




Illinois Department of Transportation

Memorandum

To: *

From: Rich Dotson 

Subject: **Special Provision Changes**

Date: January 26, 2011

The following special provisions have been revised for the April 29, 2011 letting. Please revise your special provision books as indicated.

Interim Special Provisions	
ISP Number	Description
Alphabetic ISP Index (Revised)	Remove existing alphabetic index and insert revised index.
Numerical ISP Index (Revised)	Remove existing numeric index and insert revised index.
105.07 (New)	"Utility Coordination and Conflicts (BDE)" New special to compensate for unforeseen utility delays.
108.06a (Revised)	"Disadvantaged Business Enterprise Participation (BDE)" Minor revisions.
108.09 (Revised)	"Liquidated Damages (BDE)" Revised the values.
251.00 (Revised)	"Mulch and Erosion Control Blankets (BDE)" Revised the name of the special.
400.01 (Revised)	"Preventive Maintenance – Cape Seal (BDE)" Revised Designer Note.
400.02 (Revised)	"Preventive Maintenance – Micro-Surfacing (BDE)" Revised Designer Note.
400.03 (Revised)	"Preventive Maintenance – Slurry Seal (BDE)" Revised Designer Note.
400.04 (Revised)	"Preventive Maintenance – Bituminous Surface Treatment (BDE)" Revised Designer Note.
406.05 (New)	"Safety Edge (BDE)" New special to require the use of the safety edge.
420.00 (Revised)	"Portland Cement Concrete Inlay or Overlay (BDE)" Revised Designer Note.

Interim Special Provisions (continued)

ISP Number	Description
603.02 (New)	“Drainage and Inlet Protection Under Traffic (BDE)” New special to allow use of rubber ramps.
671.00 (Revised)	“Subcontractor Mobilization Payments (BDE)” Revised for clarification.
701.10 (Revised)	“Traffic Control Surveillance (BDE)” Revised Designer Note.
702.00c (Revised)	“Impact Attenuators (BDE)” Revised Designer Note.
702.00d (Revised)	“Impact Attenuators, Temporary (BDE)” Revised Designer Note.
1001.00 (Revised)	“Cement (BDE)” Revised due to AASHTO and ASTM cement specification changes.

District Special Provisions

District Number	Description
Alphabetic District Index (Revised)	Remove existing alphabetic index and insert revised index.
Numerical District Index (Revised)	Remove existing numeric index for Section 200, 400 and 1000 and insert revised index.
250.06a (Revised)	“Mowing” Revised to clarify that ramps are included in the length of project and not measured separately.
406.02 (New)	“Hot-Mix Asphalt – Prime Coat” New requirement for asphalt dilution.
406.05 (Delete)	“Hot-Mix Asphalt – Prime Coat (BMPR)” Deleted.
407.14d (Delete)	“Raised Rumble Strip” The temporary rumble strip is included in Highway Standard 701901.
1004.01 (New)	“Coarse Aggregate Fill” New special for shallow undercuts or as a capping material.
1004.04 (Revised)	“Aggregate Quality” Minor revision.

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Attachment(s)

cc: * J. Miller Team 2 Team 6 Team 10 Galesburg Design
 K. Emert Team 3 Team 7 Team 11 Local Roads (M. Augspurger)
 T. Phillips Team 4 Team 8 Geometrics Materials (H. Shoup)
 Team 1 Team 5 Team 9 Bridge (T. Inglis)

BDE Special Provisions Checklist
For the April 29 & June 17, 2011 Letting

Contract No.: _____

Letting: _____

Note: Specials that go in every contract have already been marked with an "X" for you.

BDE SPECIAL PROVISIONS
For the April 29 and June 17, 2011 Lettings

The following special provisions indicated by an "x" are applicable to this contract and will be included by the Project Development and Implementation Section of the BD&E. An * indicates a new or revised special provision for the letting.

<u>File Name</u>	<u>#</u>	<u>Special Provision Title</u>	<u>Effective</u>	<u>Revised</u>
80240	1	Above Grade Inlet Protection	July 1, 2009	
80099	2	Accessible Pedestrian Signals (APS)	April 1, 2003	Jan. 1, 2007
80243	3	American Recovery and Reinvestment Act Provisions	April 1, 2009	
80236	4	American Recovery and Reinvestment Act Signing	April 1, 2009	April 15, 2009
80186	5	Alkali-Silica Reaction for Cast-in-Place Concrete	Aug. 1, 2007	Jan. 1, 2009
80213	6	Alkali-Silica Reaction for Precast and Precast Prestressed Concrete	Jan. 1, 2009	
80207	7	X Approval of Proposed Borrow Areas, Use Areas, and/or Waste Areas (NOTE: This special provision was previously named "Approval of Proposed Borrow Areas, Use Areas, and/or Waste Areas Inside Illinois State Borders".)	Nov. 1, 2008	Nov. 1, 2010
80192	8	Automated Flagger Assistance Device	Jan. 1, 2008	
80173	9	Bituminous Materials Cost Adjustments	Nov. 2, 2006	April 1, 2009
80241	10	Bridge Demolition Debris	July 1, 2009	
50261	11	Building Removal-Case I (Non-Friable and Friable Asbestos)	Sept. 1, 1990	April 1, 2010
50481	12	Building Removal-Case II (Non-Friable Asbestos)	Sept. 1, 1990	April 1, 2010
50491	13	Building Removal-Case III (Friable Asbestos)	Sept. 1, 1990	April 1, 2010
50531	14	Building Removal-Case IV (No Asbestos)	Sept. 1, 1990	April 1, 2010
* 80166	15	Cement	Jan. 1, 2007	April 1, 2011
80260	16	Certification of Metal Fabricator	July 1, 2010	
80198	17	Completion Date (via calendar days)	April 1, 2008	
0199	18	Completion Date (via calendar days) Plus Working Days	April 1, 2008	
00094	19	Concrete Admixtures	Jan. 1, 2003	April 1, 2009
80215	20	Concrete Joint Sealer	Jan. 1, 2009	
80226	21	Concrete Mix Designs	April 1, 2009	
80261	22	Construction Air Quality – Diesel Retrofit	June 1, 2010	
80237	23	X Construction Air Quality – Diesel Vehicle Emissions Control	April 1, 2009	July 1, 2009
80239	24	X Construction Air Quality – Idling Restrictions	April 1, 2009	
80227	25	Determination of Thickness	April 1, 2009	
80177	26	Digital Terrain Modeling for Earthwork Calculations	April 1, 2007	
* 80029	27	Disadvantaged Business Enterprise Participation	Sept. 1, 2000	Jan. 1 2011
* 80271	28	Drainage and Inlet Protection Under Traffic	April 1, 2011	
80179	29	Engineer's Field Office Type A	April 1, 2007	Jan. 1, 2011
80205	30	Engineer's Field Office Type B	Aug. 1, 2008	Jan. 1, 2011
80189	31	X Equipment Rental Rates	Aug. 2, 2007	Jan. 2, 2008
80228	32	Flagger at Side Roads and Entrances	April 1, 2009	
80249	33	Frames and Grates	Jan. 1, 2010	
80265	34	Friction Aggregate	Jan. 1, 2011	
80229	35	Fuel Cost Adjustment	April 1, 2009	July 1, 2009
80169	36	High Tension Cable Median Barrier	Jan. 1, 2007	April 1, 2009
80194	37	HMA – Hauling on Partially Completed Full-Depth Pavement	Jan. 1, 2008	
80245	38	Hot-Mix Asphalt – Anti-Stripping Additive	Nov. 1, 2009	
80246	39	Hot-Mix Asphalt – Density Testing of Longitudinal Joints	Jan. 1, 2010	
80250	40	Hot-Mix Asphalt – Drop-Offs	Jan. 1, 2010	
80259	41	Hot Mix Asphalt – Fine Aggregate	April 1, 2010	
80109	42	Impact Attenuators	Nov. 1, 2003	Nov. 1, 2008
80110	43	Impact Attenuators, Temporary	Nov. 1, 2003	Jan. 1, 2007
0252	44	Improved Subgrade	Jan. 1, 2010	

Contract No.: _____

Letting: _____

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<u>Item Name #</u>	<u>Special Provision Title</u>	<u>Effective</u>	<u>Revised</u>
J266 45	Lane Closure, Multilane, Intermittent or Moving Operation, for Speeds ≤ 40 MPH	Jan. 1, 2011	Jan. 2, 2011
* 80230 46	X Liquidated Damages	April 1, 2009	April 1, 2011
80267 47	Long-Span Guardrail over Culvert	Jan. 1, 2011	
80045 48	Material Transfer Device	June 15, 1999	Jan. 1, 2009
80203 49	Metal Hardware Cast into Concrete	April 1, 2008	April 1, 2009
80165 50	Moisture Cured Urethane Paint System	Nov. 1, 2006	Jan. 1, 2010
80238 51	Monthly Employment Report	April 1, 2009	Jan. 1, 2010
80253 52	Movable Traffic Barrier (NOTE: This special provision was previously named "Movable Traffic Barrier System".)	Jan. 1, 2010	Jan. 1, 2011
* 80262 53	X Mulch and Erosion Control Blankets (NOTE: This special provision was previously named "Mulch")	Nov. 1, 2010	April 1, 2011
80180 54	X National Pollutant Discharge Elimination System / Erosion and Sediment Control Deficiency Deduction	April 1, 2007	Nov. 1, 2009
80208 55	Nighttime Work Zone Lighting	Nov. 1, 2008	
80231 56	Pavement Marking Removal	April 1, 2009	
80254 57	Pavement Patching	Jan. 1, 2010	
80022 58	X Payments to Subcontractors	June 1, 2000	Jan. 1, 2006
80232 59	Pipe Culverts	April 1, 2009	April 1, 2010
80263 60	Planting Perennial Plants	Jan. 1, 2011	
80210 61	Portland Cement Concrete Inlay or Overlay	Nov. 1, 2008	
80217 62	Post Clips for Extruded Aluminum Signs	Jan. 1, 2009	
80268 63	X Post Mounting of Signs	Jan. 1, 2011	
80171 64	Precast Handling Holes	Jan. 1, 2007	
80218 65	Preventive Maintenance – Bituminous Surface Treatment	Jan. 1, 2009	April 1, 2009
J219 66	Preventive Maintenance – Cape Seal	Jan. 1, 2009	April 1, 2009
80220 67	Preventive Maintenance – Micro-Surfacing	Jan. 1, 2009	
80221 68	Preventive Maintenance – Slurry Seal	Jan. 1, 2009	
80015 69	Public Convenience and Safety	Jan. 1, 2000	
34261 70	Railroad Protective Liability Insurance	Dec. 1, 1986	Jan. 1, 2006
80157 71	Railroad Protective Liability Insurance (5 and 10)	Jan. 1, 2006	
80247 72	Raised Reflective Pavement Markers	Nov. 1, 2009	April 1, 2010
80172 73	Reclaimed Asphalt Pavement (RAP)	Jan. 1, 2007	Jan. 1, 2011
80224 74	Restoring Bridge Approach Pavements Using High-Density Foam	Jan. 1, 2009	
* 80271 75	X Safety Edge	April 1, 2011	
80131 76	Seeding	July 1, 2004	July 1, 2010
80264 77	Selection of Labor	July 2, 2010	
80152 78	Self-Consolidating Concrete for Cast-In-Place Construction	Nov. 1, 2005	July 1, 2010
80132 79	Self-Consolidating Concrete for Precast Products	July 1, 2004	July 1, 2010
80127 80	Steel Cost Adjustment	April 2, 2004	April 1, 2009
80255 81	Stone Matrix Asphalt	Jan. 1, 2010	
80234 82	Storm Sewers	April 1, 2009	April 1, 2010
* 80143 83	X Subcontractor Mobilization Payments	April 2, 2005	April 1, 2011
80075 84	Surface Testing of Pavements	April 1, 2002	Jan. 1, 2007
80087 85	Temporary Erosion Control	Nov. 1, 2002	Jan. 1, 2011
80225 86	Temporary Raised Pavement Marker	Jan. 1, 2009	
80256 87	Temporary Water Filled Barrier (NOTE: This special provision was previously named "Temporary Longitudinal Traffic Barrier System".)	Jan. 1, 2010	Jan. 1, 2011
80257 88	Traffic Barrier Terminal, Type 6	Jan. 1, 2010	
J269 89	X Traffic Control Surveillance	Jan. 1, 2011	
J338 90	Training Special Provisions	Oct. 15, 1975	

Contract No.: _____

Letting: _____

Note: Specials that go in every contract have already been marked with an "X" for you.

<u>File Name</u>	<u>#</u>	<u>Special Provision Title</u>	<u>Effective</u>	<u>Revised</u>
J258	91	Truck Mounted/Trailer Mounted Attenuators	Jan. 1, 2010	
* 80270	92	X Utility Coordination and Conflicts	April 1, 2011	
80071	93	Working Days	Jan. 1, 2002	

The following special provisions are in the 2011 Supplemental Specifications and Recurring Special Provisions:

<u>File Name</u>	<u>Special Provision Title</u>	<u>New Location</u>	<u>Effective</u>	<u>Revised</u>
80214	Concrete Gutter, Type A	Article 606.07	Jan. 1, 2009	
80178	Dowel Bars	Article 1006.11	April 1, 2007	Jan. 1, 2008
80201	Hot-Mix Asphalt – Plant Test Frequency	Article 1030.05	April 1, 2008	Jan. 1, 2010
80251	Hot-Mix Asphalt – QC/QA Acceptance Criteria	Article 1030.05	Jan. 1, 2010	
80202	Hot-Mix Asphalt – Transportation	Article 1030.08	April 1, 2008	
80196	Mast Arm Assembly and Pole	Article 1077.03	Jan. 1, 2008	Jan. 1, 2009
80182	Notification of Reduced Width	Article 701.06	April 1, 2007	
80069	Organic Zinc-Rich Paint System	Article 1008.05	Nov. 1, 2001	Jan. 1, 2010
80216	Partial Exit Ramp Closure for Freeway/Expressway	Section 701	Jan. 1, 2009	
80209	Personal Protective Equipment	Article 701.12	Nov. 1, 2008	
80119	Polyurea Pavement Marking	Sections 780, 1095 and 1105	April 1, 2004	Jan. 1, 2009
80170	Portland Cement Concrete Plants	Article 1020.11	Jan. 1, 2007	
80211	Prismatic Curb Reflectors	Articles 782.03 and 1097.04	Nov. 1, 2008	
80223	Ramp Closure for Freeway/Expressway	Section 701	Jan. 1, 2009	
80183	Reflective Sheeting on Channelizing Devices	Article 1106.02	April 1, 2007	Nov. 1, 2008
80206	Reinforcement Bars – Storage and Protection	Article 508.03	Aug. 1, 2008	April 1, 2009
80176	Thermoplastic Pavement Marking	Article 1095.01	Jan. 1, 2007	

The following special provisions require additional information from the designer. The additional information needs to be included in a separate document attached to this check sheet. The Project Development and Implementation section will then include the information in the applicable special provision. The Special Provisions are:

- Bridge Demolition Debris
- Building Removal-Case I
- Building Removal-Case II
- Building Removal-Case III
- Building Removal-Case IV
- Completion Date
- Completion Date Plus Working Days
- DBE Participation
- Material Transfer Device
- Railroad Protective Liability Insurance
- Training Special Provisions
- Working Days

**Index for
Supplemental Specifications
and
Recurring Special Provisions**

INDEX
FOR
SUPPLEMENTAL SPECIFICATIONS
AND RECURRING SPECIAL PROVISIONS

Adopted January 1, 2011

This index contains a listing of SUPPLEMENTAL SPECIFICATIONS and frequently used RECURRING SPECIAL PROVISIONS.

ERRATA Standard Specifications for Road and Bridge Construction (Adopted 1-1-07) (Revised 1-1-11)

SUPPLEMENTAL SPECIFICATIONS

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253 Planting Woody Plants	4
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501 Removal of Existing Structures	15
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503 Concrete Structures	17
504 Precast Concrete Structures	18
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540 Box Culverts	21
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RECURRING SPECIAL PROVISIONS

The following RECURRING SPECIAL PROVISIONS indicated by an "X" are applicable to this contract and are included by reference:

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BDE Special Provisions

Alphabetic Index

ALPHABETIC LIST OF DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

Get a copy of the current check list from the Program Development Secretary, indicate which ISP's are to be included in your set of special provisions, fill in any blanks as indicated on the check list, and include with your set of special provisions to be sent to Springfield where they will be inserted.

<u>Standard Spec. No.</u>	<u>PC No.</u>	<u>Item</u>
280.02	28002	Above Grade Inlet Protection
888.00	88800	Accessible Pedestrian Signals (APS)
1020.02	102002	Alkali-Silica Reaction for Cast-in-Place Concrete
1020.03	102003	Alkali-Silica Reaction for Precast and Precast Prestressed Concrete
109.12	10912	American Recovery and Reinvestment Act Provisions
701.04	70104	American Recovery and Reinvestment Act Signing
107.22	10722	Approval of Proposed Borrow Areas, Use Areas, and/or Waste Areas Inside Illinois State Borders
701.00	70100	Automated Flagger Assistance Devices
109.01	10901	Bituminous Materials Cost Adjustment
107.38	10738	Bridge Demolition Debris
107.19a	10719a	Building Removal Case I
107.19b	10719b	Building Removal Case II
107.19c	10719c	Building Removal Case III
107.19d	10719d	Building Removal Case IV
1001.00	100100	Cement
106.08	10608	Certification of Metal Fabricators
108.05a	10805a	Completion Date (Via Calendar Days)
108.05b	10805b	Completion Date (Via Calendar Days) Plus working Days
1020.05b	102005b	Concrete Admixtures
503.19	50319	Concrete Joint Sealer
1020.05c	102005c	Concrete Mix Designs

ALPHABETIC LIST OF DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

<u>Standard Spec. No.</u>	<u>PC No.</u>	<u>Item</u>
107.00	10700	Construction Air Quality – Diesel Vehicle Emissions Control
107.01	10701	Construction Air Quality – Diesel Retrofit
107.37	10737	Construction air Quality – Idling Restrictions
353.00	35300	Determination of Thickness
202.07	20207	Digital Terrain Modeling for Earthwork Calculations
108.06a	10806a	Disadvantaged Business Enterprise Participation
603.02	60302	Drainage and Inlet Protection Under Traffic
670.02	67002	Engineer's Field Office Type A
670.03	67003	Engineer's Field Office Type B
109.04	10904	Equipment Rental Rates
701.13	70113	Flagger at Side Roads and Entrances
609.02	60902	Frames and Grates
1004.01	100401	Friction Aggregate
109.03	10903	Fuel Cost Adjustment
643.00	64300	High Tension Cable Median Barrier
407.08	40708	HMA-Hauling on Partially Completed Full-Depth Pavement
1030.04c	103004c	Hot-Mix Asphalt – Anti-Stripping Additive
406.07	40607	Hot-Mix Asphalt-Density Testing of Longitudinal Joints
701.07a	70107a	Hot-Mix Asphalt Drop-Offs
1003.01	100301	Hot-Mix Asphalt – Fine Aggregate
702.00c	70200c	Impact Attenuators
702.00d	70200d	Impact Attenuators, Temporary
302.04	30204	Improved Subgrade
701.19	70119	Lane Closure, Multilane, Intermittent or Moving Operation, for Speeds ≤ 40 MPH

REVISED INDEX

01/21/11

ALPHABETIC LIST OF DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

<u>Standard Spec. No.</u>	<u>PC No.</u>	<u>Item</u>
108.09	10809	Liquidated Damages
630.07	63007	Long-Span Guardrail over Culvert
406.00f	40600f	Material Transfer Device
503.02	50302	Metal Hardware Cast into Concrete
1008.27	100827	Moisture Cured Urethane Paint System
109.11	10911	Monthly Employment Report
1106.02i	110602i	Movable Traffic Barrier
251.00	25100	Mulch and Erosion Control Blankets
105.03	10503	National Pollutant Discharge Elimination System / Erosion and Sediment Control Deficiency Deduction
701.01	70101	Nighttime Work Zone Lighting
783.03	78303	Pavement Marking Removal
701.17	70117	Pavement Patching
109.07	10907	Payments to Subcontractors
542.03	54203	Pipe Culverts
254.00	25400	Planting Perennial Plants
420.00	42000	Portland Cement Concrete Inlay or Overlay
1090.03	109003	Post Clips for Extruded Aluminum Signs
701.14	70114	Post Mounting of Signs
540.02	54002	Precast Concrete Handling Holes
400.04	40004	Preventive Maintenance - Bituminous Surface Treatment
400.01	40001	Preventive Maintenance – Cape Seal
400.02	40002	Preventive Maintenance – Micro-Surfacing
400.03	40003	Preventive Maintenance – Slurry Seal
107.09	10709	Public Convenience and Safety

ALPHABETIC LIST OF DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

Standard Spec. No.	PC No.	Item
107.11	10711a	Railroad Protective Liability Insurance
107.11	10711b	Railroad Protective Liability Insurance (5 and 10)
781.03	78103	Raised Reflective Pavement Markers
1031.00	103100	Reclaimed Asphalt Pavement (RAP)
420.16	42016	Restoring Bridge Approach Pavements Using High-Density Foam
406.05	40605	Safety Edge
250.00	25000	Seeding
108.00	10800	Selection of Labor
1020.01	102001	Self-Consolidating Concrete for Cast-in-Place Construction
1020.00	102000	Self-Consolidating Concrete for Precast Products
109.00	10900a	Steel Cost Adjustment
406.06	40606	Stone Matrix Asphalt
550.02	55002	Storm Sewers
671.00	67100	Subcontractor Mobilization Payments
406.21	40621	Surface Testing of Pavements
280.04	28004	Temporary Erosion Control
703.00	70300	Temporary Raised Pavement Marker
1106.02k	110602k	Temporary Water Filled Barrier
631.07	63107	Traffic Barrier Terminal, Type 6
701.10	70110	Traffic Control Surveillance
108.06	10806	Training Special Provision
701.03	70103	Truck Mounted/Trailer Mounted Attenuators
105.07	10507	Utility Coordination and Conflicts
108.05	10805	Working Days

BDE Special Provisions

Numeric Index

REVISED INDEX

NUMERIC DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

Get a copy of the current check list from the Program Development Secretary, indicate which ISP's are to be included in your set of special provisions, fill in any blanks as indicated on the check list, and include with your set of special provisions to be sent to Springfield where they will be inserted.

