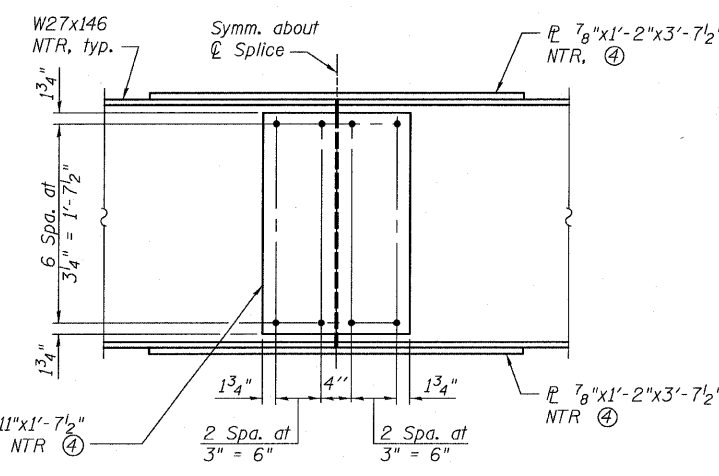
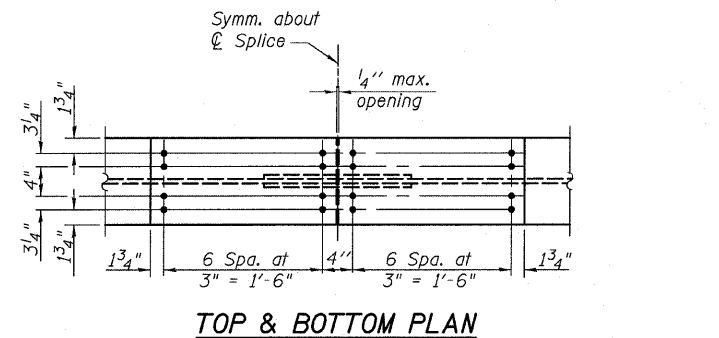
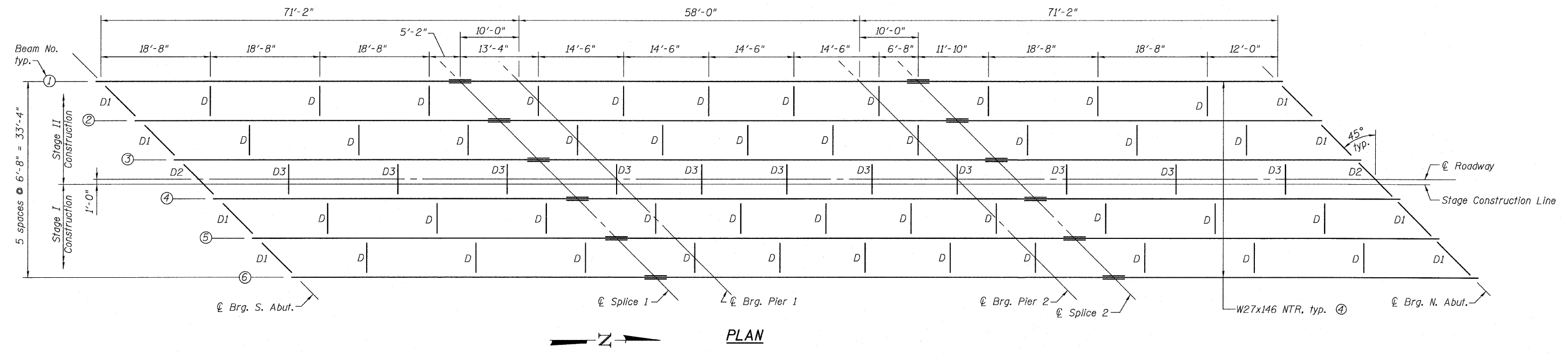


STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION



DESIGNED	MJP
CHECKED	JGS
DRAWN	MJP
CHECKED	JGS

ELEVATION
SPLICE DETAIL
(12 Required)

	0.4 Span 1 & 3	Pier 1 & 2	0.5 Span 2
I_s	5660	5660	5660
$I_c(n)$	14446	14446	14446
$I_c(3n)$	10577	10577	10577
S_s	414	414	414
$S_c(n)$	584	584	584
$S_c(3n)$	530	530	530
Z	464	464	464
ρ	0.864	1.314	0.864
$M \rho$	373.4	547.0	-16.0
$s \rho$	0.450	0.450	0.450
$M_s \rho$	206.4	21.5	21.5
M_t	510.9	243.5	349.8
M_i	130.3	64.3	95.5
$\sum_3 [M_t + M_i]$	1068.7	513.0	742.3
M_a	2143.0	1377.9	972.1
M_u	2703.8	1911.1	2703.8
$f_s \rho$ non-comp	10.82	15.85	-0.46
$f_s \rho$ (comp)	4.67	0.49	0.49
$f_s \rho \sum_3 [M_t + M_i]$	21.96	14.87	15.25
f_s (Overload)	37.46	30.72	15.27
f_s (Total)			
VR	54.2	47.7	47.7

*Compact section
**Braced non-compact and partially braced section

	N. & S. Abut.	Pier 1 & 2
$R \rho$	39.1	92.5
R_t	41.5	45.1
R_i	10.6	8.9
R Total	91.2	146.5

I_s, S_s : Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total and Overload) due to non-composite dead loads (in.⁴ and in.³).

$I_c(n), S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total and Overload) due to short-term composite live loads (in.⁴ and in.³).

$I(3n), S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total and Overload) due to long-term composite (superimposed) dead loads (in.⁴ and in.³).

Z : Plastic Section Modulus of the steel section in non-composite areas (in.³).

ρ : Un-factored non-composite dead load (kips/ft.).

$M \rho$: Un-factored moment due to non-composite dead load (kip-ft.).

$s \rho$: Un-factored long-term composite (superimposed) dead load (kips/ft.).

$M_s \rho$: Un-factored moment due to long-term composite (superimposed) dead load (kip-ft.).

M_t : Un-factored live load moment (kip-ft.).

M_i : Un-factored moment due to impact (kip-ft.).

M_a : Factored design moment (kip-ft.).

$1.3 [M \rho + M_s \rho + \frac{5}{3} (M_t + M_i)]$

M_u : Compact composite moment capacity according to AASHTO LFD 10.50.1.1 or compact non-composite moment capacity according to AASHTO LFD 10.48.1 (kip-ft.).

f_s (Overload): Sum of stresses as computed from the moments below (ksi).

$M \rho + M_s \rho + \frac{5}{3} (M_t + M_i)$

f_s (Total): Sum of stresses as computed from the moments below on non-compact section (ksi).

$1.3 [M \rho + M_s \rho + \frac{5}{3} (M_t + M_i)]$

VR: Maximum t + impact horizontal shear range within the composite portion of the span for stud shear connector design (kips).

FRAMING PLAN
IL 47 OVER JOHNNY RUN
FAP ROUTE 326 - SECTION 119BR
GRUNDY COUNTY
STATION 582+65.75
STRUCTURE NO. 032-0112

Eastport Business Center 1
100 Lanter Court, Suite 1
Collinsville, Illinois 62234
618-345-2200
Design Firm License No. 184.001115

- Notes:
- Load carrying components designated "NTR" shall conform to the Supplemental Requirements for Notch Toughness, Zone 2.
 - For beam elevation and details, see sheet 14 of 27.
 - All diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted. Individual diaphragms at supports may be temporarily disconnected to install bearing anchor rods.
 - AASHTO M 270 Grade 50W steel.