INDEX OF SHEETS

SHEET NO.

DESCRIPTION **ROADWAY PLANS**

COVER SHEET
CENERAL NOTES AND STANDARDS
SUMMARY OF QUANTITIES
TYPICAL SECTIONS
SCHEDULES OF QUANTITIES
FAP RTE 681 (IL 116) PLAN AND PROFILE
TRAFFIC CONTROL AND PROTECTION FOR TEMPORARY DETOUR
EROSION CONTROL AND DRAINAGE PLAN
MISCELLANEOUS DETAILS
GUARDRAIL DETAILS

STRUCTURE PLANS

PIER 2 PIER DETAILS BAR SPLICER ASSEMBLY DETAILS

EXISTING STRUCTURE PLANS

28.-32. EXISTING STRUCTURE PLANS

CROSS SECTIONS

33.-37. FAP RTE 681 (IL 116) CROSS SECTIONS

FULL SIZE PLANS HAVE BEEN PREPARED USING STANDARD ENGINEERING SCALES, REDUCED SIZED PLANS WILL NOT CONFORM TO STANDARD SCALES. IN MAKING MEASUREMENTS ON REDUCED PLANS, THE ABOVE SCALES MAY BE USED.

MIGROFILMED
REEL NUMBER AWARDED
AWARDED
RESIDENT ENGINEER
AS BUILT CHANGES WERE MADE
ON THE FOLLOWING SHEETS

JOINT UTILITY LOCATION INFORMATION FOR EXCAVATION 1-800-892-0123

DISTRICT 3 NO. (815) 434-6131 PROJECT ENGINEER: RICK POWELL UNIT CHIEF: BRAD DUNCAN TOWNSHIP: MONA CONTRACT NO.: 66561

STATE OF ILLINOIS

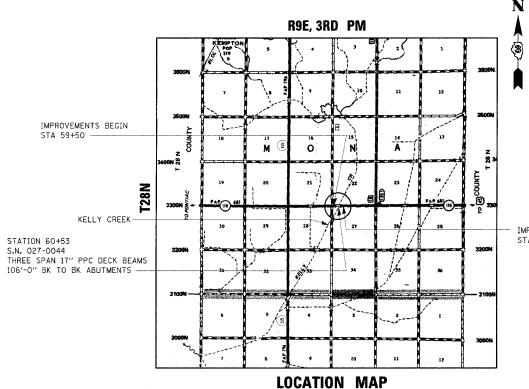
DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS**

PROPOSED HIGHWAY PLANS

FAP ROUTE 681 (IL 116) SECTION 116BR-1 PROJECT BHF - 681 (030) **FORD COUNTY**

C - 93 - 067 - 05

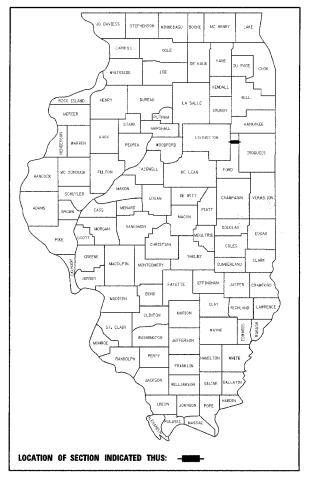
ILLINOIS ROUTE 116 OVER KELLY CREEK SUPERSTRUCTURE REPLACEMENT



IMPROVEMENTS END STA 61+78.5

> **DESIGN DESIGNATION** N.A.

CONTRACT NO. 66561 COUNTY TOTAL SHEET NO. FORD 37 1 SECTION 116BR-1



FUNCTIONAL CLASSIFICATION: MINOR ARTERIAL (RURAL) 55 mph

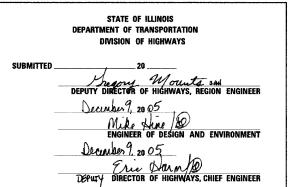
DESIGN SPEED: POSTED SPEED: ADT: PV:

SU:

MU:

55 mph 1200 (2006) 79.5% 9.1%

11.4% STATE OF ILLINOIS



PRINTED BY THE AUTHORITY OF THE STATE OF ILLINOIS



SCALE 7 MILES 5000 15000 35000 FEE

GROSS LENGTH = 228.5 FT. = 0.043 MI. NET LENGTH = 228.5 FT. = 0.043 MI.

FAP	SECTION	C	OUNT	Y	SHEETS	SHEET NO.
681	116BR-1		FORD		37	2
STA.		TO	STA.			
FED. RO	AD DIST. NO.	ILLINOIS	FED.	AID	PROJECT	

LIST OF ILLINOIS DOT HIGHWAY STANDARDS

STANDARD NO.	DESCRIPTION
000001-04	STANDARD SYMBOLS, ABBREVIATIONS AND PATTERNS
001001	AREAS OF REINFORCEMENT BARS
001006	DECIMAL OF AN INCH AND OF A FOOT
280001-02	TEMPORARY EROSION CONTROL SYSTEMS
420001-06	PAVEMENT JOINTS
420401~05	BRIDGE APPROACH PAVEMENT
421001-01	BAR REINFORCEMENT FOR CRC PAVEMENT
482011-01	BIT. SHLD. STRIPS/SHLDS. WITH RESURFACING OR WIDENING AND RESURFACING PROJECTS
515001-02	NAME PLATE FOR BRIDGES
601101	CONCRETE HEADWALL FOR PIPE DRAIN
630001-05	STEEL PLATE BEAM GUARDRAIL
631032-01	TRAFFIC BARRIER TERMINAL, TYPE 6A
635001	DELINEATORS
635006-02	REFLECTOR AND TERMINAL MARKER PLACEMENT
635011-01	REFLECTOR MARKER AND MOUNTING DETAILS
701001-01	OFF-RD OPERATION 2L, 2W, 4.5 m (15') MIN. AWAY FOR SPEEDS \geq 45 MPH
701006-02	OFF-RD OPERATIONS 2L, 2W 4.5 m (15') TO PAVEMENT EDGE FOR SPEEDS \geq 45 MPH
701011-01	OFF-RD MOVING OPERATIONS 2L, 2W DAY ONLY FOR SPEEDS \geq 45 MPH
701301-02	LANE CLOSURE 2L, 2W SHORT TIME OPERATIONS
701306-01	LANE CLOSURE 2L, 2W, SLOW MOVING OPERATIONS DAY ONLY, FOR SPEEDS > 45 MPH
701311-02	LANE CLOSURE 2L, 2W MOVING OPERATIONS - DAY ONLY
702001-05	TRAFFIC CONTROL DEVICES
720001	SIGN PANEL MOUNTING DETAILS
720006	SIGN PANEL ERECTION DETAILS
720011	METAL POSTS FOR SIGNS, MARKERS & DELINEATORS
729001	APPLICATIONS OF TYPES A & B METAL POSTS (FOR SIGNS & MARKERS)
780001-01	TYPICAL PAVEMENT MARKINGS
781001-02	TYPICAL APPLICATIONS RAISED REFLECTIVE PAVEMENT MARKERS

GENERAL NOTES

- THE TOP 6" OF TOPSOIL SHALL BE STRIPPED FROM ALL AREAS WITHIN THE CONSTRUCTION LIMITS OUTSIDE OF THE PROPOSED ABUTMENTS. THIS MATERIAL SHALL BE STOCKPILED AT A LOCATION APPROVED BY THE ENGINEER AND REPLACED AFTER MAJOR GRADING OPERATIONS ARE COMPLETE. THIS WORK WILL BE PAID FOR AS TOPSOIL EXCAVATION AND PLACEMENT.
- 2. THE ENGINEER SHALL BE THE SOLE JUDGE CONCERNING CURING TIME FOR THE VARIOUS BITUMINOUS LIFTS.
- 3. EXCEPT AS NOTED ON THE PLANS, PAVEMENT GRADES SHOWN ARE AT THE TOP OF PAVEMENT SURFACES.
- 4. ALL SAWCUTTING OF EXISTING PAVEMENT SHALL BE CONSIDERED INCLUDED IN THE PAY ITEMS INVOLVED. THE MINIMUM SAW DEPTH IN THE PAVEMENT SHALL BE $1/2^{\prime\prime}$ UNLESS OTHERWISE NOTED.
- 5. WHERE SECTION OR SUBSECTION MONUMENTS ARE ENCOUNTERED, THE ENGINEER SHALL BE NOTIFIED BEFORE SUCH MONUMENTS ARE REMOVED. THE CONTRACTOR SHALL PROTECT AND CAREFULLY PRESERVE ALL MONUMENTS UNTIL AN AUTHORIZED SURVEYOR OR AGENT HAS WITNESSED OR OTHERWISE REFERENCED THEIR LOCATION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR HAVING AN AUTHORIZED SURVEYOR REESTABLISH ANY SECTION OR SUBSECTION MONUMENTS DESTROYED BY HIS OPERATIONS.
- THE THICKNESS OF BITUMINOUS MIXTURES SHOWN ON THE PLANS IS THE NOMINAL THICKNESS. DEVIATIONS FROM THE NOMINAL THICKNESS WILL BE PERMITTED WHEN SUCH DEVIATIONS OCCUR DUE TO IRREGULARITIES IN THE EXISTING SURFACE OR BASE ON WHICH THE BITUMINOUS MAY THE PRIMINIOUS AND THE PRIMINIOUS OF THE PLANS OF THE PRIMINIOUS OF THE TO IRREGULARITIES IN THE EXISTING S THE BITUMINOUS MIXTURE IN PLACED.
- BEFORE ORDERING PIPE CULVERTS OR PIPE DRAINS, THE CONTRACTOR SHALL CONSULT THE ENGINEER FOR EXACT LENGTHS.
- 8. ANY REFERENCE TO A STANDARD IN THESE PLANS SHALL BE INTERPRETED TO MEAN THE EDITION AS INDICATED BY THE SUBNUMBER LISTED ON THE INDEX OF SHEETS OR THE COPY OF THE STANDARD
- 9. THE FOLLOWING RATES OF APPLICATION HAVE BEEN USED IN CALCULATING PLAN QUANTITIES:

GRANULAR MATERIALS BITUMINOUS MATERIALS PRIME COAT BITUMINOUS RESURFACING SHORT TERM PAVEMENT MARKING

2.05 TON/CU YD 0.08 GAL/SO YD 112 LBS/SO YD/INCH 10 FT/100 FT OF APPLICATION

- 10. ABANDONED UNDERGROUND UTILITIES THAT CONFLICT WITH CONSTRUCTION SHALL BE DISPOSED OF OUTSIDE THE LIMITS OF THE RIGHT OF WAY ACCORDING TO ARTICLE 202.03 OF THE STANDARD SPECIFICATIONS AND AS DIRECTED BY THE ENGINEER. THIS WORK WILL NOT BE PAID FOR SEPARATELY BUT SHALL BE INCLUDED IN THE COST OF EARTH EXCAVATION.
- 11. ALL DISTURBED AREAS WITHIN THE CONSTRUCTION LIMITS SHALL BE FERTILIZED AND SEEDED. SEEDING SHALL BE CLASS 2A ACCORDING TO THE APPLICABLE ARTICLES OF SECTION 250 OF THE STANDARD SPECIFICATIONS. SEEDING SHALL NOT BE PERMITTED AT ANY TIME WHEN THE GROUND IS FROZEN, WET, OR IN AN UNTILLABLE CONDITION. LOCATIONS TO BE SEEDED WILL BE DETERMINED BY THE ENGINEER.
- 12. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROTECTING UTILITY PROPERTY FROM CONSTRUCTION OPERATIONS AS OUTLINED IN ARTICLE 107.31 OF THE STANDARD SPECIFICATIONS. THE JULIE NUMBER IS 800-892-0123. A MINIMUM OF 48 HOURS ADVANCE NOTICE
- 13. EXISTING TRAFFIC BARRIER TERMINALS TO BE REMOVED SHALL BE PAID FOR AS GUARDRAIL REMOVAL.
- 14. FOR STABILIZATION, ALL TYPE III BARRICADES SHALL REQUIRE A MINIMUM OF FOUR SANDBAGS PER BARRICADE.
- 15. ALL ELEVATIONS REFERRING TO U.S.G.S. MEAN SEA LEVEL DATUM.
- 16. ONLY THOSE TREES DESIGNATED BY THE ENGINEER SHALL BE REMOVED. THE CONTRACTOR SHALL PROTECT ALL REMAINING TREES FROM DAMAGE DUE TO HIS OPERATIONS.
- 17. SKIP-DASH LINES FOR PAINT PAVEMENT MARKING SHALL BE 6" WIDE.
- 18. EXCELSIOR BLANKET SHALL BE USED AT ALL EROSION CONTROL BLANKET LOCATIONS.
- 19. THE CURBS SHOWN ON STANDARD 420401 ARE NOT REQUIRED AND SHALL NOT BE CONSTRUCTED.
- 20. THE CONTRACTOR SHALL CONTACT J.U.L.I.E. AT LEAST 48 HOURS PRIOR TO EXCAVATION TO DETERMINE WHICH UTILITIES ARE IN THE AREA.