<u>Standard Spec. No.</u>	<u>PC No.</u>	<u>Item</u>
105.03	10503	National Pollutant Discharge Elimination System / Erosion and Sediment Control Deficiency Deduction
105.07	10507	Utility Coordination and Conflicts
106.08	10608	Certification of Metal Fabricator
107.00	10700	Construction Air Quality – Diesel Vehicle Emissions Control
107.01	10701	Construction Air Quality – Diesel Retrofit
107.09	10709	Public Convenience and Safety
107.11a	10711a	Railroad Protective Liability Insurance
107.11b	10711b	Railroad Protective Liability Insurance (5 and 10)
107.19a	10719a	Building Removal Case I
107.19b	10719b	Building Removal Case II
107.19c	10719c	Building Removal Case III
107.19d	10719d	Building Removal Case IV
107.22	10722	Approval of Proposed Borrow Areas, Use Areas, and/or Waste Areas Inside Illinois State Borders
107.37	10737	Construction Air Quality – Idling Restrictions
107.38	10738	Bridge Demolition Debris
108.00	10800	Selection of Labor
108.05	10805	Working Days
108.05a	10805a	Completion Date (Via Calendar Days)
108.05b	10805b	Completion Date (Via Calendar Days) Plus Working Days
108.06	10806	Training Special Provision
108.06a	10806a	Disadvantaged Business Enterprise Participation

NUMERIC DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

<u>Standard Spec. No.</u>	<u>PC No.</u>	<u>Item</u>
108.09	10809	Liquidated Damages
109.00a	10900a	Steel Cost Adjustment
109.01	10901	Bituminous Materials Cost Adjustments
109.03	10903	Fuel Cost Adjustment
109.04	10904	Equipment Rental Rates
109.07	10907	Payments to Subcontractors
109.11	10911	Monthly Employment Report
109.12	10912	American Recovery and Reinvestment Act Provisions
202.07	20207	Digital Terrain Modeling for Earthwork Calculations
250.00	25000	Seeding
251.00	25100	Mulch and Erosion Control Blankets
254.00	25400	Planting Perennial Plants
280.02	28002	Above Grade Inlet Protection
280.04	28004	Temporary Erosion Control
302.04	30204	Improved Subgrade
353.00	35300	Determination of Thickness
400.01	40001	Preventive Maintenance – Cape Seal
400.02	40002	Preventive Maintenance – Micro-Surfacing
400.03	40003	Preventive Maintenance – Slurry Seal
400.04	40004	Preventive Maintenance – Bituminous Surface Treatment
406.00f	40600f	Material Transfer Device
406.05	40605	Safety Edge
406.06	40606	Stone Matrix Asphalt
406.07	40607	Hot-Mix Asphalt – Density Testing of Longitudinal Joints
406.21	40621	Surface Testing of Interstate Pavements

NUMERIC DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

<u>Standard Spec. No.</u>	<u>PC No.</u>	<u>Item</u>
407.08	40708	HMA-Hauling on Partially Completed Full-Depth Pavement
420.00	42000	Portland Cement Concrete Inlay or Overlay
420.16	42016	Restoring Bridge Approach Pavements Using High-Density Foam
503.02	50302	Metal Hardware Cast Into Concrete
503.19	50319	Concrete Joint Sealer
540.02	54002	Precast Concrete Handling Holes
542.03	54203	Pipe Culverts
550.02	55002	Storm Sewers
603.02	60302	Drainage and Inlet Protection Under Traffic
609.02	60902	Frames and Grates
630.07	63007	Long-Span Guardrail over Culvert
631.07	63107	Traffic Barrier Terminal, Type 6
643.00	64300	High Tension Cable Median Barrier
670.02	67002	Engineers Field Office Type A
670.03	67003	Engineers Field Office Type B
671.00	67100	Subcontractor Mobilization Payments
701.00	70100	Automated Flagger Assistance Devices
701.01	70101	Nighttime Work Zone Lighting
701.03	70103	Truck Mounted/Trailer Mounted Attenuators
701.04	70104	American Recovery and Reinvestment Act Signing
701.07a	70107a	Hot-Mix Asphalt – Drop-Offs
701.10	70110	Traffic Control Surveillance
701.13	70113	Flagger at Side Roads and Entrances
701.14	70114	Post Mounting of Signs
701.17	70117	Pavement Patching

NUMERIC DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

<u>Standard Spec. No.</u>	<u>PC No.</u>	<u>Item</u>
701.19	70119	Lane Closure, Multilane, Intermittent or Moving Operations, for Speeds \leq 40 MPH
702.00c	70200c	Impact Attenuators
702.00d	70200d	Impact Attenuators, Temporary
703.00	70300	Temporary Raised Pavement Marker
781.03	78103	Raised Reflective Pavement Markers
783.03	78303	Pavement Marking Removal
888.00	88800	Accessible Pedestrian Signals (APS)
1001.00	100100	Cement
1003.01	100301	Hot-Mix Asphalt – Fine Aggregate
1004.01	100401	Friction Aggregate
1008.27	100827	Moisture Cured Urethane Paint System
1020.00	102000	Self-Consolidating Concrete for Precast Products
1020.01	102001	Self-Consolidating Concrete for Cast-in-Place Construction
1020.02	102002	Alkali-Silica Reaction for Cast-in-Place Concrete
1020.03	102003	Alkali-Silica Reaction for Precast and Precast Prestressed Concrete
1020.05b	102005b	Concrete Admixtures
1020.05c	102005c	Concrete Mix Designs
1030.04c	103004c	Hot-Mix Asphalt – Anti-Stripping Additive
1031.00	103100	Reclaimed Asphalt Pavement (RAP)
1090.03	109003	Post Clips for Extruded Aluminum Signs
1106.02i	110602i	Movable Traffic Barrier
1106.02k	110602k	Temporary Water Filled Barrier

BDE Special Provisions

Designer Note: Insert into all contracts.

UTILITY COORDINATION AND CONFLICTS (BDE)

Effective: April 1, 2011

Revise Article 105.07 of the Standard Specifications to read:

“105.07 Cooperation with Utilities. The Department reserves the right at any time to allow work by utilities on or near the work covered by the contract. The Contractor shall conduct his/her work so as not to interfere with or hinder the progress or completion of the work being performed by utilities. The Contractor shall also arrange the work and shall place and dispose of the materials being used so as not to interfere with the operations of utility work in the area.

The Contractor shall cooperate with the owners of utilities in their removal and rearrangement operations so work may progress in a reasonable manner, duplication or rearrangement of work may be reduced to a minimum, and services rendered by those parties will not be unnecessarily interrupted.

The Contractor shall coordinate with any planned utility adjustment or new installation and the Contractor shall take all precautions to prevent disturbance or damage to utility facilities. Any failure on the part of the utility owner, or their representative, to proceed with any planned utility adjustment or new installation shall be reported promptly by the Contractor to the Engineer.”

Revise the first sentence of the last paragraph of Article 107.19 of the Standard Specifications to read:

“When the Contractor encounters unexpected regulated substances due to the presence of utilities in unanticipated locations, the provisions of Article 107.40 shall apply; otherwise, if the Engineer does not direct a resumption of operations, the provisions of Article 108.07 shall apply.”

Revise Article 107.31 of the Standard Specification to read:

“107.31 Reserved.”

Add the following four Articles to Section 107 of the Standard Specifications:

“107.37 Locations of Utilities within the Project Limits. All known utilities existing within the limits of construction are either indicated on the plans or visible above ground. For the purpose of this Article, the limits of proposed construction are defined as follows:

(a) Limits of Proposed Construction for Utilities Paralleling the Roadway.

- (1) The horizontal limits shall be a vertical plane, outside of, parallel to, and 2 ft (600 mm) distant at right angles from the plan or revised slope limits.

In cases where the limits of excavation for structures are not shown on the plans, the horizontal limits shall be a vertical plane 4 ft (1.2 m) outside the edges of structure footings or the structure where no footings are required.

- (2) The upper vertical limits shall be the regulations governing the roadbed clearance for the specific utility involved.
 - (3) The lower vertical limits shall be either the top of the utility at the depth below the proposed grade as prescribed by the governing agency or the limits of excavation, whichever is less.
- (b) Limits of Proposed Construction for Utilities Crossing the Roadway in a Generally Transverse Direction.
- (1) Utilities crossing excavations for structures that are normally made by trenching such as sewers, underdrains, etc. and all minor structures such as manholes, inlets, foundations for signs, foundations for traffic signals, etc., the limits shall be the space to be occupied by the proposed permanent construction, unless otherwise required by the regulations governing the specific utility involved.
 - (2) For utilities crossing the proposed site of major structures such as bridges, sign trusses, etc., the limits shall be as defined above for utilities extending in the same general direction as the roadway.

It is understood and agreed that the Contractor has considered in the bid all of the permanent and temporary utilities in their present and/or adjusted positions as indicated in the contract. It is further understood the actual location of the utilities may be located anywhere within the tolerances provided in 220 ILCS 50/2.8 or Administrative Code Title 92 Part 530.40(c), and the proximity of some utilities to construction may require extraordinary measures by the Contractor to protect those utilities.

No additional compensation will be allowed for any delays, inconveniences, or damages sustained by the Contractor due to the presence of or any claimed interference from known utility facilities or any adjustment of them, except as specifically provided in the contract.

107.38 Adjustments of Utilities within the Project Limits. The adjustment of utilities consists of the relocation, removal, replacement, rearrangements, reconstruction, improvement, disconnection, connection, shifting, new installation, or altering of an existing utility facility in any manner.

Utilities which are to be adjusted shall be adjusted by the utility owner or the owner's representative or by the Contractor as a contract item. Generally, arrangements for adjusting known utilities will be made by the Department prior to project construction; however, utilities will not necessarily be adjusted in advance of project construction and, in some cases, utilities will not be removed from the proposed construction limits as described in Article 107.37. When utility adjustments must be performed in conjunction with construction, the utility adjustment work will be indicated in the contract.

The Contractor may make arrangements for adjustment of utilities indicated in the contract, but not scheduled by the Department for adjustment, provided the Contractor furnishes the Department with a signed agreement with the utility owner covering the adjustments to be made. The cost of any such adjustments shall be the responsibility of the Contractor.

107.39 Contractor's Responsibility for Locating and Protecting Utility Property and Services. At points where the Contractor's operations are adjacent to properties or facilities of utility companies, or are adjacent to other property, damage to which might result in considerable expense, loss, or inconvenience, work shall not be commenced until all arrangements necessary for the protection thereof have been made.

Within the State of Illinois, a State-Wide One Call Notice System has been established for notifying utilities. Outside the city limits of the City of Chicago, the system is known as the Joint Utility Locating Information for Excavators (JULIE) System. Within the city limits of the City of Chicago the system is known as DIGGER. All utility companies and municipalities which have buried utility facilities in the State of Illinois are a part of this system.

The Contractor shall call JULIE (800-892-0123) or DIGGER (312-744-7000), a minimum of 48 hours in advance of work being done in the area, and they will notify all member utility companies involved their respective utility should be located.

For utilities which are not members of JULIE or DIGGER, the Contractor shall contact the owners directly. The plan general notes will indicate which utilities are not members of JULIE or DIGGER.

The following table indicates the color of markings required of the State-Wide One Call Notification System.

Utility Service	Color
Electric Power, Distribution and Transmission	Safety Red
Municipal Electric Systems	Safety Red
Gas Distribution and Transmission	High Visibility Safety Yellow
Oil Distribution and Transmission	High Visibility Safety Yellow
Telephone and Telegraph System	Safety Alert Orange
Community Antenna Television Systems	Safety Alert Orange
Water Systems	Safety Precaution Blue
Sewer Systems	Safety Green
Non-Potable Water and Slurry Lines	Safety Purple
Temporary Survey	Safety Pink
Proposed Excavation	Safety White (Black when snow is on the ground)

The State-Wide One Call Notification System will provide for horizontal locations of utilities. When it is determined that the vertical location of the utility is necessary to facilitate construction, the Engineer may make the request for location from the utility after receipt of notice from the Contractor. If the utility owner does not field locate their facilities to the satisfaction of the Engineer, the Engineer will authorize the Contractor in writing to proceed to locate the facilities in the most economical and reasonable manner, subject to the approval of the Engineer, and be paid according to Article 109.04.

The Contractor shall be responsible for maintaining the excavations or markers provided by the utility owners.

The Contractor shall take all necessary precautions for the protection of the utility facilities. The Contractor shall be responsible for any damage or destruction of utility facilities resulting from neglect, misconduct, or omission in the Contractor's manner or method of execution or

nonexecution of the work, or caused by defective work or the use of unsatisfactory materials. Whenever any damage or destruction of a utility facility occurs as a result of work performed by the Contractor, the utility company will be immediately notified. The utility company will make arrangements to restore such facility to a condition equal to that existing before any such damage or destruction was done.

In the event of interruption of utility services as a result of accidental breakage or as a result of being exposed or unsupported, the Contractor shall promptly notify the proper authority and shall cooperate with the said authority in the restoration of service. If water service is interrupted, repair work shall be continuous until the service is restored. No work shall be undertaken around fire hydrants until provisions for continued service have been approved by the local fire authority.

107.40 Conflicts with Utilities. Except as provided hereinafter, the discovery of a utility in an unanticipated location will be evaluated according to Article 104.03. It is understood and agreed that the Contractor has considered in the bid all facilities not meeting the definition of a utility in an unanticipated location and no additional compensation will be allowed for any delays, inconveniences, or damages sustained by the Contractor due to the presence of or any claimed interference from such facilities.

When the Contractor discovers a utility in an unanticipated location, the Contractor shall not interfere with said utility, shall take proper precautions to prevent damage or interruption of the utility, and shall promptly notify the Engineer of the nature and location of said utility.

- (a) Definition. A utility in an unanticipated location is defined as an active or inactive utility, which is either:
- (1) Located underground and (a) not shown in any way in any location on the contract documents; (b) not identified in writing by the Department to the Contractor prior to the letting; or (c) not located relative to the location shown in the contract within the tolerances provided in 220 ILCS 50/2.8 or Administrative Code Title 92 Part 530.40(c); or
 - (2) Located above ground or underground and not relocated as provided in the contract.

Service connections shall not be considered to be utilities in unanticipated locations.

- (b) Compensation. Compensation will not be allowed for delays, inconveniences, or damages sustained by the Contractor from conflicts with facilities not meeting the above definition; or if a conflict with a utility in an unanticipated location does not cause a shutdown of the work applicable to the utility or a documentable reduction in the rate of progress exceeding the limits set herein. The provisions of Article 104.03 notwithstanding, compensation for delays caused by a utility in an unanticipated location will be paid according to the provisions of this Article governing minor and major delays or reduced rate of production which are defined as follows:
- (1) Minor Delay. A minor delay occurs when the Contractor's operation is completely stopped by a utility in an unanticipated location for more than two hours, but not to exceed three weeks.
 - (2) Major Delay. A major delay occurs when the Contractor's operation is completely stopped by a utility in an unanticipated location for more than three weeks.

- (3) Reduced Rate of Production Delay. A reduced rate of production delay occurs when the contractor's rate of production decreases by more than 25 percent and lasts longer than seven days.
- (c) Payment. Payment for Minor, Major and Reduced Rate of Production Delays will be made as follows.

- (1) Minor Delay. Labor idled which cannot be used on other work will be paid for according to Article 109.04(b)(1) and (2) for the time between start of the delay and the minimum remaining hours in the work shift required by the prevailing practice in the area.

Equipment idled which cannot be used on other work will be paid for according to Article 109.04(b)(4). The length of time paid for will be the time between start of delay and eight hours working time from start of shift being worked.

For delays exceeding the initial shift, excluding Saturdays, Sundays, and holidays, Contractor-owned equipment idled by the delay which cannot be used on other work and remaining at the work site, will be paid at one-half the rate permitted in Article 109.04(b)(4) using a maximum eight hours per day for computation purposes. Equipment rented from an independent source will be paid at rates being paid by the Contractor plus move-in move-out costs, but the total amount paid will not exceed three weeks rental.

- (2) Major Delay. Labor will be the same as for a minor delay.

Equipment will be the same as for a minor delay, except Contractor-owned equipment will be limited to three weeks plus the cost of move-out to either the Contractor's yard or another job, whichever is less. Rental equipment may be paid for longer than three weeks provided the Contractor presents adequate support to the Department (including lease agreement) to show retaining equipment on the job is the most economical course to follow and in the public interest.

- (3) Reduced Rate of Production Delay. The Contractor will be compensated for the reduced productivity for labor and equipment time in excess of the 25 percent threshold for that portion of the delay in excess of seven days. Determination of compensation will be in accordance with Article 104.02, except labor and material additives will not be permitted.

Whether covered by (1), (2) or (3) above, additional traffic control required as a result of the operation(s) delayed will be paid for according to Article 109.04 for the total length of the delay.

If the delay is clearly shown to have caused work, which would have otherwise been completed, to be done after material or labor costs have increased, such increases may be paid. Payment for materials will be limited to increased cost substantiated by documentation furnished by the Contractor. Payment for increased labor rates will include those items in Article 109.04(b)(1) and (2), except the 35 percent and ten percent additives will not be permitted. On a working day contract, a delay occurring between November 30 and May 1, when work has not started, will not be considered as eligible for payment of measured labor and material costs.

Project overhead (not including interest) will be allowed when all progress on the contract has been delayed, and will be calculated as 15 percent of the delay claim.

(d) Other Obligations of Contractor. Upon payment of a claim under this provision, the Contractor shall assign subrogation rights to the Department for the Department's efforts of recovery from any other party for monies paid by the Department as a result of any claim under this Provision. The Contractor shall fully cooperate with the Department in its efforts to recover from another party any money paid to the Contractor for delay damages under this Provision."

Designer Note: If the District EEO Officer or Regional Engineer sets a DBE goal for your contract, you must insert this special provision. These goals could be for either a Federal or State contract. Check with the District EEO Officer. The Designer must insert the percentage.

DISADVANTAGED BUSINESS ENTERPRISE PARTICIPATION (DBE)

Effective: September 1, 2000

Revised: January 1, 2011

FEDERAL OBLIGATION. The Department of Transportation, as a recipient of federal financial assistance, is required to take all necessary and reasonable steps to ensure nondiscrimination in the award and administration of contracts. Consequently, the federal regulatory provisions of 49 CFR part 26 apply to this contract concerning the utilization of disadvantaged business enterprises. For the purposes of this Special Provision, a disadvantaged business enterprise (DBE) means a business certified by the Department in accordance with the requirements of 49 CFR part 26 and listed in the Illinois Unified Certification Program (IL UCP) DBE Directory.

STATE OBLIGATION. This Special Provision will also be used by the Department to satisfy the requirements of the Business Enterprise for Minorities, Females, and Persons with Disabilities Act, 30 ILCS 575. When this Special Provision is used to satisfy state law requirements on 100 percent state-funded contracts, the federal government has no involvement in such contracts (not a federal-aid contract) and no responsibility to oversee the implementation of this Special Provision by the Department on those contracts. DBE participation on 100 percent state-funded contracts will not be credited toward fulfilling the Department's annual overall DBE goal required by the US Department of Transportation to comply with the federal DBE program requirements.

CONTRACTOR ASSURANCE. The Contractor makes the following assurance and agrees to include the assurance in each subcontract that the Contractor signs with a subcontractor:

The Contractor, subrecipient, or subcontractor shall not discriminate on the basis of race, color, national origin, or sex in the performance of this contract. The Contractor shall carry out applicable requirements of 49 CFR part 26 in the award and administration of contracts funded in whole or in part with federal or state funds. Failure by the Contractor to carry out these requirements is a material breach of this contract, which may result in the termination of this contract or such other remedy as the recipient deems appropriate.

OVERALL GOAL SET FOR THE DEPARTMENT. As a requirement of compliance with 49 CFR part 26, the Department has set an overall goal for DBE participation in its federally assisted contracts. That goal applies to all federal-aid funds the Department will expend in its federally assisted contracts for the subject reporting fiscal year. The Department is required to make a good faith effort to achieve the overall goal. The dollar amount paid to all approved DBE companies performing work called for in this contract is eligible to be credited toward fulfillment of the Department's overall goal.

CONTRACT GOAL TO BE ACHIEVED BY THE CONTRACTOR. This contract includes a specific DBE utilization goal established by the Department. The goal has been included because the Department has determined that the work of this contract has subcontracting

opportunities that may be suitable for performance by DBE companies. This determination is based on an assessment of the type of work, the location of the work, and the availability of DBE companies to do a part of the work. The assessment indicates that, in the absence of unlawful discrimination, and in an arena of fair and open competition, DBE companies can be expected to perform _____% of the work. This percentage is set as the DBE participation goal for this contract. Consequently, in addition to the other award criteria established for this contract, the Department will only award this contract to a bidder who makes a good faith effort to meet this goal of DBE participation in the performance of the work. A bidder makes a good faith effort for award consideration if either of the following is done in accordance with the procedures set forth in this Special Provision:

- (a) The bidder documents that enough DBE participation has been obtained to meet the goal; or
- (b) The bidder documents that a good faith effort has been made to meet the goal, even though the effort did not succeed in obtaining enough DBE participation to meet the goal.

DBE LOCATOR REFERENCES. Bidders may consult the IL UCP DBE Directory as a reference source for DBE-certified companies. In addition, the Department maintains a letting and item specific DBE locator information system whereby DBE companies can register their interest in providing quotes on particular bid items advertised for letting. Information concerning DBE companies willing to quote work for particular contracts may be obtained by contacting the Department's Bureau of Small Business Enterprises at telephone number (217)785-4611, or by visiting the Department's web site at www.dot.il.gov.

BIDDING PROCEDURES. Compliance with this Special Provision is a material bidding requirement. The failure of the bidder to comply will render the bid not responsive.

- (a) The bidder shall submit a Disadvantaged Business Utilization Plan on Department forms SBE 2025 and 2026 with the bid.
- (b) The Utilization Plan shall indicate that the bidder either has obtained sufficient DBE participation commitments to meet the contract goal or has not obtained enough DBE participation commitments in spite of a good faith effort to meet the goal. The Utilization Plan shall further provide the name, telephone number, and telefax number of a responsible official of the bidder designated for purposes of notification of plan approval or disapproval under the procedures of this Special Provision.
- (c) The Utilization Plan shall include a DBE Participation Commitment Statement, Department form SBE 2025, for each DBE proposed for the performance of work to achieve the contract goal. For bidding purposes, submission of the completed SBE 2025 forms, signed by the DBEs and faxed to the bidder will be acceptable as long as the original is available and provided upon request. All elements of information indicated on the said form shall be provided, including but not limited to the following:
 - (1) The names and addresses of DBE firms that will participate in the contract;
 - (2) A description, including pay item numbers, of the work each DBE will perform;

- (3) The dollar amount of the participation of each DBE firm participating. The dollar amount of participation for identified work shall specifically state the quantity, unit price, and total subcontract price for the work to be completed by the DBE. If partial pay items are to be performed by the DBE, indicate the portion of each item, a unit price where appropriate and the subcontract price amount;
- (4) DBE Participation Commitment Statements, form SBE 2025, signed by the bidder and each participating DBE firm documenting the commitment to use the DBE subcontractors whose participation is submitted to meet the contract goal;
- (5) If the bidder is a joint venture comprised of DBE companies and non-DBE companies, the plan must also include a clear identification of the portion of the work to be performed by the DBE partner(s); and,
- (6) If the contract goal is not met, evidence of good faith efforts.

GOOD FAITH EFFORT PROCEDURES. The contract will not be awarded until the Utilization Plan submitted by the apparent successful bidder is approved. All information submitted by the bidder must be complete, accurate and adequately document that enough DBE participation has been obtained or document the good faith efforts of the bidder, in the event enough DBE participation has not been obtained, before the Department will commit to the performance of the contract by the bidder. The Utilization Plan will be approved by the Department if the Utilization Plan documents sufficient commercially useful DBE work performance to meet the contract goal or the bidder submits sufficient documentation of a good faith effort to meet the contract goal pursuant to 49 CFR part 26, Appendix A. The Utilization Plan will not be approved by the Department if the Utilization Plan does not document sufficient DBE participation to meet the contract goal unless the apparent successful bidder documented in the Utilization Plan that it made a good faith effort to meet the goal. This means that the bidder must show that all necessary and reasonable steps were taken to achieve the contract goal. Necessary and reasonable steps are those which, by their scope, intensity and appropriateness to the objective, could reasonably be expected to obtain sufficient DBE participation, even if they were not successful. The Department will consider the quality, quantity, and intensity of the kinds of efforts that the bidder has made. Mere *pro forma* efforts, in other words, efforts done as a matter of form, are not good faith efforts; rather, the bidder is expected to have taken genuine efforts that would be reasonably expected of a bidder actively and aggressively trying to obtain DBE participation sufficient to meet the contract goal.

