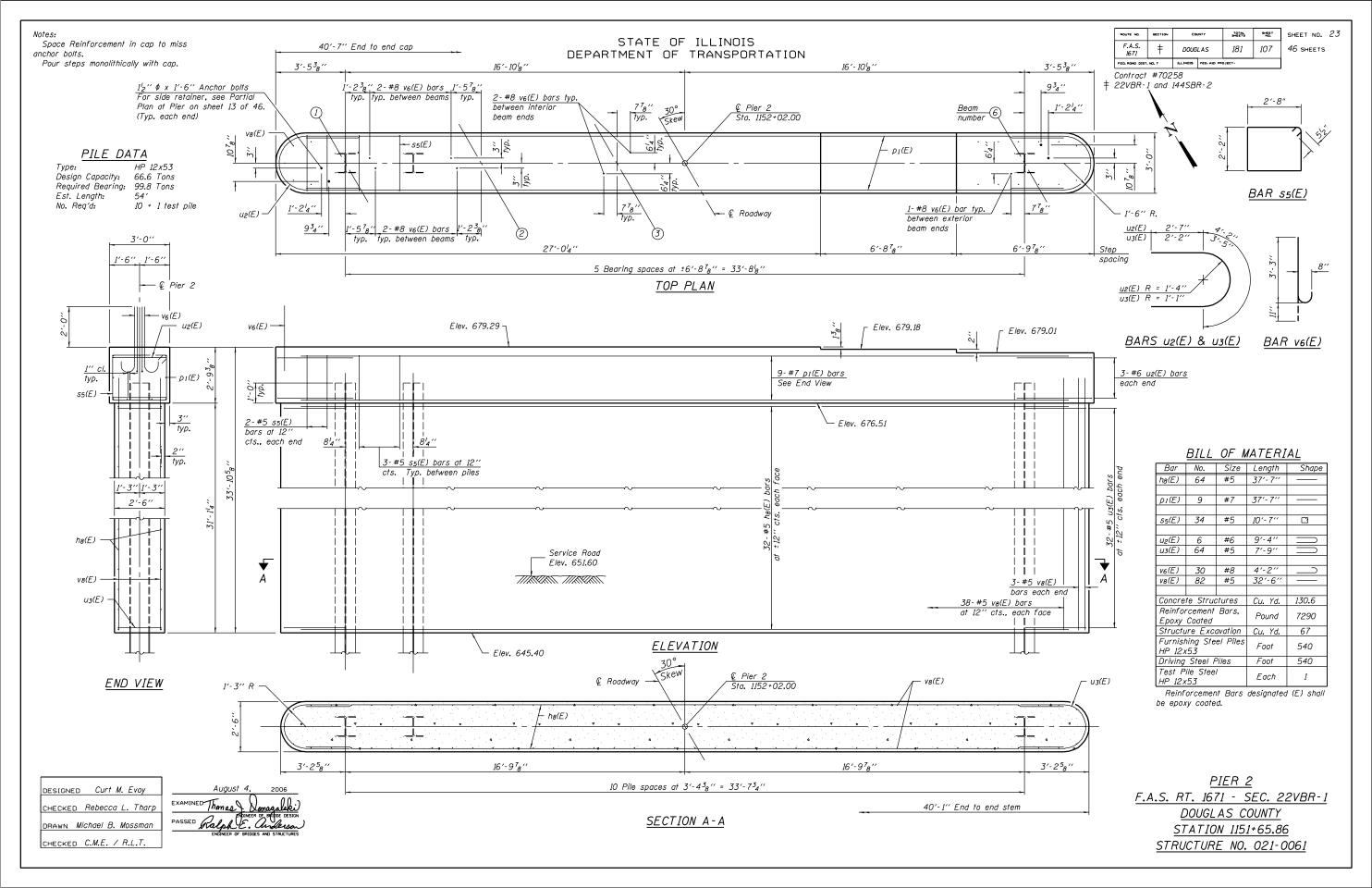
F.A.S. RT. 1671 - SEC. 22VBR-1

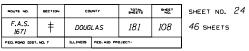
DOUGLAS COUNTY

STATION 1151+65.86

STRUCTURE NO. 021-0061







Contract #70258 ‡ 22VBR-1 and 144SBR-2

NOTES

Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars.

Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length. All reinforcement bars shall be lapped and tied to the splicer rods or dowel bars. Bar splicer assemblies shall be epoxy coated according to the requirements for

reinforcement bars. Other systems of similar design may be submitted to the Engineer for approval. Approval shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

Minimum Capacity (Tension in kips) = $1.25 \times fy \times A_t$

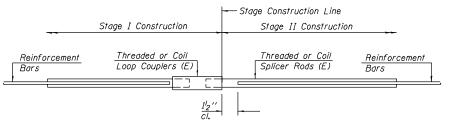
Minimum *Pull-out Strength = 1.25 x fs_{allow} x A₁

Where fy = Yield strength of lapped reinforcement bars in ksi.

fsallow = Allowable tensile stress in lapped reinforcement bars in ksi (Service Load) A_t = Tensile stress area of lapped reinforcement bars. * = 28 day concrete

BAR SPLICER ASSEMBLIES						
		Strength Requirements				
Bar Size to be Spliced	Splicer Rod or Dowel Bar Length	Min. Capacity kips - tension				
#4	1′-8′′	14.7	5.9			
#5	2'-0''	23.0	9.2			
#6	2′-7′′	33.1	13.3			
#7	3′-5″	45.1	18.0			
#8	4'-6''	58.9	23.6			
#9	5′-9′′	75.0	30.0			
#10	7′-3′′	95.0	38.0			
#11	9′-0′′	117.4	46.8			

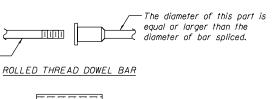
Bar splicer assemblies shall be according to Section 508 of the Standard Specifications, except as noted. The furnishing and installation of bar splicer assemblies will be measured and paid for at the contract unit price each for "BAR SPLICERS."



STANDARD

	Bar Size	No. Assemblies Required	Location
Γ			

BAR SPLICER ASSEMBLY DETAILS F.A.S. RT. 1671 - SEC. 22VBR-1 DOUGLAS COUNTY STATION 1151+65.86 STRUCTURE NO. 021-0061



** ONE PIECE - Wire Connector ווויווויווו **WELDED SECTIONS**

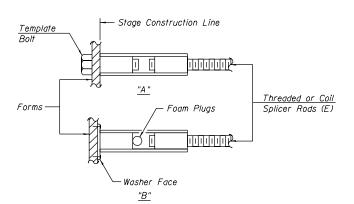
The diameter of this part

of the bar spliced.

is the same as the diameter

BAR SPLICER ASSEMBLY ALTERNATIVES

** Heavy Hex Nuts conforming to ASTM A 563, Grade C, D or DH may be used.

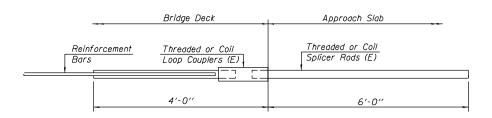


INSTALLATION AND SETTING METHODS

"A": Set bar splicer assembly by means of a template bolt. "B": Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E): Indicates epoxy coating.

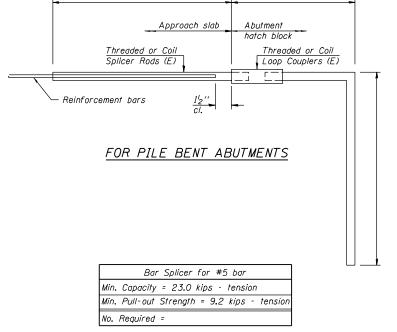
6'-0"



FOR INTEGRAL OR SEMI-INTEGRAL ABUTMENTS

Bar Splicer for #5 bar Min. Capacity = 23.0 kips - tension Min. Pull-out Strength = 9.2 kips - tension No. Required = 64

DESIGNED Curt M. Evoy CHECKED Rebecca L. Tharp DRAWN Michael B. Mossman CHECKED C.M.E. / R.L.T.



10-22-04

BSD-1

SECTION THRU INTEGRAL ABUTMENT (Horiz, dim, @ Rt, L's)

* Included in the cost of Pipe Underdrains for Structures, 4".

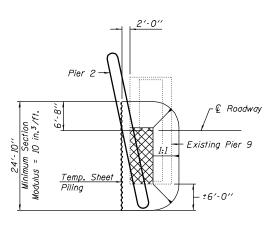
All drainage system components shall extend to 2'-0" from the end of each wingwall except an outlet pipe shall extend until intersecting with the side slopes. The pipes shall drain into concrete headwalls. (See Article 601.05 of the Standard Specifications and Highway Standard 601101).



Hatched area indicates Removal



to be removed as required to install piles for proposed pier. Cost included with Removal of Existing Structures.

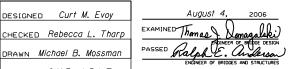


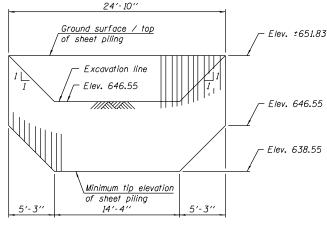
<u>PL A N</u> (at Pier 2)

DESIGNED Curt M. Evoy

DRAWN Michael B. Mossman

CHECKED C.M.E. / R.L.T.





If the Contractor chooses to alter the temporary cantilevered sheet piling design requirements shown on the plans, a design submittal including plan details and calculations will be required for review and acceptance by the Engineer.

ELEVATION

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

INDEX OF SHEETS

(for 021-0060)

25 General Details 31 Top of Slab Elevations

Superstructure

33

Superstructure Details 34 Diaphraam Details

Structural Steel 35 36 Structural Steel Details

37 Bearing Details 38

Anchor Bolt Details

39 South Abutment 40 North Abutment

41 Pier 1 42 Pier 2

43 Bar Splicer Assembly Details

STATION 1154+99.02 BUILT 200 BY STATE OF ILLINOIS F.A.S. RT. 1671 SEC. 22VBR-1 LOADING HS20-44 STR. NO. 021-0060

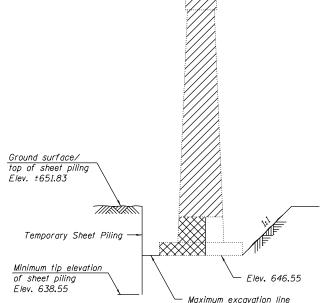
> NAME PLATE See Std. 515001

CONSTRUCTION SEQUENCE

- Remove the superstructures on both sides of the existing Pier 9.
- Drive temporary sheeting at the location shown and excavate to the limits shown on the plan and elevation view.
- Remove the existing substructure per plans and Section 501 of the Standard Specifications.
- Fill and compact excavation with embankment material per Section 205 of the Standard Specifications.

5. Remove temporary shee	eting.
--------------------------	--------

of Existing Structures. Cross-hatched area indicates concrete



SECTION THRU EXISTING PIER 9

SHEET NO. 25 COUNTY TOTAL SHEETS SHEET NO. ROUTE NO. F.A.S. 181 109 46 SHEETS DOUGLAS 1671 FED. ROAD DIST. NO. 7

Contract #70258 ‡ 22VBR-1 and 144SBR-2

GENERAL NOTES

Fasteners shall be high strength bolts (AASHTO M 164, Type 3). Bolts $^{7}8''$ ϕ , open holes $^{15}16''$ ϕ , unless otherwise noted. Calculated weight of Structural Steel = 159,190 Lbs.

All structural steel shall be AASHTO M 270 Grade 50W.

Field welding of construction accessories will not be permitted to beams.

Anchor bolts shall be set before bolting diaphragms over supports.

The main load carrying member components subject to tensile stress shall conform to the Supplemental Requirements for Notch Toughness Zone 2. These components are the wide flange beams and all splice plate material.

Bearing seat surfaces shall be constructed or adjusted to the designated elevations within a tolerance of $^{l}_{8}$ inch. Adjustment shall be made either by grinding the surface or by shimming the bearing. Two $\frac{1}{8}$ " adjusting shims. of the dimensions of the bottom bearing plate, shall be provided for each bearing in addition to all other plates or shims.

The Contractor shall drive two (2) HP 12x53 test piles in a permanent location, one at each abutment, and two (2) HP 12x74 test piles in a permanent location. one at each pier, as directed by the Engineer before ordering the remainder of the piles.

AASHTO M 270 Grade 50W structural steel shall only be painted, at the ends of the beams, for a distance equal to the depth of embedment into the concrete cap plus 3 inches. Those areas shall be primed in the shop with an inorganic zinc rich primer per AASHTO M 300, Type 1. No field painting shall be required. All structural steel shall be cleaned as specified in the special provision for "Surface Preparation and Painting Requirements for Weathering

STRUCTURE NO. 021-0060 BILL OF MATERIAL

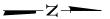
ITEM	UNIT	SUPER	SUB	TOTAL
Porous Granular Embankment (Special)	Cu. Yd.		110	110
Removal of Existing Structures No. 2	Each			0.5
Structure Excavation	Cu. Yd.		236	236
Driving Steel Piles	Foot		1340	1340
Concrete Structures	Cu. Yd.		231.6	231.6
Concrete Superstructure	Cu. Yd.	232.3		232.3
Bridge Deck Grooving	Sq. Yd.	640		640
Protective Coat	Sq. Yd.	843		843
Furnishing and Erecting Structural Steel	L. Sum	1		1
Stud Shear Connectors	Each	3330		3330
Reinforcement Bars, Epoxy Coated	Pound	49600	15750	65350
Slopewall 4''	Sq. Yd.		27	27
Bituminous Coated Aggregate Slopewall 6''	Sq. Yd.		489	489
Furnishing Steel Piles HP 12x53	Foot		645	645
Furnishing Steel Piles HP 12x74	Foot		695	695
Test Pile Steel HP 12x53	Each		2	2
Test Pile Steel HP 12x74	Each		2	2
Temporary Sheet Piling	Sq. Ft.		330	330
Name Plates	Each	1		1
Geocomposite Wall Drain	Sq. Yd.		62	62
Pipe Underdrains for Structures, 4''	Foot		132	132
Diamond Grinding (Bridge Section)	Sq. Yd.	821		821
Bar Splicers	Each	64		64

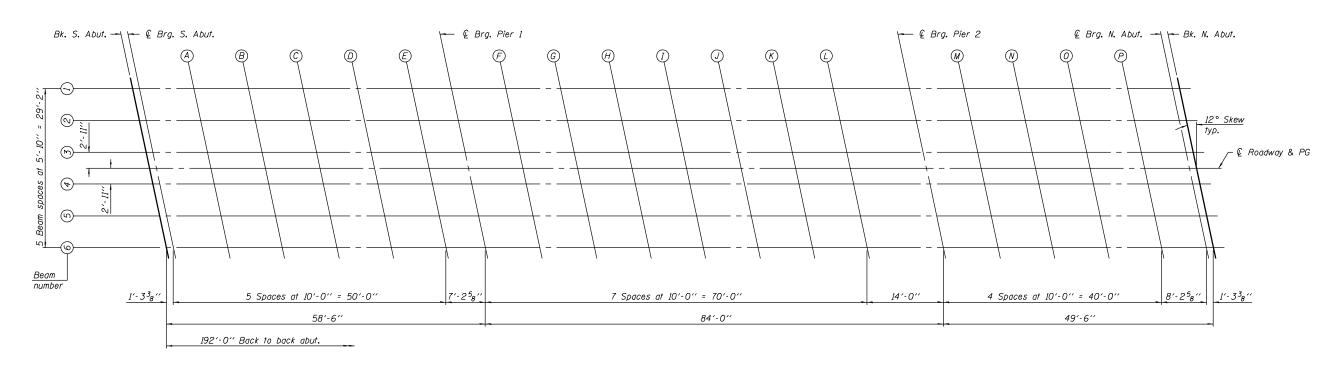
GENERAL DETAILS F.A.S. RT. 1671 - SEC. 22VBR-1 DOUGLAS COUNTY STATION 1154+99.02 STRUCTURE NO. 021-0060

TEMPORARY SHEET PILING

ROUTE NO.	SECTION	COUNTY		707AL SHEETS	SHEET NO.	sнеет No. 26
F.A.S. 1671	†	DOUGLAS		181	110	46 SHEETS
FED. ROAD DIST	NO. 7	ILL INDIS	FED. AID PROJECT-			

Contract #70258 ‡ 22VBR-1 and 144SBR-2





<u>PL AN</u>

DESIGNED Curt M. Evoy

CHECKED Rebecca L. Tharp

DRAWN Michael B. Mossman

CHECKED C.M.E. / R.L.T.



ROUTE NO.	SECTION	COUNTY		TOTAL SHEETS	SHEET NO.	SHEET NO. 27
F.A.S. 1671	‡	DOUGLAS		181	111	46 SHEETS
FED. ROAD DIST	A ROAD DIST. NO. 7 ILLINOIS FED. AID PROJECT-					

Contract #70258 ‡ 22VBR-1 and 144SBR-2

BEAM 1

<u>BEAW 1</u>								
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding				
Bk. S. Abut.	1154+03.40	-14.58	684.98	685.00				
Brg. S. Abut.	1154+04.68	-14.58	684.98	685.00				
A B C D E	1154+14.68 1154+24.68 1154+34.68 1154+44.68 1154+54.68	-14.58 -14.58 -14.58 -14.58 -14.58	684.97 684.94 684.91 684.86 684.81	685.00 684.98 684.94 684.89 684.83				
© Brg. Pier 1	1154+61.90	-14.58	684.77	684.79				
F G H J K L	1154+71.90 1154+81.90 1154+91.90 1155+01.90 1155+11.90 1155+21.90 1155+31.90	-14.58 -14.58 -14.58 -14.58 -14.58 -14.58	684.70 684.63 684.54 684.45 684.35 684.24 684.12	684.76 684.71 684.65 684.57 684.46 684.33 684.19				
© Brg. Pier 2	1155+45.90	-14.58	683.94	683.96				
М N О Р	1155+55.90 1155+65.90 1155+75.90 1155+85.90	-14.58 -14.58 -14.58 -14.58	683.80 683.65 683.49 683.33	683.81 683.67 683.51 683.35				
€ Brg. N. Abut.	1155+94.12	-14.58	683.18	683.20				
Bk. N. Abut.	1155+95.40	-14.58	683.16	683.18				
		l	l	ı				

<u>BEAM 2</u>

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abut.	1154+04.64	-8.75	685.09	685.11
₡ Brg. S. Abut.	1154+05.92	-8.75	685.09	685.11
A B C D E	1154+15.92 1154+25.92 1154+35.92 1154+45.92 1154+55.92	-8.75 -8.75 -8.75 -8.75 -8.75	685.07 685.04 685.01 684.96 684.91	685.10 685.08 685.04 684.99 684.93
© Brg. Pier 1	1154+63.14	-8.75	684.87	684.89
F G H J K L	1154+73.14 1154+83.14 1154+93.14 1155+03.14 1155+13.14 1155+23.14 1155+33.14	-8.75 -8.75 -8.75 -8.75 -8.75 -8.75 -8.75	684.80 684.72 684.64 684.54 684.44 684.33 684.21	684.85 684.81 684.74 684.67 684.55 684.42 684.28
© Brg. Pier 2	1155+47.14	-8.75	684.03	684.05
М N О Р	1155+57.14 1155+67.14 1155+77.14 1155+87.14	-8.75 -8.75 -8.75 -8.75	683.89 683.74 683.58 683.41	683.90 683.75 683.60 683.43
© Brg. N. Abut.	1155+95.36	-8.75	683.27	683.29
Bk. N. Abut.	1155+96.64	-8.75	683.24	683.26

<u>BEAM 3</u>

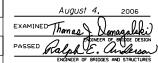
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abut.	1154+05.88	-2.92	685.18	685.20
© Brg. S. Abut.	1154+07.16	-2.92	685.18	685.20
A B C D E	1154+17.16 1154+27.16 1154+37.16 1154+47.16 1154+57.16	-2.92 -2.92 -2.92 -2.92 -2.92	685.16 685.13 685.09 685.05 684.99	685.19 685.17 685.13 685.07 685.02
⊈ Brg. Pier 1	1154+64.38	-2.92	684.95	684.97
F G H I J K L	1154+74.38 1154+84.38 1154+94.38 1155+04.38 1155+14.38 1155+24.38 1155+34.38	-2.92 -2.92 -2.92 -2.92 -2.92 -2.92 -2.92	684.88 684.80 684.72 684.62 684.52 684.41 684.29	684.93 684.89 684.82 684.75 684.63 684.50 684.35
© Brg. Pier 2	1155+48.38	-2.92	684.10	684.12
M N O P	1155+58.38 1155+68.38 1155+78.38 1155+88.38	-2.92 -2.92 -2.92 -2.92	683.96 683.81 683.65 683.48	683.97 683.83 683.67 683.50
© Brg. N. Abut.	1155+96.60	-2.92	683.34	683.36
Bk. N. Abut.	1155+97.88	-2.92	683.31	683.33

DESIGNED Curt M. Evoy

CHECKED Rebecca L. Tharp

DRAWN Michael B. Mossman

CHECKED C.M.E. / R.L.T.



