GEOTECHNICAL LETTER REPORT RETAINING WALL ALONG 55th St. at Sergo Dr. Section No. 0102N&T CONTRACT NO. 62C25 COOK COUNTY, ILLINOIS

> for Accurate Group, Inc. 101 Schelter Road, Suite B200 Lincolnshire, IL 60069

Submitted by Wang Engineering, Inc. 1145 North Main Street Lombard, IL 60148

> Original Report: April 4, 2018 Revised Report: NA



April 4, 2018

Sapan Trivedi, EIT Accurate Group, Inc. 101 Schelter Road, Suite B200 Lincolnshire, IL 60069

Re: Foundation Recommendations
Retaining Wall along 55th St. at Sergo Dr.
Section No. 0102N&T,
Cook County, Illinois
Wang No. 491-03-01

Dear Mr. Trivedi,

Wang Engineering Inc. (Wang) is pleased to present this letter report presenting the results of our geotechnical investigations, evaluations, and recommendations for a proposed retaining wall along 55th Street in Cook County. A *Site Location Map* is presented in Exhibit 1. Based on information provided by Accurate Group, Inc. (Accurate), we understand that the improvements include a Segmental Block Wall located along westbound 55th Street. The 152-foot long wall begins at Station 319+42.03, offset 56.04 feet left and ends at Station 320+94.00, offset 56.36 feet left with an approximate maximum exposed height of 2.5 feet.

On the USGS *Barrington Quadrangle 7.5 Minute Series* map, the project site is located in the SW ¹/₄ of Sections 10, Tier 38 N, Range 12 E. A *Site Location Map* is presented in Exhibit 1.

The purpose of our investigation was to characterize the subsurface conditions and provide recommendations for the design and construction of the proposed wall.

Subsurface Investigation

Three borings, designated as RWB-23, RWB-24, and RWB-25, were drilled by Wang in October, 2017 for depths of 18.7 to 20.0 feet below ground surface (bgs). Boring coordinates were surveyed by



Wang using a GPS unit. Boring stations and offsets were taken from design drawings provided by Accurate. Station, offset and elevation information is shown in the *Boring Logs* (Appendix A). As-drilled boring locations are shown in the *Boring Location Plan* (Exhibit 2).

A truck-mounted drilling rig, equipped with hollow stem augers, was used to advance and maintain an open borehole. Soil sampling was performed according to AASHTO T 206, *"Penetration Test and Split Barrel Sampling of Soils."* The soil was sampled at 2.5-foot intervals to the boring termination depths. Soil samples collected from each interval were placed in sealed jars for further examination and laboratory testing.

Field boring logs, prepared and maintained by a Wang geologist, included lithologic descriptions, visual-manual soil classifications, results of Rimac and pocket penetrometer unconfined compressive strength tests, and results of standard penetration tests (SPT) recorded as blows per 6 inches of penetration.

Groundwater levels were observed while drilling and at completion of each boring. The boreholes were backfilled with soil cuttings and/or bentonite chips and the surface was restored as close as possible to its original condition.

Laboratory Testing

Soil samples were tested in the laboratory for moisture content (AASHTO T 265). The soils were classified according to the IDH soil classification system. Field visual-manual classifications were also verified in the laboratory. The results of the lab testing are shown in Boring logs (Appendix A).

Subsurface Conditions

Detailed descriptions of encountered lithologic units are presented in the attached *Boring Logs* (Appendix A) and in the *Soil Profile* (Exhibit 3). Please note the lithologic boundaries shown on logs and profiles represent approximate limits between soil types. In the field, the actual transition between soil types might be gradual in horizontal and vertical directions.

The pavement consists of 6 to 15-inch thick asphalt. An 11-inch thick concrete base was encountered beneath the asphalt pavement in Boring RWB-23. Beneath the pavement section, the borings encountered cohesive and granular fills consisting of stiff to very stiff, black, brown and gray silty clay loam, clay loam to medium dense gravelly sand. The cohesive fill has unconfined compressive strength



 (Q_u) values of 1.0 to 2.8 tsf, with an average of 1.9 tsf and moisture content values of 21 to 28%, with an average of 25%. Underlying the fill, borings encountered buried topsoil in Borings RWB-23 and RWB-24, consisting of stiff, black and brown silty clay. At an elevation of 643.2 to 646.1 feet, the borings advanced through 11 to 13 feet of stiff to hard silty clay to clay with sand lenses. The soil has Q_u values of 1.8 to 6.2 tsf with an average of 3.7 tsf and moisture content values of 16 to 26% with an average of 21%. Borings RWB-23 and RWB-24 encountered weathered bedrock at a depth of 18 feet bgs.

Groundwater was not encountered during or at the completion of drilling.

Foundation Design Recommendations

Wang understands a segmental block wall with a maximum retained height of 2.5 feet is proposed. We assume the wall will retain primarily new granular fill. Below the footing level, at approximately elevation 646 feet, the wall will be supported by stiff to hard silty clay. Based on the soil conditions encountered along the wall, Wang estimates the soil conditions are appropriate for the support of the wall. The wall should be designed based on a maximum factored bearing resistance (q_b) of 3,500 psf, considering a geotechnical resistance factor (ϕ_b) of 0.55 (AASHTO 2017). The maximum long-term settlement of the wall is estimated less than 0.5 inch.