- (a) The following is a list of types of action that the Department will consider as part of the evaluation of the bidder's good faith efforts to obtain participation. These listed factors are not intended to be a mandatory checklist and are not intended to be exhaustive. Other factors or efforts brought to the attention of the Department may be relevant in appropriate cases, and will be considered by the Department.
 - (1) Soliciting through all reasonable and available means (e.g. attendance at pre-bid meetings, advertising and/or written notices) the interest of all certified DBE companies that have the capability to perform the work of the contract. The bidder must solicit this interest within sufficient time to allow the DBE companies to respond to the solicitation. The bidder must determine with certainty if the DBE companies are interested by taking appropriate steps to follow up initial solicitations.

- (2) Selecting portions of the work to be performed by DBE companies in order to increase the likelihood that the DBE goals will be achieved. This includes, where appropriate, breaking out contract work items into economically feasible units to facilitate DBE participation, even when the prime Contractor might otherwise prefer to perform these work items with its own forces.
 - (3) Providing interested DBE companies with adequate information about the plans, specifications, and requirements of the contract in a timely manner to assist them in responding to a solicitation.
 - (4) a. Negotiating in good faith with interested DBE companies. It is the bidder's responsibility to make a portion of the work available to DBE subcontractors and suppliers and to select those portions of the work or material needs consistent with the available DBE subcontractors and suppliers, so as to facilitate DBE participation. Evidence of such negotiation includes the names, addresses, and telephone numbers of DBE companies that were considered; a description of the information provided regarding the plans and specifications for the work selected for subcontracting; and evidence as to why additional agreements could not be reached for DBE companies to perform the work.
 - b. A bidder using good business judgment would consider a number of factors in negotiating with subcontractors, including DBE subcontractors, and would take a firm's price and capabilities as well as contract goals into consideration. However, the fact that there may be some additional costs involved in finding and using DBE companies is not in itself sufficient reason for a bidder's failure to meet the contract DBE goal, as long as such costs are reasonable. Also, the ability or desire of a bidder to perform the work of a contract with its own organization does not relieve the bidder of the responsibility to make good faith efforts. Bidders are not, however, required to accept higher quotes from DBE companies if the price difference is excessive or unreasonable.
 - (5) Not rejecting DBE companies as being unqualified without sound reasons based on a thorough investigation of their capabilities. The bidder's standing within its industry, membership in specific groups, organizations, or associations and political or social affiliations (for example union vs. non-union employee status) are not legitimate causes for the rejection or non-solicitation of bids in the bidder's efforts to meet the project goal.
 - (6) Making efforts to assist interested DBE companies in obtaining bonding, lines of credit, or insurance as required by the recipient or Contractor.
 - (7) Making efforts to assist interested DBE companies in obtaining necessary equipment, supplies, materials, or related assistance or services.
 - (8) Effectively using the services of available minority/women community organizations; minority/women contractors' groups; local, state, and federal minority/women business assistance offices; and other organizations as allowed on a case-by-case basis to provide assistance in the recruitment and placement of DBE companies.
- (b) If the Department determines that the apparent successful bidder has made a good faith effort to secure the work commitment of DBE companies to meet the contract goal, the

Department will award the contract provided that it is otherwise eligible for award. If the Department determines that the bidder has failed to meet the requirements of this Special Provision or that a good faith effort has not been made, the Department will notify the responsible company official designated in the Utilization Plan that the bid is not responsive. The notification shall include a statement of reasons for the determination.

- (c) The bidder may request administrative reconsideration of a determination adverse to the bidder within the five working days after receipt of the notification date of the determination by delivering the request to the Department of Transportation, Bureau of Small Business Enterprises, Contract Compliance Section, 2300 South Dirksen Parkway, Room 319, Springfield, Illinois 62764 (Telefax: (217)785-1524). Deposit of the request in the United States mail on or before the fifth business day shall not be deemed delivery. The determination shall become final if a request is not made and delivered. A request may provide additional written documentation and/or argument concerning the issues raised in the determination statement of reasons, provided the documentation and arguments address efforts made prior to submitting the bid. The request will be forwarded to the Department's Reconsideration Officer. The Reconsideration Officer will extend an opportunity to the bidder to meet in person in order to consider all issues of documentation and whether the bidder made a good faith effort to meet the goal. After the review by the Reconsideration Officer, the bidder will be sent a written decision within ten working days after receipt of the request for reconsideration, explaining the basis for finding that the bidder did or did not meet the goal or make adequate good faith efforts to do so. A final decision by the Reconsideration Officer that a good faith effort was made shall approve the Utilization Plan submitted by the bidder and shall clear the contract for award. A final decision that a good faith effort was not made shall render the bid not responsive.

CALCULATING DBE PARTICIPATION. The Utilization Plan values represent work anticipated to be performed and paid for upon satisfactory completion. The Department is only able to count toward the achievement of the overall goal and the contract goal the value of payments made for the work actually performed by DBE companies. In addition, a DBE must perform a commercially useful function on the contract to be counted. A commercially useful function is generally performed when the DBE is responsible for the work and is carrying out its responsibilities by actually performing, managing, and supervising the work involved. The Department and Contractor are governed by the provisions of 49 CFR part 26.55(c) on questions of commercially useful functions as it affects the work. Specific counting guidelines are provided in 49 CFR part 26.55, the provisions of which govern over the summary contained herein.

- (a) DBE as the Contractor: 100 percent goal credit for that portion of the work performed by the DBE's own forces, including the cost of materials and supplies. Work that a DBE subcontracts to a non-DBE does not count toward the DBE goals.
- (b) DBE as a joint venture Contractor: 100 percent goal credit for that portion of the total dollar value of the contract equal to the distinct, clearly defined portion of the work performed by the DBE's own forces.
- (c) DBE as a subcontractor: 100 percent goal credit for the work of the subcontract performed by the DBE's own forces, including the cost of materials and supplies, excluding the purchase of materials and supplies or the lease of equipment by the DBE

subcontractor from the prime Contractor or its affiliates. Work that a DBE subcontractor in turn subcontracts to a non-DBE does not count toward the DBE goal.

(d)DBE as a trucker: 100 percent goal credit for trucking participation provided the DBE is responsible for the management and supervision of the entire trucking operation for which it is responsible. At least one truck owned, operated, licensed, and insured by the DBE must be used on the contract. Credit will be given for the following:

(1) The DBE may lease trucks from another DBE firm, including an owner-operator who is certified as a DBE. The DBE who leases trucks from another DBE receives credit for the total value of the transportation services the lessee DBE provides on the contract.

(2) The DBE may also lease trucks from a non-DBE firm, including from an owner-operator. The DBE who leases trucks from a non-DBE is entitled to credit only for the fee or commission it receives as a result of the lease arrangement.

(e)DBE as a material supplier:

(1) 60 percent goal credit for the cost of the materials or supplies purchased from a DBE regular dealer.

(2) 100 percent goal credit for the cost of materials or supplies obtained from a DBE manufacturer.

(3) 100 percent credit for the value of reasonable fees and commissions for the procurement of materials and supplies if not a regular dealer or manufacturer.

CONTRACT COMPLIANCE. Compliance with this Special Provision is an essential part of the contract. The Department is prohibited by federal regulations from crediting the participation of a DBE included in the Utilization Plan toward either the contract goal or the Department's overall goal until the amount to be applied toward the goals has been paid to the DBE. The following administrative procedures and remedies govern the compliance by the Contractor with the contractual obligations established by the Utilization Plan. After approval of the Utilization Plan and award of the contract, the Utilization Plan and individual DBE Participation Statements become part of the contract. If the Contractor did not succeed in obtaining enough DBE participation to achieve the advertised contract goal, and the Utilization Plan was approved and contract awarded based upon a determination of good faith, the total dollar value of DBE work calculated in the approved Utilization Plan as a percentage of the awarded contract value shall become the amended contract goal.

(a)No amendment to the Utilization Plan may be made without prior written approval from the Department's Bureau of Small Business Enterprises. All requests for amendment to the Utilization Plan shall be submitted to the Department of Transportation, Bureau of Small Business Enterprises, Contract Compliance Section, 2300 South Dirksen Parkway, Room 319, Springfield, Illinois 62764. Telephone number (217) 785-4611. Telefax number (217) 785-1524.

(b)The Contractor must notify and obtain written approval from the Department's Bureau of Small Business Enterprises prior to replacing a DBE or making any change in the

participation of a DBE. Approval for replacement will be granted only if it is demonstrated that the DBE is unable or unwilling to perform. The Contractor must make every good faith effort to find another certified DBE subcontractor to substitute for the original DBE. The good faith efforts shall be directed at finding another DBE to perform at least the same amount of work under the contract as the original DBE, to the extent needed to meet the contract goal.

- (c) Any deviation from the DBE condition-of-award or contract specifications must be approved, in writing, by the Department. The Contractor shall notify affected DBEs in writing of any changes in the scope of work which result in a reduction in the dollar amount condition-of-award to the contract.
- (d) In addition to the above requirements for reductions in the condition of award, additional requirements apply to the two cases of Contractor-initiated work substitution proposals. Where the contract allows alternate work methods which serve to delete or create underruns in condition of award DBE work, and the Contractor selects that alternate method or, where the Contractor proposes a substitute work method or material that serves to diminish or delete work committed to a DBE and replace it with other work, then the Contractor must demonstrate one of the following:
 - (1) That the replacement work will be performed by the same DBE (as long as the DBE is certified in the respective item of work) in a modification of the condition of award; or
 - (2) That the DBE is aware that its work will be deleted or will experience underruns and has agreed in writing to the change. If this occurs, the Contractor shall substitute other work of equivalent value to a certified DBE or provide documentation of good faith efforts to do so; or
 - (3) That the DBE is not capable of performing the replacement work or has declined to perform the work at a reasonably competitive price. If this occurs, the Contractor shall substitute other work of equivalent value to a certified DBE or provide documentation of good faith efforts to do so.
- (e) Where the revision includes work committed to a new DBE subcontractor, not previously involved in the project, then a Request for Approval of Subcontractor, Department form BC 260A, must be signed and submitted.
- (f) If the commitment of work is in the form of additional tasks assigned to an existing subcontract, then a new Request for Approval of Subcontractor shall not be required. However, the Contractor must document efforts to assure that the existing DBE subcontractor is capable of performing the additional work and has agreed in writing to the change.
- (g) All work indicated for performance by an approved DBE shall be performed, managed, and supervised by the DBE executing the Participation Statement. The Contractor shall not terminate for convenience a DBE listed in the Utilization Plan and then perform the work of the terminated DBE with its own forces, those of an affiliate or those of another subcontractor, whether DBE or not, without first obtaining the written consent of the Bureau of Small Business Enterprises to amend the Utilization Plan. The Contractor shall notify the Bureau of Small Business Enterprises of any termination for reasons

other than convenience, and shall obtain approval for inclusion of the substitute DBE in the Utilization Plan. If good faith efforts following a termination of a DBE for cause are not successful, the Contractor shall contact the Bureau of Small Business Enterprises and provide a full accounting of the efforts undertaken to obtain substitute DBE participation. The Bureau of Small Business Enterprises will evaluate the good faith efforts in light of all circumstances surrounding the performance status of the contract, and determine whether the contract goal should be amended.

- (h) The Contractor shall maintain a record of payments for work performed to the DBE participants. The records shall be made available to the Department for inspection upon request. After the performance of the final item of work or delivery of material by a DBE and final payment therefore to the DBE by the Contractor, but not later than thirty calendar days after payment has been made by the Department to the Contractor for such work or material, the Contractor shall submit a DBE Payment Agreement on Department form SBE 2115 to the Regional Engineer. If full and final payment has not been made to the DBE, the DBE Payment Agreement shall indicate whether a disagreement as to the payment required exists between the Contractor and the DBE or if the Contractor believes that the work has not been satisfactorily completed. If the Contractor does not have the full amount of work indicated in the Utilization Plan performed by the DBE companies indicated in the Utilization Plan and after good faith efforts are reviewed, the Department may deduct from contract payments to the Contractor the amount of the goal not achieved as liquidated and ascertained damages. The Contractor may request an administrative reconsideration of any amount deducted as damages pursuant to subsection (j) of this part.
- (i) The Department reserves the right to withhold payment to the Contractor to enforce the provisions of this Special Provision. Final payment shall not be made on the contract until such time as the Contractor submits sufficient documentation demonstrating achievement of the goal in accordance with this Special Provision or after liquidated damages have been determined and collected.
- (j) Notwithstanding any other provision of the contract, including but not limited to Article 109.09 of the Standard Specifications, the Contractor may request administrative reconsideration of a decision to deduct the amount of the goal not achieved as liquidated damages. A request to reconsider shall be delivered to the Contract Compliance Section and shall be handled and considered in the same manner as set forth in paragraph (c) of "Good Faith Effort Procedures" of this Special Provision, except a final decision that a good faith effort was not made during contract performance to achieve the goal agreed to in the Utilization Plan shall be the final administrative decision of the Department.

Designer Notes: Insert into all contracts.

LIQUIDATED DAMAGES (BDE)

Effective: April 1, 2009

Revised: April 1, 2011

Revise the table in Article 108.09 of the Standard Specifications to read:

"Schedule of Deductions for Each Day of Overrun in Contract Time			
Original Contract Amount		Daily Charges	
From More Than	To and Including	Calendar Day	Work Day
\$ 0	\$ 100,000	\$ 475	\$ 675
100,000	500,000	750	1,050
500,000	1,000,000	1,025	1,425
1,000,000	3,000,000	1,275	1,725
3,000,000	6,000,000	1,425	2,000
6,000,000	12,000,000	2,300	3,450
12,000,000	And over	5,800	8,125"

Designer Note: Insert into all contracts utilizing mulch, erosion blankets, or turf reinforcement mat.

MULCH AND EROSION CONTROL BLANKETS (BDE)

Effective: November 1, 2010

Revised: April 1, 2011

Revise the first sentence of Article 251.03 of the Standard Specifications to read:

“Within 24 hours of seed placement, mulch by one of the following methods shall be placed on the areas specified.”

Revise Article 251.03(b)(2) of the Standard Specifications to read:

“(2) Procedure 2. This procedure shall consist of stabilizing the straw with an approved mulch blower followed immediately by an overspray application of light-duty hydraulic mulch. The hydraulic mulch shall be according to Article 251.03(c) except that it shall be applied as a slurry of 900 lb (1020 kg) of mulch and 1000 gal (9500 L) of water per acre (hectare) using a hydraulic mulch applicator. The light-duty hydraulic mulch shall be agitated a minimum of five minutes before application and shall be agitated during application. The light-duty hydraulic mulch shall be applied from opposing directions to ensure even coverage.”

Revise Article 251.03(c) of the Standard Specification to read:

“(c) Method 3. This method shall consist of the machine application of a light-duty hydraulic mulch. Seeding shall be conducted as a separate operation and shall not be added to the hydraulic mulch slurry. Hydraulic mulch shall not be applied when the ambient temperature is at or below freezing. To achieve full and even coverage, the hydraulic mulch shall be applied from two opposing directions. Mixing and application rates shall be according to the manufacturer’s recommendations and meet the minimum application rates set in Article 1081.06(a)(2).”

Revise Article 251.03(d) of the Standard Specifications to read:

“(d) Method 3A. This method shall consist of the machine application of a heavy-duty hydraulic mulch. Seeding shall be conducted as a separate operation and shall not be added to the hydraulic mulch slurry. The hydraulic mulch shall not be applied when the ambient temperature is at or below freezing. To achieve full and even coverage, the hydraulic mulch shall be applied from two opposing directions. Mixing and application rates shall be according to the manufacturer’s recommendations and meet the minimum application rates set in Article 1081.06(a)(2). The heavy-duty hydraulic mulch shall be applied using a mechanically agitated hydraulic mulching machine.”

Add the following to Article 251.03 of the Standard Specifications:

“(e) Method 4. This method shall consist of applying compost combined with a performance additive designed to bind/stabilize the compost. The compost/performance additive mixture shall be applied to the surface of the slope using a pneumatic blower at a depth of 2 in. (50 mm).”

Revise Article 251.04 of the Standard Specifications to read:

“251.04 Erosion Control Blanket. Erosion control blanket may be placed using either excelsior blanket or knitted straw blanket. Within 24 hours of seed placement, blanket shall be placed on the areas specified. Prior to placing the blanket, the areas to be covered shall be relatively free of rocks or clods over 1 1/2 in. (40 mm) in diameter, and sticks or other foreign material which will prevent the close contact of the blanket with the seed bed. If, as a result of rain, the prepared seed bed becomes crusted or eroded, or if eroded places, ruts, or depressions exist for any reason, the Contractor shall rework the soil until it is smooth and reseed such areas which are reworked.

After the area has been properly shaped, fertilized, and seeded, the blanket shall be laid out flat, evenly, and smoothly, without stretching the material. The excelsior and knitted straw blankets shall be placed so that the netting is on the top and the fibers are in contact with the soil. The heavy duty blankets shall be placed so that the heavy duty extruded plastic mesh is on the bottom.

For placement in ditches, the erosion control blanket shall be applied parallel to the centerline of the ditch so that there are no longitudinal seams within 2 ft (600 mm) of the bottom centerline of the ditch. The blanket shall be toed in on the upslope edge and shingled or overlapped with the flow.

On slopes, the blanket shall be applied either horizontally or vertically to the contour, toed in on the upslope edge, and shingled or overlapped with the flow.

When placed adjacent to the roadway, blankets shall be toed in along the edge of shoulder.

Anchoring the blankets shall be according to the manufacturer’s specifications.”

Revise Article 251.06(b) of the Supplemental Specifications to read:

“(b) Measured Quantities. Mulch Methods 1, 2, 3, 3A and 4 will be measured for payment in place in acres (hectares) of surface area mulched. Erosion control blanket, heavy duty erosion control blanket, and turf reinforcement mat will be measured for payment in place in square yards (square meters).”

Revise Article 251.07 of the Supplemental Specifications to read:

“251.07 Basis of Payment. This work will be paid for at the contract unit price per acre (hectare) for MULCH, METHOD 1; MULCH, METHOD 2; MULCH, METHOD 3; MULCH, METHOD 3A; MULCH, METHOD 4; and at the contract unit price per square yard (square meter) for EROSION CONTROL BLANKET, HEAVY DUTY EROSION CONTROL BLANKET, or TURF REINFORCEMENT MAT.”

Revise Article 1081.06(a)(2) of the Standard Specifications to read:

“(2) Hydraulic Mulch. The mulch component shall be comprised of a minimum of 70 percent biodegradable material such as wood cellulose, paper fibers, straw or cotton and shall contain no growth or germination inhibiting factors. The remainder of the components shall consist of the manufacturer’s choice of tackifiers and/or strengthening fibers needed to meet the performance specifications. Tackifiers shall be non-toxic and LC 50 test results shall be provided along with the manufacturer’s certification. Hydraulic mulch shall disperse evenly and rapidly and remain in slurry

when agitated with water. When uniformly applied, the slurry shall form an absorbent cover allowing percolation of water to the underlying surface. Hydraulic mulch shall be packaged in UV and moisture resistant factory labeled packages or bags with the net quantity of the packaged material plainly shown on each package. The biodegradable material shall be relatively free of glossy papers and shall not be water soluble. The hydraulic mulches shall be according to the following.

Light-Duty Hydraulic Mulch	
Property ^{1/}	Value
Functional Longevity ^{2/}	3 months
Minimum Application Rates	2000 lb/acre (2240 kg/ha)
Typical Maximum Slope Gradient (V:H)	≤ 1:3
Maximum Uninterrupted Slope Length	50 ft (15 m)
Maximum C Factor	0.15
Minimum Vegetation Establishment ^{5/}	200 %

Heavy-Duty Hydraulic Mulch	
Property ^{1/}	Value
Functional Longevity ^{2/}	12 months
Minimum Application Rates	3000 lb/acre (3360 kg/ha)
Typical Maximum Slope Gradient (V:H)	≤ 1:2
Maximum Uninterrupted Slope Length	100 ft (30 m)
Maximum C Factor ^{3/ 4/}	0.02
Minimum Vegetation Establishment ⁵	400 %

- 1/ This table sets minimum requirements only. Refer to manufacturer recommendations for application rates, instructions, gradients, maximum continuous slope lengths and other site specific recommendations.
- 2/ Manufacturer's estimated time period, based upon field observations, that a material can be anticipated to provide erosion control as influenced by its composition and site-specific conditions.
- 3/ "C" Factor calculated as ratio of soil loss from HECP protected slope (tested at specified or greater gradient, h:v) to ratio of soil loss from unprotected (control) plot based on large-scale testing.
- 4/ Large-scale test methods shall be according to ASTM D 6459.
- 5/ Minimum vegetation establishment shall be calculated according to ASTM D 7322.

The manufacturer shall furnish a certification with each shipment of hydraulic mulch stating the number of packages or bags furnished and that the material complies with these requirements."

Designer Note: Insert into all contracts using cape seal. Use of this special provision shall be according to the Bureau of Design and Environment Manual, Chapter 52.

The designer must specify the following information on the plans:

- Aggregate gradation for the A-1 bituminous surface treatment.
- Friction aggregate mixture for the micro-surfacing.

Districts are encouraged to use the CA 20 gradation as it has proven to perform well for A-1 surface treatments.

Include the following information in the Traffic Control Plan Special Provision:

- Contractor shall post the roadway with "LOOSE GRAVEL" and SPEED LIMIT 35" signs in accordance with applicable articles of Division 700 of the Standard Specifications. These signs shall be placed at the start of the work, near intersecting roadways and then at an average spacing of 0.5 mi (0.8 km). The signs may be removed as soon as the sweeping operation has been completed.

Include Special Provision on Temporary Flexible Raised Pavement Marker with this work.

PREVENTIVE MAINTENANCE - CAPE SEAL (BDE)

Effective: January 1, 2009

Revised: April 1, 2009

Description. This work shall consist of constructing a single bituminous surface treatment (A-1) and a micro-surfacing on existing hot-mix asphalt (HMA) surfaces.

Materials. Materials shall be according to the following.

- (a) A-1 Surface Treatment. Materials shall be according to the following Articles/Sections of the Standard Specifications.

Item	Article/Section
(1) Seal Coat Aggregate (Note 1)	1003, 1004.03
(2) Bituminous Materials (Note 2)	1032

Note 1. The seal coat aggregate shall be either fine or coarse aggregate.

When fine aggregate is used, it shall be stone sand (other than limestone and dolomite), wet bottom boiler slag, slag sand, or steel slag sand. The aggregate quality shall be Class C. The aggregate gradation shall be FA 1 (Special) or FA 4 (Special) as specified on the plans at shall meet the following.

