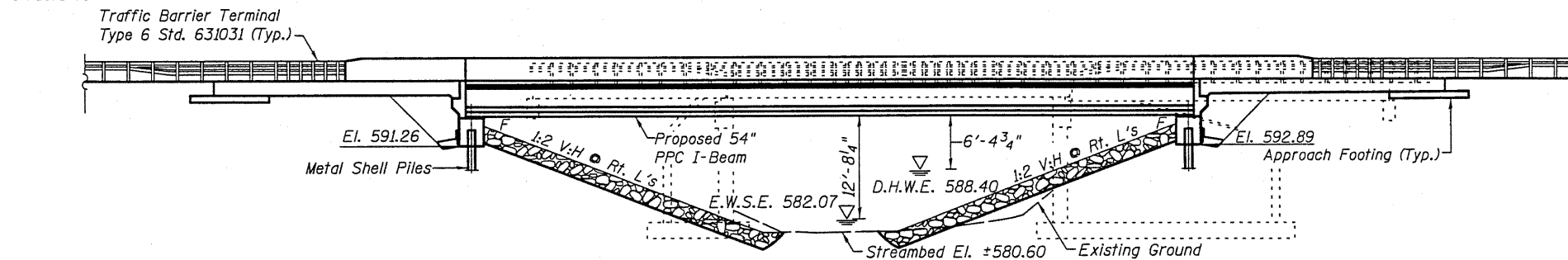


Benchmark: Benchmark disk set in concrete post in Northeast quad of intersection of N. Hoffman Avenue and State Highway 9 ±57' north of \varnothing IL-9 and ±106' east of \varnothing N. Hoffman Avenue. Elevation 591.41

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

Existing Structure: S.N. 090-0064 built in 1928 as SBI Route 164, Section 119-B. The superstructure was replaced in 1975 with (11)- 21" x 36" PPC Deck Beams and a bituminous overlay. The substructure consists of RC closed abutments on untreated timber piles. Structure measures 44'-9⁵/₈" bk to bk abutments and 33'-0" out-to-out of deck. Existing structure is to be removed. Traffic to be maintained using stage construction. Wide load detour to be provided.

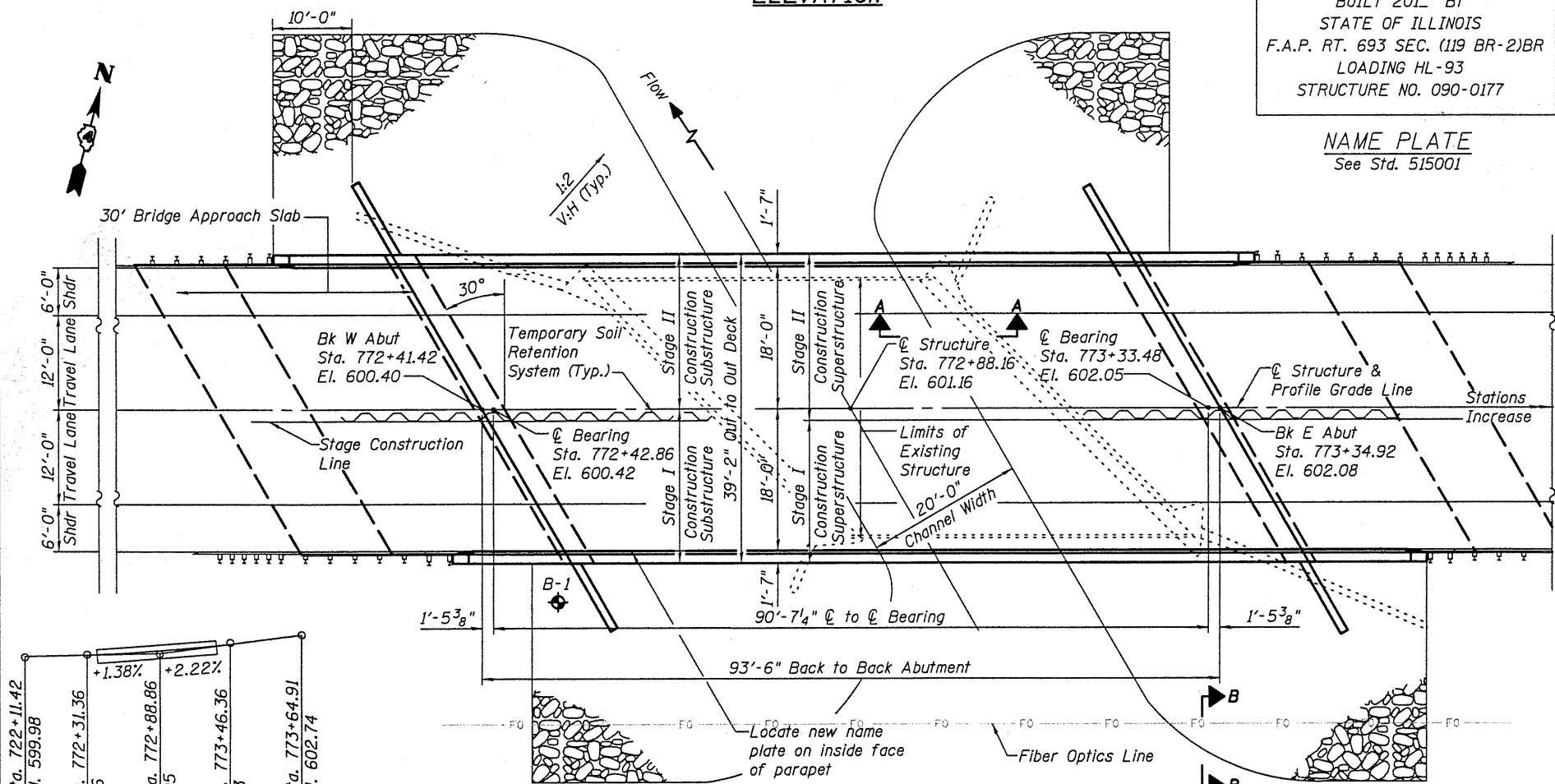
Salvage all steel used for bracing and support. All salvaged steel should be shipped to East Peoria Maintenance Yard at 604 Camp St., East Peoria, IL 61611. Contact Brain Ruder at (309) 699-3822. Cost incidental to Removal of Existing Structures.



