



Illinois Department of Transportation

Memorandum

To: *

From: Rich Dotson

Subject: **Special Provision Changes** *RJD*

Date: January 25, 2012

The following special provisions have been revised for the April 27, 2012 letting. Please revise your special provision books as indicated.

Recurring Special Provisions

Replace Designer Notes with updated sheets for 2012.

Interim Special Provisions

ISP Number	Description
Alphabetic ISP Index (Revised)	Remove existing alphabetic index and insert revised index.
Numerical ISP Index (Revised)	Remove existing numeric index and insert revised index.
100.00 (New)	"Errata for the 2012 Standard Specifications (BDE)" Corrects errors in Specifications Book.
107.00 (Revised)	"Construction Air Quality – Diesel Vehicle Emissions Control (BDE)" Revised reporting requirement.
108.00 (New)	"Payrolls and Payroll Records (BDE)" New special to comply with Public Act 097-0571.
303.00 (New)	"Aggregate Subgrade Improvement (BDE)" New special to allow RAP for sub-grade improvement.
400.01 (Revised)	"Preventative Maintenance – Cape Seal (BDE)" Minor revisions.
400.02 (Revised)	"Preventative Maintenance – Micro-Surfacing (BDE)" Minor revisions.
400.03 (Revised)	"Preventative Maintenance – Slurry Seal (BDE)" Minor revisions.
400.04 (Revised)	"Preventive Maintenance – Bituminous Surface (BDE)" Minor revisions.
406.07 (Revised)	"Hot-Mix Asphalt-Density Testing of Longitudinal Joints (BDE)" Increased the minimum edge distance for the location of density test and include density requirements for HMA Mixture IL-4.75.
420.00 (New)	"Portland Cement Concrete Inlay or Overlay (BDE)" Revised material specifications and acceptance of synthetic fibers.

Interim Special Provisions (Continued)

ISP Number	Description
442.00 (New)	"Calcium Chloride Accelerator for Class PP-2 Concrete (BDE)" Limits the use of calcium chloride accelerator to Class PP-2 concrete.
504.00 (New)	"Concrete Box Culverts with Skews > 30 Degrees and Design Fills ≤ 5 Feet (BDE)" Specifies use of LRFD method for precast and cast-in place concrete box culverts.
504.04 (New)	"Concrete Box Culverts with Skews ≤ 30 Degrees Regardless of Design Fill and Skews > 30 Degrees with Design Fills > 5 Feet (BDE)" Specifies use of LRFD method for precast and cast-in-place concrete box culverts.
780.01 (New)	"Modified Urethane Pavement Marking (BDE)" Provides a statewide specification for urethane markings to replace the District special.
703.02 (New)	"Pavement Marking Tape Type IV (BDE)" New product with better wet condition reflectivity.
780.02 (New)	"Preformed Plastic Pavement Marking Type D-Inlaid (BDE)" Allows a new option for better wet condition preformed plastic pavement markings.
1004.02 (New)	"Coarse Aggregate in Bridge Approach Slabs/Footings (BDE)" Clarifies aggregate materials allowed in approach slabs and approach footings.
1020.00 (Revised)	"Self-Consolidating Concrete for Precast and Precast Prestressed Products (BDE)" Revised for tighter requirements.
1020.01 (Revised)	"Self-Consolidating Concrete For Cast-in-Place Construction (BDE)" Revised for tighter requirements.

District Special Provisions & General Notes

District Number	Description
Alphabetic District Index (Revised)	Remove existing alphabetic index and insert revised index.
Numerical District Index (Revised)	Remove existing numeric index and insert revised index.
630.08 (Revised)	"Steel Plate Beam Guardrail, Type A, 6.75 Foot Posts" Minor revisions to clarify when to use.
668.02 (Revised)	"Permanent Survey Ties" Added a sentence to remind people about the survey requirements.
780.01 (Delete)	"Urethane Pavement Marking" Replaced by a BDE special.

RD:tdp:kme\mgr1\winword\progdev\special provisions\interim spec provs\specprovchnsgsmemo.doc

Attachment(s)

cc: *N. Jack Team 2 Team 6 Team 10 Galesburg Design
K. Emert Team 3 Team 7 Team 11 Local Roads (M. Augspurger)
T. Phillips Team 4 Team 8 Geometrics Materials (H. Shoup)
Team 1 Team 5 Team 9 Bridge (T. Inglis)

**Index for
Supplemental Specifications
and
Recurring Special Provisions**

INDEX
FOR
SUPPLEMENTAL SPECIFICATIONS
AND RECURRING SPECIAL PROVISIONS

Adopted January 1, 2012

This index contains a listing of SUPPLEMENTAL SPECIFICATIONS and frequently used RECURRING SPECIAL PROVISIONS.

SUPPLEMENTAL SPECIFICATIONS

Std. Spec. Sec.

Page No.

No Supplemental Specifications this year.

RECURRING SPECIAL PROVISIONS

The following RECURRING SPECIAL PROVISIONS indicated by an "X" are applicable to this contract and are included by reference:

<u>CHECK SHEET #</u>	<u>PAGE NO.</u>
1	1
2	4
3	5
4	15
5	20
6	25
7	26
8	27
9	28
10	31
11	34
12	36
13	40
14	42
15	43
16	45
17	46
18	48
19	49
20	50
21	54
22	56
23	58
24	60
25	61
26	62
27	63
28	64
29	65
30	68
31	76

BDE Special Provisions Checklist
For the April 27 and June 15, 2012 Lettings

Note: Specials that go in every contract have already been marked with an "X" for you.

Designer: _____

FAP: _____

Contract No.: _____

Section: _____

Letting: _____

County: _____

BDE SPECIAL PROVISIONS
For the April 27 and June 15, 2012 Lettings

The following special provisions indicated by an "x" are applicable to this contract and will be included by the Project Development and Implementation Section of the BD&E. An * indicates a new or revised special provision for the letting.

<u>File Name</u>	<u>#</u>	<u>Special Provision Title</u>	<u>Effective</u>	<u>Revised</u>
80240	1	Above Grade Inlet Protection	July 1, 2009	Jan. 1, 2012
80099	2	Accessible Pedestrian Signals (APS)	April 1, 2003	Jan. 1, 2007
80275	3	Agreement to Plan Quantity	Jan. 1, 2012	
* 80274	4	Aggregate Subgrade Improvement	April 1, 2012	
80192	5	Automated Flagger Assistance Device	Jan. 1, 2008	
80173	6	Bituminous Materials Cost Adjustments	Nov. 2, 2006	Jan. 1, 2012
80241	7	Bridge Demolition Debris	July 1, 2009	
80276	8	Bridge Relief Joint Sealer (NOTE: This special provision was previously named "Concrete Joint Sealer".)	Jan. 1, 2012	
5026I	9	Building Removal-Case I (Non-Friable and Friable Asbestos)	Sept. 1, 1990	April 1, 2010
5048I	10	Building Removal-Case II (Non-Friable Asbestos)	Sept. 1, 1990	April 1, 2010
5049I	11	Building Removal-Case III (Friable Asbestos)	Sept. 1, 1990	April 1, 2010
5053I	12	Building Removal-Case IV (No Asbestos)	Sept. 1, 1990	April 1, 2010
* 80291	13	Calcium Chloride Accelerator for Class PP-2 Concrete	April 1, 2012	
* 80292	14	Coarse Aggregate in Bridge Approach Slabs/Footings	April 1, 2012	
80198	15	Completion Date (via calendar days)	April 1, 2008	
80199	16	Completion Date (via calendar days) Plus Working Days	April 1, 2008	
* 80293	17	Concrete Box Culverts with Skews > 30 Degrees and Design Fills ≤ 5 Feet	April 1, 2012	
* 80294	18	Concrete Box Culverts with Skews ≤ 30 Degrees Regardless of Design Fill and Skews > 30 Degrees with Design Fills > 5 Feet	April 1, 2012	
80277	19	Concrete Mix Design – Department Provided	Jan. 1, 2012	
80261	20	Construction Air Quality – Diesel Retrofit	June 1, 2010	
* 80237	21	Construction Air Quality – Diesel Vehicle Emissions Control	April 1, 2009	Jan. 2, 2012
80239	22	Construction Air Quality – Idling Restrictions	April 1, 2009	
80177	23	Digital Terrain Modeling for Earthwork Calculations	April 1, 2007	
80029	24	Disadvantaged Business Enterprise Participation	Sept. 1, 2000	Aug. 2, 2011
80272	25	Drainage and Inlet Protection Under Traffic	April 1, 2011	Jan. 1, 2012
* 80296	26	X Errata for the 2012 Standard Specifications	April 1, 2012	
80228	27	Flagger at Side Roads and Entrances	April 1, 2009	
80265	28	Friction Aggregate	Jan. 1, 2011	
80229	29	Fuel Cost Adjustment	April 1, 2009	July 1, 2009
80169	30	High Tension Cable Median Barrier	Jan. 1, 2007	April 1, 2009
* 80246	31	Hot-Mix Asphalt – Density Testing of Longitudinal Joints	Jan. 1, 2010	April 1, 2012
80109	32	Impact Attenuators	Nov. 1, 2003	Jan. 1, 2012
80110	33	Impact Attenuators, Temporary	Nov. 1, 2003	Jan. 1, 2012
80045	34	Material Transfer Device	June 15, 1999	Jan. 1, 2009
80203	35	Metal Hardware Cast into Concrete	April 1, 2008	Jan. 1, 2012
* 80297	36	Modified Urethane Pavement Marking	April 1, 2012	
80165	37	Moisture Cured Urethane Paint System	Nov. 1, 2006	Jan. 1, 2010
80253	38	Movable Traffic Barrier	Jan. 1, 2010	Jan. 1, 2012
80231	39	Pavement Marking Removal	April 1, 2009	
* 80298	40	Pavement Marking Tape Type IV	April 1, 2012	
80254	41	Pavement Patching	Jan. 1, 2010	
80022	42	X Payments to Subcontractors	June 1, 2000	Jan. 1, 2006

Note: Specials that go in every contract have already been marked with an "X" for you.

File Name	#		Special Provision Title	Effective	Revised
* 80290	43	X	Payrolls and Payroll Records	Jan. 2, 2012	
80278	44		Planting Woody Plants	Jan. 1, 2012	
80279	45		Portland Cement Concrete	Jan. 1, 2012	
* 80299	46		Portland Cement Concrete Inlay or Overlay	April 1, 2012	
80280	47		Portland Cement Concrete Sidewalk	Jan. 1, 2012	
* 80300	48		Preformed Plastic Pavement Marking Type D - Inlaid	April 1, 2012	
* 80218	49		Preventive Maintenance – Bituminous Surface Treatment	Jan. 1, 2009	April 1, 2012
* 80219	50		Preventive Maintenance – Cape Seal	Jan. 1, 2009	April 1, 2012
* 80220	51		Preventive Maintenance – Micro-Surfacing	Jan. 1, 2009	April 1, 2012
* 80221	52		Preventive Maintenance – Slurry Seal	Jan. 1, 2009	April 1, 2012
80281	53		Quality Control/Quality Assurance of Concrete Mixtures	Jan. 1, 2012	
34261	54		Railroad Protective Liability Insurance	Dec. 1, 1986	Jan. 1, 2006
80157	55		Railroad Protective Liability Insurance (5 and 10)	Jan. 1, 2006	
80172	56		Reclaimed Asphalt Pavement (RAP)	Jan. 1, 2007	Jan. 1, 2012
80282	57		Reclaimed Asphalt Shingles (RAS)	Jan. 1, 2012	
80283	58		Removal and Disposal of Regulated Substances	Jan. 1, 2012	
80224	59		Restoring Bridge Approach Pavements Using High-Density Foam	Jan. 1, 2009	Jan. 1, 2012
80271	60		Safety Edge	April 1, 2011	
* 80152	61		Self-Consolidating Concrete for Cast-In-Place Construction	Nov. 1, 2005	April 1, 2012
* 80132	62		Self-Consolidating Concrete for Precast and Precast Prestressed Products (NOTE: This special provision was previously named "Self-Consolidating Concrete for Precast Products")	July 1, 2004	April 1, 2012
80284	63		Shoulder Rumble Strips	Jan. 1, 2012	
80285	64		Sidewalk, Corner or Crosswalk Closure	Jan. 1, 2012	
80127	65		Steel Cost Adjustment	April 2, 2004	April 1, 2009
80255	66		Stone Matrix Asphalt	Jan. 1, 2010	Jan. 1, 2012
80143	67	X	Subcontractor Mobilization Payments	April 2, 2005	April 1, 2011
80075	68		Surface Testing of Pavements	April 1, 2002	Jan. 1, 2007
80286	69		Temporary Erosion and Sediment Control	Jan. 1, 2012	
80225	70		Temporary Raised Pavement Marker	Jan. 1, 2009	
80256	71		Temporary Water Filled Barrier	Jan. 1, 2010	Jan. 1, 2012
80287	72		Type G Inlet Box	Jan. 1, 2012	
80273	73		Traffic Control Deficiency Deduction	Aug. 1, 2011	
20338	74		Training Special Provisions	Oct. 15, 1975	
80270	75	X	Utility Coordination and Conflicts	April 1, 2011	Jan. 1, 2012
80288	76		Warm Mix Asphalt	Jan. 1, 2012	
80289	77		Wet Reflective Thermoplastic Pavement Marking	Jan. 1, 2012	
80071	78		Working Days	Jan. 1, 2002	

The following special provisions are either in the 2012 Standard Specifications, the 2012 Recurring Special Provisions, or the special provision Portland Cement Concrete:

File Name	Special Provision Title	New Location	Effective	Revised
80186	Alkali-Silica Reaction for Cast-in-Place Concrete	The special provision Portland Cement Concrete	Aug. 1, 2007	Jan. 1, 2009
80213	Alkali-Silica Reaction for Precast and Precast Prestressed Concrete	The special provision Portland Cement Concrete	Jan. 1, 2009	
80207	Approval of Proposed Borrow Areas, Use Areas, and/or Waste Areas	Article 107.22	Nov. 1, 2008	Nov. 1, 2010
80166	Cement	Section 1001	Jan. 1, 2007	April 1, 2011
80260	Certification of Metal Fabricator	Article 106.08	July 1, 2010	
80094	Concrete Admixtures	Section 1021 and the special provision Portland Cement Concrete	Jan. 1, 2003	April 1, 2009

Note: Specials that go in every contract have already been marked with an "X" for you.

<u>File Name</u>	<u>Special Provision Title</u>	<u>New Location</u>	<u>Effective</u>	<u>Revised</u>
80226	Concrete Mix Designs	The special provision	April 1, 2009	
80227	Determination of Thickness	Portland Cement Concrete Articles 353.12, 353.13, 353.14, 354.09, 355.09, 356.07, 407.10, 482.06, and 483.07	April 1, 2009	
80179	Engineer's Field Office Type A	Articles 670.02 and 670.07	April 1, 2007	Jan. 1, 2011
80205	Engineer's Field Office Type B	Articles 670.04 and 670.07	Aug. 1, 2008	Jan. 1, 2011
80189	Equipment Rental Rates	Articles 105.07 and 109.04	Aug. 2, 2007	Jan. 2, 2008
80249	Frames and Grates	Articles 609.02 and 609.04	Jan. 1, 2010	
80194	HMA – Hauling on Partially Completed Full-Depth Pavement	Article 407.08	Jan. 1, 2008	
80245	Hot-Mix Asphalt – Anti-Stripping Additive	Article 1030.04	Nov. 1, 2009	
80250	Hot-Mix Asphalt – Drop-Offs	Article 701.07	Jan. 1, 2010	
80259	Hot Mix Asphalt – Fine Aggregate	Articles 1003.01 and 1003.03	April 1, 2010	
80252	Improved Subgrade	Articles 302.04, 302.07, 302.08, 302.10, 302.11, 310.04, 310.08, 310.10, 310.11, and 311.05	Jan. 1, 2010	
80266	Lane Closure, Multilane, Intermittent or Moving Operation, for Speeds ≤ 40 MPH	Article 701.19	Jan. 1, 2011	Jan. 2, 2011
80230	Liquidated Damages	Article 108.09	April 1, 2009	April 1, 2011
80267	Long-Span Guardrail over Culvert	Articles 630.07 and 630.08	Jan. 1, 2011	
80262	Mulch and Erosion Control Blankets	Articles 251.03, 251.04, 251.06, 251.07, and 1081.06	Nov. 1, 2010	April 1, 2011
80180	National Pollutant Discharge Elimination System / Erosion and Sediment Control Deficiency Deduction	Article 105.03	April 1, 2007	Nov. 1, 2009
80208	Nighttime Work Zone Lighting	Section 702	Nov. 1, 2008	
80232	Pipe Culverts	Articles 542.03, 542.04, 542.11, and 1040.04	April 1, 2009	April 1, 2010
80263	Planting Perennial Plants	Section 254 and Article 1081.02	Jan. 1, 2011	
80210	Portland Cement Concrete Inlay or Overlay	Recurring CS #29	Nov. 1, 2008	
80217	Post Clips for Extruded Aluminum Signs	Article 1090.03	Jan. 1, 2009	
80268	Post Mounting of Signs	Article 701.14	Jan. 1, 2011	
80171	Precast Handling Holes	Articles 540.02, 540.06, 542.02, 542.04, 550.02, 550.06, 602.02, 602.07, and 1042.16	Jan. 1, 2007	
80015	Public Convenience and Safety	Article 107.09	Jan. 1, 2000	
80247	Raised Reflective Pavement Markers	Article 781.03	Nov. 1, 2009	April 1, 2010
80131	Seeding	Articles 250.07 and 1081.04	July 1, 2004	July 1, 2010
80264	Selection of Labor	Recurring CS #5	July 2, 2010	
80234	Storm Sewers	Articles 550.02, 550.03, 550.06, 550.07, 550.08, and 1040.04	April 1, 2009	April 1, 2010
80087	Temporary Erosion Control	Articles 280.02, 280.03, 280.04, 280.07, 280.08, and 1081.15	Nov. 1, 2002	Jan. 1, 2011
80257	Traffic Barrier Terminal, Type 6	Article 631.07	Jan. 1, 2010	
80269	Traffic Control Surveillance	Article 701.10	Jan. 1, 2011	
80258	Truck Mounted/Trailer Mounted Attenuators	Articles 701.03, 701.15, and 1106.02	Jan. 1, 2010	

Note: Specials that go in every contract have already been marked with an "X" for you.

<u>File Name</u>	<u>Special Provision Title</u>	<u>New Location</u>	<u>Effective</u>	<u>Revised</u>
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The following special provisions require additional information from the designer. The additional information needs to be included in a separate document attached to this check sheet. The Project Development and Implementation section will then include the information in the applicable special provision. The Special Provisions are:

- Bridge Demolition Debris
- Building Removal-Case I
- Building Removal-Case II
- Building Removal-Case III
- Building Removal-Case IV
- Completion Date
- Completion Date Plus Working Days
- DBE Participation
- Material Transfer Device
- Railroad Protective Liability Insurance
- Training Special Provisions
- Working Days

Designer Notes
Recurring Special Provisions

Designer Notes for January 1, 2012 Recurring Special Provisions

(Updated for April 27, 2012 letting)

1. Designer Note: This check sheet is required in all contracts that involve Federal funds.
2. Designer Note: This check sheet is required in all Federal contracts.
3. Designer Note: This check sheet is required in all contracts.
4. Designer Note: This check sheet is required in all contracts involving State funds only.
5. Designer Note: This check sheet is required in all contracts involving State funds only.
6. Designer Note: Include in all contracts where Asbestos Bearing Pad Removal is part of the structure work.
7. Designer Note: Include in all contracts where the existing bridge deck HMA surface is to be removed and the waterproofing membrane contains asbestos and will be removed. The designer must have in the project files a completed "Asbestos Determination Certificate" for every bridge within the project limits. The District Bridge Maintenance Engineer and/or the District Hydraulics Engineer can provide copies of these certificates. If your project has any bridge deck containing asbestos, insert this special provision as well as the General Notes entitled "Asbestos Bridge Wearing Surface Removal".
8. Designer Note: This check sheet will be required for those contracts that will involve Contractor work on haul road stream crossings, other temporary stream crossings, and in stream work pads. Contracts that would generally involve this type of work would be bridges/structures, new or rebuilt, and contracts involving earth excavation, embankment or borrow excavation. Discuss these types of work operations and any other stream related work with your Project Engineer. Any in-stream crossing or other work will require an individual 404 permit from the Corps of Engineers. Be sure to let the Hydraulics Engineer (Jim Miller) know as soon as possible that a Corps permit will be needed. The permit has a lead-time and is required for the project to proceed to letting.
9. Designer Note: (See #10 below.) Depending on IDOT manpower, this check sheet will be included as a pay item when the Contractor will be required to do all contract staking, except bridges. A large span culvert measuring more than 6 meters (20 feet) along the survey line will require a structure number be assigned to the structure. This will require that the Designer, if he is calling for Contractor staking, use the check sheet entitled Construction Layout Stakes and not the check sheet entitled Construction Layout Stakes Except for Structures. Discuss with the Bureau of Project Implementation (Construction) as to what manpower sources are available.
10. Designer Note: Depending on IDOT manpower needs, this check sheet will be included as a pay item when the Contractor will be required to do all contract staking, including bridges. This check sheet should be used for a large box culvert or a multi pipe that will require a structure number. This would be a structure that will have a span length along survey line of more than 6 meters (20 feet).

Discuss this check sheet with the Bureau of Project Implementation (Construction) as to what manpower sources are available.

11. Designer Note: This special provision specifies the requirements for geotextile fabric for use on railroad crossings.

Include only on projects where the railroad crossing is a contract pay item. Also may be required for temporary crossings.

Railroad crossings are generally (99%) handled by the Railroad through an agreement and not part of our contract. If in doubt as to how to handle, discuss with Project Support.

12. Designer Note: Use this check sheet where existing pavement is being reconstructed and voids are evident under the existing pavement that can be filled by grouting. Discuss with Maintenance Field Engineer responsible for the area.

NOTE: A detail of the slab movement detection device is included in CADD and this drawing must be included in your contract plans.

13. Designer Note: This check sheet will be required on a contract where cold milling is required but where the cold milled area will not be overlaid. Include CADD Standard 440001 in your plans. If your contract is to be cold milled and the area overlaid, you should use one of the two District special provisions on this subject, not this check sheet.

14. Designer Note: This check sheet requires that once a left of bituminous resurfacing is placed on a lane of pavement, any adjoining bituminous shoulder shall be resurfaced with an equal thickness before any other lane is resurfaced for each left of resurfacing. Insert this special on resurfacing projects which meet the following criteria: All four lane interstates and freeways, all four lane expressways, four lane highways with ADT > 25,000 or peak one-way VPH > 1700, two lane highways with ADT > 10,000 or peak one-way VPH > 800.

15. Designer Note: This check sheet should be used on resurfacing projects to address areas which need repair, but do not warrant full depths repair. Joints and cracks, which exhibit environmental distresses such as spalling and "D" cracking or contain maintenance patching, are eligible for using this method of repair. Joints and cracks which exhibit load related distresses such as pumping, alligator cracking, corner breaks, compression failures, subgrade failures or punch outs should not use this method of repair. Discuss use with your Project Engineer.

16. Designer Note: Intended to remove thick bituminous overlay so that the original pavement can be examined and then patched, if necessary. It also further defines specific pay items for work involved.

17. Designer Note: This check sheet was developed by Materials and Physical Research as an alternate to replacing Preformed Joint Sealer and Neoprene Expansion Joints up to 65 mm (2 ½ inches). Include with any projects that have POLYMER CONCRETE as a pay item.

18. Designer Note: This rehabilitation process can be used in a variety of gravity applications such as trenchless rehabilitation of sanitary sewers, storm sewers, and process piping. Insert this special provision if trenchless repair of the items listed above is selected. Prior to selection consult your Project Engineer. Additional information such as size of pipe to be lined, number of laterals, and manhole treatment may be necessary.
19. Designer Note: This check sheet calls for CA 16 for backfill and wrapping the trench. Discuss usage with Implementation.
20. Designer Note: This check sheet was developed by the Central Bureau of Traffic and should be incorporated into all plans containing guardrail, barrier wall or bridge rail. The designer is required to specify the color of all reflectors to be placed and to provide appropriate traffic control standards for the installation of reflectors/markers. It is the District's option to select the type of reflector marker for use on guardrail and barrier walls, and the type of terminal marker for guardrail. This option should be specified by the pay item used. The District prefers use of the top mounted reflector Type C on barrier walls. Include Highway Standards 635006 and 635011 in the plans if this Check Sheet is used.
21. Designer Note: This check sheet was developed to obtain the desired pipe coating on bike racks. Use on all projects with bike racks.
22. Designer Note: This special provision covers the installation of temporary glare screens on temporary concrete barrier. Glare screens may be needed on temporary concrete barriers separating opposing lanes of traffic, especially on horizontal and vertical curves where oncoming headlight glare could be a problem. Discuss usage with your project engineer.
23. Designer Note: This special provision is for use on bridge contracts where staging is required and the District wants the contractor to have an option to post-mounting the temporary bridge and traffic signals. Discuss use with the District Traffic Control Technician.
24. Designer Note: Intended for use on all freeway/expressway contracts with lane closures as shown on Highway Standard 701400. It may also be used at the District's discretion on high visibility projects and/or projects that will require several months to complete.
25. Designer Note: This check sheet should be included for all projects containing roadway lighting. The designer should also include CADD Standard 701301-D4 in the plans.
26. Designer Note: This check sheet was developed to address difficulties with obtaining metric sized bolts. Include in all metric projects, which contain or could contain any type of bolted connection.
27. Designer Note: This check sheet was developed to address difficulties with obtaining metric sized reinforcement bars. Include in all metric projects containing reinforcement bars.

28. Designer Note: This special provision not to be used in District Four. Not recommended for use on recently constructed pavements or bridge decks. This is not recommended when there is steel in the patches due to the corrosion the calcium chloride causes.
29. Designer Note: Insert into contracts where a PCC inlay or overlay is selected. This method is for locations where excessive rutting has become a problem. Discuss with the Project Engineer, Operations, and Implementation before using. Also, refer to BDE Manual, Chapter 53 before using.
30. Designer Note: Do not use Check Sheet #30 unless requested by Materials.
31. Designer Note: Do not use Check #31. Use BDE special instead. This check sheet has been modified by BDE Special "Quality Control/Quality Assurance of Concrete Mixtures."

BDE Special Provisions

Alphabetic Index

REVISED INDEX
01/20/12

ALPHABETIC LIST OF DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

Get a copy of the current check list from the Program Development Secretary, indicate which ISP's are to be included in your set of special provisions, fill in any blanks as indicated on the check list, and include with your set of special provisions to be sent to Springfield where they will be inserted.