FINE AGGREGATE GRADATIONS						
Grad. No.	Sieve Size and Percent Passing					
	3/8 in. (9.5 mm)	No. 4 (4.75 mm)	No. 8 (2.36 mm)	No. 16 (1.18 mm)	No. 40 (425 um)	No. 200 (75 um)
FA 1 (Special)	100	90 ± 10	62.5 ± 17.5	32.5 ± 7.5	7.5 ± 7.5	1.5 ± 1
FA 4 (Special)	100	--	--	2 ± 2	--	1.5 ± 1

When coarse aggregate is used, it shall be crushed gravel, crushed stone, wet bottom boiler slag, crushed slag, crushed sandstone, or crushed steel slag. The aggregate quality shall be Class C and the aggregate shall have a maximum of 25 percent chert by weight (mass). The aggregate gradation shall be CA 15, CA 16, or CA 20 as specified on the plans.

Note 2. The bituminous material shall be either a CRSP or an HFP polymer modified emulsified asphalt meeting the requirements of Article 1032.06(f)(2) of the Standard Specifications.

- (b) Micro-Surfacing. Materials shall be according to the following Articles/Sections of the Standard Specifications.

Item	Article/Section
(1) Mineral Filler (Note 1)	1001
(2) Water	1002
(3) Coarse Aggregate (Note 2)	1004.03
(4) Latex-Modified Emulsified Asphalt (Note 3)	
(5) Additives (Note 4)	

Note 1. The mineral filler shall be Type 1 portland cement.

Note 2. The coarse aggregate material shall be selected from the table in Article 1004.03(a) of the Standard Specifications based upon the friction aggregate mixture specified. The quality of the aggregate shall be Class B and the gradation shall be as shown in the table below.

Sieve Size	% Passing
3/8 in. (9.5 mm)	100
No. 4 (4.75 mm)	95 ± 5
No. 8 (2.36 mm)	77 ± 13
No. 16 (1.18 mm)	57 ± 13
No. 30 (600 µm)	40 ± 10
No. 50 (330 µm)	24 ± 6
No. 100 (150 µm)	15 ± 6
No. 200 (75 µm)	10 ± 5

To assure the material is totally crushed, 100 percent of the parent aggregate shall be larger than the largest stone in the gradation to be used.

The blending, alternate use, and/or substitutions of aggregates from different sources for use in this work will not be permitted without the approval of the Engineer. Any blending shall be by interlocked mechanical feeders. The blending shall be uniform, compatible with the other components of the mix, and the equipment shall be approved by the Engineer.

If blending aggregates, the blend shall have a washed gradation performed every other day or a minimum of three tests per week. Testing shall be completed before the aggregate receives final acceptance for use in the mix. All gradation tests shall be conducted according to the aggregate gradation control system (AGCS).

Aggregates shall be screened at the stockpile prior to delivery to the paving machine to remove oversized material or contaminants.

Note 3. CSS-1h Latex Modified Emulsified Asphalt. The emulsified asphalt shall be a quick-traffic latex modified asphalt emulsion containing a minimum of 3.0 percent latex solids by weight of asphalt binder. The latex shall be milled or blended into the emulsifier solution prior to the emulsification process. The CSS-1h latex modified emulsified asphalt shall be according to the following.

Test (AASHTO T 59)	Result
Viscosity, Saybolt Furol, 77 °F (25 °C), SFS	20-100
Storage Stability Test, 24 hours, %	1 max.
Particle Charge Test	Positive
Sieve Test, No. 20 (850 µm), retained on sieve, %	0.10 max.
Distillation Test, Residue from distillation test to 347 ± 9 °F (175 ± 5 °C), %	62 min.

Tests on residue from distillation	Result
Penetration, 77 °F (25 °C), 100 grams, 5 seconds, (AASHTO T 49), dmm	40-90
Ductility, 77 °F (25 °C), 50 mm/min, (AASHTO T 51), mm	400 min.
Solubility in trichloroethylene, (AASHTO T 44), %	97.5 min.
Softening Point, (AASHTO T 53), °F (°C)	135 (57) min.
Absolute Viscosity, 140 °F (60 °C), (AASHTO T 202), Poises (Pa · sec)	8,000 (800) min.

Note 4. Additives may be added to the emulsion mix or any of the component materials to provide the control of the quick-traffic properties. They shall be included as part of the mix design and be compatible with the other components of the mix.

(c) Crack/Joint Sealant. The crack/joint sealant shall be a fiber-modified asphalt binder mixed at the jobsite or premixed.

(1) Jobsite-Mixed Sealant. The sealant shall consist of an asphalt binder and fibers, and be according to the following.

- a. Asphalt Binder. The asphalt binder shall be PG 58-28, PG 58-22, or PG 64-22 and meet the requirements of Article 1032.05 of the Standard Specifications.
- b. Fibers. Fibers shall be short cut polypropylene fibers meeting the properties listed below. The fiber may be accepted on certification from the manufacturer that it meets the specified requirements.

Property	Value
Length, in. (mm)	0.3 - 0.5 (8 - 12)
Denier	13-16
Crimps	None
Tensile Strength, min., psi (kPa)	40,000 (275,000)
Specific Gravity (typical)	0.91
Moisture Regain @ 70 °F (21 °C) and 65% RH (typical), %	0.1

- c. Percent Fibers. The sealant shall contain a minimum of 8.0 percent of fibers by weight (mass).
 - d. Sealant Heating. The sealant shall be heated in the kettle at temperatures between 255 and 285 °F (124 and 141 °C).
- (2) Premixed Sealant. The sealant shall be packaged and consist of an asphalt binder, fibers, and other modifiers meeting the following requirements. The sealant and its components may be accepted on certification from the manufacturer that it meets the specified requirements.
- a. Asphalt Binder. The asphalt binder shall be PG 64-22 and meet the requirements of Article 1032.05 of the Standard Specifications.
 - b. Fibers. Fibers shall be short cut polyester fibers meeting the following.

Property	Value
Length, in. (mm)	0.25 ± 0.02 (6.3 ± 0.5)
Denier	3 - 6
Crimps	None
Tensile Strength, min., psi (kPa)	70,000 (482,000)
Specific Gravity (typical)	1.32 - 1.40
Elongation at Break, %	35 - 38
Melt Temperature, °F (°C)	475 - 490 (246 - 254)

- c. Percent Fibers. The sealant shall contain 5.0 ± 0.5 percent of fibers by weight (mass).

The sealant, in its final form, shall meet the following requirements when sampled and heated to the manufacturer's recommended maximum heating temperature according to ASTM D 5167.

Test	Value
Cone Penetration @ 77 °F (25 °C), ASTM D 5329	10 - 35 dmm
Softening Point, ASTM D 36	175 °F (79 °C) min.
Maximum Heating Temperature	400°F (204 °C)
Application Temperature	350°F (177 °C) min.

Equipment. Equipment shall be according to the following.

- (a) A-1 Surface Treatment. Equipment shall be according to the following Articles/Sections of the Standard Specifications.

Item	Article/Section
(1) Self-Propelled Pneumatic-Tired Roller (Note 1)	1101.01
(2) Mechanical Sweeper (Note 2)	1101.03
(3) Aggregate Spreaders (Note 3)	1102.04
(4) Pressure Distributor (Note 4)	1102.05
(5) Heating Equipment	1102.07

Note 1. There shall be a minimum of two rollers, with the final number of rollers determined by the rollers' abilities to maintain proper spacing with the aggregate spreader as directed by the Engineer.

Note 2. The mechanical sweeper shall be power driven and self-propelled with the broom located between the axles. The mechanical sweeper shall not use a cantilever-mounted broom and the broom rotation shall not be operated by forward movement.

Note 3. The aggregate spreader shall be a self-propelled mechanical type with the receiving hopper in the rear and shall pull the aggregate truck. The spreader shall be fitted with an automated system which provides positive interconnected control of the aggregate flow with the forward speed of the spreader. The automated system shall provide uniform and consistent aggregate application at the rate specified.

The Engineer will check the spread roll of the aggregate spreader for straightness each day before operations begin. Should the surface of the spread roll vary off a straight line along its longitudinal dimension by more than 1/16 in. (1.5 mm), the Engineer will inspect the application of aggregate for corrugations and, should these occur, the machine shall be repaired or replaced. The forward speed of the spreader during calibration shall be the same as is to be used during construction. The equipment required for aggregate spreader calibration may consist of several sheets of canvas, each being exactly 1 sq yd (0.8 sq m), and a weight scale. By making several runs at different gate openings over the sheets of canvas, placed to cover the full width applied by the spreader, and carefully measuring the aggregate on each canvas sheet, the gate opening at the pre-established speed required to apply aggregate at the specified rate may be determined.

Note 4. The pressure distributor shall have a minimum capacity of 3000 gal (11,500 L). The application rate control shall be automated and shall control the application rate regardless of ground speed or spray bar width. The computer shall have the capability of recording the application rate, gallons sprayed, square yards, and feet traveled. The pressure distributor shall be capable of maintaining the asphalt emulsion at the specified temperature. The spray bar nozzles shall produce a uniform triple lap application fan spray, and the shutoff shall be instantaneous, with no dripping. The pressure distributor shall be capable of maintaining the specified application rate within ± 0.015 gal/sq yd (± 0.070 L/sq m) for each load. The spray-bar nozzles shall be turned to make the same angle with the longitudinal axis of the spray bar as recommended by the manufacturer.

Application rates shall be determined by the procedures listed in ASTM D 2995, except the sample may be taken on three 8 x 12 in. (200 x 300 mm) metal plates. The three plates shall be positioned as directed by the Engineer.

(b) Micro-Surfacing. Equipment shall be according to the following.

- (1) Micro-Surfacing Mixing Machine. The machine shall be either a continuous (self-loading) machine or a non-continuous (self-contained) machine depending on the size of the project as described below. Both types of machines shall have sufficient storage capacity for aggregate, emulsified asphalt, mineral filler, control additive and water to maintain an adequate supply to the proportioning controls.

Machines that are the continuous (self-loading) type shall be an automatic-sequenced, self-propelled, continuous-flow mixing unit able to accurately deliver and proportion the aggregate, emulsified asphalt, mineral filler, control setting additive, and water to a revolving multi-blade, double-shafted mixer and to discharge the mixed product on a continuous-flow basis. The machine shall be equipped to allow the operator to have full control of the forward and reverse speeds during

applications of the material and be equipped with opposite-side driver stations to assist in alignment.

Non-continuous (self-contained) machines will be allowed on projects with a length of 2 lane-miles (3.2 lane-km) or less. For mainline paving, the Contractor shall have at least three self-contained machines in continuous operation to ensure appropriate production rates. Self-contained machines will also be allowed on shoulders, ramps, short applications as bridge decks, or where the material can be placed in a single loading capacity of the machine.

Each mixing unit to be used in the performance of the work shall be calibrated in the presence of the Engineer prior to construction. Each new or different aggregate requires a new calibration. Previous calibration documentation covering the exact materials to be used may be acceptable, provided that no more than 30 days have lapsed. The documentation shall include an individual calibration of each material at various settings, which can be related to the machine metering devices. Prior to the calibration process, portable scales used to calibrate the mixing machine for emulsion and aggregate shall be checked with 25 lb and 50 lb weights, respectively. Results from the standard weight checks shall be furnished to the Engineer. No machine will be allowed to work on the project until the calibration has been completed and/or accepted.

- (2) Micro-Surfacing Spreader. The mixture shall be agitated and spread uniformly in the surfacing box by means of twin-shafted paddles or spiral augers fixed in the spreader box. A front seal shall be provided to insure no loss of the mixture at the road contact point. The rear seal shall act as a final strike-off and shall be adjustable. The spreader box and rear strike-off shall be so designed and operated that a uniform consistency is achieved to produce a free flow of material to the rear strike-off. The spreader box shall have suitable means provided to side shift the box to compensate for variations in the pavement geometry.

A secondary strike-off shall be provided to improve surface texture on the surface course. The secondary strike-off shall have the same adjustments as the spreader box and shall not bounce, wobble, or chatter.

When required on the plans, before the final surface course is placed, preliminary micro-surfacing material may be required to fill ruts, utility cuts, depressions in the existing surface, etc. Ruts of 1/2 in. (13 mm) or greater in depth shall be filled independently with a rut-filling spreader box, either 5 or 6 ft (1.5 or 1.8 m) in width. For irregular or shallow rutting of less than 1/2 in. (13 mm) in depth, a full-width scratch-coat pass may be used as directed by the Engineer utilizing either a stiff primary rubber or else a metal primary strike off. Ruts that are in excess of 1 1/2 in. (38 mm) in depth may require multiple placements with the rut-filling spreader box to restore the cross-section. All rut-filling level-up material should cure under traffic for a minimum of 24 hours before additional material is placed on top of the level up.

- (3) Micro-Surfacing Proportioning Devices. Individual volume or weight controls for proportioning each material to be added to the mix (i.e. aggregate, mineral filler, emulsified asphalt, additive, and water) shall be provided and properly marked. These proportioning devices are used in material calibration and determining the material output at any time. Calibration records, conversion formulas, and daily run sheets including the beginning and final numbers shown on the proportioning devices shall be submitted to the Engineer for approval. During production any deviations from the original JMF shall be approved by the Engineer.

(c) Crack/Joint Sealing. Equipment shall be according to the following.

- (1) Air Compressor. The air compressor shall be capable of producing a minimum pressure of 90 psi (620 kPa) at the end of the discharge hose. The air stream shall discharge onto the pavement through an appropriate air lance. The tool lubricator shall be bypassed and a filter installed on the discharge valve to keep water and oil out of the line.
- (2) Oil Kettle. The crack sealant shall be heated in an oil jacketed double wall kettle equipped with an agitator (reversing rotary auger action) and separate thermometers for the oil bath and mixing chamber. The unit shall also be equipped with a reversible hydraulic 2 in. (50 mm) hot asphalt pump and a recirculating pump to circulate the oil bath.

CONSTRUCTION REQUIREMENTS

Weather Limitations. Placement of the A-1 bituminous surface treatment shall be done between May 1 and August 31, with the micro-surfacing being placed according to the timeframe specified herein. Bituminous materials shall be applied only when the temperature of the air in the shade is above 55 °F (13 °C). No work shall be started if local conditions indicate that rain is imminent.

The A-1 bituminous surface treatment may be done between September 1 and September 15 provided both of the following conditions are met:

- (a) The temperature of the air in the shade is above 70 °F (20 °C) and the temperature of the surface to which the asphalt will be applied is 70 °F (20 °C) or above, and
- (b) The National Weather Service forecast for the area does not show any rain or any temperatures below 55 °F (13 °C) for the day the work is to be done or for the following five days.

Mix Design. A Contractor provided laboratory shall develop the mix design for the micro-surfacing mixture, shall verify the functioning of the set regulating additives, and shall present certified test results for the Engineer's approval. This laboratory shall be recognized by the International Slurry Surfacing Association (ISSA) as being capable of performing mix designs. The Engineer will verify the laboratory tests required in ISSA A143 have been conducted.

Proportions for the mix design shall be within the following limits.

Mineral Aggregate, dry weight (mass) lb/sq yd (kg/sq m)	15-50 (8-30)
Latex Emulsified Asphalt Residue, % by wt. of Aggregate	5.5-10.5
Latex Base Modifier	As required with % by weight (mass) of binder min. of 3.0
Mix Set Additive	As required
Mineral Filler, % by weight (mass) of aggregate	0.25 - 3 depending on weather conditions

The amount of mineral filler needed shall be determined by the laboratory mix design and will be considered as part of the aggregate gradation.

The amount and type of latex shall be determined by the laboratory performing the mix design. The minimum amount required shall be based on asphalt weight content and shall be certified by the emulsion supplier.

Compatibility of the aggregate, latex-modified emulsified asphalt, mineral filler, and other additives shall be verified by the mix design. The materials shall meet the following requirements for ISSA A143.

ISSA Test No.	Description	Specification
ISSA TB-139	Wet Cohesion @ 30 minutes min. (Set) @ 60 minutes min. (Traffic)	12 kg-cm min. 20 kg-cm min. or Near Spin
ISSA TB-109	Excess Asphalt by LWT Sand Adhesion	50 gm/sq ft (538 gm/sq m) max.
ISSA TB-114	Wet Stripping	Pass (90% min.)
ISSA TB-100	Wet-Track Abrasion Loss One-hour Soak Six-day Soak	50 gm/sq ft (538 gm/sq m) max. 75 gm/sq ft (807 gm/sq m) max.
ISSA TB-147	Lateral Displacement	5% max.
	Specific Gravity after 1,000 Cycles of 25 lb (11.34 kg)	2.10 max.
ISSA TB-144	Classification Compatibility	11 Grade Points min. (AAA, BAA)
ISSA TB-113	Mix Time @ 77 °F (25 °C)	Controllable to 120 seconds min.

The mixing test and set-time test shall be checked at the highest temperatures expected during construction.

The mix design shall report the quantitative effects of moisture content on the unit weight of the aggregate (bulking effect). The report shall clearly show the proportions of aggregate, mineral filler (minimum and maximum), water (minimum and maximum), additive usage, and latex-modified asphalt emulsion based on the dry weight of the aggregate.

For the aggregate blend in the mix design, test results for AASHTO T 176 shall be provided with the mix information to the Engineer. Aggregate test values below 65 shall require review and approval from the Engineer.

Before the work commences, the Contractor shall submit to the Engineer a complete mix design covering the specific materials to be used on the project. The percentages of each individual material required shall be shown in the laboratory report. The Engineer shall approve the mix design prior to its use. After approval, no substitutions will be permitted, unless approved by the Engineer, and the Contractor shall maintain continuous control of the latex-modified emulsified asphalt to dry aggregate proportioning to conform to the approved mix design within a tolerance of ± 2 gal/ton (± 8 L/metric ton).

Micro-Surfacing Test Strip. For projects over 100,000 sq yd (83,600 sq m), at least one day prior to starting the project the Contractor shall designate a mutually agreeable location and apply a test strip of micro-surfacing using the aggregate indicated in the mix design. The Engineer will evaluate the micro-surfacing application rate and cure time.

Surface Preparation. Pavement markings shall be removed according to Article 783.03(a) of the Standard Specifications. Only very small particles of tightly adhering existing markings may remain in place.

When specified in the plans, pavement markers shall be removed according to Article 783.03(b) of the Standard Specifications.

Bumps greater than or equal to 1/2 in. (13 mm) shall be removed by grinding. The Contractor shall determine bump grinding locations in the presence of the Engineer by using a 16 ft (5 m) straightedge with the scratcher bolts set to 1/2 in. (13 mm). All locations marked by the scratcher bolts shall be ground using either a grinding machine consisting of multiple saws or a cold-milling machine with a double- or triple-wrap milling head.

Joints and cracks 3/16 in. (5 mm) or wider shall be cleaned of loose and unsound material and sealed. The sealant shall be applied only when the joints and cracks are clean and dry and the ambient temperature is 40-85 °F (4-29 °C). The sealant shall be applied using a pressurized wand delivery system with such devices as necessary to seal the cracks/joints and form a nominal 0.125 in. (3 mm) thick by 3 in. (75 mm) wide overseal band centered so that the center of the 3 in. (75 mm) wide band is within 1 in. (25 mm) of the crack. The sealant shall be allowed to cure before opening to traffic. When approved by the Engineer, the sealer may be dusted with fine sand, portland cement, or mineral filler to prevent tracking.

Prior to applying the A-1 bituminous surface treatment, the pavement surface shall be cleaned.

Manholes, valve boxes, drop inlets, and other service entrances shall be protected from the cape seal by a suitable method. The surface preparation shall be approved by the Engineer prior to application of the A-1 bituminous surface treatment. No dry aggregate either spilled from the lay-down machine or existing on the road, will be permitted.

Calibration. The working day prior to starting construction of the A-1 bituminous surface course, the pressure distributor and aggregate spreader shall be calibrated and adjusted according to the manufacturer's recommendations. At least three days prior to starting the work the Contractor shall provide the Engineer with a copy of the manufacturer's recommendations for the equipment to be used. All calibrations and adjustments shall be made in the presence of the Engineer on a level surface at a location approved by the Engineer. The Contractor shall maintain proper calibration and adjustment of the equipment and the Engineer reserves the right to check application rates as the work progresses. Should the equipment fail to consistently apply the specified rates, the work shall be stopped and the Contractor shall recalibrate and readjust the equipment.

Application. The cape seal shall be applied as shown on the plans and the following.

- (a) A-1 Bituminous Surface Treatment. The bituminous material and aggregate shall be applied according to the following.
 - (1) Application Rates. Based upon the aggregate gradation to be used, the Contractor shall determine the application rates of bituminous material and seal coat aggregate. The application rates along with the seal coat gradations shall be submitted to the Engineer for approval prior to the start of work. Application rates shall be according to the following table for the aggregate type shown on the plans, and shall result in aggregate embedment between 50 and 70 percent behind the roller. Changes in the application rate of greater than 15 percent shall be resubmitted to the Engineer for approval.

Aggregate Type	Bituminous Material Rate	Aggregate Rate
CA 15	0.38 – 0.46 gal/sq yd (1.7 – 2.1 L/sq m)	22 – 30 lb/sq yd (12 – 16 kg/sq m)
CA 16	0.36 – 0.40 gal/sq yd (1.6 – 1.8 L/sq m)	18 – 26 lb/sq yd (8 – 14 kg/sq m)
CA 20	0.36 – 0.40 gal/sq yd (1.6 – 1.8 L/sq m)	18 – 26 lb/sq yd (8 – 14 kg/sq m)
FA 1 (Special)	0.26 – 0.30 gal/sq yd (1.2 – 1.4 L/sq m)	16 – 20 lb/sq yd (9 – 11 kg/sq m)
FA 4 (Special)	0.28 – 0.36 gal/sq yd (1.3 – 1.6 L/sq m)	18 – 24 lb/sq yd (10 – 13 kg/sq m)

- (2) Preparation of Bituminous Material. The temperature of the bituminous material at the time of application shall be such that it will spray uniformly without clogging the spraying nozzles and shall be applied within the temperature ranges of 150 – 190 °F (65 – 90 °C).
- (3) Preparation of Aggregate. The aggregate shall be stockpiled near the jobsite according to Article 1003.01(e) or 1004.01(e) of the Standard Specifications. The aggregate used shall contain no free moisture. Slightly damp aggregate may be used with the approval of the Engineer.
- (4) Application of Bituminous Material. The bituminous material shall be applied with a pressure distributor. The entire length of the spray bar shall be set at the height above the surface recommended by the manufacturer for even distribution of the bituminous material.

The distributor shall be operated in a manner such that missing or overlapping of transverse joints is avoided. To prevent overlapping of successive applications of bituminous material at transverse joints, heavy paper shall be spread over the previously applied bituminous material and aggregates. In order to obtain a uniform application of the bituminous material, the distributor shall be traveling at the speed required for the specified rate of application when the spray bar crosses the paper.

Adjacent construction, such as concrete pavement, curb and gutter, bridge floors, raised reflective pavement markers, and bridge handrails, shall be protected by shields, covers or other means. If bituminous material is applied to adjacent construction, the Contractor shall remove such material to the satisfaction of the Engineer.

The emulsified asphalt shall not be applied when the wind conditions will inhibit uniform coverage from the fans of asphalt being applied.

