

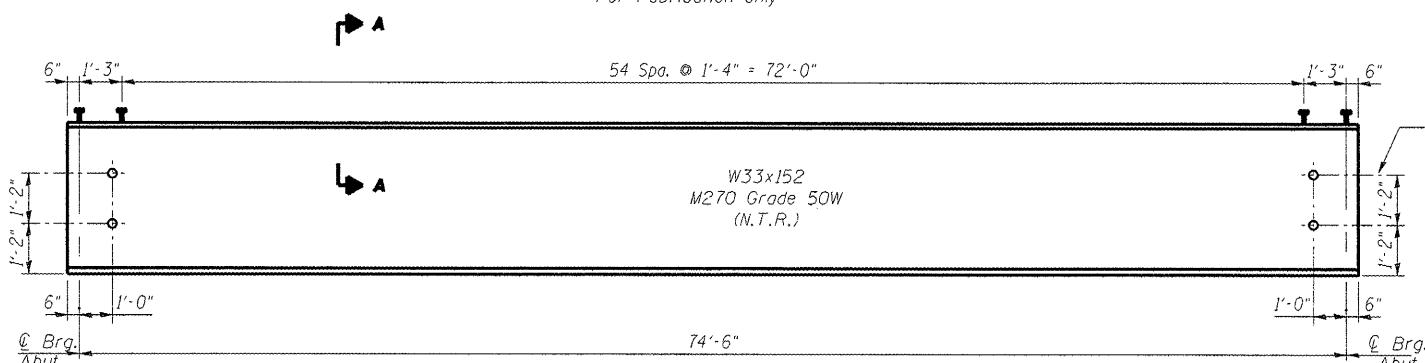
PLAN



TOP OF BEAM ELEVATIONS

Beam Number	Q Brdg. S. Abut.	Q Brdg. N. Abut.
1	642.48	642.51
2	642.59	642.62
3	642.68	642.72
4	642.68	642.72
5	642.59	642.62
6	642.48	642.51

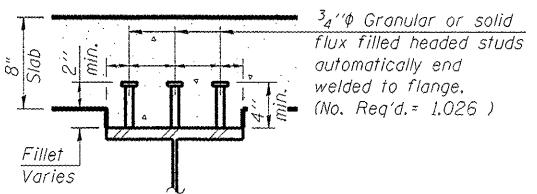
* For Fabrication only



BEAM ELEVATION

Note:

Load carrying components designated "NTR" shall conform to the Supplemental Requirements for Notch Toughness, Zone 2.



SECTION A-A

G-1

II-1-06

INTERIOR GIRDER MOMENT TABLE	
	0.5 Sp.
I_s	(in ⁴) 8,160
$I_c(n)$	(in ⁴) 20,780
$I_c(3n)$	(in ⁴) 14,979
S_s	(in ³) 487.0
$S_c(n)$	(in ³) 702.2
$S_c(3n)$	(in ³) 629.1
DCl	(k') 0.769
$MDC1$	(k) 534.2
$DC2$	(k') 0.15
$MDC2$	(k) 104.0
DW	(k') 0.266
MDW	(k) 185.0
$M_L + Imp$	(k) 1,006.9
$M_u(\text{Strength I})$	(k) 2,837.5
ϕM_n	(k) 3,464.7
$f_s DC1$	(ksi) 13.32
$f_s DC2$	(ksi) 1.98
$f_s DW$	(ksi) 3.53
$f_s 1.3(L+D)$	(ksi) 22.36
$f_s (\text{Service II})$	(ksi) 41.19
$f_s (\text{Total})(\text{Strength I})$	(ksi) 54.52
V_f	(k) 23.4

ROUTE NO.	SECTION	COUNTY	ROUTE	POST
FAS 253 (IL 251)	68 BR-1	MARSHALL	97	79
FED. HIGH DIST. NO.	ILLINOIS	FED. AND PROJECT		

