



Illinois Department of Transportation

Memorandum

To: *

From: Rich Dotson *RJD*

Subject: **Special Provision Changes**

Date: October 8, 2009

The following special provisions have been revised for the January 15, 2010 letting. Please revise your special provision books as indicated.

Interim Special Provisions

ISP Number	Description
Alphabetic ISP Index (Revised)	Remove existing alphabetic index and insert revised index.
Numerical ISP Index (Revised)	Remove existing numeric index and insert revised index.
(Delete all)	The following were incorporated into the Supplemental Specifications.
109.10	Payrolls and Payroll Records (BDE)
406.00a	Hot-Mix Asphalt Mixture IL-4.75 (BDE)
406.17	Watched Wedge longitudinal Joint (BDE)
420.11	Variable Spaced Tining (BDE)
630.02	Plastic Blockouts for guardrail (BDE)
637.00	Concrete Barrier (BDE)
720.00	Sign Panels and Sign Panel Overlays (BDE)
1004.03	Hot-Mix Asphalt Mixture IL-9.5L (BDE)
1005.01	Stone Gradation Testing (BDE)
1006.10a	Reinforcement Bars (BDE)
1006.25	Steel Plate Beam Guardrail (BDE)
1006.28	Woven Wire Fence (BDE)
1009.01	Retroreflective Sheeting, Nonreflective Sheeting, and Translucent Overlay Film for Highway Signs (BDE)
1030.05	Hot-Mix Asphalt – Fields Voids in Mineral Aggregate (BDE)
1080.02	Silt Filter Fence (BDE)
1090.00	Type ZZ Retroreflective Sheeting, nonreflective Sheeting, and Translucent Overlay Film for Highway Signs (BDE)
1095.04	Epoxy Pavement Markings (BDE)
107.04	Right of Entry Permit (BDE) (deleted from ISP checklist. Not moved to Supplemental Specifications).
250.00 (Revised)	“Seeding (BDE)” Updates the seed testing requirements.

Interim Special Provisions (Continued)

ISP Number	Description
280.04 (Revised)	“Temporary Erosion Control (BDE)” Modifies temporary ditch checks and payment.
302.04 (New)	“Improved Subgrade (BDE)” Addresses problem with improved subgrades exposed to winter before entire pavement thickness is placed.
406.06 (New)	“Stone Matrix Asphalt (BDE)” Provides specifications for SMA.
406.07 (New)	“Hot-Mix Asphalt – Density Testing of Longitudinal Joints (BDE)” Developed to improve the HMA longitudinal joints.
609.02 (New)	“Frames and Grates (BDE)” Eliminates steel option.
631.07 (New)	“Traffic Barrier Terminal, Type 6 (BDE)” Eliminates wood blockout that was creating a snag point.
701.03 (New)	“Truck Mounted/Trailer Mounted Attenuators (BDE)” Brings specifications up-to-date with industry.
701.07a (New)	“Hot-Mix Asphalt – Drop-Offs (BDE)” Corrects the spacing of devices delineating drop-offs.
701.17 (New)	“Pavement Patching (BDE)” Improves delineation of patches on two lane roads.
1008.26 (Revised)	“Organic Zinc-Rich Paint System (BDE)” Revised to comply with IEPA.
1008.27 (Revised)	“Moisture Cured Urethane Paint System (BDE)” Revised to comply with IEPA.
1030.04 (Revised)	“Hot-Mix Asphalt – Plant Test Frequency (BDE)” Brings testing in line with nationwide standards.
1030.05 (New)	“Hot-Mix Asphalt – QC/QA Acceptance Criteria (BDE)” Adds VMA to the HMA QC/QA acceptance criteria.
1031.00 (Revised)	“Reclaimed Asphalt Pavement (RAP) (BDE)” Revised to require quality testing for fractionated RAP and to define allowable fractions.
1080.03 (Revised)	“Filter Fabric (BDE)” Minor corrections.
1106.02k (New)	“Longitudinal Temporary Traffic Barrier System (BDE)” Provides a new means of protection.
1106.02l (New)	“Movable Traffic Barrier System (BDE)” Provides a means of positive protection.

District Special Provisions

Alphabetic District Index (Revised)	Remove existing alphabetic index and insert revised index.
Numerical District Index (Revised)	Remove existing numeric index for Section 400 and insert revised index.
406.07 (Delete)	“Hot-Mix Asphalt – Density Testing of longitudinal Joint” Covered by BDE special.

Designer Notes for January 1, 2010 Recurring Special Provisions

Updated to 2010 Recurring Special Book

General Notes

No Changes

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Attachment(s)

cc: * J. Miller	Team 2	Team 6	Team 10	Galesburg Design (D. Painter)
K. Emert	Team 3	Team 7	Team 11	Local Roads (M. Augspurger)
T. Phillips	Team 4	Team 8	Geometrics	Materials (H. Shoup)
Team 1	Team 5	Team 9	Bridge (T. Inglis)	

BDE Special Provisions

Alphabetic Index

ALPHABETIC LIST OF DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

Get a copy of the current check list from the Program Development Secretary, indicate which ISP's are to be included in your set of special provisions, fill in any blanks as indicated on the check list, and include with your set of special provisions to be sent to Springfield where they will be inserted.

<u>Standard Spec. No.</u>	<u>PC No.</u>	<u>Item</u>
280.02	28002	Above Grade Inlet Protection
888.00	88800	Accessible Pedestrian Signals (APS)
1020.02	102002	Alkali-Silica Reaction for Cast-in-Place Concrete
1020.03	102003	Alkali-Silica Reaction for Precast and Precast Prestressed Concrete
109.12	10912	American Recovery and Reinvestment Act Provisions
701.04	70104	American Recovery and Reinvestment Act Signing
107.22	10722	Approval of Proposed Borrow Areas, Use Areas, and/or Waste Areas Inside Illinois State Borders
701.00	70100	Automated Flagger Assistance Devices
109.01	10901	Bituminous Materials Cost Adjustment
107.38	10738	Bridge Demolition Debris
107.19a	10719a	Building Removal Case I
107.19b	10719b	Building Removal Case II
107.19c	10719c	Building Removal Case III
107.19d	10719d	Building Removal Case IV
1001.00	100100	Cement
108.05a	10805a	Completion Date (Via Calendar Days)
108.05b	10805b	Completion Date (Via Calendar Days) Plus working Days
1020.05b	102005b	Concrete Admixtures
606.07	60607	Concrete Gutter, Type A
503.19	50319	Concrete Joint Sealer
1020.05c	102005c	Concrete Mix Designs

ALPHABETIC LIST OF DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

<u>Standard Spec. No.</u>	<u>PC No.</u>	<u>Item</u>
107.00	10700	Construction air Quality – Diesel Vehicle Emissions Control
10737	10737	Construction air Quality – Idling Restrictions
353.00	35300	Determination of Thickness
202.07	20207	Digital Terrain Modeling for Earthwork Calculations
108.06a	10806a	Disadvantaged Business Enterprise Participation
1006.11	100611	Dowel Bars
670.02	67002	Engineer's Field Office, Type A
670.03	67003	Engineer's Field Office, Type B
109.04	10904	Equipment Rental Rates
1080.03	108003	Filter Fabric
701.13	70113	Flagger at Side Roads and Entrances
609.02	60902	Frames and Grates
109.03	10903	Fuel Cost Adjustment
643.00	64300	High Tension Cable Median Barrier
407.08	40708	HMA-Hauling on Partially Completed Full-Depth Pavement
1030.04c	103004c	Hot-Mix Asphalt – Anti-Stripping Additive
406.07	40607	Hot-Mix Asphalt-Density Testing of Longitudinal Joints
701.07a	70107a	Hot-Mix Asphalt Drop-Offs
1030.04	103004	Hot-Mix Asphalt – Plant Test Frequency
1030.05	103005	Hot-Mix Asphalt – QC/QA Acceptance Criteria
1030.08	103008	Hot-Mix Asphalt - Transportation
702.00c	70200c	Impact Attenuators
702.00d	70200d	Impact Attenuators, Temporary
302.04	30204	Improved Subgrade

ALPHABETIC LIST OF DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

<u>Standard Spec. No.</u>	<u>PC No.</u>	<u>Item</u>
108.09	10809	Liquidated Damages
1106.02k	110602k	Longitudinal Temporary Traffic Barrier System
1077.03	107703	Mast Arm Assembly and Pole
406.00f	40600f	Material Transfer Device
503.02	50302	Metal Hardware Cast into Concrete
1008.27	100827	Moisture Cured Urethane Paint System
109.11	10911	Monthly Employment Report
1106.02l	110602l	Movable Traffic Barrier System
442.08	44208	Multilane Pavement Patching
105.03	10503	National Pollutant Discharge Elimination System / Erosion and Sediment Control Deficiency Deduction
701.01	70101	Nighttime Work Zone Lighting
701.06	70106	Notification of Reduced Width
1008.26	100826	Organic Zinc-Rich Paint System
701.07	70107	Partial Exit Ramp Closure for Freeway/Expressway
783.03	78303	Pavement Marking Removal
701.17	70117	Pavement Patching
109.07	10907	Payments to Subcontractors
701.12	70112	Personal Protective Equipment
542.03	54203	Pipe Culverts
780.00	78000	Polyurea Pavement Marking
420.00	42000	Portland Cement Concrete Inlay or Overlay
1020.11	102011	Portland Cement Concrete Plants
400.04	40004	Preventive Maintenance - Bituminous Surface Treatment
400.01	40001	Preventive Maintenance - Cape Seal

ALPHABETIC LIST OF DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

Standard Spec. No.	PC No.	Item
400.02	40002	Preventive Maintenance – Micro-Surfacing
400.03	40003	Preventive Maintenance – Slurry Seal
1090.03	109003	Post Clips for Extruded Aluminum Signs
540.02	54002	Precast Concrete Handling Holes
782.03	78203	Prismatic Curb Reflectors
107.09	10709	Public Convenience and Safety
107.11	10711a	Railroad Protective Liability Insurance
107.11	10711b	Railroad Protective Liability Insurance (5 and 10)
781.03	78103	Raised Reflective Pavement Markers
701.02	70102	Ramp Closure for Freeway/Expressway
1031.00	103100	Reclaimed Asphalt Pavement (RAP)
1106.02	110602	Reflective Sheeting on Channelizing Devices
420.16	42016	Restoring Bridge Approach Pavements Using High-Density Foam
250.00	25000	Seeding
1020.01	102001	Self-Consolidating Concrete for Cast-in-Place Construction
1020.00	102000	Self-Consolidating Concrete for Precast Products
109.00	10900a	Steel Cost Adjustment
406.06	40606	Stone Matrix Asphalt
508.03	50803	Storage and Protection of Reinforcement Bars
550.02	55002	Storm Sewers
671.00	67100	Subcontractor Mobilization Payments
406.21	40621	Surface Testing of Pavements
280.04	28004	Temporary Erosion Control

ALPHABETIC LIST OF DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

Standard Spec. No.	PC No.	Item
703.00	70300	Temporary Raised Pavement Marker
1095.01	109501	Thermoplastic Pavement Markings
631.07	63107	Traffic Barrier Terminal, Type 6
108.06	10806	Training Special Provision
701.03	70103	Truck Mounted/Trailer Mounted Attenuators
108.05	10805	Working Days

BDE Special Provisions

Numeric Index

REVISED INDEX

NUMERIC DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

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<u>Standard Spec. No.</u>	<u>PC No.</u>	<u>Item</u>
105.03	10503	National Pollutant Discharge Elimination System / Erosion and Sediment Control Deficiency Deduction
107.00	10700	Construction Air Quality – Diesel Vehicle Emissions Control
107.09	10709	Public Convenience and Safety
107.11a	10711a	Railroad Protective Liability Insurance
107.11b	10711b	Railroad Protective Liability Insurance (5 and 10)
107.19a	10719a	Building Removal Case I
107.19b	10719b	Building Removal Case II
107.19c	10719c	Building Removal Case III
107.19d	10719d	Building Removal Case IV
107.22	10722	Approval of Proposed Borrow Areas, Use Areas, and/or Waste Areas Inside Illinois State Borders
107.37	10737	Construction Air Quality – Idling Restrictions
107.38	10738	Bridge Demolition Debris
108.05	10805	Working Days
108.05a	10805a	Completion Date (Via Calendar Days)
108.05b	10805b	Completion Date (Via Calendar Days) Plus Working Days
108.06	10806	Training Special Provision
108.06a	10806a	Disadvantaged Business Enterprise Participation
108.09	10809	Liquidated Damages
109.00a	10900a	Steel Cost Adjustment
109.01	10901	Bituminous Materials Cost Adjustments