- (5) Application of Aggregates. The seal coat aggregates shall be spread evenly with an aggregate spreader over the entire surface being treated. When treating one-half of the pavement width at a time, an inside strip of uncovered emulsified asphalt 3 in. (75 mm) wide shall be left during construction of the first half to provide center joint overlap when the second half of the treatment is placed. In all cases, the aggregate shall be applied ahead of the truck or spreader wheels. Hand spreading will be permitted only when approved by the Engineer and, when so permitted, the aggregate shall be spread uniformly and at the approximate rate specified. Any ridges of aggregate left by the aggregate spreader shall be smoothed out with hand brooms immediately behind the aggregate spreader.

All equipment involved in the work shall operate as close to each other as practical. The aggregate shall cover the asphalt emulsion within 30 seconds of applications. At no time shall the aggregate spreader trail the pressure distributor by more than 150 ft (45 m) to ensure proper asphalt/aggregate adhesion.

Each aggregate truck shall be equipped with a suitable hitch for connection to the aggregate spreader while unloading. The trucks shall avoid contact between the truck body or bed and the aggregate spreader. The body or bed of the truck shall be modified, if necessary, to empty cleanly and completely into the receiving hopper of the aggregate spreader. No aggregate shall be allowed to spill onto the road surface when the truck is emptying into this hopper.

The aggregate shall be rolled following spreading. A maximum time of five minutes will be allowed between the spreading of aggregate and completion of the initial rolling of the aggregate. The rollers shall proceed in a longitudinal direction at a speed less than or equal to 5 mph (8 km/h). Each roller shall travel over the aggregate a minimum of two times. The entire surface shall be rolled immediately with a self-propelled pneumatic-tired roller. Rolling shall proceed in a longitudinal direction beginning at the edges and progressing toward the center, overlapping on successive trips by at least 1/2 the width of the roller. The aggregate shall then be rolled with a separate pneumatic-tired roller until the aggregate is properly seated in the bituminous material.

The Contractor shall use the appropriate sweeping equipment to perform an initial sweeping after a minimum of two hours curing and not less than one hour before sunset on the day the A-1 surface treatment is placed. The initial sweeping shall remove excess aggregate by lightly sweeping each pavement lane. The sweeping shall be sufficient to prevent migration of loose aggregate back onto any part of the pavement.

The Contractor shall sweep the pavement surface as needed to remove excess aggregate.

- (b) Micro-Surfacing. This method shall consist of applying the surface mix within a maximum of 12 calendar days of placing the A-1 bituminous surface treatment. The Contractor shall sweep the pavement surface immediately prior to applying the micro-surfacing.

The surface shall be prewetted by water fogging ahead of the spreader box when road conditions require, as determined by the Engineer. The rate of fogging shall be adjusted during the day based on pavement temperature, surface texture, and dryness.

- (1) Application. The micro-surfacing shall be applied over the entire width of each lane in a single pass at a rate of 24 lb/sq yd (13 kg/sq m). The application rate shall be verified from daily readings taken from the proportioning devices during the progress of the work.

The paving mixture shall be spread to leave a uniform surface. A sufficient amount of material shall be carried at all times in all parts of the spreader box to ensure complete coverage. Overloading of the spreader shall be avoided. No lumps or uncoated aggregate will be permitted in the finished surface.

Adjustments to the mix design may be required during construction, based on field conditions. The percent of mineral filler in the mix design may be increased or decreased by less than 0.3 percent when the slurry seal is being placed if it is found

to be necessary for better consistency or set times. The Engineer will give final approval for all adjustments.

- (2) **Mix Consistency.** The finished product shall be uniform in color and composition. No streaks, such as those caused by oversized aggregate, shall be left in the finished surface. If excess streaking develops, the job will be stopped until the Contractor proves to the Engineer that the situation has been corrected. Excessive streaking is defined as more than four drag marks greater than 1/2 in. (13 mm) wide and 4 in. (100 mm) long, or 1 in. (25 mm) wide and 3 in. (75 mm) long, in any 30 sq yd (25 sq m) area. No transverse ripples or longitudinal streaks of 0.25 in. (6 mm) in depth will be permitted, when measured by placing a 10 ft (3 m) straightedge over the surface.
- (3) **Mix Stability.** The micro-surfacing shall possess sufficient stability so that premature breaking of the material in the spreader box does not occur. The mixture shall be homogeneous during and following mixing and spreading. It shall be free of excess water or emulsified asphalt and free of segregation of the emulsified asphalt and aggregate fines from the coarser aggregate. Under no circumstances shall water be sprayed directly into the lay-down box while placing micro-surfacing material.
- (4) **Joints and Edges.** The Contractor shall devise a joint plan according to ISSA A143 and submit to the Engineer for approval. When practical, the surface course joint shall be at least 10 in. (255 mm) away from the nearest edge of any subsequent permanent pavement markings.

Micro-surfacing edges shall be parallel with the existing pavement edges. If the existing pavement edge cannot be used to give a straight edge, a stringline or other guide will be required. Edge lines shall not vary by more than ± 2 in. (± 50 mm) horizontally in any 100 ft (30 m) of length.

A smooth, neat seam shall be provided where two passes meet. Excess material shall be immediately removed from the ends of each run. Any damage to, or irregularities in, the micro-surfacing shall be repaired, as directed by the Engineer. All repairs shall be made with a paver box, except areas designated as hand work areas.

- (5) **Hand Work.** Those areas inaccessible to the spreader box and other areas approved by the Engineer shall be designated as hand work areas. Adjustments to the additive will be permitted to provide a slower setting time when hand spreading is needed. If hand spreading is necessary, the mixture shall be poured in a small windrow along one edge of the surface to be covered and then spread uniformly by a hand squeegee or lute. Hand work areas shall have an appearance consistent with that being placed with a spreader box.

Clean-Up. All areas, such as manholes, gutters, and intersections, shall have the cape seal removed as specified by the Engineer. The Contractor shall, on a daily basis, remove any debris associated with the performance of the work.

Sampling and Testing. The Contractor shall check yield of the application after the first 1000 ft (300 m), and throughout each day's paving, with a minimum of three tests per day. Yield check results shall be furnished to the Engineer daily.

The Contractor shall submit a daily "run sheet" for each day's work as soon as all the data is available. The run sheet shall provide a breakdown of the actual meter numbers and quantities of all materials actually used each day, as well as the respective locations.

Opening to Traffic. The A-1 bituminous surface treatment portion shall be opened to traffic according to Article 701.17(c)(4) of the Standard Specifications.

The micro-surfacing shall be opened to traffic within one hour of its application.

Curing. The micro-surfacing shall cure for a minimum of seven days before placement of the permanent pavement markings.

Method of Measurement. Crack/joint sealing will be measured for payment in feet (meters), measured along the crack.

Pavement marking removal and pavement marker removal will be measured for payment according to Article 783.05 of the Standard Specifications.

The cape seal will be measured for payment in place and the area computed in square yards (square meters). The width for measurement will be the width of the top surface as shown on the plans or as directed by the Engineer.

Basis of Payment. Crack/joint sealing will be paid for at the contract unit price per foot (meter) for FIBER-MODIFIED ASPHALT CRACK SEALING.

Bump removal will be paid for at the contract unit price per each for BUMP REMOVAL.

Pavement marking removal and pavement marker removal will be paid for according to Article 783.06 of the Standard Specifications.

Cape seal will be paid for at the contract unit price per square yard (square meter) for CAPE SEAL.

Designer Note: Insert into all contracts using micro-surfacing. Use of this special provision shall be according to the Bureau of Design and Environment Manual, Chapter 52.

The designer must specify the friction aggregate mixture and the following information on the plans:

- (a) "Micro-Surfacing, Single Pass, Type II, 20 lb/sq yd (11 kg/sq m)", or
- (b) "Micro-Surfacing Rut Filling, Type III" and "Micro-Surfacing, Single Pass, Type II, 20 lb/sq yd (11 kg/sq m)"

PREVENTIVE MAINTENANCE - MICRO-SURFACING (BDE)

Effective: January 1, 2009

Description. This work shall consist of micro-surfacing hot-mix asphalt (HMA) surfaces.

Materials. Materials shall be according to the following.

- (a) Micro-Surfacing. Materials shall be according to the following Articles/Sections of the Standard Specifications.

Item	Article/Section
(1) Mineral Filler (Note 1)	1001
(2) Water	1002
(3) Coarse Aggregate (Note 2)	1004.03
(4) Bituminous Material (Prime Coat)	1032.06
(5) Latex-Modified Emulsified Asphalt (Note 3)	
(6) Additives (Note 4)	

Note 1. The mineral filler shall be Type 1 portland cement.

Note 2. The coarse aggregate material shall be selected from the table in Article 1004.03(a) of the Standard Specifications based upon the friction aggregate mixture specified. The quality of the aggregate shall be Class B and the gradation shall be as shown in the table below.

Sieve Size	Type II % Passing	Type III % Passing ^{1/}
3/8 in. (9.5 mm)	100	100
#4 (4.75 mm)	95 ± 5	80 ± 10
#8 (2.36 mm)	77 ± 13	57 ± 13
#16 (1.18 mm)	57 ± 13	39 ± 11
#30 (600 µm)	40 ± 10	26 ± 8
#50 (330 µm)	24 ± 6	18 ± 7
#100 (150 µm)	15 ± 6	12 ± 6
#200 (75 µm)	10 ± 5	10 ± 5

1/ Rut filling mixes shall be constructed using a Type III gradation. All surface mixes shall be constructed using a Type II gradation.

To assure the material is totally crushed, 100 percent of the parent aggregate shall be larger than the largest stone in the gradation to be used.

The blending, alternate use, and /or substitutions of aggregates from different sources for use in this work will not be permitted without the approval of the Engineer. Any blending shall be by interlocked mechanical feeders. The blending shall be uniform, compatible with the other components of the mix, and the equipment shall be approved by the Engineer.

If blending aggregates, the blend shall have a washed gradation performed every other day or a minimum of three tests per week. Testing shall be completed before the aggregate receives final acceptance for use in the mix. All gradation tests shall be conducted according to the aggregate gradation control system (AGCS).

Aggregates shall be screened at the stockpile prior to delivery to the paving machine to remove oversized material or contaminants.

Note 3. CSS-1h Latex Modified Emulsified Asphalt. The emulsified asphalt shall be a quick-traffic latex modified asphalt emulsion containing a minimum of 3.0 percent latex solids by weight of asphalt binder. The latex shall be milled or blended into the emulsifier solution prior to the emulsification process. The CSS-1h latex modified emulsified asphalt shall be according to the following.

Test (AASHTO T 59)	Result
Viscosity, Saybolt Furol, 77 °F (25 °C), SFS	20-100
Storage Stability Test, 24 hours, %	1 max.
Particle Charge Test	Positive
Sieve Test, No. 20 (850 µm), retained on sieve, %	0.10 max.
Distillation Test, Residue from distillation test to 347 ± 9 °F (175 ± 5 °C), %	62 min.

Tests on residue from distillation	Result
Penetration, 77 °F (25 °C), 100 grams, 5 seconds, (AASHTO T 49), dmm	40-90
Ductility, 77 °F (25 °C), 50 mm/min, (AASHTO T 51), mm	400 min.
Solubility in trichloroethylene, (AASHTO T 44), %	97.5 min.
Softening Point, (AASHTO T 53), °F (°C)	135 (57) min.
Absolute Viscosity, 140 °F (60 °C), (AASHTO T 202), Poises (Pa · sec)	8,000 (800) min.

Note 4. Additives may be added to the emulsion mix or any of the component materials to provide the control of the quick-traffic properties. They shall be included as part of the mix design and be compatible with the other components of the mix.

(b) Crack/Joint Sealant. The crack/joint sealant shall be a fiber-modified asphalt binder mixed at the jobsite or premixed.

(1) Jobsite-Mixed Sealant. The sealant shall consist of an asphalt binder and fibers, and be according to the following.

- a. Asphalt Binder. The asphalt binder shall be PG 58-28, PG 58-22, or PG 64-22 and meet the requirements of Article 1032.05 of the Standard Specifications.
- b. Fibers. Fibers shall be short cut polypropylene fibers meeting the properties listed below. The fiber may be accepted on certification from the manufacturer that it meets the specified requirements.

Property	Value
Length, in. (mm)	0.3 - 0.5 (8 - 12)
Denier	13 - 16
Crimps	None
Tensile Strength, min., psi (kPa)	40,000 (275,000)
Specific Gravity (typical)	0.91
Moisture Regain @ 70 °F (21 °C) and 65% RH (typical), %	0.1

- c. Percent Fibers. The sealant shall contain a minimum of 8.0 percent of fibers by weight (mass).
 - d. Sealant Heating. The sealant shall be heated in the kettle at temperatures between 255 and 285 °F (124 and 141 °C).
- (2) Premixed Sealant. The sealant shall be packaged and consist of an asphalt binder, fibers, and other modifiers meeting the following requirements. The sealant and its components may be accepted on certification from the manufacturer that it meets the specified requirements.

- a. Asphalt Binder. The asphalt binder shall be PG 64-22 and meet the requirements of Article 1032.05 of the Standard Specifications.
- b. Fibers. Fibers shall be short cut polyester fibers meeting the following.

Property	Value
Length, in. (mm)	0.25 ± 0.02 (6.3 ± 0.5)
Denier	3 - 6
Crimps	None
Tensile Strength, minimum, psi (kPa)	70,000 (482,000)
Specific Gravity (typical)	1.32 - 1.40
Elongation at Break, %	35 - 38
Melt Temperature, °F (°C)	475 - 490 (246 - 254)

- c. Percent Fibers. The sealant shall contain 5.0 ± 0.5 percent of fibers by weight (mass).

The sealant, in its final form, shall meet the following requirements when sampled and heated to the manufacturer's recommended maximum heating temperature according to ASTM D 5167.

Test	Value
Cone Penetration @ 77 °F (25 °C), ASTM D 5329	10-35 dmm
Softening Point, ASTM D 36	175 °F (79 °C) min.
Maximum Heating Temperature	400°F (204 °C)
Application Temperature	350°F (177 °C) min.

Equipment. Equipment shall be according to the following.

(a) Micro-Surfacing. Equipment shall be according to the following.

- (1) Micro-Surfacing Mixing Machine. The machine shall be either a continuous (self-loading) machine or a non-continuous (self-contained) machine depending on the size of the project as described below. Both types of machines shall have sufficient storage capacity for aggregate, emulsified asphalt, mineral filler, control additive and water to maintain an adequate supply to the proportioning controls.

Machines that are the continuous (self-loading) type shall be an automatic-sequenced, self-propelled, continuous-flow mixing unit able to accurately deliver and proportion the aggregate, emulsified asphalt, mineral filler, control setting additive, and water to a revolving multi-blade, double-shafted mixer and to discharge the mixed product on a continuous-flow basis. The machine shall be equipped to allow the operator to have full control of the forward and reverse speeds during applications of the material and be equipped with opposite-side driver stations to assist in alignment.

Non-continuous (self-contained) machines will be allowed on projects with a length of 2 lane-miles (3.2 lane-km) or less. For mainline paving, the Contractor shall have at least three self-contained machines in continuous operation to ensure appropriate production rates. Self-contained machines will also be allowed on shoulders, ramps, short applications such as bridge decks, or where the material can be placed in a single loading capacity of the machine.

Each mixing unit to be used in the performance of the work shall be calibrated in the presence of the Engineer prior to construction. Each new or different aggregate requires a new calibration. Previous calibration documentation covering the exact materials to be used may be acceptable, provided that no more than 30 days have lapsed. The documentation shall include an individual calibration of each material at various settings, which can be related to the machine metering devices. Prior to the calibration process, portable scales used to calibrate the mixing machine for emulsion and aggregate shall be checked with 25 lb and 50 lb weights, respectively. Results from the standard weight checks shall be furnished to the Engineer. No machine will be allowed to work on the project until the calibration has been completed and/or accepted.

- (2) Micro-Surfacing Spreader. The mixture shall be agitated and spread uniformly in the surfacing box by means of twin shafted paddles or spiral augers fixed in the spreader box. A front seal shall be provided to insure no loss of the mixture at the road contact point. The rear seal shall act as a final strike-off and shall be adjustable. The spreader box and rear strike-off shall be so designed and operated that a uniform consistency is achieved to produce a free flow of material to the rear strike-off. The spreader box shall have suitable means provided to side shift the box to compensate for variations in the pavement geometry.

A secondary strike-off shall be provided to improve surface texture on the surface course. The secondary strike-off shall have the same adjustments as the spreader box and shall not bounce, wobble, or chatter.

When required on the plans, before the final surface course is placed, preliminary micro-surfacing material may be required to fill ruts, utility cuts, depressions in the existing surface, etc. Ruts of 1/2 in. (13 mm) or greater in depth shall be filled independently with a rut-filling spreader box, either 5 or 6 ft (1.5 or 1.8 m) in width.

For irregular or shallow rutting of less than 1/2 in. (13 mm) in depth, a full-width scratch-coat pass may be used as directed by the Engineer utilizing either a stiff primary rubber or else a metal primary strike off. Ruts that are in excess of 1 1/2 in. (38 mm) in depth may require multiple placements with the rut-filling spreader box to restore the cross-section. All rut-filling level-up material should cure under traffic for a minimum of 24 hours before additional material is placed on top of the level up.

- (3) Micro-Surfacing Proportioning Devices. Individual volume or weight controls for proportioning each material to be added to the mix (i.e. aggregate, mineral filler, emulsified asphalt, additive, and water) shall be provided and properly marked. These proportioning devices are used in material calibration and determining the material output at any time. Calibration records, conversion formulas, and daily run sheets including the beginning and final numbers shown on the proportioning devices shall be submitted to the Engineer for approval. During production any deviations from the original JMF shall be approved by the Engineer.

(b) Crack/Joint Sealing. Equipment shall be according to the following.

- (1) Air Compressor. The air compressor shall be capable of producing a minimum pressure of 90 psi (620 kPa) at the end of the discharge hose. The air stream shall discharge onto the pavement through an appropriate air lance. The tool lubricator shall be bypassed and a filter installed on the discharge valve to keep water and oil out of the line.
- (2) Oil Kettle. The crack sealant shall be heated in an oil jacketed double wall kettle equipped with an agitator (reversing rotary auger action) and separate thermometers for the oil bath and mixing chamber. The unit shall also be equipped with a reversible hydraulic 2 in. (50 mm) hot asphalt pump and a recirculating pump to circulate the oil bath.

CONSTRUCTION REQUIREMENTS

General. The paving mixture shall be capable of filling up to 1 1/2 in. (38 mm) wheel ruts in one pass, be capable of field regulation of the setting time, and be suitable for nighttime placement. The compatibility of all ingredients of the mix, including the mix set additive, shall be certified by the emulsified asphalt manufacturer.

Weather Limitations. Placement of the micro-surfacing shall be done between May 1 and October 15, and when the temperature is at least 50 °F (10 °C) and rising and the forecast for the next 24 hours is above 40 °F (5 °C).

Mix Design. A Contractor provided laboratory shall develop the mix design for the micro-surfacing mixture, shall verify the functioning of the set regulating additives, and shall present certified test results for the Engineer's approval. This laboratory shall be recognized by the International Slurry Surfacing Association (ISSA) as being capable of performing mix designs. The Engineer will verify the laboratory tests required in ISSA A143 have been conducted.

Proportions for the mix design shall be within the following limits.

Mineral Aggregate, dry weight (mass) lb/sq yd (kg/sq m)	15-50 (8-30)
Latex Emulsified Asphalt Residue, % by wt. of Aggregate	5.5-10.5
Latex Base Modifier	As required with % by weight (mass) of binder, min. of 3.0
Mix Set Additive	As required
Mineral Filler, % by weight (mass) of Aggregate	0.25 - 3 depending on weather conditions

The amount of mineral filler needed shall be determined by the laboratory mix design and will be considered as part of the aggregate gradation.

The amount and type of latex shall be determined by the laboratory performing the mix design. The minimum amount required shall be based on asphalt weight content and shall be certified by the emulsion supplier.

Compatibility of the aggregate, latex-modified emulsified asphalt, mineral filler, and other additives shall be verified by the mix design. The materials shall meet the following requirements for ISSA A143.

ISSA Test No.	Description	Specification
ISSA TB-139	Wet Cohesion @ 30 minutes min. (Set) @ 60 minutes min. (Traffic)	12 kg-cm min. 20 kg-cm min. or Near Spin
ISSA TB-109	Excess Asphalt by LWT Sand Adhesion	50 gm/sq ft (538 gm/sq m) max.
ISSA TB-114	Wet Stripping	Pass (90% min.)
ISSA TB-100	Wet-Track Abrasion Loss One-hour Soak Six-day Soak	50 gm/sq ft (538 gm/sq m) max. 75 gm/sq ft (807 gm/sq m) max.
ISSA TB-147	Lateral Displacement	5% max.
	Specific Gravity after 1,000 Cycles of 25 lb (11.34 kg)	2.10 max.
ISSA TB-144	Classification Compatibility	11 Grade Points min. (AAA, BAA)
ISSA TB-113	Mix Time @ 77 °F (25 °C)	Controllable to 120 seconds Min.

The mixing test and set-time test shall be checked at the highest temperatures expected during construction.

The mix design shall report the quantitative effects of moisture content on the unit weight of the aggregate (bulking effect). The report shall clearly show the proportions of aggregate, mineral filler (minimum and maximum), water (minimum and maximum), additive usage, and latex-modified asphalt emulsion based on the dry weight of the aggregate.

For the aggregate blend in the mix design, test results for AASHTO T 176 shall be provided with the mix information to the Engineer. Aggregate test values below 65 shall require review and approval from the Engineer.

Before the work commences, the Contractor shall submit to the Engineer a complete mix design covering the specific materials to be used on the project. The percentages of each individual material required shall be shown in the laboratory report. The Engineer shall approve the mix design prior to its use. After approval, no substitutions will be permitted, unless approved by the Engineer, and the Contractor shall maintain continuous control of the latex-modified emulsified asphalt to dry aggregate proportioning to conform to the approved mix design within a tolerance of ± 2 gal/ton (± 8 L/metric ton).

Test Strip. For projects over 100,000 sq yd (83,600 sq m), at least one day prior to starting the project the Contractor shall designate a mutually agreeable location and apply a test strip of micro-surfacing using the aggregate indicated in the mix design. The Engineer will evaluate the micro-surfacing application rate and cure time.

Surface Preparation. Pavement markings shall be removed according to Article 783.03(a) of the Standard Specifications. Only very small particles of tightly adhering existing markings may remain in place.

When specified in the plans, pavement markers shall be removed according to Article 783.03(b) of the Standard Specifications.

Bumps greater than or equal to 1/2 in. (13 mm) shall be removed by grinding. The Contractor shall determine bump grinding locations in the presence of the Engineer by using a 16-ft (5-m) straightedge with the scratcher bolts set to 1/2 in. (13 mm). All locations marked by the scratcher bolts shall be ground using either a grinding machine consisting of multiple saws or a cold-milling machine with a double- or triple-wrap milling head.