REVIEWED BY:

DISTRICT STUDIES & PLANS ENGINEER (MONK)

NOVEMBER 4, 2005

EXAMINED BY:

DISTRICT CONSTRUCTION ENGINEER

GENERAL NOTES AND STANDARDS FAP RTE 681 (IL 116) SECTION 116BR-1 FORD COUNTY

DESIGNED BY: MTD 7/05 DRAWN BY: DWH 7/05
CHECKED BY: MTD 7/05 APPROVED BY: RDP 8/05

SUMMARY OF QUANTITIES

			80% FED
			CONSTRUCTION
CODE NO.	ITEM	UNIT	TYPE CODE
			X080-2A
	EARTH EXCAVATION	CU YD	60
	POROUS GRANULAR EMBANKMENT	CU YD	72
21101505	TOPSOIL EXCAVATION AND PLACEMENT	CU YD	5
25000210	SEEDING, CLASS 2A	ACRE	0.10
25000350	SEEDING, CLASS 7	ACRE	0.10
25000400	NITROGEN FERTILIZER NUTRIENT	POUND	9
25000500	PHOSPHORUS FERTILIZER NUTRIENT	POUND	9
25000600	POTASSIUM FERTILIZER NUTRIENT	POUND	9
25100115	MULCH, METHOD 2	ACRE	0.1
25100630	EROSION CONTROL BLANKET	SQ YD	10
28000250	TEMPORARY EROSION CONTROL SEEDING	POUND	20
28000400	PERIMETER EROSION BARRIER	FOOT	430
28000500	INLET AND PIPE PROTECTION	EACH	1
40600100	BITUMINOUS MATERIALS (PRIME COAT)	GALLON	12
42001165	BRIDGE APPROACH PAVEMENT	SQ YD	214
	PROTECTIVE COAT	SQ YD	214
	BRIDGE APPROACH PAVEMENT CONNECTOR (FLEXIBLE)	SQ YD	43
	BITUMINOUS SURFACE REMOVAL (VARIABLE DEPTH)	SQ YD	150
	PAVEMENT REMOVAL	SQ YD	215
	AGGREGATE SHOULDERS, TYPE B	TON	10
	REMOVAL OF EXISTING SUPERSTRUCTURES	EACH	1
	CONCRETE REMOVAL	CU YD	4.4
	STRUCTURE EXCAVATION	CU YD	72
	CONCRETE STRUCTURES	CU YD	6.0
	BRIDGE DECK GROOVING	SQ YD	341
50300300	PROTECTIVE COAT	SQ YD	364
X5030305	CONCRETE WEARING SURFACE, 5"	SQ YD	364
50301245	FORMED CONCRETE REPAIR (DEPTH EQUAL TO OR LESS THAN 5")	SQ FT	457.1
50400305	PRECAST PRESTRESSED CONCRETE DECK BEAMS (17" DEPTH)	SQ FT	3264
50800205	REINFORCEMENT BARS, EPOXY COATED	POUND	5640
50901005	STEEL BRIDGE RAIL, TYPE SM	FOOT	205
51500100	NAME PLATES	EACH	1
58700200	BRIDGE SEAT SEALER	SQ FT	48
59000100	EPOXY CRACK SEALING	FOOT	221
60100060	CONCRETE HEADWALL FOR PIPE DRAINS	EACH	4
63100087	TRAFFIC BARRIER TERMINAL, TYPE 6A	EACH	4

ES	C	A INC.
DESIGNED BY	MTD	7/05
DRAWN BY:	DWH	7/05
CHECKED BY:	MTD	7/05
APPROVED BY	RDP	8/05

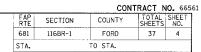
| CONTRACT NO. 66561 | FAP | SECTION | COUNTY | TOTAL | SHEET | SHOOT | SHOTT | SHOTT

SUMMARY OF QUANTITIES

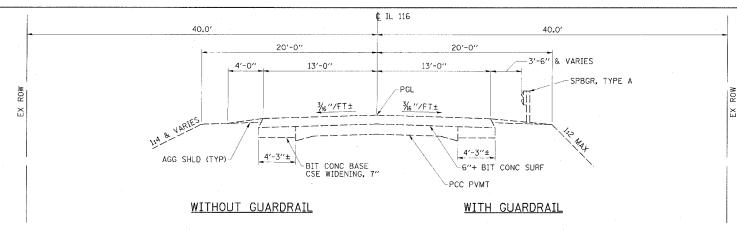
	SUMMANT OF QUANTITIES		80% FEP
CODE NO.	ITEM	UNIT	TYPE CODE 20% STATE X080-2A
	GUARDRAIL REMOVAL	FOOT	128
	REMOVE AND RE-ERECT STEEL PLATE BEAM GUARD RAIL, TYPE A	FOOT	100
67000400	ENGINEER'S FIELD OFFICE, TYPE A	CAL MO	4
67100100	MOBILIZATION	L SUM	1
70100460	TRAFFIC CONTROL AND PROTECTION, STANDARD 701306	L SUM	1
70300100	SHORT-TERM PAVEMENT MARKING	FOOT	48
70301000	WORK ZONE PAVEMENT MARKING REMOVAL	SQ FT	16
78001110	PAINT PAVEMENT MARKING - LINE 4"	FOOT	457
78001130	PAINT PAVEMENT MARKING - LINE 6"	FOOT	60
78100105	RAISED REFLECTIVE PAVEMENT MARKER (BRIDGE)	EACH	2
	GUARDRAIL MARKERS, TYPE A	EACH	4
	BARRIER WALL MARKERS, TYPE B	EACH	2
	BITUMINOUS CONCRETE SURFACE COURSE, SUPERPAVE, MIX "C", N50	TON	12.6
	i		
	TRAFFIC CONTROL AND PROTECTION FOR TEMPORARY DETOUR	L SUM	11
	STRIP SEAL EXPANSION JOINT ASSEMBLY	FOOT	74
	ASBESTOS BEARING PAD REMOVAL	EACH	44
Z0002600	BAR SPLICERS	EACH	64
-			
			·

* SPECIALTY ITEM

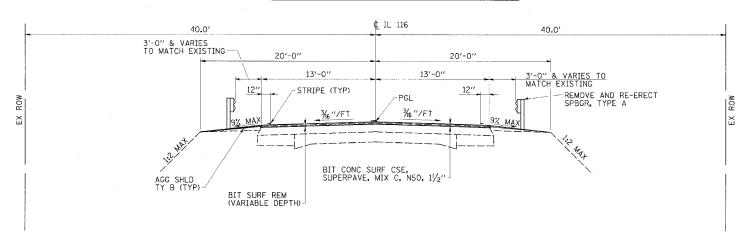
SUMMARY OF QUANTITIES
FAP RTE 681 (IL 116)
SECTION 116BR-1
FORD COUNTY



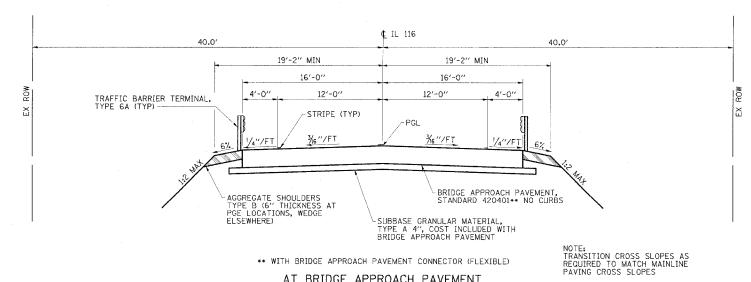
FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT



EXISTING TYPICAL ROADWAY SECTION STA. 54+00 TO 67+00 BRIDGE OMISSION STA. 60+00 TO 61+06



PROPOSED TYPICAL ROADWAY SECTION STA. 59+50 TO 59+64.63 STA. 61+41.37 TO 61+78.5



** WITH BRIDGE APPROACH PAVEMENT CONNECTOR (FLEXIBLE)

AT BRIDGE APPROACH PAVEMENT

PROPOSED TYPICAL ROADWAY SECTION

STA 59+64.63 TO 61+41.37 BRIDGE OMISSION STA 60+00.00 TO 61+06.00

TYPICAL SECTIONS FAP ROUTE 681 (IL 116) SECTION 116BR-1 FORD COUNTY

BITUMINOUS MIXTURES REQUIREMENTS

	SUPERPAVE BINDER	SUPERPAVE SURFACE
PG GRADE	PG64-22	PG64-22
MAX % RAP ALLOWABLE ***	25%	15%
DESIGN AIR VOIDS	4.0% @ N50	4.0% @ N50
MIXTURE COMPOSITION	IL 19.0	IL 12.5 OR IL 9.5
FRICTION AGGREGATE	N.A.	MIXTURE C
PLANT CONTROL LIMITS	CLASS I	CLASS I
DENSITY TEST METHOD	CORES/NUCLEAR	CORES/NUCLEAR

*** IF RAP OPTION IS SELECTED, THE ASPHALT CEMENT GRADE MAY NEED TO BE ADJUSTED. THIS WILL BE DETERMINED BY THE ENGINEER.

ESCA CONSULTANTS, INC. DESIGNED BY: MTD 7/05
DRAWN BY: JDK 7/05
CHECKED BY: MTD 7/05
APPROVED BY: RDP 8/05

EARTHWORK SCHEDULE								
LOCATION	SUITABLE EARTH EXCAVATION	SUITABLE EARTH EXCAVATION ADJUSTED FOR SHRINKAGE	SUITABLE INCIDENTAL EXCAVATION MATERIAL	SUITABLE INCIDENTAL EXC. MATERIAL ADJUSTED FOR SHRINKAGE	EMBANKMENT (NOT A PAY ITEM)	EARTHWORK BALANCE WASTE (+) OR SHORTAGE (~)		
	CU. YD	CU YD	CU YD	CU YD	CU YD	CU_YD		
NW QUADRANT CUTS & FILLS SW QUADRANT CUTS & FILLS NE QUADRANT CUTS & FILLS SE QUADRANT CUTS & FILLS	15 15 15 15	11 11 11			2 2 2 2	+9 +9 +9 +9		
STRUCTURE EXCAVATION CONC PAD UNDER PYMT CONNECTOR	Accessed the Colon Parks		72 12	54 9		+54 +9		
TOTALS	60	44	84	63	8	199		

NOTES: 1. EXCAVATION USED AS EMBANKMENT - (SUITABLE EARTH EXCAVATION + SUITABLE INCIDENTAL EXCAVATION)+0.75 2. TOPSOIL EXCAVATION AND PLACEMENT NOT INCLUDED IN THE ABOVE NUMBERS

EROSION CONTROL SCHEDULE								
LOCATION	EROSION CONTROL BLANKET	PERIMETER EROSION BARRIER	TEMPORARY EROSION CONTROL SEEDING (2 APPLICATIONS)	INLET & PIPE PROTECTION				
-	SQ YD	FOOT	POUND	EACH				
STRUCTURE NO. 027-0044 ~ NORTHWEST	2.5	125	4					
STRUCTURE NO. 027-0044 - SOUTHWEST STRUCTURE NO. 027-0044 - NORTHEAST	2.5 2.5	75 95	4 6					
STRUCTURE NO. 027-0044 - SOUTHEAST	2,5	135	6					
STA 62-46.1, 33.8' LT	· London A. T.			1				
Salle Comment of Comme								
W.C. 194								
A DO NOTE OF THE PARTY OF THE P								
TOTALS	10.0	430	20	1				