NUMERIC DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

<u>Standard Spec. No.</u>	<u>PC No.</u>	<u>Item</u>
109.03	10903	Fuel Cost Adjustment
109.04	10904	Equipment Rental Rates
109.07	10907	Payments to Subcontractors
109.11	10911	Monthly Employment Report
109.12	10912	American Recovery and Reinvestment Act Provisions
202.07	20207	Digital Terrain Modeling for Earthwork Calculations
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302.04	30204	Improved Subgrade
353.00	35300	Determination of Thickness
400.01	40001	Preventive Maintenance – Cape Seal
400.02	40002	Preventive Maintenance – Micro-Surfacing
400.03	40003	Preventive Maintenance – Slurry Seal
400.04	40004	Preventive Maintenance – Bituminous Surface Treatment
406.00f	40600f	Material Transfer Device
406.06	40606	Stone Matrix Asphalt
406.07	40607	Hot-Mix Asphalt – Density Testing of Longitudinal Joints
406.21	40621	Surface Testing of Interstate Pavements
407.08	40708	HMA-Hauling on Partially Completed Full-Depth Pavement
420.00	42000	Portland Cement Concrete Inlay or Overlay
420.16	42016	Restoring Bridge Approach Pavements Using High-Density Foam
442.08	44208	Multilane Pavement Patching
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609.02	60902	Frames and Grates
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643.00	64300	High Tension Cable Median Barrier
670.02	67002	Engineers Field Office Type A
670.03	67003	Engineers Field Office Type B
671.00	67100	Subcontractor Mobilization Payments
701.00	70100	Automated Flagger Assistance Devices
701.01	70101	Nighttime Work Zone Lighting
701.02	70102	Ramp Closure for Freeway/Expressway
701.03	70103	Truck Mounted/Trailer Mounted Attenuators
701.04	70104	American Recovery and Reinvestment Act Signing
701.06	70106	Notification of Reduced Width
701.07	70107	Partial Exit Ramp Closure for Freeway/Expressway
701.07a	70107a	Hot-Mix Asphalt – Drop-Offs
701.12	70112	Personal Protective Equipment
701.13	70113	Flagger at Side Roads and Entrances
701.17	70117	Pavement Patching
702.00c	70200c	Impact Attenuators
702.00d	70200d	Impact Attenuators, Temporary
703.00	70300	Temporary Raised Pavement Marker

NUMERIC DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

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780.00	78000	Polyurea Pavement Marking
781.03	78103	Raised Reflective Pavement Markers
782.03	78203	Prismatic Curb Reflectors
783.03	78303	Pavement Marking Removal
888.00	88800	Accessible Pedestrian Signals (APS)
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1008.26	100826	Organic Zinc-Rich Paint System
1008.27	100827	Moisture Cured Urethane Paint System
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1020.01	102001	Self-Consolidating Concrete for Cast-in-Place Construction
1020.02	102002	Alkali-Silica Reaction for Cast-in-Place Concrete
1020.03	102003	Alkali-Silica Reaction for Precast and Precast Prestressed Concrete
1020.05b	102005b	Concrete Admixtures
1020.05c	102005c	Concrete Mix Designs
1020.11	102011	Portland Cement Concrete Plants
1030.04	103004	Hot-Mix Asphalt – Plant Test Frequency
1030.04c	103004c	Hot-Mix Asphalt – Anti-Stripping Additive
1030.05	103005	Hot-Mix Asphalt – QC/QA Acceptance Criteria
1030.08	103008	Hot-Mix Asphalt - Transportation
1031.00	103100	Reclaimed Asphalt Pavement (RAP)
1077.03	107703	Mast Arm Assembly and Pole
1080.03	108003	Filter Fabric
1090.03	109003	Post Clips for Extruded Aluminum Signs
1095.01	1095.01	Thermoplastic Pavement Markings

NUMERIC DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

<u>Standard Spec. No.</u>	<u>PC No.</u>	<u>Item</u>
1106.02	110602	Reflective Sheeting on Channelizing Devices
1106.02k	110602k	Longitudinal Temporary Traffic Barrier System
1106.02l	110602l	Movable Traffic Barrier System

BDE Special Provisions

Designer Note: Insert into all contracts with permanent seeding.

SEEDING (BDE)

Effective: July 1, 2004

Revised: January 1, 2010

Revise the following seeding mixtures shown in Table 1 of Article 250.07 of the Standard Specifications to read:

"Table 1 - SEEDING MIXTURES		
Class – Type	Seeds	lb/acre (kg/hectare)
1A Salt Tolerant Lawn Mixture 7/	Bluegrass Perennial Ryegrass Red Fescue (Audubon, Sea Link, or Epic) Hard Fescue (Rescue 911, Spartan II, or Reliant IV) Fults Salt Grass 1/ or Salty Alkaligrass	60 (70) 20 (20) 20 (20) 20 (20) 60 (70)
2 Roadside Mixture 7/	Tall Fescue (Inferno, Tarheel II, Quest, Blade Runner, or Falcon IV) Perennial Ryegrass Creeping Red Fescue Red Top	100 (110) 50 (55) 40 (50) 10 (10)
2A Salt Tolerant Roadside Mixture 7/	Tall Fescue (Inferno, Tarheel II, Quest, Blade Runner, or Falcon IV) Perennial Ryegrass Red Fescue (Audubon, Sea Link, or Epic) Hard Fescue (Rescue 911, Spartan II, or Reliant IV) Fults Salt Grass 1/ or Salty Alkaligrass	60 (70) 20 (20) 30 (20) 30 (20) 60 (70)
3 Northern Illinois Slope Mixture 7/	Elymus Canadensis (Canada Wild Rye) Perennial Ryegrass Alsike Cover 2/ Desmanthus Illinoensis (Illinois Bundleflower) 2/, 5/ Andropogon Scoparius (Little Bluestem) 5/ Bouteloua Curtipendula (Side-Oats Grama) Fults Salt Grass 1/ or Salty Alkaligrass Oats, Spring Slender Wheat Grass 5/ Buffalo Grass (Cody or Bowie) 4/, 5/, 9/	5 (5) 20 (20) 5 (5) 2 (2) 12 (12) 10 (10) 30 (35) 50 (55) 15 (15) 5 (5)

"Table 1 - SEEDING MIXTURES			
6A	Salt Tolerant Conservation Mixture	Andropogon Scoparius (Little Bluestem) 5/	5 (5)
		Elymus Canadensis (Canada Wild Rye) 5/	2 (2)
		Buffalo Grass (Cody or Bowie) 4/, 5/, 9/	5 (5)
		Vernal Alfalfa 2/	15 (15)
		Oats, Spring	48 (55)
		Fults Salt Grass 1/ or Salty Alkaligrass	20 (20)"

Revise Note 7 of Table 1 – Seeding Mixtures of Article 250.07 of the Standard Specifications to read:

"7/ In Districts 1 through 6, the planting times shall be April 1 to June 15 and August 1 to November 1. In Districts 7 through 9, the planting times shall be March 1 to June 1 and August 1 to November 15. Seeding may be performed outside these dates provided the Contractor guarantees a minimum of 75 percent uniform growth over the entire seeded area(s) after a period of establishment. Inspection dates for the period of establishment will be as follows: Seeding conducted in Districts 1 through 6 between June 16 and July 31 will be inspected after April 15 and seeding conducted between November 2 and March 31 will be inspected after September 15. Seeding conducted in Districts 7 through 9 between June 2 and July 31 will be inspected after April 15 and seeding conducted between November 16 and February 28 will be inspected after September 15. The guarantee shall be submitted to the Engineer in writing prior to performing the work. After the period of establishment, areas not exhibiting 75 percent uniform growth shall be interseeded or reseeded, as determined by the Engineer, at no additional cost to the Department."

Revise the first paragraph of Article 1081.04(a) of the Standard Specifications to read:

"(a) Sampling and Testing. Each lot of seed furnished shall be tested by a State Agriculture Department (including other States) or by land grant college or university agricultural sections or by a Registered Seed Technologist. Testing of seed shall be accomplished within the 12 months prior to the seed being installed on the project."

Delete the last sentence of the first paragraph of Article 1081.04(c)(2) of the Standard Specifications.

Revise Table II of Article 1081.04(c)(6) of the Standard Specifications to read:

TABLE II						
Variety of Seeds	Hard Seed %	Purity %	Pure Live Seed %	Weed %	Secondary * Noxious Weeds No. per oz (kg)	Notes
	Max.	Min.	Min.	Max.	Max. Permitted	
Alfalfa	20	92	89	0.50	6 (211)	1/
Clover, Alsike	15	92	87	0.30	6 (211)	2/
Red Fescue, Audubon	0	97	82	0.10	3 (105)	-
Red Fescue, Creeping	-	97	82	1.00	6 (211)	-
Red Fescue, Epic	-	98	83	0.05	1 (35)	-
Red Fescue, Sea Link	-	98	83	0.10	3 (105)	-
Tall Fescue, Blade Runner	-	98	83	0.10	2 (70)	-
Tall Fescue, Falcon IV	-	98	83	0.05	1 (35)	-
Tall Fescue, Inferno	0	98	83	0.10	2 (70)	-
Tall Fescue, Tarheel II	-	97	82	1.00	6 (211)	-
Tall Fescue, Quest	0	98	83	0.10	2 (70)	-
Fults Salt Grass	0	98	85	0.10	2 (70)	-
Salty Alkaligrass	0	98	85	0.10	2 (70)	-
Kentucky Bluegrass	-	97	80	0.30	7 (247)	4/
Oats	-	92	88	0.50	2 (70)	3/
Redtop	-	90	78	1.80	5 (175)	3/
Ryegrass, Perennial, Annual	-	97	85	0.30	5 (175)	3/
Rye, Grain, Winter	-	92	83	0.50	2 (70)	3/
Hard Fescue, Reliant IV	-	98	83	0.05	1 (35)	-
Hard Fescue, Rescue 911	0	97	82	0.10	3 (105)	-
Hard Fescue, Spartan II	-	98	83	0.10	3 (105)	-
Timothy	-	92	84	0.50	5 (175)	3/
Wheat, hard Red Winter	-	92	89	0.50	2 (70)	3/

Revise the first sentence of the first paragraph of Article 1081.04(c)(7) of the Standard Specifications to read:

"The seed quantities indicated per acre (hectare) for Prairie Grass Seed in Classes 3, 3A, 4, 4A, 6, and 6A in Article 250.07 shall be the amounts of pure, live seed per acre (hectare) for each species listed."

Designer Note: Insert into all contracts utilizing temporary erosion control.

TEMPORARY EROSION CONTROL (BDE)

Effective: November 1, 2002

Revised: January 1, 2010

Add the following to Article 280.02 of the Standard Specifications to read:

“(k) Filter Fabric1080.03”

Revise the third paragraph of Article 280.03 of the Standard Specifications to read:

“Erosion control systems shall be installed prior to beginning any activities which will potentially create erodible conditions. Erosion control systems for areas outside the limits of construction such as storage sites, plant sites, waste sites, haul roads, and Contractor furnished borrow sites shall be installed prior to beginning soil disturbing activities at each area. These offsite systems shall be designed by the Contractor and be subject to the approval of the Engineer.”

Add the following paragraph after the third paragraph of Article 280.03 of the Standard Specifications:

“The temporary erosion and sediment control systems shown on the plans represent the minimum systems anticipated for the project. Conditions created by the Contractor's operations, or for the Contractor's convenience, which are not covered by the plans, shall be protected as directed by the Engineer at no additional cost to the Department. Revisions or modifications of the erosion and sediment control systems shall have the Engineer's written approval.”

Revise Article 280.04(a) of the Standard Specifications to read:

“(a) Temporary Ditch Checks. This system consists of the construction of temporary ditch checks to prevent siltation, erosion, or scour of ditches and drainage ways. Temporary ditch checks shall be constructed with rolled excelsior, products from the Department's approved list, or with aggregate placed on filter fabric when specified. Filter fabric shall be installed according to the requirements of Section 282. Riprap shall be placed according to Article 281.04. Manufactured ditch checks shall be installed according to the manufacturer's specifications. Spacing of ditch checks shall be such that the low point in the center of one ditch check is at the same elevation as the base of the ditch check immediately upstream. Temporary ditch checks shall be sufficiently long enough that the top of the device in the middle of the ditch is lower than the bottom of the terminating ends of the ditch side slopes.”

Revise the last sentence of the first paragraph of Article 280.04(g) of the Standard Specifications to read:

“The temporary mulch cover shall be according to either Article 251.03 or 251.04 except for any reference to seeding.”

Revise Article 280.07(b) of the Standard Specifications to read:

“(b) Temporary Ditch Checks. This work will be measured for payment along the long axis of the device in place in feet (meters) except for aggregate ditch checks which will be measured for payment in tons (metric tons). Payment will not be made for aggregate in excess of 108 percent of the amount specified by the Engineer.”

Revise Article 280.07(f) of the Standard Specifications to read:

“(f) Temporary Mulch. This work will be measured for payment according to Article 251.05(b).”

Add the following paragraph after the ninth paragraph of Article 280.07 of the Standard Specifications:

“Temporary or permanent erosion control systems required for areas outside the limits of construction will not be measured for payment.”

Revise Article 280.08(b) of the Standard Specifications to read:

“(b) Temporary Ditch Checks. This work will be paid for at the contract unit price per foot (meter) for TEMPORARY DITCH CHECKS except for aggregate ditch checks which will be paid for at the contract unit price per ton (metric ton) for AGGREGATE DITCH CHECKS.”

Revise Article 280.08(f) of the Standard Specifications to read:

“(f) Temporary Mulch. Temporary Mulch will be paid for according to Article 251.06.”

Delete the tenth (last) paragraph of Article 280.08 of the Standard Specifications.