Joints and cracks 3/16 in. (5 mm) or wider shall be cleaned of loose and unsound material and sealed. The sealant shall be applied only when the joints and cracks are clean and dry, and the ambient temperature is 40-85 °F (4-29 °C). The sealant shall be applied using a pressurized wand delivery system with such devices as necessary to seal the cracks/joints and form a nominal 0.125 in. (3 mm) thick by 3 in. (75 mm) wide overseal band centered so that the center of the 3 in. (75 mm) wide band is within 1 in. (25 mm) of the crack. The sealant shall be allowed to cure before opening to traffic. When approved by the Engineer, the sealant may be dusted with fine sand, portland cement, or mineral filler to prevent tracking.

Micro-Surfacing. The micro-surfacing shall be applied as shown on the plans and the following.

- (a) Preparation. Prior to applying the micro-surfacing, the pavement surface shall be cleaned. On highly oxidized surfaces, a prime coat shall be applied at a rate of 0.05-0.10 gal/sq yd (0.22-0.45 L/sq m) according to Article 406.05(b) of the Standard Specifications. Manholes, valve boxes, drop inlets, and other service entrances shall be protected from the micro-surfacing by a suitable method. The surface preparation shall be approved by the Engineer prior to the application of the micro-surfacing. No dry aggregate either spilled from the lay-down machine or existing on the road will be permitted.

The Contractor shall apply the micro-surfacing according to the following methods.

- (1) Micro-Surfacing Rut Filling. This method shall consist of filling each of the two wheelpath ruts in a lane using the specially designed rutbox and the rutfill (Type III) mix. It shall be the Contractor's responsibility to determine and estimate the quantities of rutfill mix required for rut filling. This work is then followed by one pass of micro-surfacing as described below.

- (2) Micro-Surfacing, Single Pass. This method shall consist of applying the surface mix over the entire width of each lane in one pass using the gradation and application rate shown on the plans.

Determinations of application rates shall be from daily readings taken from the material control devices during the progress of the work.

The pavement surface shall be prewetted by water fogging ahead of the spreader box when road conditions require, as determined by the Engineer. The rate of fogging shall be adjusted during the day based on pavement temperature, surface texture, and dryness.

The paving mixture shall be spread to fill minor cracks and shallow potholes and leave a uniform surface. Care shall be taken when rut filling to restore the designed profile of the pavement cross section. Excess crowning (over-filling) of rut areas shall be avoided. A sufficient amount of material shall be carried at all times in all parts of the spreader box to ensure complete coverage. Overloading of the spreader shall be avoided. No lumps or uncoated aggregate will be permitted in the finished surface.

Adjustments to the mix design may be required during construction, based on field conditions. The percent of mineral filler in the mix design may be increased or decreased by less than 0.3 percent when the slurry seal is being placed if it is found to be necessary for better consistency or set times. The Engineer will give final approval for all adjustments.

- (b) Mix Consistency. The finished product shall be uniform in color and composition. No streaks, such as those caused by oversized aggregate, shall be left in the finished surface. If excess streaking develops, the job will be stopped until the Contractor proves to the Engineer that the situation has been corrected. Excessive streaking is defined as more than four drag marks greater than 1/2 in. (13 mm) wide and 4 in. (100 mm) long, or 1 in. (25 mm) wide and 3 in. (75 mm) long, in any 30 sq yd (25 sq m) area. No transverse ripples or longitudinal streaks of 0.25 in. (6 mm) in depth will be permitted, when measured by placing a 10 ft (3 m) straightedge over the surface.
- (c) Mix Stability. The micro-surfacing shall possess sufficient stability so that premature breaking of the material in the spreader box does not occur. The mixture shall be homogeneous during and following mixing and spreading. It shall be free of excess water or emulsified asphalt and free of segregation of the emulsified asphalt and aggregate fines from the coarser aggregate. Under no circumstances shall water be sprayed directly into the lay-down box while placing micro-surfacing material.
- (d) Joints and Edges. The Contractor shall devise a joint plan according to ISSA A143 and submit to the Engineer for approval. When practical, the surface course joint shall be at least 10 in. (255 mm) away from the nearest edge of any subsequent permanent pavement markings.

Micro-surfacing edges shall be parallel with the existing pavement edges. If the existing pavement edge cannot be used to give a straight edge, a stringline or other guide will be required. Edge lines shall not vary by more than ± 2 in. (50 mm) horizontally in any 100 ft (30 m) of length.

A smooth, neat seam shall be provided where two passes meet. Excess material shall be immediately removed from the ends of each run. Any damage to, or irregularities in, the micro-surfacing shall be repaired, as directed by the Engineer. All repairs shall be made with a paver box, except areas designated as hand work areas.

- (e) Hand Work. Those areas inaccessible to the spreader box and approved by the Engineer shall be designated as hand work areas. Adjustments to the additive will be permitted to provide a slower setting time when hand spreading is needed. If hand spreading is necessary, the mixture shall be poured in a small windrow along one edge of the surface to be covered and then spread uniformly by a hand squeegee or lute. Hand work areas shall have an appearance consistent with that being placed with a spreader box.

Clean-Up. All areas, such as manholes, gutters, and intersections, shall have the micro-surfacing mix removed as specified by the Engineer. The Contractor shall, on a daily basis, remove any debris associated with the performance of the work.

Sampling and Testing. The Contractor shall check yield of the application after the first 1000 ft (300 m), and throughout each day's paving, with a minimum of three tests per day. Yield check results shall be furnished to the Engineer daily.

The Contractor shall submit a daily "run sheet" for each day's work as soon as all the data is available. The run sheet shall provide a breakdown of the actual meter numbers and quantities of all materials actually used each day, as well as the respective locations.

Opening to Traffic. The micro-surfacing shall be opened to traffic within one hour of its application.

Curing. The micro-surfacing shall cure for a minimum of 7 days before placement of the permanent pavement markings.

Method of Measurement. This work will be measured for payment as follows.

- (a) Contract Quantities. The requirements for the use of contract quantities shall conform to Article 202.07(a) of the Standard Specifications.
- (b) Measured Quantities. Crack/Joint sealing will be measured for payment in feet (meters), measured along the crack.

Pavement marking removal will be measured for payment according to Article 783.05 of the Standard Specifications.

The micro-surfacing will be measured according to the following for the method of application provided in the plans.

- (1) Micro-Surfacing Rut Filling. Micro-surfacing rut filling will be measured for payment in place in feet (meters) along the wheel path or filled rut.
- (2) Micro-surfacing, Single Pass. Micro-surfacing, single pass will be measured for payment in place and the area computed in square yards (square meters). The width for measurement will be the width of the top surface as shown on the plans or as directed by the Engineer.

Prime coat, when required, will be measured for payment according to Article 406.13(b) of the Standard Specifications.

Basis of Payment. Crack/joint sealing will be paid for at the contract unit price per foot (meter) of FIBER-MODIFIED ASPHALT CRACK SEALING.

Bump removal will be paid for at the contract unit price per each for BUMP REMOVAL.

Pavement marking removal and pavement marker removal will be paid for according to Article 783.06 of the Standard Specifications.

Rut filling will be paid for at the contract unit price per foot (meter) for MICRO-SURFACING RUT FILLING.

Micro-surfacing, single pass will be paid for at the contract unit price per square yard (square meter) for MICRO-SURFACING, SINGLE PASS, of the type specified.

Prime coat, when required, will be paid for according to Article 406.14 of the Standard Specifications.

Designer Note: Insert into all contracts using slurry seal. Use of this special provision shall be according to the Bureau of Design and Environment Manual, Chapter 52.

The designer must specify the friction aggregate mixture on the plans.

PREVENTIVE MAINTENANCE - SLURRY SEAL (BDE)

Effective: January 1, 2009

Description. This work shall consist of slurry sealing existing hot-mix asphalt (HMA) surfaces.

Materials. Materials shall be according to the following.

- (a) Slurry Seal. Materials shall be according to the following Articles/Sections of the Standard Specifications.

Item	Article/Section
(1) Mineral Filler (Note 1)	1001
(2) Water.....	1002
(3) Coarse Aggregate (Note 2).....	1004.03
(4) Bituminous Material (Prime Coat)	1032.06
(5) Emulsified Asphalt (Note 3)	
(6) Additives (Note 4)	

Note 1. The mineral filler shall be Type 1 portland cement.

Note 2. The coarse aggregate material shall be selected from the table in Article 1004.03(a) of the Standard Specifications based upon the friction aggregate mixture specified. The quality of the aggregate shall be Class B and the gradation shall be as shown in the table below.

Sieve Size	% Passing
3/8 in. (9.5 mm)	100
#4 (4.75 mm)	95 ± 5
#8 (2.36 mm)	77 ± 13
#16 (1.18 mm)	57 ± 13
#30 (600 µm)	40 ± 10
#50 (330 µm)	24 ± 6
#100 (150 µm)	15 ± 6
#200 (75 µm)	10 ± 5

To assure the material is totally crushed, 100 percent of the parent aggregate shall be larger than the largest stone in the gradation to be used.

The blending, alternate use, and/or substitutions of aggregates from different sources for use in this work will not be permitted without the approval of the Engineer. Any blending shall be by interlocked mechanical feeders. The blending shall be uniform, compatible with the other components of the mix, and the equipment shall be approved by the Engineer.

If blending aggregates, the blend shall have a washed gradation performed every other day or a minimum of three tests per week. Testing shall be completed before the aggregate receives final acceptance for use in the mix. All gradation tests shall be conducted according to the aggregate gradation control system (AGCS).

Aggregates shall be screened at the stockpile prior to delivery to the paving machine to remove oversized material or contaminants.

Note 3. The emulsified asphalt shall be a quick-traffic, asphalt emulsion conforming to the requirements of AASHTO M 208 for CSS-1h, and the following.

AASHTO Test No.	Quality	Specification
AASHTO T 59 ^{1/}	Residue after Distillation	60% Minimum

1/ When using a latex-modified emulsion, the distillation temperature shall be 347 ± 9 °F (175 ± 5 °C).

AASHTO Test No.	Tests on Residue	Specification
AASHTO T 49	Penetration at 77 °F (25 °C)	40 - 90 dmm ^{2/}

2/ Climatic conditions should be considered when establishing this range.

The cement mixing test will be waived for this emulsion.

If a latex-modified emulsion is used, the latex shall be milled or blended into the emulsifier solution prior to the emulsification process.

Note 4. Additives may be added to the emulsion mix or any of the component materials to provide control of the quick-traffic properties. They shall be included as part of the mix design and be compatible with the other components of the mix.

(b) Crack/Joint Sealant. The crack/joint sealant shall be a fiber-modified asphalt binder mixed at the jobsite or premixed.

(1) Jobsite-Mixed Sealant. The sealant shall consist of an asphalt binder and fibers, and be according to the following.

a. Asphalt Binder. The asphalt binder shall be PG 58-28, PG 58-22, or PG 64-22 and meet the requirements of Article 1032.05 of the Standard Specifications.

b. Fibers. Fibers shall be short cut polypropylene fibers meeting the properties listed below. The fiber may be accepted on certification from the manufacturer that it meets the specified requirements.

Property	Value
Length, in. (mm)	0.3 - 0.5 (8 - 12)
Denier	13 - 16
Crimps	None
Tensile Strength, min., psi (kPa)	40,000 (275,000)
Specific Gravity (typical)	0.91
Moisture Regain @ 70 °F (21 °C) and 65% RH (typical), %	0.1

- c. Percent Fibers. The sealant shall contain a minimum of 8.0 percent of fibers by weight (mass).
 - d. Sealant Heating. The sealant shall be heated in the kettle at temperatures between 255 and 285 °F (124 and 141 °C).
- (2) Premixed Sealant. The sealant shall be packaged and consist of an asphalt binder, fibers, and other modifiers meeting the following requirements. The sealant and its components may be accepted on certification from the manufacturer that it meets the specified requirements.
- a. Asphalt Binder. The asphalt binder shall be PG 64-22 and meet the requirements of Article 1032.05 of the Standard Specifications.
 - b. Fibers. Fibers shall be short cut polyester fibers meeting the following.

Property	Value
Length, in. (mm)	0.25 ± 0.02 (6.3 ± 0.5)
Denier	3 - 6
Crimps	None
Tensile Strength, min., psi (kPa)	70,000 (482,000)
Specific Gravity (typical)	1.32 - 1.40
Elongation at Break, %	35 - 38
Melt Temperature, °F (°C)	475 - 490 (246 - 254)

- c. Percent Fibers. The sealant shall contain 5.0 ± 0.5 percent of fibers by weight (mass).

The sealant, in its final form, shall meet the following requirements when sampled and heated to the manufacturer's recommended maximum heating temperature according to ASTM D 5167.

Test	Value
Cone Penetration @ 77 °F (25 °C), ASTM D 5329	10-35 dmm
Softening Point, ASTM D 36	175 °F (79 °C) min.
Maximum Heating Temperature	400°F (204 °C)
Application Temperature	350°F (177 °C) min.

Equipment. Equipment shall be according to the following.

- (a) Slurry Seal. Equipment shall be according to the following.

- (1) Slurry Seal Mixing Machine. The machine shall be either a continuous (self-loading) machine or a non-continuous (self-contained) machine depending on the size of the project as described below. Both types of machines shall have sufficient storage capacity for aggregate, emulsified asphalt, mineral filler, control additive and water to maintain an adequate supply to the proportioning controls.

Machines that are the continuous (self-loading) type shall be an automatic-sequenced, self-propelled, continuous-flow mixing unit able to accurately deliver and proportion the aggregate, emulsified asphalt, mineral filler, control setting additive, and water to a revolving multi-blade, double-shafted mixer and to discharge the mixed product on a continuous-flow basis. The machine shall be equipped to allow

the operator to have full control of the forward and reverse speeds during applications of the material and be equipped with opposite-side driver stations to assist in alignment.

Non-continuous (self-contained) machines will be allowed on projects with a length of 2 lane-miles (3.2 lane-km) or less. For mainline paving, the Contractor shall have at least three self-contained machines in continuous operation to ensure appropriate production rates. Self-contained machines will also be allowed on shoulders, ramps, short applications such as bridge decks, or where the material can be placed in a single loading capacity of the machine.

Each mixing unit to be used in the performance of the work shall be calibrated in the presence of the Engineer prior to construction. Each new or different aggregate requires a new calibration. Previous calibration documentation covering the exact materials to be used may be acceptable, provided that no more than 30 days have lapsed. The documentation shall include an individual calibration of each material at various settings, which can be related to the machine metering devices. Prior to the calibration process, portable scales used to calibrate the mixing machine for emulsion and aggregate shall be checked with 25 lb and 50 lb weights, respectively. Results from the standard weight checks shall be furnished to the Engineer. No machine will be allowed to work on the project until the calibration has been completed and/or accepted.

- (2) Slurry Seal Spreader. The mixture shall be agitated and spread uniformly in the surfacing box by means of twin shafted paddles or spiral augers fixed in the spreader box. A front seal shall be provided to insure no loss of the mixture at the road contact point. The rear seal shall act as a final strike-off and shall be adjustable. The spreader box and rear strike-off shall be so designed and operated that a uniform consistency is achieved to produce a free flow of material to the rear strike-off. The spreader box shall have suitable means provided to side shift the box to compensate for variations in the pavement geometry.
 - (3) Slurry Seal Proportioning Devices. Individual volume or weight controls for proportioning each material to be added to the mix (i.e. aggregate, mineral filler, emulsified asphalt, additive, and water) shall be provided and properly marked. These proportioning devices are used in material calibration and determining the material output at any time. Calibration records, conversion formulas, and daily run sheets including the beginning and final numbers shown on the proportioning devices shall be submitted to the Engineer for approval. During production any deviations from the original JMF shall be approved by the Engineer.
- (b) Crack/Joint Sealing. Equipment shall be according to the following.
- (1) Air Compressor. The air compressor shall be capable of producing a minimum pressure of 90 psi (620 kPa) at the end of the discharge hose. The air stream shall discharge onto the pavement through an appropriate air lance. The tool lubricator shall be bypassed and a filter installed on the discharge valve to keep water and oil out of the line.
 - (2) Oil Kettle. The crack sealant shall be heated in an oil jacketed double wall kettle equipped with an agitator (reversing rotary auger action) and separate thermometers for the oil bath and mixing chamber. The unit shall also be equipped with a reversible hydraulic 2-in. (50-mm) hot asphalt pump and a recirculating pump to circulate the oil bath.

CONSTRUCTION REQUIREMENTS

General. The slurry seal shall be capable of field regulation of the setting time. The compatibility of all ingredients of the mix, including the mix set additive, shall be certified by the emulsified asphalt manufacturer.

Weather Limitations. Placement of the slurry seal shall be done between May 1 and October 15, and when the temperature is at least 50 °F (10 °C) and rising and the forecast for the next 24 hours is above 40 °F (5 °C).

Mix Design. A Contractor provided laboratory shall develop the mix design for the paving mixture, shall verify the functioning of the set regulating additives, and shall present certified test results for the Engineer's approval. This laboratory shall be recognized by the International Slurry Surfacing Association (ISSA) as being capable of performing mix designs. The Engineer will verify the laboratory tests required in ISSA A105 have been conducted.

Proportions for the mix design shall be within the following limits.

Mineral Aggregate, dry weight (mass) lb/sq yd (kg/sq m)	15 - 25 (8 - 15)
Emulsified Asphalt Residue, % by wt. of Aggregate	7.5 - 13.5
Latex Base Modifier (if required)	As required with % by weight (mass) of binder, min. of 5.0
Mix Set Additive	As required
Mineral Filler, % by weight (mass) of Aggregate	0.5 - 2.0 depending on weather conditions

The amount of mineral filler needed shall be determined by the laboratory mix design and will be considered as part of the aggregate gradation.

The amount and type of latex shall be determined by the laboratory performing the mix design. The minimum amount required shall be based on asphalt weight content and shall be certified by the emulsion supplier.

Compatibility of the aggregate, emulsified asphalt, mineral filler, and other additives shall be verified by the mix design. The materials shall meet the following requirements for ISSA A105.

ISSA Test No.	Description	Specification
ISSA TB-139	Wet Cohesion @ 30 minutes min. (Set) @ 60 minutes min. (Traffic)	12 kg-cm min. 20 kg-cm min. or Near Spin
ISSA TB-109	Excess Asphalt by LWT Sand Adhesion	50 gm/sq ft (538 gm/sq m) max.
ISSA TB-114	Wet Stripping	Pass (90% min.)
ISSA TB-100	Wet-Track Abrasion Loss One-hour Soak	75 gm/sq ft (807 gm/sq m) max.
ISSA TB-113	Mix Time @ 77 °F (25 °C)	Controllable to 180 seconds, min.

The mixing test and set-time test shall be checked at the highest temperatures expected during construction.

The mix design shall report the quantitative effects of moisture content on the unit weight of the aggregate (bulking effect). The report shall clearly show the proportions of aggregate, mineral filler (minimum and maximum), water (minimum and maximum), additive usage, and asphalt emulsion based on the dry weight of the aggregate.

For the aggregate blend in the mix design, test results for AASHTO T 176 shall be provided with the mix information to the Engineer. Aggregate test values below 45 shall require review and approval from the Engineer.

Before the work commences, the Contractor shall submit to the Engineer a complete mix design covering the specific materials to be used on the project. The percentages of each individual material required shall be shown in the laboratory report. The Engineer shall approve the mix design prior to its use. After approval, no substitutions will be permitted unless approved by the Engineer, and the Contractor shall maintain continuous control of the emulsified asphalt to dry aggregate proportioning to conform to the approved mix design within a tolerance of ± 2 gal/ton (± 8 L/metric ton).

Test Strip. For projects over 100,000 sq yd (83,600 sq m), at least one day prior to starting the project the Contractor shall designate a mutually agreeable location and apply a test strip of slurry seal using the aggregate indicated in the mix design. The Engineer will evaluate the slurry seal application rate and cure time.

Surface Preparation. Pavement markings shall be removed according to Article 783.03(a) of the Standard Specifications. Only very small particles of tightly adhering existing markings may remain in place.

When specified in the plans, pavement markers shall be removed according to Article 783.03(b) of the Standard Specifications.

Bumps greater than or equal to 1/2 in. (13 mm) shall be removed by grinding. The Contractor shall determine bump grinding locations in the presence of the Engineer by using a 16-ft (5-m) straightedge with the scratcher bolts set to 1/2 in. (13 mm). All locations marked by the scratcher bolts shall be ground using either a grinding machine consisting of multiple saws or a cold-milling machine with a double- or triple-wrap milling head.

Joints and cracks, 3/16 in. (5 mm) or wider, shall be cleaned of loose and unsound material and sealed. The sealant shall be applied only when the joints and cracks are clean and dry, and the ambient temperature is 40 - 85 °F (4 - 29 °C). The sealant shall be applied using a pressurized wand delivery system with such devices as necessary to seal the cracks/joints and form a nominal 0.125 in. (3 mm) thick by 3 in. (75 mm) wide overseal band centered so that the center of the 3 in. (75 mm) wide band is within 1 in. (25 mm) of the crack. The sealant shall be allowed to cure before opening to traffic. When approved by the Engineer, the sealant may be dusted with fine sand, portland cement, or mineral filler to prevent tracking.

Slurry Sealing. The slurry seal shall be applied as follows.

- (a) Preparation. Prior to applying the slurry seal, the pavement surface shall be cleaned. On highly oxidized surfaces, a prime coat shall be applied at a rate of 0.05-0.10 gal/sq yd (0.22-0.45 L/sq m) according to Article 406.05(b) of the Standard Specifications. Manholes, valve boxes, drop inlets, and other service entrances shall be protected from the slurry seal by a suitable method. The surface preparation shall be approved by the Engineer prior to application of the slurry seal. No dry aggregate either spilled from the lay-down machine or existing on the road, will be permitted.

The pavement surface shall be prewetted by water fogging ahead of the spreader box when road conditions require, as determined by the Engineer. The rate of fogging shall be adjusted during the day based on pavement temperature, surface texture, and dryness.

- (b) Application. The slurry seal shall be applied over the entire width of each lane in a single pass at a rate 20 lb/sq yd (11 kg/sq m). The application rate shall be verified from daily readings taken from the proportioning devices during the progress of the work.

The paving mixture shall be spread to fill minor cracks and shallow potholes and leave a uniform surface. A sufficient amount of material shall be carried at all times in all parts of the spreader box to ensure complete coverage. Overloading of the spreader shall be avoided. No lumps or uncoated aggregate will be permitted in the finished surface.

Adjustments to the mix design may be required during construction, based on field conditions. The percent of mineral filler in the mix design may be increased or decreased by less than 0.3 percent when the slurry seal is being placed if it is found to be necessary for better consistency or set times. The Engineer will give final approval for all adjustments.

- (c) Mix Consistency. The finished product shall be uniform in color and composition. No streaks, such as those caused by oversized aggregate, shall be left in the finished surface. If excess streaking develops, the job will be stopped until the Contractor proves to the Engineer that the situation has been corrected. Excessive streaking is defined as more than four drag marks greater than 1/2 in. (13 mm) wide and 4 in. (100 mm) long, or 1 in. (25 mm) wide and 3 in. (75 mm) long, in any 30 sq yd (25 sq m) area. No transverse ripples or longitudinal streaks of 0.25 in. (6 mm) in depth will be permitted, when measured by placing a 10 ft (3 m) straightedge over the surface.
- (d) Mix Stability. The slurry seal shall possess sufficient stability so that premature breaking of the material in the spreader box does not occur. The mixture shall be homogeneous during and following mixing and spreading. It shall be free of excess water or emulsified asphalt and free of segregation of the emulsified asphalt and aggregate fines from the coarser aggregate. Under no circumstances shall water be sprayed directly into the lay-down box while placing slurry seal material.
- (e) Joints and Edges. The Contractor shall devise a joint plan according to ISSA A105 and submit it to the Engineer for approval. When practical, the surface course joint shall be at least 10 in. (255 mm) away from the nearest edge of any subsequent permanent pavement markings.