SEEDING SCHEDULE								
LOCATION	SEEDING, CLASS 2A	SEEDING, CLASS 7	NITROGEN FERTILIZER NUTRIENT	PHOSPHORUS FERTILIZER NUTRIENT	POTASSIUM FERTILIZER NUTRIENT	MULCH, METHOD 2		
	ACRE	ACRE	POUND	POUND	POUND	ACRE		
STA 59+50 TO STA 60+13, LT STA 59+50 TO STA 59+92, RT STA 60+92 TO STA 61+78.5, RT STA 61+13 TO STA 61+78.5, LT	0.02 0.02 0.03 0.03	0.02 0.02 0.03 0.03	1.8 1.8 2.7 2.7	1.8 1.8 2.7 2.7	1.8 1.8 2.7 2.7	0.02 0.02 0.03 0.03		
TOTALS	0,10	0.10	9,0	9.0	9.0	0.10		

AGGREGATE SHOULDERS, TYPE E	SCHEDULE
LOCATION	TON
STA 59+50 TO STA 60-13. LT STA 59+50 TO STA 59+92. RT STA 60+92 TO STA 61-78.5. RT STA 61+13 TO STA 61+78.5. LT	2.5 2.0 3.0 2.5
TOTAL	10.0

REMOVAL SCHEDULE	
LOCATION	PAVEMENT REMOVAL
STA 59+64.63 TO STA, 60+01.78 STA, 61+04.22 TO STA 61+41.37	SQ YD 107.5 107.5
TOTAL	215

	PAVEMENT MARKING	SCHEDULE		
LOCATION	DESCRIPTION	SHORT-TERM PAVEMENT MARKING	PAINT PAVEMENT MARKING - LINE	
	'	(2 APPLICATIONS)	4"	6′′
		FOOT	FOOT	FOOT
STA 59+50 TO STA 61+78,5 STA 59+50 TO STA 61+78,5	SOLID WHITE EDGE LINE SKIP-DASH YELLOW CENTERLINE	48	457	60
	TOTALS	48	457	60

PAVEMENT	MARK	(ERS	SCHEDULE
LOCATION			RRPM (BRIDGE)
			EACH
STA 60÷20			1
STA 61+00			1
	TOTAL		2

BRIDGE APPROACH PA	VEMENT	SCHEDUL	E
LOCATION	BRIDGE APPROACH PAVEMENT	PROTECTIVE COAT	CONNECTOR (FLEXIBLE)
	SQ YD	SQ YD	SQ YD
STRUCTURE NO. 027-0044 - WEST APPROACH STRUCTURE NO. 027-0044 - EAST APPROACH	107 107	107 107	21.5 21.5
TOTALS	214	214	43

GUARDRAIL SCHEDULE						
LOCATION	REMOVE AND RE-ERECT SPBGR, TYPE A	TRAFFIC BARRIER TERMINAL, TYPE 6A	GUARDRAIL MARKERS, TYPE A	BARRIER WALL MARKERS, TYPE B	STEEL BRIDGE RAIL, TYPE SM	
	FOOT	EACH	EACH	EACH	FOOT	
STRUCTURE NO. 027-0044 - NORTHWEST	25	1	1			
STRUCTURE NO. 027-0044 - SCUTHWEST	25	11	1			
STRUCTURE NO. 027-0044 - NORTHEAST	25	1	1			
STRUCTURE NO. 027-0044 - SOUTHEAST STRUCTURE NO. 027-0044 - BRIDGE	25	1	1	2	205	
TOTALS	100	4	4	2	205	

GUARDRAIL REMOVAL SCHEDULE		
LOCATION	FOOT	
STRUCTURE NO. 027-0044 - NORTHWEST STRUCTURE NO. 027-0044 - SOUTHWEST STRUCTURE NO. 027-0044 - NORTHEAST STRUCTURE NO. 027-0044 - SOUTHEAST	32 32 32 32 32	
TOTAL	128	

			·	INO	RACI	NU. bb:
FAP RTE	SECTION	C	OUNT	Y	TOTAL	SHEET NO.
681	116BR-1		FORD		37	5
STA.		TO	STA.			
FED. ROAL	DIST. NO.	ILLINOIS	FED.	AID	PROJEC	Т

WORK ZONE PAVEME	NT MARKING	REMOVAL SCHEDULE
LOCATION	PAVEMENT MARKING DESCRIPTION	WORK ZONE PAVEMENT MARKING REMOVAL
CENTERLINE	SHORT-TERM	SQ FT
	TO	DTAL 16

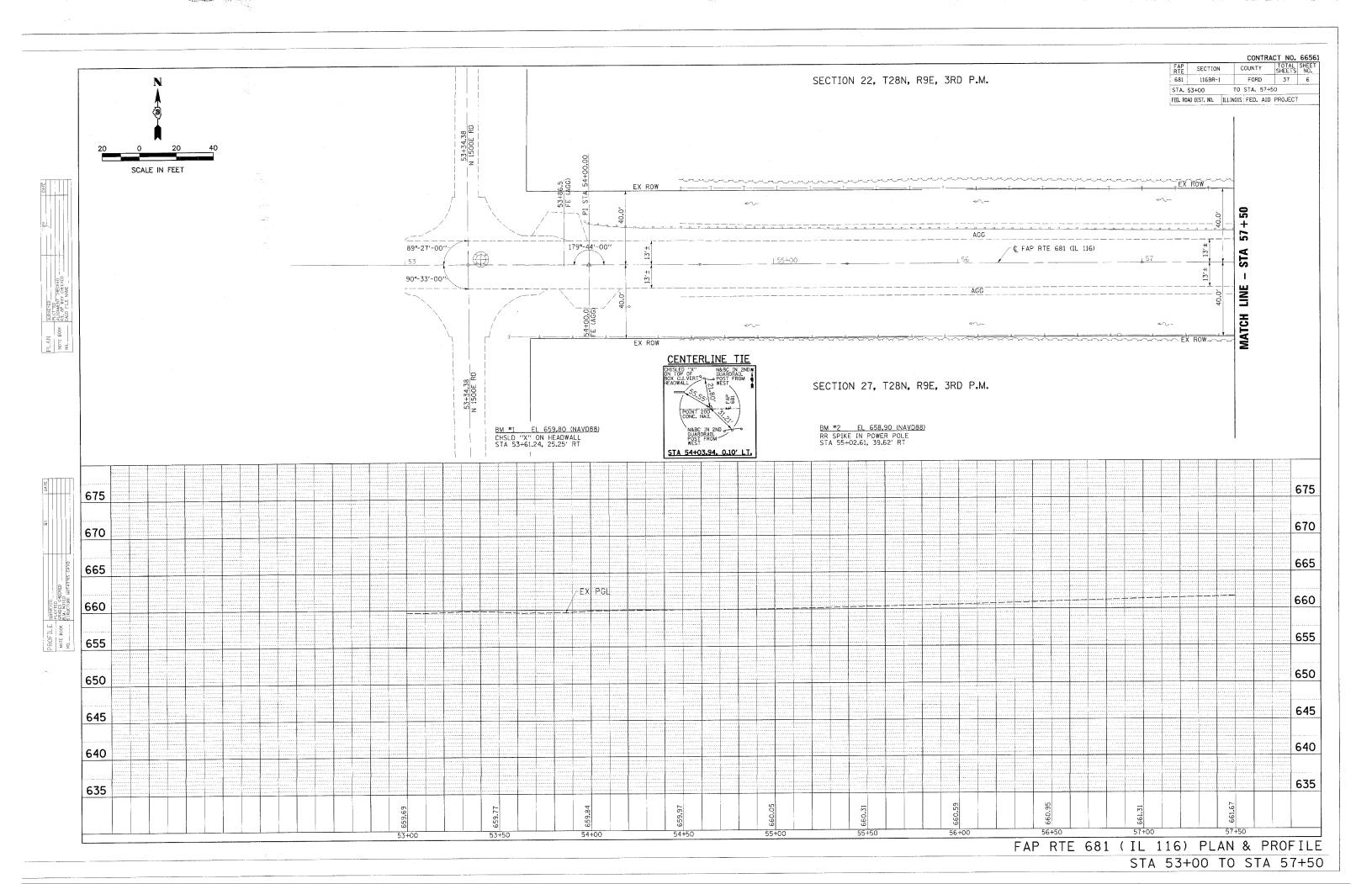
DRAINAGE SCHEDULE	
LOCATION	CONCRETE HEADWALL FOR PIPE DRAINS
	EACH
STRUCTURE NO. 027-0044 - NW CORNER	1
STRUCTURE NO. 027-0044 - SW CORNER	1
STRUCTURE NO. 027-0044 - NE CORNER	1
STRUCTURE NO. 027-0044 - SE CORNER	1
TOTAL	4

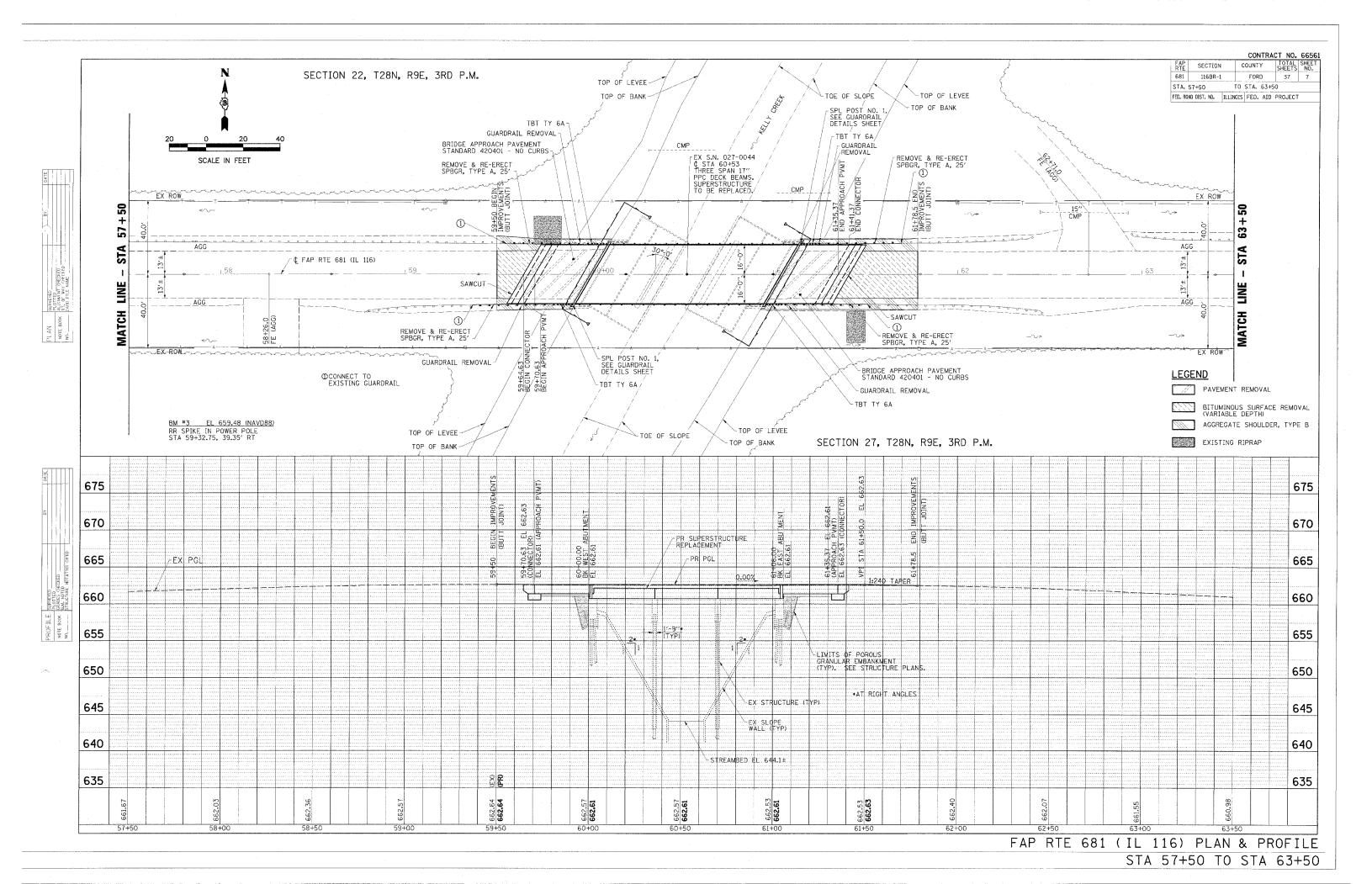
BIT SURF REMOVAL (VARIABLE DEPTH) SC	HEDULE
LOCATION	SQ YD
STA 59+50 TO STA 59+64.63 STA 61+41.37 TO STA 61+78.5	42.5 107.5
TOTAL	150.0