ROUTE NO.	SECTION	COUNTY		TOTAL SHEETS	SHEET NO.	SHEET NO. 28	
F.A.S. 1671	‡	DOUGLAS		181	112	46 SHEETS	
FED. ROAD DIST. NO. 7 ILLINOIS FED. AID PROJECT-		DJECT-					

Contract #70258 ‡ 22VBR-1 and 144SBR-2

© ROADWAY & PG

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abut.	1154+06.50	0.00	685.22	685.24
© Brg. S. Abut.	1154+07.78	0.00	685.22	685.24
A B C D E	1154+17.78 1154+27.78 1154+37.78 1154+47.78 1154+57.78	0.00 0.00 0.00 0.00 0.00	685.20 685.17 685.14 685.09 685.04	685.23 685.21 685.17 685.12 685.06
© Brg. Pier 1	1154+65.00	0.00	684.99	685.01
F G H I J K L	1154+75.00 1154+85.00 1154+95.00 1155+05.00 1155+15.00 1155+25.00 1155+35.00	0.00 0.00 0.00 0.00 0.00 0.00	684.92 684.85 684.76 684.66 684.56 684.45 684.32	684.97 684.93 684.86 684.78 684.67 684.54 684.39
© Brg. Pier 2	1155+49.00	0.00	684.14	684.16
М N О Р	1155+59.00 1155+69.00 1155+79.00 1155+89.00	0.00 0.00 0.00 0.00	684.00 683.84 683.68 683.52	684.01 683.86 683.71 683.54
© Brg. N. Abut.	1155+97.22	0.00	683.37	683.39
Bk. N. Abut.	1155+98.50	0.00	683.35	683.37
		I	1	ı

BEAM 4

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abut.	1154+07.12	2.92	685.18	685.20
₡ Brg. S. Abut.	1154+08.40	2.92	685.17	685.19
A B C D E	1154+18.40 1154+28.40 1154+38.40 1154+48.40 1154+58.40	2.92 2.92 2.92 2.92 2.92	685.15 685.13 685.09 685.04 684.99	685.19 685.16 685.12 685.07 685.01
© Brg. Pier 1	1154+65.62	2.92	684.94	684.96
F G H I J K L	1154+75.62 1154+85.62 1154+95.62 1155+05.62 1155+15.62 1155+25.62 1155+35.62	2.92 2.92 2.92 2.92 2.92 2.92 2.92	684.87 684.79 684.71 684.61 684.51 684.39 684.27	684.92 684.88 684.81 684.73 684.62 684.49 684.34
© Brg. Pier 2	1155+49.62	2.92	684.08	684.10
M N O P	1155+59.62 1155+69.62 1155+79.62 1155+89.62	2.92 2.92 2.92 2.92	683.94 683.79 683.63 683.46	683.96 683.81 683.65 683.48
© Brg. N. Abut.	1155+97.84	2.92	683.31	683.33
Bk. N. Abut.	1155+99.12	2.92	683.29	683.31

<u>BEAM 5</u>

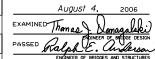
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abut.	1154+08.36	8.75	685.08	685.10
₽ Brg. S. Abut.	1154+09.64	8.75	685.08	685.10
A B C D E	1154+19.64 1154+29.64 1154+39.64 1154+49.64 1154+59.64	8.75 8.75 8.75 8.75 8.75	685.06 685.03 684.99 684.94 684.89	685.09 685.07 685.03 684.97 684.91
© Brg. Pier 1	1154+66.86	8.75	684.84	684.86
F G H I J K L	1154+76.86 1154+86.86 1154+96.86 1155+06.86 1155+16.86 1155+26.86 1155+36.86	8.75 8.75 8.75 8.75 8.75 8.75 8.75	684.77 684.69 684.60 684.51 684.40 684.29 684.16	684.82 684.77 684.71 684.63 684.51 684.38 684.23
₢ Brg. Pier 2	1155+50.86	8.75	683.98	684.00
M N O P	1155+60.86 1155+70.86 1155+80.86 1155+90.86	8.75 8.75 8.75 8.75	683.83 683.68 683.52 683.35	683.85 683.70 683.54 683.37
	1155+99.08	8.75	683.20	683.22
Bk. N. Abut.	1156+00.36	8.75	683.18	683.20
	•			•

DESIGNED Curt M. Evoy

CHECKED Rebecca L. Tharp

DRAWN Michael B. Mossman

CHECKED C.M.E. / R.L.T.



ROUTE NO.	SECTION	COUNTY		TOTAL SHEETS	SHEET NO.	SHEET NO. 29
F.A.S. 1671	†	DOUGLAS		181	113	46 SHEETS
FED. ROAD DIST	NO. 7	ILL INDIS	FED. AID PROJECT-			

Contract #70258 ‡ 22VBR-1 and 144SBR-2

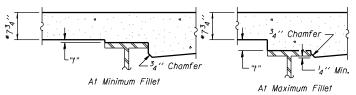
Theoretical Grade

© Brg. S. Abut.	<u>€</u> Brg. Pier I	<u>€ Brg.</u> Pier 2		© Brg. N. Abut.
, , , , , , , , , , , , , , , , , , , ,	14. '. 'A'.'	, p	,, 8, ,, c	
4 Spaces at ±14'-3 ⁵ g'' = 57'-2 ⁵ g''	4 Spaces at 21'-0" = 84'-0"			aces at = 48'-2 ⁵ 8''

DEAD LOAD DEFLECTION DIAGRAM

(Includes weight of concrete only.)

The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections and grinding as shown on sheets 27 through 29 of 46.

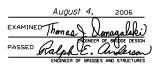


*Prior to Grinding

To determine "t": After all structural steel has been erected, elevations of the top flanges of the beams shall be taken at intervals shown on sheet 26 of 46. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflections and Grinding" shown on sheets 27 through 29 of 46, minus the 7^3_4 " deck thickness, equals the fillet heights "t" above top flanges of beams. The slab is to be ground after curing to achieve smoothness, but the slab is not to be ground to elevations below the "Theoretical Grade Elevations" shown on sheets 27 through 29 of 46. For grinding the deck, see Special Provisions.

FILLET HEIGHTS

DESIGNED Curt M. Evoy CHECKED Rebecca L. Tharp DRAWN Michael B. Mossman CHECKED C.M.E. / R.L.T.



Location	Station	Offset	Grade Elevations	Elevations Adjusted For Dead Load Deflection and Grinding
Bk. S. Abut.	1154+09.60	14.58	684.98	685.00
© Brg. S. Abut.	1154+10.88	14.58	684.97	684.99
A B C D E	1154+20.88 1154+30.88 1154+40.88 1154+50.88 1154+60.88	14.58 14.58 14.58 14.58 14.58	684.95 684.92 684.88 684.83 684.78	684.98 684.96 684.92 684.86 684.80
© Brg. Pier 1	1154+68.10	14.58	684.73	684.75
F G H I J K L	1154+78.10 1154+88.10 1154+98.10 1155+08.10 1155+18.10 1155+28.10 1155+38.10	14.58 14.58 14.58 14.58 14.58 14.58	684.66 684.58 684.49 684.39 684.28 684.17 684.04	684.71 684.66 684.59 684.51 684.39 684.26 684.11
© Brg. Pier 2	1155+52.10	14.58	683.85	683.87
M N O P	1155+62.10 1155+72.10 1155+82.10 1155+92.10	14.58 14.58 14.58 14.58	683.71 683.55 683.39 683.22	683.72 683.57 683.41 683.24
© Brg. N. Abut.	1156+00.32	14.58	683.07	683.09
Bk. N. Abut.	1156+01.60	14.58	683.05	683.07

<u>BEAM 6</u>

ROUTE NO.	SECTION	cox	JNTY	TOTAL SHEETS	SHEET NO.	SHEET NO. <i>30</i>
F.A.S. 1671	‡	DOUGLAS		181	114	46 SHEETS
FED. ROAD DIST. NO. 7		ILL INDIS	FED. AID PROJECT-			

Contract #70258 ‡ 22VBR-1 and 144SBR-2

WEST CURB LINE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
End S. Appr. Pav't.	1153+73.10	-16.00	684.95	684.97
A B	1153+83.10 1153+93.10	-16.00 -16.00	684.96 684.96	684.98 684.98
Bk. S. Abut.	1154+03.10	-16.00	684.96	684.98

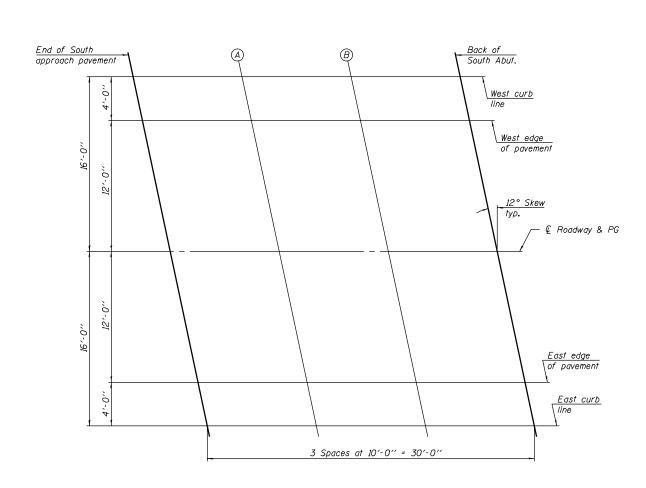
WEST EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding		
End S. Appr. Pav't.	1153+73.95	-12.00	685.03	685.05		
A B	1153+83.95 1153+93.95	-12.00 -12.00	685.04 685.04	685.06 685.06		
Bk. S. Abut.	1154+03.95	-12.00	685.04	685.06		

@ ROADWAY & PG

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
End S. Appr. Pav't.	1153+76.50	0.00	685.22	685.24
A B	1153+86.50 1153+96.50	0.00 0.00	685.23 685.23	685.25 685.25
Bk. S. Abut.	1154+06.50	0.00	685.22	685.24

___Z___



EAST EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
End S. Appr. Pav't.	1153+79.05	12.00	685.04	685.06
A B	1153+89.05 1153+99.05	12.00 12.00	685.05 685.04	685.07 685.06
Bk. S. Abut.	1154+09.05	12.00	685.03	685.05

EAST CURB LINE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
End S. Appr. Pav't.	1153+79.90	16.00	684.96	684.98
A B	1153+89.90 1153+99.90	16.00 16.00	684.96 684.96	684.98 684.98
Bk. S. Abut.	1154+09.90	16.00	684.95	684.97

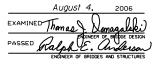
<u>PLAN</u>

DESIGNED Curt M. Evoy

CHECKED Rebecca L. Tharp

DRAWN Michael B. Mossman

CHECKED C.M.E. / R.L.T.



ROUTE NO.	SECTION	cox	JNTY	1014L SHEETS	9-EET NO.	SHEET NO. 31
F.A.S. 1671	†	DOUG	LAS	181	<i>11</i> 5	46 SHEETS
FED. ROAD DIST. NO. 7		JLL INOIS	PED. AID PROJECT-			

Contract #70258 ‡ 22VBR-1 and 144SBR-2

WEST CURB LINE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
Bk. N. Abut.	1155+95.10	-16.00	683.14	683.16
A B	1156+05.10 1156+15.10	-16.00 -16.00	682.95 682.76	682.97 682.78
End N. Appr. Pav't.	1156+25.10	-16.00	682.56	682.58

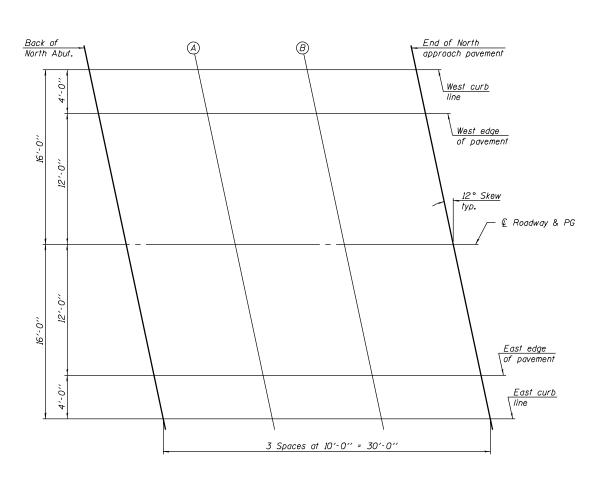
WEST EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding		
Bk. N. Abut.	1155+95.95	-12.00	683.21	683.23		
A B	1156+05.95 1156+15.95	-12.00 -12.00	683.02 682.83	683.04 682.85		
End N. Appr. Pav't.	1156+25.95	-12.00	682.63	682.65		

@ ROADWAY & PG

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
Bk. N. Abut.	1155+98.50	0.00	683.35	683.37
A B	1156+08.50 1156+18.50	0.00 0.00	683.16 682.97	683.18 682.99
End N. Appr. Pav't.	1156+28.50	0.00	682.76	682.78

___Z__



EAST EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
Bk. N. Abut.	1156+01.05	12.00	683.11	683.13
A B	1156+11.05 1156+21.05	12.00 12.00	682.92 682.73	682.94 682.75
End N. Appr. Pav't.	1156+31.05	12.00	682.52	682.54

EAST CURB LINE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Grinding
Bk. N. Abut.	1156+01.90	16.00	683.01	683.03
A B	1156+11.90 1156+21.90	16.00 16.00	682.82 682.63	682.84 682.65
End N. Appr. Pav't.	1156+31.90	16.00	682.42	682.44

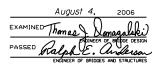
<u>PL A N</u>

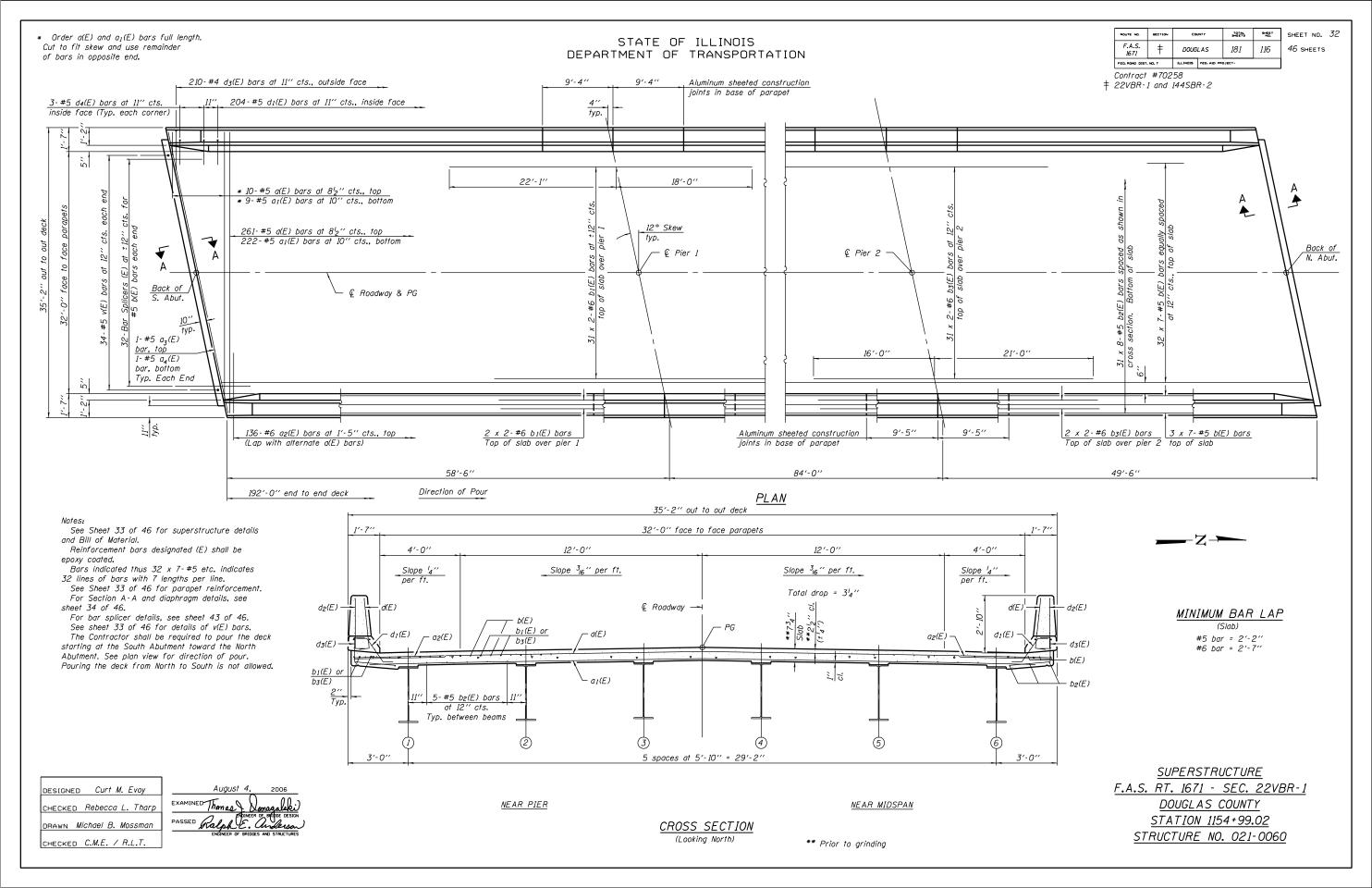
DESIGNED Curt M. Evoy

CHECKED Rebecca L. Tharp

DRAWN Michael B. Mossman

CHECKED C.M.E. / R.L.T.