<u>Standard Spec. No.</u>	<u>PC No.</u>	<u>Item</u>
280.02	28002	Above Grade Inlet Protection
888.00	88800	Accessible Pedestrian Signals (APS)
303.00	30300	Aggregate Subgrade Improvement
202.07a	20207a	Agreement to Plan Quantity
701.00	70100	Automated Flagger Assistance Devices
109.01	10901	Bituminous Materials Cost Adjustment
107.38	10738	Bridge Demolition Debris
503.19	50319	Bridge Relief Joint Sealer
107.19a	10719a	Building Removal Case I
107.19b	10719b	Building Removal Case II
107.19c	10719c	Building Removal Case III
107.19d	10719d	Building Removal Case IV
442.00	44200	Calcium Chloride Accelerator for Class PP-2 Concrete
1004.02	100402	Coarse Aggregate in Bridge Approach Slabs/Footings
108.05a	10805a	Completion Date (Via Calendar Days)
108.05b	10805b	Completion Date (Via Calendar Days) Plus working Days
504.00	50400	Concrete Box Culverts with Skews > 30 Degrees and Design Fills ≤ 5 Feet
504.04	50404	Concrete Box Culverts with Skews ≤ 30 Degrees Regardless of Design Fill and Skews >30 Degrees with Design Fills > 5 Feet
503.19	50319	Concrete Joint Sealer
1020.05a	102005a	Concrete Mix Design – Department Provided

REVISED INDEX
01/25/12

ALPHABETIC LIST OF DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

<u>Standard Spec. No.</u>	<u>PC No.</u>	<u>Item</u>
107.00	10700	Construction Air Quality – Diesel Vehicle Emissions Control
107.01	10701	Construction Air Quality – Diesel Retrofit
107.37	10737	Construction air Quality – Idling Restrictions
202.07	20207	Digital Terrain Modeling for Earthwork Calculations
108.06a	10806a	Disadvantaged Business Enterprise Participation
603.02	60302	Drainage and Inlet Protection Under Traffic
100.00	10000	Errata for the 2012 Standard Specifications
701.13	70113	Flagger at Side Roads and Entrances
1004.01	100401	Friction Aggregate
109.03	10903	Fuel Cost Adjustment
643.00	64300	High Tension Cable Median Barrier
406.07	40607	Hot-Mix Asphalt-Density Testing of Longitudinal Joints
702.00c	70200c	Impact Attenuators
702.00d	70200d	Impact Attenuators, Temporary
406.00f	40600f	Material Transfer Device
503.02	50302	Metal Hardware Cast into Concrete
780.01	78001	Modified Urethane Pavement Marking
1008.27	100827	Moisture Cured Urethane Paint System
1106.02i	110602i	Movable Traffic Barrier
783.03	78303	Pavement Marking Removal
703.02	70302	Pavement Marking Tape Type IV
701.17	70117	Pavement Patching
109.07	10907	Payments to Subcontractors

REVISED INDEX
01/20/12

ALPHABETIC LIST OF DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

Standard Spec. No.	PC No.	Item
108.00	10800	Payrolls and Payroll Records
253.00	25300	Planting Woody Plants
312.26	31226	Portland Cement Concrete
420.00	42000	Portland Cement Concrete Inlay or Overlay
420.07	42007	Portland Cement Concrete Sidewalk
780.00	78000	Preformed Plastic Pavement Marking Type D - Inlaid
400.04	40004	Preventive Maintenance - Bituminous Surface Treatment
400.01	40001	Preventive Maintenance – Cape Seal
400.02	40002	Preventive Maintenance – Micro-Surfacing
400.03	40003	Preventive Maintenance – Slurry Seal
1020.16	102016	Quality Control/Quality Assurance of Concrete Mixtures
107.11	10711a	Railroad Protective Liability Insurance
107.11	10711b	Railroad Protective Liability Insurance (5 and 10)
1031.00	103100	Reclaimed Asphalt Pavement (RAP)
1031.01	103101	Reclaimed Asphalt Shingles (RAS)
669.01	66901	Removal and Disposal of Regulated Substances
420.16	42016	Restoring Bridge Approach Pavements Using High-Density Foam
406.05	40605	Safety Edge
1020.01	102001	Self-Consolidating Concrete for Cast-in-Place Construction
1020.00	102000	Self-Consolidating Concrete for Precast and Precast Prestressed Products
642.05	64205	Shoulder Rumble Strips
701.15	70115	Sidewalk, Corner or Crosswalk Closure

REVISED INDEX
01/20/12

ALPHABETIC LIST OF DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

Standard Spec. No.	PC No.	Item
109.00	10900a	Steel Cost Adjustment
406.06	40606	Stone Matrix Asphalt
671.00	67100	Subcontractor Mobilization Payments
406.21	40621	Surface Testing of Pavements
280.04	28004	Temporary Erosion and Sediment Control
703.00	70300	Temporary Raised Pavement Marker
1106.02k	110602k	Temporary Water Filled Barrier
105.04	10504	Traffic Control Deficiency Deduction
108.06	10806	Training Special Provision
610.09	61009	Type G Inlet Box
105.07	10507	Utility Coordination and Conflicts
406.00	40600	Warm Mix Asphalt
780.00	78000	Wet Reflective Thermoplastic Pavement Marking
108.05	10805	Working Days

BDE Special Provisions

Numeric Index

REVISED INDEX

NUMERIC DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

Get a copy of the current check list from the Program Development Secretary, indicate which ISP's are to be included in your set of special provisions, fill in any blanks as indicated on the check list, and include with your set of special provisions to be sent to Springfield where they will be inserted.

<u>Standard Spec. No.</u>	<u>PC No.</u>	<u>Item</u>
100.00	10000	Errata for the 2012 Standard Specifications
105.04	10504	Traffic Control Deficiency Deduction
105.07	10507	Utility Coordination and Conflicts
107.00	10700	Construction Air Quality – Diesel Vehicle Emissions Control
107.01	10701	Construction Air Quality – Diesel Retrofit
107.11a	10711a	Railroad Protective Liability Insurance
107.11b	10711b	Railroad Protective Liability Insurance (5 and 10)
107.19a	10719a	Building Removal Case I
107.19b	10719b	Building Removal Case II
107.19c	10719c	Building Removal Case III
107.19d	10719d	Building Removal Case IV
107.37	10737	Construction Air Quality – Idling Restrictions
107.38	10738	Bridge Demolition Debris
108.00	10800	Payrolls and Payroll Records
108.05	10805	Working Days
108.05a	10805a	Completion Date (Via Calendar Days)
108.05b	10805b	Completion Date (Via Calendar Days) Plus Working Days
108.06	10806	Training Special Provision
108.06a	10806a	Disadvantaged Business Enterprise Participation
109.00a	10900a	Steel Cost Adjustment
109.01	10901	Bituminous Materials Cost Adjustments

NUMERIC DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

Standard Spec. No.	PC No.	Item
109.03	10903	Fuel Cost Adjustment
109.07	10907	Payments to Subcontractors
202.07	20207	Digital Terrain Modeling for Earthwork Calculations
202.07a	20207a	Agreement to Plan Quantity
253.00	25300	Planting Woody Plants
280.02	28002	Above Grade Inlet Protection
280.04	28004	Temporary Erosion and Sediment Control
303.00	30300	Aggregate Subgrade Improvement
312.26	31226	Portland Cement Concrete
400.01	40001	Preventive Maintenance – Cape Seal
400.02	40002	Preventive Maintenance – Micro-Surfacing
400.03	40003	Preventive Maintenance – Slurry Seal
400.04	40004	Preventive Maintenance – Bituminous Surface Treatment
406.00	40600	Warm Mix Asphalt
406.00f	40600f	Material Transfer Device
406.05	40605	Safety Edge
406.06	40606	Stone Matrix Asphalt
406.07	40607	Hot-Mix Asphalt – Density Testing of Longitudinal Joints
406.21	40621	Surface Testing of Interstate Pavements
420.00	42000	Portland Cement Concrete Inlay or Overlay
420.07	42007	Portland Cement Concrete Sidewalk
420.16	42016	Restoring Bridge Approach Pavements Using High-Density Foam
442.00	44200	Calcium Chloride Accelerator for Class PP-2 Concrete
503.02	50302	Metal Hardware Cast Into Concrete

NUMERIC DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

Standard Spec. No.	PC No.	Item
503.19	50319	Bridge Relief Joint Sealer
504.00	50400	Concrete Box Culverts with Skews > 30 Degrees and Design Fills ≤ 5 Feet
504.04	50404	Concrete Box Culverts with Skews ≤ 30 Degrees Regardless of Design Fill and Skews >30 Degrees with Design Fills > 5 Feet
603.02	60302	Drainage and Inlet Protection Under Traffic
610.09	61009	Type G Inlet Box
642.05	64205	Shoulder Rumble Strips
643.00	64300	High Tension Cable Median Barrier
669.01	69901	Removal and Disposal of Regulated Substances
671.00	67100	Subcontractor Mobilization Payments
701.00	70100	Automated Flagger Assistance Devices
701.13	70113	Flagger at Side Roads and Entrances
701.15	70115	Sidewalk, Corner or Crosswalk Closure
701.17	70117	Pavement Patching
702.00c	70200c	Impact Attenuators
702.00d	70200d	Impact Attenuators, Temporary
703.00	70300	Temporary Raised Pavement Marker
703.02	70302	Pavement Marking Tape Type IV
780.01	78001	Modified Urethane pavement Marking
780.02	78002	Preformed Plastic Pavement Marking Type D - Inlaid
781.00	78100	Wet Reflective Thermoplastic Pavement Marking
783.03	78303	Pavement Marking Removal
888.00	88800	Accessible Pedestrian Signals (APS)
1004.01	100401	Friction Aggregate
1004.02	100402	Coarse Aggregate in Bridge Approach Slabs/Footings

NUMERIC DESIGN INTERIM SPECIAL PROVISIONS (ISP's)

Standard Spec. No.	PC No.	Item
1008.27	100827	Moisture Cured Urethane Paint System
1020.00	102000	Self-Consolidating Concrete for Precast and Precast Prestressed Products
1020.01	102001	Self-Consolidating Concrete for Cast-in-Place Construction
1020.05a	102005a	Concrete Mix Design – Department Provided
1020.16	102016	Quality Control/Quality Assurance of Concrete Mixtures
1031.00	103100	Reclaimed Asphalt Pavement (RAP)
1031.01	103101	Reclaimed Asphalt Shingles (RAS)
1106.02i	110602i	Movable Traffic Barrier
1106.02k	110602k	Temporary Water Filled Barrier

BDE Special Provisions

10000

100.00

Designer Note: Insert into all contracts.

ERRATA FOR THE 2012 STANDARD SPECIFICATIONS (BDE)

Effective: April 1, 2012

- Page 337 Article 505.04. Revise the subparagraph "(i) Match Making." to read "(i) Match Marking."
- Page 360 Article 506.07. In the first line of the second paragraph change "AASHTO/AWS D1.5/D1.5:" to "AASHTO/AWS D1.5M/D1.5:".
- Page 361 Article 506.08. In the third line of the sixth paragraph change "506.08(a)" to "506.08(b)".
- Page 531 Article 609.07. In the first paragraph delete "TYPE B, C, or D INLET BOX STANDARD 609001 or".
- Page 609 Article 703.05. In the first line of the second paragraph delete "or Type II".
- Page 989 Article 1083.02(a). In the seventh line of the first paragraph change "Table 14.7.5.2-2" to "Table 14.7.5.2-1".

Designer Notes: Insert into all contracts.

CONSTRUCTION AIR QUALITY - DIESEL VEHICLE EMISSIONS CONTROL (BDE)

Effective: April 1, 2009

Revised: January 2, 2012

Diesel Vehicle Emissions Control. The reduction of construction air emissions shall be accomplished by using cleaner burning diesel fuel. The term "equipment" refers to any and all diesel fuel powered devices rated at 50 hp and above, to be used on the project site in excess of seven calendar days over the course of the construction period on the project site (including any "rental" equipment).

All equipment on the jobsite, with engine ratings of 50 hp and above, shall be required to: use Ultra Low Sulfur Diesel fuel (ULSD) exclusively (15 ppm sulfur content or less).

Diesel powered equipment in non-compliance will not be allowed to be used on the project site, and is also subject to a notice of non-compliance as outlined below.

The Contractor shall certify that only ULSD will be used in all jobsite equipment. The certification shall be presented to the Department prior to the commencement of the work.

If any diesel powered equipment is found to be in non-compliance with any portion of this specification, the Engineer will issue the Contractor a notice of non-compliance and identify an appropriate period of time, as outlined below under environmental deficiency deduction, in which to bring the equipment into compliance or remove it from the project site.

Any costs associated with bringing any diesel powered equipment into compliance with these diesel vehicle emissions controls shall be considered as included in the contract unit prices bid for the various items of work involved and no additional compensation will be allowed. The Contractor's compliance with this notice and any associated regulations shall also not be grounds for a claim.

Environmental Deficiency Deduction. When the Engineer is notified, or determines that an environmental control deficiency exists, he/she will notify the Contractor in writing, and direct the Contractor to correct the deficiency within a specified time period. The specified time-period, which begins upon Contractor notification, will be from 1/2 hour to 24 hours long, based on the urgency of the situation and the nature of the deficiency. The Engineer shall be the sole judge regarding the time period.

The deficiency will be based on lack of repair, maintenance and diesel vehicle emissions control.

If the Contractor fails to correct the deficiency within the specified time frame, a daily monetary deduction will be imposed for each calendar day or fraction thereof the deficiency continues to exist. The calendar day(s) will begin when the time period for correction is exceeded and end with the Engineer's written acceptance of the correction. The daily monetary deduction will be \$1,000.00 for each deficiency identified.

If a Contractor or subcontractor accumulates three environmental deficiency deductions in a contract period, the Contractor will be shutdown until the deficiency is corrected. Such a shutdown will not be grounds for any extension of contract time, waiver of penalties, or be grounds for any claim.

Designer Note: Insert into all contracts. Recurring Check Sheet #5 should also be marked.

PAYROLLS AND PAYROLL RECORDS (BDE)

Effective: January 2, 2012

Revise Section IV of Check Sheet #5 of the Recurring Special Provisions to read:

“IV.COMPLIANCE WITH THE PREVAILING WAGE ACT

1. **Prevailing Wages.** All wages paid by the Contractor and each subcontractor shall be in compliance with The Prevailing Wage Act (820 ILCS 130), as amended, except where a prevailing wage violates a federal law, order, or ruling, the rate conforming to the federal law, order, or ruling shall govern. The Contractor shall be responsible to notify each subcontractor of the wage rates set forth in this contract and any revisions thereto. If the Department of Labor revises the wage rates, the Contractor will not be allowed additional compensation on account of said revisions.
2. **Payroll Records.** The Contractor and each subcontractor shall make and keep, for a period of three years from the later of the date of final payment under the contract or completion of the contract, records of the wages paid to his/her workers. The payroll records shall include each worker's name, address, telephone number, social security number, classification, rate of pay, number of hours worked each day, starting and ending times of work each day, total hours worked each week, itemized deductions made, and actual wages paid. Upon seven business days' notice, these records shall be available at a location within the State, during reasonable hours, for inspection by the Department; the Department of Labor; and Federal, State or local law enforcement agencies and prosecutors.
3. **Submission of Payroll Records.** The Contractor and each subcontractor shall submit payroll records to the Engineer each week from the start to the completion of their respective work, except that full social security numbers and home addresses shall not be included on weekly transmittals. Instead the payrolls shall include an identification number for each employee (e.g., the last four digits of the employee's social security number). In addition, starting and ending times of work each day may be omitted from the payroll records submitted to the Engineer. The submittals shall be on the Department's form SBE 48, or an approved facsimile. When there has been no activity during a work week, a payroll record shall still be submitted with the appropriate box (“No Work”, “Suspended”, or “Complete”) checked on the form.

Each submittal shall be accompanied by a statement signed by the Contractor or subcontractor, or an officer, employee or officer thereof, which avers that: (i) he or she has examined the records and such records are true and accurate; (ii) the hourly rate paid to each worker is not less than the general prevailing rate of hourly wages required by the Act; and (iii) the Contractor or subcontractor is aware that filing a payroll record that he/she knows to be false is a Class A misdemeanor.

4. Employee Interviews. The Contractor and each subcontractor shall permit his/her employees to be interviewed on the job, during working hours, by compliance investigators of the Department or the Department of Labor.”

Designer Note: Insert into all projects utilizing aggregate subgrade improvements. When using also include BDE special "Reclaimed Asphalt Pavement (RAP)." Check with district Soils Engineer to determine thickness.

AGGREGATE SUBGRADE IMPROVEMENT (BDE)

Effective: April 1, 2012

Add the following Section to the Standard Specifications:

"SECTION 303. AGGREGATE SUBGRADE IMPROVEMENT

303.01 Description. This work shall consist of constructing an aggregate subgrade improvement.

303.02 Materials. Materials shall be according to the following.

Item	Article/Section
(a) Coarse Aggregate	1004.06
(b) Reclaimed Asphalt Pavement (RAP) (Notes 1, 2, and 3)	1031

Note 1. RAP shall not contain more than 10 percent steel slag or any materials considered expansive by the Department.

Note 2. Crushed RAP, from either full depth or single lift removal, may be mechanically blended with aggregate gradations CS 01, CS 02, and RR 01 but shall not exceed 40 percent of the total product. The top size of the RAP shall be less than 4 in. (100 mm) and well graded.

Note 3. RAP having 100 percent passing the 1 1/2 in. (37.5 mm) sieve and being well graded, may be used as capping aggregate in the top 3 in. (75 mm) when aggregate gradations CS 01, CS 02, or RR 01 are used in lower lifts.

303.03 Equipment. The vibratory machine shall be according to Article 1101.01, or as approved by the Engineer.

303.04 Soil Preparation. The stability of the soil shall be according to the Department's Subgrade Stability Manual for the aggregate thickness specified.

303.05 Placing Aggregate. The maximum nominal lift thickness of aggregate gradations CA 02, CA 06, or CA 10 shall be 12 in. (300 mm). The maximum nominal lift thickness of aggregate gradations CS 01, CS 02, and RR 01 shall be 24 in. (600 mm).

303.06 Capping Aggregate. The top surface of the aggregate subgrade shall consist of a minimum 3 in. (75 mm) of aggregate gradations CA 06 or CA 10. When the contract specifies that a granular subbase is to be placed on the aggregate subgrade improvement, the 3 in. (75 mm) of capping aggregate shall be the same gradation and may be placed with the underlying aggregate subgrade improvement material.

303.07 Compaction. All aggregate lifts shall be compacted to the satisfaction of the Engineer. If the moisture content of the material is such that compaction cannot be obtained, sufficient water shall be added so that satisfactory compaction can be obtained.

303.08 Finishing and Maintenance of Aggregate Subgrade Improvement. The aggregate subgrade improvement shall be finished to the lines, grades, and cross sections shown on the plans, or as directed by the Engineer. The aggregate subgrade improvement shall be maintained in a smooth and compacted condition.

303.09 Method of Measurement. This work will be measured for payment according to Article 311.08.

303.10 Basis of Payment. This work will be paid for at the contract unit price per square yard (square meter), cubic yard (cubic meter), or ton (metric ton) for AGGREGATE SUBGRADE IMPROVEMENT, of the thickness specified."

Add the following to Section 1004 of the Standard Specifications:

"1004.06 Coarse Aggregate for Aggregate Subgrade Improvement. The aggregate shall be according to Article 1004.01 and the following.

- (a) Description. The coarse aggregate shall be crushed gravel, crushed stone, or crushed concrete.
- (b) Quality. The coarse aggregate shall consist of sound durable particles reasonably free of deleterious materials.
- (c) Gradation.
 - (1) The coarse aggregate gradation for total subgrade thickness less than or equal to 12 in. (300 mm) shall be CA 2, CA 6, CA 10, or CS 01.

The coarse aggregate gradation for total subgrade thickness more than 12 in. (300 mm) shall be CS 01, CS 02 or RR 01(see Article 1005.01(c)).

COARSE AGGREGATE SUBGRADE GRADATIONS						
Grad No.	Sieve Size and Percent Passing					
	8"	6"	4"	2"	#4	#200
CS 01	100	97 ± 3	90 ± 10	45 ± 25	20 ± 20	5 ± 5
CS 02		100	80 ± 10	25 ± 15		

COARSE AGGREGATE SUBGRADE GRADATIONS (Metric)						
Grad No.	Sieve Size and Percent Passing					
	200 mm	150 mm	100 mm	50 mm	4.75 mm	75 µm
CS 01	100	97 ± 3	90 ± 10	45 ± 25	20 ± 20	5 ± 5
CS 02		100	80 ± 10	25 ± 15		

- (2) The 3 in. (75 mm) capping aggregate shall be gradation CA 6 or CA 10."

Designer Note: Insert into all contracts using cape seal. Use of this special provision shall be according to the Bureau of Design and Environment Manual, Chapter 52.

Districts are encouraged to use the CA 20 gradation as it has proven to perform well for A-1 surface treatments.

The designer must specify the aggregate gradation for the A-1 bituminous surface treatment. Districts are encouraged to use the CA 20 gradation as it has proven to perform well for A-1 surface treatments.

The designer must specify the proper friction aggregate for the micro-surfacing layer on the plans using the following note:

“The aggregates for the micro-surfacing shall meet the friction aggregate requirements for Mixture __ in Article 1004.03(a).”

Insert either “C” or “D” into the note to indicate which mixture is to be used according to the ADT volume on the project. ADT ≤ 5,000 shall use Mixture C and ADT > 5,000 shall use Mixture D.

Include the following information in the Traffic Control Plan Special Provision:

- Contractor shall post the roadway with “LOOSE GRAVEL” and SPEED LIMIT 35” signs in accordance with applicable articles of Division 700 of the Standard Specifications. These signs shall be placed at the start of the work, near intersecting roadways and then at an average spacing of 0.5 mi (0.8 km). The signs may be removed as soon as the sweeping operation has been completed.

Include Special Provision on Temporary Flexible Raised Pavement Marker with this work.

PREVENTIVE MAINTENANCE - CAPE SEAL (BDE)

Effective: January 1, 2009
Revised: April 1, 2012

Description. This work shall consist of constructing a single bituminous surface treatment (A-1) and a micro-surfacing on existing hot-mix asphalt (HMA) surfaces.

Materials. Materials shall be according to the following.

- (a) A-1 Surface Treatment. Materials shall be according to the following Articles/Sections of the Standard Specifications.

Item	Article/Section
(1) Seal Coat Aggregate (Note 1)	1003, 1004.03
(2) Bituminous Materials (Note 2)	1032

Note 1. The seal coat aggregate shall be either fine or coarse aggregate.

When fine aggregate is used, it shall be stone sand, wet bottom boiler slag, slag sand, or steel slag sand. The aggregate quality shall be Class C. The aggregate gradation shall be FA 1 (Special), FA 4 (Special), or FA 22 as specified on the plans and shall meet the following.

FINE AGGREGATE GRADATIONS						
Grad. No.	Sieve Size and Percent Passing					
	3/8 in. (9.5 mm)	No. 4 (4.75 mm)	No. 8 (2.36 mm)	No. 16 (1.18 mm)	No. 40 (425 µm)	No. 200 (75 µm)
FA 1 (Special)	100	90 ± 10	62.5 ± 17.5	32.5 ± 7.5	7.5 ± 7.5	1.5 ± 1
FA 4 (Special)	100	--	--	2 ± 2	--	1.5 ± 1
FA 22	100	*	*	8 ± 8	--	2 ± 2

* For the fine aggregate gradation FA 22, the aggregate producer shall set the midpoint percent passing, and the Department will apply a range of ± 10 percent. The midpoint shall not be changed without Department approval.

When coarse aggregate is used, it shall be crushed gravel, crushed stone, wet bottom boiler slag, crushed slag, crushed sandstone, or crushed steel slag. The aggregate quality shall be Class C and the deleterious chert count shall be no more than 25.0 percent by weight (mass) as determined by the Illinois Test Procedure 203. The aggregate gradation shall be CA 15, CA 16, or CA 20 as specified on the plans.

Note 2. The bituminous material shall be either a CRSP or an HFP polymer modified emulsified asphalt meeting the requirements of Article 1032.06(f)(2) of the Standard Specifications.

- (b) Micro-Surfacing. Materials shall be according to the following Articles/Sections of the Standard Specifications.

Item	Article/Section
(1) Mineral Filler (Note 1)	1001
(2) Water	1002
(3) Coarse Aggregate (Note 2)	1004.03
(4) Latex-Modified Emulsified Asphalt (Note 3)	
(5) Additives (Note 4)	

Note 1. The mineral filler shall be Type 1 portland cement.

Note 2. The coarse aggregate material shall be selected from the table in Article 1004.03(a) of the Standard Specifications based upon the friction aggregate mixture specified. The quality of the aggregate shall be Class B and the gradation shall be as shown in the table below.

Sieve Size	% Passing
3/8 in. (9.5 mm)	100
No. 4 (4.75 mm)	95 ± 5
No. 8 (2.36 mm)	77 ± 13
No. 16 (1.18 mm)	57 ± 13
No. 30 (600 µm)	35 ± 10
No. 50 (330 µm)	19 ± 6
No. 100 (150 µm)	15 ± 6
No. 200 (75 µm)	10 ± 5

To assure the material is totally crushed, 100 percent of the parent aggregate shall be larger than the largest stone in the gradation to be used.

The blending, alternate use, and/or substitutions of aggregates from different sources for use in this work will not be permitted without the approval of the Engineer. Any blending shall be by interlocked mechanical feeders. The blending shall be uniform, compatible with the other components of the mix, and the equipment shall be approved by the Engineer.

If blending aggregates, the blend shall have a washed gradation performed every other day or a minimum of three tests per week. Testing shall be completed before the aggregate receives final acceptance for use in the mix. All gradation tests shall be conducted according to the aggregate gradation control system (AGCS).

Aggregates shall be screened at the stockpile prior to delivery to the paving machine to remove oversized material or contaminants.

Note 3. CSS-1h Latex Modified Emulsified Asphalt. The emulsified asphalt shall be a quick-traffic latex modified asphalt emulsion containing a minimum of 3.0 percent latex solids by weight of asphalt binder. The latex shall be milled or blended into the emulsifier solution prior to the emulsification process. The CSS-1h latex modified emulsified asphalt shall be according to the following.

Test (AASHTO T 59)	Result
Viscosity, Saybolt Furol, 77 °F (25 °C), SFS	20-100
Storage Stability Test, 24 hours, %	1 max.
Particle Charge Test	Positive
Sieve Test, No. 20 (850 µm), retained on sieve, %	0.10 max.
Distillation Test, Residue from distillation test to 347 ± 9 °F (175 ± 5 °C), %	62 min.

Tests on residue from distillation	Result
Penetration, 77 °F (25 °C), 100 grams, 5 seconds, (AASHTO T 49), dmm	40-90
Ductility, 77 °F (25 °C), 50 mm/min, (AASHTO T 51), mm	400 min.
Solubility in trichloroethylene, (AASHTO T 44), %	97.5 min.
Softening Point, (AASHTO T 53), °F (°C)	135 (57) min.
Absolute Viscosity, 140 °F (60 °C), (AASHTO T 202), Poises (Pa · sec)	8,000 (800) min.

Note 4. Additives may be added to the emulsion mix or any of the component materials to provide the control of the quick-traffic properties. They shall be included as part of the mix design and be compatible with the other components of the mix.

(c) Crack/Joint Sealant. The crack/joint sealant shall be a fiber-modified asphalt binder mixed at the jobsite or premixed.

(1) Jobsite-Mixed Sealant. The sealant shall consist of an asphalt binder and fibers, and be according to the following.

a. Asphalt Binder. The asphalt binder shall be PG 58-28, PG 58-22, or PG 64-22 and meet the requirements of Article 1032.05 of the Standard Specifications.

- b. **Fibers.** Fibers shall be short cut polypropylene fibers meeting the properties listed below. The fiber may be accepted on certification from the manufacturer that it meets the specified requirements.

Property	Value
Length, in. (mm)	0.3 - 0.5 (8 - 12)
Denier	13-16
Crimps	None
Tensile Strength, min., psi (kPa)	40,000 (275,000)
Specific Gravity (typical)	0.91
Moisture Regain @ 70 °F (21 °C) and 65% RH (typical), %	0.1

- c. **Percent Fibers.** The sealant shall contain a minimum of 8.0 percent of fibers by weight (mass).
- d. **Sealant Heating.** The sealant shall be heated in the kettle at temperatures between 255 and 285 °F (124 and 141 °C).
- (2) **Premixed Sealant.** The sealant shall be packaged and consist of an asphalt binder, fibers, and other modifiers meeting the following requirements. The sealant and its components may be accepted on certification from the manufacturer that it meets the specified requirements.