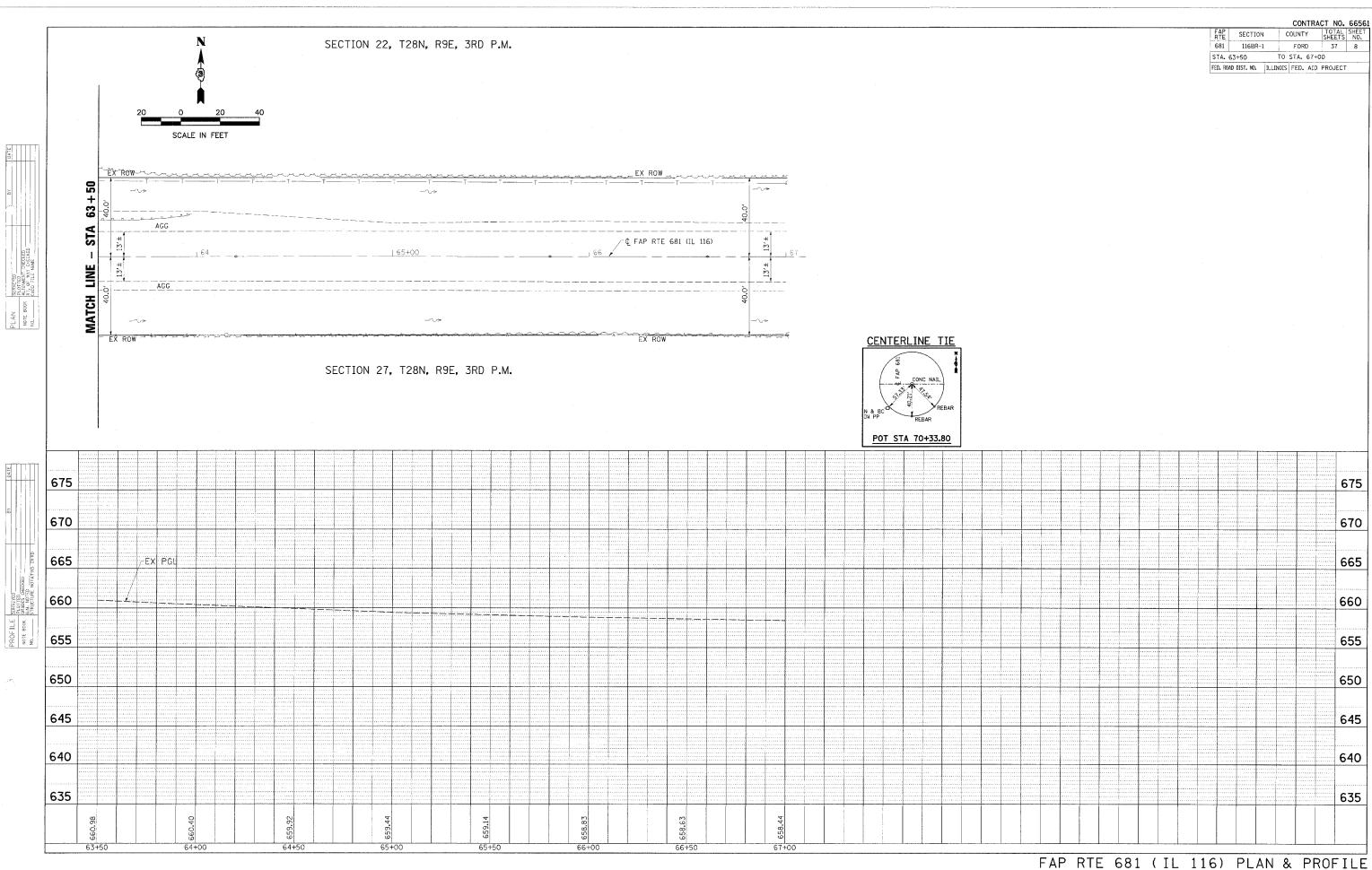
PAVIN	G SCHEDULE	
LOCATION	BITUMINOUS MATERIALS (PRIME COAT)	BITUMINOUS CONCRETE SURFACE COURSE, SUPERPAVE, MIX "C", N50
	GALLON	TON
STA 59-50 TO STA 59-64.63 STA 61-41.37 TO STA 61+78.5	3.4 8.6	3.6 9.0
TOTALS	12.0	12.6

SCHEDULES OF QUANTITIES
FAP RTE 681 (IL 116) SECTION 116BR-1 FORD COUNTY

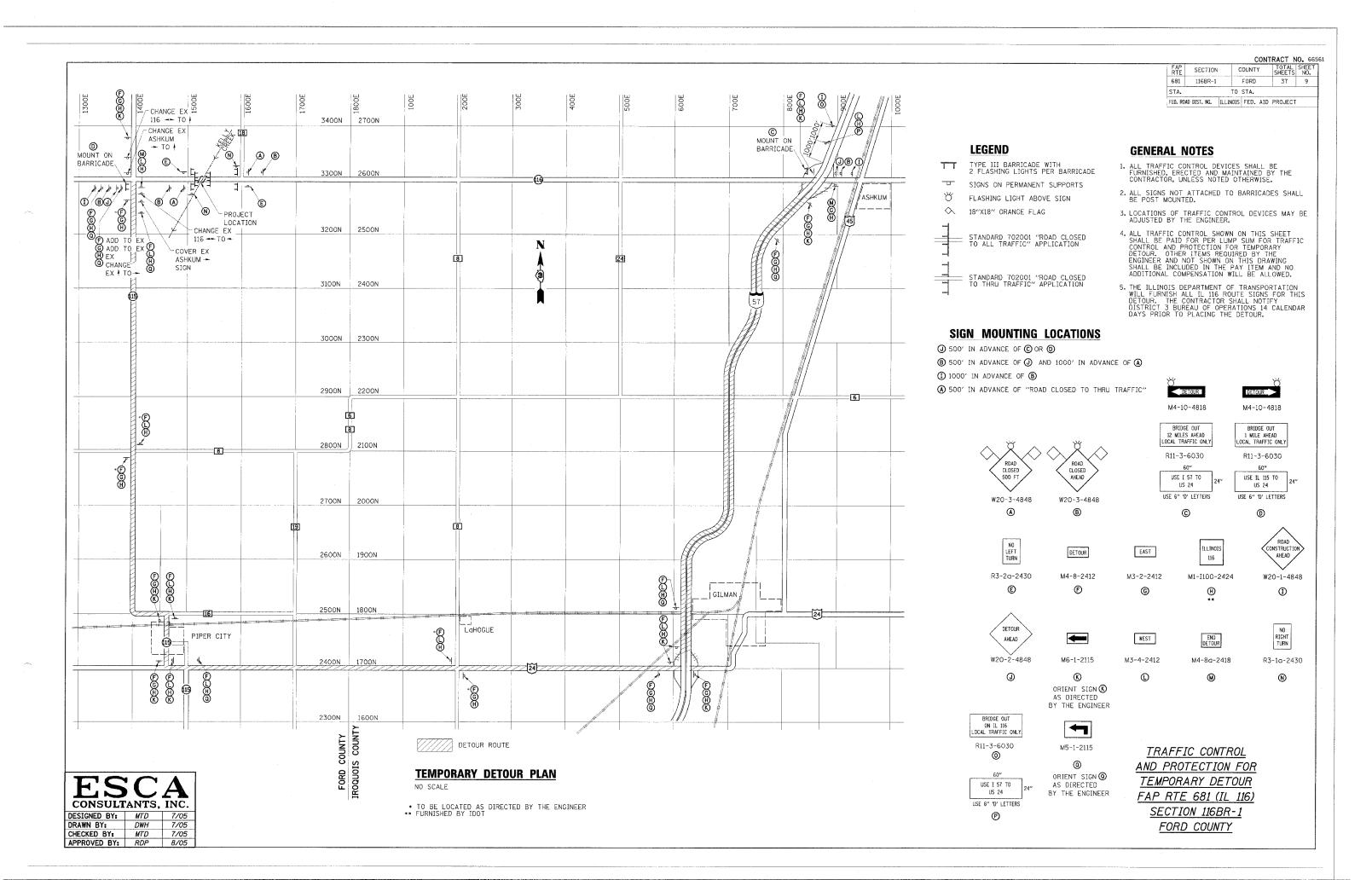
ES	CANTS,	A INC.
DESIGNED BY:	MTD	7/05
DRAWN BY:	DWH	7/05
CHECKED BY:	MTD	7/05
APPROVED BY:	RDP	8/05