Revise the second sentence of the first paragraph of Article 1081.015(e) of the Standard Specifications to read:

“The upstream facing of the aggregate ditch check shall be constructed of gradation CA 3. The remainder of the ditch check shall be constructed of gradation RR 3.”

Designer Note: Insert into contracts using soil modification, lime stabilization, or granular subbase for improving the subgrade.

IMPROVED SUBGRADE (BDE)

Effective: January 1, 2010

Revise the second paragraph of Article 302.04 of the Standard Specifications to read:

“The quantity of modified soil constructed shall be limited to that which can be covered by the full thickness of portland cement concrete pavement or HMA binder during the same construction season.”

Revise the first paragraph of Article 302.07 of the Standard Specifications to read:

“**302.07 Application of Modifier.** The modifier shall be applied uniformly on the soil. The application of modifier shall be limited to that amount which can be mixed with the soil within the same working day.”

Revise the first paragraph of Article 302.08 of the Standard Specifications to read:

“**302.08 Mixing.** The modifier, soil, and water shall be thoroughly mixed. Mixing shall continue until a homogenous layer of the required thickness has been obtained and a minimum of 75 percent of the mixture is smaller than 1 in. (25 mm). The moisture content of the modified soil shall be above optimum moisture content with a maximum of three percent above optimum.”

Revise Article 302.10 of the Standard Specifications to read:

“**302.10 Finishing and Curing.** When multiple lifts are used to construct the modified soil layer, the top lift shall be a minimum of 6 in. (150 mm) thick when compacted.

Construction of pipe underdrains shall follow the requirements of Article 407.07. The surface of the modified soil shall be kept drained according to Article 301.09 and shall maintain moisture content not exceeding three percent above optimum prior to pavement construction.

When compaction of the modified soil is nearing completion, the surface shall be shaped to the required lines, grades, and cross section shown on the plans. For HMA base course and pavement (full-depth) and portland cement concrete base course and pavement, the surface of the modified soil shall be brought to true shape and correct elevation according to Article 301.07, except well compacted earth shall not be used to fill low areas.

The modified soil shall be cured for a minimum of 24 hours. The ambient air temperature shall be above 45 °F (7 °C) during curing.

During the curing period, the moisture content of the modified soil shall be maintained at optimum by sprinkling with water, use of plastic sheeting, or applying bituminous materials according to Article 312.14. During this period, no equipment or traffic will be permitted on the completed work beyond that required for maintenance of curing.

Equipment of such weight, or used in such a way as to cause a rut depth of 1/2 in. (13 mm) or more in the finished modified soil, shall be removed, or the rutting otherwise prevented, as directed by the Engineer.”

Revise the first paragraph of Article 302.11 of the Standard Specifications to read:

“302.11 Subgrade Stability. Following curing, the Engineer will determine the stability of the modified soil in terms of the immediate bearing value (IBV), according to Illinois Test Procedure 501. The IBV shall be a minimum of 10.0 measured within 10 calendar days prior to pavement construction.”

Revise the second paragraph of Article 310.04 of the Standard Specifications to read:

“The quantity of lime stabilized soil mixture constructed shall be limited to that which can be covered by the full thickness of portland cement concrete pavement or HMA binder during the same construction season.”

Revise the first paragraph of Article 310.08(a) of the Standard Specifications to read:

“(a) Initial Mixing. The lime, soil, and water shall be thoroughly mixed until a uniform mixture throughout the required depth and width is obtained. All clods and lumps shall be reduced to a maximum size of 2 in. (50 mm). The moisture content of the stabilized soil shall be above optimum moisture content with a maximum of three percent above optimum.”

Insert the following paragraph after the first paragraph of Article 310.10 of the Standard Specifications:

“Construction of pipe underdrains shall follow the requirements of Article 407.07. The surface of the lime stabilized soil shall be kept drained according to Article 301.09 and shall maintain a maximum moisture content of three percent above optimum prior to pavement construction.”

Revise the first paragraph of Article 310.11 of the Standard Specifications to read:

“310.11 Subgrade Stability. Following curing, the Engineer will determine the stability of the lime stabilized soil mixture in terms of the immediate bearing value (IBV) according to Illinois Test Procedure 501. The IBV shall be a minimum of 23.0 measured within 10 calendar days prior to pavement construction.”

Revise the second paragraph of Article 311.05 of the Standard Specifications to read:

“The granular material shall be placed and compacted at least three days prior to the placement of pavement or base course. Except where required for temporary access, the quantity of subbase granular material Types A or B to be placed shall be limited to that which can be covered by the full thickness of PCC pavement or HMA binder during the same construction season.”

Designer Note: Insert into contracts where use of a Stone Matrix Asphalt mix has been selected. Consult Implementation and Project Engineer prior to use. Additional cost will have to be programmed as this mix is generally more expensive.

STONE MATRIX ASPHALT (BDE)

Effective: January 1, 2010

Description. This work shall consist of constructing polymer modified 1/2 in. (12.5 mm) stone matrix asphalt (SMA) surface course and binder course. Work shall be according to Sections 406, 407 and 1030 of the Standard Specifications, except as modified herein.

Materials.

Add the following to the end of the first paragraph of Article 1003.03(a) of the Standard Specifications:

“Fine aggregate for SMA shall consist of stone sand, slag sand, or steel slag sand.”

Add the following to the end of the first paragraph of Article 1003.03(c) of the Standard Specifications.

“The fine aggregate gradation for SMA shall be FA/FM 20.”

Add the following to the end of Article 1004.03(a) of the Standard Specifications:

“(1) For SMA surface course, the coarse aggregate shall be crushed aggregate meeting the friction requirement specified.

(2) For SMA binder course, the coarse aggregate shall be crushed aggregate. Steel slag will not be permitted in the binder course.”

Revise Article 1004.03(b) of the Standard Specifications to read:

“(b) Quality. For surface courses and binder courses when used as surface course, the coarse aggregate shall be Class B quality or better. For SMA surface and binder courses the coarse aggregate shall be Class B Quality or better. For Class A (seal or cover coat), other binder courses, and surface course IL-9.5L (Low ESAL), the coarse aggregate shall be Class C quality or better. For All Other courses, the coarse aggregate shall be Class D quality or better.”

Revise Article 1004.03(c) of the Standard Specifications to read:

“(c) Gradation. The coarse aggregate gradations shall be as listed in the following table.

Use	Size/Application	Gradation No.
Class A-1, 2, & 3	3/8 in. (10 mm) Seal	CA 16
Class A-1	1/2 in. (13 mm) Seal	CA 15
Class A-2 & 3	Cover	CA 14
HMA High ESAL	IL-25.0 IL-19.0 IL-12.5 IL-9.5	CA 7 ^{1/} or CA 8 ^{1/} CA 11 ^{1/} CA 16 and/or CA 13 CA 16
HMA Low ESAL	IL-19.0L IL-9.5L	CA 11 ^{1/} CA 16
HMA All Other	Stabilized Subbase or Shoulders	CA 6 ^{2/} , CA 10, or CA 12
SMA	1/2 in. (12.5 mm) Binder & Surface	3/

1/ CA 16 or CA 13 may be blended with the gradations listed.

2/ CA 6 will not be permitted in the top lift of shoulders.

3/ No individual coarse aggregate gradation is specified. The coarse aggregates used shall be capable of being combined with stone sand, slag sand, or steel slag sand meeting the FA/FM 20 gradation and mineral filler to meet the approved mix design and the mix requirements noted herein.”

Add the following to Article 1004.03 of the Standard Specifications:

“(d) Flat and Elongated Particles. For SMA the coarse aggregate shall meet the criteria for Flat and Elongated Particles listed in Illinois Modified AASHTO M 325.

(e) Absorption. For SMA the coarse aggregate shall also have water absorption ≤ 2.5 percent.”

Add the following to Article 1011.01 of the Standard Specifications:

“(c) Additional requirements for SMA. Mineral filler for use in SMA shall be free from organic impurities and have a Plasticity Index ≤ 4 .”

Revise Article 1030.02(c) of the Standard Specifications to read:

“(c) RAP Material (Note 4) 1031”

Revise Article 1030.02(g) of the Standard Specifications to read:

“(g) Performance Graded Asphalt Binder (Notes 2 & 5) 1032”

Add the following to Article 1030.02 of the Standard Specifications:

“(h) Fibers (Note 6)”

Add the following notes to Article 1030.02 of the Standard Specifications:

“Note 4. RAP will not be permitted in SMA.

Note 5. The asphalt cement shall be an SBS PG 76-28 when the SMA is used on a full depth asphalt pavement and a SBS PG76-22 when used as an overlay.

Note 6. A stabilizing additive such as cellulose or mineral fiber shall be added to the SMA mixture according to Illinois Modified AASHTO M 325. The stabilizing additive shall meet the Fiber Quality Requirements listed in Illinois Modified AASHTO M 325. Prior to approval and use of fibers, the Contractor shall submit a notarized certification by the producer of these materials stating they meet these requirements.”

Mix Design.

Add the following below the referenced AASHTO standards in Article 1030.04 of the Standard Specifications:

“The SMA mixture shall be designed according to the following additional Illinois Modified AASHTO references listed below, except as modified herein.

AASHTO M 325	Standard Specification for Designing Stone Matrix Asphalt (SMA)
AASHTO R 46	Standard Practice for Designing Stone Matrix Asphalt (SMA)
AASHTO T 305	Determination of Draindown Characteristics in Uncompacted Mixtures”

Revise Article 1030.04(a)(1) of the Standard Specifications to read:

“(1) High ESAL Mixtures. The Job Mix Formula (JMF) shall fall within the following limits.”

“High ESAL, MIXTURE COMPOSITION (% PASSING) ^{1/}										
Sieve Size	IL-25.0 mm		IL-19.0 mm		IL-12.5 mm		IL-9.5 mm		SMA ^{4/}	
	min	max	min	max	min	max	min	max	min	max
1 1/2 in. (37.5 mm)		100								
1 in. (25 mm)	90	100		100						
3/4 in. (19 mm)		90	82	100		100				100
1/2 in. (12.5 mm)	45	75	50	85	90	100		100	90	99
3/8 in. (9.5 mm)						89	90	100	50	85
#4 (4.75 mm)	24	42 ^{2/}	24	50 ^{2/}	28	65	28	65	20	40
#8 (2.36 mm)	16	31	20	36	28	48 ^{3/}	28	48 ^{3/}	16	24 ^{5/}
#16 (1.18 mm)	10	22	10	25	10	32	10	32		
#50 (300 μm)	4	12	4	12	4	15	4	15		
#100 (150 μm)	3	9	3	9	3	10	3	10		
#200 (75 μm)	3.0	6.0	3.0	6.0	4.0	6.0	4.0	6.0	8.0	11.0 ^{6/}
Ratio Dust/Asphalt Binder		1.0		1.0		1.0		1.0		

1/ Based on percent of total aggregate weight.

2/ The mixture composition shall not exceed 40 percent passing the #4 (4.75 mm) sieve for binder courses with Ndesign ≥ 90.

3/ The mixture composition shall not exceed 40 percent passing the #8 (2.36 mm) sieve for surface courses with Ndesign ≥ 90.

4/ The maximum percent passing the 20 μm sieve shall be ≤3 percent.

5/ When establishing the Adjusted Job Mix Formula (AJMF) the #8 (2.36 mm) sieve shall not be adjusted above 24 percent.

6/ Additional minus No. 200 (0.075 mm) material required by the mix design shall be mineral filler."

Add the following to Article 1030.04(b) of the Standard Specifications:

"(4) SMA Mixtures. The mix design shall meet the SMA Mixture Specifications for SGC listed in AASHTO M 325 except as listed below:

ESAL's (million)	Ndesign	Design Air Voids Target %	Voids in the Mineral Aggregate (VMA), % min.
≤ 10	50 ^{1/}	4.0	16.0
> 10	80 ^{2/}	4.0	17.0

1/ Coarse aggregate shall be limestone, dolomite, crushed gravel, diabase, granite, quartzite, sandstone, or steel slag.

2/ Coarse aggregate shall be crushed gravel, diabase, granite, quartzite, sandstone, or steel slag."

Plant Requirements.

Add the following to Article 1102.01(a) of the Standard Specifications:

"(13) Requirements for SMA.

- a. Mineral Filler. When producing SMA, the mineral filler system shall accurately proportion the large amounts of mineral filler required for the mixture. Alteration or adjustment of the current system may be required. Mineral filler shall not be stored in the same silo as collected dust.

Only dust collected during the production of SMA may be returned to the SMA mixture. Any additional minus No. 200 (0.075 mm) material needed to produce the SMA shall be mineral filler meeting the requirements stated herein. Mineral filler shall not be collected dust.

- b. Stabilizing Additive. Adequate dry storage shall be provided for the stabilizing fiber additive. A separate feed system shall be provided to proportion the fiber into the mixture uniformly and in desired quantities. The feed system shall be interlocked with the aggregate feed or weigh system to maintain the correct proportions for all rates of production and batch sizes. The proportion of fibers shall be controlled at all times within ± ten percent of the amount of fibers required. The fiber system shall provide in-process monitoring consisting of either a digital display of output or a printout of the feedrate, in pounds per minute. Flow indicators or sensing devices for the fiber system shall be provided

and interlocked with plant controls so mix production shall be interrupted if fiber introduction fails, or if the output rate is not within the specified tolerances.

