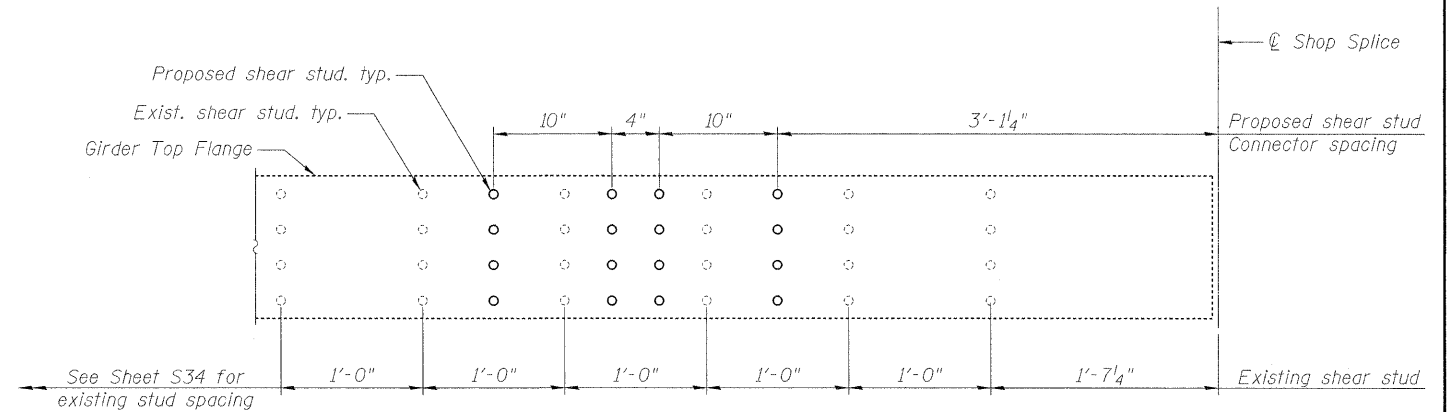


INTERIOR GIRDER MOMENT TABLE		
	0.4 Sp. 1 or 0.6 Sp. 2	Pier
I_s	(in ⁴) 12911	26715
$I_c(n)$	(in ⁴) 36703	—
$I_c(3n)$	(in ⁴) 26902	—
S_s	(in ³) 787	1214
$S_c(n)$	(in ³) 1053	—
$S_c(3n)$	(in ³) 980	—
Z	(in ³) —	—
ϕ	(k/ft) 0.866	1.565
$M\phi$	(k) 398	1495
$s\phi$	(k/ft) 0.605	—
$M_s\phi$	(k) 294	—
M_L	(k) 636	585
M_{IM}	(k) 151	139
$^{5/8} [M_L + i]$	(k) 1314	1209
M_a	(k) 2606	3516
M_u	(k) 3500	—
$f_s \phi$ non-comp	(ksi) 6.1	14.8
$f_s \phi$ (comp)	(ksi) 3.6	—
$f_s^{5/8} [M_L + M_I]$	(ksi) 15.0	12.0
f_s (Overload)	(ksi) 24.6	26.7
f_s (Total)	(ksi) —	34.7
VR	(k) 45.0	—

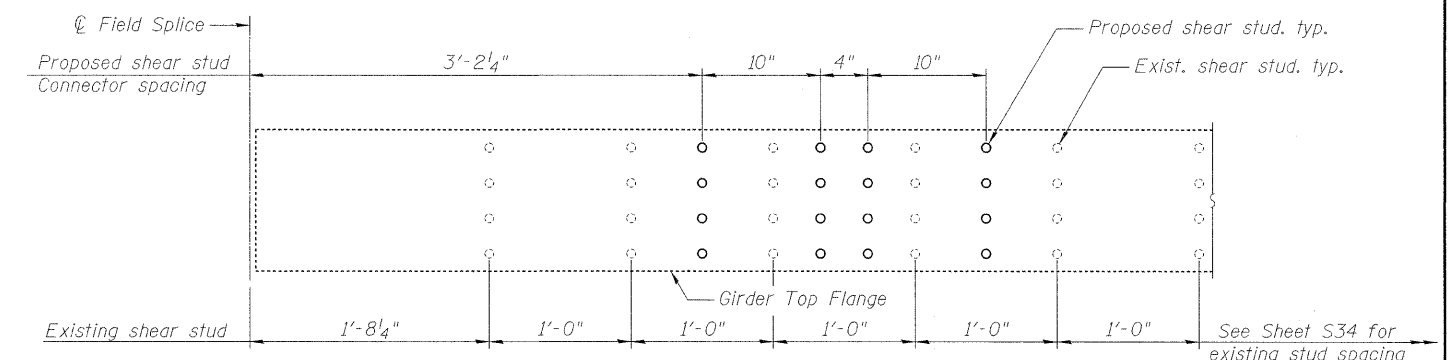
INTERIOR GIRDER REACTION TABLE		
	Abut.	Pier
$R\phi$	(k) 46.2	162.6
R_L	(k) 46.9	60.1
R_I	(k) 8.0	10.2
R_{Total}	(k) 101.1	232.9