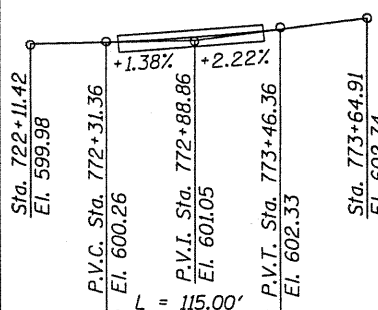
ELEVATION

STATION 772+88.16
BUILT 20L BY
STATE OF ILLINOIS
F.A.P. RT. 693 SEC. (119 BR-2)BR
LOADING HL-93
STRUCTURE NO. 090-0177

NAME PLATE
See Std. 515001



PLAN



PROFILE GRADE

(F.A.P. 693 along \varnothing of roadway)

DESIGNED - RRD
CHECKED - AJS
DRAWN - KAS
CHECKED - AJS

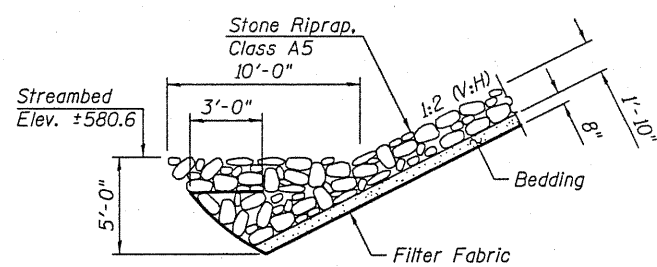
WATERWAY INFORMATION

Drainage Area = 3.3 square miles		Low Grade Elev. 600.24 @ Sta. 772+29							
Flood Yr.	Freq. C.F.S.	Opening Sq. Ft.	Nat. H.W.E.	Head - Ft.	Headwater El.				
		Exist.	Prop.	Exist.	Prop.				
Design	10	1060	162	199	586.70	0.65	0.44	587.35	587.14
Base	50	1810	211	280	588.40	1.16	0.61	589.56	589.01
Overtopping	100	2160	229	314	589.05	1.42	0.69	590.47	589.74
Max. Calc.	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
	500	3030	268	391	590.40	2.15	0.91	592.55	591.31

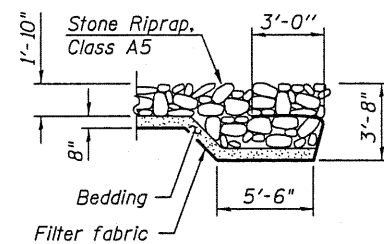
10-year velocity through existing bridge = 6.77 fps 10 year velocity through proposed bridge = 5.55 fps
Scour counter measures to be provided. No scour is anticipated.

