

Original Report Date: 0	04-14-2014	Proposed SN:	046-2017	Route:	FAP 796 (IL 115)
Revised Date: 09-22-20	014	Existing SN:	046-0067	Section:	104 BR
Geotechnical Engineer:	Michael Short,	County:	Kankakee		
Structural Engineer: Mil	ke Okrent - Bloo	m Companies		Contract:	66B74

Indicate the proposed structure type, substructure types, and foundation locations (attach plan and elevation drawing): The proposed structure is a 10' by 7' double precast concrete box culvert. The end treatment for the box culvert will be precast concrete end sections.

Discuss the existing boring data, existing plans foundation information, new subsurface exploration and need for any additional exploration to be provided with SGR Technical Memo (attach all data and subsurface profile plot): *Two soil borings have been completed and are sufficient to design the proposed improvements.* 

Provide the location and maximum height of any new soil fill or magnitude of footing bearing pressure. Estimate the amount and time of the expected settlement. Indicate if further testing, analysis, and/or ground improvement/treatment is necessary: *The roadway profile will not change. There will be approximately 2 feet of soil fill over the top of the box culvert, however, due to the high strengths of the underlying soils, settlement is not expected. The existing structure and roadway do not have any signs of settlement. No further testing or analysis for settlement is necessary.* Identify any new cuts or fill slope angles and heights. Estimate the factor of safety against slope failure. Indicate if further testing, analysis or ground improvement/treatment is necessary. *The foreslopes will be widened and flattened throughout the project and will generally be less than 8 feet tall. The existing slopes do not have any signs of slope failure. No further testing slopes do not have any signs of slope failure. No further testing slopes do not have any signs of slope failure. No further testing slopes do not have any signs of slope failure. No further testing slopes do not have any signs of slope failure. No further testing slopes do not have any signs of slope failure. No further testing or analysis for slope stability is necessary.* 

Indicate at each substructure, the 100-year and 500-year total scour depths in the Hydraulics report, the non-granular scour depth reduction, the proposed ground surface, and the recommended foundation design scour elevations. *Assuming culvert invert elevations of 659.20 (upstream) and 659.05 (downstream), the design scour elevations are 656.20 (upstream) and 656.05 (downstream) if 3 ft. deep toe walls are used.* 

Determining the seismic soil site class, the seismic performance zone, the 0.2 and 1.0 second design spectral accelerations and indicate if that the soils are liquefiable. *Not applicable to box culverts* 

Confirm feasibility of the proposed foundation or wall type and provide design parameters. Attach a pile design table indicating feasible pile types, various nominal required bearings, factored resistances available and corresponding estimated lengths at locations where piles will be used. Provide factored bearing resistance and unit sliding resistance at various elevations and confirm no ground improvement/treatment is necessary where spread footings are proposed. Estimated top of rock elevations as well as preliminary skin friction and end bearing values shall be indicated when drilled shafts are proposed. The foundation soils are acceptable for construction of the proposed precast concrete box culvert. The only aggregate needed under the precast concrete box culvert is the 6 inches required by Article 540.06 of the Standard Specifications.

Calculate the estimated water surface elevation and determine the need for cofferdam(s) and seal coat: The proposed box culvert can be constructed using conventional construction methods.

Assess the need for sheeting/soil retention versus using a temporary construction slope and provide recommendation for the most feasible option. If stage construction is utilized, the pay item Temporary Soil Retention System should be used because soil strengths exceed the allowable soil strengths allowed for use of temporary sheet piling.