- a. **Asphalt Binder.** The asphalt binder shall be PG 64-22 and meet the requirements of Article 1032.05 of the Standard Specifications.
- b. **Fibers.** Fibers shall be short cut polyester fibers meeting the following.

Property	Value
Length, in. (mm)	0.25 ± 0.02 (6.3 ± 0.5)
Denier	3 - 6
Crimps	None
Tensile Strength, min., psi (kPa)	70,000 (482,000)
Specific Gravity (typical)	1.32 - 1.40
Elongation at Break, %	35 - 38
Melt Temperature, °F (°C)	475 - 490 (246 - 254)

- c. **Percent Fibers.** The sealant shall contain 5.0 ± 0.5 percent of fibers by weight (mass).

The sealant, in its final form, shall meet the following requirements when sampled and heated to the manufacturer's recommended maximum heating temperature according to ASTM D 5167.

Test	Value
Cone Penetration @ 77 °F (25 °C), ASTM D 5329	10 - 35 dmm
Softening Point, ASTM D 36	175 °F (79 °C) min.
Maximum Heating Temperature	400°F (204 °C)
Application Temperature	350°F (177 °C) min.

Equipment. Equipment shall be according to the following.

- (a) A-1 Surface Treatment. Equipment shall be according to the following Articles/Sections of the Standard Specifications.

Item	Article/Section
(1) Self-Propelled Pneumatic-Tired Roller (Note 1)	1101.01
(2) Mechanical Sweeper (Note 2)	1101.03
(3) Aggregate Spreaders (Note 3)	1102.04
(4) Pressure Distributor (Note 4)	1102.05
(5) Heating Equipment	1102.07

Note 1. There shall be a minimum of two rollers, with the final number of rollers determined by the rollers' abilities to maintain proper spacing with the aggregate spreader as directed by the Engineer.

Note 2. The mechanical sweeper shall be power driven and self-propelled with the broom located between the axles. The mechanical sweeper shall not use a cantilever-mounted broom and the broom rotation shall not be operated by forward movement.

Note 3. The aggregate spreader shall be a self-propelled mechanical type with the receiving hopper in the rear and shall pull the aggregate truck. The spreader shall be fitted with an automated system which provides positive interconnected control of the aggregate flow with the forward speed of the spreader. The automated system shall provide uniform and consistent aggregate application at the rate specified.

The Engineer will check the spread roll of the aggregate spreader for straightness each day before operations begin. Should the surface of the spread roll vary off a straight line along its longitudinal dimension by more than 1/16 in. (1.5 mm), the Engineer will inspect the application of aggregate for corrugations and, should these occur, the machine shall be repaired or replaced. The forward speed of the spreader during calibration shall be the same as is to be used during construction. The equipment required for aggregate spreader calibration may consist of several sheets of canvas, each being exactly 1 sq yd (0.8 sq m), and a weight scale. By making several runs at different gate openings over the sheets of canvas, placed to cover the full width applied by the spreader, and carefully measuring the aggregate on each canvas sheet, the gate opening at the pre-established speed required to apply aggregate at the specified rate may be determined.

Note 4. The pressure distributor shall have a minimum capacity of 3000 gal (11,500 L). The application rate control shall be automated and shall control the application rate regardless of ground speed or spray bar width. The computer shall have the capability of recording the application rate, gallons sprayed, square yards, and feet traveled. The pressure distributor shall be capable of maintaining the asphalt emulsion at the specified temperature. The spray bar nozzles shall produce a uniform triple lap application fan spray, and the shutoff shall be instantaneous, with no dripping. The pressure distributor shall be capable of maintaining the specified application rate within ± 0.015 gal/sq yd (± 0.070 L/sq m) for each load. The spray-bar nozzles shall be turned to make the same angle with the longitudinal axis of the spray bar as recommended by the manufacturer.

Application rates shall be determined by the procedures listed in ASTM D 2995, except the sample may be taken on three 8 x 12 in. (200 x 300 mm) metal plates. The three plates shall be positioned as directed by the Engineer.

(b) Micro-Surfacing. Equipment shall be according to the following.

- (1) Micro-Surfacing Mixing Machine. The machine shall be either a continuous (self-loading) machine or a non-continuous (self-contained) machine depending on the size of the project as described below. Both types of machines shall have sufficient storage capacity for aggregate, emulsified asphalt, mineral filler, control additive and water to maintain an adequate supply to the proportioning controls. The mixing unit shall be able to accurately deliver and proportion the aggregate, emulsified asphalt, mineral filler, control setting additive, and water to a revolving multi-blade, double-shafted mixer.

Machines that are the continuous (self-loading) type shall be an automatic-sequenced, self-propelled, continuous-flow mixing unit able to discharge the mixed product on a continuous-flow basis. The machine shall be equipped to allow the operator to have full control of the forward and reverse speeds during applications of the material and be equipped with opposite-side driver stations to assist in alignment.

Non-continuous (self-contained) machines will be allowed on projects with a length of 2 lane-miles (3.2 lane-km) or less. For mainline paving, the Contractor shall have at least three self-contained machines in continuous operation to ensure appropriate production rates. Self-contained machines will also be allowed on shoulders, ramps, short applications as bridge decks, or where the material can be placed in a single loading capacity of the machine.

Each mixing unit to be used in the performance of the work shall be calibrated in the presence of the Engineer prior to construction. Each new or different aggregate requires a new calibration. Previous calibration documentation covering the exact materials to be used may be acceptable, provided that no more than 30 days have lapsed. The documentation shall include an individual calibration of each material at various settings, which can be related to the machine metering devices. Prior to the calibration process, portable scales used to calibrate the mixing machine for emulsion and aggregate shall be checked with 25 lb and 50 lb weights, respectively. Results from the standard weight checks shall be furnished to the Engineer. No machine will be allowed to work on the project until the calibration has been completed and/or accepted.

- (2) Micro-Surfacing Spreader. The mixture shall be agitated and spread uniformly in the surfacing box by means of twin-shafted paddles or spiral augers fixed in the spreader box. A front seal shall be provided to insure no loss of the mixture at the road contact point. The rear seal shall act as a final strike-off and shall be adjustable. The spreader box and rear strike-off shall be so designed and operated that a uniform consistency is achieved to produce a free flow of material to the rear strike-off. The spreader box shall have suitable means provided to side shift the box to compensate for variations in the pavement geometry.

A secondary strike-off shall be provided to improve surface texture on the surface course. The secondary strike-off shall have the same adjustments as the spreader box and shall not bounce, wobble, or chatter.

When required on the plans, before the final surface course is placed, preliminary micro-surfacing material may be required to fill ruts, utility cuts, depressions in the existing surface, etc. Ruts of 1/2 in. (13 mm) or greater in depth shall be filled independently with a rut-filling spreader box, either 5 or 6 ft (1.5 or 1.8 m) in width. For irregular or shallow rutting of less than 1/2 in. (13 mm) in depth, a full-width scratch-coat pass may be used as directed by the Engineer utilizing either a stiff

primary rubber or else a metal primary strike off. Ruts that are in excess of 1 1/2 in. (38 mm) in depth may require multiple placements with the rut-filling spreader box to restore the cross-section. All rut-filling level-up material should cure under traffic for a minimum of 24 hours before additional material is placed on top of the level up.

- (3) Micro-Surfacing Proportioning Devices. Individual volume or weight controls for proportioning each material to be added to the mix (i.e. aggregate, mineral filler, emulsified asphalt, additive, and water) shall be provided and properly marked. These proportioning devices are used in material calibration and determining the material output at any time. Calibration records, conversion formulas, and daily run sheets including the beginning and final numbers shown on the proportioning devices shall be submitted to the Engineer for approval. During production any deviations from the original JMF shall be approved by the Engineer.

(c) Crack/Joint Sealing. Equipment shall be according to the following.

- (1) Air Compressor. The air compressor shall be capable of producing a minimum pressure of 90 psi (620 kPa) at the end of the discharge hose. The air stream shall discharge onto the pavement through an appropriate air lance. The tool lubricator shall be bypassed and a filter installed on the discharge valve to keep water and oil out of the line.
- (2) Oil Kettle. The crack sealant shall be heated in an oil jacketed double wall kettle equipped with an agitator (reversing rotary auger action) and separate thermometers for the oil bath and mixing chamber. The unit shall also be equipped with a reversible hydraulic 2 in. (50 mm) hot asphalt pump and a recirculating pump to circulate the oil bath.

CONSTRUCTION REQUIREMENTS

Weather Limitations. Placement of the A-1 bituminous surface treatment shall be done between May 1 and August 31, with the micro-surfacing being placed according to the timeframe specified herein. Bituminous materials shall be applied only when the temperature of the air in the shade is above 55 °F (13 °C). No work shall be started if local conditions indicate that rain is imminent.

The A-1 bituminous surface treatment may be done between September 1 and September 15 provided both of the following conditions are met:

- (a) The temperature of the air in the shade is above 70 °F (20 °C) and the temperature of the surface to which the asphalt will be applied is 70 °F (20 °C) or above, and
- (b) The National Weather Service forecast for the area does not show any rain or any temperatures below 55 °F (13 °C) for the day the work is to be done or for the following five days.

Mix Design. A Contractor provided laboratory shall develop the mix design for the micro-surfacing mixture, shall verify the functioning of the set regulating additives, and shall present certified test results for the Engineer's approval. This laboratory shall be recognized by the International Slurry Surfacing Association (ISSA) as being capable of performing mix designs. The Engineer will verify the laboratory tests required in ISSA A143 have been conducted.

Proportions for the mix design shall be within the following limits.

Mineral Aggregate, dry weight (mass) lb/sq yd (kg/sq m)	15-50 (8-30)
Latex Emulsified Asphalt Residue, % by wt. of Aggregate	5.5-10.5
Latex Base Modifier	As required with % by weight (mass) of binder min. of 3.0
Mix Set Additive	As required
Mineral Filler, % by weight (mass) of aggregate	0.25 - 3 depending on weather conditions

The amount of mineral filler needed shall be determined by the laboratory mix design and will be considered as part of the aggregate gradation.

The amount and type of latex shall be determined by the laboratory performing the mix design. The minimum amount required shall be based on asphalt weight content and shall be certified by the emulsion supplier.

Compatibility of the aggregate, latex-modified emulsified asphalt, mineral filler, and other additives shall be verified by the mix design. The materials shall meet the following requirements for ISSA A143.

ISSA Test No.	Description	Specification
ISSA TB-139	Wet Cohesion @ 30 minutes min. (Set) @ 60 minutes min. (Traffic)	12 kg-cm min. 20 kg-cm min. or Near Spin
ISSA TB-109	Excess Asphalt by LWT Sand Adhesion	50 gm/sq ft (538 gm/sq m) max.
ISSA TB-114	Wet Stripping	Pass (90% min.)
ISSA TB-100	Wet-Track Abrasion Loss One-hour Soak Six-day Soak	50 gm/sq ft (538 gm/sq m) max. 75 gm/sq ft (807 gm/sq m) max.
ISSA TB-147	Lateral Displacement	5% max.
	Specific Gravity after 1,000 Cycles of 25 lb (11.34 kg)	2.10 max.
ISSA TB-144	Classification Compatibility	11 Grade Points min. (AAA, BAA)
ISSA TB-113	Mix Time @ 77 °F (25 °C)	Controllable to 120 seconds min.

The mixing test and set-time test shall be checked at the highest temperatures expected during construction.

The mix design shall report the quantitative effects of moisture content on the unit weight of the aggregate (bulking effect). The report shall clearly show the proportions of aggregate, mineral filler (minimum and maximum), water (minimum and maximum), additive usage, and latex-modified asphalt emulsion based on the dry weight of the aggregate.

For the aggregate blend in the mix design, test results for AASHTO T 176 shall be provided with the mix information to the Engineer. Aggregate test values below 65 shall require review and approval from the Engineer.

Before the work commences, the Contractor shall submit to the Engineer a complete mix design covering the specific materials to be used on the project. The percentages of each individual material required shall be shown in the laboratory report. The Engineer shall approve the mix design prior to its use. After approval, no substitutions will be permitted, unless approved by the

Engineer, and the Contractor shall maintain continuous control of the latex-modified emulsified asphalt to dry aggregate proportioning to conform to the approved mix design within a tolerance of ± 2 gal/ton (± 8 L/metric ton).

Micro-Surfacing Test Strip. For projects over 100,000 sq yd (83,600 sq m), at least one day prior to starting the project the Contractor shall designate a mutually agreeable location and apply a test strip of micro-surfacing using the aggregate indicated in the mix design. The Engineer will evaluate the micro-surfacing application rate and cure time.

Surface Preparation. Pavement markings shall be removed according to Article 783.03(a) of the Standard Specifications. Only very small particles of tightly adhering existing markings may remain in place.

When specified in the plans, pavement markers shall be removed according to Article 783.03(b) of the Standard Specifications.

Bumps greater than or equal to 1/2 in. (13 mm) shall be removed by grinding. The Contractor shall determine bump grinding locations in the presence of the Engineer by using a 16 ft (5 m) straightedge with the scratcher bolts set to 1/2 in. (13 mm). All locations marked by the scratcher bolts shall be ground using either a grinding machine consisting of multiple saws or a cold-milling machine with a double- or triple-wrap milling head.

Joints and cracks 3/16 in. (5 mm) or wider shall be cleaned of loose and unsound material and sealed. The sealant shall be applied only when the joints and cracks are clean and dry and the ambient temperature is 40-85 °F (4-29 °C). The sealant shall be applied using a pressurized wand delivery system with such devices as necessary to seal the cracks/joints and form a nominal 0.125 in. (3 mm) thick by 3 in. (75 mm) wide overseal band centered so that the center of the 3 in. (75 mm) wide band is within 1 in. (25 mm) of the crack. The sealant shall be allowed to cure before opening to traffic. When approved by the Engineer, the sealer may be dusted with fine sand, portland cement, or mineral filler to prevent tracking.

Prior to applying the A-1 bituminous surface treatment, the pavement surface shall be cleaned.

Manholes, valve boxes, drop inlets, and other service entrances shall be protected from the cape seal by a suitable method. The surface preparation shall be approved by the Engineer prior to application of the A-1 bituminous surface treatment. No dry aggregate either spilled from the lay-down machine or existing on the road, will be permitted.

Calibration. The working day prior to starting construction of the A-1 bituminous surface course, the pressure distributor and aggregate spreader shall be calibrated and adjusted according to the manufacturer's recommendations. At least three days prior to starting the work the Contractor shall provide the Engineer with a copy of the manufacturer's recommendations for the equipment to be used. All calibrations and adjustments shall be made in the presence of the Engineer on a level surface at a location approved by the Engineer. The Contractor shall maintain proper calibration and adjustment of the equipment and the Engineer reserves the right to check application rates as the work progresses. Should the equipment fail to consistently apply the specified rates, the work shall be stopped and the Contractor shall recalibrate and readjust the equipment.

Application. The cape seal shall be applied as shown on the plans and the following.

- (a) A-1 Bituminous Surface Treatment. The bituminous material and aggregate shall be applied according to the following.

- (1) Application Rates. Based upon the aggregate gradation to be used, the Contractor shall determine the application rates of bituminous material and seal coat aggregate. The application rates along with the seal coat gradations shall be submitted to the Engineer for approval prior to the start of work. Application rates shall be according to the following table for the aggregate type shown on the plans, and shall result in aggregate embedment between 50 and 70 percent behind the roller. Changes in the application rate of greater than 15 percent shall be resubmitted to the Engineer for approval.

Aggregate Type	Bituminous Material Rate	Aggregate Rate
CA 15	0.38 – 0.46 gal/sq yd (1.7 – 2.1 L/sq m)	22 – 30 lb/sq yd (12 – 16 kg/sq m)
CA 16	0.36 – 0.40 gal/sq yd (1.6 – 1.8 L/sq m)	18 – 26 lb/sq yd (8 – 14 kg/sq m)
CA 20	0.36 – 0.40 gal/sq yd (1.6 – 1.8 L/sq m)	18 – 26 lb/sq yd (8 – 14 kg/sq m)
FA 1 (Special)	0.26 – 0.30 gal/sq yd (1.2 – 1.4 L/sq m)	16 – 20 lb/sq yd (9 – 11 kg/sq m)
FA 4 (Special)	0.28 – 0.36 gal/sq yd (1.3 – 1.6 L/sq m)	18 – 24 lb/sq yd (10 – 13 kg/sq m)
FA 22	0.32 – 0.40 gal/sq yd (1.5 – 1.8 L/sq m)	15 – 22 lb/sq yd (8 – 12 kg/sq m)

- (2) Preparation of Bituminous Material. The temperature of the bituminous material at the time of application shall be such that it will spray uniformly without clogging the spraying nozzles and shall be applied within the temperature ranges of 150 – 190 °F (65 – 90 °C).
- (3) Preparation of Aggregate. The aggregate shall be stockpiled near the jobsite according to Article 1003.01(e) or 1004.01(e) of the Standard Specifications. The aggregate used shall contain no free moisture. Slightly damp aggregate may be used with the approval of the Engineer.
- (4) Application of Bituminous Material. The bituminous material shall be applied with a pressure distributor. The entire length of the spray bar shall be set at the height above the surface recommended by the manufacturer for even distribution of the bituminous material.

The distributor shall be operated in a manner such that missing or overlapping of transverse joints is avoided. To prevent overlapping of successive applications of bituminous material at transverse joints, heavy paper shall be spread over the previously applied bituminous material and aggregates. In order to obtain a uniform application of the bituminous material, the distributor shall be traveling at the speed required for the specified rate of application when the spray bar crosses the paper.

Adjacent construction, such as concrete pavement, curb and gutter, bridge floors, raised reflective pavement markers, and bridge handrails, shall be protected by shields, covers or other means. If bituminous material is applied to adjacent construction, the Contractor shall remove such material to the satisfaction of the Engineer.

The emulsified asphalt shall not be applied when the wind conditions will inhibit uniform coverage from the fans of asphalt being applied.

- (5) Application of Aggregates. The seal coat aggregates shall be spread evenly with an aggregate spreader over the entire surface being treated. When treating one-half of the pavement width at a time, an inside strip of uncovered emulsified asphalt 3 in. (75 mm) wide shall be left during construction of the first half to provide center joint overlap when the second half of the treatment is placed. In all cases, the aggregate shall be applied ahead of the truck or spreader wheels. Hand spreading will be permitted only when approved by the Engineer and, when so permitted, the aggregate shall be spread uniformly and at the approximate rate specified. Any ridges of aggregate left by the aggregate spreader shall be smoothed out with hand brooms immediately behind the aggregate spreader.

All equipment involved in the work shall operate as close to each other as practical. The aggregate shall cover the asphalt emulsion within 30 seconds of applications. At no time shall the aggregate spreader trail the pressure distributor by more than 150 ft (45 m) to ensure proper asphalt/aggregate adhesion.

Each aggregate truck shall be equipped with a suitable hitch for connection to the aggregate spreader while unloading. The trucks shall avoid contact between the truck body or bed and the aggregate spreader. The body or bed of the truck shall be modified, if necessary, to empty cleanly and completely into the receiving hopper of the aggregate spreader. No aggregate shall be allowed to spill onto the road surface when the truck is emptying into this hopper.

The aggregate shall be rolled following spreading. A maximum time of five minutes will be allowed between the spreading of aggregate and completion of the initial rolling of the aggregate. The rollers shall proceed in a longitudinal direction at a speed less than or equal to 5 mph (8 km/h). Each roller shall travel over the aggregate a minimum of two times. The entire surface shall be rolled immediately with a self-propelled pneumatic-tired roller. Rolling shall proceed in a longitudinal direction beginning at the edges and progressing toward the center, overlapping on successive trips by at least 1/2 the width of the roller. The aggregate shall then be rolled with a separate pneumatic-tired roller until the aggregate is properly seated in the bituminous material.

The Contractor shall use the appropriate sweeping equipment to perform an initial sweeping after a minimum of two hours curing and not less than one hour before sunset on the day the A-1 surface treatment is placed. The initial sweeping shall remove excess aggregate by lightly sweeping each pavement lane. The sweeping shall be sufficient to prevent migration of loose aggregate back onto any part of the pavement.

The Contractor shall sweep the pavement surface as needed to remove excess aggregate.

- (b) Micro-Surfacing. This method shall consist of applying the surface mix within a maximum of 12 calendar days of placing the A-1 bituminous surface treatment. The Contractor shall sweep the pavement surface immediately prior to applying the micro-surfacing.

The surface shall be prewetted by water fogging ahead of the spreader box when road conditions require, as determined by the Engineer. The rate of fogging shall be adjusted during the day based on pavement temperature, surface texture, and dryness.

- (1) Application. The micro-surfacing shall be applied over the entire width of each lane in a single pass at a rate of 24 lb/sq yd (13 kg/sq m). The application rate shall be

verified from daily readings taken from the proportioning devices during the progress of the work.

The paving mixture shall be spread to leave a uniform surface. A sufficient amount of material shall be carried at all times in all parts of the spreader box to ensure complete coverage. Overloading of the spreader shall be avoided. No lumps or uncoated aggregate will be permitted in the finished surface.

Adjustments to the mix design may be required during construction, based on field conditions. The percent of mineral filler in the mix design may be increased or decreased by less than 0.3 percent when the slurry seal is being placed if it is found to be necessary for better consistency or set times. The Engineer will give final approval for all adjustments.

- (2) **Mix Consistency.** The finished product shall be uniform in color and composition. No streaks, such as those caused by oversized aggregate, shall be left in the finished surface. If excess streaking develops, the job will be stopped until the Contractor proves to the Engineer that the situation has been corrected. Excessive streaking is defined as more than four drag marks greater than 1/2 in. (13 mm) wide and 4 in. (100 mm) long, or 1 in. (25 mm) wide and 3 in. (75 mm) long, in any 30 sq yd (25 sq m) area. No transverse ripples or longitudinal streaks of 0.25 in. (6 mm) in depth will be permitted, when measured by placing a 10 ft (3 m) straightedge over the surface.
- (3) **Mix Stability.** The micro-surfacing shall possess sufficient stability so that premature breaking of the material in the spreader box does not occur. The mixture shall be homogeneous during and following mixing and spreading. It shall be free of excess water or emulsified asphalt and free of segregation of the emulsified asphalt and aggregate fines from the coarser aggregate. Under no circumstances shall water be sprayed directly into the lay-down box while placing micro-surfacing material.
- (4) **Joints and Edges.** The Contractor shall devise a joint plan according to ISSA A143 and submit to the Engineer for approval. When practical, the surface course joint shall be at least 10 in. (255 mm) away from the nearest edge of any subsequent permanent pavement markings.

Micro-surfacing edges shall be parallel with the existing pavement edges. If the existing pavement edge cannot be used to give a straight edge, a stringline or other guide will be required. Edge lines shall not vary by more than ± 2 in. (± 50 mm) horizontally in any 100 ft (30 m) of length.

A smooth, neat seam shall be provided where two passes meet. Excess material shall be immediately removed from the ends of each run. Any damage to, or irregularities in, the micro-surfacing shall be repaired, as directed by the Engineer. All repairs shall be made with a paver box, except areas designated as hand work areas.

- (5) **Hand Work.** Those areas inaccessible to the spreader box and other areas approved by the Engineer shall be designated as hand work areas. Adjustments to the additive will be permitted to provide a slower setting time when hand spreading is needed. If hand spreading is necessary, the mixture shall be poured in a small windrow along one edge of the surface to be covered and then spread uniformly by a hand squeegee or lute. Hand work areas shall have an appearance consistent with that being placed with a spreader box.

Clean-Up. All areas, such as manholes, gutters, and intersections, shall have the cape seal removed as specified by the Engineer. The Contractor shall, on a daily basis, remove any debris associated with the performance of the work.

Sampling and Testing. The Contractor shall check yield of the application after the first 1000 ft (300 m), and throughout each day's paving, with a minimum of three tests per day. Yield check results shall be furnished to the Engineer daily.

The Contractor shall submit a daily "run sheet" for each day's work as soon as all the data is available. The run sheet shall provide a breakdown of the actual meter numbers and quantities of all materials actually used each day, as well as the respective locations.

Opening to Traffic. The A-1 bituminous surface treatment portion shall be opened to traffic according to Article 701.17(c)(4) of the Standard Specifications.

The micro-surfacing shall be opened to traffic within one hour of its application.

Curing. The micro-surfacing shall cure for a minimum of seven days before placement of the permanent pavement markings.

Method of Measurement. Crack/joint sealing will be measured for payment in feet (meters), measured along the crack.

Pavement marking removal and pavement marker removal will be measured for payment according to Article 783.05 of the Standard Specifications.

The cape seal will be measured for payment in place and the area computed in square yards (square meters). The width for measurement will be the width of the top surface as shown on the plans or as directed by the Engineer.

Basis of Payment. Crack/joint sealing will be paid for at the contract unit price per foot (meter) for FIBER-MODIFIED ASPHALT CRACK SEALING.

Bump removal will be paid for at the contract unit price per each for BUMP REMOVAL.

Pavement marking removal and pavement marker removal will be paid for according to Article 783.06 of the Standard Specifications.

Cape seal will be paid for at the contract unit price per square yard (square meter) for CAPE SEAL, of the gradation type and friction aggregate mixture specified.

Designer Note: Insert into all contracts using micro-surfacing. Use of this special provision shall be according to the Bureau of Design and Environment Manual, Chapter 52.

The designer must specify the friction aggregate mixture and the following information on the plans using the following note:

“The aggregates for the surface lift of micro-surfacing shall meet the friction aggregate requirements for Mixture ___ in Article 1004.03(a).”

Insert either “C” or “D” into the note to indicate which mixture is to be used according to the ADT volume on the project. ADT ≤ 5,000 shall use Mixture C, and ADT > 5,000 shall use Mixture D.

PREVENTIVE MAINTENANCE - MICRO-SURFACING (BDE)

Effective: January 1, 2009

Revised: April 1, 2012

Description. This work shall consist of micro-surfacing hot-mix asphalt (HMA) surfaces.

Materials. Materials shall be according to the following.

- (a) Micro-Surfacing. Materials shall be according to the following Articles/Sections of the Standard Specifications.

Item	Article/Section
(1) Mineral Filler (Note 1)	1001
(2) Water	1002
(3) Coarse Aggregate (Note 2)	1004.03
(4) Bituminous Material (Prime Coat)	1032.06
(5) Latex-Modified Emulsified Asphalt (Note 3)	
(6) Additives (Note 4)	

Note 1. The mineral filler shall be Type 1 portland cement.

Note 2. The coarse aggregate material shall be selected from the table in Article 1004.03(a) of the Standard Specifications based upon the friction aggregate mixture specified. The quality of the aggregate shall be Class B and the gradation shall be as shown in the table below.

Sieve Size	Type II % Passing	Type III % Passing ^{1/}
3/8 in. (9.5 mm)	100	100
#4 (4.75 mm)	95 ± 5	80 ± 10
#8 (2.36 mm)	77 ± 13	57 ± 13
#16 (1.18 mm)	57 ± 13	39 ± 11
#30 (600 µm)	35 ± 10	26 ± 8
#50 (330 µm)	19 ± 6	18 ± 7
#100 (150 µm)	15 ± 6	12 ± 6
#200 (75 µm)	10 ± 5	10 ± 5

1/ Rut filling mixes shall be constructed using a Type III gradation. All surface mixes shall be constructed using a Type II gradation.

To assure the material is totally crushed, 100 percent of the parent aggregate shall be larger than the largest stone in the gradation to be used.