Slurry seal edges shall be parallel with the existing pavement edges. If the existing pavement edge cannot be used to give a straight edge, a stringline or other guide will be required. Edge lines shall not vary by more than ± 2 in. (50 mm) horizontally in any 100 ft (30 m) of length.

A smooth, neat seam shall be provided where two passes meet. Excess material shall be immediately removed from the ends of each run. Any damage to, or irregularities in, the slurry seal shall be repaired, as directed by the Engineer. All repairs shall be made with a paver box, except areas designated as hand work areas.

- (f) Hand Work. Those areas inaccessible to the spreader box and approved by the Engineer shall be designated as hand work areas. Adjustments to the additive will be permitted to provide a slower setting time when hand spreading is needed. If hand spreading is necessary, the mixture shall be poured in a small windrow along one edge

of the surface to be covered and then spread uniformly by a hand squeegee or lute. Hand work areas shall have an appearance consistent with that being placed with a spreader box.

Clean-Up. All areas, such as manholes, gutters, and intersections, shall have the slurry seal mix removed as specified by the Engineer. The Contractor shall, on a daily basis, remove any debris associated with the performance of the work.

Sampling and Testing. The Contractor shall check yield of the application after the first 1000 ft (300 m), and throughout each day's paving, with a minimum of three tests per day. Yield check results shall be furnished to the Engineer daily.

The Contractor shall submit a daily "run sheet" for each day's work as soon as all the data is available. The run sheet shall provide a breakdown of the actual meter numbers and quantities of all materials actually used each day, as well as the respective locations.

Opening to Traffic. The slurry seal shall be opened to traffic within two hours of its application.

Curing. The slurry seal shall cure for a minimum of 7 days before placement of the permanent pavement markings.

Method of Measurement. This work will be measured for payment as follows.

- (a) Contract Quantities. The requirements for the use of contract quantities shall conform to Article 202.07(a) of the Standard Specifications.
- (b) Measured Quantities. Crack/Joint sealing will be measured for payment in feet (meters), measured along the crack.

Pavement marking removal will be measured for payment according to Article 783.05 of the Standard Specifications.

The slurry seal will be measured for payment in place and the area computed in square yards (square meters). The width for measurement will be the width of the top surface as shown on the plans or as directed by the Engineer.

Prime coat, when required, will be measured for payment according to Article 406.13(b) of the Standard Specifications.

Basis of Payment. Crack/joint sealing will be paid for at the contract unit price per foot (meter) of FIBER-MODIFIED ASPHALT CRACK SEALING.

Bump removal will be paid for at the contract unit price per each for BUMP REMOVAL.

Pavement marking removal and pavement marker removal will be paid for according to Article 783.06 of the Standard Specifications.

Slurry seal will be paid for at the contract unit price per square yard (square meter) for ASPHALTIC EMULSION SLURRY SEAL, of the friction aggregate mixture specified.

Prime coat, when required, will be paid for according to Article 406.14 of the Standard Specifications.

Designer Note: Insert this special into contracts using an A-1 bituminous surface treatment. Use of this special provision shall be according to the Bureau of Design and Environment Manual, Chapter 52.

The designer must specify the gradation for the bituminous surface treatment on the plans. Districts are encouraged to use the CA 20 gradation as it has proven to perform well for A-1 surface treatments.

Include Special Provision on Temporary Flexible Raised Pavement Marker with this work.

Include the following information in the Traffic Control Plan Special Provision:

- Contractor shall post the roadway with "LOOSE GRAVEL" and SPEED LIMIT 35" signs in accordance with applicable articles of Division 700 of the Standard Specifications.
- These signs shall be placed at the start of the work, near intersecting roadways and then at an average spacing of 0.5 mi (0.8 km).
- The signs may be removed as soon as the sweeping operation has been completed.

PREVENTIVE MAINTENANCE - BITUMINOUS SURFACE TREATMENT (BDE)

Effective: January 1, 2009

Revised: April 1, 2009

Description. This work shall consist of constructing a single bituminous surface treatment (A-1).

Materials. Materials shall be according to the following Articles/Sections of the Standard Specifications.

Item	Article/Section
(a) Seal Coat Aggregate (Note 1)	1003, 1004.03
(b) Bituminous Materials (Note 2)	1032

Note 1. The seal coat aggregate shall be either fine or coarse aggregate.

When fine aggregate is used, it shall be stone sand (other than limestone and dolomite), wet bottom boiler slag, slag sand, or steel slag sand. The aggregate quality shall be Class C. The aggregate gradation shall be FA 1 (Special) or FA 4 (Special) as specified on the plans at shall meet the following.

FINE AGGREGATE GRADATIONS						
Grad. No.	Sieve Size and Percent Passing					
	3/8 in. (9.5 mm)	No. 4 (4.75 mm)	No. 8 (2.36 mm)	No. 16 (1.18 mm)	No. 40 (425 µm)	No. 200 (75 µm)
FA 1 (Special)	100	90 ± 10	62.5 ± 17.5	32.5 ± 7.5	7.5 ± 7.5	1.5 ± 1
FA 4 (Special)	100	--	--	2 ± 2	--	1.5 ± 1

When coarse aggregate is used, it shall be crushed gravel, crushed stone, wet bottom boiler slag, crushed slag, crushed sandstone, or crushed steel slag. The aggregate quality shall be Class C and the aggregate shall have a maximum of 25 percent chert by weight (mass). The aggregate gradation shall be CA 15, CA 16, or CA 20 as specified on the plans.

Note 2. The bituminous material shall be either a CRSP or an HFP polymer modified emulsified asphalt meeting the requirements of Article 1032.06(f)(2) of the Standard Specifications.

Equipment. Equipment shall be according to the following Articles/Sections of the Standard Specifications.

Item	Article/Section
(a) Self-Propelled Pneumatic-Tired Roller (Note 1)	1101.01
(b) Mechanical Sweeper (Note 2)	1101.03
(c) Aggregate Spreaders (Note 3)	1102.04
(d) Pressure Distributor (Note 4)	1102.05
(e) Heating Equipment	1102.07

Note 1. There shall be a minimum of two rollers, with the final number of rollers determined by the rollers' abilities to maintain proper spacing with the aggregate spreader as directed by the Engineer.

Note 2. The mechanical sweeper shall be power driven and self-propelled with the broom located between the axles. The mechanical sweeper shall not use a cantilever-mounted broom and the broom rotation shall not be operated by forward movement.

Note 3. The aggregate spreader shall be a self-propelled mechanical type with the receiving hopper in the rear and shall pull the aggregate truck. The spreader shall be fitted with an automated system which provides positive interconnected control of the aggregate flow with the forward speed of the spreader. The automated system shall provide uniform and consistent aggregate application at the rate specified.

The Engineer will check the spread roll of the aggregate spreader for straightness each day before operations begin. Should the surface of the spread roll vary off a straight line along its longitudinal dimension by more than 1/16 in. (1.5 mm), the Engineer will inspect the application of aggregate for corrugations and, should these occur, the machine shall be repaired or replaced. The forward speed of the spreader during calibration shall be the same as is to be used during construction. The equipment required for aggregate spreader calibration may consist of several sheets of canvas, each being exactly 1 sq yd (0.8 sq m), and a weight scale. By making several runs at different gate openings over the sheets of canvas, placed to cover the full width applied by the spreader, and carefully measuring the aggregate on each canvas sheet, the gate opening at the pre-established speed required to apply aggregate at the specified rate may be determined.

Note 4. The pressure distributor shall have a minimum capacity of 3000 gal (11,500 L). The application rate control shall be automated and shall control the application rate regardless of ground speed or spray bar width. The computer shall have the capability of recording the application rate, gallons sprayed, square yards, and feet traveled. The pressure distributor shall be capable of maintaining the asphalt emulsion at the specified temperature. The spray bar nozzles shall produce a uniform triple lap application fan spray, and the shutoff shall be instantaneous, with no dripping. The pressure distributor shall be capable of maintaining the specified application rate within ± 0.015 gal/sq yd (± 0.070 L/sq m) for each load. The spray-bar nozzles shall be turned to make the same angle with the longitudinal axis of the spray bar as recommended by the manufacturer.

Application rates shall be determined by the procedures listed in ASTM D 2995, except the sample may be taken on three 8 x 12 in. (200 x 300 mm) metal plates. The three plates shall be positioned as directed by the Engineer.

CONSTRUCTION REQUIREMENTS

Weather Limitations. This work shall be done between May 1 and August 31. Bituminous materials shall be applied only when the temperature of the air in the shade is above 55 °F (13 °C). No work shall be started if local conditions indicate that rain is imminent.

This work may be done between September 1 and September 15 provided both of the following conditions are met:

- (a) The temperature of the air in the shade is above 70 °F (20 °C) and the temperature of the surface to which the asphalt will be applied is 70 °F (20 °C) or above, and
- (b) The National Weather Service forecast for the area does not show any rain or any temperatures below 55 °F (13 °C) for the day the work is to be done or for the following five days.

Repair and Preparation of Base or Existing Surface. The base or existing surface shall be prepared according to Section 358 of the Standard Specifications.

Calibration. The working day prior to starting construction, the pressure distributor and aggregate spreader shall be calibrated and adjusted according to the manufacturer's recommendations. At least three days prior to starting the work the Contractor shall provide the Engineer with a copy of the manufacturer's recommendations for the equipment to be used. All calibrations and adjustments shall be made in the presence of the Engineer on a level surface at a location approved by the Engineer. The Contractor shall maintain proper calibration and adjustment of the equipment and the Engineer reserves the right to check application rates as the work progresses. Should the equipment fail to consistently apply the specified rates, the work shall be stopped and the Contractor shall recalibrate and readjust the equipment.

Application Rates. Based upon the aggregate gradation to be used, the Contractor shall determine the application rates of bituminous material and seal coat aggregate. The application rates along with the seal coat gradations shall be submitted to the Engineer for approval prior to the start of work. Application rates shall be according to the following table for the aggregate type shown on the plans, and shall result in aggregate embedment between 50 and 70 percent behind the roller. Changes in the application rate of greater than 15 percent shall be resubmitted to the Engineer for approval.

Aggregate Type	Bituminous Material Rate	Aggregate Rate
CA 15	0.38 – 0.46 gal/sq yd (1.7 – 2.1 L/sq m)	22 – 30 lb/sq yd (12 – 16 kg/sq m)
CA 16	0.36 – 0.40 gal/sq yd (1.6 – 1.8 L/sq m)	18 – 26 lb/sq yd (8 – 14 kg/sq m)
CA 20	0.36 – 0.40 gal/sq yd (1.6 – 1.8 L/sq m)	18 – 26 lb/sq yd (8 – 14 kg/sq m)
FA 1 (Special)	0.26 – 0.30 gal/sq yd (1.2 – 1.4 L/sq m)	16 – 20 lb/sq yd (9 – 11 kg/sq m)
FA 4 (Special)	0.28 – 0.36 gal/sq yd (1.3 – 1.6 L/sq m)	18 – 24 lb/sq yd (10 – 13 kg/sq m)

Preparation of Bituminous Material. The temperature of the bituminous material at the time of application shall be such that it will spray uniformly without clogging the spraying nozzles and shall be applied within the temperature ranges of 150 – 190 °F (65 – 90 °C).

Preparation of Aggregate. The aggregate shall be stockpiled near the jobsite according to Article 1003.01(e) or 1004.01(e) of the Standard Specifications. The aggregate used shall contain no free moisture. Slightly damp aggregate may be used with the approval of the Engineer.

Application of Bituminous Material. The bituminous material shall be applied with a pressure distributor. The entire length of the spray bar shall be set at the height above the surface recommended by the manufacturer for even distribution of the bituminous material.

The distributor shall be operated in a manner such that missing or overlapping of transverse joints will be avoided. To prevent overlapping of successive applications of bituminous material at transverse joints, heavy paper shall be spread over the previously applied bituminous material and aggregates. In order to obtain a uniform application of the bituminous material, the distributor shall be traveling at the speed required for the specified rate of application when the spray bar crosses the paper.

Adjacent construction, such as concrete pavement, curb and gutter, bridge floors, raised reflective pavement markers, and bridge handrails, shall be protected by shields, covers or other means. If bituminous material is applied to adjacent construction, the Contractor shall remove such material to the satisfaction of the Engineer.

The emulsified asphalt shall not be applied when the wind conditions will inhibit uniform coverage from the fans of asphalt being applied.

Application of Aggregates. The seal coat aggregates shall be spread evenly with an aggregate spreader over the entire surface being treated. When treating one-half of the pavement width at a time, an inside strip of uncovered emulsified asphalt 3 in. (75 mm) wide shall be left during construction of the first half to provide center joint overlap when the second half of the treatment is placed. In all cases, the aggregate shall be applied ahead of the truck or spreader wheels. Hand spreading will be permitted only when approved by the Engineer and, when so permitted, the aggregate shall be spread uniformly and at the approximate rate specified. Any ridges of aggregate left by the aggregate spreader shall be smoothed out with hand brooms immediately behind the aggregate spreader.

All equipment involved in the work shall operate as close to each other as practical. The aggregate shall cover the asphalt emulsion within 30 seconds of applications. At no time shall the aggregate spreader trail the pressure distributor by more than 150 ft (45 m) to ensure proper asphalt/aggregate adhesion.

Each aggregate truck shall be equipped with a suitable hitch for connection to the aggregate spreader while unloading. The trucks shall avoid contact between the truck body or bed and the aggregate spreader. The body or bed of the truck shall be modified, if necessary, to empty cleanly and completely into the receiving hopper of the aggregate spreader. No aggregate shall be allowed to spill onto the road surface when the truck is emptying into this hopper.

The aggregate shall be rolled following spreading. A maximum time of five minutes will be allowed between the spreading of aggregate and completion of the initial rolling of the aggregate. The rollers will proceed in a longitudinal direction at a speed less than or equal to 5 mph (8 km/h). Each roller will travel over the aggregate a minimum of two times. The entire surface shall be rolled immediately with a self-propelled pneumatic-tired roller. Rolling shall proceed in a longitudinal direction beginning at the edges and progressing toward the center, overlapping on successive trips by at least 1/2 the width of the roller. The aggregate shall then be rolled with a separate pneumatic-tired roller until the aggregate is properly seated in the bituminous material.

The Contractor shall use the appropriate sweeping equipment to perform an initial sweeping after a minimum of two hours curing and not less than one hour before sunset on the day the bituminous surface treatment is placed. The initial sweeping shall remove excess aggregate by lightly sweeping each pavement lane. The sweeping shall be sufficient to prevent migration of loose aggregate back onto any part of the pavement.

The Contractor shall sweep the pavement surface as needed to remove excess aggregate.

Opening to Traffic. The road shall be opened to traffic according to Article 701.17(c)(4) of the Standard Specifications.

Method of Measurement. The bituminous surface treatment will be measured for payment in place and the area computed in square yards (square meters). The width for measurement will be the top width of the bituminous surface treatment as shown on the plans or as directed by the Engineer.

Basis of Payment. This work will be paid for at the contract unit price per square yard (square meter) for BITUMINOUS SURFACE TREATMENT (PREVENTIVE MAINTENANCE).

When provided as a payment item, the preparation of the existing surface will be measured and paid for as specified in Section 358 of the Standard Specifications. If not provided as a payment item, preparation of existing surface will be paid for according to Article 109.04.

Designer Note: It should be inserted into 3P/SMART type projects with aggregate/earth shoulders at the edge of pavement or edge of shoulder strips (typical 1' safety shoulder). Note: The safety edge is intended to address edge of pavement drop-offs. It should not be applied lane to lane nor should it be applied at the edge of full paved shoulders (typically 3'/4' or wider). Also include CADD Standard for Safety Edge when using this special.

SAFETY EDGE (BDE)

Effective: April 1, 2011

Add the following to Article 406.06 of the Standard Specifications:

“(h) Safety Edge. The HMA surface course at the edge of pavement, or edge of shoulder strip, shall be finished with a safety edge as shown on the plans.

The device which forms the safety edge shall be mounted on the paver screed against the end gate and shall be removable or be able to be lifted when not in use. The device shall be designed to maintain contact with surface of the shoulder and allow automatic transition to cross roads, driveways and obstructions. The device shall also constrain the HMA material and increase the consolidation of the extruded profile. The use of a conventional single plate strike-off will not be allowed.

Rollers will not be allowed on the sloped face of the safety edge.”

Designer Note: Insert into contracts where a PCC inlay or overlay is selected. This method is for locations where excessive rutting has become a problem. Discuss with the Project Engineer, Operations and Implementation before using. Also, refer to BDE Manual, Chapter 53 before using.

PORTLAND CEMENT CONCRETE INLAY OR OVERLAY (BDE)

Effective: November 1, 2008

Description. This work shall consist of constructing a portland cement concrete inlay or overlay.

Materials. Materials shall be according to the following Articles/Sections of the Standard Specifications.

Item	Article/Section
(a) Portland Cement Concrete (Note 1)	1020
(b) Synthetic Fibers (Note 2)	
(c) Protective Coat	1023.01

Note 1. Class PV concrete shall be used, except the cement factor for central mixed concrete shall be 6.05 cwt/cu yd (360 kg/cu m). A cement factor reduction according to Article 1020.05(b)(9) of the Standard Specifications will be permitted, but shall not exceed a maximum 0.30 cwt/cu yd (18 kg/cu m). CA 5 shall not be used and CA 7 may only be used for overlays that are a minimum of 4.5 in. (113 mm) thick. The Class PV concrete shall have a minimum flexural strength of 550 psi (3800 kPa) or a minimum compressive strength of 3000 psi (20,700 kPa) at 14 days.

Note 2. Synthetic fibers shall be Type III according to ASTM C 1116. The synthetic fiber shall be a monofilament with a minimum length of 1/2 in. (13 mm) and a maximum length of 2 1/2 in. (63 mm), and shall have a maximum aspect ratio (length divided by the equivalent diameter of the fiber) of 100. The quantity of synthetic fiber(s) added to the concrete mixture shall be sufficient to have a minimum residual strength ratio (R_{150,3}) of 20.0 percent according to Illinois Modified ASTM C 1609. The maximum dosage rate shall not exceed 5.0 lb/cu yd (3.0 kg/cu m), unless the manufacturer can prove through a field demonstration that the concrete mixture will be workable and fiber clumping is not a problem as determined by the Engineer.

The synthetic fibers shall be added to the concrete and mixed per the manufacturer's recommendation.

The Department will maintain an approved list of synthetic fibers, which will include the minimum required dosage rate.

Equipment. Equipment shall be according to Article 420.03 of the Standard Specifications, except the mechanical saw used for cutting joints shall be equipped with an upcutting blade and a restricting skid plate to prevent spalling of the finished saw cut.

CONSTRUCTION REQUIREMENTS

CONSTRUCTION REQUIREMENTS

Preparation of Existing Pavement. The area to be overlaid shall be milled as shown on the plans. When patching is required, the patches shall be milled or their surface given a rough texture.

When detector loops are required, the loops shall be Type I or Type II according to Section 886 of the Standard Specifications. The detector loops shall be installed into the milled surface prior to cleaning.

Following milling, the surface shall be cleaned as follows.

- (a) Inlay/Overlay Plan Thickness less than 5 in. (125 mm). The surface shall be pressure washed with water at 2,000 psi (13,800 kPa) minimum.
- (b) Inlay/Overlay Plan Thickness of 5 in. (125 mm) or greater. The surface shall be cleaned with a mechanical broom, compressed air, or water.

The prepared surface shall meet the approval of the Engineer prior to proceeding with the work.

Forms and Form Setting. This work shall be according to Article 420.06 of the Standard Specifications, except wood forms of a height equal to the proposed inlay or overlay thickness may be used. Shims or wedges may be used to raise the forms to the specified plan elevation.

Treatment of Structures in the Pavement. Pavement round-outs shall be used at structures in the pavement. This work shall be as shown on the plans.

Placing. This work shall be according to Article 420.07 of the Standard Specifications, except standing water on the existing pavement surface shall be removed prior to concrete placement.

Final Strike Off, Consolidation, and Finishing. This work shall be according to Article 420.09 of the Standard Specifications, except when a Type B final finish is specified the artificial turf drag shall be replaced with a rough broom finish struck perpendicular to the direction of traffic flow. The rough broom finish shall be performed over the entire surface including tooled joints.

Joints. Joints shall be constructed at the locations and spacing shown on the plans. Field adjustments to the transverse joint locations will be permitted provided no transverse joint exceeds the planned spacing by more than ten percent.

The joints shall be mechanically sawed or hand tooled to 1/4 the depth of the inlay or overlay, and shall be a minimum 1/8 in. (3 mm) and a maximum 1/4 in. (6 mm) wide. Hand tooled joints may be used on mainline pavement when the posted speed limit is less than or equal to 40 mph, and on other locations as approved by the Engineer. Sawed joints shall be constructed as soon as the concrete will support the weight of the saw and operator without disturbing the final finish. Hand tooled joints shall be edged with an edging tool having a 1/8 in. (3 mm) radius. Care shall be taken to minimize displacement of the finished surface.

Opening to Traffic. The road shall be opened to traffic according to Article 420.13 of the Standard Specifications, except curing may be discontinued and the pavement opened to traffic when a minimum flexural strength of 550 psi (3800 kPa) or a minimum compressive strength of 3000 psi (20,700 kPa) is attained.

Method of Measurement. This work will be measured for payment according to Article 420.19 of the Standard Specifications.

Detector loops, when required, will be measured for payment according to Article 886.05 of the Standard Specifications.

Basis of Payment. This work will be paid for at the contract unit price per square yard (square meter) for PORTLAND CEMENT CONCRETE INLAY or PORTLAND CEMENT CONCRETE OVERLAY, of the thickness specified.

Protective coat will be paid for according to Article 420.20 of the Standard Specifications.

Detector loops, when required, will be paid for according to according to Article 886.06 of the Standard Specifications.

Designer Note: Insert into contracts with milling and resurfacing with permanent speed limits up to 40 mph.

DRAINAGE AND INLET PROTECTION UNDER TRAFFIC (BDE)

Effective: April 1, 2011

Add the following to Article 603.02 of the Standard Specifications:

- “(i) Temporary Hot-Mix Asphalt (HMA) Ramp (Note 1) 1030
- “(j) Temporary Rubber Ramps (Note 2)

Note 1. The HMA shall have maximum aggregate size of 3/8 in. (95 mm).

Note 2. The rubber material shall be according to the following.

Property	Test Method	Requirement
Durometer Hardness, Shore A	ASTM D 2240	75 ±15
Tensile Strength, psi (kPa)	ASTM D 412	300 (2000) min
Elongation, percent	ASTM D 412	90 min
Specific Gravity	ASTM D 792	1.0 - 1.3
Brittleness, °F (°C)	ASTM D 746	-40 (-40)°

Revise Article 603.07 of the Standard Specifications to read:

“603.07 Protection Under Traffic. After the casting has been adjusted and Class SI concrete has been placed, the work shall be protected by a barricade and two lights for at least 72 hours.