SHEET NO. 8

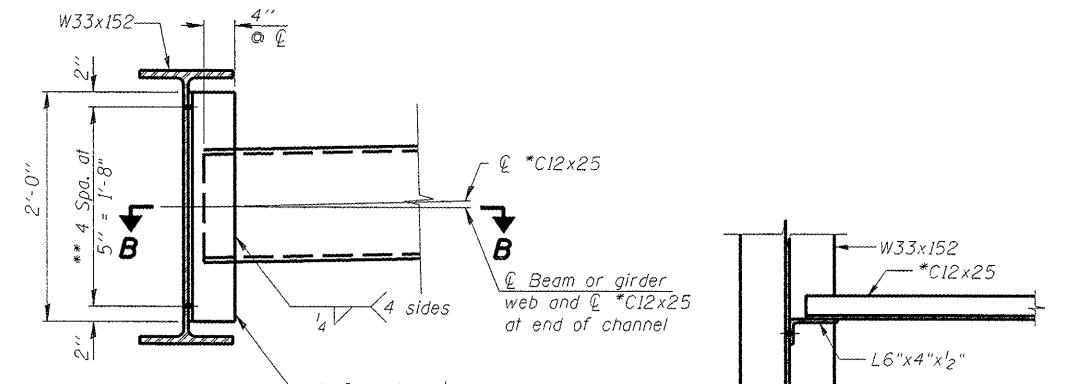
15 SHEETS

Contract #68573

I_s, S_s : Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total-Strength I, and Service II) due to non-composite dead loads (in.⁴ and in.³).
 $I_c(n), S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total-Strength I, and Service II) due to short-term composite live loads (in.⁴ and in.³).
 $I_c(3n), S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total-Strength I, and Service II) due to long-term composite (superimposed) dead loads (in.⁴ and in.³).
 $DC1$: Un-factored non-composite dead load (kip-ft.).
 $DC2$: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).
 $MDC1$: Un-factored moment due to non-composite dead load (kip-ft.).
 $MDC2$: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).
 DW : Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).
 MDW : Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).
 $M_L + Imp$: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).
 $M_u(\text{Strength I})$: Factored design moment (kip-ft.).
 $1.25(MDC1 + MDC2) + 1.5 MDW + 1.75 M_L + Imp$

ϕM_n : Compact composite positive moment capacity computed according to Article 6.10.7.1 (kip-ft.).
 $f_s (\text{Service II})$: Sum of stresses as computed from the moments below (ksi).
 $MDC1 + MDC2 + MDW + 1.3 M_L + Imp$
 $f_s (\text{Total})(\text{Strength I})$: Sum of stresses as computed from the moments below on non-compact section (ksi).
 $1.25(MDC1 + MDC2) + 1.5 MDW + 1.75 M_L + Imp$
 V_f : Factored shear range computed according to Article 6.10.10.

INTERIOR GIRDER REACTION TABLE	
	Abut.
RDC1	(k) 28.6
RDC2	(k) 5.55
RDW	(k) 9.95
$M_L + Imp$	(k) 72.1
RTotal	(k) 116.2



SECTION B-B

INTERIOR DIAPHRAGM

Note:

Two hardened washers required for each set of oversized holes.

* C12x30 are permitted to facilitate material acquisition. Calculated weight of structural steel is based on the lighter section.

** 3/4" Ø HS bolts, 15/16" Ø holes

REVISIONS	
NAME	DATE

ILLINOIS DEPARTMENT OF TRANSPORTATION	
STEEL FRAMING PLAN AND DETAILS	
ILL. RTE. 251 OVER SANDY CREEK F.A.S. 253 SEC. 68 BR-1 MARSHALL COUNTY STA. 133+80.00 S.N. 062-0085	
DESIGNED BY: S.G. DATE: 11-20-08	DRAWN BY: T.C.S. CHECKED BY: O.A.O.