SHEET NO. 33 1014L 9-EET ROUTE NO. COUNTY STATE OF ILLINOIS F.A.S. 181 117 46 SHEETS DOUGLAS DEPARTMENT OF TRANSPORTATION FED. ROAD DIST. NO. 7 Contract #70258 ‡ 22VBR-1 and 144SBR-2 192'-0" End-to-end parapet 3 Spaces at $\pm 16' - 4^{5}_{8}'' = 49' - 2''$ 9'-4" 9'-4" 4 Spaces at 16'-334'' = 65'-3" 9′-5′′ 9'-5" 2 Spaces at 20'-0'2" = 40'-1" Parapet joint spacing 210-#5 d(E) bars at 11" cts. inside face 210-#4 d2(E) bars at 11" cts. outside face 3-#4 <u>e3(E) bars</u> 3-#4 <u>eı(E) bars</u> each face each face 3-#4 e2(E) bars 3-#4 e(E) bars 3-#4 e4(E) bars Bend in 2'-0" 1-#8 e6(E) bar 1- #8 es(E) bar field, typ. each face each face each face typ. each face each face 5, typ. 1 x 2-#5 e12(E) bar 1 x 2-#5 e10(E) bar \1 x 2-#8 e7(E) bar each face (1- #5 e13 (E) bar 1 x 2-#5 e # (E) bar each face each face each face each face each face 2'-6' typ. 1 x 2-#8 es(E) bar each face Aluminum sheeted construction 1 x 2-#8 e9(E) bar each face joints in base of parapet 58'-6" 84'-0'' 49'-6" © Pier 1 -- € Pier 2 1'-2" INSIDE ELEVATION OF PARAPET (West parapet shown, East parapet similar) **SUPERSTRUCTURE** BILL OF MATERIAL Bar No. Size Length Shape Bar No. Size Length Shape Non-staining gray one component a(E) 271 a₁(E) 231 a₂(E) 272 #5 34'-7' #5 32'-10 es(E) #8 non-sag elastomeric gun grade e(E) thru e4(E) polyurethane sealant meeting the * Prior to grinding #6 4'-6'' #5 25′-3″ requirements of ASTM C-920, e || (E) | 8 | #5 | 9'-0'' e || 2 (E) | 8 | #5 | 33'-4'' #5 35'-4'' -d(E)Type S. Grade NS. Class 25. $d_2(E)$ #5 Use T. e13(E) 8 #5 9'-1" 3'-98 ³⊿" Notch #5 20′-9′′ 03(E) 266 #5 29'-3 e 14 (E) 8 ⁵8" ♥ Backer Rod 70 #6 a₂(E) b₂(E) 248 #5 25′-11′′ es(E) thru es(E) #6 70 #6 19'-10'' m₁(E) 6 #6 35'-7" 12" Preformed Selfm₂(E) 16 #6 m₃(E) 8 #6 e10(E) thru e14(E) #6 8'-9'' Expanding Cork Joint Filler according to Article 1051.07 d3(E) of the Std. Spec. Cost d₁(E) 408 #5 m4(E) 10 #6 5'-8" d₂(E) 420 #4 3'-0" #6 included with Concrete 2'-9" m5(E) 4 /a1(E) or ς. #4 Superstructure. 420 04(E) d₄(E) 12 #5 2'-2" 72 #5 7'-0" Const. Jt. Const. Jts. at Piers and locations s1(E) 62 #4 9'-4" ப (Optional) as shown. '8'' Aluminum sheet #4 ASTM B 209 allov 3003-H14. e1(E) 24 #4 9'-0" 68 #5 3'-4'' 34" Drip Notch Cost included with Concrete e₂(E) 48 #4 15'-11' Const. Jt. Superstructure. 24 #4 Reinforcement Bars, (Mandatory) 9'-1' Pound 49600 24 #4 Epoxy Coated e5(E) 8 #8 26'-2" Concrete 232.3 PARAPET JOINT DETAILS Cu. Yds. 3'-0" 8 #8 9'-0'' Superstructure #8 34'-2" er(E) 8 Reinforcement bars designated (E) shall be SECTION THRU PARAPET Bars indicated thus 1 x 2-#5 etc. indicates 1 line of bars with 2 lengths per line. 1'-8'' 4" MINIMUM BAR LAP (Parapet) #5 bar = 1'-8" #8 bar = 3'-5" BAR V(E) 6" 2'-2" SUPERSTRUCTURE DETAILS DESIGNED Curt M. Evoy August 4. BAR d1(E) BAR d3(E) BAR d4(E) BAR s(E) BARS d(E) & $d_2(E)$ $BAR s_1(E)$

CHECKED Rebecca L. Tharp

DRAWN Michael B. Mossman

CHECKED C.M.E. / R.L.T.

SUPERSTRUCTURE DETAILS

F.A.S. RT. 1671 - SEC. 22VBR-1

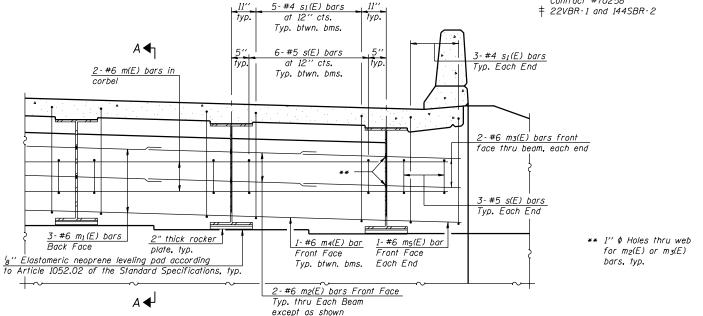
DOUGLAS COUNTY

STATION 1154+99.02

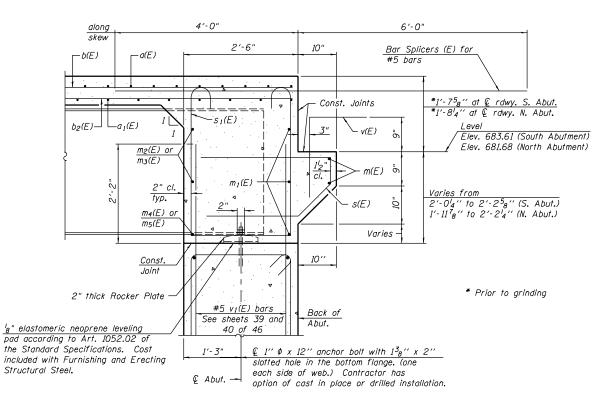
STRUCTURE NO. 021-0060

9-EET NO. SHEET NO. 34 ROUTE NO. COUNTY TOTAL SHEETS F.A.S. 181 118 46 SHEETS DOUGLAS FED. ROAD DIST. NO. 7

Contract #70258



DIAPHRAGM ELEVATION AT ABUTMENT



SECTION A-A

Dimensions at right angles to abutment, except as shown.

DESIGNED Curt M. Evoy CHECKED Rebecca L. Tharp DRAWN Michael B. Mossman CHECKED C.M.E. / R.L.T.

August 4.

Notes:

to the beams.

or $m_3(E)$ bars.

Reinforcement bars in diaphragm are billed with

Concrete in diaphragm is included with Concrete

For anchor bolt details see sheet 38 of 46.

For bar splicer details, see sheet 43 of 46.

See sheet 36 of 46 for holes thru web for m₂(E)

MIN. BAR LAP #6 bar = 2'-9"

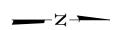
For details of bars s(E) & $s_1(E)$ see sheet 33 of 46.

The s(E) and $s_1(E)$ bars shall be placed parallel to the beams. Spacing for these bars shall be at right angles

superstructure on sheet 33 of 46.

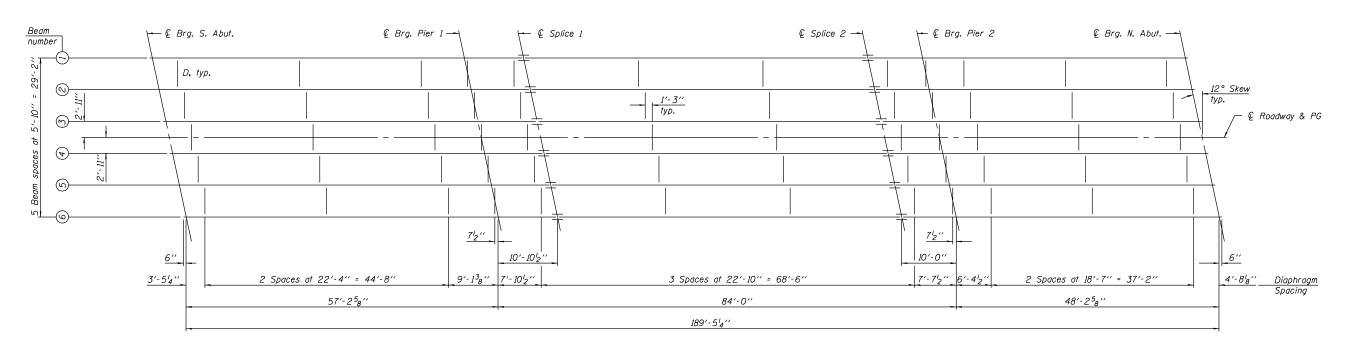
Superstructure on sheet 33 of 46.

DIAPHRAGM DETAILS F.A.S. RT. 1671 - SEC. 22VBR-1 DOUGLAS COUNTY STATION 1154+99.02 STRUCTURE NO. 021-0060

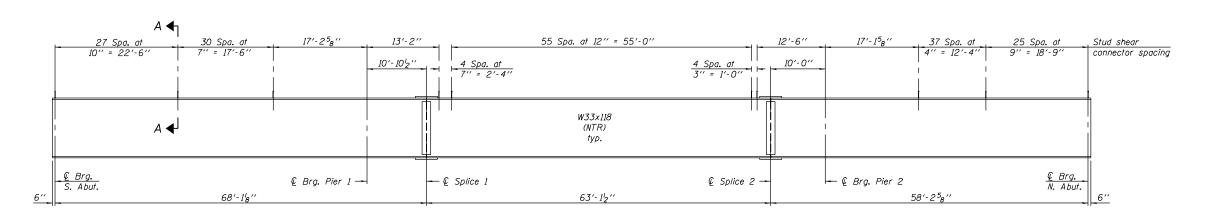


ROUTE NO.	SECTION	cox	UNTY	1014L SHEETS	SHEET NO.	SHEET NO. 35
F.A.S. 1671	‡	DOUGLAS		181	119	46 SHEETS
FED. ROAD DIST.	NO. 7	ILL INDIS	PED. AID PRI	DJECT-		

Contract #70258 ‡ 22VBR-1 and 144SBR-2



<u>PLAN</u>



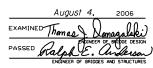
ELEVATION

DESIGNED Curt M. Evoy

CHECKED Rebecca L. Tharp

DRAWN Michael B. Mossman

CHECKED C.M.E. / R.L.T.



NOTES:

NTR denotes members to which Notch Toughness
Requirements are applicable.

For Section A-A, see sheet 36 of 46.

STRUCTURAL STEEL

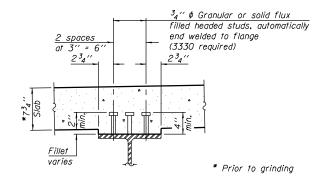
F.A.S. RT. 1671 - SEC. 22VBR-1

DOUGLAS COUNTY

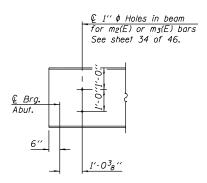
STATION 1154+99.02

STRUCTURE NO. 021-0060

	INTERIOR BEAM MOMENT TABLE							
	0.4 Sp. 1	Pier 1	0.5 Sp. 2	Pier 2	0.6 Sp. 3			
Is (in4)	5900	5900	5900	5900	5900			
Ic (n) (in4)	15505		15505	_	15505			
Ic (3n) (in4)	11349		11349	_	11349			
Ss (in 3)	359	359	359	359	359			
Sc(n) (in ³)	526		526		526			
$Sc(3n)$ (in^3)	474		474	_	474			
Z (in 3)	_	415	_	415	_			
Q (k/ft.)	0.713	1 . 155	0.713	1.155	0.713			
M2 ('k)	125	572	259	520	58			
s@ (k/ft.)	0.442		0.442		0.442			
Ms€ ('k)	100		214	_	56			
M4 ('k)	342	238	470	226	267			
M (Imp) ($'k$)	94	62	113	60	78			
⁵ ₃ [M½+M(Imp)] ('k)	727	500	972	477	575			
Ma ('k)	1238	1394	1879	1296	896			
Mu ('k)	2643	1729	2643	1729	2643			
fs@ non-comp (k.s.i.)	4.2	19.1	8.7	17.4	1.9			
fs@(comp) (k.s.i.)	2.5		5.4	_	1.4			
$fs_{3}(4+Imp)$ (k.s.i.)	16.6	16.7	22.2	15.9	13.1			
fs (Overload) (k.s.i.)	23.3	<i>35.8</i>	36.2	33.3	16.4			
VR (k)	43.9	_	40.1	_	43.7			



SECTION A-A



END OF BEAM ELEVATION

DESIGNED Curt M. Evoy
CHECKED Rebecca L. Tharp
DRAWN Michael B. Mossman
CHECKED C.M.E. / R.L.T.



STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

INTERIOR BEAM REACTION TABLE							
		S. Abut.	Pier 1	Pier 2	N. Abut.		
R₽	(k)	23.0	92.1	86.5	17.1		
R4	(k)	31.0	41.0	39.7	29.7		
Imp.	(k)	8. 5	<i>10.</i> 5	10.4	8.6		
R (Total)	(k)	62.5	143.6	136.6	55.4		

Is and Ss are the moment of inertia and section modulus of the steel section used in computing fs (Total & Overload).

 $Ic_{(n)}$ and $Sc_{(n)}$ are the moment of inertia and section modulus of the composite section used in computing stresses due to Live Load.

 $Ic_{(3n)}$ and $Sc_{(3n)}$ are the moment of inertia and section modulus of the composite section used in computing stresses due to superimposed dead loads. (see AASHTO 10.38) VR is the maximum Live Load + Impact shear

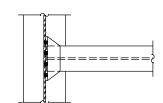
range within the composite portion of the span.

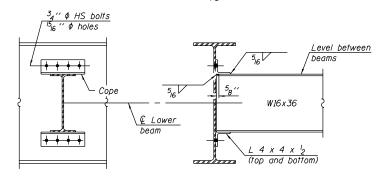
Z is the plastic section modulus used to determine

the fully plastic moments in the non-composite areas. Ma (Applied Moment)= $1.3 \text{EM Q} + \text{MsQ} + \frac{5}{3} (\text{ML} + \text{M(Imp)})$].

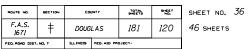
The Plastic Moment capacity (Mu) is computed according to AASHTO 10.48.1 and 10.50.1.1.

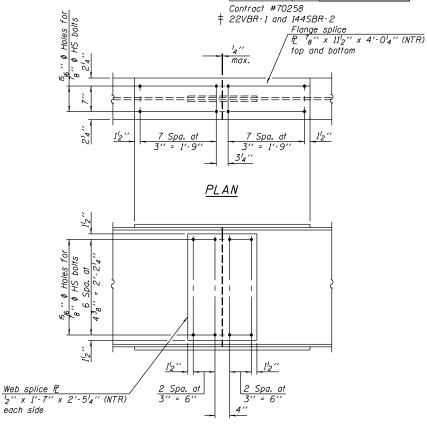
fs (Overload) is the sum of the stresses due to $MQ + MsQ + 5_3(MQ + M(Imp))$.





DIAPHRAGM D (60 Required)





ELEVATION

SPLICE DETAIL

Typical for splice 1 and splice 2

(12 Required)

*TOP OF BEAM ELEVATIONS

Beam	1	2	3	4	5	6
	684.31	684.42	684.51	684.50	684.41	684.30
₢ Brg. Pier 1	684.02	684.11	684.20	684.19	684.09	683.97
€ Splice 1	683.96	684.05	684.14	684.13	684.03	683.91
€ Splice 2	683.33	683.42	683.50	683.48	683.37	<i>683.25</i>
	683.19	<i>683.28</i>	<i>683.3</i> 5	683.34	<i>683.23</i>	683.11
	682.51	682.60	682.67	682.64	682.53	682.40

* For fabrication only.

Notes:

Two hardened washers shall be required over all oversize holes for diaphragms.

NTR denotes members to which Notch Toughness Requirements are applicable.

STRUCTURAL STEEL DETAILS

F.A.S. RT. 1671 - SEC. 22VBR-1

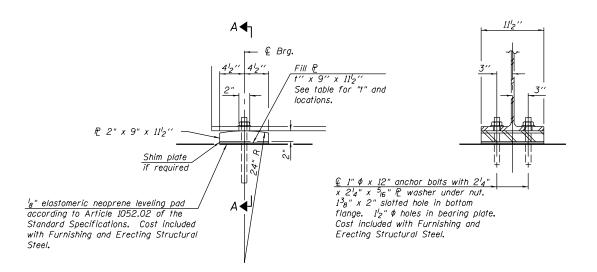
DOUGLAS COUNTY

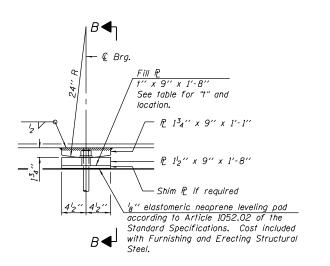
STATION 1154+99.02

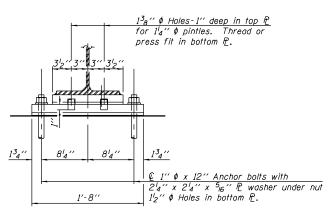
STRUCTURE NO. 021-0060

ROUTE NO.	SECTION	COUNTY		707AL 34EETS	9-667 NO.	SHEET NO. 37
F.A.S. 1671	‡	DOUGLAS		181	121	46 SHEETS
FED. ROAD DIST	NO. 7	ILL INDIS	FED. AID FRI	DJECT-		

Contract #70258 ‡ 22VBR-1 and 144SBR-2







ELEVATION AT ABUTMENT

SECTION A-A

ELEVATION AT PIER

SECTION B-B

FIXED BEARING

Notes:

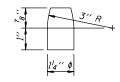
Contractor has the option of cast in place or drilled installation of anchor bolts.

See sheet 38 of 46 for Anchor Bolt installation.

FIXED BEARING (12 Required)

FILL P LOCATION AND THICKNESS

Location	Beam	Size (t)
Pier 2	3	4"
North Abutment	3	34"
North Abutment	4	2"



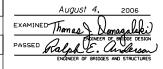
PINTLE

DESIGNED Curt M. Evoy

CHECKED Rebecca L. Tharp

DRAWN Michael B. Mossman

CHECKED C.M.E. / R.L.T.



BEARING DETAILS

F.A.S. RT. 1671 - SEC. 22VBR-1

DOUGLAS COUNTY

STATION 1154+99.02

STRUCTURE NO. 021-0060

item which is the property of the Illinois Department of Transportation. Use, reproduction or disclosure without express written permission is prohibited and protected under Federal copyright laws. The production and 'd'' 🖣 Holes with zerk the fabrication of this bolt for use on highway projects for epoxy grout in the State of Illinois shall be permitted and there shall be no incurred charges or fees to the manufacturer or the fabricator for producing or fabricating this bolt. ′′D′′ Ø 1316 " 134" 1³8′′ 1/16'' Anchor Bolt (See Bearing Details 12" 1⁵8′′ 1⁵16′′ 218'' \equiv for number, size and length.) 垂 2" 28" 1¹³16 '' 278" =258" 256" 338" 垂 ▆▋ ___ Top of base plate == End of End of groove coil lock $^{5}_{32}$ " wide x $^{3}_{32}$ " deep groove in anchor bolt with 18" \$ of coil PLAN-COIL WIRE

'R" Notch

ILLINOIS COIL-LOCK ANCHOR BOLT

The Illinois Coil-Lock Anchor Bolt is a proprietary

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY		TOTAL SHEETS	9-667 NO.	SHEET NO. 3	38
F.A.S. 1671	†	DOUGLAS		181	122	46 SHEETS	
FED. ROAD DIST	. NO. 7	ILL INDIS	FEO. AID PROJECT-				

Contract #70258 ‡ 22VBR-1 and 144SBR-2

Holes in the masonry for anchor bolts shall be drilled through the base

plates to the diameter and depth shown or according to the manufacturer's

Prior to setting the bolts, the holes shall be dry and all dust and loose

The anchor bolts, furnished and installed and including the epoxy grout or

capsules shall not be paid for separately but shall be included in the unit bid

recommendation after beams or girders have been erected and adjusted.

particles shall be removed by the use of compressed air or vacuuming.

price for Furnishing and Erecting Structural Steel.

GENERAL NOTES

MATERIALS FOR ILLINOIS COIL-LOCK ANCHOR BOLT

The anchor bolt shall be fabricated from cold drawn or hot finished seamless carbon steel mechanical tubing conforming to ASTM A 519, Grade 1026, CW and supplied with hexagonal nuts and cut washers.

The coil wire shall be made of any suitable soft steel wire.

The finished anchor bolt shall be cleaned of rust and other foreign materials and wrapped or packaged to prevent contamination until they are installed. The epoxy grout shall be a two-component, epoxy resin bonding system conforming to ASTM C 881, Type 1, Grade 1 and of a Class suitable for the temperature at installation.

INSTALLATION PROCEDURE for the ILLINOIS COIL-LOCK ANCHOR BOLT

- 1. With the coil wire in place, the bolt shall be inserted into the hole and turned clockwise to a snug fit in the hole. Nut and washer shall be placed on the bolt. The nut shall be tensioned until the steel base plates are held securely to the concrete bearing seat.
- 2. Epoxy grout shall be pumped through the zerk fitting with a pressure gun. Pumping shall continue until the epoxy overflows the hole around the bolt shank. After pumping is discontinued, excess epoxy shall be immediately wiped off.

ALTERNATE ANCHOR BOLTS

The Contractor may use, at his option, the capsule or the adhesive cartridge type anchor rods that have been previously tested and given a prior approval by the Department. The Contractor shall install these anchor rods in pre-drilled holes according to the manufacturer's recommendations and procedures.