* Compact section
 ** Braced non-compact and partially braced section

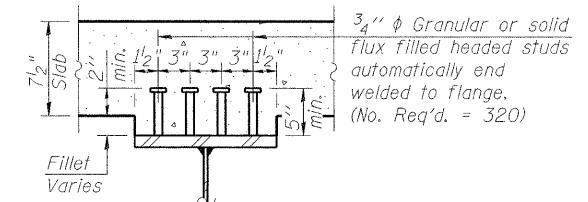
I_s, S_s : Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total and Overload) due to non-composite dead loads (in.⁴ and in.³).
 $I_c(n), S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total and Overload) due to short-term composite live loads (in.⁴ and in.³).
 $I_c(3n), S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total and Overload) due to long-term composite (superimposed) dead loads (in.⁴ and in.³).
 Z : Plastic Section Modulus of the steel section in non-composite areas (in.³).
 ϕ : Un-factored non-composite dead load (kips/ft.).
 $M\phi$: Un-factored moment due to non-composite dead load (kip-ft.).
 $s\phi$: Un-factored long-term composite (superimposed) dead load (kips/ft.).
 $M_s\phi$: Un-factored moment due to long-term composite (superimposed) dead load (kip-ft.).
 M_L : Un-factored live load moment (kip-ft.).
 M_I : Un-factored moment due to impact (kip-ft.).
 M_a : Factored design moment (kip-ft.).
 $1.3 [M\phi + M_s\phi + \frac{5}{8} (M_L + M_I)]$
 M_u : Compact composite moment capacity according to AASHTO LFD 10.50.1.1 or compact non-composite moment capacity according to AASHTO LFD 10.48.1 (kip-ft.).
 f_s (Overload): Sum of stresses as computed from the moments below (ksi).
 $M\phi + M_s\phi + \frac{5}{8} (M_L + M_I)$
 f_s (Total): Sum of stresses as computed from the moments below on non-compact section (ksi).
 $1.3 [M\phi + M_s\phi + \frac{5}{8} (M_L + M_I)]$
 VR: Maximum $\frac{1}{4}$ + impact horizontal shear range within the composite portion of the span for stud shear connector design (kips).



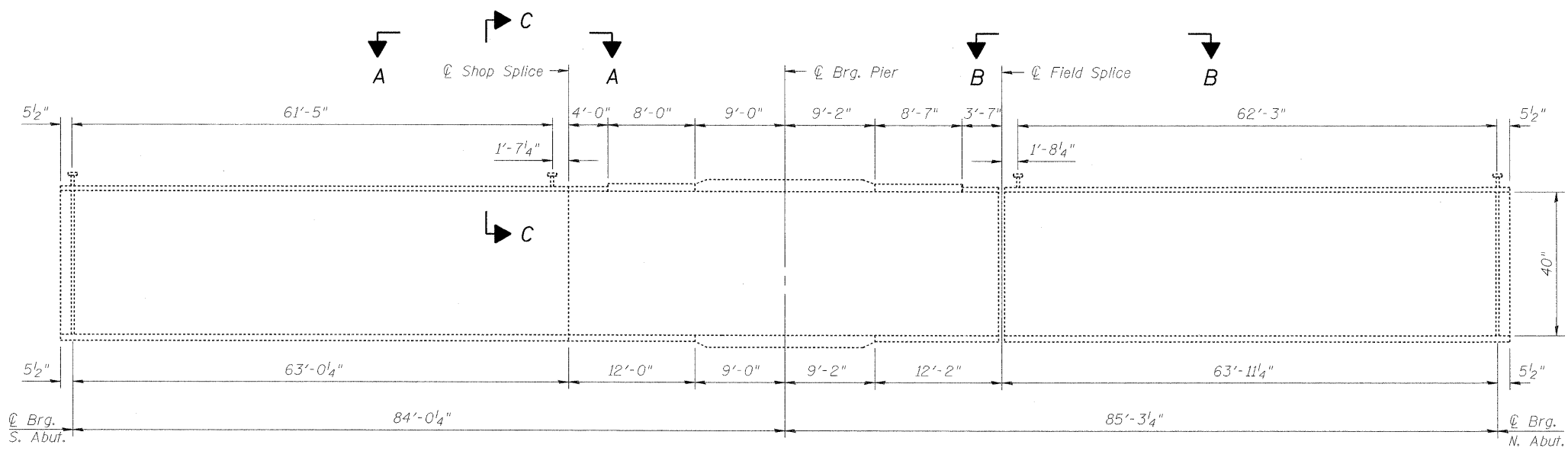
VIEW A-A



VIEW B-B



SECTION C-C



GIRDER ELEVATION

Note: Intermediate stiffeners not shown.

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