1. Batch Plant. Stabilizing additive shall be pneumatically added through a separate inlet directly into the weigh hopper above the pugmill. The addition of fiber shall be timed to occur during the hot aggregate charging of the hopper. Adequate mixing time will be required to ensure proper blending of the aggregate and fiber additive. Both the wet and dry mixing times shall each be increased a minimum of five seconds beyond the standard mixing time. The actual mixing time increase shall be determined by the Engineer based on individual plant characteristics. If concentrations of mastic (fiber, AC and fines) are visible behind the paver the batch size shall be reduced in ten percent increments until the problem is alleviated.
2. Drum Mix Plant. Stabilizing additive shall be introduced using specialized equipment to mix the asphalt cement with loose fiber at the time of introduction into the drum mixer. This equipment shall be approved by the Engineer. Care shall be taken to ensure the loose fiber does not become entrained in the exhaust system of the plant.

A manufacturer's representative for the fiber and fiber equipment shall be present for the fiber system calibration and mixture startup and shall be available at all times during production and lay-down of the mix.

- c. Hot-mix Storage. SMA mixtures containing steel slag coarse aggregate shall have a combined silo storage time plus haul time not less than 1 1/2 hours.
- d. Production Rate. The Bureau of Materials and Physical Research will establish the maximum production rate for SMA based items such as the plant's ability to (1) add mineral filler consistently within 0.3 percent of the target by total weight of mix and (2) thoroughly disperse the stabilizing additive."

QC/QA.

Revise Article 1030.05(d)(4) of the Standard Specifications to read:

“(4) Control Limits. Target values shall be determined by applying adjustment factors to the AJMF where applicable. The target values shall be plotted on the control charts within the following control limits.

CONTROL LIMITS					
Parameter	High ESAL Low ESAL		SMA		All Other
	Individual Test	Moving Avg. of 4	Individual Test	Moving Avg. of 4	Individual Test
% Passing: ^{1/}					
1/2 in. (12.5 mm)	± 6 %	± 4 %	± 6 %	± 4 %	± 15 %
3/8 in. (9.5 mm)			± 4%	± 3%	
No. 4 (4.75 mm)	± 5 %	± 4 %	± 5 %	± 4 %	± 10 %
No. 8 (2.36 mm)	± 5 %	± 3 %	± 4%	± 2%	
No. 30 (600 µm)	± 4 %	± 2.5 %	± 4 %	± 2.5 %	
Total Dust Content No. 200 (75 µm)	± 1.5 %	± 1.0 %			± 2.5 %
Asphalt Binder Content	± 0.3 %	± 0.2 %	± 0.2%	± 0.1%	± 0.5 %
Voids	± 1.2 %	± 1.0 %	± 1.2%	± 1.0%	± 1.2 %

1/ Based on washed ignition oven

DENSITY CONTROL LIMITS		
Mixture Composition	Parameter	Individual Test
IL-9.5, IL-12.5	N _{design} ≥ 90	92.0 – 96.0 %
IL-9.5, IL-9.5L, IL-12.5	N _{design} < 90	92.5 – 97.4 %
IL-19.0, IL-25.0	N _{design} ≥ 90	93.0 – 96.0 %
IL-19.0, IL-19.0L, IL-25.0	N _{design} < 90	93.0 – 97.4 %
SMA	N _{design} = 50 & 80	93.5 – 97.4 %
All Other	N _{design} = 30	93.0 ^{1/} - 97.4 %

1/
when placed as first lift on an unimproved subgrade.”

92.0 percent

Replace the first and second paragraphs of Article 1030.06(a) of the Standard Specifications with the following:

“(a) High ESAL, Low ESAL and SMA Mixture.

During the mixture start-up for High or Low ESAL mixture the Contractor shall follow the QC/QA document “Hot-Mix Asphalt QC/QA Start-Up Procedures”. At the start of High or Low ESAL mixture production, QC/QA mixture start-up will be required for the following situations: at the beginning of production of a new mixture design, at the beginning of

each production season, and at every plant utilized to produce mixtures, regardless of the mix.

For SMA, a preliminary test strip shall be constructed according to the document "Off-Site Preliminary Test Strip and Modified Start-Up Procedures" at an off-site location approved by the Engineer to determine mix properties, density and laydown characteristics. At the start of SMA production, a modified start-up shall be performed on the jobsite. The modified start-up shall not begin until the Engineer has reviewed, evaluated, and approved the mixture based on the results from the off-site preliminary test strip."

Revise the table in Article 1030.06(a) of the Standard Specifications to read:

"Parameter	Adjustment
1/2 in. (12.5 mm)	± 5.0 %
No. 4 (4.75 mm)	± 4.0 %
No. 8 (2.36 mm)	± 3.0 %
No. 30 (600 µm)	1/
No. 200 (75 µm)	1/
Asphalt Binder Content	± 0.3 % ^{2/}

1/ In no case shall the target for the amount passing be greater than the JMF.

2/ For SMA, the asphalt binder content shall not be adjusted by more than 0.2 percent."

Transportation.

Add the following after the first paragraph of Article 1030.08 of the Standard Specifications:

"(d) The mixture being placed is SMA."

Construction Requirements.

Add the following paragraph after the first paragraph of Article 406.06(b) of the Standard Specifications:

"Additional temperature requirements for SMA. SMA mixture shall be placed on a dry surface when the temperature of the roadbed is above 50 °F (10 °C). The mixture shall be placed at a minimum mixture temperature of 310° F (154° C) when using SBS PG76-28 and 300 °F (149 °C) when using SBS PG76-22. The mixture temperature shall be measured immediately behind the paver screed."

Revise the last sentence of the third paragraph of Article 406.06(e) of the Standard Specifications to read:

"In no case shall the speed of the paver exceed 50 ft (15 m) per minutes for High and Low ESAL mixes or 30 ft (9 m) per minute for SMA."

Revise Table 1 in Article 406.07(a) of the Standard Specifications to read:

"TABLE 1 - MINIMUM ROLLER REQUIREMENTS FOR HMA
--

	Breakdown Roller (one of the following)	Intermediate Roller	Final Roller (one or more of the following)	Density Requirement
Level Binder: (When the density requirements of Article 406.05(c) do not apply.)	P ^{3/}	--	V _S , P ^{3/} , T _B , T _F , 3W	To the satisfaction of the Engineer.
Binder and Surface ^{1/} Level Binder ^{1/} : (When the density requirements of Article 406.05(c) apply.)	V _D , P ^{3/} , T _B , 3W	P ^{3/}	V _S , T _B , T _F	As specified in Articles: 1030.05(d)(3), (d)(4), and (d)(7).
SMA ^{4/}	T _B ^{5/}	--	T _F	
Bridge Decks ^{2/}	T _B	--	T _F	As specified in Articles: 582.05 and 582.06.

- 1/ If the average delivery at the job site is 85 ton/hr (75 metric ton/hr) or less, any roller combination may be used provided it includes a steel wheeled roller and the required density and smoothness is obtained.
- 2/ One T_B may be used for both breakdown and final rolling on bridge decks 300 ft (90 m) or less in length, except when the air temperature is less than 60 °F (15 °C).
- 3/ A vibratory roller (V_D) may be used in lieu of the pneumatic-tired roller on mixtures containing polymer modified asphalt binder.
- 4/ Pneumatic-tired and vibratory rollers will not be allowed. Rollers shall be operated at a uniform speed not to exceed 3 mph (5 km/h) with the drive roll nearest the paver.
- 5/ The Contractor shall provide a minimum of two steel-wheeled tandem rollers for breakdown (T_B). The breakdown rollers shall maintain an effective rolling distance of not more than 150 ft (45 m) behind the paver."

Prepaving Conference. A prepaving conference shall be held a minimum of one week prior to the start of mix production. Those in attendance shall include the QC Manager, Construction Supervising Field Engineer, Resident Engineer, Mixture Control Engineer, BMPR representative, fiber supplier representative, asphalt binder supplier representative, as well as plant, paver and roller operators.

Basis of Payment. This work will be measured and paid for according to Article 406.13 and 406.14 of the Standard Specifications at the contract unit price per metric ton (ton) for POLYMERIZED HOT-MIX ASPHALT SURFACE COURSE, STONE MATRIX ASPHALT, of the N design specified; and POLYMERIZED HOT-MIX ASPHALT BINDER COURSE, STONE MATRIX ASPHALT, of the N design specified.

The preliminary test strip will be paid for at the contract unit price per each for PRELIMINARY TEST STRIP, which price shall include the 272 metric tons (300 tons) of mix as well as the appropriate testing, provided the bituminous mixture is placed within the JMF tolerances.

Designer Note: Insert into all contracts with HMA items.

HOT-MIX ASPHALT - DENSITY TESTING OF LONGITUDINAL JOINTS (BDE)

Effective: January 1, 2010

Description. This work shall consist of testing the density of longitudinal joints as part of the quality control/quality assurance (QC/QA) of hot-mix asphalt (HMA). Work shall be according to Section 1030 of the Standard Specifications except as follows.

Quality Control/Quality Assurance (QC/QA). Delete the second and third sentence of the third paragraph of Article 1030.05(d)(3) of the Standard Specifications.

Add the following paragraphs to the end of Article 1030.05(d)(3) of the Standard Specifications:

“Longitudinal joint density testing shall be performed at each random density test location. Longitudinal joint testing shall be located at a distance equal to the lift thickness or a minimum of 2 in. (50 mm), from each pavement edge. (i.e. for a 4 in. (100 mm) lift the near edge of the density gauge or core barrel shall be within 4 in. (100 mm) from the edge of pavement.) Longitudinal joint density testing shall be performed using either a correlated nuclear gauge or cores.

- a. Confined Edge. Each confined edge density shall be represented by a one-minute nuclear density reading or a core density and shall be included in the average of density readings or core densities taken across the mat which represents the Individual Test.
- b. Unconfined Edge. Each unconfined edge joint density shall be represented by an average of three one-minute density readings or a single core density at the given density test location and shall meet the density requirements specified herein. The three one-minute readings shall be spaced ten feet apart longitudinally along the unconfined pavement edge and centered at the random density test location.”

Revise the Density Control Limits table in Article 1030.05(d)(4) of the Standard Specifications to read:

"Mixture Composition	Parameter	Individual Test (includes confined edges)	Unconfined Edge Joint Density Minimum
IL-9.5, IL-12.5	Ndesign ≥ 90	92.0 – 96.0%	90.0%
IL-9.5, IL-9.5L, IL-12.5	Ndesign < 90	92.5 – 97.4%	90.0%
IL-19.0, IL-25.0	Ndesign ≥ 90	93.0 – 96.0%	90.0%
IL-19.0, IL-19.0L, IL-25.0	Ndesign < 90	93.0 – 97.4%	90.0%
SMA	Ndesign = 50 & 80	93.5 – 97.4%	91.0%
All Other	Ndesign = 30	93.0 - 97.4%	90.0%”

Designer Note: Use on any contract with Highway Standards 609001, 609006, or 610001. Steel is no longer used by industry.

FRAMES AND GRATES (BDE)

Effective: January 1, 2010

Revise Article 609.02 of the Standard Specifications to read:

“**609.02 Materials.** Materials shall be according to the following.

Item	Article/Section
(a) Portland Cement Concrete	1020
(b) Gray Iron Castings	1006.14
(c) Ductile Iron Castings	1006.15
(d) Reinforcement Bars	1006.10
(e) Bedding Layer (Note 1)	1004.01
(f) Precast Concrete Bridge Approach Drains	1042

Note 1. Gradation CA 6, CA 10, or CA 12 of D quality or better.”

Revise Article 609.04 of the Standard Specifications to read:

“**609.04 Frames and Grates.** Cast iron frames and grates shall be used. Grates shall seat firmly in the frame.”

63107

63107

Designer Note: Use on any contracts using Highway Standard 631031.

TRAFFIC BARRIER TERMINAL, TYPE 6 (BDE)

Effective: January 1, 2010

Delete the fourth paragraph of Article 631.07 of the Standard Specifications.

Designer Note: Insert into all contracts requiring truck mounted or trailer mounted attenuators. These Highway Standards include 701311 and 701426.

TRUCK MOUNTED/TRAILER MOUNTED ATTENUATORS (BDE)

Effective: January 1, 2010

Revise Article 701.03(k) of the Standard Specifications to read:

“ (k)Truck Mounted/Trailer Mounted Attenuators1106.02”

Revise Article 701.15(h) of the Standard Specifications to read:

“ (h) Truck Mounted/Trailer Mounted Attenuators (TMA). TMA units shall have a roll ahead distance in the event of an impact. The TMA shall be between 100 and 200 ft (30 and 60 m) behind the vehicle ahead or the workers. This distance may be extended by the Engineer.

TMA host vehicles shall have the parking brake engaged when stationary.

The driver and passengers of the TMA host vehicle should exit the vehicle if the TMA is to remain stationary for 15 minutes or more in duration.”

Revise Article 1106.02(g) of the Standard Specifications to read:

“ (g) Truck Mounted/Trailer Mounted Attenuators. The attenuator shall be a NCHRP 350 approved unit for Test Level 3. Test Level 2 may be used as directed by the Engineer for normal posted speeds less than or equal to 45 mph.”

Designer Note: Insert into all HMA contracts.