Global stability evaluations for the wall were performed using *Slide V6*. IDOT requires a minimum Factor of Safety (FOS) of 1.5 (IDOT 2015). The analyses were performed for both short-term (undrained) and long-term (drained) conditions based on the soils encountered in Boring RWB-24 at Station 320+00. Analysis results are presented in Exhibits 4-1 and 4-2. We determined the proposed wall has a FOS of 7.5 (undrained) and 2.8 (drained) which meet the IDOT minimum FOS of 1.5.

Construction Consideration

Site Preparation and Excavation

All vegetation, surface topsoil, and debris should be cleared and stripped where fills and structures will be placed. Any unstable or unsuitable materials should be removed and replaced with compacted structural fill as described in section *filling and backfilling*.

Foundation excavations should be performed in accordance with local, state, and federal regulations. The potential effect of ground movements upon nearby roadways and utilities should also be taken into consideration. Excavations must be sloped at no steeper than 1:1.5 (V: H). Excavated material should



not be stockpiled immediately adjacent to the top of slopes, nor should equipment be allowed to operate too closely to open excavations.

Dewatering

Groundwater was not observed during and at the end of drilling. Contractor should be prepared for dewatering measures if groundwater is encountered above the proposed excavation depth. Depending upon prevailing climate conditions and the time of the year when wall construction takes place, control runoff and maintenance of existing flows may require temporary water diversion and control.

Filling and Backfilling

Backfill materials must be preapproved by the site engineer. The fill material should be free of organic matter and debris and should be placed in lifts and compacted in accordance with the IDOT Standard Specifications for Road and Bridge Construction (2016). To backfill the walls, we recommend porous granular material conforming to the requirements specified in the IDOT Recurring Special Provision, Granular Backfill for Structures (2017).

Wall Construction

The walls should be constructed according to the current IDOT Standard Specifications for Road and Bridge Construction.

Construction Monitoring

There is no need for a special construction monitoring for the retaining walls except normally required by the IDOT Standard Specifications.

Qualifications

The analysis and recommendations contained in this letter report are based on the soils encountered at the boring locations shown in Exhibit 2. This letter report does not reflect any variations that may occur between the borings or elsewhere on the site, variations whose nature and extent may not become evident until a later stage of construction. Should conditions encountered during excavation and construction operations differ from those encountered in the borings, Wang should be notified so that recommendations can be reviewed and revised if necessary.



It has been a pleasure to assist Accurate Group, Inc. in this project. Please contact us if there are any questions, or if we can be of further service.

Respectfully Submitted,

WANG ENGINEERING, INC.

ant Ramesh KC, EIT

Geotechnical Engineer

Corina T. Farez, P.E., P.G.

QC/QA Reviewer

Attachments Exhibit 1: Site Location Map Exhibit 2: Boring Locations Plan Exhibit 3: Soil Profile Exhibit 4: Global Stability Analyses

Andri Kurnia, P.E. Project Manager

Appendix A: *Boring Logs* Appendix B: *GPE Drawing* Appendix C: *Cross-Section Drawing*



REFERENCES

- AMERICAN ASSOCIATION OF STATE HIGHWAY TRANSPORTATION OFFICIALS (2017) *LRFD Bridge Design Specifications*. United States Department of Transportation, Washington, D.C.
- ILLINOIS DEPARTMENT OF TRANSPORTATION (2015) *Geotechnical Manual*. IDOT Bureau of Materials and Physical Research, Springfield, IL.
- ILLINOIS DEPARTMENT OF TRANSPORTATION (2016) Standard Specifications for Road and Bridge Construction. IDOT Division of Highways, Springfield, IL.
- ILLINOIS DEPARTMENT OF TRANSPORTATION (2017) Supplemental Specifications and Recurring Special Provisions. DOT Bureau of Bridges and Structures, Springfield, IL.



EXHIBITS

Exhibit 1: Site Location Map Exhibit 2: Boring Locations Plan Exhibit 3: Soil Profile Exhibit 4: Global Stability Analyses





| Retaining Wall Borings | | | SCALE: GRAPHICAL | EXHIBIT 2 | RAWN BY: R. KC HECKED BY: A. Kurnia |
|------------------------|-----|----------|------------------|----------------------------|---|
| 0 | 150 | 300 Feet | | Wang Engineering | 1145 N. Main Stree Lombard, IL 6014 www.wangeng.cor |
| | | | FOR ACCURA | ATE GROUP. INC. | 491-03-01 |



EI 11X17 4910301.GPJ WANGENG.GDT 3/26/18







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APPENDIX A: BORING LOGS



VANGENGINC 4910301.GPJ WANGENG.GDT



WANGENGINC 4910301.GPJ WANGENG.GDT





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APPENDIX B: GPE DRAWING

Benchmark

Existing Structure: None

Traffic on 55th St. and Sego Dr. will be maintained utilizing staged construction.



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ILLINOIS FED. AID PROJECT



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APPENDIX C: X-SECTION DRAWING