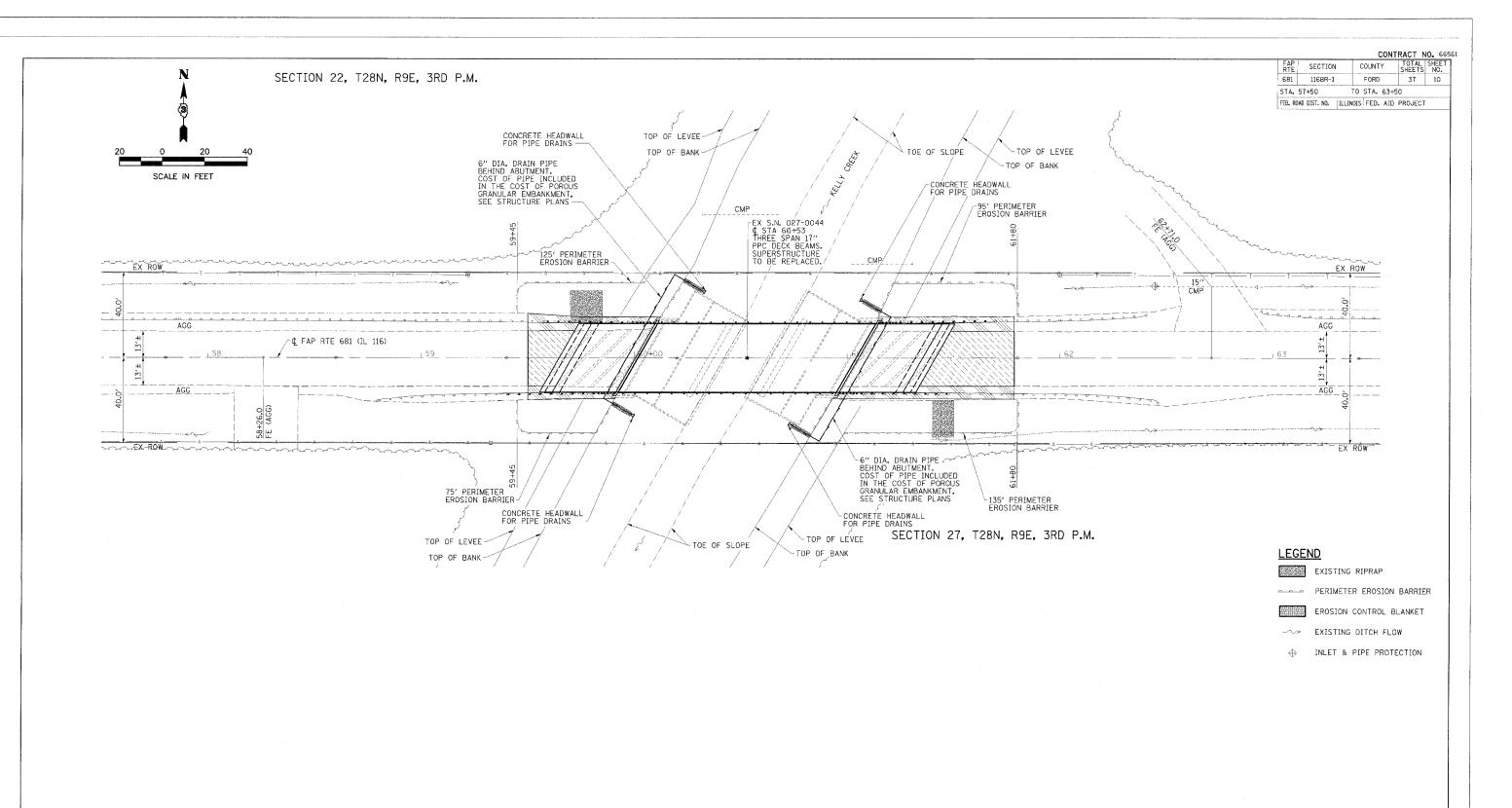






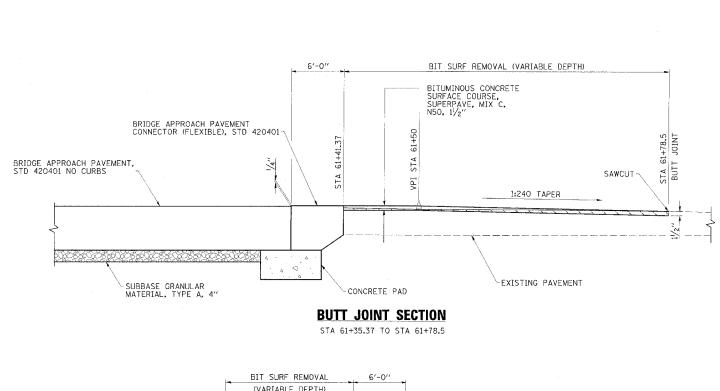
STA 63+50 TO STA 67+00

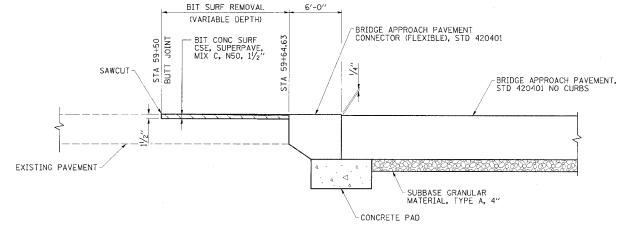




ESCA
CONSULTANTS, INC.
DESIGNED BY: MTD 7/05
DRAWN BY: DWH 7/05
CHECKED BY: MTD 7/05
APPROVED BY: RDP 8/05

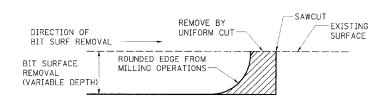
EROSION CONTROL
AND DRAINAGE PLAN
FAP RTE 681 (IL 116)
SECTION 116BR-1
FORD COUNTY





BUTT JOINT SECTION

STA 59+50 TO STA 59+64.63



DETAIL AT BUTT JOINT

CONSULTANTS, INC.

DESIGNED BY: MTD 7/05

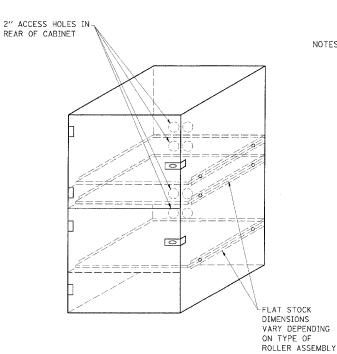
DRAWN BY: HAG 7/05

CHECKED BY: MTD 7/05

APPROVED BY: RDP 8/05

WHEN MILLING OPERATIONS PRODUCE A ROUNDED EDGE, THEN A SAWCUT SHALL BE USED TO MANUFACTURE A PERPENDICULAR EDGE AS SHOWN IN THE DETAIL. THE COST OF ALL WORK SHOWN IN THE DETAIL IS INCLUDED IN BITUMINOUS SURFACE REMOVAL (VARIABLE DEPTH). THE ENGINEER SHALL BE THE SOLE JUDGE CONCERNING THE USE OF THIS DETAIL.

STA. TO STA. FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT 1/2" X 11/2" FLAT STOCK WELD TO FORM DOOR FRAME AND ALSO AROUND PERIMETER SHELF SIDE VIEW **SHELF FRONT VIEW** CL. BETWEEN TOP SHELVES WELD BACKSTOP TO MAINTAIN 2" CLEARANCE BETWEEN SHELF AND CABINET * ALLOWS FOR 1/8" CLEARANCE AT TOP AND BOTTOM OF CABINET AND BETWEEN DOORS. SIDE VIEW FRONT VIEW



NOTES: 1. USE 16 GAUGE STEEL FOR CABINET.

- 2. THE TOP SHELF SHALL SLIDE IN OR OUT WITH THE TOP DOOR OPEN.
- 3. ALL HINGES AND HASPS WILL BE WELDED TO THE CABINET.
- 4. ALL EDGES SHALL BE GROUND SMOOTH.
- 5. TWO (2" DIA.) ACCESS HOLES WILL BE REQUIRED FOR EACH SHELF.
- 6. CABINET SHALL BE PAINTED WITH TWO COATS OF FLAT PAINT.
- 7. 2 EACH MATCHING KEY PADLOCKS, WITH 3 KEYS PROVIDED, MASTER MODEL 3 T OR EQUIVALENT.
- 8. 4 EACH PLAIN STEEL, NON-REMOVABLE PIN, NO HOLE 4"X4" SQUARE CORNER HINGES TO BE WELDED ON.
- 9. 2 EACH EXTRA HEAVY, PLAIN STEEL, FIXED STAPLE, NO HOLE, 7 1/4 " HASPS TO BE WELDED ON.

LOCKABLE COMPUTER CABINET

MISCELLANEOUS DETAILS
FAP RTE 681 (IL 116)
SECTION 116BR-1
FORD COUNTY

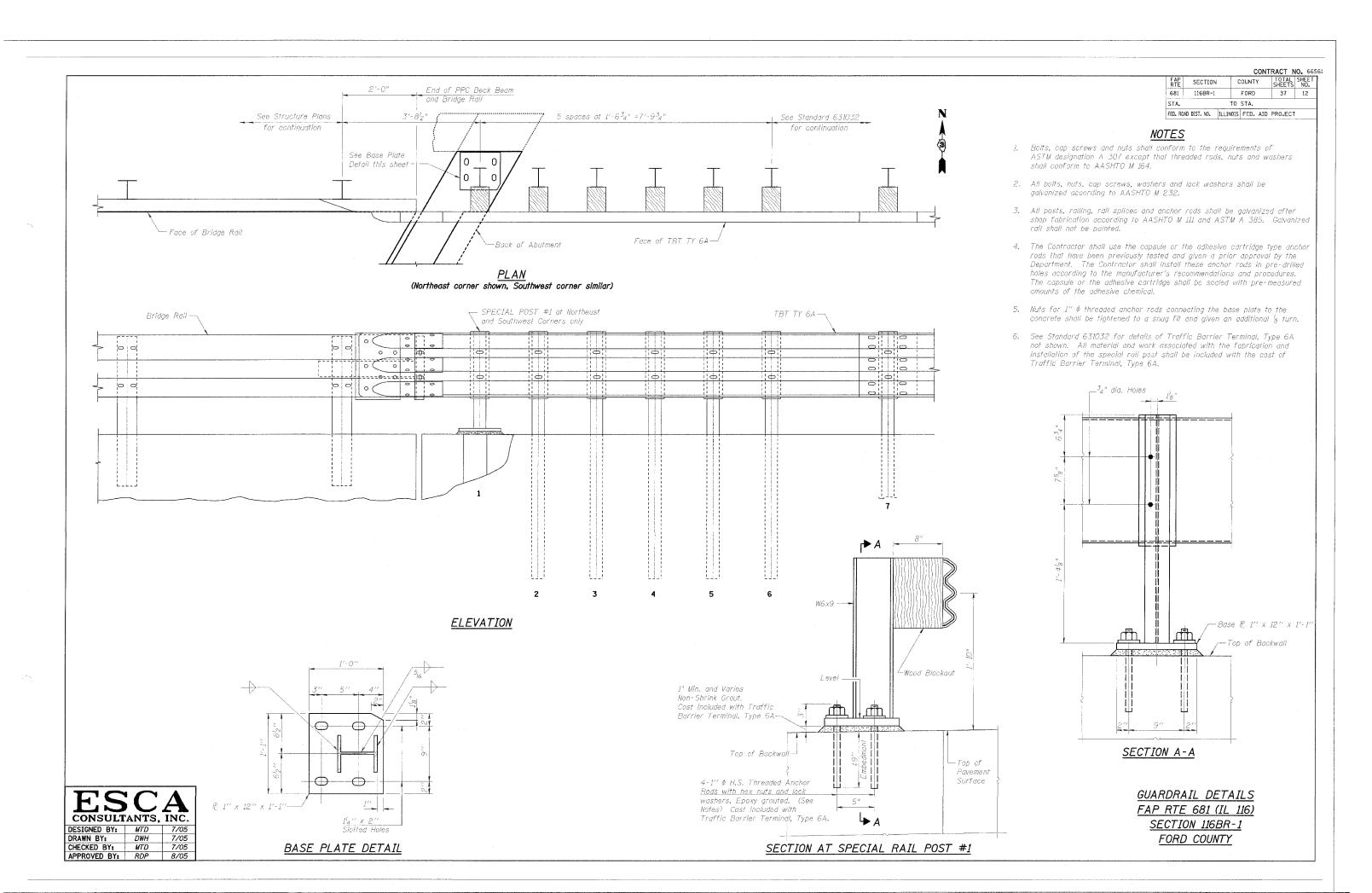
CONTRACT NO. 66561

TOTAL SHEET SHEETS NO.

SECTION

COUNTY

FORD



BENCHMARK: Railroad spike in power pole, station 59+32.75, 39.35' Rt. Elev. 659.48

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

TOTAL BIGGET 37 13 DWG. NO. 1 OF 15 CONTRACT NO. 66561

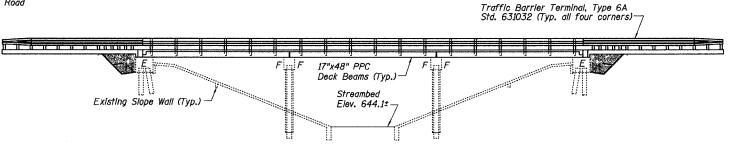
EXISTING STRUCTURE: SN 027-0044 was originally built in 1971 as Section 116 BR. The superstructure consists of 3 simple spans of 17" PPC deck beams on pile bent abutments and piers. The back-to-back abutments dimension measures 106'-0" while the out-to-out width measures 33'-0". The existing superstructure shall be removed and replaced. Road closure shall be used during construction.

No salvage

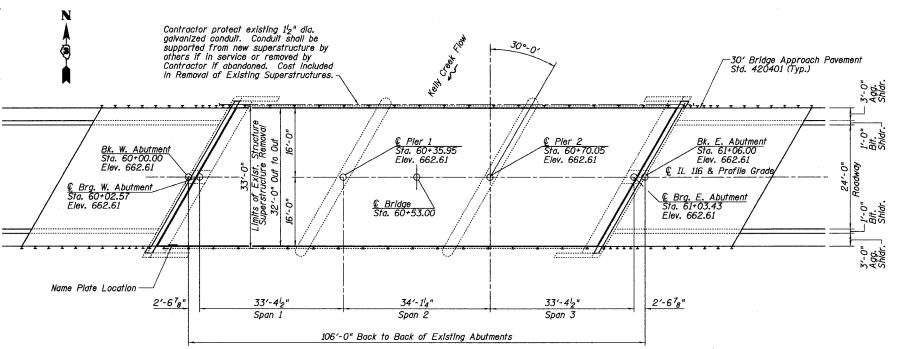
STATION 60+53 BUILT 200_ BY STATE OF ILLINOIS F.A.P. RT. 681 SEC. 116BR-1 LOADING HS20 STR. NO. 027-0044

NAME PLATE See Std. 515001

Note: Existing Name Plate shall be cleaned and relocated adjacent to the new plate. Cost included with Name Plates.