DESIGN SCOUR ELEVATION TABLE

Design Scour Elevation (ft.)	West Abut.	East Abut.
	591.26	592.89



SECTION A-A



SECTION B-B

SCOPE OF WORK

Complete removal of Existing Superstructure, and Approach Slabs. Removal of Existing Abutments and Wingwalls. Construct Reinforced Concrete Bridge Deck, P.P.C. I-Beams, Approach Slab, and Abutments.

GENERAL NOTES

Reinforcement bars shall conform to the requirements of ASTM A 706 Gr 60. See Special Provisions. Reinforcement bars designated (E) shall be epoxy coated. Layout of the slope protection system may be varied to suit ground conditions in the field as directed by the Engineer. The embankment configuration shown shall be the minimum that must be placed and compacted prior to construction of the abutments. The Contractor shall drive test piles to 110% of the nominal required bearing specified in production locations at substructures specified or approved by the Engineer before ordering the remainder of piles. Excavation behind existing abutment walls shall be performed to balance front and back soil pressure before removing the existing superstructure. The Contractor shall sawcut the upper portion of the existing abutment at the stage removal line before Stage I removal to ensure the remaining portion will not be prematurely damaged. The Contractor is advised that the existing PPC Deck Beams are in a deteriorated condition with reduced load carrying capacity. It is the Contractor's responsibility to account for the condition of the beams when developing construction procedures for removal and replacement of the superstructure.

APPROVED
For Structural Adequacy Only

Ralph E. Anderson (SE)
Engineer of Bridges & Structures

ILLINOIS STRUCTURAL NO. 081-005819 (Expires 11/30/10)

INDEX OF SHEETS

- 1 General Plan and Elevation
- 2 Bill of Material and Suggested Stage Construction
- 3 Temporary Concrete Barrier Details
- 4-5 Top of Slab Elevations
- 6 Top of Approach Slab Elevations
- 7 Superstructure
- 8 Superstructure Details
- 9 Diaphragm Details
- 9a Concrete Parapet Slipforming Option
- 10 Framing Plan
- 11-12 Beam Details
- 13-14 Bridge Approach Slab
- 15-16 Abutment Details
- 17 Metal Shell Pile Details
- 18 Suggested Stage Construction Bracing Details
- 19 Bar Splicer and Assembly Details
- 20-21 Soil Boring Logs
- 22 Existing Structure Plan/Original Construction Drawing

DESIGN SPECIFICATIONS

2007 AASHTO LRFD Bridge Design Specifications with 2008 Interims

LOADING HL-93

Allow 50#/sq. ft. for future wearing surface.

SEISMIC DATA

Seismic Performance Zone (SPZ) = 1
Design Spectral Acceleration at 1.0 sec. (S_{D1}) = 0.119
Design Spectral Acceleration at 0.2 sec. (S_{D5}) = 0.191
Soil Site Class = D

DESIGN STRESSES

FIELD UNITS

f'_c = 3,500 psi
f_y = 60,000 psi (Reinforcement)

PRECAST PRESTRESSED UNITS

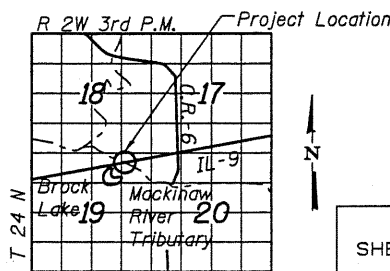
f'_c = 6,000 psi
f'_{ci} = 5,000 psi
f_{pu} = 270,000 psi (1/2" low lax. strands)
f_{pbt} = 201,960 psi (1/2" low lax. strands)



Anthony J. Standish
ILLINOIS STRUCTURAL NO. 081-005819 (Expires 11/30/10)

GENERAL PLAN AND ELEVATION
IL-9 OVER
MACKINAW RIVER TRIBUTARY
F.A.P. 693 - SECTION (119 BR-2)BR
TAZEWELL COUNTY
STA. 772+88.16
S.N. 090-0177

STRAND ASSOCIATES, INC.



LOCATION SKETCH

SHEET NO. 1	F.A.P. RTE. 693	SECTION (119 BR-2)BR	COUNTY TAZEWELL	TOTAL SHEETS 65	SHEET NO. 18
22 SHEETS	CONTRACT NO. 68660		FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT		