

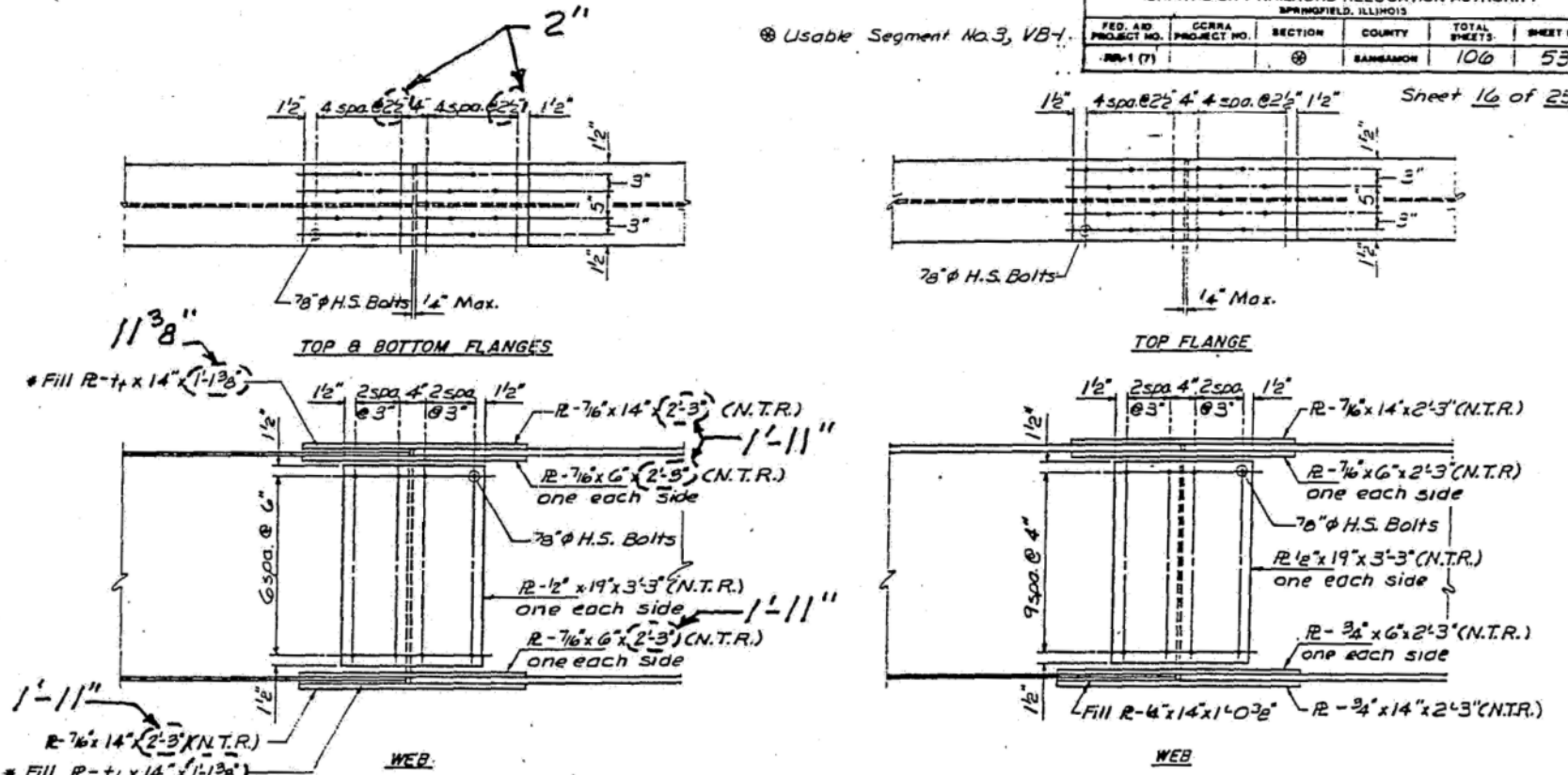
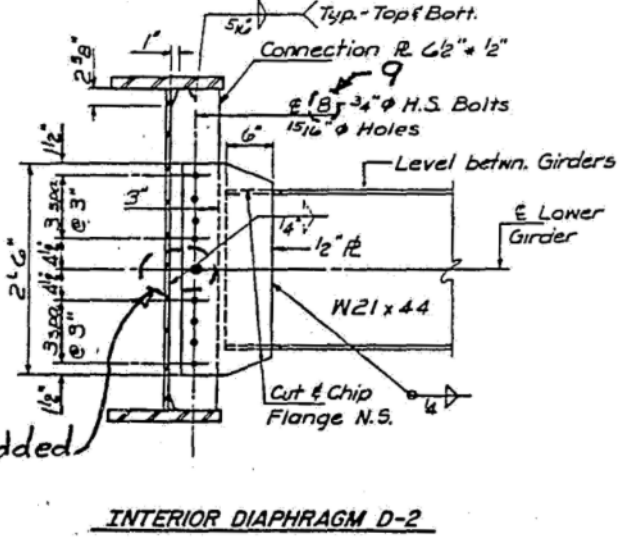
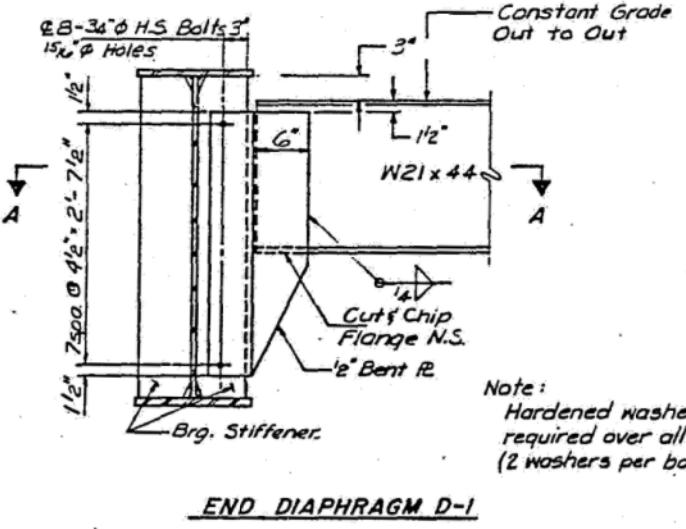
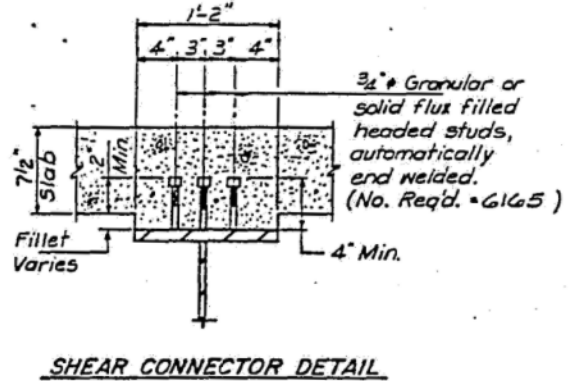
**INTERIOR GIRDER MOMENT TABLE**