The capsule or the adhesive cartridge type anchor rods shall be a two part system composed of:

- 1. A threaded rod stud with nut and washer of the type specified.
- 2. A sealed glass capsule or a sealed glass adhesive cartridge containing premeasured amounts of the adhesive chemical.

Location	Туре
Abutments	A325
Piers	A325

ASTM F 1554 Grade 105, ASTM A 449 and AASHTO M 314 Grade 105 anchor bolts may be substituted for the anchor bolts shown above.

ANCHOR BOLT DETAILS

FOR BEARINGS

F.A.S. RT. 1671 - SEC. 22VBR-1

DOUGLAS COUNTY

STATION 1154+99.02

STRUCTURE NO. 021-0060

DESIGNED Curt M. Evoy
CHECKED Rebecca L. Tharp
DRAWN Michael B. Mossman

CHECKED C.M.E. / R.L.T.

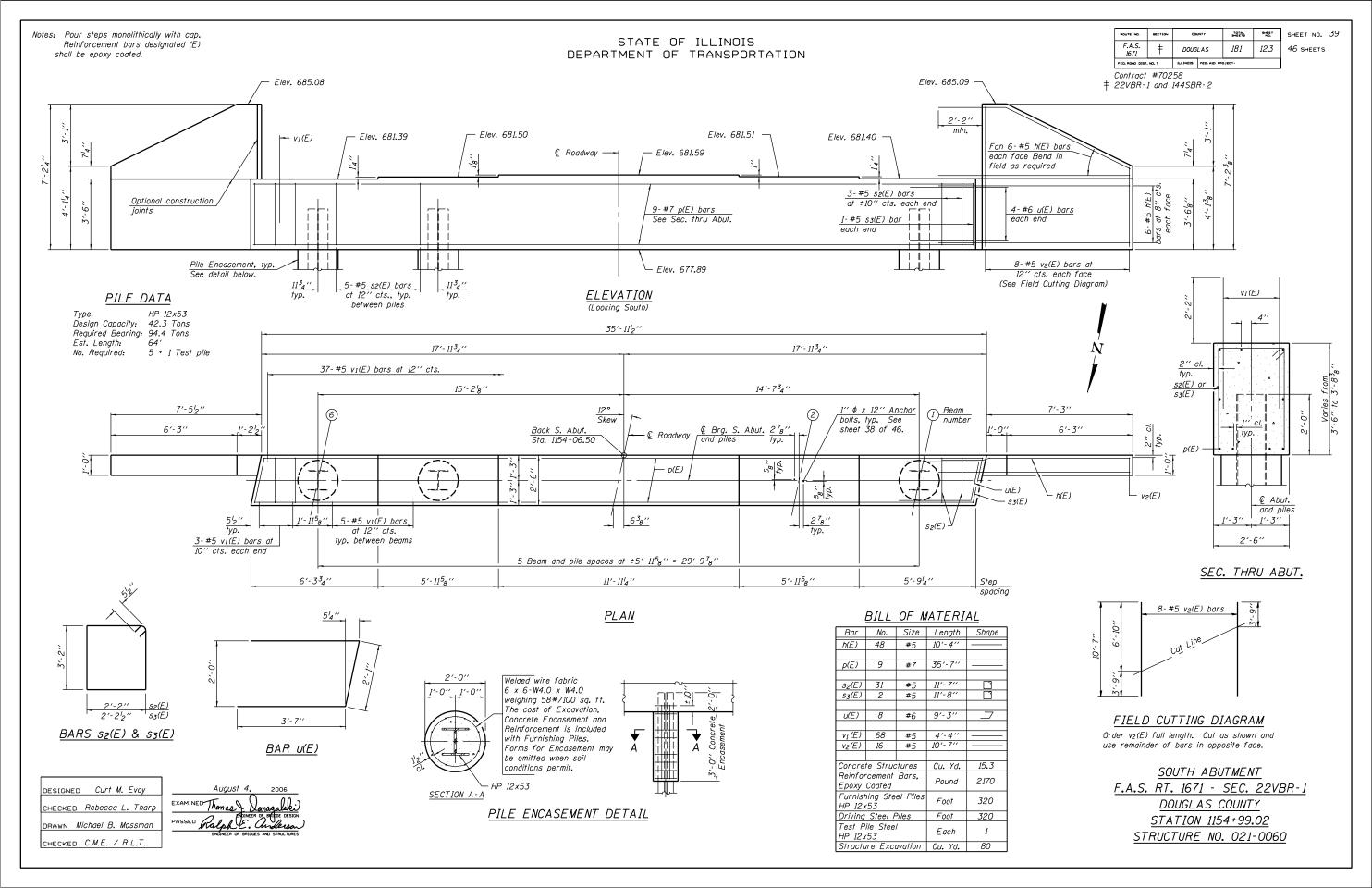
August 4. 2006

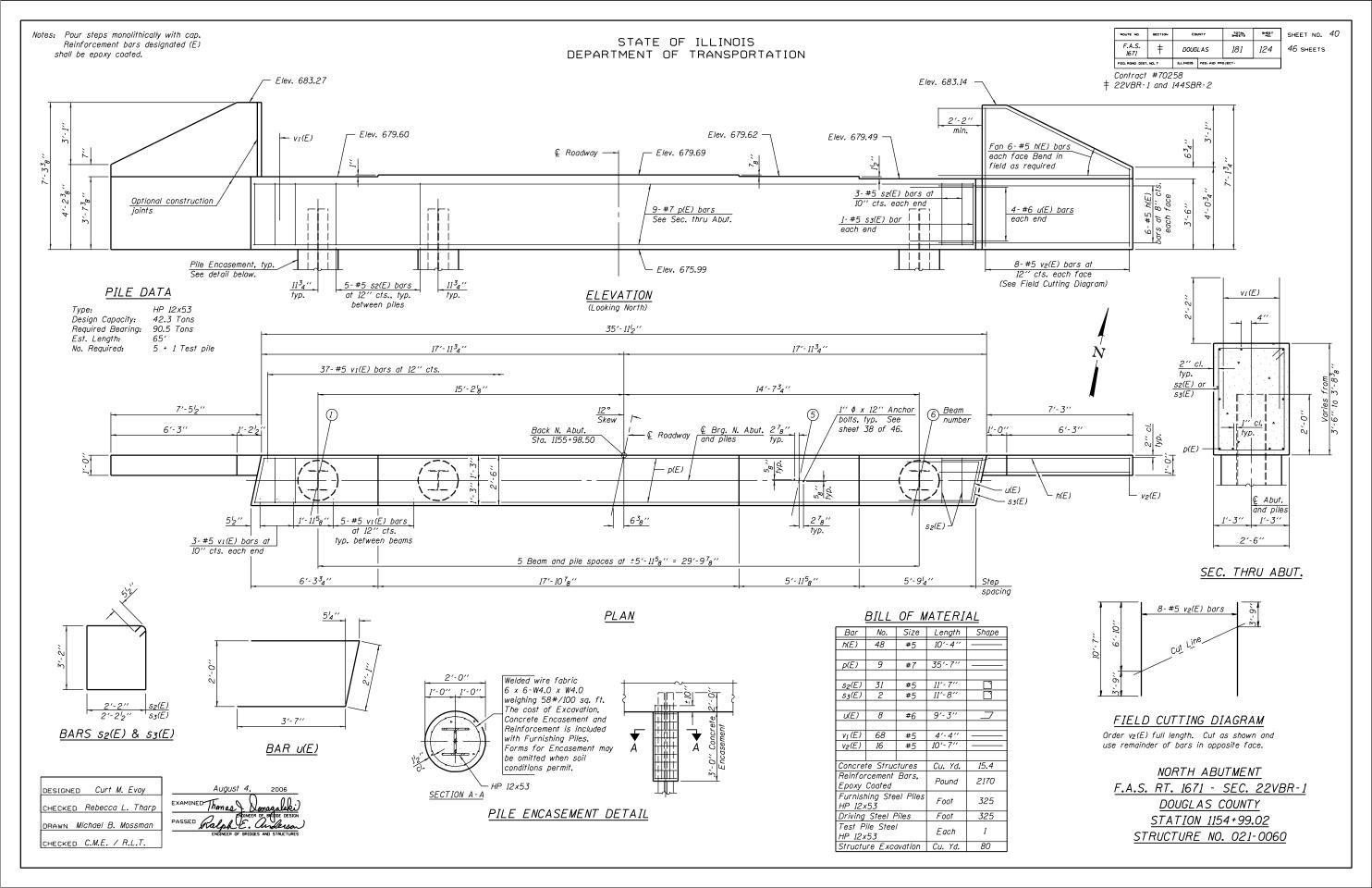
EXAMINED Thomas Some of office coston

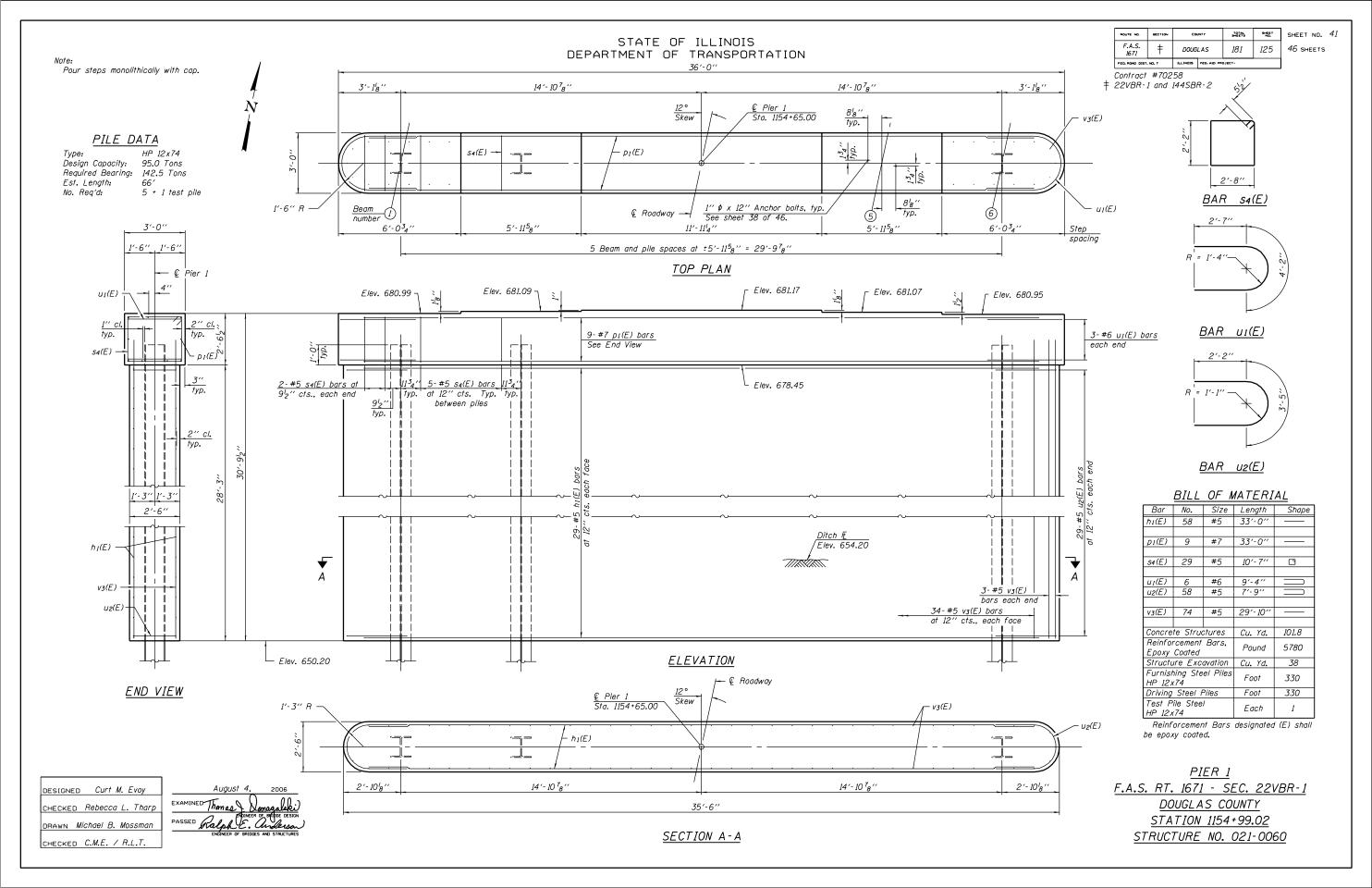
PASSED Ralph C. Underson

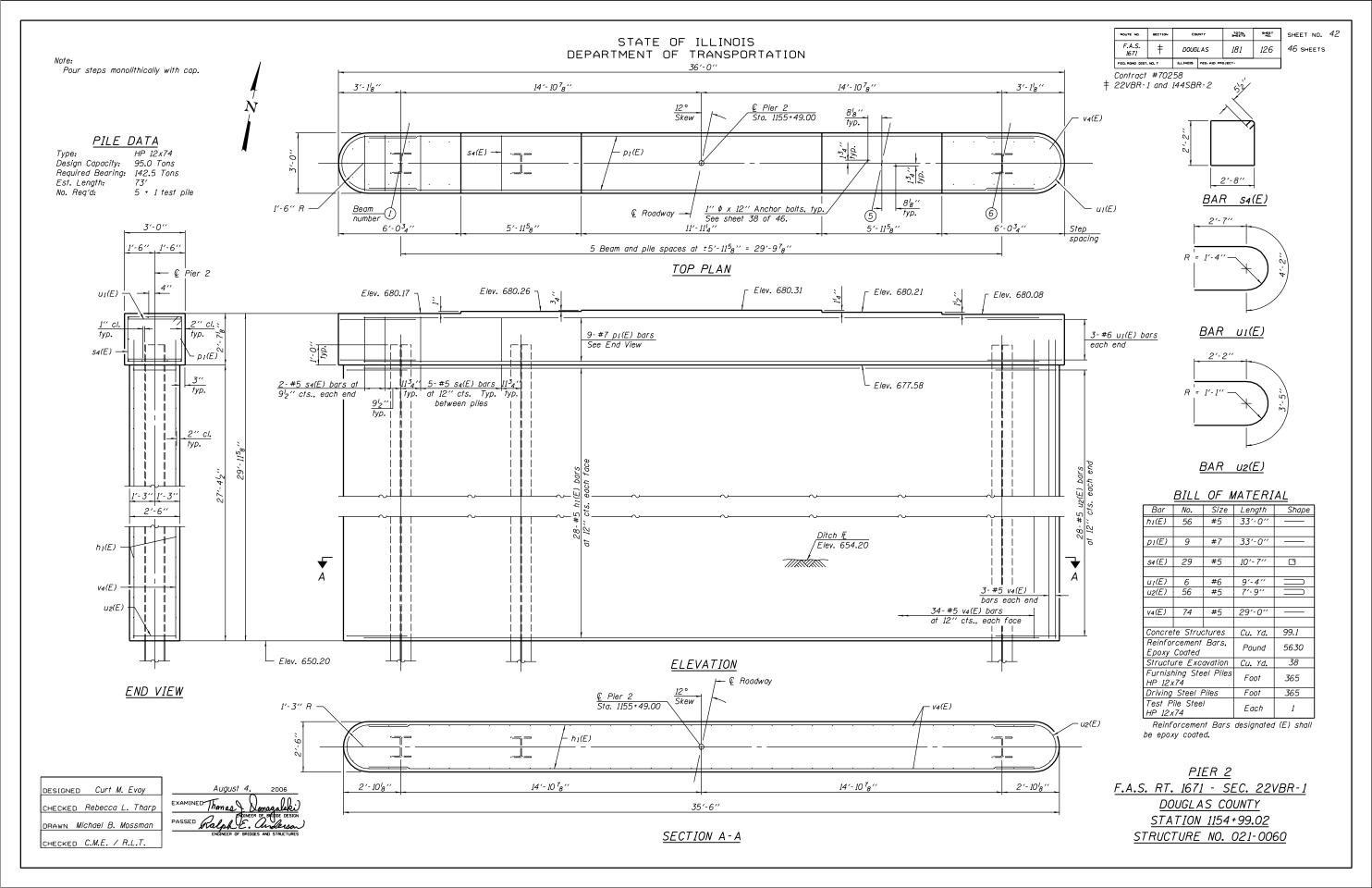
ENGINEER OF OFFICES AND STRUCTURES

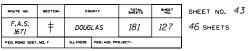
ABB-1 10-22-04











Contract #70258 ‡ 22VBR-1 and 144SBR-2

<u>NOTES</u>

Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars.

Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length. All reinforcement bars shall be lapped and tied to the splicer rods or dowel bars.

Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars.

Other systems of similar design may be submitted to the Engineer for approval. Approval shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

- Minimum Capacity (Tension in kips) = 1.25 x fy x A_t
- Minimum *Pull-out Strength = 1.25 x fs_{allow} x A_1 (Tension in kips)

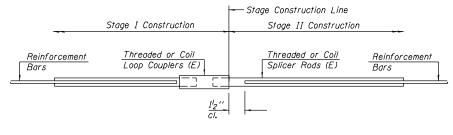
Where fy = Yield strength of lapped reinforcement bars in ksi.

 fs_{allow} = Allowable tensile stress in lapped reinforcement bars in ksi (Service Load)

 A_t = Tensile stress area of lapped reinforcement bars. * = 28 day concrete

ER ASSEMBLIES Strength Requirement	s
	s
	-
14.7 5.9	
23.0 9.2	
33.1 13.3	
45.1 18.0	
58.9 23.6	
75.0 30.0	
95.0 38.0	
117.4 46.8	
th	kips - tension kips - te 14.7 5.9 23.0 9.2 33.1 13.3 45.1 18.0 58.9 23.6 75.0 30.0 95.0 38.0

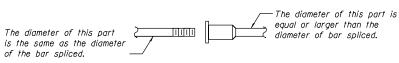
Bar splicer assemblies shall be according to Section 508 of the Standard Specifications, except as noted. The furnishing and installation of bar splicer assemblies will be measured and paid for at the contract unit price each for "BAR SPLICERS."



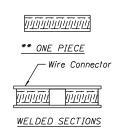
STANDARD

	Bar Size	No. Assemblies Required	Location
ŀ			
Į			

BAR SPLICER ASSEMBLY DETAILS F.A.S. RT. 1671 - SEC. 22VBR-1 DOUGLAS COUNTY STATION 1154+99.02 STRUCTURE NO. 021-0060

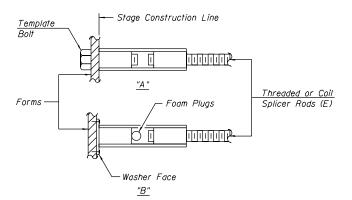


ROLLED THREAD DOWEL BAR



BAR SPLICER ASSEMBLY ALTERNATIVES

** Heavy Hex Nuts conforming to ASTM A 563, Grade C, D or DH may be used.

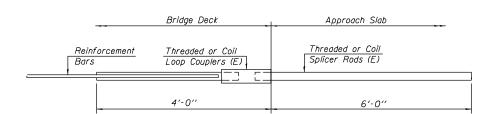


INSTALLATION AND SETTING METHODS

"A": Set bar splicer assembly by means of a template bolt. "B": Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E): Indicates epoxy coating.

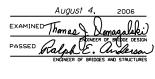
6'-0"

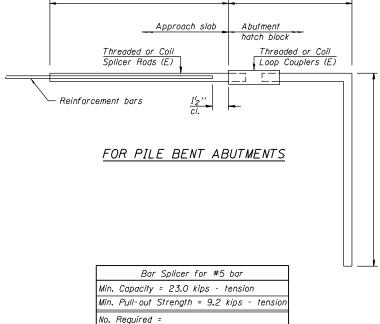


FOR INTEGRAL OR SEMI-INTEGRAL ABUTMENTS

Bar Splicer for #5 bar Min. Capacity = 23.0 kips - tension Min. Pull-out Strength = 9.2 kips - tension No. Required = 64

DESIGNED Curt M. Evoy CHECKED Rebecca L. Tharp DRAWN Michael B. Mossman CHECKED C.M.E. / R.L.T.