HOT-MIX ASPHALT – DROP-OFFS (BDE)

Effective: January 1, 2010

Revise the third paragraph of Article 701.07 of the Standard Specifications to read:

“At locations where construction operations result in a differential in elevation exceeding 3 in. (75 mm) between the edge of pavement or edge of shoulder within 3 ft (900 mm) of the edge of the pavement and the earth or aggregate shoulders, Type I or II barricades or vertical panels shall be placed at 100 ft (30 m) centers on roadways where the posted speed limit is 45 mph or greater and at 50 ft (15 m) centers on roadways where the posted speed limit is less than 45 mph.”

Designer Note: Insert into contracts with pavement patching.

PAVEMENT PATCHING (BDE)

Effective: January 1, 2010

Revise the first sentence of the second paragraph of Article 701.17(e)(1) of the Standard Specifications to read:

“In addition to the traffic control and protection shown elsewhere in the contract for pavement, two devices shall be placed immediately in front of each open patch, open hole, and broken pavement where temporary concrete barriers are not used to separate traffic from the work area.”

Designer Note: Insert into contracts using an organic zinc-rich paint system.

ORGANIC ZINC-RICH PAINT SYSTEM (BDE)

Effective: November 1, 2001

Revised: January 1, 2010

Add the following to Section 1008 of the Standard Specifications:

"1008.05 Organic Zinc-Rich Paint System. The organic zinc-rich paint system shall consist of an organic zinc-rich primer, an epoxy or urethane intermediate coat, and aliphatic urethane finish coats. It is intended for use over blast-cleaned steel when three-coat shop applications are specified. The system is also suitable for field painting blast-cleaned existing structures.

The coating system shall be evaluated for performance through the National Transportation Product Evaluation Program (NTPEP) for Structural Steel Coatings following the requirements of AASHTO R 31, and shall meet the performance criteria listed herein. After successful NTPEP testing, the coatings shall be submitted to the Illinois Department of Transportation, Bureau of Materials and Physical Research, for qualification and acceptance testing.

(a) General Requirements.

- (1) Compatibility. Each coating in the system shall be supplied by the same paint manufacturer.
- (2) Toxicity. Each coating shall contain less than 0.01 percent lead in the dry film and no more than trace amounts of hexavalent chromium, cadmium, mercury or other toxic heavy metals.
- (3) Volatile Organics. The volatile organic compounds of each coating shall not exceed 2.8 lb/gal (340 g/L) as applied.

- (b) Panel Preparation for NTPEP testing. The test panels shall be prepared according to AASHTO R 31, except for the following: Test panels shall be scribed according to ASTM D 1654 with a single "X" mark centered on the panel. The rectangular dimensions of the scribe shall have a top width of 2 in. (50 mm) and a height of 4 in. (100 mm). The scribe cut shall expose the steel substrate as verified with a microscope.

(c) Zinc-Rich Primer Requirements.

- (1) Generic Type. This material shall be an organic zinc-rich epoxy or urethane primer. It shall be suitable for topcoating with epoxies, urethanes, and acrylics.
- (2) Zinc Dust. The zinc dust pigment shall comply with ASTM D 520, Type II.

- (3) Slip Coefficient. The organic zinc coating shall meet a Class B AASHTO slip coefficient (0.50 or greater) for structural steel joints using ASTM A 325 (A 325M) or A 490 (A 490M) bolts.
- (4) Adhesion. The adhesion to an abrasively blasted steel substrate shall not be less than 900 psi (6.2 MPa) when tested according to ASTM D 4541 Annex A4.
- (5) Unit Weight. The unit weight of the mixed material shall be within 0.4 lb/gal (48 kg/cu m) of the original qualification sample unit weight when tested according to ASTM D 1475.
- (6) Percent Solids by Weight of Mixed Primer. The percent solids by weight for the mixed material shall be a minimum of 70 percent and shall not vary more than ± 2 percentage points from the percent solids by weight of the original qualification samples when tested according to ASTM D 2369.
- (7) Percent Solids by Weight of Vehicle Component. The percent solids by weight of the vehicle component shall not vary more than ± 2 percentage points from the percent solids by weight of the original qualification samples when tested according to ASTM D 2369.
- (8) Viscosity. The viscosity of the mixed material shall not vary more than ± 10 Krebs Units from the original qualification sample viscosity when tested according to ASTM D 562 at 77 °F (25 °C).
- (9) Dry Set to Touch. The mixed material when applied at 6 mils (150 microns) wet film thickness shall have a dry set to touch of 30 minutes or less when tested according to ASTM D 1640 at 77°F (25 °C).
- (10) Pot Life. After sitting eight hours at 77°F (25 °C), the mixed material shall not show curdling, gelling, gassing, or hard caking.

(d) Intermediate Coat Requirements.

- (1) Generic Type. This material shall be an epoxy or urethane. It shall be suitable as an intermediate coat over inorganic and organic zinc primers and compatible with acrylic, epoxy, and polyurethane topcoats.
- (2) Color. The color of the intermediate coat shall be white, off-white, or beige.
- (3) Unit Weight. The unit weight of the mixed material and the unit weight of the individual components shall be within 0.20 lb/gal (24 kg/cu m) of the original qualification sample unit weights when tested according to ASTM D 1475.
- (4) Percent Solids by Weight. The percent solids by weight for the mixed material shall not vary more than ± 2 percentage points from the percent solids by weight of the original qualification samples when tested according to ASTM D 2369.
- (5) Dry Time. The mixed material shall be dry to touch in two hours and dry hard in eight hours when applied at 10 mils (255 microns) wet film thickness and tested according to ASTM D 1640.

- (6) Viscosity. The viscosity of the mixed material shall not vary more than ± 10 Krebs Units from the original qualification samples when tested according to ASTM D 562 at 77 °F (25 °C).
- (7) Pot Life. After sitting two hours at 77°F (25 °C), the mixed material shall not show curdling, gelling, gassing, or hard caking.

(e) Urethane Finish Coat Requirements.

- (1) Generic Type. This material shall be an aliphatic urethane. It shall be suitable as a topcoat over epoxies and urethanes.
- (2) Color and Hiding Power. The finish coat shall match Munsell Glossy Color 7.5G 4/8 Interstate Green, 2.5YR 3/4 Reddish Brown, 10B 3/6 Blue, or 5B 7/1 Gray. The color difference shall not exceed 3.0 Hunter Delta E Units. Color difference shall be measured by instrumental comparison of the designated Munsell standard to a minimum dry film thickness of 3 mils (75 microns) of sample coating produced on a test panel according to ASTM D 823, Practice E, Hand-Held, Blade Film Application. Color measurements shall be determined on a spectrophotometer with 45 degrees circumferential/zero degrees geometry, illuminant C, and two degrees observer angle. The spectrophotometer shall measure the visible spectrum from 380-720 nanometers with a wavelength interval and spectral bandpass of 10 nanometers.
- (3) Contrast Ratio. The contrast ratio of the finish coat applied at 3 mils (75 microns) dry film thickness shall not be less than 0.99 when tested according to ASTM D 2805.
- (4) Weathering Resistance. Test panels shall be aluminum alloy measuring 12 x 4 in. (300 x 100 mm) prepared according to ASTM D 1730 Type A, Method 1 Solvent Cleaning. A minimum dry film thickness of 3 mils (75 microns) of finish coat shall be applied to three test panels according to ASTM D 823, Practice E, Hand Held Blade Film Application. The coated panels shall be cured at least 14 days at 75 °F \pm 2 °F (24 °C \pm 1 °C) and 50 \pm 5 percent relative humidity. The panels shall be subjected to 300 hours of accelerated weathering using the light and water exposure apparatus (fluorescent UV - condensation type) as specified in ASTM G 53-96 and ASTM G 154 (equipped with UVB-313 lamps). The cycle shall consist of eight hours UV exposure at 140 °F (60 °C) followed by four hours of condensation at 104 °F (40 °C). After exposure, rinse the panel with clean water; allow to dry at room temperature for one hour. The exposed panels shall not show a color change of more than 3 Hunter Delta E Units.
- (5) Dry Time. The mixed material shall be dry to touch in two hours and dry hard in six hours when applied at 6 mils (150 microns) wet film thickness and tested according to ASTM D 1640.

(f) Three Coat System Requirements.

- (1) Finish Coat Color. For NTPEP testing purposes, the color of the finish coat shall match the latest applicable AASHTO R 31 specified color.

- (2) Salt Fog. When tested according to ASTM B 117 and evaluated according to AASHTO R 31, the paint system shall exhibit no spontaneous delamination and not exceed the following acceptance levels after scraping after 5,000 hours of salt fog exposure:

Salt Fog Acceptance Criteria		
Blister Criteria	Rust Criteria	
Conversion Value	Maximum Creep	Average Creep
9	4 mm	2 mm

- (3) Cyclic Exposure. When tested according to ASTM D 5894 and evaluated according to AASHTO R 31, the paint system shall exhibit no spontaneous delamination and not exceed the following acceptance levels after 5,000 hours of cyclic exposure:

Cyclic Exposure Acceptance Criteria		
Blister Criteria	Rust Criteria	
Conversion Value	Maximum Creep	Average Creep
9	7 mm	4 mm

- (4) Abrasion. The abrasion resistance shall be evaluated according to ASTM D 4060 using a Taber Abrader with a 2.20 lb (1000 gram) load and CS 17 wheels. The duration of the test shall be 1,000 cycles. The loss shall be calculated by difference and be less than 0.00049 lb (220 mgs).
- (5) Adhesion. The adhesion to an abrasively blasted steel substrate shall not be less than 900 psi (6.2 MPa) when tested according to ASTM D 4541 Annex A4.
- (6) Freeze Thaw Stability. There shall be no reduction of adhesion, which exceeds the test precision, after 30 days of freeze/thaw/immersion testing. One 24 hour cycle shall consist of 16 hours of approximately -22 °F (-30 °C) followed by four hours of thawing at 122 °F (50 °C) and four hours tap water immersion at 77 °F (25 °C). The test panels shall remain in the freezer mode on weekends and holidays.
- (g) Sampling, Testing, Acceptance, and Certification. Sampling, testing, acceptance, and certification of the coating system shall be according to Article 1008.01."

Designer Note: Insert into all projects requiring a moisture cured urethane paint system. Consult District Paint Inspector.

MOISTURE CURED URETHANE PAINT SYSTEM (BDE)

Effective: November 1, 2006

Revised: January 1, 2010

Add the following to Section 1008 of the Standard Specifications:

"1008.06 Moisture Cured Urethane Paint System. The moisture cured urethane paint system shall consist of an aromatic moisture cured urethane primer, an aromatic moisture cured urethane intermediate coat, and aliphatic moisture cured urethane finish coats. It is intended for field painting blast-cleaned existing structures.

(a) General Requirements.

- (1) Compatibility. Each coating in the system shall be supplied by the same paint manufacturer.
- (2) Toxicity. Each coating shall contain less than 0.01 percent lead in the dry film and no more than trace amounts of hexavalent chromium, cadmium, mercury or other toxic heavy metals.
- (3) Volatile Organics. The volatile organic compounds of each coating shall not exceed 2.8 lb/gal (340 g/L) as applied.

(b) Test Panel Preparation.

- (1) Substrate and Surface Preparation. Test panels shall be ASTM A 36, hot-rolled steel measuring 4 x 6 in. (100 x 150 mm). Panels shall be blast-cleaned per SSPC-SP5 white metal condition using recyclable metallic abrasive according to SSPC AB-3. The abrasive shall be a 60/40 mix of shot and grit. The shot shall be an SAE shot number S230 and the grit an SAE number G40. Hardness of the shot and grit shall be Rockwell C45. The anchor profile shall be 1.5-2.5 mils (40-65 microns) measured according to ASTM D 4417, Method C.
- (2) Application and Curing. All coatings shall be spray applied at the manufacturer's recommended film thickness. The coated panels shall be cured at least 30 days and not more than 45 days at 77 °F ± 2 °F (25 °C ± 2 °C) and 65 ± 5 percent relative humidity.
- (3) Scribing. The test panels shall be scribed according to ASTM D 1654 with a single "X" mark centered on the panel. The rectangular dimensions of the scribe shall have a top width of 2 in. (50 mm) and a height of 4 in. (100 mm). The scribe cut shall expose the steel substrate as verified with a microscope.

(4) Number of Panels. All testing shall be performed on triplicate panels.

(c) Zinc-Rich Primer Requirements.

(1) Generic Type. This material shall be a single component zinc-rich aromatic moisture cured urethane primer. It shall be suitable for topcoating with urethanes.

(2) Zinc Dust. The zinc dust pigment shall be according to ASTM D 520, Type II.

(3) Slip Coefficient. The organic zinc coating shall meet a Class B AASHTO slip coefficient (0.50 or greater) for structural steel joints using ASTM A 325 (A 325M) or A 490 (A 490M) bolts.

(4) Adhesion. The adhesion to an abrasively blasted steel substrate shall not be less than 900 psi (6.2 MPa) when tested according to ASTM D 4541 Annex A4.

(d) Intermediate Coat Requirements.

(1) Generic Type. This material shall be a single component aromatic moisture cured urethane. It shall be suitable as an intermediate coat over the primer and compatible with the finish coat.