ELEVATION



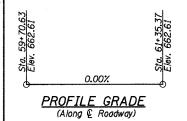
STRUCTURE INDEX OF SHEETS

General Plan	Dwg. No. 1 of 15
General Data	Dwg. No. 2 of 15
Superstructure	Dwg. No. 3 of 15
Superstructure Details	Dwg. No. 4-5 of 15
Type SM Steel Bridge Rail	Dwg. No. 6 of 15
Strip Seal Expansion Joint	Dwg. No. 7 of 15
Anchor Bolt Details	Dwg. No. 8 of 15
West Abutment	Dwg. No. 9 of 15
East Abutment	Dwg. No. 10 of 15
Abutment Details	Dwg. No. 11 of 15
Pier 1	Dwg. No. 12 of 15
Pier 2	Dwg. No. 13 of 15
Pier Details	Dwg. No. 14 of 15
Bar Splicer Assembly Details	Dwg. No. 15 of 15

SCOPE OF WORK

- 1. Remove existing surfacing, steel railing, and deck beams.
- Seal existing cracks and repair delaminated/spalled concrete areas on substructure units.
- Repair beam bearing seats at abutments and piers as required.
- 4. Reconstruct a three-span PPCD beam superstructure with concrete wearing surface and Steel Bridge Rail Type SM, and new bridge approach pavements.

<u>PLAN</u>



CONSULTANTS, INC. DESIGNED BY: ELH 6/05
 DRAWN BY:
 CJG
 6/05

 CHECKED BY:
 ELH
 10/05

 APPROVED BY:
 RDP
 10/05

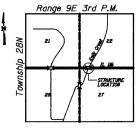


<u>DESIGN SPECIFICATION</u> 2002 AASHTO LOADING HS20-44
Allow 25 psf future wearing surface DESIGN STRESSES FIELD UNITS f'c = 5,000 psi (Concrete Wearing Surface) f'c = 3,500 psi (All concrete except CWS) fy = 60,000 psi (reinf.)

PRECAST PRESTRESSED UNITS

f'c = 5.000 psi f'cl = 4.000 psi f's = 270,000 psi ('2"\$ low lax strands) fsi = 201,960 psi ('2"\$ low lax strands)

SEISMIC DATA
Seismic Performance Category (SPC) = A
Bedrock Acceleration Coefficient (A) = 0.043g
Site Coefficient (S) = 1.2



LOCATION SKETCH

GENERAL PLAN IL 116 OVER KELLY CREEK FAP ROUTE 681 - SECTION 116BR-1 FORD COUNTY STATION 60+53.00 STRUCTURE NO. 027-0044

TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Removal of Existing Superstructures	Each	1		1
Concrete Removal	Cu. Yd.		4.4	4.4
Structure Excavation	Cu. Yd.		72	72
Concrete Structures	Cu. Yd.		6.0	6.0
Bridge Deck Grooving	Sq. Yd.	341		341
Concrete Wearing Surface, 5"	Sg. Yd.	364		364
Bridge Seat Sealer	Sq. Ft.	ĺ	48	48
Formed Concrete Repair (Depth Equal to or Less Than 5")	Sq. Ft.		457.1	457.1
Precast Prestressed Concrete Deck Beams (17" Depth)	Sq. Ft.	3264		3264
Reinforcement Bars, Epoxy Coated	Pound	4750	890	5640
Steel Bridge Rail, Type SM	Foot	205		205
Name Plates	Each	1		1
Porous Granular Embankment	Cu. Yd.		72	72
Epoxy Crack Sealing	Foot		- 221	221
Strip Seal Expansion Joint Assembly	Foot	74		74
Asbestos Bearing Pad Removal	Each		44	44
Bar Splicers	Each		64	64
Protective Coat	Sq. Yd.	364		364

102'-3" End to End of Rall Steel Bridge Rail, Type SM C Pier 1 5 rail post spaces @ 5'-9" = 28'-9" 3'-378" 2'-818" 2'-8'8" 3'-378" 5 rail post spaces @ 5'-9" = 28'-9" 5 rail post spaces @ 5'-9" = 28'-9" Traffic Barrier Traffic Barrier Terminal, Type 6A - Terminal, Type 6A See Standard 631032 See Standard 631032 1^l8" gap (±) _1^l8" gap (±) Locate Name Plates at See Dwg. No. 6 of 15 for Railing Details. Outside Face of Top Rail Tube at Southwest Corner of Bridge RAIL ELEVATION

North Railing Similar)

(Showing Inside Face of South Railing;

| MOUTE NO. | GETTON | COUNTY | GETTON | GETTON

GENERAL NOTES

- Reinforcement bars shall conform to the requirements of AASHTO M31 or M322 Grade 60.
- 2. Plan dimensions and details relative to existing structure have been taken from existing plans and are subject to nominal construction variations. It shall be the Contractor's responsibility to verify such dimensions and details in the field and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in the scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price for the work.
- 3. All construction joints shall be bonded.
- 4. Bridge Seat Sealer shall be applied to abutment bearing seats where formed concrete repairs are performed.
- All new structural steel shall be shop painted with an inorganic zinc rich primer per AASHTO M300 Type 1 unless noted otherwise.
- 6. Side retainers shall be AASHTO M270 Grade 36 minimum.
- 7. No work will be allowed in the stream.
- 8. The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project.
- 9. The Contractor is advised that the existing PPC Deck Beams are in a deteriorated condition with reduced load carrying capacity. It is the Contractor's responsibility to account for the condition of the beams when developing construction procedures for removal and replacement of the superstructure.
- 10. If the contractor's procedure for existing beam removal or placement of new beams involves placement of cranes or other heavy equipment on new beams, a detailed procedure shall be submitted to the Engineer for approval. The procedure shall include calculations, prepared and sealed by an Illinois Licensed Structural Engineer, verifying that the equipment and procedure used will not overstress the new beams. To distribute load to multiple beams and protect the concrete, in all cases a double layer mat of heavy timbers shall be used at all times under crane tracks or wheels and any outriggers in the down position. If necessary, shims shall be used under the crane mat to ensure uniform contact with the underlying beams. Prior to placement of the timber mats the following shall be done: placement and tightening of transverse tie assemblies, grouting and curing the dowel rods 24 hours minimum and grouting and curing the shear keys. A temporary means of lateral restraint will be required for fascia beams at expansion ends of beams to prevent movement of the beams. The side retainers shown in the drawing no. 11 of 15 shall be installed once the beams are in their final locations. These side retainers may also be used to provide the temporary lateral restraint required during construction. This work shall be considered included in the cost of Precast Prestressed Concrete Deck Beams.
- 11. Existing reinforcement bars extending into the removal area shall be cleaned, straightened, and incorporated into the new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with an approved bar splicer or anchorage system. Cost included with Concrete Removal.
- 12. The cut strands at each beam end shall be given two coats of zinc dust spray or paint meeting the requirements of ASTM A780. The zinc dust spray or paint shall be applied before corrosion appears and allowed to dry according to the manufacturer's specifications prior to another coat of zinc. A concrete sealer meeting the requirements of section 587 of the Standard Specifications shall be applied to the exterior face and 9" in on the underside of the fascia beams. The sealer shall be applied after visible crack growth has subsided. This work shall be performed by the producer and included with the cost of the beam.
- 13. The minimum thickness of the concrete overlay shall be 5" and varies as required to adjust for the new profile grade and beam camber.
- 14. Repair of the substructure shall be completed prior to placement of the new deck heams

GENERAL DATA

IL 116 OVER KELLY CREEK

FAP ROUTE 681 - SECTION 116BR-1

FORD COUNTY

STATION 60+53.00

STRUCTURE NO. 027-0044

ESCA
CONSULTANTS, INC.
DESIGNED BY: ELH 6/05
DRAWN BY: CJG 6/05
CHECKED BY: ELH 10/05
APPROVED BY: RDP 10/05

Backfill with uncompacted Porous

Granular Embankment with a aradation of CA-5 or CA-7

by Bridge Contractor after

superstructure is in place. Limits shall be 12" from

the end of each wingwall.

Geotechnical fabric for french drains*

Standard 601101).

Excavation for placing Porous

Granular Embankment is paid

A 6" ϕ perforated drain pipe shall be situated at the bottom of an approx.

fabric for french drains. Extend pipe parallel with the cap until intersecting with the sideslope.* Pipes shall drain

onto concrete headwalls (Article 601.05 of the Std. Specification and Highway

2' x 2' area of Porous Granular Embankment. The 2' x 2' area shall be wrapped completely in geotechnical

for as Structure Excavation.

30'-0" Appr. Pavement Standard 420401

Bk. of Exist. Abut.