10-22-04

BSD-1

ROUTE NO.	SECTION	COUNTY		707AL SHEETS	S-EET NO.	SHEET NO. 44
F.A.S. 1671	‡	DOUGLAS		181	128	46 SHEETS
ED. ROAD DIST.	NO. 7	ILL INDIS	FED. AID FRI	DJECT-		

Contract #70258 ‡ 22VBR-1 and 144SBR-2

Illinois Depa	ırtm atıc	en n	τ		S	OIL BORING LOG			r-aye	1	•
Division of Highways [DOT - District 5	auo	"			•	OIL DOMING LOG			Date	10	0
ROUTEFAS_1671 (US_45)	DES	CRIE	TION	Re	oute 4	5 over the U.P. & C.S.X.R.R. Tracks	LO	GGE	D BY	c	;;
SECTION 22VBR-1		_ L	OCAT	ION _	NE, SE	C. 33, TWP. 16N, RNG. 8E, 3rd PM					
COUNTY <u>Douglas</u> DRIL	LING	ME	THOD	_	Holle	ow Stem Auger HAMMER TYP	E _		Auto	matic	:
STRUCT, NO. 021-0013(Exist.)	_ [D E	B	n	М	Surface Water Elev ft		D E	B L	U	Ī
Station 1153 + 86.5	-	Р	0	s	1	Stream Bed Elev ft		Р	0	s	I
BORING NO. 1 South Abut.	-	T	w	اما	S	Groundwater Elev.: First Encounter ft		T	w	Qυ	I
Station 1150 + 80 Offset 6.6 ft Rt.	_					Upon Completionft					I
Ground Surface Elev. 672.8 Pavement	_ ft 672.8	(ft)	(46")	(tsf)	(%)	After Hrs ft Gray Mottled Silty Clay to Clay	\perp	(ft)	(/6")	(tsf)	4
ravellielit	0/2.6	-				(Embankment) (continued) 65	1.8	-			I
		\exists				Gray/Green Mottled Silty Clay		ℸ	1 2	1.2	4
Black to Brown Sifty Clay	670.8	-				Black Sity Clay Loam	3.8	\dashv	2	B	I
(Embankment)						, ,	_	⊐			t
		-	2					\dashv	2		I
			2		24		-	╛	4		t
		-5	2				2	-25	7		4
	666.8	٦				646	3.6	\exists			I
Gray Silty Clay (Embankment)			2	1.3	28	Dark Gray Mottled Silty Clay Logm		\exists	1	2.0	4
		-	2	1.3 B	28	Louin	-		3	2.U S	I
							_	コ			1
		-	1			643		\dashv	0		I
	-		2	1.4	28	Brown to Gray Clay Loam		╛	1	1.2	t
Brown/Black Mottled Silty Clay	662.8	-10	4	В		(Weathered Till) (Trace of Free Water)	-	-30	2	В	4
(Embankment)						(made of the trade)	_				I
		-	2		27	Gray Clay Loam Till	1.3	7	5	3.1	4
	-		4			City City Court 111	-		6	s	I
		_					-	4			1
		-	0					\exists	2		I
			3		29		-		7	4.5	1
		-15	3				-	-35	10	В	1
		⊐					_				I
		+	2	1.7	29			\dashv			١
		J	4	В			-	J			I
Gray Mottled Silty Clay to Clay	654.8	4					-	4			١
(Embankment)		Н	1					\dashv	5		I
			2	1.3 B	30		-	٦	8	5.5	1
		-20	. 3		_	is used when this information is not	_	-40	10	S	1

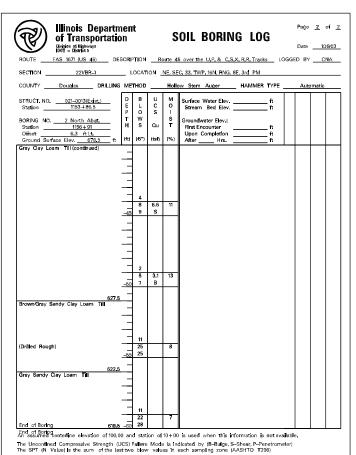
of Transportation Division of Highways DIVIT - District 5	,			٠	OIL BORING LOG	Date10/8/03
	SCRIPT	ION	Ro	ute 4	5 over the U.P. & C.S.X. R.R. Tracks LOG	GED BYCNA_
SECTION 22VBR-1	_ LO	CATIO	ON _	NE, SE	C. 33, TWP. 16N, RNG. 8E, 3rd. PM	
COUNTY <u>Douglas</u> DRILLING	METH	IOD		Holle	ow Stem Auger HAMMER TYPE _	Automatic
STRUCT. NO. 021-0013[Exist.]	E P T	B L O W S	Ou Ou (tsf)	M O I S T	Surface Weter Elev.	
Gray Clay Loam Till (continued)	1,55	_	,,	(7-)	Alter ns n	+
Gray Fine Sand Seam 626.8 GrayBrown Sandy Clay Losm Till	-45	3 4 11	3.7 S	13		
	J	11 16 21		3		
Gray Sandy Clay Loam TIII 620.8	1	11				
		24 26		3		
	=					
(Drilled Hard/Rough)		16				
End of Borling 612.8	_	35 25		8		

Illinois Dep of Transpor	artm tatio	en n	t		S	OIL BORING LOG			1 10	
	DEC	COL	TON.			5 over the U.P. & C.S.X.R.R. Tracks LC				
·							Juuc	ום ט		IVA
·		_				C. 33, TWP. 16N, RNG. 8E, 3rd PM				
COUNTY <u>Douglas</u> DF	HLLING	ME.	THOD	_	Holk	ow Stem Auger HAMMER TYPE	_	Auto	matic	
STRUCT, NO. 021–0013(Exist.) Station 1153 + 86.5	_	D E P	B L O	U C S	М О	Surface Water Elevft Stream Bed Elevft	D E P	B L O	U C s	М О
BORING NO. 2 North Abut. Station 1156+91 Offset 6.3 ft Lt.	_	T H	w s	Qu	S	Groundwater Elev.: First Encounter ft	T H	W S	Qu	S T
Ground Surface Elev. 678.5	ft	(ft)	{⁄6"}	(tsf)	{%}	Upon Completionft After Hrsft	(ft)	(6")	(tsf)	(%)
Pavement	678.5					Black to Dark Gray Silty Clay to Silty Clay Loam (Embankment) (continued)	Ξ	1		
Brown Mottled Sandy Clay Loam	677.0	_					_	2	1.8	26
(Embankment)		_					_	3	Ş	
		-	1				-	1		
		_	1 2	2.1 B	18	653,5	25	2	1.8 S	26
		-	_			Black to Green Mottled Sility Clay to Clay	-25			
(No Sample Rocovered)	672.0	_	1 2				_	3	1.4	28
(NO DESTINATION NOCOVERED)			2				=	4	В	20
		٦	1			650.5 Black Silty Clay Loam to Silty Clay	_	2		
	669.0	-	1	1.3	22		-	3		29
Green/Gray Mottled Slity Clay Loam (Embankment)		_10	2	В			_30	4		
		-	1				-	,		
		\exists	1 2	1.2 B	30		_	1	1.4 S	28
	665.6	+		ь			-	-	-	
Black to Dark Gray Silty Clay to Silty Clay Loam (Embankment)		L	1				_	,		
,,,		Ⅎ	2	1.7	30	644.0	_	3	1.6	22
		-15	3	В		Brown Mottled Sandy Clay Loam (Weathered Till)	_35	3	S	
		╛				642.5				
		\exists	2	1.4	28	Gray Clay Loam Till	=			
		Ⅎ	2	В						
		-					_			
		Ξ	1		L.,		_	5		L
		-20	2	1.5 S	28		_	7	4.5 B	22

An assumed centerline elevation of 103,00 and station of 10+00 is used when this information is not available.

The Unconfined Compressive Strength (UCS) Fallure Mode is Indicated by (8-Bulge, S-Shear, P-Penetrometer)

The SPT (N Value) is the sum of the last two blow values in each sampling zone (AASHTO T208)



SOIL BORING LOGS

F.A.S. RT. 1671 - SEC. 22VBR-1

DOUGLAS COUNTY

STATION 1151+65.86 (SOUTH)

STATION 1154+99.02 (NORTH)

STRUCTURE NO. 021-0061 (SOUTH)

STRUCTURE NO. 021-0060 (NORTH)

ROUTE NO.	SECTION	cox	JNTY	TOTAL SHEETS	SHEET NO.	SHEET NO. 45
F.A.S. 1671	‡	DOUG	LAS	181	129	46 SHEETS
FED. ROAD DIST. NO. 7 ILLINOIS		ILL INDIS	FED. AID FRI	DJECT-		

Contract #70258 ‡ 22VBR-1 and 144SBR-2

Illinois Dep of Transpor	artn	ien	t		G	OIL BORING LOG	<u>.</u>		Page	1	of
División of Highways 1007 - District 5	laut	,,,			3	OIL DOMING LOC	•		Date	2	/64
ROUTE FAS 1671 (US 45)	_ DE	SCRIF	ионт	Ro	oute 4	5 over the U.P.& C.S.X.R.R. Tracks	LC	OGGE	D BY	c	N/
SECTION 22VBR-1		_ L	OCAT	ON _	NE, SE	C. 33, TWP. 16N, RNG. 8E, 3rd PM					
COUNTY <u>Douglas</u> DR	ILLING	ME	THOD	_	Holle	ow Stem Auger HAMMER	TYPE	_	Auto	matic	
STRUCT, NO. 021-0013(Exist.) Station 1153 + 86.5	_	D E P T	B L O W	3 C W	≥ 0 – 0	Stream Bed Elev.	ft ft	1480	BLOW	UCS	
BORING NO. 3 Fler 2 Station 1152+04 Offset 19.0 ft Lt. Ground Surface Elev. 651.5	 π	н	S (/6")	Qu (tsf)	T (%)	After Hrs.	ft ft	H (ft)	S (46")	Qu (tsf)	
Brown/Black Mottled Silty Clay Loam		=				Gray Clay Loam Till with Interbedded Sand & Sift Seams (continued)		_			
		\exists				Gray Sandy Clay Loam to Sand Loam Till	629.5	Ξ			
		5	2 3 4	0.8 B	30				27 43 50–4″		
Brown/Gray Mottled Silty Clay Losm	644.5	\exists	1	1.8	21	Green-Gray Sandy Clay Loam Till	625.5	_			
Brown Clay Loam TII		=	2	В				Ξ	17		
Gray Clay Loam TIII	642.0	_10	5	2.7 B	16			_30	28 46	9.2 S	
		4	2 4 5	2.6 B	16	Gray Sandy Clay Loam TIII	619.5	Ξ			
	637.5	\exists	2					_	37		
Gray Clay Loam Till with Interbedded Sand & Silt Seams		 15	6 10	3.3 B	13	End of Borling	616.5	_	50–5"		
		\exists						=			
		4	2	2.0				\exists			
		-20	5	E				-4 0			L

Division of Highways IDOT - District 5 ROUTE FAS 1671 (US 46) DE	SCRIPTION	I Ro	oute 4	5 over the U.P. & C.S.X.R.R. Tracks LC	OGGE	D BY	c	NΑ
SECTION 22VBR-1	LOCA	ON_	NE, SE	C. 33, TWP. 16N, RNG. 8E, 3rd PM				
COUNTY Douglas DRILLING	METHOD		Holk	ow Stem Auger HAMMER TYPE	_	Auto	omatic	
STRUCT. NO. 021-0013(Exist.)	D B L P O T W H S	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev. ft Stream Bed Elev. ft Groundwater Elev.: ft First Encounter ft Upon Completion ft After Hrs. ft	D E P T H	B L O ≷ S (6°)	U C S Qu (tsf)	N C I S T
Brown'Gray Mottled Silty Clay 651.	' <u>- </u> - <u>- </u>			Gray Clay Loam Till (continued) 629.1 Gray/Green Sand Loam Till				
	1 2 -5 3	0.9 B	28	625,1		9 22 20		1
(Trace of Free Water) 644. Gray Clay Loam TII	1 4 5			Gray Sandy Clay Loam 111	_	12		
	3 _10 4	2.7 B	16		_30	23 27		10
	4 5	2.9 B	16		=	25		
	7 _15 8	6.6 S	12	End of Boring 616.1	_	50–5"		8
	2 4 4	2.7 B	14		40			

Illinois Departn of Transportation	nen	t		S	OIL BORING LOG	Page <u>1</u> of
Division of Highways IDOT - District 5	<i>,</i> ,,,				OIL DOMING LOG	Date
ROUTE FAS 1671 (US 48) DE	SCRI	иоіт	R	oute 4	5 over the U.P. & C.S.X.R.R. Tracks LOGO	ED BYCN/
SECTION 22VBR-1	_ [OCAT	ON _	NE, SE	C. 33, TWP. 16N, RNG. 8E, 3rd PM	
COUNTY <u>Douglas</u> DRILLING	ME	THOD	_	Holk	ow Stem Auger HAMMER TYPE	Automatic
STRUCT, NO. 021–0013(Exist.) Station 1153 + 86.5	D E P	B L O	U C S	M 0 -	Surface Water Elevft Stream Bed Elevft	
BORING No. 5 Pler 5 (Settlement)	H	W S	Qu	S T	Groundwater Elev.: First Encounter	
Ground Surface Elev. 651.4 ft Black Silty Clay with Roadpack	(ft)	(/6")	(tsf)	(%)	After Hrs ft	
Shor Shy Shay Will Household	П					
649.4	. =					
Brown Mottled Silty Clay Loam (Trace of Sand)	_					
	=	2		23		
	_5	3				
Brown Sandy Clay Loam Till	¥_	3				
	_=	5	2.6	15		
643,4	_	5	S			
Gray Clay Loam Till	_	3				
	-	4	3.3	14		
	_10	5	S			
	-	2				
	\exists	6	3.9 S	14		
	\equiv	-	r			
	_	2				
		8	4.4 S	11		
	\exists					
	_					
	_					
	_	2				
631.4	-20	4 6	2.1 B	14		
End of Boring	00 an	d stat	on of	10+00	is used when this information is not availab	ale

SOIL BORING LOGS

F.A.S. RT. 1671 - SEC. 22VBR-1

DOUGLAS COUNTY

STATION 1151+65.86 (SOUTH)

STATION 1154+99.02 (NORTH)

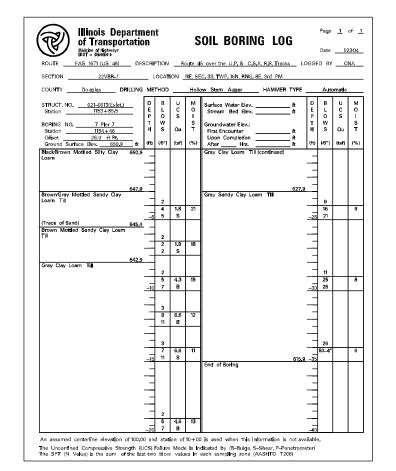
STRUCTURE NO. 021-0061 (SOUTH)

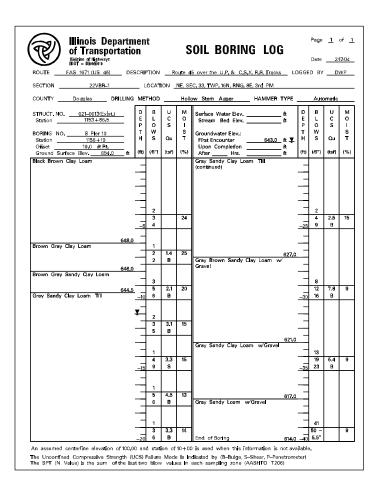
STRUCTURE NO. 021-0060 (NORTH)

ROUTE NO.	SECTION	cox	UNTY	TOTAL SHEETS	SHEET NO.	SHEET NO. 46
F.A.S. 1671	†	DOUG	LAS	181	130	46 SHEETS
FED. ROAD DIST. NO. 7 ILL INDIS		FED. AID PR	DJECT-			

Contract #70258 ‡ 22VBR-1 and 144SBR-2

Illinois Departn of Transportation	n	•		S	OIL BORING LOG				
Division of Highways IDOT - District 5								2	
<u> </u>					5 over the U.P. & C.S.X.R.R. Tracks L	OGGE	D BA		NA.
SECTION 22VBR-1									_
COUNTY <u>Douglas</u> DRILLING	ME	THOD	_	Holle	ow Stem Auger HAMMER TYPE	_	Auto	omatic	_
STRUCT. NO. 02F-0013(Exist.)	DEP⊤+ (£)	B L O W S	U C S Qu (tsf)	M 0 S T (%)	Surface Water Elev. ft Stream Bed Elev. ft Groundwater Elev.: First Encounter ft Upon Completion ft After frs. ft	D E P T H	B L O W S	U C S Qu (tsf)	8 1
Brown/Gray Mottled Silty Clay	1,14	70 /	(631)	(75)	After Hrs ft Gray Clay Loam Till (continued)	-	,,	(23,	-
Losm	=				Gray Sandy Clay Loam TIII	9			
	_					_			
	-					_			
	_=	2				Ξ	5		
	_5	3 4	1.2 B	25		-25	17 33	8.5 S	1
645.9	-					_			
Brown Clay Loam TII		1	4.1	16		_			
	_	6	4.1 B	10		_			
Gray Clay Loam Till	-					_			
	\equiv	3	4.6	14		_	10 25		9
	_10	8	s			-30	39		
	Ξ					_			
	-	6	6.2	13		-			
	_	10	S			_			
	\equiv	2				_	30		
	_	6	4.7	13		_	50-5"		8
	15	10	В		End of Boring	9 –35			_
	-					_			
	_					_			
	_					_			
	_	2				_			
	-20	5 8	4.7 B	13		-40			





SOIL BORING LOGS

F.A.S. RT. 1671 - SEC. 22VBR-1

DOUGLAS COUNTY

STATION 1151+65.86 (SOUTH)

STATION 1154+99.02 (NORTH)

STRUCTURE NO. 021-0061 (SOUTH)

STRUCTURE NO. 021-0060 (NORTH)

GUARDRAIL DETAIL S.N. 021-0062

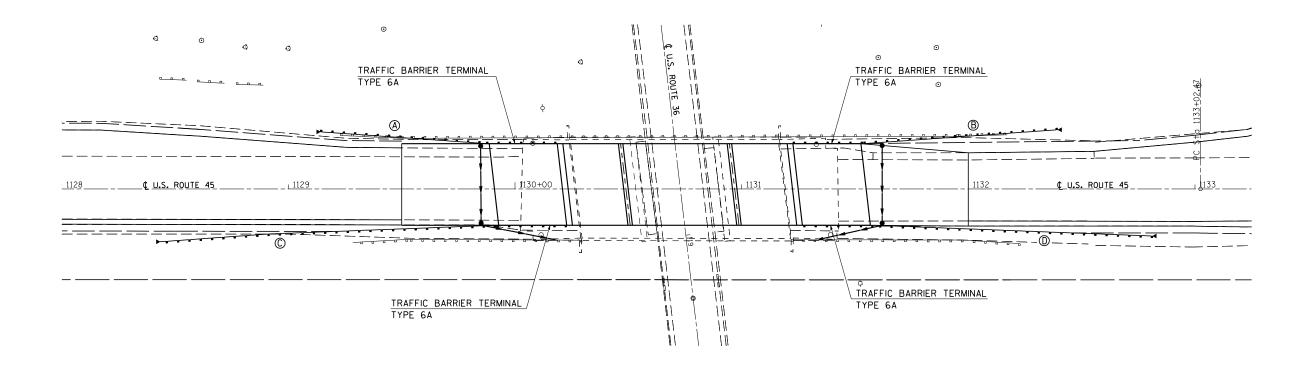
CONTRACT NO. 70258

COUNTY TOTAL SHEET NO. STA. TO STA. FED. ROAD DIST. NO. | ILLINOIS | FED. AID PROJECT

• 144SBR-2 & 22VBR-1



SEGMENT (A) 25' SPBGTA @ 1:14 FLARE, I TRAFFIC BARRIER TERMINAL TYPE I, SPECIAL (FLARED) @ 1:14 FLARE SEGMENT (B) 37.5' SPBGTA @ 1:14 FLARE, I TRAFFIC BARRIER TERMINAL TYPE I, SPECIAL (FLARED) @ 1:14 FLARE SEGMENT © 100' SPBGTA @ 1:26 FLARE, I TRAFFIC BARRIER TERMINAL TYPE I, SPECIAL (FLARED) @ 1:14 FLARE SEGMENT (1) 75' SPBGTA @ 1:26 FLARE, I TRAFFIC BARRIER TERMINAL TYPE 1, SPECIAL (FLARED) @ 1:26 FLARE



ILLINOIS DEPARTMENT OF TRANSPORTATION

GUARDRAIL DETAIL S.N. 021-0062

F.A.S. ROUTE 1671 (U.S. ROUTE 45) SECTION 144SBR-2 & 22VBR-1 DOUGLAS COUNTY

SCALE: 1" = 20" DATE: 06/07/06 DRAWN BY: B.B.P.