(2) Color. The color of the intermediate coat shall provide a distinct contrast between the primer and the finish coat.

(e) Urethane Finish Coat Requirements.

(1) Generic Type. This material shall be a single component aliphatic moisture cured urethane. It shall be suitable as a topcoat over the intermediate coat.

(2) Color and Hiding Power. The finish coat shall match Munsell Glossy Color 7.5G 4/8 Interstate Green, 2.5YR 3/4 Reddish Brown, 10B 3/6 Blue, or 5B 7/1 Gray. The color difference shall not exceed 3.0 Hunter Delta E Units. Color difference shall be measured by instrumental comparison of the designated Munsell standard to a minimum dry film thickness of 3 mils (75 microns) of sample coating produced on a test panel according to ASTM D 823, Practice E, Hand-Held, Blade Film Application. Color measurements shall be determined on a spectrophotometer with 45 degrees circumferential/zero degrees geometry, illuminant C, and two degrees observer angle. The spectrophotometer shall measure the visible spectrum from 380-720 nanometers with a wavelength interval and spectral bandpass of 10 nanometers.

The contrast ratio of the finish coat at 3 mils (75 microns) dry film thickness shall not be less than 0.99 when tested according to ASTM D 2805.

(3) Accelerated Weathering Resistance. Test panels shall be aluminum alloy measuring 12 x 4 in. (300 x 100 mm) prepared according to ASTM D 1730 Type A, Method 1 Solvent Cleaning. A minimum dry film thickness of 3 mils (75 microns) of finish coat shall be applied to three test panels according to ASTM D 823, Practice E, Hand Held Blade Film Application. The coated panels shall be cured at

least 30 days and not more than 45 days at 77 °F ± 2 °F (25 °C ± 2 °C) and 65 ± 5 percent relative humidity. The panels shall be subjected to 300 hours of accelerated weathering using the light and water exposure apparatus (fluorescent UV - condensation type) as specified in ASTM G 53-96 and ASTM G 154 (equipped with UVB-313 lamps). The cycle shall consist of eight hours UV exposure at 140 °F (60 °C) followed by four hours of condensation at 104 °F (40 °C). After exposure, the panel shall be rinsed with clean water and allowed to dry at room temperature for one hour. The exposed panels shall not show a color change of more than 3 Hunter Delta E Units.

(f) Three Coat System Requirements.

- (1) Finish Coat Color. For testing purposes, the color of the finish coat shall match Federal Standard No 595, color chip 14062 (green).
- (2) Salt Fog. When tested according to ASTM B 117 and evaluated according to AASHTO R 31, the paint system shall exhibit no spontaneous delamination and not exceed the following acceptance levels after 5,000 hours of salt fog exposure.

Salt Fog Acceptance Criteria (max.)		
Blister Conversion Value	Rust Criteria	
After 4000 Hours	Maximum Creep	Average Creep
10	6 mm	2 mm

- (3) Cyclic Exposure. When tested according to ASTM D 5894 and evaluated according to AASHTO R 31, the paint system shall exhibit no spontaneous delamination and not exceed the following acceptance levels after 5,040 hours of cyclic exposure.

Cyclic Exposure Acceptance Criteria (max.)		
Blister Conversion Value	Rust Criteria	
	Maximum Creep	Average Creep
10	13 mm	7 mm

- (4) Adhesion. The adhesion to an abrasively blasted steel substrate shall not be less than 900 psi (6.2 MPa) when tested according to ASTM D 4541 Annex A4.
 - (5) Freeze Thaw Stability. There shall be no reduction of adhesion, which exceeds the test precision, after 30 days of freeze/thaw/immersion testing. One 24 hour cycle shall consist of 16 hours of approximately -22 °F (-30 °C) followed by four hours of thawing at 122 °F (50 °C) and four hours tap water immersion at 77 °F (25 °C). The test panels shall remain in the freezer mode on weekends and holidays.
- (g) Qualification Samples and Tests. The manufacturer shall supply, to an independent test laboratory and to the Department, samples of the moisture cured zinc-rich urethane primer, moisture cured urethane intermediate coat, and moisture cured aliphatic urethane finish coats for evaluation. Prior to approval and use, the manufacturer shall submit a notarized certification of the independent laboratory, together with results of all tests, stating that these materials meet the requirements as set forth herein. The certified test report shall state lots tested, manufacturer's name, product names, and dates of manufacture. New certified test results and samples for testing by the

Department shall be submitted any time the manufacturing process or paint formulation is changed. All costs of testing, other than tests conducted by the Department, shall be borne by the manufacturer.

- (h) Acceptance Samples and Certification. A 1 qt (1 L) sample of each lot of paint produced for use on state or local agency projects shall be submitted to the Department for testing, together with a manufacturer's certification. The certification shall state that the formulation for the lot represented is essentially identical to that used for qualification testing. All acceptance samples shall be witnessed by a representative of the Illinois Department of Transportation. The moisture cured zinc-rich primer, moisture cured urethane intermediate coat, and moisture cured aliphatic urethane finish coat shall not be used until tests are completed and they have met the requirements as set forth herein."

Designer Note: Use on all projects with HMA.

HOT-MIX ASPHALT – PLANT TEST FREQUENCY (BDE)

Effective: April 1, 2008
 Revised: January 1, 2010

Revise the table in Article 1030.05(d)(2)a. of the Standard Specifications to read:

"Parameter	Frequency of Tests	Frequency of Tests	Test Method See Manual of Test Procedures for Materials
	High ESAL Mixture Low ESAL Mixture	All Other Mixtures	
Aggregate Gradation % passing sieves: 1/2 in. (12.5 mm), No. 4 (4.75 mm), No. 8 (2.36 mm), No. 30 (600 μm) No. 200 (75 μm) Note 1.	1 washed ignition oven test on the mix per half day of production Note 4.	1 washed ignition oven test on the mix per day of production Note 4.	Illinois Procedure
Asphalt Binder Content by Ignition Oven Note 2.	1 per half day of production	1 per day	Illinois-Modified AASHTO T 308
VMA Note 3.	Day's production ≥ 1200 tons: 1 per half day of production Day's production < 1200 tons: 1 per half day of production for first 2 days and 1 per day thereafter (first sample of the day)	N/A	Illinois Modified AASHTO R 35
Air Voids Bulk Specific Gravity of Gyratory Sample	Day's production ≥ 1200 tons: 1 per half day of production Day's production < 1200 tons: 1 per half day of production for first 2 days and 1 per day thereafter (first sample of the day)	1 per day	Illinois-Modified AASHTO T 312

"Parameter	Frequency of Tests	Frequency of Tests All Other Mixtures	Test Method See Manual of Test Procedures for Materials
	High ESAL Mixture Low ESAL Mixture		
Maximum Specific Gravity of Mixture	Day's production \geq 1200 tons: 1 per half day of production	1 per day	Illinois-Modified AASHTO T 209
	Day's production < 1200 tons: 1 per half day of production for first 2 days and 1 per day thereafter (first sample of the day)		

Note 1. The No. 8 (2.36 mm) and No. 30 (600 μ m) sieves are not required for All Other Mixtures.

Note 2. The Engineer may waive the ignition oven requirement for asphalt binder content if the aggregates to be used are known to have ignition asphalt binder content calibration factors which exceed 1.5 percent. If the ignition oven requirement is waived, other Department approved methods shall be used to determine the asphalt binder content.

Note 3. The G_{sb} used in the voids in the mineral aggregate (VMA) calculation shall be the same average G_{sb} value listed in the mix design.

Note 4. The Engineer reserves the right to require additional hot bin gradations for batch plants if control problems are evident."

103005

1030.05

Designer Note: Insert into all HMA contracts.

HOT-MIX ASPHALT – QC/QA ACCEPTANCE CRITERIA (BDE)

Effective: January 1, 2010

Revise Article 1030.05(f)(3) of the Standard Specifications to read:

“(3) Department assurance tests for voids, field VMA, and density.”

Designer Note: Insert into all projects with Hot-Mix Asphalt (HMA). It provides the following:

1. Allow a new RAP stockpile referred to as conglomerate 3/8 which requires conglomerate RAP to be processed to minus 3/8 inch.
2. Allow conglomerate 3/8 RAP to be used in additional applications at higher rates as stated in the table below.
3. Allow RAP to be used in Polymer HMA.
4. Require HMA plants to have automated recordation when producing HMA mixes containing RAP.
5. Require HMA plants to utilize Positive Dust Control systems when producing mixes containing conglomerate 3/8 RAP.

RECLAIMED ASPHALT PAVEMENT (RAP) (BDE)

Effective: January 1, 2007

Revised: January 1, 2010

In Article 1030.02(g), delete the last sentence of the first paragraph in (Note 2).

Revise Section 1031 of the Standard Specifications to read:

"SECTION 1031. RECLAIMED ASPHALT PAVEMENT

1031.01 Description. Reclaimed asphalt pavement (RAP) is reclaimed asphalt pavement resulting from cold milling or crushing of an existing dense graded hot-mix asphalt (HMA) pavement. The Contractor shall supply written documentation that the RAP originated from routes or airfields under federal, state, or local agency jurisdiction.

1031.02 Stockpiles. The Contractor shall construct individual, sealed RAP stockpiles meeting one of the following definitions. No additional RAP shall be added to the pile after the pile has been sealed. Stockpiles shall be sufficiently separated to prevent intermingling at the base. Stockpiles shall be identified by signs indicating the type as listed below (i.e. "Homogeneous Surface").

Prior to milling, the Contractor shall request the District to provide verification of the quality of the RAP to clarify appropriate stockpile.

- (a) Fractionated RAP (FRAP). FRAP shall consist of RAP from Class I, Superpave (High ESAL), HMA (High ESAL), or equivalent mixtures. The coarse aggregate in FRAP shall be crushed aggregate and may represent more than one aggregate type and/or quality but shall be at least C quality. All FRAP shall be fractionated prior to testing by screening into a minimum of two size fractions with the separation occurring on or between the #4 (4.75 mm) and 1/2 in. (12.5 mm) sieves. Agglomerations shall be minimized such that 100 percent of the RAP in the coarse fraction shall pass one sieve size larger than the maximum sieve size specified for the mix the RAP will be used in.

- (b) Homogeneous. Homogeneous RAP stockpiles shall consist of RAP from Class I, Superpave (High ESAL), HMA (High ESAL), or equivalent mixtures and represent: 1) the same aggregate quality, but shall be at least C quality; 2) the same type of crushed aggregate (either crushed natural aggregate, ACBF slag, or steel slag); 3) similar gradation; and 4) similar asphalt binder content. If approved by the Engineer, combined single pass surface/binder millings may be considered "homogenous" with a quality rating dictated by the lowest coarse aggregate quality present in the mixture.
- (c) Conglomerate. Conglomerate RAP stockpiles shall consist of RAP from Class I, Superpave (High ESAL), HMA (High ESAL), or equivalent mixtures. The coarse aggregate in this RAP shall be crushed aggregate and may represent more than one aggregate type and/or quality but shall be at least C quality. This RAP may have an inconsistent gradation and/or asphalt binder content prior to processing. All conglomerate RAP shall be processed prior to testing by crushing to where all RAP shall pass the 5/8 in. (16 mm) or smaller screen. Conglomerate RAP stockpiles shall not contain steel slag or other expansive material as determined by the Department.
- (d) Conglomerate "D" Quality (DQ). Conglomerate DQ RAP stockpiles shall consist of RAP from Class I, Superpave (High or Low ESAL), HMA (High or Low ESAL), or equivalent mixtures. The coarse aggregate in this RAP may be crushed or round but shall be at least D quality. This RAP may have an inconsistent gradation and/or asphalt binder content. Conglomerate DQ RAP stockpiles shall not contain steel slag or other expansive material as determined by the Department.
- (e) Non-Quality. RAP stockpiles that do not meet the requirements of the stockpile categories listed above shall be classified as "Non-Quality".

RAP/FRAP containing contaminants, such as earth, brick, sand, concrete, sheet asphalt, bituminous surface treatment (i.e. chip seal), pavement fabric, joint sealants, etc., will be unacceptable unless the contaminants are removed to the satisfaction of the Engineer. Sheet asphalt shall be stockpiled separately.

1031.03 Testing. When used in HMA, the RAP/FRAP shall be sampled and tested either during or after stockpiling.

For testing during stockpiling, washed extraction samples shall be run at the minimum frequency of one sample per 500 tons (450 metric tons) for the first 2000 tons (1800 metric tons) and one sample per 2000 tons (1800 metric tons) thereafter. A minimum of five tests shall be required for stockpiles less than 4000 tons (3600 metric tons).

For testing after stockpiling, the Contractor shall submit a plan for approval to the District proposing a satisfactory method of sampling and testing the RAP/FRAP pile either in-situ or by restockpiling. The sampling plan shall meet the minimum frequency required above and detail the procedure used to obtain representative samples throughout the pile for testing.

Before extraction, each field sample shall be split to obtain two samples of test sample size. One of the two test samples from the final split shall be labeled and stored for Department use. The Contractor shall extract the other test sample according to Department procedure. The Engineer reserves the right to test any sample (split or Department-taken) to verify Contractor test results.