	Q4 Span 1	Pier 1	Q5 Span 2	Pier 2	Q5 Span 3	Pier 3	Q5 Span 4	Pier 4	Q5 Span 5	Pier 5	Q6 Span 6
$I_s$ (in <sup>4</sup> )	12680	14345	12680	16030	12680	17730	12680	17730	12680	24735	16205
$I_c$ (in <sup>4</sup> )	22990	—	22990	—	22990	—	22990	—	22990	—	30705
$I_c$ (n <sup>9</sup> ) (in <sup>4</sup> )	30540	—	30540	—	30540	—	30540	—	30540	—	42550
$S_s$ (in <sup>3</sup> )	583	656	583	729	583	801	583	801	583	1093	873
$S_c$ (n <sup>9</sup> ) (in <sup>3</sup> )	735	—	735	—	735	—	735	—	735	—	1087
$S_c$ (n <sup>9</sup> ) (in <sup>3</sup> )	802	—	802	—	802	—	802	—	802	—	1180
$\phi$ (K/I)	1.105	1.50	1.105	1.50	1.105	1.50	1.105	1.50	1.105	1.50	1.105
$M\phi$ (K/I)	260	794	328	970	329	903	320	954	290	1492	736
$s\phi$ (K/I)	.395	—	.395	—	.395	—	.395	—	.395	—	.395
$M_s\phi$ (K)	102	—	144	—	138	—	133	—	137	—	283
$M\phi$ (K)	607	451	731	537	730	592	734	603	800	718	1013
$M_{IMP}$ (K)	164	113	176	129	175	142	176	138	184	165	233
$S_3$ (M $\phi$ +I) (K)	1285	940	1512	1110	1509	1224	1518	1235	1640	1472	2077
$M_a$ (K)	2141	2254	2579	2704	2569	2765	2562	2846	2687	3853	4025
$M_u$ (K)	—	2541	—	2745	—	3086	—	2986	—	4118	—
$f_s$ @ non-comp. (K/si)	5.4	14.5	6.8	16.0	6.8	13.5	6.6	14.3	6.0	16.4	10.1
$f_s$ @ comp. (K/si)	1.7	—	2.3	—	2.3	—	2.2	—	2.2	—	3.1
$f_s$ $S_3$ (K/si)	19.2	17.2	22.6	18.3	22.6	18.3	22.7	18.5	24.5	16.2	21.1
$f_w$ (Total) (K/si)	1.2	1.1	2.2	1.2	1.8	1.1	1.8	1.2	1.2	1.0	1.0
$f_s$ (Overload) (K/si)	27.5	32.8	33.9	35.5	33.5	32.9	33.3	34.0	33.9	33.6	35.3
$f_s$ (Total) (K/si)	35.8	42.6	44.1	46.2	43.6	42.8	43.3	44.2	44.1	43.7	45.9
VR (K)	72	—	64	—	64	—	65	—	65	—	71