When castings are under traffic before the final surfacing operation has been started, properly sized temporary ramps shall be placed around the drainage and/or utility castings according to the following methods.

- (a) Temporary Asphalt Ramps. Temporary hot-mix asphalt ramps shall be placed around the casting, flush with its surface and decreasing to a featheredge in a distance of 2 ft (600 mm) around the entire surface of the casting.
- (b) Temporary Rubber Ramps. Temporary rubber ramps shall only be used on roadways with permanent posted speeds of 40 mph or less and when the height of the casting to be protected meets the proper sizing requirements for the rubber ramps as shown below.

Dimension	Requirement
Inside Opening	Outside dimensions of casting + 1 in. (25 mm)
Thickness at inside edge	Height of casting \pm 1/4 in. (6 mm)
Thickness at outside edge	1/4 in. (6 mm) max.
Width, measured from inside opening to outside edge	8 1/2 in. (215 mm) min

Placement shall be according to the manufacturer's specifications.

Temporary ramps for castings shall remain in place until surfacing operations are undertaken within the immediate area of the structure. Prior to placing the surface course, the temporary ramp shall be removed. Excess material shall be disposed of according to Article 202.03."

67100

671.00

Designer Note: Insert into all contracts.

SUBCONTRACTOR MOBILIZATION PAYMENTS (BDE)

Effective: April 2, 2005

Revised: April 1, 2011

To account for the preparatory work and operations necessary for the movement of subcontractor personnel, equipment, supplies, and incidentals to the project site and for all other work or operations that must be performed or costs incurred when beginning work approved for subcontracting according to Article 108.01 of the Standard Specifications, the Contractor shall make a mobilization payment to each subcontractor.

This mobilization payment shall be made at least 14 days prior to the subcontractor starting work. The amount paid shall be equal to 3 percent of the amount of the subcontract reported on form BC 260A submitted for the approval of the subcontractor's work.

The mobilization payment to the subcontractor is an advance payment of the reported amount of the subcontract and is not a payment in addition to the amount of the subcontract; therefore, the amount of the advance payment will be deducted from future progress payments.

This provision shall be incorporated directly or by reference into each subcontract approved by the Department.

70110

701.10

Designer Note: Insert into all contracts with the Traffic Control Surveillance pay item.

TRAFFIC CONTROL SURVEILLANCE (BDE)

Effective: January 1, 2011

Revise the first sentence of the first paragraph of Article 701.10 of the Standard Specifications to read:

“When open holes, broken pavement, trenches over 3 in. (75 mm) deep and 4 in. (100 mm) wide or other hazards are present within 8 ft (2.4 m) of the edge of an open lane, the Contractor shall furnish traffic control surveillance during all hours when the Contractor is not engaged in construction operations.”

Designer Note: Use on all contracts using impact attenuators in a **permanent** application. This special provision supercedes both the Recurring Special Provision "Traffic Barrier Terminal Type 3, Special" and the BDE Special Provision "Sand Module Impact Attenuators. These pay items should no longer be used. See BDE Manual, Chapter 38 before using. Attenuator bases are now required for sand module installations.

IMPACT ATTENUATORS (BDE)

Effective: November 1, 2003

Revised: November 1, 2008

Description. This work shall consist of furnishing and installing impact attenuators of the category and test level specified.

Materials. Materials shall meet the requirements of the impact attenuator manufacturer and the following:

Item	Article/Section
(a) Fine Aggregate (Note 1).....	1003.01
(b) Steel Posts, Structural Shapes, and Plates	1006.04
(c) Rail Elements, End Section Plates, and Splice Plates	1006.25
(d) Bolts, Nuts, Washers and Hardware.....	1006.25
(e) Hollow Structural Tubing	1006.27(b)
(f) Wood Posts and Wood Blockouts	1007.01, 1007.02, 1007.06
(g) Preservative Treatment.....	1007.12

Note 1. Fine aggregate shall be FA 1 or FA 2, Class A quality. The sand shall be unbagged and shall have a maximum moisture content of five percent.

CONSTRUCTION REQUIREMENTS

General. Impact attenuators shall meet the testing criteria contained in National Cooperative Highway Research Program (NCHRP) Report 350 for the test level specified and shall be on the Department's approved list. Fully redirective and partially redirective attenuators shall also be designed for bi-directional impacts.

Installation. Regrading of slopes or approaches for the installation shall be as shown on the plans.

Bases for impact attenuators, other than sand modules, shall be installed when required by the manufacturer. The bases shall be constructed on a prepared subgrade according to the manufacturer's specifications. The surface of the base shall be slightly sloped or crowned to facilitate drainage.

Bases for sand module impact attenuators will be required. The bases shall be constructed of either portland cement concrete or hot-mix asphalt (HMA). Portland cement concrete bases shall be 6 in. (150 mm) thick and be according to the applicable requirements of Section 424 of the Standard Specifications. HMA bases shall be 8 in. (200 mm) thick and be according to the applicable requirements of Section 408 of the Standard Specifications. The surface of the base

shall be slightly sloped or crowned to facilitate drainage. The perimeter of each module and the specified weight (mass) of sand in each module shall be painted on the surface of the base.

Impact attenuators shall be installed according to the manufacturer's specifications and include all necessary transitions between the impact attenuator and the item to which it is attached.

Method of Measurement. This work will be measured for payment as each, where each is defined as one complete installation.

Contract quantities for sand module attenuator bases may be accepted according to Article 202.07(a) of the Standard Specifications. When measured, sand module attenuator bases will be measured in place and the dimensions used to calculate square yards (square meters) will not exceed those as shown on the plans.

Basis of Payment. This work, will be paid for at the contract unit price per each for IMPACT ATTENUATORS (FULLY REDIRECTIVE, NARROW); IMPACT ATTENUATORS (FULLY REDIRECTIVE, WIDE); IMPACT ATTENUATORS (FULLY REDIRECTIVE, RESETTABLE); IMPACT ATTENUATORS (SEVERE USE, NARROW); IMPACT ATTENUATORS (SEVERE USE, WIDE); IMPACT ATTENUATORS (PARTIALLY REDIRECTIVE); or IMPACT ATTENUATORS (NON-REDIRECTIVE), of the test level specified.

Sand module attenuator bases will be paid for at the contract unit price per square yard (square meter) for ATTENUATOR BASE.

Regrading of slopes or approaches will be paid for according to Section 202 and/or Section 204 of the Standard Specifications.

Designer Note: Use on all contracts using impact attenuators in a **temporary** application. This special provision supercedes both the Recurring Special Provision, "Traffic Barrier Terminal Type 3, Special" and BDE Special Provision, "Sand Module Impact Attenuators." These pay items should no longer be used. See BDE Manual, Chapter 38 before using.

IMPACT ATTENUATORS, TEMPORARY (BDE)

Effective: November 1, 2003

Revised: January 1, 2007

Description. This work shall consist of furnishing, installing, maintaining, and removing temporary impact attenuators of the category and test level specified.

Materials. Materials shall meet the requirements of the impact attenuator manufacturer and the following:

Item	Article/Section
(a) Fine Aggregate (Note 1)	1003.01
(b) Steel Posts, Structural Shapes, and Plates	1006.04
(c) Rail Elements, End Section Plates, and Splice Plates	1006.25
(d) Bolts, Nuts, Washers and Hardware	1006.25
(e) Hollow Structural Tubing	1006.27(b)
(f) Wood Posts and Wood Blockouts	1007.01, 1007.02, 1007.06
(g) Preservative Treatment	1007.12
(h) Packaged Rapid Hardening Mortar	1018.01

Note 1. Fine aggregate shall be FA 1 or FA 2, Class A quality. The sand shall be unbagged and shall have a maximum moisture content of five percent.

CONSTRUCTION REQUIREMENTS

General. Impact Attenuators shall meet the testing criteria contained in National Cooperative Highway Research Program (NCHRP) Report 350 for the test level specified and shall be on the Department's approved list.

Installation. Regrading of slopes or approaches for the installation shall be as shown on the plans.

Attenuator bases, when required by the manufacturer, shall be constructed on a prepared subgrade according to the manufacturer's specifications. The surface of the base shall be slightly sloped or crowned to facilitate drainage.

Impact attenuators shall be installed according to the manufacturer's specifications and include all necessary transitions between the impact attenuator and the item to which it is attached.

When water filled attenuators are used between November 1 and April 15, they shall contain anti-freeze according to the manufacturer's recommendations.

Markings. Sand module impact attenuators shall be striped with alternating reflectorized Type AA or Type AP fluorescent orange and reflectorized white horizontal, circumferential stripes. There shall be at least two of each stripe on each module.

Other types of impact attenuators shall have a terminal marker applied to their nose and reflectors along their sides.

Maintenance. All maintenance of the impact attenuators shall be the responsibility of the Contractor until removal is directed by the Engineer.

Relocate. When relocation of temporary impact attenuators is specified, they shall be removed, relocated and reinstalled at the new location. The reinstallation requirements shall be the same as those for a new installation.

Removal. When the Engineer determines the temporary impact attenuators are no longer required, the installation shall be dismantled with all hardware becoming the property of the Contractor.

Surplus material shall be disposed of according to Article 202.03. Anti-freeze, when present, shall be disposed of/recycled according to local ordinances.

When impact attenuators have been anchored to the pavement, the anchor holes shall be repaired with rapid set mortar. Only enough water to permit placement and consolidation by rodding shall be used and the material shall be struck-off flush.

Method of Measurement. This work will be measured for payment as each, where each is defined as one complete installation.

Basis of Payment. This work will be paid for at the contract unit price per each for IMPACT ATTENUATORS, TEMPORARY (FULLY REDIRECTIVE, NARROW); IMPACT ATTENUATORS, TEMPORARY (FULLY REDIRECTIVE, WIDE); IMPACT ATTENUATORS, TEMPORARY (FULLY REDIRECTIVE, RESETTABLE); IMPACT ATTENUATORS, TEMPORARY (SEVERE USE, NARROW); IMPACT ATTENUATORS, TEMPORARY (SEVERE USE, WIDE); or IMPACT ATTENUATORS, TEMPORARY (NON-REDIRECTIVE) of the test level specified.

Relocation of the devices will be paid for at the contract unit price per each for IMPACT ATTENUATORS, RELOCATE (FULLY REDIRECTIVE); IMPACT ATTENUATORS, RELOCATE (SEVERE USE); or IMPACT ATTENUATORS, RELOCATE (NON-REDIRECTIVE); of the test level specified.

Regrading of slopes or approaches will be paid for according to Section 202 and/or Section 204 of the Standard Specifications.

Designer Note: Insert into all contracts using PCC or mixtures using cement.

CEMENT (BDE)

Effective: January 1, 2007

Revised: April 1, 2011

Revise Section 1001 of the Standard Specifications to read:

“SECTION 1001. CEMENT

1001.01 Cement Types. Cement shall be according to the following.

- (a) Portland Cement. Acceptance of portland cement shall be according to the current Bureau of Materials and Physical Research’s Policy Memorandum, “Portland or Blended Cement Acceptance Procedure for Qualified and Non-Qualified Plants”.

Portland cement shall be according to AASHTO M 85, and shall meet the standard physical and chemical requirements. The Contractor has the option to use any type of portland cement listed in AASHTO M 85 unless a specific cement is specified for a construction item. Inorganic processing additions shall be limited to granulated blast-furnace slag according to the chemical requirements of AASHTO M 302, Class C or F fly ash according to the chemical requirements of AASHTO M 295, and cement kiln dust.

- (b) Portland-Pozzolan Cement. Acceptance of portland-pozzolan cement shall be according to the current Bureau of Materials and Physical Research’s Policy Memorandum, “Portland or Blended Cement Acceptance Procedure for Qualified and Non-Qualified Plants”.

Portland-pozzolan cement shall be according to AASHTO M 240 and shall meet the standard physical and chemical requirements. The Contractor has the option to use portland-pozzolan cement unless a specific cement is specified for a construction item. Inorganic processing additions shall be limited to granulated blast-furnace slag according to the chemical requirements of AASHTO M 302, Class C or F fly ash according to the chemical requirements of AASHTO M 295, and cement kiln dust. The pozzolan constituent for Type IP using Class F fly ash shall be a maximum of 25 percent of the weight (mass) of the portland-pozzolan cement. The pozzolan constituent for Type IP using Class C fly ash shall be a maximum of 30 percent of the weight (mass) of the portland-pozzolan cement. The pozzolan constituent for Type IP using microsilica or high-reactivity metakaolin shall be a maximum of ten percent. The pozzolan constituent for Type IP using other materials shall have the approval of the Engineer.

Portland-pozzolan cement may be used in concrete mixtures when the air temperature is below 40 °F (4 °C), but the Engineer may request a trial batch of the concrete mixture to show the mix design strength requirement will be met.

- (c) Portland Blast-Furnace Slag Cement. Acceptance of portland blast-furnace slag cement shall be according to the current Bureau of Materials and Physical Research’s Policy

Memorandum, "Portland or Blended Cement Acceptance Procedure for Qualified and Non-Qualified Plants".

Portland blast-furnace slag cement shall be according to AASHTO M 240 and shall meet the standard physical and chemical requirements. The Contractor has the option to use portland blast-furnace slag cement unless a specific cement is specified for a construction item. Inorganic processing additions shall be limited to granulated blast-furnace slag according to the chemical requirements of AASHTO M 302, Class C or F fly ash according to the chemical requirements of AASHTO M 295, and cement kiln dust. The blast-furnace slag constituent for Type IS shall be a maximum of 35 percent of the weight (mass) of the portland blast-furnace slag cement.

Portland blast-furnace slag cement may be used in concrete mixtures when the air temperature is below 40 °F (4 °C), but the Engineer may request a trial batch of the concrete mixture to show the mix design strength requirement will be met.

- (d) Rapid Hardening Cement. Rapid hardening cement shall be used according to Article 1020.04 or when approved by the Engineer. The cement shall be on the Department's current "Approved List of Packaged, Dry, Rapid Hardening Cementitious Materials for Concrete Repairs", and shall be according to the following.
- (1) The cement shall have a maximum final set of 25 minutes, according to Illinois Modified AASHTO T 131.
 - (2) The cement shall have a minimum compressive strength of 2000 psi (13,800 kPa) at 3.0 hours, 3200 psi (22,100 kPa) at 6.0 hours, and 4000 psi (27,600 kPa) at 24.0 hours, according to Illinois Modified AASHTO T 106.
 - (3) The cement shall have a maximum drying shrinkage of 0.050 percent at seven days, according to Illinois Modified ASTM C 596.
 - (4) The cement shall have a maximum expansion of 0.020 percent at 14 days, according to Illinois Modified ASTM C 1038.
 - (5) The cement shall have a minimum 80 percent relative dynamic modulus of elasticity; and shall not have a weight (mass) gain in excess of 0.15 percent or a weight (mass) loss in excess of 1.0 percent, after 100 cycles, according to Illinois Modified AASHTO T 161, Procedure B.
- (e) Calcium Aluminate Cement. Calcium aluminate cement shall be used according to Article 1020.04 or when approved by the Engineer. The cement shall meet the standard physical requirements for Type I cement according to AASHTO M 85, except the time of setting shall not apply. The chemical requirements shall be determined according to AASHTO T 105 and shall be as follows: minimum 38 percent aluminum oxide (Al_2O_3), maximum 42 percent calcium oxide (CaO), maximum 1 percent magnesium oxide (MgO), maximum 0.4 percent sulfur trioxide (SO_3), maximum 1 percent loss on ignition, and maximum 3.5 percent insoluble residue.

1001.02 Uniformity of Color. Cement contained in single loads or in shipments of several loads to the same project shall not have visible differences in color.

1001.03 Mixing Brands and Types. Different brands or different types of cement from the same manufacturing plant, or the same brand or type from different plants shall not be mixed or used alternately in the same item of construction unless approved by the Engineer.

1001.04 Storage. Cement shall be stored and protected against damage, such as dampness which may cause partial set or hardened lumps. Different brands or different types of cement from the same manufacturing plant, or the same brand or type from different plants shall be kept separate.”

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Designer Note: Include on all interstate or four-lane highway projects that have thirty (30) working days or more. Calculate quantities assuming one mowing per fifty (50) working days. Do not use on new construction since mowing would have to be done only in areas where vegetation has been established.

MOWING

Effective December 11, 2001 Revised April 29, 2011

This work shall consist of mowing the entire median up to 60' (20m) in width and the roadway foreslopes of the outside lanes to the ditchline or for a width of 15' (4.572 meters) from the edge of pavement or paved shoulder, whichever is less. At intersecting roadways, the mowing shall extend to the proposed right of way for a distance of 150' (45 m) on either side of the intersection. The height of the mowing shall not be more than 6" (150 mm). Equipment used shall be capable of completely severing all growth at the cutting height and distributing it evenly over the mowed area. The Contractor will not be required to mow continuously wet ditches and drainage ways, slopes greater than 1:3 (V:H), or areas which may be designated by the Engineer as not mowable. Mowing shall be done within the project limits during the construction of the project as directed by the Engineer and prior to the final inspection of the project. Any subsequent mowing required to disperse mowed material shall be considered as included in the cost of the mowing. Debris encountered during mowing, which interferes with the mowing operation or is visible from the roadway shall be removed and disposed of according to Article 250.05.

Method of Measurement: Mowing will be measured for payment in units of 100' (30 m) in horizontal distances along the roadway center line/survey line. For purposes of measurement, the quantity of units to be paid for each individual mowing is defined as the net length of the project as shown on the cover sheet of the construction plans divided by 100' (30 m). On and off ramps will not be measured separately. No allowances will be made for variations in width of mowing.

Basis of Payment: This work will be paid for at the contract unit price per unit for MOWING.

Designer Note: Include in all contracts with HMA overlays or full depth HMA pavements.

HOT-MIX ASPHALT – PRIME COAT

Effective: April 29, 2011

Revise the second paragraph of Article 406.02 of the Standard Specifications to read:

“When emulsified asphalts are used, any dilution with water must be performed by the manufacturer. The emulsified asphalt shall be thoroughly agitated within 24 hours of application and show no separation of water and emulsion.”

Revise the first paragraph of Article 406.05(b) of the Standard Specifications to read:

“Prime Coat. The base, or base and gutter shall be clean and dry. The bituminous priming material shall be prepared according to Article 403.05 and applied according to Article 403.10.”

Revise the first paragraph of Article 406.05(b)(1) of the Standard Specifications to read:

“(1) Brick, Concrete or HMA Bases. The prime shall be applied uniformly at a residual asphalt rate of 0.02 to 0.06 gal/sq yd (0.1 to 0.3 L/sq m). The exact residual asphalt rate will be specified by the Engineer, typically 0.04 gal/sq yd for milled surfaces and 0.025 gal/sq yd for smooth surfaces. Prior to priming, the residual asphalt rate shall be verified by passing the applicator truck over a 1 ft x 1 ft pre-weighed cardboard square, drying the cardboard and prime to a constant mass, and determining the final dry weight. The difference between the two weights will be the residual asphalt weight per square foot. The residual asphalt weight per square foot shall be converted to gallons per square yard using a residual asphalt specific gravity of 1.03.”

Add the following to the second paragraph of Article 406.05(b)(1):

“When prime coat is applied on two lane roadways, the pavement shall be primed one lane at a time. The primed lane shall remain closed for a minimum of one hour and shall remain closed until the prime does not pickup under traffic. On multi-lane pavements, traffic will not be allowed on the primed surface until it is fully cured, such that it does not pickup under traffic.”

Replace the last sentence of the third paragraph of Article 406.05(b)(1) with the following:

“Prime coat shall be fully cured prior to placement of HMA to prevent pickup by haul trucks or paving equipment. If pickup occurs, paving shall cease in order to provide additional cure time, or an approved release agent may be applied to the tires of the haul trucks or paving equipment as needed to prevent pickup of the prime coat.”

100401

1004.01

Designer Note: Check with Materials before using this special provision. This special can be used Districtwide since this is the lowest quality (B) rock. This special provision is intended to be used when Coarse Aggregate Fill is recommended for ground stabilization, undercuts less than 18" in thickness, or as a capping material for Rockfill. Remember when using precast box culverts, a 6" bedding layer is included in the box culvert pay item.

A filter fabric may be required. Discuss with the District Geotechnical Engineer (DGE). The DGE may want you to provide a pay item/quantity for "Geotechnical Fabric for Ground Stabilization."

You will need to include a detail and quantity table in the plans showing the depth and location. This could be on the typical or cross-sections.

COARSE AGGREGATE FILL

Effective April 29, 2011

This work shall consist of furnishing, transporting and placing Coarse Aggregate Fill for ground stabilization.

The material shall meet Quality Designation "B" as required in Article 1004.01 of the Standard Specifications for Road and Bridge Construction. The material shall be crushed stone and meet the gradation of CA 7 or CA 11 per Article 1004.01 of the Standard Specifications for Road and Bridge Construction.

The aggregate shall be placed in 6 in. (150 mm) lifts, loose measurement, and compacted in a manner approved by the Engineer, except that if the desired results are being obtained, the compacted thickness of any lift may be increased to a maximum of 8 in. (200 mm).

Coarse Aggregate Fill will be measured for payment in tons (metric tons), in accordance with Article 311.08 except that all references to cubic yard (cubic meter) measurement and payment shall be deleted.

This work will be paid for at the contract unit price per ton (metric ton) for COARSE AGGREGATE FILL.

Designer Note:

1. In order to facilitate the use of available local crushed stone aggregates, which do not meet the Standard Specifications, the following Special Provision should be considered when you have the following pay items:
 - A. Granular Embankment, Special
 - B. Sub-base Granular Material
 - C. Aggregate Shoulders
 - D. Aggregate Surface Course
 - E. Aggregate Base Course
 - F. Erosion Control Aggregate
2. Check with the District Bureau of Materials to see if this Special Provision should be used. Intended for the western area of District - approximate dividing line is Route 97.
3. Be sure to include this Special Provision when you have incidental aggregate items, such as the granular sub-base under approach slabs when in the areas as specified in E. above.

9-23-96 Revised Article.

AGGREGATE QUALITY

Effective July 1, 1990

Revised September 23, 1996

Coarse aggregate for Granular Embankment Special, Sub-base Granular Material, Aggregate Shoulders, Aggregate Surface and Base Courses, and Erosion Control Aggregate shall conform to Article 1004.04 of the Standard Specifications for Road and Bridge Construction except that all of the following revisions to Article 1004.04(b) shall apply unless the Contractor chooses to use RAP for aggregate shoulders:

1. Revise the maximum allowable percentage of weighted average loss when the material is subjected to 5 cycles of sodium sulfate soundness test from 25%, as shown under the Class D of the Quality Chart in Article 1004.01(b) of the Standard Specifications, to 40%; and
2. Revise the maximum allowable percentage of wear as determined by the Los Angeles Abrasion Method from 45%, as shown under Class D of the Quality Chart in Article 1004.01(b) of the Standard Specifications, to 65%; and
3. The sum of the percentages of weighted average loss when the material is subjected to 5 cycles of the sodium sulfate soundness test and the percentage of wear as determined by the Los Angeles Abrasion Method shall not exceed 95%.