Evaluation of Test Results. All of the extraction results shall be compiled and averaged for asphalt binder content and gradation and, when applicable G_{mm} . Individual extraction test results, when compared to the averages, will be accepted if within the tolerances listed below.

Parameter	FRAP/Homogeneous /Conglomerate	Conglomerate "D" Quality
1 in. (25 mm)		± 5 %
1/2 in. (12.5 mm)	± 8 %	± 15 %
No. 4 (4.75 mm)	± 6 %	± 13 %
No. 8 (2.36 mm)	± 5 %	
No. 16 (1.18 mm)		± 15 %
No. 30 (600 μm)	± 5 %	
No. 200 (75 μm)	± 2.0 %	± 4.0 %
Asphalt Binder	± 0.4 % ^{1/}	± 0.5 %
G _{mm}	± 0.03	

1/ The tolerance for FRAP shall be ± 0.3 %.

If more than 20 percent of the individual sieves are out of the gradation tolerances, or if more than 20 percent of the asphalt binder content test results fall outside the appropriate tolerances, the RAP/FRAP shall not be used in HMA unless the RAP/FRAP representing the failing tests is removed from the stockpile. All test data and acceptance ranges shall be sent to the District for evaluation.

With the approval of the Engineer, the ignition oven may be substituted for extractions according to the Illinois Test Procedure, "Calibration of the Ignition Oven for the Purpose of Characterizing Reclaimed Asphalt Pavement (RAP)".

1031.04 Quality Designation of Aggregate in RAP/FRAP.

(a) The aggregate quality of the RAP for homogenous, conglomerate, and conglomerate "D" quality stockpiles shall be set by the lowest quality of coarse aggregate in the RAP stockpile and are designated as follows.

- (1) RAP from Class I, Superpave (High ESAL)/HMA (High ESAL), or HMA (Low ESAL) IL-9.5L surface mixtures are designated as containing Class B quality coarse aggregate.
- (2) RAP from Superpave (Low ESAL)/HMA (Low ESAL) IL-19.0L binder mixture is designated as Class D quality coarse aggregate.
- (3) RAP from Class I, Superpave (High ESAL), or HMA (High ESAL) binder mixtures, bituminous base course mixtures, and bituminous base course widening mixtures are designated as containing Class C quality coarse aggregate.
- (4) RAP from bituminous stabilized subbase and BAM shoulders are designated as containing Class D quality coarse aggregate.

(b) The aggregate quality of FRAP shall be determined as follows.

Fractionated stockpiles containing plus #4 (4.75 mm) sieve coarse aggregate shall have a maximum tonnage of 5000 tons (4500 metric tons). The Contractor shall obtain a representative sample witnessed by the Engineer. The sample shall be a minimum of 50 lb (25 kg). The sample shall be extracted according to Illinois Modified AASHTO T 164 by a consultant prequalified by the Department for the specified testing.

The consultant shall submit the test results along with the recovered aggregate to the District Office. The cost for this testing shall be paid by the Contractor. The District will forward the sample to the BMPR Aggregate Lab for MicroDeval Testing, according to Illinois Modified AASHTO T 327. A maximum loss of 15.0 percent will be applied for all HMA applications."

1031.05 Use of RAP/FRAP in HMA. The use of RAP/FRAP shall be a Contractor's option when constructing HMA in all contracts. The use of RAP/FRAP in HMA shall be as follows.

- (a) Coarse Aggregate Size. The coarse aggregate in all RAP shall be equal to or less than the nominal maximum size requirement for the HMA mixture to be produced.
- (b) Steel Slag Stockpiles. RAP stockpiles containing steel slag or other expansive material, as determined by the Department, shall be homogeneous and will be approved for use in HMA (High ESAL and Low ESAL) surface mixtures only.
- (c) Use in HMA Surface Mixtures (High and Low ESAL). RAP/FRAP stockpiles for use in HMA surface mixtures (High and Low ESAL) shall be FRAP or homogeneous in which the coarse aggregate is Class B quality or better.
- (d) Use in HMA Binder Mixtures (High and Low ESAL), HMA Base Course, and HMA Base Course Widening. RAP/FRAP stockpiles for use in HMA binder mixtures (High and Low ESAL), HMA base course, and HMA base course widening shall be FRAP, homogeneous, or conglomerate, in which the coarse aggregate is Class C quality or better.
- (e) Use in Shoulders and Subbase. RAP/FRAP stockpiles for use in HMA shoulders and stabilized subbase (HMA) shall be FRAP, homogeneous, conglomerate, or conglomerate DQ.
- (f) When the Contractor chooses the RAP option, the percentage of RAP shall not exceed the amounts indicated in the table below for a given N Design.

Max RAP Percentage

HMA Mixtures ^{1/, 3/}	Maximum % RAP			
	Ndesign	Binder/Leveling Binder	Surface	Polymer Modified
30	30	30	30	10
50	25	15	15	10
70	15 / 25 ^{2/}	10 / 15 ^{2/}	10 / 15 ^{2/}	10
90	10	10	10	10
105	10	10	10	10

1/ For HMA shoulder and stabilized subbase (HMA) N-30, the amount of RAP shall not exceed 50% of the mixture.

2/ Value of Max % RAP if homogeneous RAP stockpile of IL-9.5 RAP is utilized.

3/ When RAP exceeds 20 percent, the high and low virgin asphalt binder grades shall each be reduced by one grade (i.e. 25 percent RAP would require a virgin asphalt binder grade of PG64-22 to be reduced to a PG58-28). If warm mix asphalt (WMA)

technology is utilized, and production temperatures do not exceed 275°F (135 °C) the grades shall be reduced as follows:

Overlays:

When WMA contains between 20 and 30 percent RAP the high temperature shall be reduced by one grade (i.e. 25 percent RAP would require a virgin asphalt binder grade of PG64-22 to be reduced to a PG58-22). When WMA contains 30 percent or more RAP the high and low temperature grades shall each be reduced by one grade (i.e. 35 percent RAP would require a virgin asphalt binder grade of PG64-22 to be reduced to a PG58-28).

Full Depth:

When WMA contains between 20 and 30 percent RAP, the low temperature shall be reduced by one grade (i.e. 25 percent RAP would require a virgin asphalt binder grade of PG64-22 to be reduced to a PG64-28). When the WMA contains 30 percent or more RAP the high and low temperature grades shall each be reduced by one grade (i.e. 35 percent RAP would require a virgin asphalt binder grade of PG64-22 to be reduced to a PG58-28).

- (g) When the Contractor chooses the FRAP option, the percentage of FRAP shall not exceed the amounts indicated in the table below for a given N Design.

Max FRAP Percentage

HMA Mixtures ^{1/, 2/}	Maximum % FRAP		
	Binder/Leveling Binder	Surface	Polymer Modified
Ndesign			
30	35	35	10
50	30	25	10
70	25	20	10
90	20	15	10
105	10	10	10

1/ For HMA shoulder and stabilized subbase (HMA) N30, the amount of FRAP shall not exceed 50 percent of the mixture.

2/ When FRAP exceeds 20 percent, the high and low virgin asphalt binder grades shall each be reduced by one grade (i.e. 25 percent FRAP would require a virgin asphalt binder grade of PG64-22 to be reduced to a PG58-28). If warm mix asphalt (WMA) technology is utilized, and production temperatures do not exceed 275°F (135 °C) the grades shall be reduced as follows:

Overlays:

When WMA contains between 20 and 30 percent FRAP the high temperature shall be reduced by one grade (i.e. 25 percent FRAP would require a virgin asphalt binder grade of PG64-22 to be reduced to a PG58-22). When WMA contains 30 percent or more FRAP the high and low temperature grades shall each be reduced by one grade (i.e. 35 percent FRAP would require a virgin asphalt binder grade of PG64-22 to be reduced to a PG58-28).

Full Depth:

When WMA contains between 20 and 30 percent FRAP, the low temperature shall be reduced by one grade (i.e. 25 percent FRAP would require a virgin

asphalt binder grade of PG64-22 to be reduced to a PG64-28). When the WMA contains 30 percent or more FRAP the high and low temperature grades shall each be reduced by one grade (i.e. 35 percent FRAP would require a virgin asphalt binder grade of PG64-22 to be reduced to a PG58-28).

1031.06 HMA Mix Designs. At the Contractor's option, HMA mixtures may be constructed utilizing RAP/FRAP material meeting the above detailed requirements.

RAP/FRAP designs shall be submitted for volumetric verification. If additional RAP/FRAP stockpiles are tested and found that no more than 20 percent of the results, as defined under "Testing" herein, are outside of the control tolerances set for the original RAP/FRAP stockpile and HMA mix design, and meets all of the requirements herein, the additional RAP/FRAP stockpiles may be used in the original mix design at the percent previously verified.

1031.07 HMA Production. The coarse aggregate in all RAP used shall be equal to or less than the nominal maximum size requirement for the HMA mixture being produced.

To remove or reduce agglomerated material, a scalping screen, gator, crushing unit, or comparable sizing device approved by the Engineer shall be used in the RAP feed system to remove or reduce oversized material. If material passing the sizing device adversely affects the mix production or quality of the mix, the sizing device shall be set at a size specified by the Engineer.

If the RAP/FRAP control tolerances or QC/QA test results require corrective action, the Contractor shall cease production of the mixture containing RAP/FRAP and either switch to the virgin aggregate design or submit a new RAP/FRAP design.

HMA plants utilizing RAP/FRAP shall be capable of automatically recording and printing the following information.

(a) Dryer Drum Plants.

- (1) Date, month, year, and time to the nearest minute for each print.
- (2) HMA mix number assigned by the Department.
- (3) Accumulated weight of dry aggregate (combined or individual) in tons (metric tons) to the nearest 0.1 ton (0.1 metric ton).
- (4) Accumulated dry weight of RAP/FRAP in tons (metric tons) to the nearest 0.1 ton (0.1 metric ton).
- (5) Accumulated mineral filler in revolutions, tons (metric tons), etc. to the nearest 0.1 unit.
- (6) Accumulated asphalt binder in gallons (liters), tons (metric tons), etc. to the nearest 0.1 unit.
- (7) Residual asphalt binder in the RAP/FRAP material as a percent of the total mix to the nearest 0.1 percent.
- (8) Aggregate and RAP/FRAP moisture compensators in percent as set on the control panel. (Required when accumulated or individual aggregate and RAP/FRAP are printed in wet condition.)

(b) Batch Plants.

- (1) Date, month, year, and time to the nearest minute for each print.
- (2) HMA mix number assigned by the Department.
- (3) Individual virgin aggregate hot bin batch weights to the nearest pound (kilogram).
- (4) Mineral filler weight to the nearest pound (kilogram).
- (5) RAP/FRAP weight to the nearest pound (kilogram).
- (6) Virgin asphalt binder weight to the nearest pound (kilogram).
- (7) Residual asphalt binder in the RAP/FRAP material as a percent of the total mix to the nearest 0.1 percent.

The printouts shall be maintained in a file at the plant for a minimum of one year or as directed by the Engineer and shall be made available upon request. The printing system will be inspected by the Engineer prior to production and verified at the beginning of each construction season thereafter.

1031.08 RAP in Aggregate Surface Course and Aggregate Shoulders. The use of RAP in aggregate surface course and aggregate shoulders shall be as follows.

- (a) Stockpiles and Testing. RAP stockpiles may be any of those listed in Article 1031.02, except "Non-Quality" and "FRAP". The testing requirements of Article 1031.03 shall not apply.
- (b) Gradation. One hundred percent of the RAP material shall pass the 1 1/2 in. (37.5 mm) sieve. The RAP material shall be reasonably well graded from coarse to fine. RAP material that is gap-graded or single sized will not be accepted."

Designer Note: Insert into all contracts utilizing filter fabric.

FILTER FABRIC (BDE)

Effective: November 1, 2009

Revised: January 1, 2010

Revise the physical property tables in Article 1080.03 of the Standard Specifications to read:

"Physical Properties	Gradation 4 & 5	Gradation 6 & 7
Weight of Fabric (oz/sq yd), ASTM D 3776 (Mod.)	6.0 min.	8.0 min.
Burst Strength (psi), ASTM D 3786 ^{1/}	250 min.	300 min.
Trapezoidal Tear Strength (lb), ASTM D 5733 ^{2/}	60 min.	75 min.
Grab Tensile Strength (lb), ASTM D 4632 ^{2/}	160 min.	200 min.
Grab Tensile Elongation (%), ASTM D 4632 ^{2/}	50 max.	50 max.

Physical Properties (Metric)	Gradation 4 & 5	Gradation 6 & 7
Weight of Fabric (g/sq m), ASTM D 3776 (Mod.)	200 min.	270 min.
Burst Strength (kPa), ASTM D 3786 ^{1/}	1720 min.	2070 min.
Trapezoidal Tear Strength (N), ASTM D 5733 ^{2/}	265 min.	335 min.
Grab Tensile Strength (N), ASTM D 4632 ^{2/}	700 min.	900 min.
Grab Tensile Elongation (%), ASTM D 4632 ^{2/}	50 max.	50 max.

1/ Manufacturer's certification of fabric to meet requirements.

2/ Test sample shall be tested wet."

Designer Note: Use at the district's discretion when using Highway Standards 701321, 701402, or 701423.

