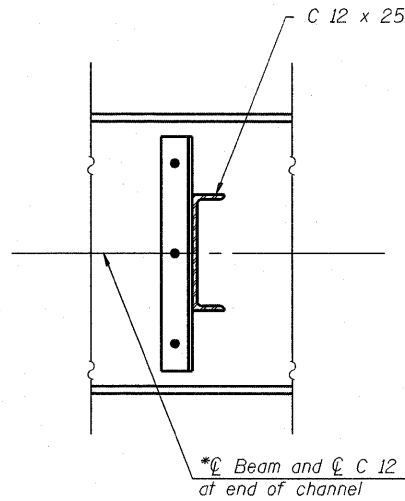


INTERIOR DIAPHRAGM D

Note:
Two hardened washers required for each set of oversized holes.
*Alternate channels (C 12 x 30) are permitted to facilitate material acquisition. Calculated weight of structural steel is based on the lighter section. The alternate, if utilized, shall be provided at no additional cost to the Department.
**3/4" ϕ HS bolts, 15/16" ϕ holes



SECTION A-A

INTERIOR GIRDER MOMENT TABLE		
0.5 Span		
I_s	(in ⁴)	3,270
$I_c(n)$	(in ⁴)	10,383
$I_c(3n)$	(in ⁴)	7,688
S_s	(in ³)	243
$S_c(n)$	(in ³)	390
$S_c(3n)$	(in ³)	352
DC1	(k/ft)	0.736
M _{DC1}	(k)	257
DC2	(k/ft)	0.129
M _{DC2}	(k)	45
DW	(k/ft)	0.286
M _{DW}	(k)	100
M ϕ + IM	(k)	560
M _u (Strength I)	(k)	1,506
$\phi_r M_n$	(k)	2,091
f_s DC1	(ksi)	12.70
f_s DC2	(ksi)	1.53
f_s DW	(ksi)	3.41
f_s (ϕ + IM)	(ksi)	17.23
f_s (Service II)	(ksi)	40.04
0.95R _n F _y f	(ksi)	47.50
V _r	(k)	23.75

INTERIOR GIRDER REACTION TABLE		
	Abut.	
R _{DC1}	(k)	20.1
R _{DC2}	(k)	3.4
R _{DW}	(k)	7.6
R ϕ + IM	(k)	79.4
R _{Total}	(k)	110.5

All cross frames or diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted. Individual cross frames or diaphragms at supports may be temporarily disconnected to install bearing anchor rods.

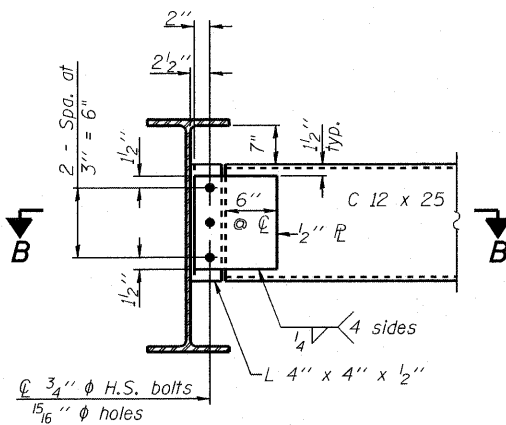
For Information Only

I_s, S_s : Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total-Strength I, and Service II) due to non-composite dead loads (in⁴ and in³).

$I_c(n), S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections, due to short-term composite live loads (in⁴ and in³).

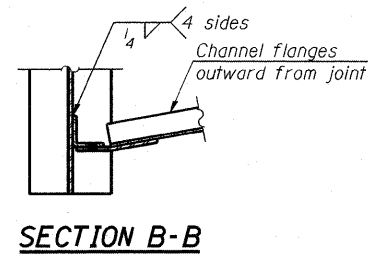
$I_c(3n), S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections, due to long-term composite (superimposed) dead loads (in⁴ and in³).

DC1: Un-factored non-composite dead load (kips/ft.).
M_{DC1}: Un-factored moment due to non-composite dead load (kip-ft.).
DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).
M_{DC2}: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).
DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).
M_{DW}: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).
M ϕ + IM: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).
M_u (Strength I): Factored design moment (kip-ft.).
1.25 (M_{DC1} + M_{DC2}) + 1.5 M_{DW} + 1.75 M ϕ + IM
 $\phi_r M_n$: Compact composite positive moment capacity computed according to Article 6.10.7.1 (kip-ft.).
 f_s DC1: Un-factored stress at edge of flange for controlling steel flange due to vertical non-composite dead loads as calculated below (ksi).
M_{DC1} / S_{nc}
 f_s DC2: Un-factored stress at edge of flange for controlling steel flange due to vertical composite dead loads as calculated below (ksi).
M_{DC2} / S_{c(3n)}.
 f_s DW: Un-factored stress at edge of flange for controlling steel flange due to vertical composite future wearing surface loads as calculated below (ksi).
M_{DW} / S_{c(3n)}.
 f_s (ϕ + IM): Un-factored stress at edge of flange for controlling steel flange due to vertical composite live plus impact loads as calculated below (ksi).
M ϕ + IM / S_{c(n)}.
 f_s (Service II): Sum of stresses as computed below (ksi).
 f_s DC1 + f_s DC2 + f_s DW + 1.3 f_s (ϕ + IM)
0.95R_nF_yf: Composite stress capacity for Service II loading according to Article 6.10.4.2 (ksi).
V_r: Maximum factored shear range in composite portion of span computed according to Article 6.10.10.



END DIAPHRAGM D1

Note:
Two hardened washers required for each set of oversized holes.



SECTION B-B