-Strip Seal Expansion Joint See Dwg, 7 of 15

Concrete Wearing Surface,

-17" PPC Deck Beams

-Existing Concrete Slope Wall to Remain

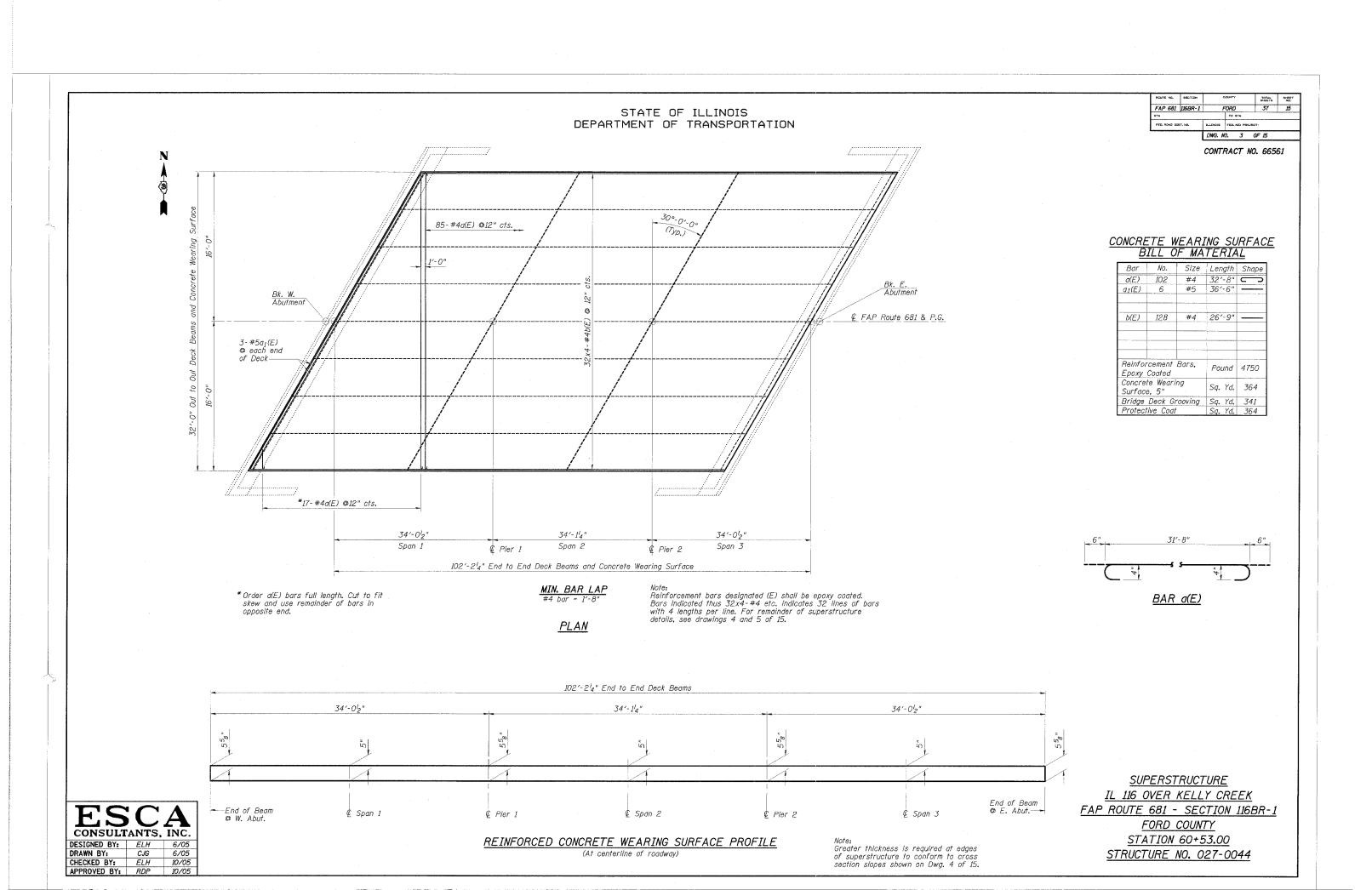
-Existing Concrete Piles

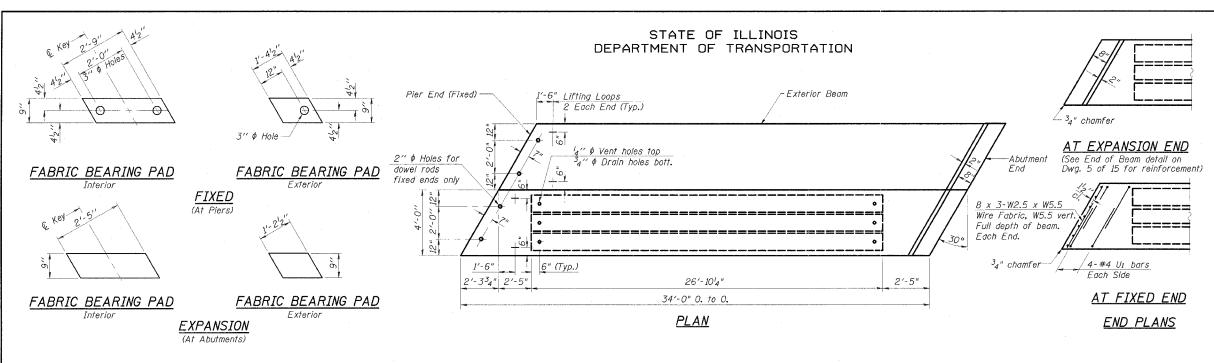
SECTION THRU ABUTMENTS

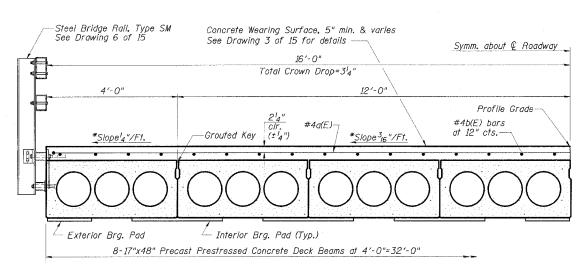
* Cost of drain pipe and fabric included in the cost of Porous Granular Embankment, Concrete

Headwalls paid for separately, see Roadway Plans.

(@ Rt. Angles)

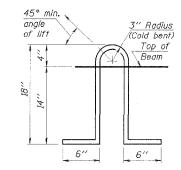




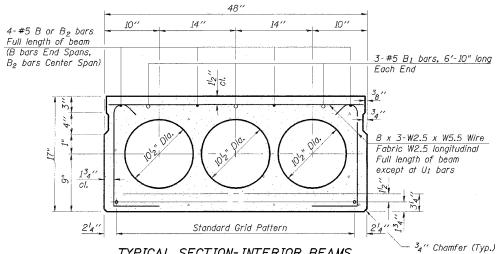


*Cross slopes shown are applicable to Concrete Wearing Surface.

HALF CROSS SECTION



LIFTING LOOP DETAIL



TYPICAL SECTION-INTERIOR BEAMS

 $11^{-1}2$ " ϕ Strands, Each Strand Stressed to 30,900 Lbs. 9-Strands 1^{3}_{4} " up, 2-Strands 3^{l}_{4} " up

Notes:

- 1. Place strands symmetrically about © of beam.
- 2. See Dwg. 5 of 15 for fascia beam details.

Prestressing steel shall be uncoated high strength, low relaxation 7-wire strand, Grade 270. The nominal diameter shall be $\frac{1}{2}$ and the nominal cross-sectional area shall be 0.153 sq. in. Lifting loops shall be $2 - \frac{1}{2}$ $\frac{1}{2}$ 6-270 ksi strands, as shown.

Non prestressing steel shall conform to AASHTO M-31 or M322 Grade 60.

The bearing seat surfaces shall be adjusted by shimming to assure firm and even bearing. Two 'g'' fabric adjusting shims of the dimensions of the Exterior Bearing Pad shall be provided for each

Keyway surfaces shall be cleaned to remove form oil or other bond breaking material prior to shipment of the beams. Cleaning shall be done by sandblasting the keyway areas between top of the beam and the bottom edge of the key.

Corrosion Inhibitor, as covered in the Special Provisions, shall be used in the concrete for precast prestressed concrete deck beams.

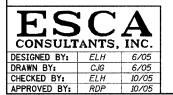
Required Release Strength, f'ci, shall be 4,000 p.s.i.

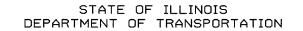
See Drawing 2 of 15 for location of rail anchors and additional notes. Bridge rail inserts shall be cast in precast beams, and the cost shall be included with Precast Prestressed Concrete Deck Beams.

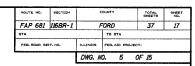
SUPERSTRUCTURE DETAILS IL 116 OVER KELLY CREEK FAP ROUTE 681 - SECTION 116BR-1 FORD COUNTY STATION 60+53.00 STRUCTURE NO. 027-0044

37 16

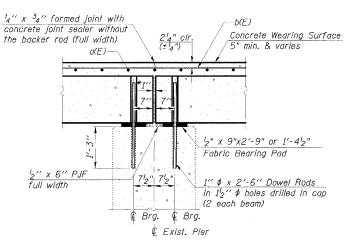
DWG. NO. 4 OF 15 CONTRACT NO. 66561







CONTRACT NO. 66561



SECTION THRU PIER

(Horizontal dimensions are at right angles to beam ends)

*1" Jt. shall be filled with non-shrink grout. 1" dimension may vary to accommodate tolerance in beam lengths.

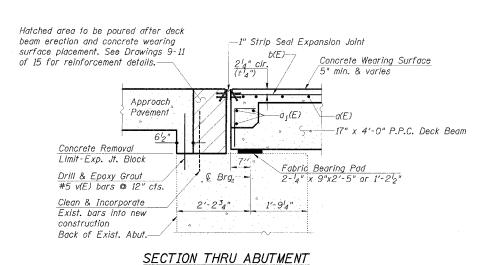
DESIGNED BY: ELH

DRAWN BY:

 DRAWN BY:
 CJG
 6/05

 CHECKED BY:
 ELH
 10/05

 APPROVED BY:
 RDP
 10/05



(Dimensions at right angles)

1'-3"

BAR D(E)

8x3-W2.5 x W5.5 $Wire Fabric, <math>1^{l}_{2}$ " clr. $(W5.5 \ vertical)$ −2 Pair-#4 U₂ bars END OF BEAM (EXPANSION END) (Dimensions at right angles)

2'-1"

BAR X

4-#5 E bars @ 12" cts.

2" cir.

Space to miss

4-#4 X bars @ 12" cts.

Flared Coil Loops

-3-#5 a₁(E) bars placed in field Full width of deck

-2 Pair-#4 U₁ bars

After beams have been erected, holes shall be drilled into substructure and dowels rods placed. Dowel holes shall be filled with non-shrink grout to top of beam and allowed to cure min. 24 hrs. prior to grouting the shear keys.

 $\mathbb{Q}^{-3}4$ " \times 6" Flared coil loops & 34 " ϕ \times 6" coil

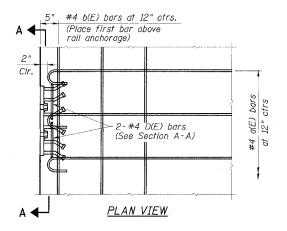
rod © 6" cts. (Cost included with Precast

Prestressed Concrete

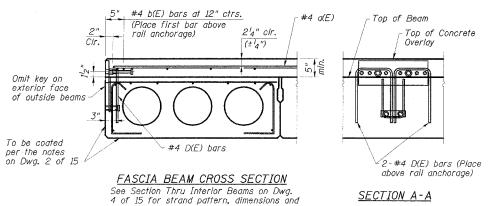
Deck Beams)

Concrete wearing surface to be poured after grouting the shear keys.

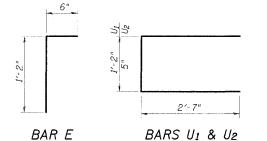
Dowel rods drilled in cap are included in the cost of Precast Prestressed Concrete Deck Beams (17" depth).



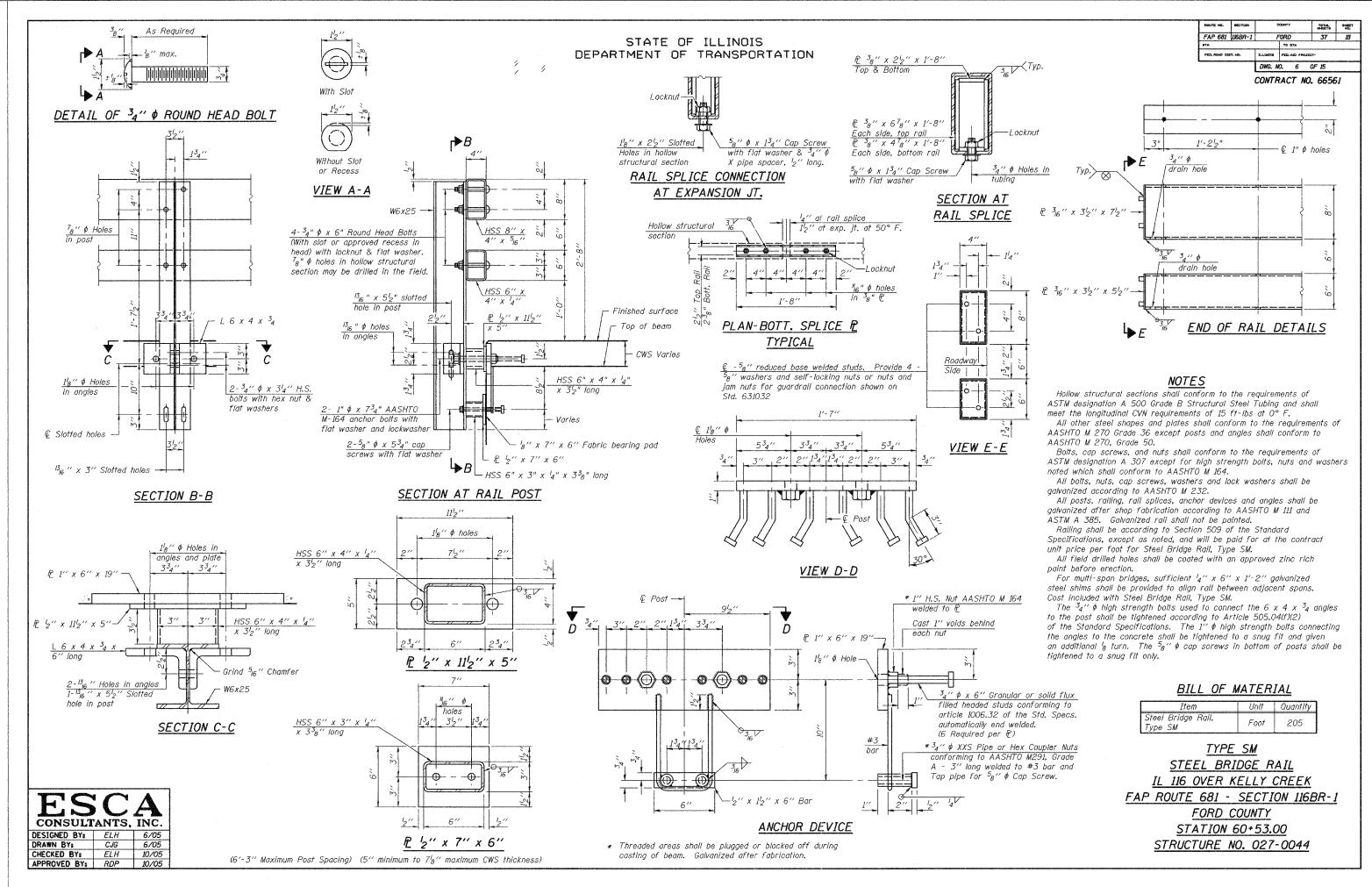
The rail anchorage shall be cast with the beam and the wearing surface shall be cast in the field. Formwork necessary for the wearing surface may be secured utilizing the bottom rail anchorage inserts and/or additional inserts cast into beam. Drilling into the beam will not be permitted.

