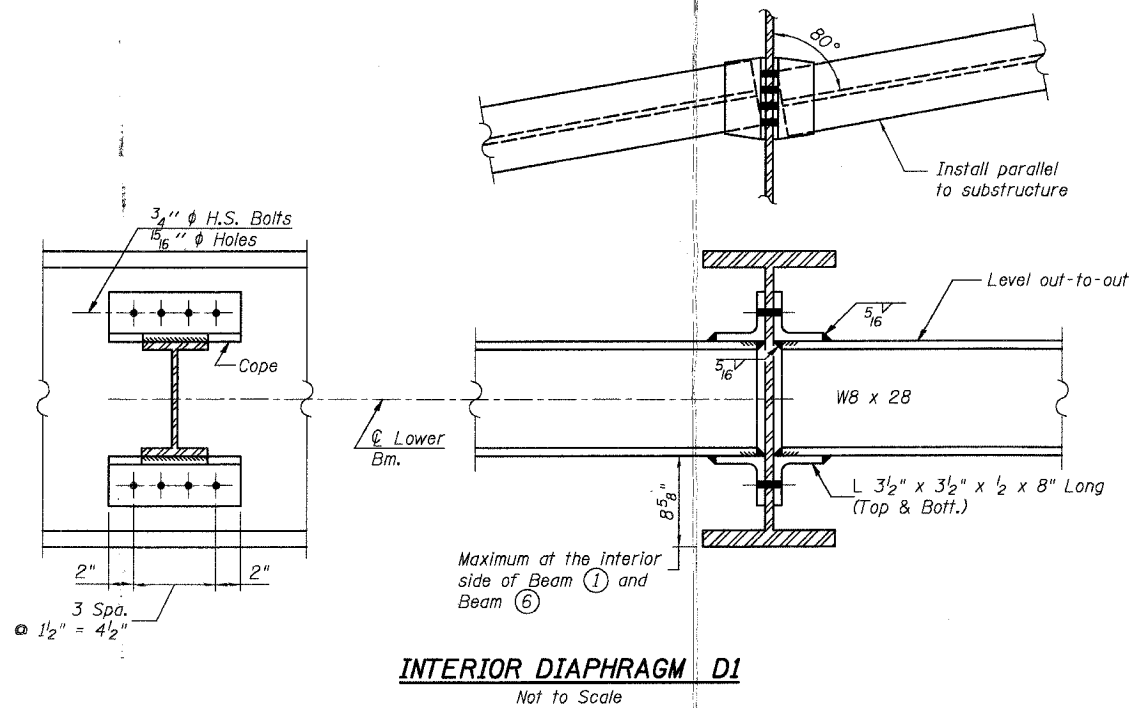


FRAMING PLAN
Not to Scale



MOMENT AND REACTION TABLES

INTERIOR GIRDER MOMENT TABLE		0.5 Span
I_s	(in ⁴)	3630
I_c (n)	(in ⁴)	10,635.1
I_c (3n)	(in ⁴)	7,732.86
S_s	(in ³)	329
S_c (n)	(in ³)	478.38
S_c (3n)	(in ³)	423.98
DL	(k/ft.)	0.87
M_{DL}	(k)	258.3
f_s DL(non-comp)	(k.s.i.)	9.42
S_{DL}	(k/ft.)	0.50
M_{SD}	(k)	147.4
f_s SD(comp)	(k.s.i.)	4.17
M_{LL}	(k)	416.1
M_I (Impact)	(k)	119.8
f_s [M _{LL} + M _{Impact}]	(k.s.i.)	13.44
f_s (Total)	(k.s.i.)	27.04
VR	(k)	59.19

INTERIOR GIRDER REACTION TABLE		Abut.
R_{DL}	(k)	33.3
R_{LL}	(k)	46.0
Imp.	(k)	13.3
R (Total)	(k)	92.6

I_s and S_s are the moment of inertia and section modulus of the steel section used in computing f_s (Total).

$I_c(n)$ and $S_c(n)$ are the moment of inertia and section modulus of the composite section used in computing stresses due to Live Load.

$I_c(3n)$ and $S_c(3n)$ are the moment of inertia and section modulus of the composite section used in computing stresses due to superimposed dead loads. (see AASHTO 10.38)

VR is the maximum Live Load + Impact shear range in span.

Live Load and Impact designed for 120,000 lb. permit loading (one lane) as well as HS20-44.

TOP OF BEAM ELEV. TABLES

GIRDER #	N. ABUT.	S. ABUT.
1	688.59	688.60
2	688.74	688.75
3	688.85	688.86
4	688.85	688.86
5	688.74	688.75
6	688.59	688.60

For Fabrication use only

NOTES:

1. Work this sheet with sheet S-07

DESIGNED	MGH
CHECKED	RGD
DRAWN	WJH
CHECKED	NRF

SMITH ENGINEERING CONSULTANTS, INC. CIVIL, STRUCTURAL, MECHANICAL AND SURVEYING www.smithengineering.com 2840 Woodlawn Drive, Chicago, IL 60616	
REVISIONS	
NAME	DATE

ILLINOIS DEPARTMENT OF TRANSPORTATION	
Steel Framing Plan Harlem Avenue/Drecksler Road Over Black Walnut Creek Will County Section 01-00139-02-BR SN. 099-3091	
DATE 5-26-2005	