**INTERIOR GIRDER REACTION TABLE**

	N. Abut.	Pier 1	Pier 2	Pier 3	Pier 4	Pier 5	S. Abut.
$R\phi$ (K)	33	120	131	133	129	162	55
$R\phi$ (K)	50	68	73	81	76	81	53
Imp. (K)	13	17	18	19	18	19	12
$R_{Total}$ (K)	96	205	222	233	223	262	120

$I_s$  and  $S_s$  are the moment of inertia and section modulus of the steel section used in computing  $f_s$  (Total & Overload).  
 $I_c$  and  $S_c$  are the moment of inertia and section modulus of the composite section used in computing  $f_s$  (Total & Overload).  
 VR is the maximum  $\pm$  Impact shear range in span.  
 $M_u$  is the moment capacity for braced non-compact hybrid section ( $F_{bu} \times S$ ).  $F_{bu}$  computed according to AASHTO Guide Spec. for Horizontally Curved Highway Bridges 2.12 & 2.19.  
 $M_a$  (Applied Moment) =  $1.3[M\phi + M_s\phi + S_3(M\phi + I)]$ .  
 $f_w$  is the normal stress due to lateral flange bending.  
 $f_s$  (Overload) is the sum of the stresses due to  $M\phi + M_s\phi + S_3(M\phi + I)$ .  
 $f_s$  (Total) is the sum of the stresses due to  $1.3[M\phi + M_s\phi + S_3(M\phi + I)]$ .  
 Girder 1 has largest moments and is used for Moment Table.



**AS REVISED**

Notes: All Girder Splice Plates, except Fill R's are M-223, Grade 50 Steel.  
 All Splice Bolts shall have threads excluded from the shear planes.  
 N.T.R. refers to the supplemental requirements for notch toughness, Zone 2.

STRUCTURE NUMBER: 084-0185  
**STRUCTURAL STEEL DETAILS**  
 CHATHAM ROAD over  
 C.M.W and N.B.W. Railroads and FAP RTE 408  
 USABLE SEGMENT NO. 3, VB-1  
 SANGAMON COUNTY  
 STATION 161+52.56

TE.H. HANSON ENGINEERS  
 M.D.M. 8553092  
 D.B.M.M.  
 T.E.H. 12-14-88

**FOR INFORMATION ONLY**

EXISTING PLANS  
 STRUCTURE NO. 084-0185  
 SHEET NO. 15 OF 15 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
72		SANGAMON	163	152

CONTRACT NO. 72B54  
 ILLINOIS FED. AID PROJECT  
 (84-9-28)RS-2&MISC STRUC REP

USER NAME =	DESIGNED -	CEH	REVISED
FILE NAME =	CHECKED -	CWC	REVISED
PLOT SCALE =	DRAWN -	DLH	REVISED
PLOT DATE =	CHECKED -	CEH/CWC	REVISED