The blending, alternate use, and /or substitutions of aggregates from different sources for use in this work will not be permitted without the approval of the Engineer. Any blending shall be by interlocked mechanical feeders. The blending shall be uniform, compatible with the other components of the mix, and the equipment shall be approved by the Engineer.

If blending aggregates, the blend shall have a washed gradation performed every other day or a minimum of three tests per week. Testing shall be completed before the aggregate receives final acceptance for use in the mix. All gradation tests shall be conducted according to the aggregate gradation control system (AGCS).

Aggregates shall be screened at the stockpile prior to delivery to the paving machine to remove oversized material or contaminants.

Note 3. CSS-1h Latex Modified Emulsified Asphalt. The emulsified asphalt shall be a quick-traffic latex modified asphalt emulsion containing a minimum of 3.0 percent latex solids by weight of asphalt binder. The latex shall be milled or blended into the emulsifier solution prior to the emulsification process. The CSS-1h latex modified emulsified asphalt shall be according to the following.

Test (AASHTO T 59)	Result
Viscosity, Saybolt Furol, 77 °F (25 °C), SFS	20-100
Storage Stability Test, 24 hours, %	1 max.
Particle Charge Test	Positive
Sieve Test, No. 20 (850 µm), retained on sieve, %	0.10 max.
Distillation Test, Residue from distillation test to 347 ± 9 °F (175 ± 5 °C), %	62 min.

Tests on residue from distillation	Result
Penetration, 77 °F (25 °C), 100 grams, 5 seconds, (AASHTO T 49), dmm	40-90
Ductility, 77 °F (25 °C), 50 mm/min, (AASHTO T 51), mm	400 min.
Solubility in trichloroethylene, (AASHTO T 44), %	97.5 min.
Softening Point, (AASHTO T 53), °F (°C)	135 (57) min.
Absolute Viscosity, 140 °F (60 °C), (AASHTO T 202), Poises (Pa · sec)	8,000 (800) min.

Note 4. Additives may be added to the emulsion mix or any of the component materials to provide the control of the quick-traffic properties. They shall be included as part of the mix design and be compatible with the other components of the mix.

(b) Crack/Joint Sealant. The crack/joint sealant shall be a fiber-modified asphalt binder mixed at the jobsite or premixed.

(1) Jobsite-Mixed Sealant. The sealant shall consist of an asphalt binder and fibers, and be according to the following.

- a. Asphalt Binder. The asphalt binder shall be PG 58-28, PG 58-22, or PG 64-22 and meet the requirements of Article 1032.05 of the Standard Specifications.
- b. Fibers. Fibers shall be short cut polypropylene fibers meeting the properties listed below. The fiber may be accepted on certification from the manufacturer that it meets the specified requirements.

Property	Value
Length, in. (mm)	0.3 - 0.5 (8 - 12)
Denier	13 - 16
Crimps	None
Tensile Strength, min., psi (kPa)	40,000 (275,000)
Specific Gravity (typical)	0.91
Moisture Regain @ 70 °F (21 °C) and 65% RH (typical), %	0.1

- c. Percent Fibers. The sealant shall contain a minimum of 8.0 percent of fibers by weight (mass).
 - d. Sealant Heating. The sealant shall be heated in the kettle at temperatures between 255 and 285 °F (124 and 141 °C).
- (2) Premixed Sealant. The sealant shall be packaged and consist of an asphalt binder, fibers, and other modifiers meeting the following requirements. The sealant and its components may be accepted on certification from the manufacturer that it meets the specified requirements.

- a. Asphalt Binder. The asphalt binder shall be PG 64-22 and meet the requirements of Article 1032.05 of the Standard Specifications.
- b. Fibers. Fibers shall be short cut polyester fibers meeting the following.

Property	Value
Length, in. (mm)	0.25 ± 0.02 (6.3 ± 0.5)
Denier	3 - 6
Crimps	None
Tensile Strength, minimum, psi (kPa)	70,000 (482,000)
Specific Gravity (typical)	1.32 - 1.40
Elongation at Break, %	35 - 38
Melt Temperature, °F (°C)	475 - 490 (246 - 254)

- c. Percent Fibers. The sealant shall contain 5.0 ± 0.5 percent of fibers by weight (mass).

The sealant, in its final form, shall meet the following requirements when sampled and heated to the manufacturer's recommended maximum heating temperature according to ASTM D 5167.

Test	Value
Cone Penetration @ 77 °F (25 °C), ASTM D 5329	10-35 dmm
Softening Point, ASTM D 36	175 °F (79 °C) min.
Maximum Heating Temperature	400°F (204 °C)
Application Temperature	350°F (177 °C) min.

Equipment. Equipment shall be according to the following.

(a) Micro-Surfacing. Equipment shall be according to the following.

- (1) Micro-Surfacing Mixing Machine. The machine shall be either a continuous (self-loading) machine or a non-continuous (self-contained) machine depending on the size of the project as described below. Both types of machines shall have sufficient storage capacity for aggregate, emulsified asphalt, mineral filler, control additive and water to maintain an adequate supply to the proportioning controls. The mixing unit shall be able to accurately deliver and proportion the aggregate, emulsified asphalt, mineral filler, control setting additive, and water to a revolving multi-blade, double-shafted mixer.

Machines that are the continuous (self-loading) type shall be an automatic-sequenced, self-propelled, continuous-flow mixing unit able to discharge the mixed product on a continuous-flow basis. The machine shall be equipped to allow the operator to have full control of the forward and reverse speeds during applications of the material and be equipped with opposite-side driver stations to assist in alignment.

Non-continuous (self-contained) machines will be allowed on projects with a length of 2 lane-miles (3.2 lane-km) or less. For mainline paving, the Contractor shall have at least three self-contained machines in continuous operation to ensure appropriate production rates. Self-contained machines will also be allowed on shoulders, ramps, short applications such as bridge decks, or where the material can be placed in a single loading capacity of the machine.

Each mixing unit to be used in the performance of the work shall be calibrated in the presence of the Engineer prior to construction. Each new or different aggregate requires a new calibration. Previous calibration documentation covering the exact materials to be used may be acceptable, provided that no more than 30 days have lapsed. The documentation shall include an individual calibration of each material at various settings, which can be related to the machine metering devices. Prior to the calibration process, portable scales used to calibrate the mixing machine for emulsion and aggregate shall be checked with 25 lb and 50 lb weights, respectively. Results from the standard weight checks shall be furnished to the Engineer. No machine will be allowed to work on the project until the calibration has been completed and/or accepted.

- (2) Micro-Surfacing Spreader. The mixture shall be agitated and spread uniformly in the surfacing box by means of twin shafted paddles or spiral augers fixed in the spreader box. A front seal shall be provided to insure no loss of the mixture at the road contact point. The rear seal shall act as a final strike-off and shall be adjustable. The spreader box and rear strike-off shall be so designed and operated that a uniform consistency is achieved to produce a free flow of material to the rear strike-off. The spreader box shall have suitable means provided to side shift the box to compensate for variations in the pavement geometry.

A secondary strike-off shall be provided to improve surface texture on the surface course. The secondary strike-off shall have the same adjustments as the spreader box and shall not bounce, wobble, or chatter.

When required on the plans, before the final surface course is placed, preliminary micro-surfacing material may be required to fill ruts, utility cuts, depressions in the existing surface, etc. Ruts of 1/2 in. (13 mm) or greater in depth shall be filled

independently with a rut-filling spreader box, either 5 or 6 ft (1.5 or 1.8 m) in width. For irregular or shallow rutting of less than 1/2 in. (13 mm) in depth, a full-width scratch-coat pass may be used as directed by the Engineer utilizing either a stiff primary rubber or else a metal primary strike off. Ruts that are in excess of 1 1/2 in. (38 mm) in depth may require multiple placements with the rut-filling spreader box to restore the cross-section. All rut-filling level-up material should cure under traffic for a minimum of 24 hours before additional material is placed on top of the level up.

- (3) **Micro-Surfacing Proportioning Devices.** Individual volume or weight controls for proportioning each material to be added to the mix (i.e. aggregate, mineral filler, emulsified asphalt, additive, and water) shall be provided and properly marked. These proportioning devices are used in material calibration and determining the material output at any time. Calibration records, conversion formulas, and daily run sheets including the beginning and final numbers shown on the proportioning devices shall be submitted to the Engineer for approval. During production any deviations from the original JMF shall be approved by the Engineer.

(b) **Crack/Joint Sealing.** Equipment shall be according to the following.

- (1) **Air Compressor.** The air compressor shall be capable of producing a minimum pressure of 90 psi (620 kPa) at the end of the discharge hose. The air stream shall discharge onto the pavement through an appropriate air lance. The tool lubricator shall be bypassed and a filter installed on the discharge valve to keep water and oil out of the line.
- (2) **Oil Kettle.** The crack sealant shall be heated in an oil jacketed double wall kettle equipped with an agitator (reversing rotary auger action) and separate thermometers for the oil bath and mixing chamber. The unit shall also be equipped with a reversible hydraulic 2 in. (50 mm) hot asphalt pump and a recirculating pump to circulate the oil bath.

CONSTRUCTION REQUIREMENTS

General. The paving mixture shall be capable of filling up to 1 1/2 in. (38 mm) wheel ruts in one pass, be capable of field regulation of the setting time, and be suitable for nighttime placement. The compatibility of all ingredients of the mix, including the mix set additive, shall be certified by the emulsified asphalt manufacturer.

Weather Limitations. Placement of the micro-surfacing shall be done between May 1 and October 15, and when the temperature is at least 50 °F (10 °C) and rising and the forecast for the next 24 hours is above 40 °F (5 °C).

Mix Design. A Contractor provided laboratory shall develop the mix design for the micro-surfacing mixture, shall verify the functioning of the set regulating additives, and shall present certified test results for the Engineer's approval. This laboratory shall be recognized by the International Slurry Surfacing Association (ISSA) as being capable of performing mix designs. The Engineer will verify the laboratory tests required in ISSA A143 have been conducted.

Proportions for the mix design shall be within the following limits.

Mineral Aggregate, dry weight (mass) lb/sq yd (kg/sq m)	15-50 (8-30)
Latex Emulsified Asphalt Residue, % by wt. of Aggregate	5.5-10.5

Latex Base Modifier	As required with % by weight (mass) of binder, min. of 3.0
Mix Set Additive	As required
Mineral Filler, % by weight (mass) of Aggregate	0.25 - 3 depending on weather conditions

The amount of mineral filler needed shall be determined by the laboratory mix design and will be considered as part of the aggregate gradation.

The amount and type of latex shall be determined by the laboratory performing the mix design. The minimum amount required shall be based on asphalt weight content and shall be certified by the emulsion supplier.

Compatibility of the aggregate, latex-modified emulsified asphalt, mineral filler, and other additives shall be verified by the mix design. The materials shall meet the following requirements for ISSA A143.

ISSA Test No.	Description	Specification
ISSA TB-139	Wet Cohesion @ 30 minutes min. (Set) @ 60 minutes min. (Traffic)	12 kg-cm min. 20 kg-cm min. or Near Spin
ISSA TB-109	Excess Asphalt by LWT Sand Adhesion	50 gm/sq ft (538 gm/sq m) max.
ISSA TB-114	Wet Stripping	Pass (90% min.)
ISSA TB-100	Wet-Track Abrasion Loss One-hour Soak Six-day Soak	50 gm/sq ft (538 gm/sq m) max. 75 gm/sq ft (807 gm/sq m) max.
ISSA TB-147	Lateral Displacement	5% max.
	Specific Gravity after 1,000 Cycles of 25 lb (11.34 kg)	2.10 max.
ISSA TB-144	Classification Compatibility	11 Grade Points min. (AAA, BAA)
ISSA TB-113	Mix Time @ 77 °F (25 °C)	Controllable to 120 seconds Min.

The mixing test and set-time test shall be checked at the highest temperatures expected during construction.

The mix design shall report the quantitative effects of moisture content on the unit weight of the aggregate (bulking effect). The report shall clearly show the proportions of aggregate, mineral filler (minimum and maximum), water (minimum and maximum), additive usage, and latex-modified asphalt emulsion based on the dry weight of the aggregate.

For the aggregate blend in the mix design, test results for AASHTO T 176 shall be provided with the mix information to the Engineer. Aggregate test values below 65 shall require review and approval from the Engineer.

Before the work commences, the Contractor shall submit to the Engineer a complete mix design covering the specific materials to be used on the project. The percentages of each individual material required shall be shown in the laboratory report. The Engineer shall approve the mix design prior to its use. After approval, no substitutions will be permitted, unless approved by the Engineer, and the Contractor shall maintain continuous control of the latex-modified emulsified asphalt to dry aggregate proportioning to conform to the approved mix design within a tolerance of ± 2 gal/ton (± 8 L/metric ton).

Test Strip. For projects over 100,000 sq yd (83,600 sq m), at least one day prior to starting the project the Contractor shall designate a mutually agreeable location and apply a test strip of micro-surfacing using the aggregate indicated in the mix design. The Engineer will evaluate the micro-surfacing application rate and cure time.

Surface Preparation. Pavement markings shall be removed according to Article 783.03(a) of the Standard Specifications. Only very small particles of tightly adhering existing markings may remain in place.

When specified in the plans, pavement markers shall be removed according to Article 783.03(b) of the Standard Specifications.

Bumps greater than or equal to 1/2 in. (13 mm) shall be removed by grinding. The Contractor shall determine bump grinding locations in the presence of the Engineer by using a 16-ft (5-m) straightedge with the scratcher bolts set to 1/2 in. (13 mm). All locations marked by the scratcher bolts shall be ground using either a grinding machine consisting of multiple saws or a cold-milling machine with a double- or triple-wrap milling head.

Joints and cracks 3/16 in. (5 mm) or wider shall be cleaned of loose and unsound material and sealed. The sealant shall be applied only when the joints and cracks are clean and dry, and the ambient temperature is 40-85 °F (4-29 °C). The sealant shall be applied using a pressurized wand delivery system with such devices as necessary to seal the cracks/joints and form a nominal 0.125 in. (3 mm) thick by 3 in. (75 mm) wide overseal band centered so that the center of the 3 in. (75 mm) wide band is within 1 in. (25 mm) of the crack. The sealant shall be allowed to cure before opening to traffic. When approved by the Engineer, the sealant may be dusted with fine sand, portland cement, or mineral filler to prevent tracking.

Micro-Surfacing. The micro-surfacing shall be applied as shown on the plans and the following.

- (a) Preparation. Prior to applying the micro-surfacing, the pavement surface shall be cleaned. On highly oxidized surfaces, a prime coat shall be applied at a rate of 0.05-0.10 gal/sq yd (0.22-0.45 L/sq m) according to Article 406.05(b) of the Standard Specifications. Manholes, valve boxes, drop inlets, and other service entrances shall be protected from the micro-surfacing by a suitable method. The surface preparation shall be approved by the Engineer prior to the application of the micro-surfacing. No dry aggregate either spilled from the lay-down machine or existing on the road will be permitted.

The Contractor shall apply the micro-surfacing according to the following methods.

- (1) Micro-Surfacing Rut Filling. This method shall consist of filling each of the two wheelpath ruts in a lane using the specially designed rutbox and the rutfill (Type III) mix. It shall be the Contractor's responsibility to determine and estimate the quantities of rutfill mix required for rut filling. This work is then followed by one pass of micro-surfacing as described below.
- (2) Micro-Surfacing, Single Pass. This method shall consist of applying the surface mix over the entire width of each lane in one pass at an application rate of 20 lb/sq yd (11 kg/sq m).

Determinations of application rates shall be from daily readings taken from the material control devices during the progress of the work.

The pavement surface shall be prewetted by water fogging ahead of the spreader box when road conditions require, as determined by the Engineer. The rate of fogging shall be adjusted during the day based on pavement temperature, surface texture, and dryness.

The paving mixture shall be spread to fill minor cracks and shallow potholes and leave a uniform surface. Care shall be taken when rut filling to restore the designed profile of the pavement cross section. Excess crowning (over-filling) of rut areas shall be avoided. A sufficient amount of material shall be carried at all times in all parts of the spreader box to ensure complete coverage. Overloading of the spreader shall be avoided. No lumps or uncoated aggregate will be permitted in the finished surface.

Adjustments to the mix design may be required during construction, based on field conditions. The percent of mineral filler in the mix design may be increased or decreased by less than 0.3 percent when the slurry seal is being placed if it is found to be necessary for better consistency or set times. The Engineer will give final approval for all adjustments.

- (b) **Mix Consistency.** The finished product shall be uniform in color and composition. No streaks, such as those caused by oversized aggregate, shall be left in the finished surface. If excess streaking develops, the job will be stopped until the Contractor proves to the Engineer that the situation has been corrected. Excessive streaking is defined as more than four drag marks greater than 1/2 in. (13 mm) wide and 4 in. (100 mm) long, or 1 in. (25 mm) wide and 3 in. (75 mm) long, in any 30 sq yd (25 sq m) area. No transverse ripples or longitudinal streaks of 0.25 in. (6 mm) in depth will be permitted, when measured by placing a 10 ft (3 m) straightedge over the surface.
- (c) **Mix Stability.** The micro-surfacing shall possess sufficient stability so that premature breaking of the material in the spreader box does not occur. The mixture shall be homogeneous during and following mixing and spreading. It shall be free of excess water or emulsified asphalt and free of segregation of the emulsified asphalt and aggregate fines from the coarser aggregate. Under no circumstances shall water be sprayed directly into the lay-down box while placing micro-surfacing material.
- (d) **Joints and Edges.** The Contractor shall devise a joint plan according to ISSA A143 and submit to the Engineer for approval. When practical, the surface course joint shall be at least 10 in. (255 mm) away from the nearest edge of any subsequent permanent pavement markings.

Micro-surfacing edges shall be parallel with the existing pavement edges. If the existing pavement edge cannot be used to give a straight edge, a stringline or other guide will be required. Edge lines shall not vary by more than ± 2 in. (50 mm) horizontally in any 100 ft (30 m) of length.

A smooth, neat seam shall be provided where two passes meet. Excess material shall be immediately removed from the ends of each run. Any damage to, or irregularities in, the micro-surfacing shall be repaired, as directed by the Engineer. All repairs shall be made with a paver box, except areas designated as hand work areas.

- (e) **Hand Work.** Those areas inaccessible to the spreader box and approved by the Engineer shall be designated as hand work areas. Adjustments to the additive will be permitted to provide a slower setting time when hand spreading is needed. If hand spreading is necessary, the mixture shall be poured in a small windrow along one edge of the surface to be covered and then spread uniformly by a hand squeegee or lute.

Hand work areas shall have an appearance consistent with that being placed with a spreader box.

Clean-Up. All areas, such as manholes, gutters, and intersections, shall have the micro-surfacing mix removed as specified by the Engineer. The Contractor shall, on a daily basis, remove any debris associated with the performance of the work.

Sampling and Testing. The Contractor shall check yield of the application after the first 1000 ft (300 m), and throughout each day's paving, with a minimum of three tests per day. Yield check results shall be furnished to the Engineer daily.

The Contractor shall submit a daily "run sheet" for each day's work as soon as all the data is available. The run sheet shall provide a breakdown of the actual meter numbers and quantities of all materials actually used each day, as well as the respective locations.

Opening to Traffic. The micro-surfacing shall be opened to traffic within one hour of its application.

Curing. The micro-surfacing shall cure for a minimum of 7 days before placement of the permanent pavement markings.

Method of Measurement. This work will be measured for payment as follows.

(a) Contract Quantities. The requirements for the use of contract quantities shall conform to Article 202.07(a) of the Standard Specifications.

(b) Measured Quantities. Crack/Joint sealing will be measured for payment in feet (meters), measured along the crack.

Pavement marking removal will be measured for payment according to Article 783.05 of the Standard Specifications.

The micro-surfacing will be measured according to the following for the method of application provided in the plans.

(1) Micro-Surfacing Rut Filling. Micro-surfacing rut filling will be measured for payment in place in feet (meters) along the wheel path or filled rut.

(2) Micro-surfacing, Single Pass. Micro-surfacing, single pass will be measured for payment in place and the area computed in square yards (square meters). The width for measurement will be the width of the top surface as shown on the plans or as directed by the Engineer.

Prime coat, when required, will be measured for payment according to Article 406.13(b) of the Standard Specifications.

Basis of Payment. Crack/joint sealing will be paid for at the contract unit price per foot (meter) of FIBER-MODIFIED ASPHALT CRACK SEALING.

Bump removal will be paid for at the contract unit price per each for BUMP REMOVAL.

Pavement marking removal and pavement marker removal will be paid for according to Article 783.06 of the Standard Specifications.

Rut filling will be paid for at the contract unit price per foot (meter) for MICRO-SURFACING RUT FILLING.

Micro-surfacing, single pass will be paid for at the contract unit price per square yard (square meter) for MICRO-SURFACING, SINGLE PASS, of the gradation type and friction aggregate mixture specified.

Prime coat, when required, will be paid for according to Article 406.14 of the Standard Specifications.

Designer Note: Insert into all contracts using slurry seal. Use of this special provision shall be according to the Bureau of Design and Environment Manual, Chapter 52.

The designer must include the following note on the plans.

“Aggregates for the slurry seal shall meet the friction aggregate requirements for Mixture C.”

PREVENTIVE MAINTENANCE - SLURRY SEAL (BDE)

Effective: January 1, 2009

Revised: April 1, 2012

Description. This work shall consist of slurry sealing existing hot-mix asphalt (HMA) surfaces.

Materials. Materials shall be according to the following.

- (a) Slurry Seal. Materials shall be according to the following Articles/Sections of the Standard Specifications.

Item	Article/Section
(1) Mineral Filler (Note 1)	1001
(2) Water.....	1002
(3) Coarse Aggregate (Note 2).....	1004.03
(4) Bituminous Material (Prime Coat)	1032.06
(5) Emulsified Asphalt (Note 3)	
(6) Additives (Note 4)	

Note 1. The mineral filler shall be Type 1 portland cement.

Note 2. The coarse aggregate material shall be selected from the table in Article 1004.03(a) of the Standard Specifications based upon the friction aggregate mixture specified. The quality of the aggregate shall be Class B and the gradation shall be as shown in the table below.

Sieve Size	% Passing
3/8 in. (9.5 mm)	100
#4 (4.75 mm)	95 ± 5
#8 (2.36 mm)	77 ± 13
#16 (1.18 mm)	57 ± 13
#30 (600 µm)	35 ± 10
#50 (330 µm)	19 ± 6
#100 (150 µm)	15 ± 6
#200 (75 µm)	10 ± 5

To assure the material is totally crushed, 100 percent of the parent aggregate shall be larger than the largest stone in the gradation to be used.

The blending, alternate use, and/or substitutions of aggregates from different sources for use in this work will not be permitted without the approval of the Engineer. Any blending shall be by interlocked mechanical feeders. The blending

shall be uniform, compatible with the other components of the mix, and the equipment shall be approved by the Engineer.

If blending aggregates, the blend shall have a washed gradation performed every other day or a minimum of three tests per week. Testing shall be completed before the aggregate receives final acceptance for use in the mix. All gradation tests shall be conducted according to the aggregate gradation control system (AGCS).

Aggregates shall be screened at the stockpile prior to delivery to the paving machine to remove oversized material or contaminants.

Note 3. The emulsified asphalt shall be a quick-traffic, asphalt emulsion conforming to the requirements of AASHTO M 208 for CSS-1h, and the following.

AASHTO Test No.	Quality	Specification
AASHTO T 59 ^{1/}	Residue after Distillation	60% Minimum

1/ When using a latex-modified emulsion, the distillation temperature shall be 347 ± 9 °F (175 ± 5 °C).

AASHTO Test No.	Tests on Residue	Specification
AASHTO T 49	Penetration at 77 °F (25 °C)	40 - 90 dmm ^{2/}

2/ Climatic conditions should be considered when establishing this range.

The cement mixing test will be waived for this emulsion.

If a latex-modified emulsion is used, the latex shall be milled or blended into the emulsifier solution prior to the emulsification process.

Note 4. Additives may be added to the emulsion mix or any of the component materials to provide control of the quick-traffic properties. They shall be included as part of the mix design and be compatible with the other components of the mix.

(b) Crack/Joint Sealant. The crack/joint sealant shall be a fiber-modified asphalt binder mixed at the jobsite or premixed.

(1) Jobsite-Mixed Sealant. The sealant shall consist of an asphalt binder and fibers, and be according to the following.

a. Asphalt Binder. The asphalt binder shall be PG 58-28, PG 58-22, or PG 64-22 and meet the requirements of Article 1032.05 of the Standard Specifications.

b. Fibers. Fibers shall be short cut polypropylene fibers meeting the properties listed below. The fiber may be accepted on certification from the manufacturer that it meets the specified requirements.

Property	Value
Length, in. (mm)	0.3 - 0.5 (8 - 12)
Denier	13 - 16
Crimps	None
Tensile Strength, min., psi (kPa)	40,000 (275,000)
Specific Gravity (typical)	0.91

Moisture Regain @ 70 °F (21 °C) and 65% RH (typical), %	0.1
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- c. Percent Fibers. The sealant shall contain a minimum of 8.0 percent of fibers by weight (mass).
 - d. Sealant Heating. The sealant shall be heated in the kettle at temperatures between 255 and 285 °F (124 and 141 °C).
- (2) Premixed Sealant. The sealant shall be packaged and consist of an asphalt binder, fibers, and other modifiers meeting the following requirements. The sealant and its components may be accepted on certification from the manufacturer that it meets the specified requirements.
- a. Asphalt Binder. The asphalt binder shall be PG 64-22 and meet the requirements of Article 1032.05 of the Standard Specifications.
 - b. Fibers. Fibers shall be short cut polyester fibers meeting the following.

Property	Value
Length, in. (mm)	0.25 ± 0.02 (6.3 ± 0.5)
Denier	3 - 6
Crimps	None
Tensile Strength, min., psi (kPa)	70,000 (482,000)
Specific Gravity (typical)	1.32 - 1.40
Elongation at Break, %	35 - 38
Melt Temperature, °F (°C)	475 - 490 (246 - 254)

- c. Percent Fibers. The sealant shall contain 5.0 ± 0.5 percent of fibers by weight (mass).

The sealant, in its final form, shall meet the following requirements when sampled and heated to the manufacturer's recommended maximum heating temperature according to ASTM D 5167.

Test	Value
Cone Penetration @ 77 °F (25 °C), ASTM D 5329	10-35 dmm
Softening Point, ASTM D 36	175 °F (79 °C) min.
Maximum Heating Temperature	400°F (204 °C)
Application Temperature	350°F (177 °C) min.

Equipment. Equipment shall be according to the following.

- (a) Slurry Seal. Equipment shall be according to the following.
 - (1) Slurry Seal Mixing Machine. The machine shall be either a continuous (self-loading) machine or a non-continuous (self-contained) machine depending on the size of the project as described below. Both types of machines shall have sufficient storage capacity for aggregate, emulsified asphalt, mineral filler, control additive and water to maintain an adequate supply to the proportioning controls. The mixing unit shall be able to accurately deliver and proportion the aggregate, emulsified asphalt, mineral filler, control setting additive, and water to a revolving multi-blade, double-shafted mixer.

Machines that are the continuous (self-loading) type shall be an automatic-sequenced, self-propelled, continuous-flow mixing unit able to discharge the mixed product on a continuous-flow basis. The machine shall be equipped to allow the operator to have full control of the forward and reverse speeds during applications of the material and be equipped with opposite-side driver stations to assist in alignment.

Non-continuous (self-contained) machines will be allowed on projects with a length of 2 lane-miles (3.2 lane-km) or less. For mainline paving, the Contractor shall have at least three self-contained machines in continuous operation to ensure appropriate production rates. Self-contained machines will also be allowed on shoulders, ramps, short applications such as bridge decks, or where the material can be placed in a single loading capacity of the machine.