R	Illinois Dep of Transpo	oartn ortati	ne on	nt		SC	DIL BORING LOG	Ì		Page	<u>    1    </u>	of <u>1</u>
	Division of Highways IDOT		•					-		Date	1/8	8/13
ROUTE	IL 115	DES	SCRI	PTION	۱L 	115 0	ver Reddick Creek, 1.7 miles North of Cabery	_ LC	oggi	ED BY	Larry	Myers
	104-B		_ L	-OCAT		NE 1/4	4, <b>SEC.</b> 5, <b>TWP.</b> 29N, <b>RNG.</b> 9E,	2				
COUNTY Kankakee DRILLING METHOD Hollow Stem Auger HAMMER TYPE CME Automatic												
STRUCT. NO. Station	046-0067 (Exist. 953+12	)	D E P T	B L O	U C S	M 0 1	Surface Water Elev. 660.47   Stream Bed Elev. 659.69	ft ft	D E P	B L O	U C S	M O I
BORING NO. Station Offset	1 (N.W. Quad.) 952+80 14.0 ft Rt.		H	S	Qu	T	Groundwater Elev.: First EncounterDry_ Upon CompletionDry_	ft ft	H	S	Qu	T
Ground Surfa	ace Elev. 669.82	ft	(ft)	(/6")	(tst)	(%)	After Hrs	ft	(ft)	(/ <b>6</b> ")	(tst)	(%)
Silty Clay Loan	n Till Fill						(continued)			8 10	5.3 S	16
		667.32								F		
Black Silty Clay	Ity Clay Loam Till & y Loam Fill	-		5 5 5	>4.5 P	22				5 6 7	4.8 S	17
		-	-5		P				-25			
				4	>4.5	26			_	4 5	4.8	18
		-		5	P		6	42.82		7	S	
Von Ctiff Drou		662.32		3			Very Stiff Gray Silty Clay Loam Till			3		
Loess	in a Gray Silly Gray	-		3	3.0 P	24				4 5	3.4 B	17
		660.32										
Till	In Silly Clay Loan	-	-10	3	3.2	23			-30	3	3.1	20
		-		3	В					4	В	
Hard Grayish E	Brown Silty Clay	057.82										
Loam Till		-		5 6	5.1	18	. (					
				9	S							
Hard Gray Silty	/ Clay Loam Till	655.32	-15						-35			
		-		6	51	20				3	36	18
		-		8	S		6	33.32		5	В	
		-					End of Boring					
		-		5	51	17						
		-		8	S							
			-20						-40			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

R	Illinois Dep of Transpo	oartme ortatio	ent n		SC	DIL BORING LOO	3		Page	<u>1</u>	of <u>1</u>
	Division of Highways DOT				115 0	or Paddiak Crack 17 miles North a	£		Date	1/8	8/13
ROUTE	IL 115	DESC	RIPTIO	N	115 0	Cabery	L	OGGI	ED BY	Larry	Myers
	104-B		LOCA		NE 1/4	4, <b>SEC.</b> 5, <b>TWP.</b> 29N, <b>RNG.</b> 9E,	7				
COUNTY	Kankakee DI		ETHOD		Hol	low Stem Auger HAMMER	TYPE		CME A	utoma	itic
STRUCT. NO. Station	046-0067 (Exist. 953+12	)   C E P	B L O	U C S	M 0 1	Surface Water Elev.660.47Stream Bed Elev.659.69	_ ft _ ft	D E P	B L O	U C S	M O I
BORING NO Station	2 (S.W. Quad.) 953+35	H	I S	Qu	5   T	Groundwater Elev.: First Encounter Dry	_ ft #	H	S S	Qu	T
Ground Surfa	ice Elev. <u>669.83</u>	ft (f	t) (/6")	(tsf)	(%)	After Hrs	ft	(ft)	(/6")	(tsf)	(%)
Augered Shoul Silty Clay Loan	der Stone, Brown n Till Fill					Hard Gray Silty Clay Loam Till (continued)			6 7 7	4.8 S	17
		667.33									
Hard Brown Sil	ty Clay Loam Till y Clay Loam Fill		5	>4.5 P	20				5 5 8	4.3 S	18
			-5	P				-25			
			4	4.5	17				5 4	4.4	19
			4	Р			642.02		6	S	
		662.33				Very Stiff Gray Silty Clay Loam Till	042.00	,	1		
Loess	n & Gray Slity Clay		3	3.5	24				4	3.8	18
				P					5	Б	
			2	25	27				4	3.0	10
			2	2.3 P	21				4	<u>В</u>	13
Very Stiff Brow	n Silty Clay Loam	657.83									
I III, Soft in top	1' (12.5' - 13.5')		2 WH	3.0	22						
wiii - weight of	nammer	655.33	5	B				4			
Hard Gray Silty	Clay Loam Till	-1	5					-35	4		
			8	5.2 S	15		633 33		5 4	3.6 B	19
						End of Boring	033.33	<u> </u>			
			5								
			6 7	4.8 S	18						
		-2	20					-40			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)