

SHEET NO. SI

S7 SHEETS

TOTAL SHEETS

24

12

- Drainage Aggregate (CA-18) full

length of both headwalls. To be

F.A.P. 649

FED. ROAD DIST. NO. 7

(107) BR

Contract # 66844

LASALLE

LOADING HS20-44

Allow 50#/sq. ft. for future wearing surface.

Design fill height = > 2 ft.

DESIGN STRESSES

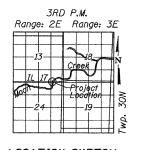
FIELD UNITS

f'o = 3,500 psi fy = 60,000 psi (reinforcement)

PRECAST UNITS

f'o = 5,000 psi

fy = 65,000 psi (welded wire fabric)



LOCATION SKETCH

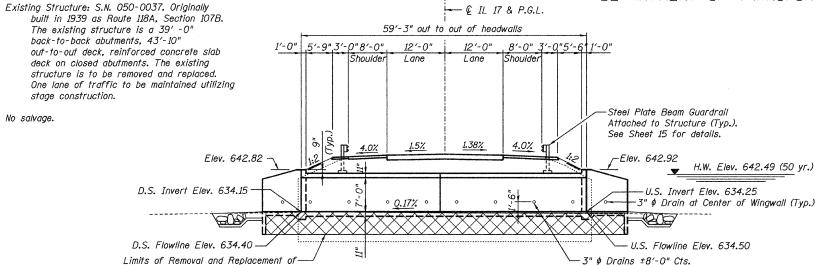
GENERAL PLAN AND ELEVATION ILLINOIS 17 OVER SOUTH BRANCH

OF MOON CREEK

F.A.P. ROUTE 649 SEC. NO. (107) BR

LASALLE COUNTY STATION 50+98.00 STRUCTURE NO. 050-2044

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION



ELEVATION

- £ IL 17 & P.G.L.

Limits of

Existing

Structure



Unsuitable Material Elev. ±630.20

Guardrail

(Typ.)

Stabilization

(See Note 11)

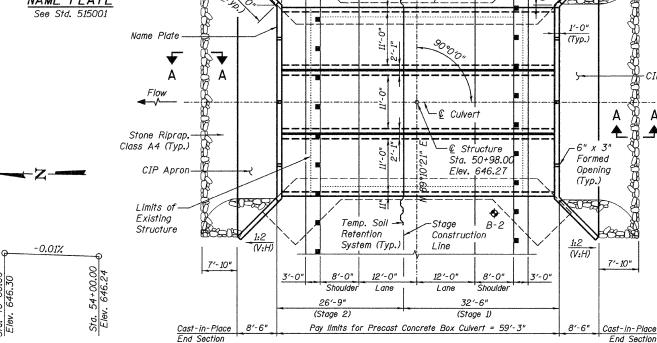
Bench Mark: Chiseled "□" on NE WW Sta. 51+17.71

23.35 Lt. Elevation 645.40.

stage construction.

No salvage.





PROFILE GRADE

WATERWAY INFORMATION - DISTRICT APPROVED

PLAN

	Drainage Ar	ea = 7	.5 mi*(P) & (E)	Low Gr	ade Elev.	644.80	ft. O	Sta. 56+	00.00
DESIGNED - MRB		Freq.	0	Openina	Sa. Ft.	Nat.	Head	- Ft.	Headwat	ter Fl
HMA	Flood	Yr.	Č.F.S.	Exist.	Prop.	H.W.E.	Exist.	Prop.		Prop.
CHECKED - TMA		10	742	246	231	641.85	0.3	0.1	642.11	641.99
144	Design	50	1177	246	231	642.49	0.6	0.4	643.07	642.91
DRAWN - VH	Base	100	1368	246	231	642.72	1.0	0.6	643.74	643.28
1400	Overtopping									
CHECKED - MRB	Max. Calc.	500	1828	246	231	643.20	1.0	1.0	644.23	644.18

DESIGN SCOUR	US	DS
ELEV.	631.2	631.1

or Min. One Per Section

Steel Plate Beam

Guardrail (Typ.)

(V:H)

Geotechnical Report and may be modified by the District Geotechnical Engineer and Field Engineers for variable subsurface encountered in the field.

SECTION THRU BARREL

Limits of Removal

and Replacement of

Unsuitable Material

(See Note 11)

39'-0" out to out

11'-0"

11'-0"

11'-0"

(Typ.)

(Typ.)

GENERAL NOTES 1. Reinforcement bars shall conform to the requirements of ASTM A 706 Gr 60 (IL Modified).

See special provision. 2. Reinforcement bars designated (E) shall be epoxy coated.

- 3. Layout of the slope protection system may be varied to suit ground conditions in the field as directed by the Engineer.
- 4. Precast Concrete Box Culvert sections shall conform to the requirements of Article 540.06 of the Standard Specifications and the applicable requirements of AASHTO M 259.
- 5. Lifting holes shall be filled with concrete plugs and mastic after box sections are in place.
- 6. Class SI Concrete shall be used for cast-in-place concrete.

7. Exposed edges shall be beveled 34".

8. For backfilling and embankment see standard specifications.

9. Excavation behind existing abutment walls shall be performed to balance front and back soil pressure before removing the existing superstructure. The Contractor shall sawcut the upper portion of the existing abutment at the stage removal line before Stage I removal to ensure the remaining portion will not be prematurely damaged.

10. Precast End Sections are not allowed.

11. The material used to replace the unsuitable material removed below the bottom of the proposed precast concrete box culvert and cast-in-place concrete aprons shall be cleaned crushed material CA-1 on the bottom 2'-6" layer and CA-7 on the top 6" layer and shall be paid for as "Porous Granular Embankment, Special".

TOTAL BILL OF MATERIAL

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	Item	Unit	Total
	POROUS GRANULAR EMBANKMENT	CU YD	596
*	POROUS GRANULAR EMBANKMENT, SPECIAL	CU YD	398
	STONE RIPRAP, CLASS A4	SQ YD	<i>11</i> 5
	FILTER FABRIC	SQ YD	99
	REMOVAL OF EXISTING STRUCTURES	EACH	1
	STRUCTURE EXCAVATION	CU YD	428
	REMOVAL AND DISPOSAL OF UNSUITABLE MATERIAL FOR STRUCTURES	CU YD	398
	NAME PLATES	EACH	1
	BOX CULVERT END SECTIONS	EACH	2
*	TEMPORARY SOIL RETENTION SYSTEM	SQ FT	327
*	PRECAST CONCRETE BOX CULVERT 11' X 7'	F00T	178

* See Special Provision.



DATE 8/7/08

benesch

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