Each mixing unit to be used in the performance of the work shall be calibrated in the presence of the Engineer prior to construction. Each new or different aggregate requires a new calibration. Previous calibration documentation covering the exact materials to be used may be acceptable, provided that no more than 30 days have lapsed. The documentation shall include an individual calibration of each material at various settings, which can be related to the machine metering devices. Prior to the calibration process, portable scales used to calibrate the mixing machine for emulsion and aggregate shall be checked with 25 lb and 50 lb weights, respectively. Results from the standard weight checks shall be furnished to the Engineer. No machine will be allowed to work on the project until the calibration has been completed and/or accepted.

- (2) Slurry Seal Spreader. The mixture shall be agitated and spread uniformly in the surfacing box by means of twin shafted paddles or spiral augers fixed in the spreader box. A front seal shall be provided to insure no loss of the mixture at the road contact point. The rear seal shall act as a final strike-off and shall be adjustable. The spreader box and rear strike-off shall be so designed and operated that a uniform consistency is achieved to produce a free flow of material to the rear strike-off. The spreader box shall have suitable means provided to side shift the box to compensate for variations in the pavement geometry.
 - (3) Slurry Seal Proportioning Devices. Individual volume or weight controls for proportioning each material to be added to the mix (i.e. aggregate, mineral filler, emulsified asphalt, additive, and water) shall be provided and properly marked. These proportioning devices are used in material calibration and determining the material output at any time. Calibration records, conversion formulas, and daily run sheets including the beginning and final numbers shown on the proportioning devices shall be submitted to the Engineer for approval. During production any deviations from the original JMF shall be approved by the Engineer.
- (b) Crack/Joint Sealing. Equipment shall be according to the following.
- (1) Air Compressor. The air compressor shall be capable of producing a minimum pressure of 90 psi (620 kPa) at the end of the discharge hose. The air stream shall discharge onto the pavement through an appropriate air lance. The tool lubricator shall be bypassed and a filter installed on the discharge valve to keep water and oil out of the line.
 - (2) Oil Kettle. The crack sealant shall be heated in an oil jacketed double wall kettle equipped with an agitator (reversing rotary auger action) and separate thermometers for the oil bath and mixing chamber. The unit shall also be equipped with a

reversible hydraulic 2-in. (50-mm) hot asphalt pump and a recirculating pump to circulate the oil bath.

CONSTRUCTION REQUIREMENTS

General. The slurry seal shall be capable of field regulation of the setting time. The compatibility of all ingredients of the mix, including the mix set additive, shall be certified by the emulsified asphalt manufacturer.

Weather Limitations. Placement of the slurry seal shall be done between May 1 and October 15, and when the temperature is at least 50 °F (10 °C) and rising and the forecast for the next 24 hours is above 40 °F (5 °C).

Mix Design. A Contractor provided laboratory shall develop the mix design for the paving mixture, shall verify the functioning of the set regulating additives, and shall present certified test results for the Engineer's approval. This laboratory shall be recognized by the International Slurry Surfacing Association (ISSA) as being capable of performing mix designs. The Engineer will verify the laboratory tests required in ISSA A105 have been conducted.

Proportions for the mix design shall be within the following limits.

Mineral Aggregate, dry weight (mass) lb/sq yd (kg/sq m)	15 - 25 (8 - 15)
Emulsified Asphalt Residue, % by wt. of Aggregate	7.5 - 13.5
Latex Base Modifier (if required)	As required with % by weight (mass) of binder, min. of 5.0
Mix Set Additive	As required
Mineral Filler, % by weight (mass) of Aggregate	0.5 - 2.0 depending on weather conditions

The amount of mineral filler needed shall be determined by the laboratory mix design and will be considered as part of the aggregate gradation.

The amount and type of latex shall be determined by the laboratory performing the mix design. The minimum amount required shall be based on asphalt weight content and shall be certified by the emulsion supplier.

Compatibility of the aggregate, emulsified asphalt, mineral filler, and other additives shall be verified by the mix design. The materials shall meet the following requirements for ISSA A105.

ISSA Test No.	Description	Specification
ISSA TB-139	Wet Cohesion @ 30 minutes min. (Set) @ 60 minutes min. (Traffic)	12 kg-cm min. 20 kg-cm min. or Near Spin
ISSA TB-109	Excess Asphalt by LWT Sand Adhesion	50 gm/sq ft (538 gm/sq m) max.
ISSA TB-114	Wet Stripping	Pass (90% min.)
ISSA TB-100	Wet-Track Abrasion Loss One-hour Soak	75 gm/sq ft (807 gm/sq m) max.
ISSA TB-113	Mix Time @ 77 °F (25 °C)	Controllable to 180 seconds, min.

The mixing test and set-time test shall be checked at the highest temperatures expected during construction.

The mix design shall report the quantitative effects of moisture content on the unit weight of the aggregate (bulking effect). The report shall clearly show the proportions of aggregate, mineral filler (minimum and maximum), water (minimum and maximum), additive usage, and asphalt emulsion based on the dry weight of the aggregate.

For the aggregate blend in the mix design, test results for AASHTO T 176 shall be provided with the mix information to the Engineer. Aggregate test values below 45 shall require review and approval from the Engineer.

Before the work commences, the Contractor shall submit to the Engineer a complete mix design covering the specific materials to be used on the project. The percentages of each individual material required shall be shown in the laboratory report. The Engineer shall approve the mix design prior to its use. After approval, no substitutions will be permitted unless approved by the Engineer, and the Contractor shall maintain continuous control of the emulsified asphalt to dry aggregate proportioning to conform to the approved mix design within a tolerance of ± 2 gal/ton (± 8 L/metric ton).

Test Strip. For projects over 100,000 sq yd (83,600 sq m), at least one day prior to starting the project the Contractor shall designate a mutually agreeable location and apply a test strip of slurry seal using the aggregate indicated in the mix design. The Engineer will evaluate the slurry seal application rate and cure time.

Surface Preparation. Pavement markings shall be removed according to Article 783.03(a) of the Standard Specifications. Only very small particles of tightly adhering existing markings may remain in place.

When specified in the plans, pavement markers shall be removed according to Article 783.03(b) of the Standard Specifications.

Bumps greater than or equal to 1/2 in. (13 mm) shall be removed by grinding. The Contractor shall determine bump grinding locations in the presence of the Engineer by using a 16-ft (5-m) straightedge with the scratcher bolts set to 1/2 in. (13 mm). All locations marked by the scratcher bolts shall be ground using either a grinding machine consisting of multiple saws or a cold-milling machine with a double- or triple-wrap milling head.

Joints and cracks, 3/16 in. (5 mm) or wider, shall be cleaned of loose and unsound material and sealed. The sealant shall be applied only when the joints and cracks are clean and dry, and the ambient temperature is 40 - 85 °F (4 - 29 °C). The sealant shall be applied using a pressurized wand delivery system with such devices as necessary to seal the cracks/joints and form a nominal 0.125 in. (3 mm) thick by 3 in. (75 mm) wide overseal band centered so that the center of the 3 in. (75 mm) wide band is within 1 in. (25 mm) of the crack. The sealant shall be allowed to cure before opening to traffic. When approved by the Engineer, the sealant may be dusted with fine sand, portland cement, or mineral filler to prevent tracking.

Slurry Sealing. The slurry seal shall be applied as follows.

- (a) Preparation. Prior to applying the slurry seal, the pavement surface shall be cleaned. On highly oxidized surfaces, a prime coat shall be applied at a rate of 0.05-0.10 gal/sq yd (0.22-0.45 L/sq m) according to Article 406.05(b) of the Standard Specifications. Manholes, valve boxes, drop inlets, and other service entrances shall be protected from the slurry seal by a suitable method. The surface preparation shall be approved by the Engineer prior to application of the slurry seal. No dry aggregate either spilled from the lay-down machine or existing on the road, will be permitted.

The pavement surface shall be prewetted by water fogging ahead of the spreader box when road conditions require, as determined by the Engineer. The rate of fogging shall be adjusted during the day based on pavement temperature, surface texture, and dryness.

- (b) Application. The slurry seal shall be applied over the entire width of each lane in a single pass at a rate 20 lb/sq yd (11 kg/sq m). The application rate shall be verified from daily readings taken from the proportioning devices during the progress of the work.

The paving mixture shall be spread to fill minor cracks and shallow potholes and leave a uniform surface. A sufficient amount of material shall be carried at all times in all parts of the spreader box to ensure complete coverage. Overloading of the spreader shall be avoided. No lumps or uncoated aggregate will be permitted in the finished surface.

Adjustments to the mix design may be required during construction, based on field conditions. The percent of mineral filler in the mix design may be increased or decreased by less than 0.3 percent when the slurry seal is being placed if it is found to be necessary for better consistency or set times. The Engineer will give final approval for all adjustments.

- (c) Mix Consistency. The finished product shall be uniform in color and composition. No streaks, such as those caused by oversized aggregate, shall be left in the finished surface. If excess streaking develops, the job will be stopped until the Contractor proves to the Engineer that the situation has been corrected. Excessive streaking is defined as more than four drag marks greater than 1/2 in. (13 mm) wide and 4 in. (100 mm) long, or 1 in. (25 mm) wide and 3 in. (75 mm) long, in any 30 sq yd (25 sq m) area. No transverse ripples or longitudinal streaks of 0.25 in. (6 mm) in depth will be permitted, when measured by placing a 10 ft (3 m) straightedge over the surface.
- (d) Mix Stability. The slurry seal shall possess sufficient stability so that premature breaking of the material in the spreader box does not occur. The mixture shall be homogeneous during and following mixing and spreading. It shall be free of excess water or emulsified asphalt and free of segregation of the emulsified asphalt and aggregate fines from the coarser aggregate. Under no circumstances shall water be sprayed directly into the lay-down box while placing slurry seal material.
- (e) Joints and Edges. The Contractor shall devise a joint plan according to ISSA A105 and submit it to the Engineer for approval. When practical, the surface course joint shall be at least 10 in. (255 mm) away from the nearest edge of any subsequent permanent pavement markings.

Slurry seal edges shall be parallel with the existing pavement edges. If the existing pavement edge cannot be used to give a straight edge, a stringline or other guide will be required. Edge lines shall not vary by more than ± 2 in. (50 mm) horizontally in any 100 ft (30 m) of length.

A smooth, neat seam shall be provided where two passes meet. Excess material shall be immediately removed from the ends of each run. Any damage to, or irregularities in, the slurry seal shall be repaired, as directed by the Engineer. All repairs shall be made with a paver box, except areas designated as hand work areas.

- (f) Hand Work. Those areas inaccessible to the spreader box and approved by the Engineer shall be designated as hand work areas. Adjustments to the additive will be permitted to provide a slower setting time when hand spreading is needed. If hand spreading is necessary, the mixture shall be poured in a small windrow along one edge

of the surface to be covered and then spread uniformly by a hand squeegee or lute. Hand work areas shall have an appearance consistent with that being placed with a spreader box.

Clean-Up. All areas, such as manholes, gutters, and intersections, shall have the slurry seal mix removed as specified by the Engineer. The Contractor shall, on a daily basis, remove any debris associated with the performance of the work.

Sampling and Testing. The Contractor shall check yield of the application after the first 1000 ft (300 m), and throughout each day's paving, with a minimum of three tests per day. Yield check results shall be furnished to the Engineer daily.

The Contractor shall submit a daily "run sheet" for each day's work as soon as all the data is available. The run sheet shall provide a breakdown of the actual meter numbers and quantities of all materials actually used each day, as well as the respective locations.

Opening to Traffic. The slurry seal shall be opened to traffic within two hours of its application.

Curing. The slurry seal shall cure for a minimum of 7 days before placement of the permanent pavement markings.

Method of Measurement. This work will be measured for payment as follows.

- (a) Contract Quantities. The requirements for the use of contract quantities shall conform to Article 202.07(a) of the Standard Specifications.
- (b) Measured Quantities. Crack/Joint sealing will be measured for payment in feet (meters), measured along the crack.

Pavement marking removal will be measured for payment according to Article 783.05 of the Standard Specifications.

The slurry seal will be measured for payment in place and the area computed in square yards (square meters). The width for measurement will be the width of the top surface as shown on the plans or as directed by the Engineer.

Prime coat, when required, will be measured for payment according to Article 406.13(b) of the Standard Specifications.

Basis of Payment. Crack/joint sealing will be paid for at the contract unit price per foot (meter) of FIBER-MODIFIED ASPHALT CRACK SEALING.

Bump removal will be paid for at the contract unit price per each for BUMP REMOVAL.

Pavement marking removal and pavement marker removal will be paid for according to Article 783.06 of the Standard Specifications.

Slurry seal will be paid for at the contract unit price per square yard (square meter) for ASPHALTIC EMULSION SLURRY SEAL, of the friction aggregate mixture specified.

Prime coat, when required, will be paid for according to Article 406.14 of the Standard Specifications.

Designer Note: Insert this special into contracts using an A-1 bituminous surface treatment. Use of this special provision shall be according to the Bureau of Design and Environment Manual, Chapter 52.

The designer must specify the gradation for the bituminous surface treatment on the plans. Districts are encouraged to use the CA 20 gradation as it has proven to perform well for A-1 surface treatments.

Include Special Provision on Temporary Flexible Raised Pavement Marker with this work.

Include the following information in the Traffic Control Plan Special Provision:

- Contractor shall post the roadway with "LOOSE GRAVEL" and SPEED LIMIT 35" signs in accordance with applicable articles of Division 700 of the Standard Specifications.
- These signs shall be placed at the start of the work, near intersecting roadways and then at an average spacing of 0.5 mi (0.8 km).
- The signs may be removed as soon as the sweeping operation has been completed.

PREVENTIVE MAINTENANCE - BITUMINOUS SURFACE TREATMENT (BDE)

Effective: January 1, 2009

Revised: April 1, 2012

Description. This work shall consist of constructing a single bituminous surface treatment (A-1).

Materials. Materials shall be according to the following Articles/Sections of the Standard Specifications.

Item	Article/Section
(a) Seal Coat Aggregate (Note 1)	1003, 1004.03
(b) Bituminous Materials (Note 2)	1032

Note 1. The seal coat aggregate shall be either fine or coarse aggregate.

When fine aggregate is used, it shall be stone sand, wet bottom boiler slag, slag sand, or steel slag sand. The aggregate quality shall be Class C. The aggregate gradation shall be FA 1 (Special), FA 4 (Special), or FA 22 as specified on the plans and shall meet the following.

FINE AGGREGATE GRADATIONS						
Grad. No.	Sieve Size and Percent Passing					
	3/8 in. (9.5 mm)	No. 4 (4.75 mm)	No. 8 (2.36 mm)	No. 16 (1.18 mm)	No. 40 (425 µm)	No. 200 (75 µm)
FA 1 (Special)	100	90 ± 10	62.5 ± 17.5	32.5 ± 7.5	7.5 ± 7.5	1.5 ± 1
FA 4 (Special)	100	--	--	2 ± 2	--	1.5 ± 1
FA 22	100	*	*	8 ± 8	--	2 ± 2

* For the fine aggregate gradation FA 22, the aggregate producer shall set the midpoint percent passing, and the Department will apply a range of ± 10 percent. The midpoint shall not be changed without Department approval.

When coarse aggregate is used, it shall be crushed gravel, crushed stone, wet bottom boiler slag, crushed slag, crushed sandstone, or crushed steel slag. The aggregate quality shall be Class C and the deleterious chert count shall be no more than 25.0 percent by weight (mass) as determined by the Illinois Test Procedure 203. The aggregate gradation shall be CA 15, CA 16, or CA 20 as specified on the plans.

Note 2. The bituminous material shall be either a CRSP or an HFP polymer modified emulsified asphalt meeting the requirements of Article 1032.06(f)(2) of the Standard Specifications.

Equipment. Equipment shall be according to the following Articles/Sections of the Standard Specifications.

Item	Article/Section
(a) Self-Propelled Pneumatic-Tired Roller (Note 1)	1101.01
(b) Mechanical Sweeper (Note 2)	1101.03
(c) Aggregate Spreaders (Note 3)	1102.04
(d) Pressure Distributor (Note 4)	1102.05
(e) Heating Equipment	1102.07

Note 1. There shall be a minimum of two rollers, with the final number of rollers determined by the rollers' abilities to maintain proper spacing with the aggregate spreader as directed by the Engineer.

Note 2. The mechanical sweeper shall be power driven and self-propelled with the broom located between the axles. The mechanical sweeper shall not use a cantilever-mounted broom and the broom rotation shall not be operated by forward movement.

Note 3. The aggregate spreader shall be a self-propelled mechanical type with the receiving hopper in the rear and shall pull the aggregate truck. The spreader shall be fitted with an automated system which provides positive interconnected control of the aggregate flow with the forward speed of the spreader. The automated system shall provide uniform and consistent aggregate application at the rate specified.

The Engineer will check the spread roll of the aggregate spreader for straightness each day before operations begin. Should the surface of the spread roll vary off a straight line along its longitudinal dimension by more than 1/16 in. (1.5 mm), the Engineer will inspect the application of aggregate for corrugations and, should these occur, the machine shall be repaired or replaced. The forward speed of the spreader during calibration shall be the same as is to be used during construction. The equipment required for aggregate spreader calibration may consist of several sheets of canvas, each being exactly 1 sq yd (0.8 sq m), and a weight scale. By making several runs at different gate openings over the sheets of canvas, placed to cover the full width applied by the spreader, and carefully measuring the aggregate on each canvas sheet, the gate opening at the pre-established speed required to apply aggregate at the specified rate may be determined.

Note 4. The pressure distributor shall have a minimum capacity of 3000 gal (11,500 L). The application rate control shall be automated and shall control the application rate regardless of ground speed or spray bar width. The computer shall have the capability of recording the application rate, gallons sprayed, square yards, and feet traveled. The pressure distributor shall be capable of maintaining the asphalt emulsion at the specified temperature. The spray bar nozzles shall produce a uniform triple lap application fan spray, and the shutoff shall be instantaneous, with no dripping. The pressure distributor shall be capable of maintaining the specified application rate within ± 0.015 gal/sq yd

(± 0.070 L/sq m) for each load. The spray-bar nozzles shall be turned to make the same angle with the longitudinal axis of the spray bar as recommended by the manufacturer.

Application rates shall be determined by the procedures listed in ASTM D 2995, except the sample may be taken on three 8 x 12 in. (200 x 300 mm) metal plates. The three plates shall be positioned as directed by the Engineer.

CONSTRUCTION REQUIREMENTS

Weather Limitations. This work shall be done between May 1 and August 31. Bituminous materials shall be applied only when the temperature of the air in the shade is above 55 °F (13 °C). No work shall be started if local conditions indicate that rain is imminent.

This work may be done between September 1 and September 15 provided both of the following conditions are met:

- (a) The temperature of the air in the shade is above 70 °F (20 °C) and the temperature of the surface to which the asphalt will be applied is 70 °F (20 °C) or above, and
- (b) The National Weather Service forecast for the area does not show any rain or any temperatures below 55 °F (13 °C) for the day the work is to be done or for the following five days.

Repair and Preparation of Base or Existing Surface. The base or existing surface shall be prepared according to Section 358 of the Standard Specifications.

Calibration. The working day prior to starting construction, the pressure distributor and aggregate spreader shall be calibrated and adjusted according to the manufacturer's recommendations. At least three days prior to starting the work the Contractor shall provide the Engineer with a copy of the manufacturer's recommendations for the equipment to be used. All calibrations and adjustments shall be made in the presence of the Engineer on a level surface at a location approved by the Engineer. The Contractor shall maintain proper calibration and adjustment of the equipment and the Engineer reserves the right to check application rates as the work progresses. Should the equipment fail to consistently apply the specified rates, the work shall be stopped and the Contractor shall recalibrate and readjust the equipment.

Application Rates. Based upon the aggregate gradation to be used, the Contractor shall determine the application rates of bituminous material and seal coat aggregate. The application rates along with the seal coat gradations shall be submitted to the Engineer for approval prior to the start of work. Application rates shall be according to the following table for the aggregate type shown on the plans, and shall result in aggregate embedment between 50 and 70 percent behind the roller. Changes in the application rate of greater than 15 percent shall be resubmitted to the Engineer for approval.

Aggregate Type	Bituminous Material Rate	Aggregate Rate
CA 15	0.38 – 0.46 gal/sq yd (1.7 – 2.1 L/sq m)	22 – 30 lb/sq yd (12 – 16 kg/sq m)
CA 16	0.36 – 0.40 gal/sq yd (1.6 – 1.8 L/sq m)	18 – 26 lb/sq yd (8 – 14 kg/sq m)
CA 20	0.36 – 0.40 gal/sq yd (1.6 – 1.8 L/sq m)	18 – 26 lb/sq yd (8 – 14 kg/sq m)
FA 1 (Special)	0.26 – 0.30 gal/sq yd (1.2 – 1.4 L/sq m)	16 – 20 lb/sq yd (9 – 11 kg/sq m)
FA 4 (Special)	0.28 – 0.36 gal/sq yd (1.3 – 1.6 L/sq m)	18 – 24 lb/sq yd (10 – 13 kg/sq m)
FA 22	0.32 – 0.40 gal/sq yd (1.5 – 1.8 L/sq m)	15 – 22 lb/sq yd (8 – 12 kg/sq m)

Preparation of Bituminous Material. The temperature of the bituminous material at the time of application shall be such that it will spray uniformly without clogging the spraying nozzles and shall be applied within the temperature ranges of 150 – 190 °F (65 – 90 °C).

Preparation of Aggregate. The aggregate shall be stockpiled near the jobsite according to Article 1003.01(e) or 1004.01(e) of the Standard Specifications. The aggregate used shall contain no free moisture. Slightly damp aggregate may be used with the approval of the Engineer.

Application of Bituminous Material. The bituminous material shall be applied with a pressure distributor. The entire length of the spray bar shall be set at the height above the surface recommended by the manufacturer for even distribution of the bituminous material.

The distributor shall be operated in a manner such that missing or overlapping of transverse joints will be avoided. To prevent overlapping of successive applications of bituminous material at transverse joints, heavy paper shall be spread over the previously applied bituminous material and aggregates. In order to obtain a uniform application of the bituminous material, the distributor shall be traveling at the speed required for the specified rate of application when the spray bar crosses the paper.

Adjacent construction, such as concrete pavement, curb and gutter, bridge floors, raised reflective pavement markers, and bridge handrails, shall be protected by shields, covers or other means. If bituminous material is applied to adjacent construction, the Contractor shall remove such material to the satisfaction of the Engineer.

The emulsified asphalt shall not be applied when the wind conditions will inhibit uniform coverage from the fans of asphalt being applied.

Application of Aggregates. The seal coat aggregates shall be spread evenly with an aggregate spreader over the entire surface being treated. When treating one-half of the pavement width at a time, an inside strip of uncovered emulsified asphalt 3 in. (75 mm) wide shall be left during construction of the first half to provide center joint overlap when the second half of the treatment is placed. In all cases, the aggregate shall be applied ahead of the truck or spreader wheels. Hand spreading will be permitted only when approved by the Engineer and, when so permitted, the aggregate shall be spread uniformly and at the approximate rate specified. Any ridges of aggregate left by the aggregate spreader shall be smoothed out with hand brooms immediately behind the aggregate spreader.

All equipment involved in the work shall operate as close to each other as practical. The aggregate shall cover the asphalt emulsion within 30 seconds of applications. At no time shall the aggregate spreader trail the pressure distributor by more than 150 ft (45 m) to ensure proper asphalt/aggregate adhesion.

Each aggregate truck shall be equipped with a suitable hitch for connection to the aggregate spreader while unloading. The trucks shall avoid contact between the truck body or bed and the aggregate spreader. The body or bed of the truck shall be modified, if necessary, to empty cleanly and completely into the receiving hopper of the aggregate spreader. No aggregate shall be allowed to spill onto the road surface when the truck is emptying into this hopper.

The aggregate shall be rolled following spreading. A maximum time of five minutes will be allowed between the spreading of aggregate and completion of the initial rolling of the aggregate. The rollers will proceed in a longitudinal direction at a speed less than or equal to 5 mph (8 km/h). Each roller will travel over the aggregate a minimum of two times. The entire surface shall be rolled immediately with a self-propelled pneumatic-tired roller. Rolling shall proceed in a longitudinal direction beginning at the edges and progressing toward the center, overlapping on successive trips by at least 1/2 the width of the roller. The aggregate shall then be rolled with a separate pneumatic-tired roller until the aggregate is properly seated in the bituminous material.

The Contractor shall use the appropriate sweeping equipment to perform an initial sweeping after a minimum of two hours curing and not less than one hour before sunset on the day the bituminous surface treatment is placed. The initial sweeping shall remove excess aggregate by lightly sweeping each pavement lane. The sweeping shall be sufficient to prevent migration of loose aggregate back onto any part of the pavement.

The Contractor shall sweep the pavement surface as needed to remove excess aggregate.

Opening to Traffic. The road shall be opened to traffic according to Article 701.17(c)(4) of the Standard Specifications.

Method of Measurement. The bituminous surface treatment will be measured for payment in place and the area computed in square yards (square meters). The width for measurement will be the top width of the bituminous surface treatment as shown on the plans or as directed by the Engineer.

Basis of Payment. This work will be paid for at the contract unit price per square yard (square meter) for BITUMINOUS SURFACE TREATMENT (PREVENTIVE MAINTENANCE).

When provided as a payment item, the preparation of the existing surface will be measured and paid for as specified in Section 358 of the Standard Specifications. If not provided as a payment item, preparation of existing surface will be paid for according to Article 109.04.

Designer Note: Insert into all contracts with HMA items.

HOT-MIX ASPHALT - DENSITY TESTING OF LONGITUDINAL JOINTS (BDE)

Effective: January 1, 2010

Revised: April 1, 2012

Description. This work shall consist of testing the density of longitudinal joints as part of the quality control/quality assurance (QC/QA) of hot-mix asphalt (HMA). Work shall be according to Section 1030 of the Standard Specifications except as follows.

Quality Control/Quality Assurance (QC/QA). Delete the second and third sentence of the third paragraph of Article 1030.05(d)(3) of the Standard Specifications.

Add the following paragraphs to the end of Article 1030.05(d)(3) of the Standard Specifications:

“Longitudinal joint density testing shall be performed at each random density test location. Longitudinal joint testing shall be located at a distance equal to the lift thickness or a minimum of 4 in. (100 mm), from each pavement edge. (i.e. for a 5 in. (125 mm) lift the near edge of the density gauge or core barrel shall be within 5 in. (125 mm) from the edge of pavement.) Longitudinal joint density testing shall be performed using either a correlated nuclear gauge or cores.

- a. Confined Edge. Each confined edge density shall be represented by a one-minute nuclear density reading or a core density and shall be included in the average of density readings or core densities taken across the mat which represents the Individual Test.
- b. Unconfined Edge. Each unconfined edge joint density shall be represented by an average of three one-minute density readings or a single core density at the given density test location and shall meet the density requirements specified herein. The three one-minute readings shall be spaced ten feet apart longitudinally along the unconfined pavement edge and centered at the random density test location.”