CONTRACT NO. 70258

COUNTY TOTAL SHEET NO. 70258 DOUGLAS 181 132

1671 STA. TO STA. FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT

• 144SBR-2 & 22VBR-1

BILL OF MATERIAL (ONE HEADWALL)

Bar	No.	Size	Length	Shape
b	2	#4	8'-10"	
b_1	1	#4	11'-3"	
b2	1	#4	13'-7"	
b3 b4	1	#4	16′-0"	
b4	2	#4	18'-0"	
d	12	#4	5'-4"	7
h h ₁	1	#4	23'-0"	
h ₁	2	#4	7′-8" 7′-6"	
h ₂	2	#4	7′-6"	
S	2	#4	23'-8"	
17	_		C/ 011	
V .	2	#4	6'-8"	
V -	2	_	6'-3"	
V 2	2	#4	5'-10"	
V 3	2	#4	5'-10" 5'-6" 5'-2"	
V 4	2	#4	4'-10"	
v ₁ v ₂ v ₃ v ₄ v ₅ v ₆	2 2 2 2 2 2 2 2	#4	4'-6"	
V 7	2	#4	4'-2"	
• /		#4	7 2	
W	6	#4	3'-10"	
w ₁	2	#4	3'-10" 3'-8"	
w ₂	2	#4	2'-10"	_
w ₃	6 2 2 2	#4	2'-10" 2'-0"	
-				
Reinfo	rcement	bars	Lb.	250.0
	SI Conc		Cu. Yd.	4.8

Item	Unit	Oty.
Box Culvert End Section	Each	2

<u>NOTES</u>

- requirements of Art. 1006.10 of Standard Specs.
- Class SI Concrete shall be used throughout.
- The Precast Concrete Box Culvert Sections shall conform to the requirements of AASHTO M259.
- All dimensions are in FEET (')-INCHES (") unless otherwise noted.
- Concrete and Rebar quantities and lengths calculated for the cast-in-place End Sections will vary based on the precast box culverts supplied.
- each for BOX CULVERT END SECTION as outlined in Art. 540.08, which prices shall include all concrete, rebar. and all other items necessary to complete the proposed
- Drain holes shall be provided in accordance with Art. 503.12.
- Drawings not to scale.

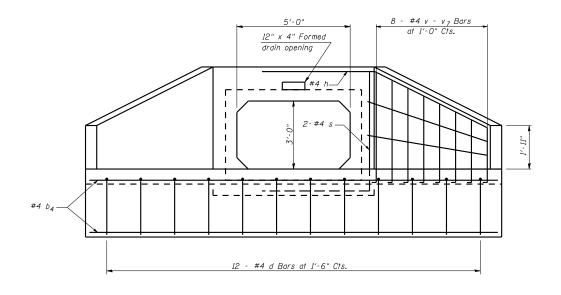
BOX CULVERT END SECTIONS (CAST IN PLACE) 2 @ 5.0' X 3.0' STA. 1155+80.000 NO SKEW REINFORCED CONCRETE HEADWALL DETAILS

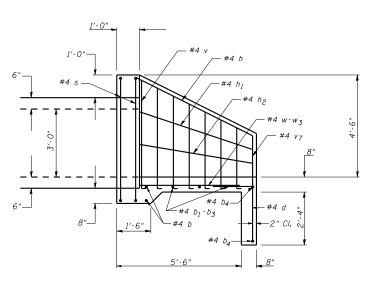
ILLINOIS DEPARTMENT OF TRANSPORTATION

BOX CULVERT END SECTION DETAIL LT. & RT. STATION 1155+80.00 F.A.S. ROUTE 1671 (U.S. ROUTE 45) SECTION 144SBR-2 & 22VBR-1

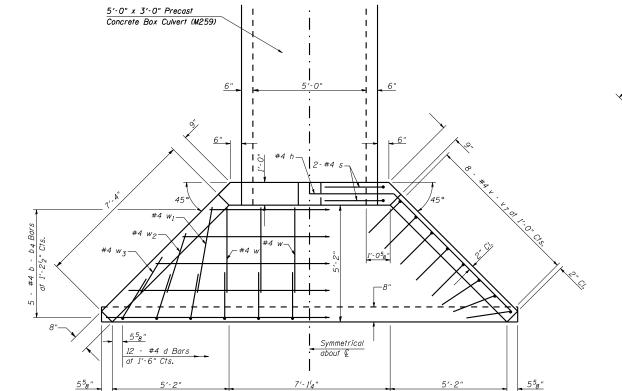
DOUGLAS COUNTY SCALE: NOT TO SCALE DRAWN BY: B.B.P. DATE: 07/06/06

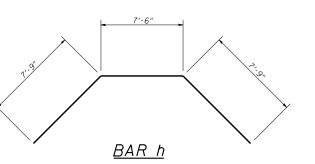
DETAIL OF BOX CULVERT END SECTION LT. & RT. STATION 1155 + 80.00

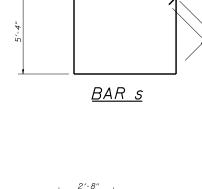




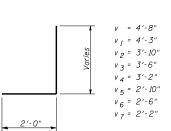
END ELEVATION HALF SIDE ELEVATION

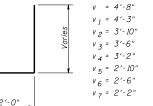


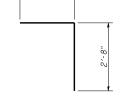




<u>6′</u>-6"







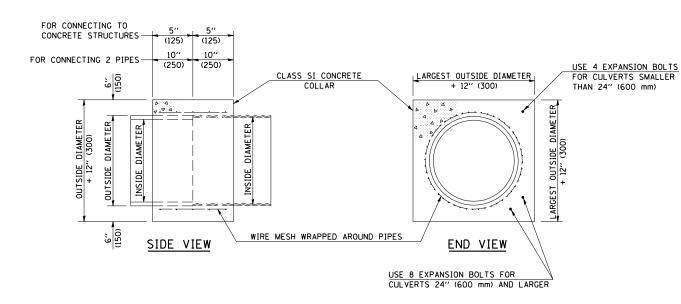
BAR d

v BARS

<u>PLAN</u>

18'-42"

DETAIL OF CONCRETE COLLARS



QUANTITIES FOR	CONCRETE PIPES
INSIDE DIAMETER	ESTIMATED
OF PIPF	CLASS SI CONCRETE
	REQUIRED
	20" (500 mm) WIDTH
INCH (mm)	CU. YD. (m3)
4" (100)	0.14 (0.11)
6" (150)	0.16 (0.12)
8" (200)	0.19 (0.14)
10" (250)	0.22 (0.17)
12" (300)	0.25 (0.19)
15" (375)	0.30 (0.23)
18" (450)	0.35 (0.27)
24" (600)	0.45 (0.35)
30" (750)	0.57 (0.43)
36" (900)	0.69 (0.53)
42" (1050)	0.83 (0.63)
48" (1200)	0.97 (0.74)
54" (1350)	1.12 (0.86)
60" (1500)	1.28 (0.98)

QUANTITIES FOR METAL PIPES	
INSIDE DIAMETER OF PIPE	ESTIMATED CLASS SI CONCRETE REQUIRED
INCH (mm)	20" (500 mm) WIDTH CU.YD. (m 3)
4" (100)	0.12 (0.09)
6" (150)	0.14 (0.11)
8" (200)	0.16 (0.12)
10" (250)	0.19 (0.14)
12" (300)	0.21 (0.16)
15" (375)	0.25 (0.19)
18" (450)	0.29 (0.22)
24" (600)	0.38 (0.29)
30" (750)	0.47 (0.36)
36" (900)	0.59 (0.45)
42" (1050)	0.69 (0.53)
48" (1200)	0.81 (0.62)
54" (1350)	0.93 (0.71)
60" (1500)	1.05 (0.81)

GENERAL NOTES

- 1. CLASS SI CONCRETE SHALL BE USED THROUGHOUT.
- 2. WHEN CONCRETE COLLARS ARE USED TO CONNECT PIPES OF DIFFERENT OUTSIDE DIAMETERS, THE CONCRETE COLLAR SHALL BE FORMED USING THE LARGEST OUTSIDE DIAMETER (SEE END VIEW).
- 3. THE WIRE MESH SHALL WEIGH NOT LESS THAN 54*/100 SQ. FT. (2.63 kg/m²).
- 4. WHEN CONCRETE COLLARS ARE CONSTRUCTED ADJACENT TO AN EXISTING CONCRETE STRUCTURE (HEADWALLS, ETC.) EXPANSION BOLTS, SHALL
 BE USED AND WILL BE PAID FOR AT THE CONTRACT UNIT PRICE, EACH, FOR EXPANSION BOLTS OF THE SIZE SPECIFIED IN THE PLANS.
- 5. CONCRETE COLLARS WILL BE PAID FOR AT THE CONTRACT UNIT PRICE, PER CUBIC YARD (CUBIC METER), FOR CONCRETE COLLARS INCLUDING ALL MATERIAL AND LABOR SPECIFIED TO COMPLETE THE WORK IN PLACE.

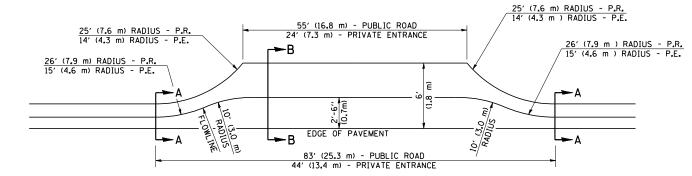


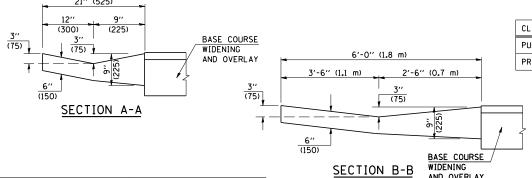
(NEXT TO RESURFACING)

COUNTY TOTAL SHEET NO. SECTION 1671 DOUGLAS 181 133 STA. TO STA. FED. ROAD DIST. NO. | ILLINOIS | FED. AID PROJECT

CONTRACT NO. 70258

• 144SBR-2 & 22VBR-1





NAME DATE REVISIONS DESIGNED D.L.P. 3-17-95 DATE ALL DIMENSIONS ARE IN INCHES (MILLIMETERS) CHECKED B.K.T. 5-25-95 K.A.G. 06/03 UNLESS OTHERWISE SHOWN. CADD NO.

ESTIMATED QUANTITIES

CLASS SI CONCRETE (OUTLET) CU. YD. (m3) PUBLIC ROAD (P.R.) 9.1 (7.0) PRIVATE ENTRANCE (P.E.) 4.5 (3.4)

NOTES

PRIVATE ENTRANCE BASED ON 12' (3.6 m) SURFACE AND 20' (6.1 m) RADII. PUBLIC ROADS BASED ON 24' (7.2 m) SURFACE AND 35' (10.7 m) RADII.

CLASS SI CONCRETE SHALL BE USED THROUGHOUT. SPECIAL ENTRANCE WILL BE PAID FOR AT THE CONTRACT UNIT PRICE PER CUBIC YARD (CUBIC METER) FOR CLASS SI CONCRETE (OUTLET).

ILLINOIS DEPARTMENT OF TRANSPORTATION

SPECIAL ENTRANCE & CONCRETE COLLARS DETAILS

F.A.S. ROUTE 1671 (U.S. ROUTE 45) SECTION 144SBR-2 & 22VBR-1 DOUGLAS COUNTY

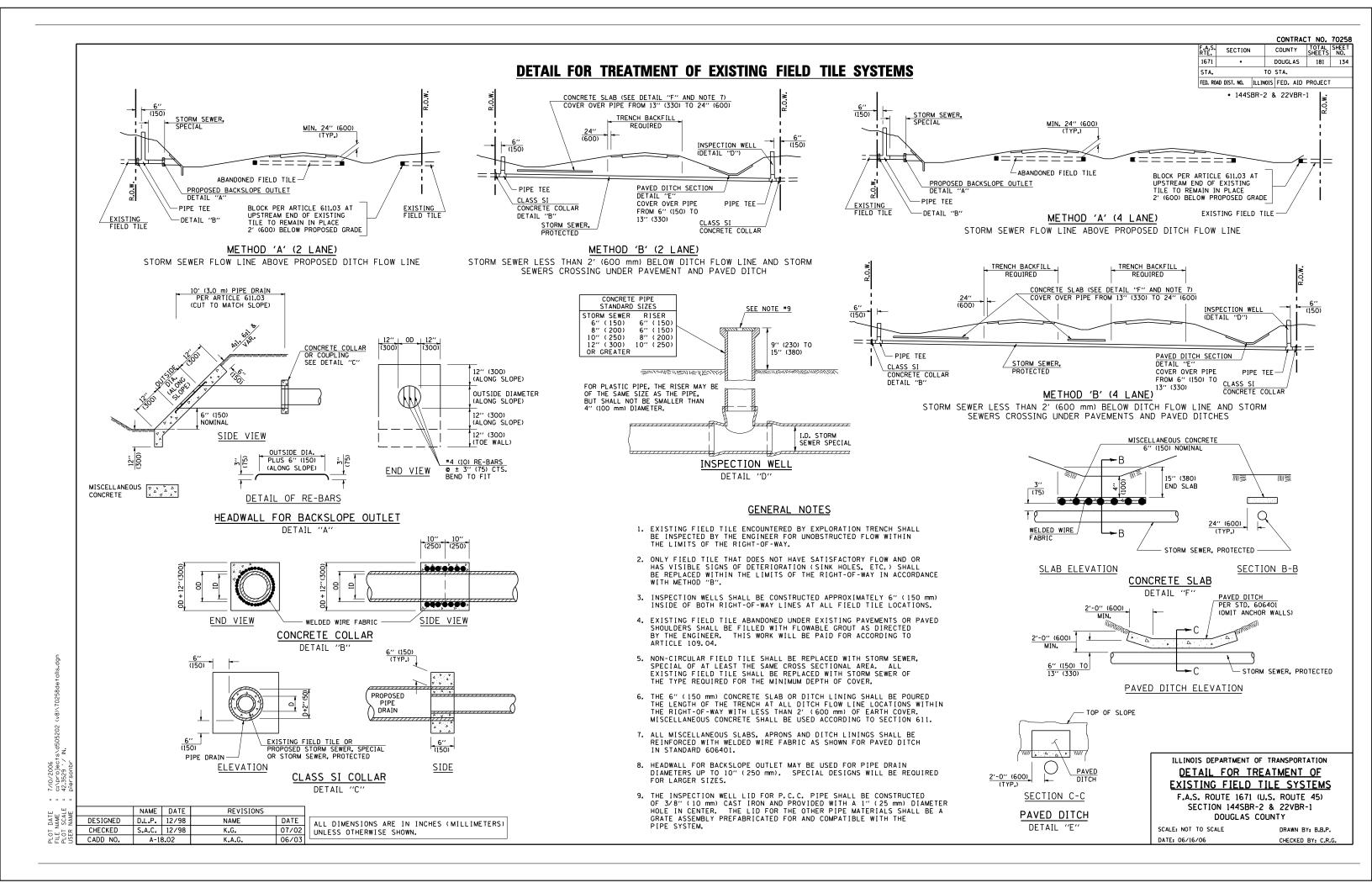
DATE: 06/07/06

CHECKED BY: C.R.G.

NAME DATE REVISIONS DESIGNED J.M.H. 4-80 NAME DATE CHECKED D.L.P. 08/00 P.E.K. 4-80 CADD NO. J-5.45 K.A.G. 09/03

ALL DIMENSIONS ARE IN INCHES (MILLIMETERS) UNLESS OTHERWISE SHOWN.

SCALE: 1" = 20" DRAWN BY: B.B.P.



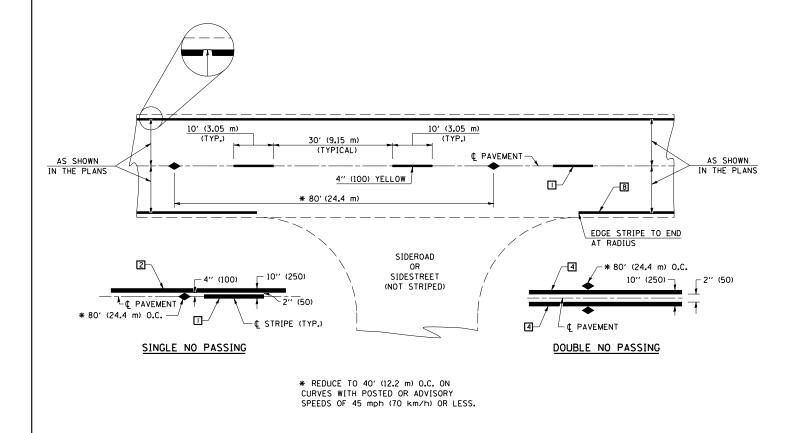
CONTRACT NO. 70258

COUNTY TOTAL SHEET NO.

DOUGLAS TO STA. FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT

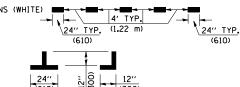
• 144SBR-2 & 22VBR-1

TYPICAL APPLICATIONS OF PAVEMENT MARKINGS AND MARKERS



TYPICAL PAVEMENT MARKING LEGEND

- 1 4" (100) SKIP-DASH (YELLOW)
- 2 4" (100) SOLID (YELLOW)
- 3 12" (300) DIAGONAL (YELLOW)
- 4 4" (100) DOUBLE YELLOW (NARROW)
- 5 RESERVED
- 6 RESERVED
- 7 4" (100) SKIP-DASH (WHITE)
- 8 4" (100) SOLID (WHITE)
- 9 12" (300) DIAGONAL (WHITE)
- 10 6" (150) CROSS WALK (WHITE)
- 11 24" (600) STOP BAR (WHITE)
- 12 8" (200) SOLID (WHITE)
- 13 4" (100) LANE LINE EXTENSIONS (WHITE) ______4′ TYP. | 4' TYP. |24" TYP. (1.22 m)
- 14 4" (100) PARKING WHITE



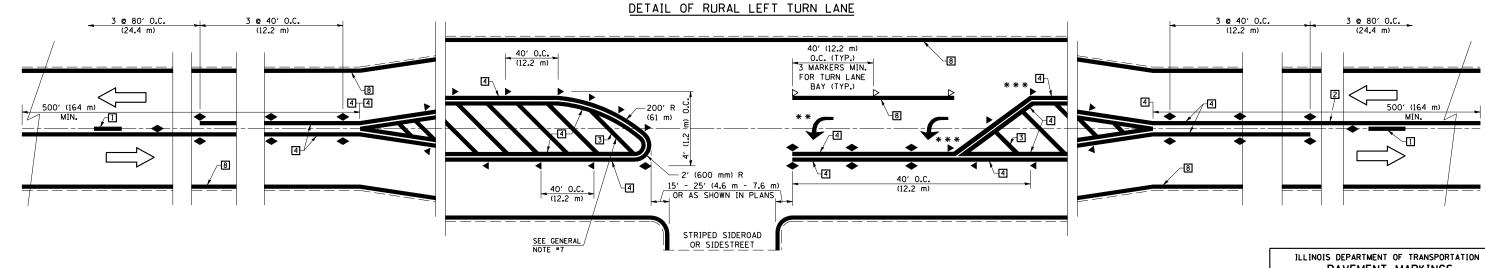
<☐ TRAFFIC

_ 2" (50)

= 6" (150) CTS.

TYPICAL PAVEMENT MARKERS LEGEND

- TWO-WAY AMBER MARKER
- ONE-WAY AMBER MARKER
- ONE-WAY CRYSTAL MARKER



NAME DATE J.M.H. 5/85 6/88 FMS CTD 6/85 6/88 DESIGNED NAME DATE GEOMETRICS/K.A.G. 07/02 CHECKED CADD NO. F-5**.**25 K.A.G.

SHEET 1 OF 4

ALL DIMENSIONS ARE IN INCHES (MILLIMETERS) UNLESS OTHERWISE SHOWN.