Use of this special provision should be limited to cases where temporary concrete barrier is not feasible due to lane closure time restrictions that require the barrier to be deployed or removed within four hours. Use of this barrier system should also be limited to work areas where larger barrier deflection from an impact event is acceptable.

LONGITUDINAL TEMPORARY TRAFFIC BARRIER SYSTEM (BDE)

Effective: January 1, 2010

Description. This work shall consist of furnishing, installing, maintaining, relocating, and removing a temporary longitudinal traffic barrier system at locations shown on the plans.

Materials.

Add the following to Article 1106.02 of the Standard Specifications:

- "(k) Temporary Longitudinal Traffic Barrier. The temporary longitudinal traffic barrier shall be of a lightweight plastic shell with internal galvanized steel framework and designed to accept water ballast. The barrier shall be an approved unit that has been successfully crash tested for Test Level 3 impact conditions for 1800 and 4400 lb (820 and 2000 kg) vehicles at speeds of 62 mph (100 km/hr).

Shop drawings shall be furnished by the manufacturer and shall indicate the deflection of the barrier as determined by NCHRP 350 acceptance testing, the configuration of the barrier in that test, and the vehicle weight, velocity, and angle of impact of the deflection test. The Engineer shall be provided one copy of the shop drawings.

The barrier shall be capable of being moved on and off the roadway in no more than four hours.

The approach end of the temporary longitudinal traffic barrier shall either serve as an NCHRP 350 Test Level 3 approved end terminal or be protected with an impact attenuator which is capable of being moved with the temporary longitudinal traffic barrier and meeting NCHRP 350 Test Level 3. When not in use, the device shall be stored longitudinally along the far edge of the shoulder or adjacent to existing concrete median barrier.

The barrier system shall be of alternating white and orange color units and shall include nighttime delineation consisting of one 6 x 36 in. (150 x 900 mm) corrugated reflective retroreflective panel per unit. Retroreflective panels shall be yellow when on center line or left lane line and white when on edge line."

CONSTRUCTION REQUIREMENTS

Add the following to Article 701.15 of the Standard Specifications:

- "(l) Temporary Longitudinal Traffic Barrier. The temporary longitudinal traffic barrier shall be assembled and installed according to the manufacturer's specifications. The system shall be installed with the orange and white units alternating to aid visibility of the barrier.

The delivery, removal, and installation of the barrier system shall be accomplished using appropriate lane closures and traffic control. The cost of the traffic control shall be included in the cost of other traffic control pay items.

Maintenance of the barrier system shall be according to the manufacturer's specifications including maintaining water level and temperature requirements for the water ballast."

Method of Measurement.

Add the following to Article 701.19 of the Standard Specifications:

- "(f) Temporary longitudinal traffic barrier system will be measured for payment in feet (meters) in place, along the centerline of the barrier system. The impact attenuator shall be included in the cost of the system. It is the Contractor's responsibility to determine how many times the system will be moved, and include this in the lump sum price for relocation of the system."

Basis of Payment.

Add the following to Article 701.20 of the Standard Specifications:

- "(j) This work will be paid for at the contract unit price per foot (meter) for TEMPORARY LONGITUDINAL TRAFFIC BARRIER SYSTEM and at the contact lump sum price for RELOCATE TEMPORARY LONGITUDINAL BARRIER SYSTEM."

Designer Note: Insert into contracts using Highway Standards 701321, 701402, or 701423 at **the district's discretion.**

Use of this special provision should be limited to cases where temporary concrete barrier is not feasible due to lane closure time restrictions that require the barrier to be deployed or removed on a daily basis. Use of this barrier system should also be limited to work areas where larger barrier deflection from an impact event is acceptable.

MOVABLE TRAFFIC BARRIER SYSTEM (BDE)

Effective: January 1, 2010

Description. This work shall consist of furnishing, installing, maintaining, relocating, and removing a movable barrier traffic system at locations shown on the plans.

Materials.

Add the following to Article 1106.02 of the Standard Specifications:

- “(l) Movable Traffic Barrier. The movable traffic barrier shall be an approved system that has been successfully crash tested for Test Level 3 impact conditions for 1800 and 4400 lb (820 and 2000 kg) vehicles at speeds of 62 mph (100 km/hr).

Shop drawings shall be furnished by the manufacturer and shall indicate the deflection of the barrier as determined by NCHRP 350 acceptance testing, the configuration of the barrier in that test, and the vehicle weight, velocity, and angle of impact of the deflection test. The Engineer shall be provided one copy of the shop drawings.

The barrier shall be capable of being moved on and off the roadway on a daily basis.

The approach end of the movable traffic barrier system shall be protected with an impact attenuator which is capable of being moved with the movable barrier system, and meeting the Test Level 3 NCHRP requirements. When not in use, the device shall be stored longitudinally along the far edge of the shoulder or adjacent to existing concrete median barrier.

The barrier system shall include nighttime delineation consisting of one 6 x 36 in. (150 x 900 mm) corrugated reflective retroreflective panel per unit. Retroreflective panels shall be yellow when on center line or left lane line and white when on edge line.”

CONSTRUCTION REQUIREMENTS

Add the following to Article 701.15 of the Standard Specifications:

- “(m) Movable Traffic Barrier. The movable traffic barrier shall be assembled and installed according to the manufacturer's specifications.

The delivery, removal, and installation of the movable traffic barrier system shall be accomplished using appropriate lane closures and traffic control. The cost of the traffic control shall be included in the cost of other traffic control pay items."

Method of Measurement.

Add the following to Article 701.19 of the Standard Specifications:

"(g) Movable traffic barrier system will be measured for payment in feet (meters) in place, along the centerline of the movable barrier system. The impact attenuator shall be included in the cost of the system. It is the Contractor's responsibility to determine how many times the system will be moved, and include this in the lump sum price for relocation of the system."

Basis of Payment.

Add the following to Article 701.20 of the Standard Specifications:

"(k) This work will be paid for at the contract unit price per foot (meter) for MOVABLE TRAFFIC BARRIER SYSTEM and at the contact lump sum price for RELOCATE MOVABLE TRAFFIC BARRIER SYSTEM."

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Designer Notes
Recurring Special Provisions

Designer Notes for January 1, 2010 Recurring Special Provisions

1. Designer Note: This check sheet is required in all contracts that involve Federal funds.
2. Designer Note: This check sheet is required in all Federal contracts.
3. Designer Note: This check sheet is required in all contracts.
4. Designer Note: This check sheet is required in all contracts involving State funds only.
5. Designer Note: This check sheet is required in all contracts involving State funds only.
6. Reserved.
7. Reserved.
8. Designer Note: This check sheet will be required for those contracts that will involve Contractor work on haul road stream crossings, other temporary stream crossings, and in stream work pads. Contracts that would generally involve this type of work would be bridges/structures, new or rebuilt, and contracts involving earth excavation, embankment or borrow excavation. Discuss these types of work operations and any other stream related work with your Project Engineer. Any in-stream crossing or other work will require an individual 404 permit from the Corps of Engineers. Be sure to let the Hydraulics Engineer (Jim Miller) know as soon as possible that a Corps permit will be needed. The permit has a lead-time and is required for the project to proceed to letting.
9. Designer Note: (See #10 below.) Depending on IDOT manpower, this check sheet will be included as a pay item when the Contractor will be required to do all contract staking, except bridges. A large span culvert measuring more than 6 meters (20 feet) along the survey line will require a structure number be assigned to the structure. This will require that the Designer, if he is calling for Contractor staking, use the check sheet entitled Construction Layout Stakes and not the check sheet entitled Construction Layout Stakes Except for Structures. Discuss with the Bureau of Project Implementation (Construction) as to what manpower sources are available.
10. Designer Note: Depending on IDOT manpower needs, this check sheet will be included as a pay item when the Contractor will be required to do all contract staking, including bridges. This check sheet should be used for a large box culvert or a multi pipe that will require a structure number. This would be a structure that will have a span length along survey line of more than 6 meters (20 feet).

Discuss this check sheet with the Bureau of Project Implementation (Construction) as to what manpower sources are available.

11. Designer Note: This special provision specifies the requirements for geotextile fabric for use on railroad crossings.

Include only on projects where the railroad crossing is a contract pay item. Also may be required for temporary crossings.

Railroad crossings are generally (99%) handled by the Railroad through an agreement and not part of our contract. If in doubt as to how to handle, discuss with Project Support.
12. Designer Note: Use this check sheet where existing pavement is being reconstructed and voids are evident under the existing pavement that can be filled by grouting. Discuss with Maintenance Field Engineer responsible for the area.

NOTE: A detail of the slab movement detection device is included in CADD and this drawing must be included in your contract plans.
13. Designer Note: This check sheet will be required on a contract where cold milling is required but where the cold milled area will not be overlaid. Include CADD Standard 440001 in your plans. If your contract is to be cold milled and the area overlaid, you should use one of the two District special provisions on this subject, not this check sheet.
14. Designer Note: This check sheet requires that once a lift of bituminous resurfacing is placed on a lane of pavement, any adjoining bituminous shoulder shall be resurfaced with an equal thickness before any other lane is resurfaced for each lift of resurfacing. Insert this special on resurfacing projects which meet the following criteria: All four lane interstates and freeways, all four lane expressways, four lane highways with ADT > 25,000 or peak one-way VPH > 1700, two lane highways with ADT > 10,000 or peak one-way VPH > 800.
15. Designer Note: This check sheet should be used on resurfacing projects to address areas which need repair, but do not warrant full depths repair. Joints and cracks, which exhibit environmental distresses such as spalling and "D" cracking or contain maintenance patching, are eligible for using this method of repair. Joints and cracks which exhibit load related distresses such as pumping, alligator cracking, corner breaks, compression failures, subgrade failures or punch outs should not use this method of repair. Discuss use with your Project Engineer.
16. Designer Note: Intended to remove thick bituminous overlay so that the original pavement can be examined and then patched, if necessary. It also further defines specific pay items for work involved.
17. Designer Note: This check sheet was developed by Materials and Physical Research as an alternate to replacing Preformed Joint Sealer and Neoprene Expansion Joints up to 65 mm (2 ½ inches). Include with any projects that have POLYMER CONCRETE as a pay item.

18. Designer Note: This rehabilitation process can be used in a variety of gravity applications such as trenchless rehabilitation of sanitary sewers, storm sewers, and process piping. Insert this special provision if trenchless repair of the items listed above is selected. Prior to selection consult your Project Engineer. Additional information such as size of pipe to be lined, number of laterals, and manhole treatment may be necessary.
19. Designer Note: This check sheet calls for CA 16 for backfill and wrapping the trench. Discuss usage with Implementation.
20. Designer Note: This check sheet was developed by the Central Bureau of Traffic and should be incorporated into all plans containing guardrail, barrier wall or bridge rail. The designer is required to specify the color of all reflectors to be placed and to provide appropriate traffic control standards for the installation of reflectors/markers. It is the District's option to select the type of reflector marker for use on guardrail and barrier walls, and the type of terminal marker for guardrail. This option should be specified by the pay item used. The District prefers use of the top mounted reflector Type C on barrier walls. Include Highway Standards 635006 and 635011 in the plans if this Check Sheet is used.
21. Designer Note: This check sheet was developed to obtain the desired pipe coating on bike racks. Use on all projects with bike racks.
22. Designer Note: This special provision covers the installation of temporary glare screens on temporary concrete barrier. Glare screens may be needed on temporary concrete barriers separating opposing lanes of traffic, especially on horizontal and vertical curves where oncoming headlight glare could be a problem. Discuss usage with your project engineer.
23. Designer Note: This special provision is for use on bridge contracts where staging is required and the District wants the contractor to have an option to post-mounting the temporary bridge and traffic signals. Discuss use with the District traffic Control Technician.
24. Designer Note: Intended for use on all freeway/expressway contracts with lane closures as shown on Highway Standard 701400. It may also be used at the District's discretion on high visibility projects and/or projects that will require several months to complete.
25. Designer Note: This check sheet should be included for all projects containing roadway lighting. The designer should also include CADD Standard 701301-D4 in the plans.
26. Designer Note: This check sheet was developed to address difficulties with obtaining metric sized bolts. Include in all metric projects, which contain or could contain any type of bolted connection.
27. Designer Note: This check sheet was developed to address difficulties with obtaining metric sized reinforcement bars. Include in all metric projects containing reinforcement bars.

28. Designer Note: This special provision is to be included in pavement and bridge deck patching projects only when specified by District Materials personnel. Not recommended for use on recently constructed pavements or bridge decks.
29. Reserved.
30. Designer Note: Don't use Check Sheet #30 unless requested by Materials.
31. Designer Note: QC/QA for concrete is generally only used on projects with relatively large concrete quantities. Check with Materials prior to use on any project. Note that QC/QA concrete is no longer paid for with a pay item.
32. Designer Note: Include in all contracts where Asbestos Bearing Pad Removal is part of the structure work.
33. Designer Note: Include in all contracts where the existing bridge deck HMA surface is to be removed and the waterproofing membrane contains asbestos and will be removed. The designer must have in the project files a completed "Asbestos Determination Certificate" for every bridge within the project limits. The District Bridge Maintenance Engineer and/or the District Hydraulics Engineer can provide copies of these certificates. If your project has any bridge deck containing asbestos, insert this special provision as well as the General Notes entitled "Asbestos Bridge Wearing Surface Removal".