Revise the Density Control Limits table in Article 1030.05(d)(4) of the Standard Specifications to read:

"Mixture Composition	Parameter	Individual Test (includes confined edges)	Unconfined Edge Joint Density Minimum
IL-4.75	N _{design} = 50	93.0 – 97.4%	91.0%
IL-9.5, IL-12.5	N _{design} ≥ 90	92.0 – 96.0%	90.0%
IL-9.5, IL-9.5L, IL-12.5	N _{design} < 90	92.5 – 97.4%	90.0%
IL-19.0, IL-25.0	N _{design} ≥ 90	93.0 – 96.0%	90.0%
IL-19.0, IL-19.0L, IL-25.0	N _{design} < 90	93.0 – 97.4%	90.0%
SMA	N _{design} = 50 & 80	93.5 – 97.4%	91.0%
All Other	N _{design} = 30	93.0 - 97.4%	90.0%”

Designer Note: Insert into contracts where PCC Inlay or Overlay is selected. This method is for locations where excessive rutting has become a problem. Discuss with the Project Engineer, Operations, and Implementation before using. Also check Recurring Special # 29.

PORTLAND CEMENT CONCRETE INLAY OR OVERLAY (BDE)

Effective: April 1, 2012

Revise Note 2 of Check Sheet #29 of the Supplemental Special and Recurring Special Provisions:

"Note 2. Synthetic fibers shall be Type III according to ASTM C 1116. The synthetic fiber shall be a monofilament or bundled monofilament with a minimum length of 1.0 in. (25 mm) and a maximum length of 2 1/2 in. (63 mm), and shall have a maximum aspect ratio (length divided by the equivalent diameter of the fiber) of 150. The quantity of synthetic fiber(s) added to the concrete mixture shall be sufficient to have a residual strength ratio ($R_{150,3}$) of 20.0 percent according to Illinois Modified ASTM C 1609. The maximum dosage rate shall not exceed 5.0 lb/cu yd (3.0 kg/cu m), unless the manufacturer can demonstrate through a field demonstration that the concrete mixture will be workable and fiber clumping is not a problem as determined by the Engineer.

The synthetic fibers shall be added to the concrete and mixed per the manufacturer's recommendation.

The Department will maintain an "Approved List of Synthetic Fibers", which will include the minimum required dosage rate. For the minimum required fiber dosage rate based on the Illinois Modified ASTM C 1609 test, a report prepared by an independent laboratory accredited by the AASHTO Materials Reference Laboratory (AMRL) for Portland Cement Concrete shall be provided."

44200

442.00

Designer Note: Do not use in District Four.

CALCIUM CHLORIDE ACCELERATOR FOR CLASS PP-2 CONCRETE (BDE)

Effective: April 1, 2012

When using Class PP-2 concrete in Class A, B, or C patches, the Contractor may substitute a calcium chloride accelerator for the non-chloride accelerator.

Designer Note: Insert into contracts with precast concrete or cast-in-place box culvert having a skew > 30 degrees and a design fill \leq 5 feet.

CONCRETE BOX CULVERTS WITH SKEWS > 30 DEGREES AND DESIGN FILLS \leq 5 FEET (BDE)

Effective: April 1, 2012

Revise the second paragraph of Article 540.04 of the Standard Specifications to read:

“Unless otherwise noted on the plans, the Contractor shall have the option, when a cast-in-place concrete box culvert is specified, of constructing the box culvert using precast box culvert sections when the design cover is 6 in. (150 mm) minimum. The precast box culvert sections shall be designed for the same design cover shown on the plans for cast-in-place box culvert; shall be of equal or larger size opening, and shall satisfy the design requirements of ASTM C 1577.”

Revise the fourth paragraph of Article 540.06 of the Standard Specifications to read:

“The excavation and backfilling for precast concrete box culverts shall be according to the requirements of Section 502, except where the design fill is less than or equal to 8 ft (2.4 m), or the design fill is less than the clear span of the box. In these cases ASTM C 1577 requires a select granular backfill (porous granular material) over the box. If a porous granular backfill is required but is not detailed on the plans for the culvert(s), the Contractor shall have the option of either furnishing porous granular backfill where required to satisfy ASTM C 1577, or submitting an alternate design, sealed by an Illinois licensed Structural Engineer, which precludes the use of a porous granular backfill. In addition for all precast boxes a layer of porous granular material, at least 6 in. (150 mm) in thickness, shall be placed below the elevation of the bottom of the box. The porous granular material shall extend at least 2 ft (600 mm) beyond each side of the box. The precast concrete box culvert shall be laid according to the applicable requirements of Article 542.04(d). After installation, the interior and exterior joint gap between precast concrete box culvert sections shall be a maximum of 1 1/2 in. (38 mm).”

Add the following after the seventh paragraph of Article 540.06 of the Standard Specifications:

“Precast concrete box culverts with skews greater than 30 degrees and having design covers less than or equal to 5 feet are not covered by the standard design table shown in ASTM C 1577. The design table provided herein is provided to address this design range. The same notes, reinforcement configurations, clearances, and requirements of ASTM C 1577 apply to this special design table. A box designated 7 x 6 x 8 indicates a span of 7 ft, a rise of 6 ft, and top slab, bottom slab, walls and haunches of 8 in. unless otherwise noted on the tables.

3 ft by 2 ft by 4 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2*	0.168	0.900	0.295	0.096	0.269	0.168	0.853	0.144	
2<3	0.134	0.180	0.182	0.096					31
3-5	0.096	0.115	0.117	0.096					29

*top slab 7 in., bottom slab 6.0 in.

3 ft by 3 ft by 4 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2*	0.168	0.956	0.326	0.096	0.290	0.168	0.849	0.144	
2<3	0.101	0.214	0.218	0.096					31
3-5	0.096	0.136	0.140	0.096					31

*top slab 7.0 in., bottom slab 6.0 in.

4 ft by 2 ft by 5 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2*	0.204	0.790	0.262	0.120	0.268	0.180	0.846	0.144	
2<3	0.201	0.203	0.196	0.120					32
3-5	0.129	0.134	0.136	0.120					32

*top slab 7.5 in., bottom slab 6.0 in.

4 ft by 3 ft by 5 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2*	0.180	0.876	0.303	0.120	0.305	0.180	0.831	0.144	
2<3	0.160	0.245	0.238	0.120					38
3-5	0.120	0.161	0.165	0.120					35

*top slab 7.5 in., bottom slab 6.0 in.

4 ft by 4 ft by 5 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2*	0.180	0.927	0.334	0.120	0.327	0.180	0.822	0.144	
2<3	0.130	0.277	0.270	0.120					38
3-5	0.120	0.181	0.188	0.120					38

*top slab 7.5 in., bottom slab 6.0 in.

5 ft by 3 ft by 6 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2*	0.197	0.682	0.269	0.144	0.280	0.192	0.705	0.168	
2<3	0.206	0.259	0.246	0.144					37
3-5	0.144	0.180	0.179	0.144					35

*top slab 8.0 in., bottom slab 7.0 in.

5 ft by 4 ft by 6 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2*	0.192	0.735	0.299	0.144	0.307	0.192	0.693	0.168	
2<3	0.180	0.294	0.282	0.144					46
3-5	0.144	0.204	0.205	0.144					40

*top slab 8.0 in., bottom slab 7.0 in.

5 ft by 5 ft by 6 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2*	0.192	0.774	0.324	0.144	0.327	0.192	0.685	0.168	
2<3	0.155	0.322	0.312	0.144					45
3-5	0.144	0.224	0.228	0.144					45

*top slab 8.0 in., bottom slab 7.0 in.

6 ft by 3 ft by 7 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2*	0.270	0.566	0.257	0.168	0.263	0.192	0.575	0.168	
2<3	0.260	0.269	0.273	0.168					41
3-5	0.186	0.192	0.197	0.168					39

*top slab 8.0 in.

6 ft by 4 ft by 7 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2*	0.245	0.617	0.297	0.168	0.293	0.192	0.565	0.168	
2<3	0.225	0.305	0.313	0.168					42
3-5	0.168	0.220	0.227	0.168					41

*top slab 8.0 in.

6 ft by 5 ft by 7 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in. / ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2*	0.226	0.657	0.331	0.168	0.317	0.192	0.551	0.168	
2<3	0.198	0.338	0.348	0.168					59
3-5	0.168	0.242	0.252	0.168					48

*top slab 8.0 in.

6 ft by 6 ft by 7 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2*	0.208	0.692	0.363	0.168	0.337	0.192	0.540	0.168	
2<3	0.176	0.364	0.379	0.168					52
3-5	0.168	0.261	0.275	0.168					52

*top slab 8.0 in.

7 ft by 4 ft by 8 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2	0.339	0.599	0.372	0.192	0.271	0.192	0.697	0.192	
2<3	0.287	0.335	0.342	0.192					44
3-5	0.206	0.241	0.248	0.192					42

7 ft by 5 ft by 8 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2	0.317	0.637	0.417	0.192	0.293	0.192	0.684	0.192	
2<3	0.256	0.370	0.381	0.192					49
3-5	0.192	0.266	0.276	0.192					46

7 ft by 6 ft by 8 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2	0.296	0.672	0.458	0.192	0.312	0.192	0.658	0.192	
2<3	0.230	0.401	0.416	0.192					59
3-5	0.192	0.288	0.302	0.192					55

7 ft by 7 ft by 8 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2	0.276	0.703	0.496	0.192	0.330	0.192	0.653	0.192	
2<3	0.210	0.428	0.447	0.192					59
3-5	0.192	0.307	0.326	0.192					59

8 ft by 4 ft by 8 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2	0.397	0.510	0.400	0.192	0.283	0.192	0.568	0.192	
2<3	0.399	0.415	0.423	0.192					45
3-5	0.285	0.298	0.306	0.192					45

8 ft by 5 ft by 8 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2	0.368	0.555	0.446	0.192	0.305	0.192	0.559	0.192	
2<3	0.360	0.458	0.470	0.192					48
3-5	0.259	0.328	0.340	0.192					45

8 ft by 6 ft by 8 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2	0.342	0.596	0.488	0.192	0.325	0.192	0.556	0.192	
2<3	0.328	0.496	0.512	0.192					56
3-5	0.237	0.355	0.371	0.192					50

8 ft by 7 ft by 8 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2	0.319	0.633	0.527	0.192	0.343	0.192	0.555	0.192	
2<3	0.301	0.529	0.551	0.192					65
3-5	0.219	0.379	0.399	0.192					61

8 ft by 8 ft by 8 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2	0.297	0.668	0.565	0.192	0.360	0.192	0.531	0.192	
2<3	0.280	0.560	0.587	0.192					65
3-5	0.204	0.400	0.427	0.192					65

9 ft by 5 ft by 9 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2	0.361	0.411	0.416	0.216	0.275	0.216	0.465	0.216	
2<3	0.425	0.484	0.496	0.216					49
3-5	0.306	0.348	0.360	0.216					49

9 ft by 6 ft by 9 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in. / ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2	0.335	0.439	0.455	0.216	0.294	0.216	0.467	0.216	
2<3	0.390	0.524	0.541	0.216					55
3-5	0.282	0.376	0.393	0.216					52

9 ft by 7 ft by 9 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in. / ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2	0.313	0.464	0.491	0.216	0.311	0.216	0.453	0.216	
2<3	0.360	0.561	0.583	0.216					64
3-5	0.262	0.402	0.423	0.216					58

9 ft by 8 ft by 9 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2	0.286	0.488	0.514	0.216	0.327	0.216	0.454	0.216	
2<3	0.336	0.594	0.621	0.216					72
3-5	0.244	0.426	0.453	0.216					73

9 ft by 9 ft by 9 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2	0.274	0.511	0.557	0.216	0.342	0.216	0.452	0.216	
2<3	0.316	0.625	0.659	0.216					72
3-5	0.231	0.448	0.481	0.216					72

10 ft by 5 ft by 10 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2	0.370	0.393	0.392	0.240	0.263	0.240	0.240	0.240	
2<3	0.492	0.509	0.522	0.240					52
3-5	0.354	0.366	0.379	0.240					52

10 ft by 6 ft by 10 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2	0.348	0.420	0.432	0.240	0.282	0.240	0.418	0.240	
2<3	0.455	0.552	0.570	0.240					56
3-5	0.329	0.397	0.414	0.240					52

10 ft by 7 ft by 10 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2	0.321	0.445	0.463	0.240	0.298	0.240	0.240	0.240	
2<3	0.423	0.591	0.614	0.240					59
3-5	0.307	0.425	0.447	0.240					56

10 ft by 8 ft by 10 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in. / ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2	0.301	0.469	0.496	0.240	0.314	0.240	0.240	0.240	
2<3	0.394	0.627	0.655	0.240					72
3-5	0.288	0.451	0.478	0.240					66

10 ft by 9 ft by 10 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2	0.284	0.492	0.527	0.240	0.329	0.240	0.240	0.240	
2<3	0.371	0.660	0.694	0.240					79
3-5	0.272	0.475	0.508	0.240					85

10 ft by 10 ft by 10 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2	0.272	0.514	0.559	0.240	0.344	0.240	0.240	0.240	
2<3	0.353	0.691	0.732	0.240					79
3-5	0.259	0.497	0.537	0.240					79

11 ft by 4 ft by 11 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2	0.414	0.341	0.333	0.264	0.264	0.264	0.264	0.264	
2<3	0.609	0.481	0.491	0.264					60
3-5	0.436	0.348	0.357	0.264					56

11 ft by 6 ft by 11 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2	0.356	0.399	0.407	0.264	0.265	0.264	0.264	0.264	
2<3	0.521	0.580	0.597	0.264					56
3-5	0.377	0.418	0.435	0.264					56

11 ft by 8 ft by 11 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2	0.314	0.449	0.471	0.264	0.298	0.264	0.264	0.264	
2<3	0.457	0.659	0.687	0.264					67
3-5	0.333	0.475	0.502	0.264					63

11 ft by 10 ft by 11 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2	0.285	0.494	0.532	0.264	0.328	0.264	0.264	0.264	
2<3	0.409	0.727	0.769	0.264					86
3-5	0.300	0.524	0.565	0.264					86

11 ft by 11 ft by 11 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2	0.276	0.516	0.562	0.264	0.342	0.264	0.264	0.264	
2<3	0.391	0.758	0.808	0.264					86
3-5	0.289	0.548	0.596	0.264					86

12 ft by 4 ft by 12 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2	0.426	0.329	0.316	0.288	0.288	0.288	0.321	0.288	
2<3	0.682	0.503	0.512	0.288					64
3-5	0.489	0.364	0.373	0.288					60

12 ft by 6 ft by 12 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2	0.367	0.385	0.387	0.288	0.288	0.288	0.320	0.288	
2<3	0.590	0.606	0.624	0.288					60
3-5	0.427	0.438	0.456	0.288					56

12 ft by 8 ft by 12 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2	0.326	0.435	0.449	0.288	0.288	0.288	0.288	0.288	
2<3	0.521	0.690	0.719	0.288					67
3-5	0.381	0.499	0.527	0.288					64

12 ft by 10 ft by 12 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2	0.298	0.481	0.507	0.288	0.305	0.288	0.288	0.288	
2<3	0.467	0.762	0.804	0.288					93
3-5	0.344	0.551	0.592	0.288					79

12 ft by 12 ft by 12 in.

Design Earth Cover, ft.	Circumferential Reinforcement Areas, sq in./ ft.								"M", in.
	As1	As2	As3	As4	As5	As6	As7	As8	
0<2	0.288	0.525	0.566	0.288	0.333	0.288	0.288	0.288	
2<3	0.431	0.827	0.886	0.288					93
3-5	0.320	0.599	0.656	0.288					93"

Designer Note: Insert into contracts with precast concrete or cast-in-place concrete box culverts with skew ≤ 30 degrees regardless of the design fill and also skews > 30 degrees with design fill > 5 feet.

CONCRETE BOX CULVERTS WITH SKEWS ≤ 30 DEGREES REGARDLESS OF DESIGN FILL AND SKEWS > 30 DEGREES WITH DESIGN FILLS > 5 FEET (BDE)

Effective: April 1, 2012

Revise the second paragraph of Article 540.04 of the Standard Specifications to read:

“Unless otherwise noted on the plans, the Contractor shall have the option, when a cast-in-place concrete box culvert is specified, of constructing the box culvert using precast box culvert sections when the design cover is 6 in. (150 mm) minimum. The precast box culvert sections shall be designed for the same design cover shown on the plans for cast-in-place box culvert; shall be of equal or larger size opening, and shall satisfy the design requirements of ASTM C 1577.”

Revise the fourth paragraph of Article 540.06 of the Standard Specifications to read:

“The excavation and backfilling for precast concrete box culverts shall be according to the requirements of Section 502, except where the design fill is less than or equal to 8 ft (2.4 m), or the design fill is less than the clear span of the box. In these cases ASTM C 1577 requires a select granular backfill (porous granular material) over the box. If a porous granular backfill is required but is not detailed on the plans for the culvert(s), the Contractor shall have the option of either furnishing porous granular backfill where required to satisfy ASTM C 1577, or submitting an alternate design, sealed by an Illinois licensed Structural Engineer, which precludes the use of a porous granular backfill. In addition for all precast boxes a layer of porous granular material, at least 6 in. (150 mm) in thickness, shall be placed below the elevation of the bottom of the box. The porous granular material shall extend at least 2 ft (600 mm) beyond each side of the box. The precast concrete box culvert shall be laid according to the applicable requirements of Article 542.04(d). After installation, the interior and exterior joint gap between precast concrete box culvert sections shall be a maximum of 1 1/2 in. (38 mm).”

Designer Note: Insert in contracts where its use has been approved by BMPR. Consult Don Hoffman prior to contacting BMPR. This item is for work zone traffic control.

PAVEMENT MARKING TAPE TYPE IV (BDE)

Effective: April 1, 2012

Revise Article 703.02 of the Standard Specifications to read:

“703.02 Materials. Materials shall be according to the following.

(a) Pavement Marking Tape, Type I and Type III	1095.06
(b) Paint Pavement Markings	1095.02
(c) Pavement Marking Tape, Type IV	1095.11”

Revise the second paragraph of Article 703.05 of the Standard Specifications to read:

“Type I marking tape or paint shall be used at the option of the Contractor, except paint shall not be applied to the final wearing surface unless authorized by the Engineer for late season applications where tape adhesion would be a problem. Type III or Type IV marking tape shall be used on the final wearing surface when the temporary pavement marking will conflict with the permanent pavement marking such as on tapers, crossovers and lane shifts.”

Revise the third paragraph of Article 703.07 of the Standard Specifications to read:

“When Pavement Marking Tape, Type III or Pavement Marking Tape, Type IV is specified in the contract other than on a Standard, the work will be paid for at the contract unit price per foot (meter) for PAVEMENT MARKING TAPE, TYPE III or PAVEMENT MARKING TAPE, TYPE IV of the line width specified and at the contract unit price per square foot (square meter) for PAVEMENT MARKING TAPE, TYPE III - LETTERS AND SYMBOLS or PAVEMENT MARKING TAPE, TYPE IV – LETTERS AND SYMBOLS.”

Add the following to Section 1095 of the Standard Specifications:

“1095.11 Pavement Marking Tape, Type IV. The temporary, preformed, patterned markings shall consist of a white or yellow tape with wet retroreflective media incorporated to provide immediate and continuing retroreflection during both wet and dry conditions. The tape shall be manufactured without the use of heavy metals including lead chromate pigments or other similar, lead-containing chemicals.

The white and yellow Type IV marking tape shall meet the Type III requirements of Article 1095.06 and the following.

- (a) Composition. The retroreflective pliant polymer pavement markings shall consist of a mixture of high-quality polymeric materials, pigments and glass beads distributed throughout its base cross-sectional area, with a layer of wet retroreflective media bonded to a durable polyurethane topcoat surface. The patterned surface shall have approximately 40% ± 10% of the surface area raised and presenting a near vertical face to traffic from any direction. The channels between the raised areas shall be substantially free of exposed beads or particles.

(b) Retroreflectance. The white and yellow markings shall meet the following for initial dry and wet retroreflectance.

(1) Dry Retroreflectance. Dry retroreflectance shall be measured under dry conditions according to ASTM D4061 and meet the values described in Article 1095.06 for Type III tape.

(2) Wet Retroreflectance. Wet retroreflectance shall be measured under wet conditions according to ASTM E2177 and meet the values shown in the following table.

Wet Retroreflectance, Initial R_L	
Color	R_L 1.05/88.76
White	300
Yellow	200

(c) Color. The material shall meet the following requirements for daylight reflectance and color, when tested, using a color spectrophotometer with 45 degrees circumferential/zero degree geometry, illuminant D65, and a two degree observer angle. The color instrument shall measure the visible spectrum from 380 to 720 nm with a wavelength measurement interval and spectral bandpass of 10 nm.

Color	Daylight Reflectance %Y
White	65 minimum
*Yellow	36-59

*Shall match Federal 595 Color No. 33538 and the chromaticity limits as follows.

x	0.490	0.475	0.485	0.530
y	0.470	0.438	0.425	0.456

(d) Skid Resistance. The surface of the markings shall provide an average minimum skid resistance of 50 BPN when tested according to ASTM E303.

(e) Sampling, Testing, Acceptance, and Certification. Prior to approval and use of the wet reflective, temporary, removable pavement marking tape, the manufacturer shall submit a notarized certification from an independent laboratory, together with the results of all tests, stating that the material meets the requirements as set forth herein. The certification test report shall state the lot tested, manufacturer's name, and date of manufacture.

After approval by the Department, samples and certification by the manufacturer shall be submitted for each batch used. The manufacturer shall submit a certification stating that the material meets the requirements as set forth herein and is essentially identical to the material sent for qualification. The certification shall state the lot tested, manufacturer's name, and date of manufacture.

All costs of testing (other than tests conducted by the Department) shall be borne by the manufacturer."

Designer Note: Insert into all contracts utilizing modified urethane pavement marking. This special replaces District Special 780.01 for "Urethane Pavement Marking."

MODIFIED URETHANE PAVEMENT MARKING (BDE)

Effective: April 1, 2012

Add the following to Article 780.02 of the Standard Specifications:

"(h) Modified Urethane Pavement Marking 1095.09"

Add the following to Article 780.03 of the Standard Specifications:

"(e) Modified Urethane 1105.04"

Revise Article 780.11 of the Standard Specifications to read:

"780.11 Modified Urethane. The pavement shall be cleaned of all dirt, grease, glaze, or any other material that would reduce the adhesion of the markings with minimum or no damage to the pavement. New PCC pavements shall be blast-cleaned to remove all curing compounds. New asphalt and seal coated shall be in place a minimum of two weeks prior to marking applications.

Markings shall be applied on the same calendar day that the pavement surface is cleaned. If this cannot be accomplished, the surface shall be re-cleaned prior to applying the markings. Existing pavement markings shall be at least 90 percent removed. No markings shall be applied until the Engineer approves the cleaning.

Widths, lengths, and shapes of the cleaned surface shall be prepared wider than the modified urethane pavement marking material to be applied, such that a prepared area is on all sides of the urethane pavement marking material after application.

The Contractor shall notify the Engineer 72 hours prior to the placement of the markings in order than an inspector can be present during the operation. At the time of this notification, the Contractor shall indicate the manufacturer and lot numbers of urethane and reflective media that will be used. The Engineer will ensure that the approved lot numbers appear on the material package.

The pavement markings shall be applied during conditions of dry weather and subsequently dry pavement surfaces at a minimum uniform wet thickness of 25 mils (0.64 mm) according to the manufacturer's installation instructions. The application and combination of reflective media (glass beads and/or reflective elements) shall be applied at a rate specified by the manufacturer. At the time of installation the pavement surface temperature shall be 40 °F (5 °C) and rising and the ambient temperature shall be 35 °F (2 °C) and rising. The pavement surface temperature and the ambient temperatures shall be determined and documented before the start of each of marking operation. The pavement markings shall not be applied if the pavement shows any visible signs of moisture or it is anticipated that moisture, such as rain showers, may occur during the installation and curing periods."

Revise Article 780.12 of the Standard Specifications to read:

“780.12 Inspection. The epoxy, thermoplastic, preformed thermoplastic, preformed plastic Type B or C, polyurea, and modified urethane pavement markings will be inspected following installation, but no later than October 15 for preformed plastic markings, November 1 for thermoplastic and preformed thermoplastic markings, and December 15 for epoxy, polyurea, and modified urethane markings. In addition, they will be inspected following a winter performance period that extends 180 days from November 1.

Within 15 calendar days after the end of the winter performance period, a final performance inspection will be made. Final acceptance requirements are as follows.

- (a) Lane lines: 90 percent intact by area of each individual dashed line segment.
- (b) Crosswalks, stop lines, arrows, and words: 90 percent intact by area of each individual line, symbol, or letter.
- (c) Center lines, edge lines, gore markings, and channelizing lines: 90 percent intact by area measured over any 10 ft (3 m) length of any individual line regardless of width.
- (d) Entire project: measured in its entirety according to (a), (b), and (c) above, the entire project shall be 95 percent intact.

Upon completion of the final performance inspection, or after satisfactory completion of any necessary correction, the Engineer will notify the Contractor, in writing, of the date of such final performance inspection and release him/her from further performance responsibility.

If this inspection discloses any work, in whole or in part, which does not meet the inspection requirements, the Contractor shall, within 30 calendar days, completely repair or replace such work to the satisfaction of the Engineer.

This performance inspection and performance acceptance of the epoxy, thermoplastic, preformed thermoplastic, preformed plastic Type B and C pavement, polyurea, and modified urethane markings shall not delay acceptance of the entire project and final payment due if the Contractor requires and receives from the subcontractor a third party "performance" bond naming the Department as obligee in the full amount of all pavement marking quantities listed in the contract, multiplied by the contract unit price. The bond shall be executed prior to acceptance and final payment of the non-pavement marking items and shall be in full force and effect until final performance inspection and performance acceptance of the epoxy, thermoplastic, preformed thermoplastic, preformed plastic, polyurea, and modified urethane pavement markings. Execution of the third party bond shall be the option of the Contractor.”

Revise Article 780.13 of the Standard Specifications to read:

“780.13 Method of Measurement. This work will be measured for payment as follows.

- (a) Contract Quantities. The requirements for the use of contract quantities shall be according to Article 202.07(a).
- (b) Measured Quantities. Lines will be measured for payment in place in feet (meters). Double yellow lines will be measured as two separate lines.

Words and symbols shall conform to the sizes and dimensions specified in the Illinois Manual on Uniform Traffic Control Devices and Standard 780001 and will be measured based on the total areas indicated in Table 1 or as specified in the plans.

Removal of existing pavement markings will be measured for payment according to Article 783.05.”

Add the following to Section 780 of the Standard Specifications:

“780.14 Basis of Payment. This work will be paid for at the contract unit prices per foot (meter) of applied line width, as specified, for THERMOPLASTIC PAVEMENT MARKING - LINE; PAINT PAVEMENT MARKING - LINE; EPOXY PAVEMENT MARKING - LINE; PREFORMED PLASTIC PAVEMENT MARKING - LINE - TYPE B, C, or B - INLAID; PREFORMED THERMOPLASTIC PAVEMENT MARKING – LINE; POLYUREA PAVEMENT MARKING TYPE I – LINE; POLYUREA PAVEMENT MARKING TYPE II - LINE; MODIFIED URETHANE PAVEMENT MARKING – LINE; and/or per square foot (square meter) for THERMOPLASTIC PAVEMENT MARKING - LETTERS AND SYMBOLS; PAINT PAVEMENT MARKING - LETTERS AND SYMBOLS; EPOXY PAVEMENT MARKING - LETTERS AND SYMBOLS; PREFORMED PLASTIC PAVEMENT MARKING - TYPE B, C, or B - INLAID - LETTERS AND SYMBOLS; PREFORMED THERMOPLASTIC PAVEMENT MARKING - LETTERS AND SYMBOLS; MODIFIED URETHANE PAVEMENT MARKING – LETTERS AND SYMBOLS.