* * * REDUCE SPACING IF NECESSARY TO ASSURE MARKERS AT CORNER POINTS.

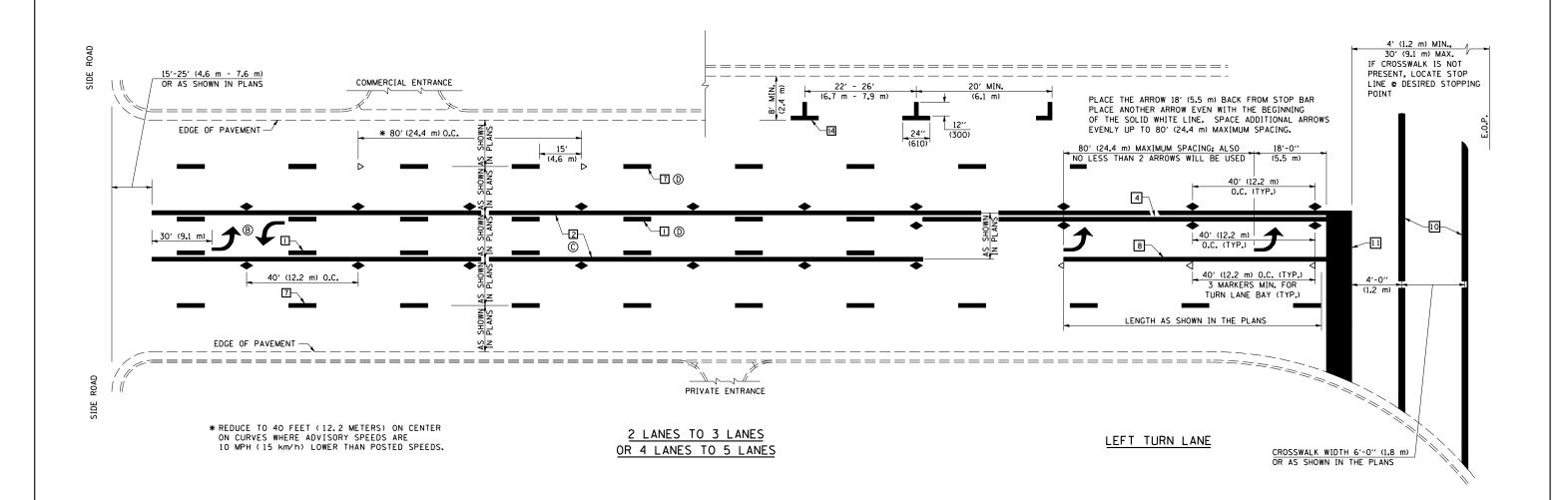
** TURN ARROWS SHALL BE PLACED AS SHOWN ON SHEET #2.

PAVEMENT MARKINGS AND MARKINGS DETAIL F.A.S. ROUTE 1671 (U.S. ROUTE 45) SECTION 144SBR-2 & 22VBR-1 DOUGLAS COUNTY SCALE: NOT TO SCALE D DRAWN BY: B.B.P.

DATE: 06/16/06

• 144SBR-2 & 22VBR-1

TYPICAL APPLICATIONS OF PAVEMENT MARKINGS AND MARKERS



SHEET 2 OF 4

ALL DIMENSIONS ARE IN INCHES (MILLIMETERS)
UNLESS OTHERWISE SHOWN.

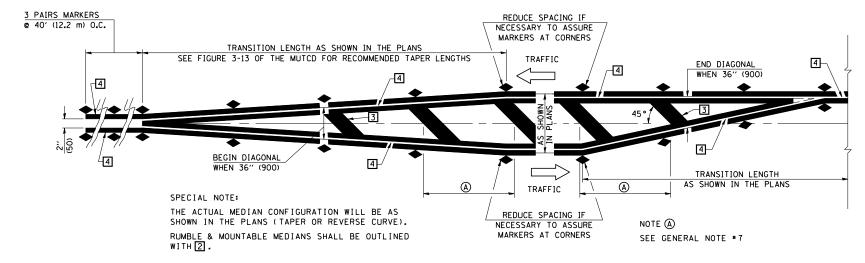
ILLINOIS DEPARTMENT OF TRANSPORTATION
PAVEMENT MARKINGS
AND MARKINGS DETAIL
F.A.S. ROUTE 1671 (U.S. ROUTE 45)
SECTION 144SBR-2 & 22VBR-1
DOUGLAS COUNTY

Scale: NOT TO Scale DRAWN BY: B.B.P.

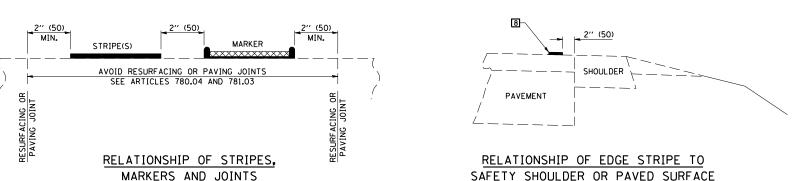
16/16/06

CHECKED BY: C.R.G.

TYPICAL APPLICATIONS OF PAVEMENT MARKINGS AND MARKERS



TYPICAL MEDIAN TRANSITIONS



SPECIAL NOTES:

- TURN ARROW PAIRS SHALL BE PLACED AT 250' (75 m) INTERVALS AND SHALL BE EVENLY SPACED BETWEEN BOTH ENDS OF THE BIDIRECTIONAL LEFT TURN LANE.
- © THE SOLID YELLOW PAVEMENT MARKINGS 2 SHOULD GENERALLY START OR END NEAR THE RADIUS POINT OF EACH STREET RETURN EXCEPT WHERE ONE OR BOTH ENDS WOULD INCLUDE STOP BARS.
- THE SKIP-DASH PAVEMENT MARKINGS [] OR [7] SHOULD BE CENTERED BETWEEN BOTH ENDS OF EACH CITY BLOCK AND SHALL BE PLACED SO THEY LINE UP ACROSS FROM EACH OTHER. SEE EXAMPLE ON SHEET 2 OF 3.
- TURN ARROW SIZE DEPENDS ON THE LOCATION. RURAL LOCATION LARGE ARROW SIZE URBAN LOCATION SMALL ARROW SIZE

GENERAL NOTES

- 1. WHEN MEDIANS ARE PRESENT, PAVEMENT MARKINGS ARE TO BE PLACED ADJACENT TO MEDIANS.
- 3. SOME OF THE INFORMATION INCLUDED WITH THIS DETAIL MAY NOT BE APPLICABLE TO THIS IMPROVEMENT.
- PAVEMENT MARKINGS ARE TO BE EXTENDED THROUGH OMISSIONS WHEN APPLICABLE.
- 5. A STRIPING KEY IS AVAILABLE ELSEWHERE AND SHALL BE SHOWN WHERE THE QUANTITIES ARE LISTED.
- 6. FINAL PAVEMENT MARKINGS SHALL BE IN PLACE PRIOR TO PLACING ANY RAISED REFLECTIVE PAVEMENT MARKERS.
- 7. THE FOLLOWING CRITERIA SHALL BE USED FOR SELECTING THE DIAGONAL PAVEMENT MARKING SPACING. <30 MPH USE 15' (<50 km/h USE 4.5 m) 30-45 MPH USE 20' (50-75 km/h USE 6.0 m) >45 MPH USE 30' (>75 km/h USE 9.0 m)

CADD NO.

NAME DATE REVISIONS DESIGNED J.M.H. NAME GEOMETRICS/K.A.G. 07/02 CHECKED F-5**.**25

SHEET 3 OF 4

ALL DIMENSIONS ARE IN INCHES (MILLIMETERS) UNLESS OTHERWISE SHOWN.

TYPICAL DOUBLE TURN ARROWS (WHITE)

LEFT ARROW

REVERSE FOR RIGHT ARROW

AREA = 15.6 SQ. FT. (1.47 m^2)

(WHITE)

DATE

09/05

K.A.G.

(660)

(1.02 m)

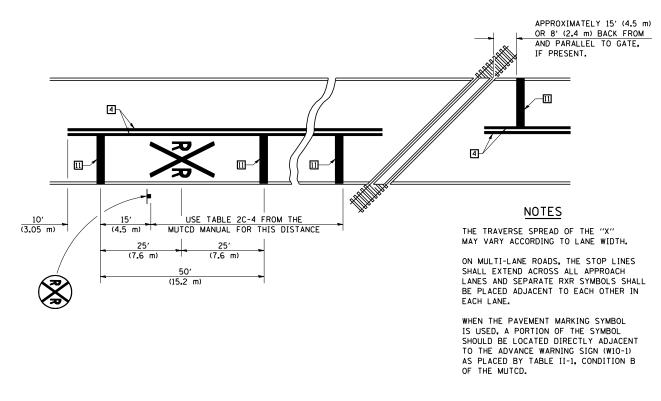
ILLINOIS DEPARTMENT OF TRANSPORTATION PAVEMENT MARKINGS AND MARKINGS DETAIL F.A.S. ROUTE 1671 (U.S. ROUTE 45) SECTION 144SBR-2 & 22VBR-1 DOUGLAS COUNTY

Sheet 3 of 4 SCALE: NOT TO SCALE DRAWN BY: B.B.P.

DATE: 06/16/06

TYPICAL APPLICATIONS OF PAVEMENT MARKINGS AND MARKERS

• 144SBR-2 & 22VBR-1



PAVEMENT MARKINGS AT
RAILROAD-HIGHWAY GRADE CROSSING

SHEET 4 OF 4

ALL DIMENSIONS ARE IN INCHES (MILLIMETERS)
UNLESS OTHERWISE SHOWN.

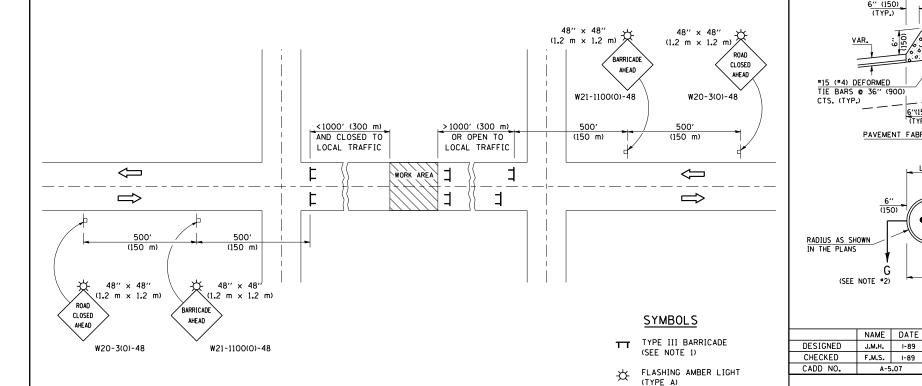
ILLINOIS DEPARTMENT OF TRANSPORTATION
PAVEMENT MARKINGS
AND MARKINGS DETAIL
F.A.S. ROUTE 1671 (U.S. ROUTE 45)

F.A.S. ROUTE 1671 (U.S. ROUTE 45)
SECTION 144SBR-2 & 22VBR-1
DOUGLAS COUNTY
Sheet 4 of 4
SCALE: NOT TO SCALE
DRAWN BY: B.B.P.

DATE: 06/16/06

CHECKED BY: C.R.G.

TYPICAL APPLICATION OF TRAFFIC CONTROL **DEVICES FOR ROAD CLOSURE**



GENERAL NOTES

- 1. TYPE III BARRICADES SHALL BE AS SHOWN ON STANDARD 702001 "TYPICAL APPLICATIONS OF TYPE III BARRICADES CLOSING A ROAD". EACH TYPE III BARRICADE SHALL HAVE TWO FLASHING AMBER LIGHTS MOUNTED ABOVE IT.
- 2. IF THE ROAD IS OPEN TO LOCAL TRAFFIC OR EXCEEDS 1000' (300 m), ANOTHER SET OF TYPE III BARRICADES, EQUIPPED AS IN NOTE 1 ABOVE, SHALL BE PLACED AT EACH END OF THE WORK AREA.
- 3. WHEN A STOP CONDITION EXISTS, NO SIGNS ARE REQUIRED IN ADVANCE OF THE "STOP" SIGN WHEN THE ROAD IS CLOSED WITHIN 100' (30 m) OF THE INTERSECTION.
- 4. STANDARD 702001 SHALL APPLY FOR THE PLACEMENT & DESIGN OF TYPE III BARRICADES.
- 5. IF A TYPE III BARRICADE WITH AN ATTACHED SIGN PANEL WHICH MEETS NCHRP 350 IS NOT AVAILABLE, THE SIGNS MAY BE MOUNTED ON AN NCHRP 350 TEMPORARY SIGN SUPPORT DIRECTLY IN FRONT OF THE BARRICADE.

- 6. REFLECTORIZED STRIPING SHALL APPEAR ON BOTH SIDES OF THE TY III BARRICADES IF ROAD IS OPEN
- 7. ALL SIGNS SHALL BE POST MOUNTED IF THE CLOSURE TIME EXCEEDS FOUR DAYS.
- 8. A MINIMUM OF TWO FLASHING LIGHTS SHALL BE USED AT NIGHT ON EACH APPROACH IN ADVANCE OF THE WORK AREA. FLASHING LIGHTS SHALL BE INSTALLED ABOVE THE FIRST TWO SIGNS IN THE SERIES.
- 9. LONGITUDINAL DIMENSIONS MAY BE ADJUSTED SLIGHTLY TO FIT FIELD CONDITIONS.
- 10. FORMS BT. 725 AND BT. 726 ARE REQUIRED.
- WHEN A SIDEROAD INTERSECTS THE HIGHWAY ON WHICH WORK IS BEING PERFORMED, ADDITIONAL TRAFFIC DEVICES SHALL BE ERECTED AND PROVIDED AS DIRECTED BY THE ENGINEER.
- 12. AN ADDITIONAL SIGN MAY BE REQUIRED AT A MAJOR INTERSECTING ROAD IN ADVANCE OF THE CLOSURE. THE ADDITIONAL SIGN SHALL GIVE THE DISTANCE TO THE BARRICADE IN MILES OR FRACTIONS OF A MILE.

REVISIONS J.H.M. 8-11-87 DESIGNED NAME DATE P.E.K. 8-25-87 CHECKED R.M.H. 12/97 CADD NO. F-5.03 CP/KAG 01/05

ALL DIMENSIONS ARE IN INCHES (MILLIMETERS)

DETAIL OF CONCRETE MEDIAN, TYPE SM-6 (SM-15) (DOWELED)

MORTAR FILLING

AROUND DOWEL

FACE

ALL DIMENSIONS ARE IN

INCHES (MILLIMETERS)

UNLESS OTHERWISE SHOWN

BARS (TYP.)

EXISTING PAVEMENT

(150)

G

14" × 14" OPENING (350 × 350)

DATE

06/02

PLAN OF MEDIAN

REVISIONS

J.Y.B.

WIDTH AS SHOWN IN THE PLANS

SECTION A-A

DOWEL BARS

(350)

SEE STD 606301

(150)

42" (1100)

6"(150) (TYP.)

PAVEMENT FABRIC

TIE BARS @ 36" (900)

(SEE NOTE #2)

NAME DATE

J.M.H. 1-89

CTS. (TYP.)

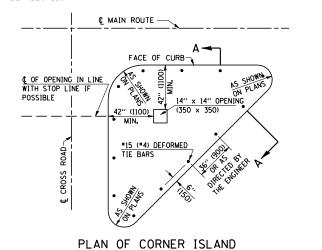
SLOPE TO DRAIN

F.A.S. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.	
1671	•		DOUGLAS	181	139	
STA.	STA. TO STA.					
FED. RO	FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT					

CONTRACT NO. 70258

* 144SBR-2 & 22VBR-1

- 1. THE GENERAL NOTES FOR STANDARD 606301 SHALL APPLY.
- 2. SECTION G-G SHALL BE THE SAME AS SHOWN ON STANDARD 606301.
- 3. DOWEL BARS @ 36" (900) CTS. OR AS DIRECTED BY THE ENGINEER.
- SQ FOOT (m 2) FOR CONCRETE MEDIAN, TYPE SM-6 (SM-15) (DOWELED). INCLUDING THE COST OF FURNISHING AND INSTALLING THE DOWEL BARS, MORTAR FILLING, PAVEMENT FABRIC AND THE REMOVAL AND DISPOSAL OF THE EXISTING PAVEMENT FOR THE 14" x 14" (350 x 350) OPENING, IF REQUIRED, AND NO ADDITIONAL COMPENSATION WILL BE ALLOWED.



GENERAL NOTES

4. THIS WORK WILL BE PAID FOR AT THE CONTRACT UNIT PRICE PER

ILLINOIS DEPARTMENT OF TRANSPORTATION TRAFFIC CONTROL DEVICES & SM-6 DOWELLED MEDIAN DETAILS

F.A.S. ROUTE 1671 (U.S. ROUTE 45) SECTION 144SBR-2 & 22VBR-1 DOUGLAS COUNTY

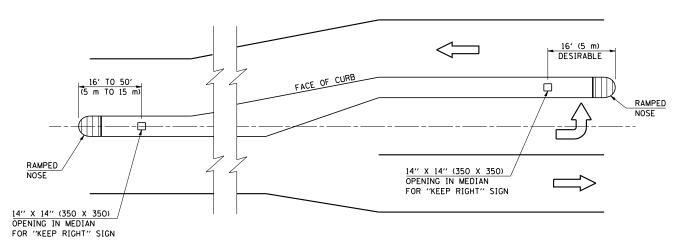
SCALE: NOT TO SCALE DATE: 06/19/06

DRAWN BY: B.B.P. CHECKED BY: C.R.G.

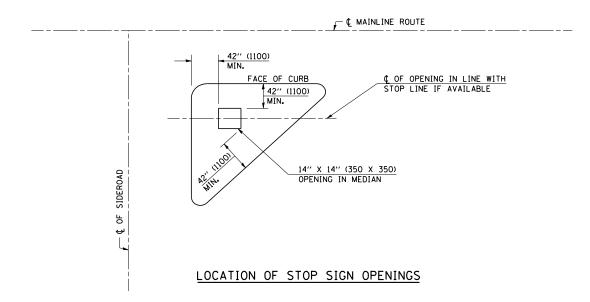
UNLESS OTHERWISE SHOWN.

	RTE.	RTE. SECTION		COUNTY	SHEETS	NC
	1671	•		DOUGLAS	181	1
	STA. FED. ROAD DIST. NO.		TO	STA.		
			ILLINOIS	FED. AID	PROJECT	

• 144SBR-2 & 22VBR-1



LOCATION OF OPENINGS FOR "KEEP RIGHT" SIGNS



GENERAL NOTES

- ALL SMALL ISLANDS SHALL BE CONSTRUCTED WITH THE STOP SIGN ISLANDS AS SHOWN, UNLESS OTHERWISE SPECIFIED.
- 2. OPENINGS FOR SIGNS IN MEDIANS SHALL BE AS SHOWN OR AS DIRECTED BY THE ENGINEER.
- 3. THIS WORK WILL NOT BE PAID FOR SEPARATELY.
 BUT SHALL BE INCLUDED IN THE CONTRACT
 UNIT PRICE FOR THE TYPE OF MEDIAN
 SPECIFIED IN THE PLANS AND NO ADDITIONAL
 COMPENSATION WILL BE ALLOWED.

	NAME	DATE	REVISIONS	
DESIGNED	J.M.H.	8/25/87	NAME	DATE
CHECKED	P.E.K.	8/25/87	D.L.P.	07/98
CADD NO.	A-5.03		K.A.G.	06/03

ALL DIMENSIONS ARE IN INCHES (MILLIMETERS) UNLESS OTHERWISE SHOWN.