When the Contractor has the option of applying Permanent Pavement Marking it shall be Thermoplastic, Preformed Plastic (Type B, C, or B - Inlaid), Epoxy, Preformed Thermoplastic, Polyurea, or Modified Urethane Pavement Markings. It will be paid for at the contract unit price per foot (meter) of applied line for PERMANENT PAVEMENT MARKING - LINE 4 (100), 5 (125), 6 (150), 8 (200), 12 (300), 16 (400), or 24 in. (600 mm) and per square foot (square meter) for PERMANENT PAVEMENT MARKING - LETTERS AND SYMBOLS.

Temporary pavement markings placed in lieu of permanent will be paid for according to Article 703.07.

Removal of existing pavement markings will be paid for according to Article 783.06.

*TABLE 1

LETTERS
sq ft (sq m)

Size	A	B	C	D	E	F	G	H	I
6 ft (1.8 m)	3.1 (0.28)	4.0 (0.37)	2.7 (0.25)	3.4 (0.31)	3.3 (0.31)	2.6 (0.24)	3.3 (0.31)	3.4 (0.31)	1.5 (0.14)
8 ft (2.4 m)	5.5 (0.51)	7.1 (0.66)	4.8 (0.45)	6.1 (0.57)	5.9 (0.55)	4.7 (0.44)	5.8 (0.54)	6.0 (0.56)	2.6 (0.24)

Size	J	K	L	M	N	O	P	Q	R
6 ft (1.8 m)	2.1 (0.2)	3.1 (0.28)	2.2 (0.20)	4.2 (0.39)	4.0 (0.37)	3.4 (0.31)	3.0 (0.28)	3.6 (0.33)	3.6 (0.33)
8 ft (2.4 m)	3.7 (0.34)	5.7 (0.53)	3.8 (0.45)	7.4 (0.69)	7.1 (0.65)	6.0 (0.56)	5.3 (0.49)	6.3 (0.59)	6.3 (0.59)

Size	S	T	U	V	W	X	Y	Z
6 ft (1.8 m)	3.2 (0.30)	2.2 (0.20)	3.2 (0.30)	2.7 (0.25)	4.2 (0.39)	2.7 (0.25)	2.2 (0.20)	2.9 (0.26)
8 ft (2.4 m)	5.7 (0.53)	3.8 (0.35)	5.6 (0.52)	4.8 (0.45)	7.3 (0.68)	4.8 (0.45)	3.9 (0.36)	5.1 (0.47)

NUMBERS
sq ft (sq m)

Size	1	2	3	4	5
6 ft (1.8 m)	1.5 (0.14)	3.3 (0.31)	3.3 (0.31)	2.9 (0.26)	3.5 (0.33)
8 ft (2.4 m)	2.6 (0.24)	5.8 (0.54)	5.8 (0.54)	5.1 (0.47)	6.1 (0.57)

Size	6	7	8	9	0
6 ft (1.8 m)	3.5 (0.33)	2.2 (0.20)	3.8 (0.35)	3.5 (0.33)	3.4 (0.31)
8 ft (2.4 m)	6.2 (0.58)	3.8 (0.35)	6.7 (0.62)	6.2 (0.58)	6.0 (0.56)

SYMBOLS

Symbol	Large Size sq ft (sq m)	Small Size sq ft (sq m)
Through Arrow	11.5 (1.07)	6.5 (0.60)
Left or Right Arrow	15.6 (1.47)	8.8 (0.82)
2 Arrow Combination Left (or Right) and Through	26.0 (2.42)	14.7 (1.37)
3 Arrow Combination Left, Right, and Through	38.4 (3.56)	20.9 (1.94)
Lane Drop Arrow	41.5 (3.86)	--
Wrong Way Arrow	24.3 (2.26)	--
Railroad "R" 6 ft (1.8 m)	3.6 (0.33)	--
Railroad "X" 20 ft (6.1 m)	54.0 (5.02)	--
Handicapped Symbol	4.6 (0.43)	--

*Table applies to all types of pavement marking materials."

Add the following Section to Section 1095 of the Standard Specifications:

"1095.09 Modified Urethane Pavement Marking. The modified urethane pavement marking material shall consist of a homogenous blend of modified urethane resins and pigments designed to provide a simple volumetric mixing ratio of two components (must be two volumes of Part A to one volume of Part B). No volatile solvent or fillers will be allowed.

- (a) Pigmentation. The pigment content by weight (mass) of Part A shall be determined by low temperature ashing according to ASTM D 3723. The pigment content shall not vary more than \pm two percent from the pigment content of the original qualified paint.

White pigment shall be Titanium Dioxide meeting ASTM D 476 Type II, Rutile.

Yellow pigment shall be Organic Yellow containing no heavy metals.

- (b) Environmental. Upon heating to application temperature, the material shall not exude fumes which are toxic or injurious persons or property when handled according to

manufacturer specifications. The modified urethane pavement marking material compositions shall not contain free isocyanate functionality.

- (c) Daylight Reflectance. The daylight directional reflectance of the cured modified urethane material (without reflective media) shall be a minimum of 80 percent (white) and 50 percent (yellow) relative to magnesium oxide when tested using a color spectrophotometer with a 45 degree circumferential / zero degrees geometry, illuminant C, and two degrees observer angle. The color instrument shall measure the visible spectrum from 380 to 720 nm with a wavelength measurement interval and spectral bandpass of 10 nm. In addition, the color of the yellow modified urethane shall visually match Color Number 33538 of Federal Standard 595a with chromaticity limits as follows:

x	0.490	0.475	0.485	0.539
y	0.470	0.438	0.425	0.456

- (d) Weathering Resistance. The modified urethane, when mixed in the proper ratio and applied at 14 to 16 mils (0.35 to 0.41 mm) wet film thickness to an aluminum alloy panel (Federal Test Std. No. 141, Method 2013) and allowed to cure for 72 hours at room temperature, shall be subjected to accelerated weathering for 75 hours. The accelerated weathering shall be completed by using the light and water exposure apparatus (fluorescent UV – condensation type) and tested according to ASTM G 53.

The cycle shall consist of four hours UV exposure at 122 °F (50 °C) and four hours of condensation at 104 °F (40 °C). UVB 313 bulbs shall be used. At the end of the exposure period, the material shall show no substantial change in color or gloss.

- (e) Drying Time. The modified urethane material, when mixed in the proper ratio and applied at 14 to 16 mils (0.35 to 0.41 mm) wet film thickness and with the proper saturation of glass beads, shall exhibit a no-tracking time of four minutes or less when tested according to ASTM D 711.

- (f) Adhesion. The catalyzed modified urethane pavement marking materials when applied to a 4 x 4 x 2 in. (100 x 100 x 50 mm) concrete block shall have a degree of adhesion which results in a 100 percent concrete failure in the performance of this test.

The concrete block shall be brushed on one side and have a minimum strength of 3,500 psi (24,100 kPa). A 2 in. (50 mm) square film of the mixed modified urethane shall be applied to the brushed surface and allowed to cure for 72 hours at room temperature. A 2 in. (50 mm) cube shall be affixed to the surface of the modified urethane by means of an epoxy glue. After the glue has cured for 24 hours, the modified urethane specimen shall be placed on a dynamic testing machine in such a fashion so that the specimen block is in a fixed position and the 2 in. (50 mm) cube (glued to the modified urethane surface) is attached to the dynamometer head. Direct upward pressure shall be slowly applied until the modified urethane system fails. The location of the break and the amount of concrete failure shall be recorded.

- (g) Hardness. The modified urethane marking materials, when tested according to ASTM D 2240, shall have a Shore D Hardness greater than 75. Films shall be cast on a rigid substrate at 14 to 16 mils (0.35 to 0.41 mm) in thickness and allowed to cure at room temperature for 72 hours before testing.

- (h) Abrasion. The abrasion resistance shall be evaluated according to ASTM D 4060 using a Taber Abrader with a 1,000 gram load and CS 17 wheels. The duration of test shall be 1,000 cycles. The loss shall be calculated by difference and be less than 80. The tests

shall be run on cured samples of modified urethane material which have been applied at a film thickness of 14 to 16 mils (0.35 to 0.41) to code S-16 stainless steel plates. The films shall be allowed to cure at room temperature for at least 72 hours and not more than 96 hours before testing.

- (i) Tensile. When tested according to ASTM D 638, the modified urethane pavement marking materials shall have an average tensile strength of not less than 6000 psi (41,300 kPa). The Type IV specimens shall be pulled at a rate of 1/4 in. (6.3 mm) per minute by a suitable dynamic testing machine. The samples shall be allowed to cure at 75 °F ± 2 °F (24 °C ± 1 °C) for a minimum of 24 hours and a maximum of 72 hours prior to performing the indicated tests.
 - (j) Compressive Strength. When tested according to ASTM D 695, the catalyzed modified urethane pavement marking materials shall have a compressive strength of not less than 12,000 psi (83,000 kPa). The cast sample shall be conditioned at 75 °F ± 2 °F (24 °C ± 1 °C) for a minimum of 72 hours before performing the indicated tests. The rate of compression of these samples shall be no more than 1/4 in. (6.3 mm) per minute.
 - (k) Glass Beads. The glass beads shall meet the requirements of Article 1095.04(m) and Article 1095.07 for first drop and second drop glass beads.
 - (l) Packaging. The material shall be shipped to the jobsite in substantial containers and shall be plainly marked with the manufacturer's name and address, the name and color of the material, date of manufacture and batch number.
 - (m) Verification. Prior to approval and use of the modified urethane pavement marking materials, the manufacturer shall submit a notarized certification of an independent laboratory, together with the results of all tests, stating these materials meet the requirements as set forth herein. The certification test report shall state the lot tested, manufacturer's name, brand name of modified urethane and date of manufacture. The certification shall be accompanied by 1 pt (1/2 L) samples each of Part A and Part B. Samples shall be sent in the appropriate volumes for complete mixing of Part A and Part B.
- After approval by the Department, certification by the modified urethane manufacturer shall be submitted for each batch used. New independent laboratory certified test results and samples for testing by the Department shall be submitted any time the manufacturing process or paint formulation is changed.
- (n) Acceptance samples. Acceptance samples shall consist of 1 pt (1/2 L) samples of Part A and Part B, of each lot of paint. Samples shall be sent in the appropriate volumes for complete mixing of Part A and Part B. The samples shall be submitted to the Department for testing, together with a manufacturer's certification. The certification shall state the formulation for the lot represented is essentially identical to that used for qualification testing. All, acceptance samples will be taken by a representative of the Illinois Department of Transportation. The modified urethane pavement marking materials shall not be used until tests are completed and they have met the requirements as set forth herein.
 - (o) Material Retainage. The manufacturer shall retain the test sample for a minimum of 18 months."

Add the following to Section 1105 of the Standard Specifications:

“1105.04 Modified Urethane. The modified urethane pavement marking compounds shall be applied through equipment specifically designed to precisely meter the two components in the ratio of 2:1 and approved by the manufacturer of the material. The equipment shall produce the required amount of heat at the mixing head and gun tip and maintain those temperatures within the tolerances specified. The equipment shall also have as an integral part of the gun carriage, a high pressure air spray capable of cleaning the pavement immediately prior to the marking application.

The equipment shall be capable of spraying both yellow and white modified urethane, according to the manufacturer's recommended proportions and be mounted on a truck of sufficient size and stability with an adequate power source to produce lines of uniform dimensions and prevent application failure. The truck shall have at least two urethane tanks each of 110 gal (415 L) minimum capacity and shall be equipped with hydraulic systems. It shall be capable of placing stripes on the left and right sides and placing two lines on a three-line system simultaneously with either line in a solid or intermittent pattern, in yellow or white, and applying glass beads by the double drop pressurized bead system. The system shall apply both the first drop glass beads and the second drop glass beads at a rate of 1.2 kg/L (10 lb/gal). The equipment shall be equipped with pressure gauges for each proportioning pump. All guns shall be in full view of operators at all times. The equipment shall have a metering device to register the accumulated installed quantities for each gun, each day. Each vehicle shall include at least one operator who shall be a technical expert in equipment operations and urethane application techniques. Certification of equipment shall be provided at the preconstruction conference.”

Designer Note: This special is for an alternative preformed plastic pavement marking that has better retro reflectivity in wet conditions. Only use this special in contracts where the use of this material has been approved by BMRP.

PREFORMED PLASTIC PAVEMENT MARKING TYPE D - INLAID (BDE)

Effective: April 1, 2012

Revise subparagraph (c) and add subparagraph (i) to Article 780.02 of the Standard Specifications:

- “(c) Preformed Plastic Pavement Markings, Type B and Type C 1095.03
 (i) Preformed Plastic Pavement Marking, Type D 1095.10”

Revise the first paragraph of Article 780.07(a) of the Standard Specifications to read:

- “(a) Type B or D - Inlaid Application. On freshly placed HMA, the inlaid markings shall be applied before final compaction and when the pavement temperature has cooled to approximately 150 °F (65 °C) and when, in the opinion of the Engineer, the pavement is acceptable for vehicular traffic.”

Revise the first paragraph of Article 780.11 of the Standard Specifications to read:

“780.11 Inspection. The epoxy, thermoplastic, preformed thermoplastic, preformed plastic Type B, C, or D, and polyurea pavement markings will be inspected following installation, but no later than October 15 for preformed plastic markings, November 1 for thermoplastic and preformed thermoplastic markings, and December 15 for epoxy and polyurea markings. In addition, they will be inspected following a winter performance period that extends 180 days from November 1.”

Revise the ninth paragraph of Article 780.11 of the Standard Specifications to read:

“This performance inspection and performance acceptance of the epoxy, thermoplastic, preformed thermoplastic, preformed plastic Type B, C, or D, and polyurea markings shall not delay acceptance of the entire project and final payment due if the Contractor requires and receives from the subcontractor a third party "performance" bond naming the Department as obligee in the full amount of all pavement marking quantities listed in the contract, multiplied by the contract unit price. The bond shall be executed prior to acceptance and final payment of the non-pavement marking items and shall be in full force and effect until final performance inspection and performance acceptance of the epoxy, thermoplastic, preformed thermoplastic, preformed plastic, and polyurea pavement markings. Execution of the third party bond shall be the option of the Contractor.”

Revise the first paragraph of Article 780.13 of the Standard Specifications to read:

“780.13 Basis of Payment. This work will be paid for at the contract unit prices per foot (meter) of applied line width, as specified, for THERMOPLASTIC PAVEMENT MARKING - LINE; PAINT PAVEMENT MARKING - LINE; EPOXY PAVEMENT MARKING - LINE; PREFORMED PLASTIC PAVEMENT MARKING - LINE - TYPE B, C, B - INLAID, or D - INLAID; PREFORMED THERMOPLASTIC PAVEMENT MARKING - LINE, POLYUREA PAVEMENT MARKING TYPE I - LINE, POLYUREA PAVEMENT MARKING TYPE II - LINE;

and/or per square foot (square meter) for THERMOPLASTIC PAVEMENT MARKING - LETTERS AND SYMBOLS; PAINT PAVEMENT MARKING - LETTERS AND SYMBOLS; EPOXY PAVEMENT MARKING - LETTERS AND SYMBOLS; PREFORMED PLASTIC PAVEMENT MARKING - TYPE B, C, B – INLAID, or D - INLAID - LETTERS AND SYMBOLS; PREFORMED THERMOPLASTIC PAVEMENT MARKING - LETTERS AND SYMBOLS.”

Add the following to Section 1095 of the Standard Specifications:

“1095.10 Preformed Plastic Pavement Marking, Type D. The preformed patterned markings shall consist of a white or yellow tape with wet retroreflective media incorporated to provide immediate and continuing retroreflection during both wet and dry conditions. The pavement marking shall be manufactured without the use of heavy metals including lead chromate pigments or other similar, lead-containing chemicals.

The white and yellow preformed plastic pavement markings shall meet the Type B requirements of Article 1095.03(b), (c), (d), (e), (i), (l), (m), (n) and the following.

- (a) **Composition.** The pliant polymer pavement markings shall consist of a mixture of high-quality polymeric materials, pigments and glass beads distributed throughout its base cross-sectional area, with a layer of wet retroreflective media bonded to a durable polyurethane topcoat surface. The patterned surface shall have approximately 40% ± 10% of the surface area raised and presenting a near vertical face to traffic from any direction. The channels between the raised areas shall be substantially free of exposed beads or particles.
- (b) **Retroreflectance.** The white and yellow markings shall meet the following for initial dry and wet retroreflectance.
 - (1) **Dry Retroreflectance.** Dry retroreflectance shall be measured under dry conditions according to ASTM D4061 and meet the values described in Article 1095.03(l) for Type B.
 - (2) **Wet Retroreflectance.** Wet retroreflectance shall be measured under wet conditions according to ASTM E2177 and meet the values shown in the following table.

Wet Retroreflectance, Initial R_L

Color	R _L 1.05/88.76
White	300
Yellow	200

- (c) **Color.** The material shall meet the following requirements for daylight reflectance and color, when tested, using a color spectrophotometer with 45 degrees circumferential/zero degree geometry, illuminant D65, and a two degree observer angle. The color instrument shall measure the visible spectrum from 380 to 720 nm with a wavelength measurement interval and spectral bandpass of 10 nm.

Color	Daylight Reflectance %Y
White	65 minimum
*Yellow	36-59

*Shall match Federal 595 Color No. 33538 and the chromaticity limits as follows.

x	0.490	0.475	0.485	0.530
y	0.470	0.438	0.425	0.456

- (d) Sampling, Testing, Acceptance, and Certification. Prior to approval and use of the preformed pavement marking materials, the manufacturer shall submit a notarized certification from an independent laboratory, together with the results of all tests, stating that the material meets the requirements as set forth herein. The certification test report shall state the lot tested, manufacturer's name, and date of manufacture.

After approval by the Department, samples and certification by the manufacturer shall be submitted for each batch used. The manufacturer shall submit a certification stating that the material meets the requirements as set forth herein and is essentially identical to the material sent for qualification. The certification shall state the lot tested, manufacturer's name, and date of manufacture."

Designer Note: Insert into contracts with bridge approach slabs and bridge approach footings.

COARSE AGGREGATE IN BRIDGE APPROACH SLABS/FOOTINGS (BDE)

Effective: April 1, 2012

Revise the third paragraph of Article 1004.01(b) of the Standard Specifications to read:

“Aggregates used in Class BS concrete (except when poured on subgrade), Class PS concrete, and Class PC concrete (superstructure products only) shall contain no more than two percent by weight (mass) of deleterious materials. Deleterious materials shall include substances whose disintegration is accompanied by an increase in volume which may cause spalling of the concrete.”

Revise the first sentence of the first paragraph of Article 1004.02(f) of the Standard Specifications to read:

“(f) Freeze-Thaw Rating. When coarse aggregate is used to produce portland cement concrete for base course, base course widening, pavement, driveway pavement, sidewalk, shoulders, curb, gutter, combination curb and gutter, median, paved ditch, concrete superstructures on subgrade such as bridge approach slabs, concrete structures on subgrade such as bridge approach footings, or their repair using concrete, the gradation permitted will be determined from the results of the Department’s Freeze-Thaw Test (Illinois Modified AASHTO T 161).”

Designer Note: Insert into all contracts using precast or precast prestressed products.

SELF-CONSOLIDATING CONCRETE FOR PRECAST AND PRECAST PRESTRESSED PRODUCTS (BDE)

Effective: July 1, 2004

Revised: April 1, 2012

Description. This work shall consist of constructing precast and precast prestressed concrete products with self-consolidating concrete. The concrete shall be according to the special provision, "Portland Cement Concrete", except as modified herein.

Definition. Self-consolidating concrete is a flowable mixture that does not require mechanical vibration for consolidation.

Mix Design Criteria. Article 1020.04 shall apply, except as follows:

- (a) If the maximum cement factor is not specified for the product, it shall not exceed 7.05 cwt/cu yd (418 kg/cu m).
- (b) If the maximum allowable water/cement ratio is not specified for the product, it shall not exceed 0.44.
- (c) The slump requirements shall not apply.
- (d) The concrete mixture shall be uniformly graded, and information in the "Portland Cement Concrete Level III Technician Course – Manual of Instructions for Design of Concrete Mixtures" shall be used to develop the uniformly graded mix design. The coarse aggregate gradations shall be CA 11, CA 13, CA 14, CA 16, or a blend of these gradations. However, the final gradation when using a single coarse aggregate or combination of coarse aggregates shall have 100 percent pass the 1 in. (25 mm) sieve, and 95 percent pass the 3/4 in. (19 mm) sieve. The fine aggregate proportion shall be a maximum 50 percent by weight (mass) of the total aggregate used.
- (e) The slump flow range shall be 22 in. (560 mm) minimum to 28 in. (710 mm) maximum.
- (f) The visual stability index shall be a maximum of 1.
- (g) The J-ring value shall be a maximum of 2 in. (50 mm).
- (h) The L-box blocking ratio shall be a minimum of 80 percent.
- (i) The hardened visual stability index shall be a maximum of 1.

Test Methods. Illinois Test Procedures SCC-1, SCC-2, SCC-3, SCC-4, SCC-6, SCC-8, (Option C) and Illinois Modified AASHTO T 22, 23, 121, 141, 152, 196, and 309 shall be used for testing of self-consolidating mixtures.

Mixing Portland Cement Concrete. In addition to Article 1020.11, the mixing time for central-mixed concrete shall not be reduced as a result of a mixer performance test. Truck-mixed concrete shall be mixed in a truck mixer for a minimum of 100 revolutions.

The batch sequence, mixing speed, and mixing time shall be appropriate to prevent cement balls and mix foaming for central-mixed and truck-mixed concrete.

Concrete Placement for Precast Products. The maximum distance of horizontal flow from the point of deposit shall not exceed 25 ft (7.6 m) for precast products. However, when the maximum distance of horizontal flow from the point of discharge exceeds 15 ft (4.6 m), the dynamic segregation index shall be a maximum 10.0 percent. If the maximum is exceeded, the maximum distance of horizontal flow from the point of deposit will not be allowed to exceed 15 ft (4.6 m).

Concrete Placement for Precast Prestressed Products. The maximum distance of horizontal flow from the point of deposit shall not exceed 15 ft (4.6 m) for precast prestressed products. In addition, the placement operation shall be moved as required to ensure the leading edge of the flowing concrete does not exceed 15 ft (4.6 m). For a bed of beams, a single beam shall be completely filled with concrete before placement of concrete in the next beam. For deck beams with void tubes installed in place prior to the pour, the concrete shall be placed on one side of the void tube until the concrete flows completely under the void tube to the other side. Once this has been completed, the concrete placement operation may be moved to the other side.

Consolidation. Concrete shall be rodded with a piece of lumber, conduit, or vibrator if the material has lost its fluidity prior to placement of additional concrete. The vibrator will be permitted if it can be used in a manner that does not cause coarse aggregate separation from the mortar as determined by the Engineer. Any other method for restoring the fluidity of the concrete shall be approved by the Engineer.

Designer Note: Insert in all projects with cast-in-place concrete items. Also use Check Sheet #31, "QC/QA of Concrete Mixtures," when using this BDE special.

SELF-CONSOLIDATING CONCRETE FOR CAST-IN-PLACE CONSTRUCTION (BDE)

Effective: November 1, 2005

Revised: April 1, 2012

Description. This work shall consist of constructing cast-in-place items involving Class DS or SI concrete with self-consolidating concrete. The concrete shall be according to the special provision, "Portland Cement Concrete", except as modified herein.

Definition. Self-consolidating concrete is a flowable mixture that does not require mechanical vibration for consolidation.

Mix Design Criteria. Article 1020.04 shall apply, except as follows:

- (a) The slump requirements shall not apply.
- (b) The concrete mixture shall be uniformly graded, and information in the "Portland Cement Concrete Level III Technician Course – Manual of Instructions for Design of Concrete Mixtures" shall be used to develop the uniformly graded mix design. The coarse aggregate gradations shall be CA 11, CA 13, CA 14, CA 16, or a blend of these gradations. However, the final gradation when using a single coarse aggregate or combination of coarse aggregates shall have 100 percent pass the 1 in. (25 mm) sieve, and 95 percent pass the 3/4 in. (19 mm) sieve. The fine aggregate proportion shall be a maximum 50 percent by weight (mass) of the total aggregate used.
- (c) The slump flow range shall be 22 in. (560 mm) minimum to 28 in. (710 mm) maximum.
- (d) The visual stability index shall be a maximum of 1.
- (e) The J-ring value shall be a maximum of 2 in. (50 mm).
- (f) The L-box blocking ratio shall be a minimum of 80 percent.
- (g) The hardened visual stability index shall be a maximum of 1.

Test Methods. Illinois Test Procedures SCC-1, SCC-2, SCC-3, SCC-4, SCC-6, SCC-8 (Option C) and Illinois Modified AASHTO T 22, 23, 121, 141, 152, 177, 196, and 309 shall be used for testing of self-consolidating concrete mixtures.

Mixing Portland Cement Concrete. In addition to Article 1020.11, the mixing time for central-mixed concrete shall not be reduced as a result of a mixer performance test. Truck-mixed or shrink-mixed concrete shall be mixed in a truck mixer for a minimum of 100 revolutions.

The batch sequence, mixing speed, and mixing time shall be appropriate to prevent cement balls and mix foaming for central-mixed, truck-mixed, and shrink-mixed concrete.

Falsework and Forms. In addition to Articles 503.05 and 503.06 of the Standard Specifications, the Contractor shall ensure the design of the falsework and forms is adequate for the additional form pressure caused by the fluid concrete. Forms shall be tight to prevent leakage of fluid concrete.

When the form height for placing the self-consolidating concrete is greater than 10.0 ft (3.0 m), direct monitoring of form pressure shall be performed according to Illinois Test Procedure SCC-10. The monitoring requirement is a minimum, and the Contractor shall remain responsible for adequate design of the falsework and forms. The Contractor shall record the formwork pressure during concrete placement. This information shall be used by the Contractor to prevent the placement rate from exceeding the maximum formwork pressure allowed, to monitor the thixotropic change in the concrete during the pour, and to make appropriate adjustments to the mix design. This information shall be provided to the Engineer during the pour.

Placing and Consolidating. Concrete placement and consolidation shall be according to Article 503.07 of the Standard Specifications, except as follows:

Revise the third paragraph of Article 503.07 of the Standard Specifications to read:

“Open troughs and chutes shall extend as nearly as practicable to the point of deposit. The drop distance of concrete shall not exceed 5 ft (1.5 m). If necessary, a tremie shall be used to meet this requirement. The maximum distance of horizontal flow from the point of deposit shall be 25 ft (7.6 m). However, when the maximum distance of horizontal flow from the point of discharge exceeds 15 ft (4.6 m), the dynamic segregation index shall be a maximum 10.0 percent. If the maximum is exceeded, the maximum distance of horizontal flow from the point of deposit will not be allowed to exceed 15 ft (4.6 m). For drilled shafts, free fall placement will not be permitted.”

Delete the seventh, eighth, ninth, and tenth paragraphs of Article 503.07 of the Standard Specifications.

Add to the end of the eleventh paragraph of Article 503.07 of the Standard Specifications the following:

“Concrete shall be rodded with a piece of lumber, conduit, or vibrator if the material has lost its fluidity prior to placement of additional concrete. The vibrator will be permitted if it can be used in a manner that does not cause coarse aggregate separation from the mortar as determined by the Engineer. Any other method for restoring the fluidity of the concrete shall be approved by the Engineer.”

If the contract requires QC/QA for concrete, the following four sections shall supplement the special provision Quality Control/Quality Assurance of Concrete Mixtures. If QC/QC is not required, the following four sections shall be disregarded by the Contractor and the Engineer will perform QA testing as appropriate.

Quality Control by Contractor at Plant. The specified test frequencies for aggregate gradation, aggregate moisture, air content, unit weight/yield, and temperature shall be performed as indicated in the contract.

Slump flow, visual stability index, and J-ring or L-box tests shall be performed as needed to control production. The hardened visual stability index test will not be required to be performed at the plant.

Quality Control by Contractor at Jobsite. The specified test frequencies for air content, strength, and temperature shall be performed as indicated in the contract.

Slump flow, visual stability index, and J-ring or L-box tests shall be performed on the first two truck deliveries of the day, and every 50 cu yd (40 cu m) thereafter. The Contractor shall select either the J-ring or L-box test for jobsite testing.

If the self-consolidating concrete horizontal flow will exceed 15 ft (4.6 m), the dynamic segregation index test shall be performed at start of production for each mix design and per contract.

The hardened visual stability index test shall be performed on the first truck delivery of the day, and every 300 cu yd (230 cu m) thereafter. Slump flow, visual stability index, J-ring value or L-box blocking ratio, air content, and concrete temperature shall be recorded for each hardened visual stability index test.

The Contractor shall retain all hardened visual stability index cut cylinder specimens until the Engineer notifies the Contractor that the specimens may be discarded.

If mix foaming or other potential detrimental material is observed during placement or at the completion of the pour, the material shall be removed while the concrete is still plastic.

Quality Assurance by Engineer at Plant. For air content and aggregate gradation, quality assurance independent sample testing and split sample testing will be performed as indicated in the contract.

For slump flow, visual stability index, and J-ring or L-box tests, quality assurance independent sample testing and split sample testing will be performed as determined by the Engineer.

Quality Assurance by Engineer at Jobsite. For air content and strength, quality assurance independent sample testing and split sample testing will be performed as indicated in the contract.

For slump flow, visual stability index, J-ring or L-box, dynamic segregation index, and hardened visual stability index tests, quality assurance independent sample testing will be performed as determined by the Engineer.

For slump flow and visual stability index quality assurance split sample testing, the Engineer will perform tests at the beginning of the project on the first three tests performed by the Contractor. Thereafter, a minimum of ten percent of total tests required of the Contractor will be performed per plant, which will include a minimum of one test per mix design. The acceptable limit of precision will be 1.5 in. (40 mm) for slump flow and a limit of precision will not apply to the visual stability index.

For the J-ring or the L-box quality assurance split sample testing, a minimum of 80 percent of the total tests required of the Contractor will be witnessed by the Engineer per plant, which will include a minimum of one witnessed test per mix design. The Engineer reserves the right to

conduct quality assurance split sample testing. The acceptable limit of precision will be 1.5 in. (40 mm) for the J-ring value and ten percent for the L-box blocking ratio.

For dynamic segregation index, quality assurance split sample testing will be performed as determined by the Engineer. The acceptable limit of precision will be 1.0 percent.

For each hardened visual stability index test performed by the Contractor, the cut cylinders shall be presented to the Engineer for determination of the rating. The Engineer reserves the right to conduct quality assurance split sample testing. A limit of precision will not apply to the hardened visual stability index.

District Special Provisions

Alphabetic Index

ALPHABETIC INDEX OF DISTRICT SPECIAL PROVISIONS

<u>Item/Description</u>	<u>Standard Specification</u>	<u>Filename</u>
AGGREGATE DITCH	283.03	28303
AGGREGATE QUALITY	1004.04	100404
ANTI-STRIP ADDITIVE FOR HOT-MIX ASPHALT	406.01	40601
BACKFILL - PIPE CULVERTS	542.04e	54204e
BACKFILL, BUILDING REMOVAL	550.07	55007
BIN-TYPE RETAINING WALL	503.00	50300
BITUMINOUS PRIME COATE FOR HOT-MIX ASPHALT PAVEMENT (FULL DEPTH)	407.06	40706
BORROW AND FURNISHED EXCAVATION	204.00	20400
BRIDGE FLOOR FINISHING MACHINE	503.17	50317
BRIDGE WEARING SURFACE REMOVAL	440.01	44001
CENTER JOINT REPAIR SYSTEM	440.03c	44003c
CLASS (*) PATCHES, TYPE (**), (***)	442.00	44200
CLEAN EXISTING PAVEMENT EDGE JOINT	406.00	40600
COARSE AGGREGATE FILL	1004.01	100401
COARSE AGGREGATE FOR BITUMINOUS COURSES, CLASS A	1004.03b	100403b
CONCRETE HANDRAIL REMOVAL	501.04	50104
CONCRETE HEADWALL REMOVAL	501.03	50103
CONCRETE SUPERSTRUCTURE AGGREGATE OPTIMIZATION	1004.02	100402
CONCRETE WEARING SURFACE	503.01	50301
CONDUIT, PUSHED OR TRENCHED	810.00	81000
CONSTRUCTION STATION LAYOUT	105.00	10500
CRACK AND JOINT SEALING`	451.00	45100
DATE OF COMPLETION	108.05a	10805a
DATE OF COMPLETION (PLUS WORKING DAYS)	108.05b	10805b
DELAYED START OF MULTIPLE CONTRACTS	108.03	10803

ALPHABETIC INDEX OF DISTRICT SPECIAL PROVISIONS

DETECTOR LOOP, SPECIAL FOR TRAFFIC COUNTERS	886.00	88600
DETECTOR LOOPS, TYPE 1	886.00a	88600a
DRAINAGE HOLES	606.12	60612
ELECTRIC CABLE CONDUIT, LEAD-IN, NO. 18	873.00	87300
EMBANKMENT	205.05	20505
EMBANKMENT (RESTRICTIONS)	205.04	205.04
EMBANKMENT (SMALL EMBANKMENTS)	205.05a	20505a
EQUIPMENT VAULT FOR NUCLEAR TESTING EQUIPMENT	670.05	67005
EROSION CONTROL CURB	630.00	63000
FILLING EXISTING CULVERTS	605.04a	60504a
FILLING EXISTING DRAINAGE STRUCTURES	605.04b	60504b
FILLING EXISTING INLETS	605.04d	60504d
FLEXIBLE DELINEATOR MAINTENANCE	635.00	63500
FLEXIBLE DELINEATORS	635.01	63501
FLOOR DRAIN EXTENSION	503.12a	50312a
GEOTECHNICAL REINFORCEMENT	205.00	20500
GROOVED-IN RUMBLE STRIP	407.13	40713
GROOVING FOR RECESSED PAVEMENT MARKING	780.02	78002
GROUT FOR USE WITH RIPRAP	281.00	28100
GUARD POST REMOVAL	632.00	63200
GUARDRAIL AGGREGATE EROSION CONTROL	630.01	63001
HOT-MIX ASPHALT CONCRETE MILLING MATERIAL	440.03f	44003f
HOT-MIX ASPHALT – PRIME COAT	406.02	40602
HOT-MIX ASPHALT SHOULDER RESURFACING CONSTRUCTED SIMULTANEOUSLY WITH MAINLINE PAVING	482.06	48206
HOT-MIX ASPHALT SHOULDER RESURFACING REQUIRED TO BE CONSTRUCTED SIMULTANEOUSLY WITH MAINLINE PAVING	482.05	48205
HOT-MIX ASPHALT SURFACE COURSE SURFACE TESTS	406.04a	40604a

ALPHABETIC INDEX OF DISTRICT SPECIAL PROVISIONS

HOT-MIX ASPHALT SURFACE REMOVAL, *** (** MM)	440.03a	44003a
HOT-MIX ASPHALT SURFACE REMOVAL, *** (** MM)	440.03b	44003b
INLET-MANHOLE, TYPE G-1, 4' (1.2 M) DIAMETER	602.00d	60200d
INLET-MANHOLE, TYPE G-1, 4' (1.2 M) DIAMETER, SPECIAL	602.00e	60200e
INLET-MANHOLE, TYPE G-1, 5' (1.5 M) DIAMETER	602.00f	60200f
INLET-MANHOLE, TYPE G-1, 5' (1.5 M) DIAMETER, DOUBLE, SPECIAL	602.00h	60200h
INLET-MANHOLE, TYPE G-1, 5' (1.5 M) DIAMETER, SPECIAL	602.00g	60200g
INLET-MANHOLE, TYPE G-1, 8' (2.4 M) DIAMETER, DOUBLE, SPECIAL	602.00i	60200i
INLETS, TYPE G-1	602.00a	60200a
INLETS, TYPE G-1, DOUBLE	602.00m	60200m
INLETS, TYPE G-1, DOUBLE, SPECIAL	602.00c	60200c
INLETS, TYPE G-1, SPECIAL	602.00b	60200b
INLETS, TYPE G-2	602.00l	60200l
ISLAND PAVEMENT CONSTRUCTED ON EXISTING PAVEMENT	606.08	60608
JACK AND REPOSITION BEARINGS	521.00b	52100b
JACKING AND CRIBBING	521.00c	52100c
LOCATION OF UNDERGROUND STATE MAINTAINED FACILITIES	107.31	10731
MANHOLE TO BE ADJUSTED WITH NEW TYPE G-1 FRAME AND GRATE	602.00j	60200j
MORTARED STONE WALL	683.00	68300
MOWING	250.06a	250.06a
MOWING	250.06b	250.06b
NATIONWIDE 404 PERMIT REQUIREMENTS	107.00a	10700a
PAVEMENT DRAINAGE AFTER COLD MILLING	440.03c	44003c
PAVEMENT MARKING REMOVAL/WORK ZONE PAVEMENT MARKING REMOVAL	703.00	70300
PAVEMENT PATCHING WITH HOT-MIX ASPHALT SURFACE REMOVAL	440.03e	44003e
PAYMENT FOR USE OF MATERIAL TRANSFER DEVICE	406.13	40613

ALPHABETIC INDEX OF DISTRICT SPECIAL PROVISIONS

PCC AUTOMATIC BATCHING EQUIPMENT	1103.03	110303
PERMANENT SURVEY MARKER, TYPE I, BRIDGE PLACEMENT	667.04	66704
PERMANENT SURVEY TIES	668.02	66802
PIPE CULVERTS	542.04	54204
PIPE UNDERDRAIN	601.00	60100
PLUG EXISTING DRAINS	503.12	50312
PREFORMED PLASTIC PAVEMENT MARKINGS	780.07	78007
PRESTAGE SITE CONSTRUCTION MEETINGS	105.06	10506
PROOF ROLLING	301.01	30101
PROTECTION OF FRAMES AND LIDS OF UTILITY STRUCTURES	440.03	44003
PROTECTIVE COAT, SPECIAL	503.19	50319
RAILROAD APPROACH PAVEMENT	420.20	42020
RAILROAD TIES REMOVAL AND DISPOSAL	680.00a	68000a
RAILROAD TRACK RAIL REMOVAL	680.00	68000
REFLECTIVE CRACK CONTROL TREATMENT	443.00	44300
REMOVAL OF ABANDONED UNDERGROUND UTILITIES	105.07	10507
REMOVE AND RELAY PIPE CULVERTS	542.01	54201
RIGHT-OF-WAY RESTRICTIONS	107.32	10732
ROCKFILL	311.00	31100
RUMBLE STRIP	407.14	40714
SEEDING, MINOR AREAS	250.00	25000
SEEDLING MIXTURE A	253.00b	15300b
SEEPAGE COLLAR	542.00	54200
SIDEWALK DRAINS	424.01	42401
SOIL MODIFICATION	302.00	30200
SPEEDING PENALTY	701.06	70106
STATUS OF UTILITIES/UTILITIES TO BE ADJUSTED	105.07	10507

ALPHABETIC INDEX OF DISTRICT SPECIAL PROVISIONS

STEEL CASINGS *" (* MM)	561.00	56100
STEEL PIPE CULVERT, SPECIAL (JACKED) *" (* MM)	552.00	55200
STEEL PLATE BEAM GUARDRAIL, TYPE A, 6.75 FOOT POSTS	630.08	63008
STONE DUMPED RIPRAP*	281.04	28104
STONE RIPRAP	281.06	28106
STORM SEWER (SPECIAL)	550.02	55002
STORM SEWER/PIPE CULVERT) JACKED IN PLACE ***" (** MM)	552.01	55201
STORM SEWER (WATER MAIN QUALITY PIPE)	550.00	55000
SUBBASE GRANULAR MATERIAL	311.01	31101
SUBGRADE TREATMENT	301.03	30103
SURFACE FILLER, SPECIAL (GALLON)	503.02	50302
TEMPORARY CONCRETE BARRIER REFLECTORS	704.00a	70400a
TEMPORARY CONCRETE BARRIER, STATE OWNED & TEMPORARY CONCRETE BARRIER TERMINAL SECTIONS, STATE OWNED	704.00d	70400d
TEMPORARY INLET DRAINAGE TREATMENT	602.00k	60200k
TEMPORARY PAVEMENT	355.00	35500
TEMPORARY RAISED REFLECTIVE PAVEMENT MARKER, TYPE II	781.00	78100
TEMPORARY SIDEWALKS	424.02	42402
TERMINAL FACILITY	863.00	86300
THERMOPLASTIC PAVEMENT MARKING EQUIPMENT	780.00	78000
TRAFFIC BARRIER TERMINALS	631.11c	63111c
TRAFFIC BARRIER TERMINALS, TYPE 1, SPECIAL (FLAMED) OR (TANGENT)	631.04	631.04
TRAFFIC BARRIER TERMINALS, TYPE 2	631.14	63114
TRAFFIC BARRIER TERMINALS, TYPE 6	631.07	63107
TRAFFIC CONTROL AND PROTECTION STANDARD 701331 (SPECIAL)	701.08b	70108b
TRAFFIC CONTROL AND PROTECTION STANDARD BLR 21 AND BLR 21 (SPECIAL)	701.20	70120

ALPHABETIC INDEX OF DISTRICT SPECIAL PROVISIONS

TRAFFIC CONTROL AND PROTECTION STANDARD BLR 22 AND BLR 22 (SPECIAL)	701.21	701.21
TRAFFIC CONTROL AND PROTECTION STANDARD 701606 (SPECIAL)	701.22	70122
TRAFFIC CONTROL PLAN	701.00	70100
TREE WHIP MIXTURE	253.00	25300
TRENCH & BACKFILL, SPECIAL FOR CONDUIT INSTALLATION BENEATH BITUMINOUS SHOULDERS	815.00	81500
WIDTH RESTRICTION SIGNING	701.14	70114

District Special Provisions

Numeric Index

SECTION 100

District Special Provisions

<u>Standard Specifications</u>	<u>Item/Description</u>	<u>Doc. #</u>
105.00	CONSTRUCTION STATION LAYOUT	10500
105.06	PRESTAGE SITE CONSTRUCTION MEETINGS	10506
105.07	REMOVAL OF ABANDONED UNDERGROUND UTILITIES	10507
105.07a	STATUS OF UTILITIES/UTILITIES TO BE ADJUSTED	10507a
107.00a	NATIONWIDE 404 PERMIT REQUIREMENTS	10700a
107.31	LOCATION OF UNDERGROUND STATE MAINTAINED FACILITIES	10731
107.32	RIGHT-OF-WAY RESTRICTIONS	10732
108.03	DELAYED START OF MULTIPLE CONTRACTS	10803
108.05a	DATE OF COMPLETION	10805a
108.05b	DATE OF COMPLETION (PLUS WORKING DAYS)	10805b

SECTION 200

District Special Provisions

<u>Standard Specifications</u>	<u>Item/Description</u>	<u>Doc. #</u>
204.00	BORROW AND FURNISHED EXCAVATION	20400
205.00	GEOTECHNICAL REINFORCEMENT	20500
205.05	EMBANKMENT	20505
205.04	EMBANKMENT (RESTRICTIONS)	20504
205.05a	EMBANKMENT (SMALL EMBANKMENTS)	20505a
250.00	SEEDING, MINOR AREAS	25000
250.06a	MOWING	25006a
250.06b	MOWING	250.06b
253.00	TREE WHIP MIXTURE	25300
253.00b	SEEDLING MIXTURE A	25300b
281.00	GROUT FOR USE WITH RIPRAP	28100
281.04	STONE DUMPED RIPRAP *	28104
281.06	STONE RIPRAP	28106
283.03	AGGREGATE DITCH	28303

January 19, 2012

SECTION 300

District Special Provisions

<u>Standard Specifications</u>	<u>Item/Description</u>	<u>Doc. #</u>
301.01	PROOF ROLLING	30101
301.03	SUBGRADE TREATMENT	30103
302.00	SOIL MODIFICATION	30200
311.00	ROCKFILL	31100
311.01	SUBBASE GRANULAR MATERIAL	31101
355.00	TEMPORARY PAVEMENT	35500

SECTION 400

District Special Provisions

<u>Standard Specifications</u>	<u>Item/Description</u>	<u>Doc. #</u>
406.00	CLEAN EXISTING PAVEMENT EDGE JOINT	40600
406.01	ANTI-STRIP ADDITIVE FOR HOT-MIX ASPHALT	40601
406.04a	HOT-MIX ASPHALT SURFACE COURSE SURFACE TESTS	40604a
406.02	HOT-MIX ASPHALT – PRIME COAT	40602
406.13	PAYMNET FOR USE OF MATERIAL TRANSFER DEVICE	40613
407.06	BITUMINOUS PRIME COAT FOR HOT-MIX ASPHALT PAVEMENT (FULL DEPTH)	40706
407.13	GROOVED-IN RUMBLE STRIP	40713
420.20	RAILROAD APPROACH PAVEMENT	42020
424.01	SIDEWALK DRAINS	42401
424.02	TEMPORARY SIDEWALKS	42402
440.01	BRIDGE WEARING SURFACE REMOVAL	44001
440.03	PROTECTION OF FRAMES AND LIDS OF UTILITY STRUCTURES	44003
440.03c	CENTER JOINT REPAIR SYSTEM	44003c
440.03a	HOT-MIX ASPHALT SURFACE REMOVAL, *** (** MM)	44003a
440.03b	HOT-MIX ASPHALT SURFACE REMOVAL, *** (** MM)	44003b
440.03d	PAVEMENT DRAINAGE AFTER COLD MILLING	44003d
440.03e	PAVEMENT PATCHING WITH HOT-MIX ASPHALT SURFACE REMOVAL	44003e
440.03f	HOT-MIX ASPHALT CONCRETE MILLING MATERIAL	44003f
442.00	CLASS (*) PATCHES, TYPE (**), (***)	44200
443.00	REFLECTIVE CRACK CONTROL TREATMENT	44300
451.00	CRACK AND JOINT SEALING	45100
482.05	HOT-MIX ASPHALT SHOULDER RESURFACING REQUIRED TO BE CONSTRUCTED SIMULTANEOUSLY WITH MAINLINE PAVING	48205
482.06	HOT-MIX ASPHALT SHOULDER RESURFACING CONSTRUCTED SIMULTANEOUSLY WITH MAINLINE PAVING	48206

SECTION 500

District Special Provisions

<u>Standard Specifications</u>	<u>Item/Description</u>	<u>Doc. #</u>
501.03	CONCRETE HEADWALL REMOVAL	50103
501.04	CONCRETE HANDRAIL REMOVAL	50104
503.00	BIN-TYPE RETAINING WALL	50300
503.01	CONCRETE WEARING SURFACE	50301
503.02	SURFACE FILLER, SPECIAL (GALLON)	50302
503.12a	FLOOR DRAIN EXTENSIONS	50312a
503.12	PLUG EXISTING DRAINS	50312
503.17	BRIDGE FLOOR FINISHING MACHINE	50317
503.19	PROTECTING COAT, SPECIAL	50319
521.00b	JACK AND REPOSITION BEARINGS	52100b
521.00c	JACKING AND CRIBBING	52100c
542.00	SEEPAGE COLLAR	54200
542.01	REMOVE AND RELAY PIPE CULVERTS	54201
542.04	PIPE CULVERTS	54204
542.04e	BACKFILL - PIPE CULVERTS	54204e
550.00	STORM SEWER (WATER MAIN QUALITY PIPE)	55000
550.02	STORM SEWER (SPECIAL)	55002
550.07	BACKFILL, BUILDING REMOVAL	55007
552.00	STEEL PIPE CULVERT, SPECIAL (JACKED) ** (* MM)	55200
552.01	(*STORM SEWER/PIPE CULVERT) JACKED IN PLACE, ** (* MM)	55201
561.00	STEEL CASINGS ** (* MM)	56100

SECTION 600

District Special Provisions

<u>Standard Specifications</u>	<u>Item/Description</u>	<u>Doc. #</u>
601.01	PIPE UNDERDRAIN	60101
602.00d	INLET-MANHOLE, TYPE G-1, 4' (1.2 M) DIAMETER	60200d
602.00f	INLET-MANHOLE, TYPE G-1, 5' (1.5 M) DIAMETER	60200f
602.00h	INLET-MANHOLE, TYPE G-1, 5' (1.5 M) DIAMETER, DOUBLE, SPECIAL	60200h
602.00i	INLET-MANHOLE, TYPE G-1, 8' (2.4 M) DIAMETER, DOUBLE, SPECIAL	60200i
602.00e	INLET-MANHOLE, TYPE G-1, 4' (1.2 M) DIAMETER, SPECIAL	60200e
602.00g	INLET-MANHOLE, TYPE G-1, 5' (1.5 M) DIAMETER, SPECIAL	60200g
602.00a	INLETS, TYPE G-1	60200a
602.00c	INLETS, TYPE G-1, DOUBLE, SPECIAL	60200c
602.00b	INLETS, TYPE G-1, SPECIAL	60200b
602.00j	MANHOLE TO BE ADJUSTED WITH NEW TYPE G-1 FRAME AND GRATE	60200j
602.00k	TEMPORARY INLET DRAINAGE TREATMENT	60200k
602.00l	INLETS, TYPE G-2	60200l
602.00m	INLETS, TYPE G-1, DOUBLE	60200m
605.04a	FILLING EXISTING CULVERTS	60504a
605.04b	FILLING EXISTING DRAINAGE STRUCTURES	60504b
605.04	FILLING EXISTING INLETS	60504
606.08	ISLAND PAVEMENT CONSTRUCTED ON EXISTING PAVEMENT	60608
606.12	DRAINAGE HOLES	60612
630.00	EROSION CONTROL CURB	63000
630.01	GUARDRAIL AGGREGATE EROSION CONTROL	63001
630.08	STEEL PLATE BEAM GUARDRAIL, TYPE A, 6.75 FOOT POSTS	63008

SECTION 600

District Special Provisions

<u>Standard Specifications</u>	<u>Item/Description</u>	<u>Doc. #</u>
631.04	TRAFFIC BARRIER TERMINAL, TYPE 1, SPECIAL (FLARED) OR (TANGENT)	63104
631.07	TRAFFIC BARRIER TERMINALS, TYPE 6	63107
631.11c	TRAFFIC BARRIER TERMINALS	63111c
631.14	TRAFFIC BARRIER TERMINALS, TYPE 2	63114
632.00	GUARD POST REMOVAL	63200
635.00	FLEXIBLE DELINEATOR MAINTENANCE	63500
635.01	FLEXIBLE DELINEATORS	63501
667.04	PERMANENT SURVEY MARKER, TYPE I, BRIDGE PLACEMENT	66704
668.02	PERMANENT SURVEY TIES	66802
670.05	EQUIPMENT VAULT FOR NUCLEAR TESTING EQUIPMENT	67005
680.00a	RAILROAD TIES REMOVAL AND DISPOSAL	68000a
680.00	RAILROAD TRACK RAIL REMOVAL	68000
683.00	MORTARED STONE WALL	68300

SECTION 700

District Special Provisions

<u>Standard Specifications</u>	<u>Item/Description</u>	<u>Doc. #</u>
701.00	TRAFFIC CONTROL PLAN	70100
701.06	SPEEDING PENALTY	70106
701.08b	TRAFFIC CONTROL AND PROTECTION STANDARD 701331 (SPECIAL)	70108b
701.14	WIDTH RESTRICTION SIGNING	70114
701.20	TRAFFIC CONTROL AND PROTECTION STANDARD BLR 21 AND BLR 21 (SPECIAL)	70120
701.21	TRAFFIC CONTROL AND PROTECTION STANDARD BLR 22 AND BLR 22 (SPECIAL)	70121
701.22	TRAFFIC CONTROL AND PROTECTION STANDARD 701606 (SPECIAL)	70122
703.00	PAVEMENT MARKING REMOVAL/WORK ZONE PAVEMENT MARKING REMOVAL	70300
704.00a	TEMPORARY CONCRETE BARRIER REFLECTORS	70400a
704.00	TEMPORARY CONCRETE BARRIER, STATE OWNED AND TEMPORARY CONCRETE BARRIER TERMINAL SECTIONS, STATE OWNED	70400
780.00	THERMOPLASTIC PAVEMENT MARKING EQUIPMENT	78000
780.02	GROOVING FOR RECESSED PAVEMENT MARKINGS	78002
780.07	PREFORMED PLASTIC PAVEMENT MARKINGS	78007
781.00	TEMPORARY RAISED REFLECTIVE PAVEMENT MARKER	78100

SECTION 800

District Special Provisions

<u>Standard Specifications</u>	<u>Item/Description</u>	<u>Doc. #</u>
810.00	CONDUIT, PUSHED OR TRENCHED	81000
815.00	TRENCH & BACKFILL, SPECIAL FOR CONDUIT INSTALLATION BENEATH BITUMINOUS SHOULDERS	81500
863.00	TERMINAL FACILITY	86300
873.00	ELECTRIC CABLE CONDUIT NO. 18	87300
886.00	DETECTOR LOOP, SPECIAL FOR TRAFFIC COUNTERS	88600
886.00a	DETECTOR LOOPS, TYPE 1	88600a

January 19, 2012

SECTION 900

District Special Provisions

Standard
Specifications

Item/Description

Doc. #

SECTION 1000

District Special Provisions

<u>Standard Specifications</u>	<u>Item/Description</u>	<u>Doc. #</u>
1004.01	COARSE AGGREGATE FILL	100401
1004.02	CONCRETE SUPERSTRUCTURE AGGREGATE OPTIMIZATION	100402
1004.03b	COARSE AGGREGATE FOR BITUMINOUS COURSES, CLASS A	d100403b
1004.04	AGGREGATE QUALITY	d100404
1103.03	PCC AUTOMATIC BATCHING EQUIPMENT	110303

District Special Provisions

63008

630.08

Designer Note: Use when matching existing rail that is to remain in place and not the MGS type rail. Also include CADD Standard "Non-MGS SPBGR."

STEEL PLATE BEAM GUARDRAIL, TYPE A, 6.75 FOOT POSTS

Effective July 31, 2009

Revised April 27, 2012

Add the following to Article 630.08 of the Standard Specifications:

"This work will be paid for at the contract unit price per foot for STEEL PLATE BEAM GUARDRAIL, TYPE A, 6.75 FOOT POSTS."

This work shall consist of installing the Non-Midwest Guardrail System type rail at the locations specified in the plans.

66802

668.02

Designer Note: Provide for all section or subsection stones and other markers. Check with the Engineer of Design Surveys on ties required, plan details, CADD drawing 667101 and use of this special provision. Section 668 would permit work to be paid under 109.04 for setting, tying, recording of markers by Land Surveyor. This item creates an each pay item that can be included in the plans.

PERMANENT SURVEY TIES

Effective April 1, 1991

Revised April 27, 2012

This work shall consist of furnishing and installing a permanent survey tie at the locations shown in the plans and in accordance with the Detail for Permanent Survey Ties included in the plans and Section 668 of the Standard Specifications. Refer to Section 668.02 of the Standard Specifications for information about the survey work required.

The Class SI concrete used in the permanent survey ties shall be in accordance with Section 503 of the Standard Specifications. The reinforcement bars used shall be in accordance with Section 508 of the Standard Specifications.

This work will be paid for at the contract unit price per each for PERMANENT SURVEY TIES.