ILLINOIS DEPARTMENT OF TRANSPORTATION

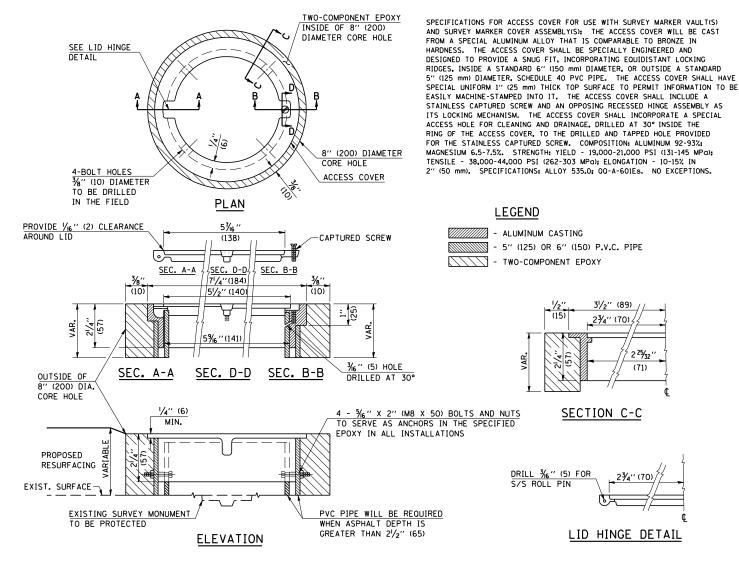
LOCATION & DESIGN OF BLOCKOUTS FOR SIGN POSTS DETAIL

F.A.S. ROUTE 1671 (U.S. ROUTE 45) SECTION 144SBR-2 & 22VBR-1 DOUGLAS COUNTY

SCALE: NOT TO SCALE
DATE: 06/19/06

DRAWN BY: B.B.P. CHECKED BY: C.R.G.

TO BE INSTALLED IN ALL PAVEMENT TYPES FOR PRESERVING PERMANENT SURVEY MARKERS (PI'S, PT'S, PC'S, POC'S, & POT'S) AND LAND SURVEY MONUMENTS (SECTION OR SUBSECTION CORNERS)



GENERAL NOTES

- 1. WORK SHALL NOT START ON THIS ITEM UNTIL THE FINAL LIFT OF SURFACE HAS BEEN COMPLETED.
- 2. THE SURVEY MONUMENT COVER ASSEMBLY SHALL BE CENTERED ABOVE THE SURVEY MONUMENT TO BE PROTECTED.
- 3. MODIFICATION OF THE ALUMINUM CASTING SHALL BE DONE BY GRINDING OR SAWING WHEN HEIGHT REDUCTION IS REQUIRED.
- 4. ALL SURVEY MONUMENT COVER ASSEMBLIES SHALL BE PLACED 1/4" (6 mm) ± BELOW THE FINAL SURFACE.
- 5. ALUMINUM CASTING SHALL BE PLACED OVER A 5" (125 mm) P.V.C. PIPE OR INSIDE OF A 6" (150 mm) P.V.C. PIPE WHEN AN INCREASE IN HEIGHT IS REQUIRED.
- 6. THE CASTING SHALL BE ANCHORED IN THE 8" (200 mm) DIAMETER CORE HOLE WITH TWO-COMPONENT EPOXY CONFORMING TO APPLICABLE PORTIONS OF ARTICLE 1025.01 OF THE STANDARD SPECIFICATIONS.
- 7. THIS WORK WILL BE PAID FOR AT THE CONTRACT UNIT PRICE EACH FOR SURVEY MONUMENT COVER ASSEMBLY WHICH PRICE SHALL INCLUDE ALL LABOR AND MATERIAL AS SPECIFIED INCLUDING CORING THE NEW PAVEMENT SURFACE AND EPOXY. NO ADDITIONAL COMPENSATION WILL BE ALLOWED.
- 8. THE 8" (200 mm) DIAMETER CORE HOLE SHALL BE SUBJECT TO THE APPROVAL OF THE ENGINEER.

BILL OF MATERIAL

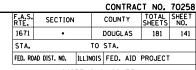
ALUMINUM CASTING OF THE DIMENSIONS AND SPECIFICATIONS SHOWN OR OTHER SUBJECT TO ENGINEER'S APPROVAL OF SHOP DRAWINGS. 4 EACH - 36" X 2" (M8 X 50) BOLTS WITH NUTS, EPOXY. 5" OR 6" (125 mm OR 150 mm) DIAMETER P.V.C. PIPE, SCHEDULE 40 (WHEN REQUIRED).

į		NAME	DATE	REVISIONS	
2	DESIGNED	A.W.H.	2-28-91	NAME	DATE
4	CHECKED	J.H.M.	2-28-91	D.L.P.	10/96
5	CADD NO.	D-I	.04	K.A.G.	08/04

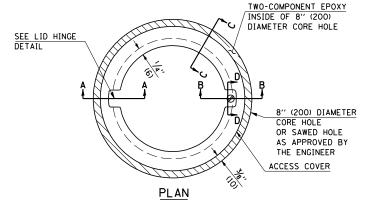
ALL DIMENSIONS ARE IN INCHES (MILLIMETERS) UNLESS OTHERWISE SHOWN.

X0301232 - SURVEY MARKER VAULT

TO BE INSTALLED IN RIGID OR COMPOSITE PAVEMENT FOR PRESERVING LAND SURVEY MONUMENTS (SECTION OR SUBSECTION CORNERS)



• 144SBR-2 & 22VBR-1



SPECIFICATIONS FOR ACCESS COVER FOR USE WITH SURVEY MARKER VAULT(S) AND SURVEY MARKER COVER ASSEMBLY(S): THE ACCESS COVER WILL BE CAST FROM A SPECIAL ALUMINUM ALLOY THAT IS COMPARABLE TO BRONZE IN HARDNESS. THE ACCESS COVER SHALL BE SPECIALLY ENGINEERED AND DESIGNED TO PROVIDE A SNUG FIT, INCORPORATING EQUIDISTANT LOCKING RIDGES. INSIDE A STANDARD 6" (150 mm) DIAMETER, OR OUTSIDE A STANDARD 5" (125 mm) DIAMETER, SCHEDULE 40 PVC PIPE. THE ACCESS COVER SHALL HAVE SPECIAL UNIFORM 1" (25 mm) THICK TOP SURFACE TO PERMIT INFORMATION TO BE EASILY MACHINE-STAMPED INTO IT. THE ACCESS COVER SHALL INCLUDE A STAINLESS CAPTURED SCREW AND AN OPPOSING RECESSED HINGE ASSEMBLY AS ITS LOCKING MECHANISM. THE ACCESS COVER SHALL INCORPORATE A SPECIAL ACCESS HOLE FOR CLEANING AND DRAINAGE, DRILLED AT 30° INSIDE THE RING OF THE ACCESS COVER, TO THE DRILLED AND TAPPED HOLE PROVIDED FOR THE STAINLESS CAPTURED SCREW. COMPOSITION: ALUMINUM 92-93%: MAGNESIUM 6.5-7.5%. STRENGTH: YIELD - 19,000-21,000 PSI (131-145 MPa); TENSILE - 38,000-44,000 PSI (262-303 MPa); ELONGATION - 10-15% IN 2" (50 mm). SPECIFICATIONS: ALLOY 535.0; OO-A-601Es. NO EXCEPTIONS.

LEGEND

T = THICKNESS OF

ALUMINUM CASTING

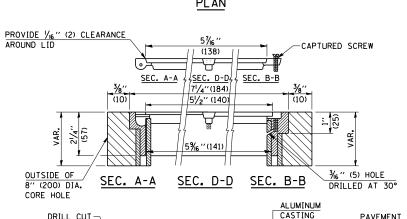
TWO-COMPONENT EPOXY

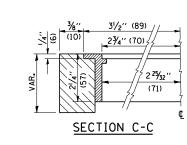
PAVEMENT STRUCTURE

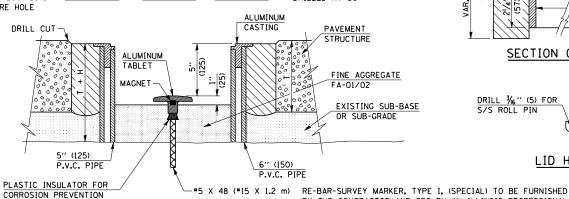
H = THE THICKNESS OF THE

5" (125) OR 6" (150) P.V.C. PIPE

SUB-BASE GRANULAR + 1" (25)







DRILL 3/6" (5) FOR S/S ROLL PIN _23/4" (70) LID HINGE DETAIL

BY THE CONTRACTOR AND SET BY AN ILLINOIS PROFESSIONAL LAND SURVEYOR (SEE SPECIAL DETAIL SHEET FOR ALUMINUM TABLET AND RE-BAR SPECIFICATIONS).

GENERAL NOTES

1. ALUMINUM CASTING SHALL BE EITHER PLACED OVER A 5" (125 mm) P.V.C. PIPE OR INSIDE OF A 6" (150 mm) P.V.C. PIPE.

ELEVATION

- 2. BACKFILL WITH FINE AGGREGATE FA-01/02.
- 3. WORK SHALL NOT START ON THIS ITEM UNTIL THE FINAL LIFT OF SURFACE HAS BEEN COMPLETED.
- 4. THIS WORK WILL BE PAID FOR AT THE CONTRACT UNIT PRICE EACH FOR SURVEY MARKER VAULT WHICH PRICE SHALL INCLUDE ALL LABOR AND MATERIAL AS SPECIFIED INCLUDING CORING. EPOXY AND FA-01/02 AGGREGATE AND NO ADDITIONAL COMPENSATION WILL BE ALLOWED.
- 5. THE CASTING SHALL BE ANCHORED IN THE 8" (200 mm) DIAMETER CORE HOLE WITH TWO-COMPONENT EPOXY CONFORMING TO APPLICABLE PORTIONS OF ARTICLE 1025.01 OF THE STANDARD SPECIFICATIONS.
- 6. ALL SURVEY MARKER (VAULTS) SHALL BE PLACED 1/4" (6 mm) ± BELOW THE FINAL SURFACE.
- 7. THE 8" (200 mm) DIAMETER CORE HOLE SHALL BE SUBJECT TO THE APPROVAL OF THE ENGINEER.

BILL OF MATERIAL

ALUMINUM CASTING OF THE DIMENSIONS AND SPECIFICATIONS SHOWN OR OTHER SUBJECT TO ENGINEER'S APPROVAL OF SHOP DRAWINGS. 5" OR 6" (125 mm OR 150 mm) DIAMETER P.V.C. PIPE, SCHEDULE 40, ALUMINUM TABLET. STAMPED IN ACCORDANCE WITH STANDARD 667101 5%" X 48" (#15 X 1.2 m) RE-BAR, EPOXY AND FA-01/02 AGGREGATE

ILLINOIS DEPARTMENT OF TRANSPORTATION

SURVEY MONUMENT COVER ASSEMBLY & SURVEY MARKER VAULT DETAILS

F.A.S. ROUTE 1671 (U.S. ROUTE 45) SECTION 144SBR-2 & 22VBR-1 DOUGLAS COUNTY

SCALE: NOT TO SCALE DATE: 06/19/06

DRAWN BY: B.B.P.

TO STA. STA. FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT

• 144SBR-2 & 22VBR-1

XZ193400 - SURVEY MARKER, TYPE 2 (SPECIAL)

TO BE INSTALLED IN RIGID OR COMPOSITE PAVEMENT FOR PRESERVING PERMANENT SURVEY MARKERS (PI'S, PT'S, PC'S, POC'S, & POT'S)

ALUMINUM TABLET WITH MAGNET (#15) REINFORCEMENT BAR - 48" (1.2 m) LONG

IN BITUMINOUS

TREATED SURFACE

XZ193300 - SURVEY MARKER, TYPE 1 (SPECIAL)

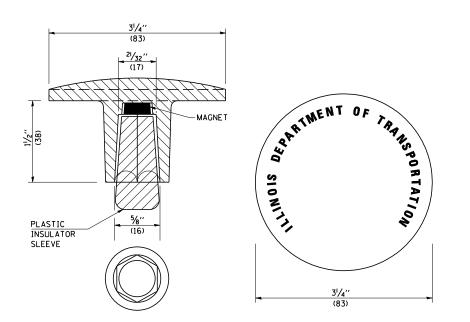
TO BE INSTALLED IN FLEXIBLE PAVEMENT OR SHOULDER, BITUMINOUS TREATED SURFACE AND TURF AREAS WITHIN

THE RIGHT-OF-WAY FOR PRESERVING PERMANENT SURVEY

IN TURF AREAS

WITHIN R.O.W.

MARKERS (PI'S, PT'S, PC'S, POC'S, & POT'S)



THE DIMENSIONS SHOWN SHALL BE EXACT, OTHERS MAY VARY. BUT SHALL BE SHOWN ON SHOP DRAWINGS.

GENERAL NOTES

- 1. THE CONTRACT UNIT PRICE, EACH, FOR SURVEY MARKER, TYPE I, (SPECIAL) SHALL BE PAYMENT IN FULL FOR FURNISHING THE SURVEY MARKER.
- 2. ALL SURVEY MARKERS, TYPE I, (SPECIAL) SHALL BE PLACED ± 1/4" (6 mm) BELOW THE FINAL SURFACE.
- 3. WHEN THE TABLET AND REBAR ARE PLACED AS PART OF A SURVEY MARKER VAULT, THEY SHALL BE CONSIDERED AS INCLUDED IN THAT PAY ITEM AND THERE WILL BE NO PAYMENT FOR THE SURVEY MARKER, TYPE I, (SPECIAL).

ų		NAME	DATE	REVISIONS		
NAME	DESIGNED	AWH	8/17/91	NAME	DATE	Ш
	CHECKED	PEK	8/17/91	D.L.P.	10-96	Ш
USER	CADD NO. D-1.		.05	K.A.G.	08-04	L

ALL DIMENSIONS ARE IN INCHES (MILLIMETERS) UNLESS OTHERWISE SHOWN.

SPECIFICATIONS FOR ALUMINUM TABLET

SURVEY CAP FOR REBAR. 31/4" (83 mm) CONVEX SURVEY CAP FOR 5/8" (15 mm) REBAR WITH ILLINOIS DEPARTMENT OF TRANSPORTATION LOGO. THIS LOGO SHALL PROVIDE LETTERS RECESSED INTO THE SURFACE A MINIMUM OF 1/32" (O.8 mm) FOR EASY AND LONG-TERM LEGIBILITY. THE ALUMINUM CAP FOR REBAR SHALL BE PRODUCED BY THE PROCESS OF ORBITAL FORGING TO PRODUCE A HIGH-STRENGTH AND DURABLE MARKER CAP WHICH WILL NOT CHIP OR BREAK AND PROVIDE A SMOOTH FINISH FOR STAMPING OF DATA IN THE FIELD. THE ALUMINUM CAP FOR REBAR SHALL BE TAPERED FOR A PERFECT COMPRESSION FIT. A SPECIAL PLASTIC INSULATOR SHALL BE INSTALLED TO PREVENT DISSIMILAR METAL CONTACT AND CORROSION. THE PLASTIC INSULATOR SHALL FORM READILY TO THE OUTER SHAPE OF THE REBAR AND TO THE INNER SHAPE OF THE ALUMINUM CAP SOCKET. THE PLASTIC INSULATOR SHALL BE LOW DENSITY POLYETHYLENE. A MINIMUM 11/2" (38 mm) LONG AND CONFORM TO FEDERAL SPECIFICATION L-P 390.

COMPOSITION: ALUMINUM 98.3-98.7%; OTHER 1.3-1.7%; STRENGTH: YIELD 28 KSI (193 MPa), ULTIMATE 32 KSI (221 MPa). ELONGATION 15% [IN 2" (50 mm)]. SPECIFICATIONS: ALUMINUM ALLOY 6101-0; ASTM B317-83 (EXCEPT TEMPER) AS FORGED.

SPECIFICATIONS FOR REBAR

REBAR FOR ALUMINUM TABLET. REINFORCEMENT BAR SHALL BE 5/8" (#15) X 48" (1.2 m) (DEFORMED).

INSPECTION OF REINFORCEMENT BAR 5%" (*15) SHALL BE DONE BY DISTRICT PERSONNEL OF THE ILLINOIS DEPARTMENT OF TRANSPORTATION, DIVISION OF HIGHWAYS.

DESIGN NOTE

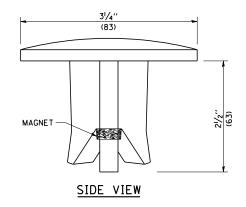
BDE 58-8.02 "PLACE MARKERS AT THE PT'S AND PC'S OF ALL HORIZONTAL CURVES AND SPACE THEM ALONG TANGENTS SO THAT TWO MARKERS ARE ALWAYS INTERVISIBLE.

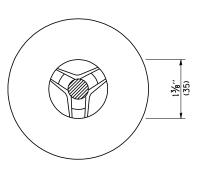
(100) P.C. CONCRETE OR COMPOSITE PAVEMENT AGGREGATE BASE OR BITUMINOUS BASE

SPECIFICATIONS FOR ALUMINUM TABLET (FORKED)

ALUMINUM TABLET (FORKED) FOR USE WITH "SURVEY MARKER, TYPE 2, (SPECIAL)" SHALL BE AS SHOWN ON THE DETAIL FOR THE 31/4" (83 mm) CONVEX SURVEY TABLET WITH ILLINOIS DEPARTMENT OF TRANSPORTATION LOGO. THIS LOGO SHALL PROVIDE FOR LETTERS RECESSED INTO THE SURFACE A MINIMUM OF 1/2" (O.8 mm) FOR EASY AND LONG-TERM LEGIBILITY. THE ALUMINUM TABLET SHALL BE PRODUCED BY THE PROCESS OF ORBITAL FORGING TO PRODUCE A HIGH-STRENGTH AND DURABLE MARKER CAP WHICH WILL NOT CHIP OR BREAK AND PROVIDE A SMOOTH FINISH FOR STAMPING OF DATA IN THE FIELD. THE ALUMINUM TABLET SHALL BE DESIGNED NOT TO TURN OR ROTATE. THREE PRONGS ON A 21/2" (63 mm) STEM SHALL BE SUCH THAT THE ALUMINUM TABLET CANNOT BE EASILY REMOVED.

COMPOSITION: ALUMINUM 92-93%; MAGNESIUM 6.5-7.5%. STRENGTH: YIELD 19,000-21,000 PSI (131-145 MPa); TENSILE 38,000-44,000 PSI (262-303 MPa); ELONGATION 10-15% [IN 2" (50 mm)]. SPECIFICATIONS: ALLOY 535.0; OO-A-601ES. NO EXCEPTIONS.





BOTTOM VIEW

THE DIMENSIONS SHOWN SHALL BE EXACT, OTHERS MAY VARY, BUT SHALL BE SHOWN ON SHOP DRAWINGS.

GENERAL NOTES

- 1. WORK ON THIS ITEM SHALL NOT START UNTIL THE FINAL SURFACE IS COMPLETED.
- 2. THE ALUMINUM TABLET (FORKED) SHALL REST UPON THE BOTTOM OF THE 4" (100 mm) CORE HOLE. IF THE HOLE IS TOO DEEP, EPOXY GROUT MUST BE USED TO DECREASE THE DEPTH AND ALLOWED TO HARDEN BEFORE PROCEEDING.
- 3. THE ALUMINUM TABLET SHALL BE ANCHORED IN THE 4" (100 mm) DIAMETER HOLE IN THE NEW PAVEMENT WITH TWO-COMPONENT EPOXY CONFORMING TO APPLICABLE PORTIONS OF ARTICLE 1025.01 OF THE STANDARD SPECIFICATIONS.
- 4. THE 4" (100 mm) CORE HOLE SHALL BE SUBJECT TO THE APPROVAL OF THE ENGINEER.
- 5. THE CONTRACT PRICE, EACH, FOR SURVEY MARKER, TYPE 2 (SPECIAL) SHALL BE PAYMENT IN FULL FOR FURNISHING THE ALUMINUM TABLET AND ALL LABOR AND MATERIAL TO SET THE MARKER IN PLACE, AS SPECIFIED, INCLUDING CORING THE NEW PAVEMENT.
- 6. ALL SURVEY MARKERS, TYPE 2 (SPECIAL) SHALL BE PLACED ± 1/4" (6 mm) BELOW THE FINAL SURFACE.

ILLINOIS DEPARTMENT OF TRANSPORTATION SURVEY MARKER TYPE 1 (SPECIAL) SURVEY MARKER TYPE 2 (SPECIAL)

DETAILS F.A.S. ROUTE 1671 (U.S. ROUTE 45) SECTION 144SBR-2 & 22VBR-1 DOUGLAS COUNTY
SCALE: NOT TO SCALE DR

DATE: 06/19/06

DRAWN BY: B.B.P. CHECKED BY: C.R.G.

IN FLEXIBLE PAVEMENT

OR SHOULDER