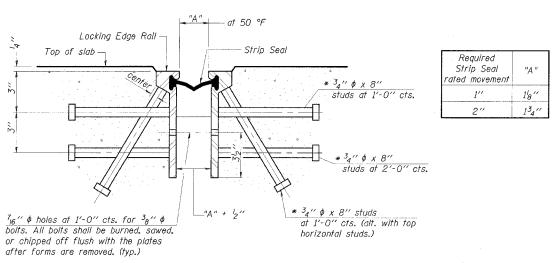


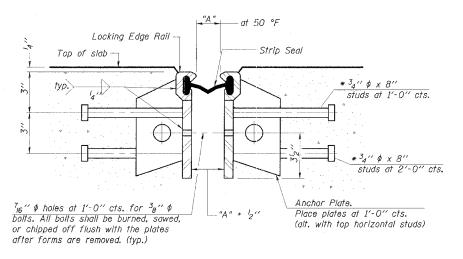
CONCRETE OVERLAY MODIFICATIONS FOR RAIL ANCHORAGE



SUPERSTRUCTURE DETAILS IL 116 OVER KELLY CREEK FAP ROUTE 681 - SECTION 116BR-1 FORD COUNTY STATION 60+53.00 STRUCTURE NO. 027-0044







GENERAL NOTES

The strip seal shall be made continuous and shall have a minimum thickness of ${}^{l}_{4}$ ". The configuration of the strip seal shall match the configuration of the Locking Edge Rails

Edge Rails.

The height and thickness of the Locking Edge Rails shown are minimum dimensions. The actual configuration of the Locking Edge Rails and matching strip seal may vary from manufacturer to manufacturer. Flanged edge rails will not be allowed.

Locking Edge Rails may be spliced at slope discontinuities and stage construction joints.

The manufacturer's recommended installation methods shall be followed.

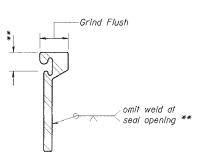
SECTION THRU ROLLED RAIL EXP. JOINT

(178 Studs Required at Each Joint)

* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.

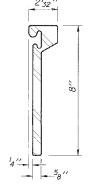
SECTION THRU WELDED RAIL AT EACH EXPANSION JOINT

(108 Studs Required at Each Joint) (70 Anchor Plates Required at Each Joint)

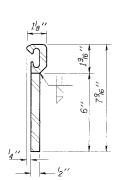


LOCKING EDGE RAIL SPLICE

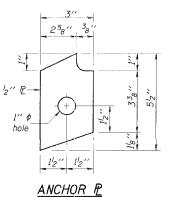
The inside of the locking edge rail groove shall be free of weld residue.



ROLLED (EXTRUDED) RAIL



WELDED RAIL



(for welded rail)

LOCKING EDGE RAILS

STRIP SEAL EXPANSION JOINT

IL 116 OVER KELLY CREEK

FAP ROUTE 681 - SECTION 116BR-1

FORD COUNTY

STATION 60+53.00

STRUCTURE NO. 027-0044

ESCA

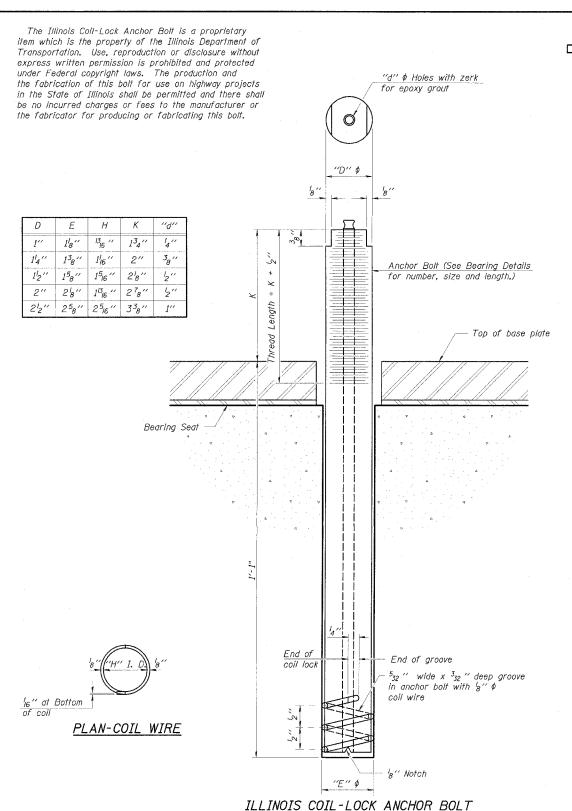
CONSULTANTS, INC.

DESIGNED BY: ELH 6/05

DRAWN BY: CJG 6/05

CHECKED BY: ELH 9/05

APPROVED BY: RDP 9/05



DWG. NO. 8 OF 15 CONTRACT NO. 66561

MATERIALS FOR ILLINOIS COIL LOCK ANCHOR BOLT

The anchor bolt shall be fabricated from cold drawn or hot finished seamless carbon steel mechanical tubing conforming to ASTM A 549, Grade 1026, CW and supplied with hexagonal nuts and cut washers.

The coil wire shall be made of any suitable soft steel wire. The finished anchor bolt shall be cleaned of rust and other foreign materials and wrapped or packaged to prevent contamination until they are installed.

The epoxy grout shall be a two-component, epoxy resin bonding system conforming to ASTM C 881, Type I, Grade 1 and of a Class suitable for the temperature at installation.

INSTALLATION PROCEDURE for the ILLINOIS COIL-LOCK ANCHOR BOLT

- 1. With the coil wire in place, the bolt shall be inserted into the hole and turned clockwise to a snug fit in the hole. Nut and washer shall be placed on the bolt. The nut shall be tensioned until the steel base plates are held securely to the concrete
- 2. Epoxy grout shall be pumped through the zerk fitting with a pressure gun. Pumping shall continue until the epoxy overflows the hole around the bolt shank. After pumping is discontinued, excess epoxy shall be immediately wiped off.

ALTERNATE ANCHOR BOLTS

The Contractor may use, at his option, the capsule or the adhesive cartridge type anchor rods that have been previously tested and given a prior approval by the Department. The Contractor shall install these anchor rods in pre-drilled holes according to the manufacturer's recommendations and procedures.

The capsule or the adhesive cartridge type anchor rods shall be a two part system composed of:

- 1. A threaded rod stud with nut and washer of the type specified.
- 2. A sealed glass capsule or a sealed glass adhesive cartridge containing premeasured amounts of the adhesive chemical

Туре
4-307 (Side Retainers)

ASTM F 1554 Grade 105, ASTM A 449 and AASHTO N 314 Grade 105 anchor bolts may be substituted for the anchor bolts shown above.

GENERAL NOTES

Holes in the masonry for anchor bolts shall be drilled through the base plates to the diameter and depth shown or according to the manufacturer's recommendation after beams or girders have been erected and adjusted.

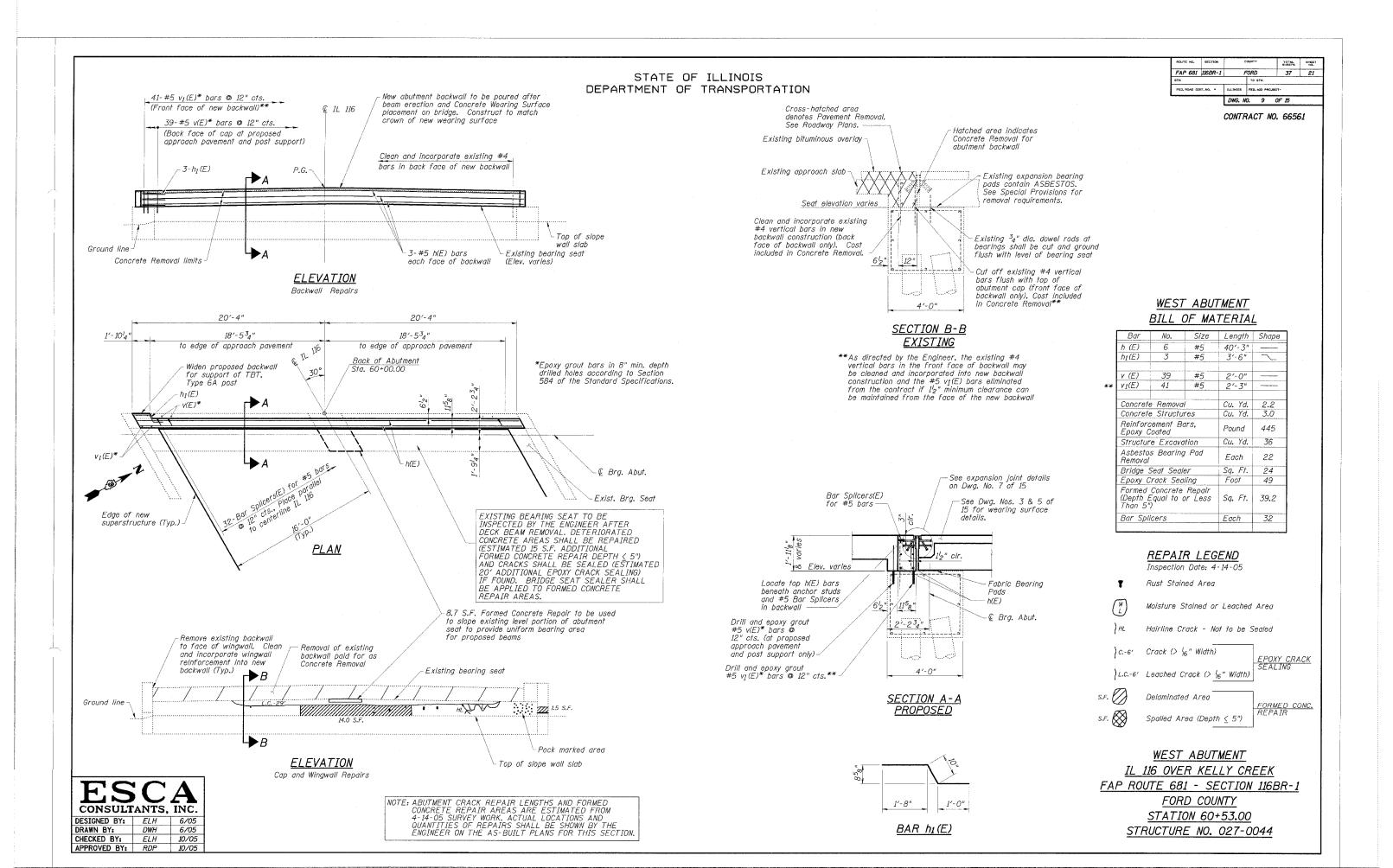
Prior to setting the bolts, the holes shall be dry and all dust and loose particles shall be removed by the use of compressed air or vacuuming.

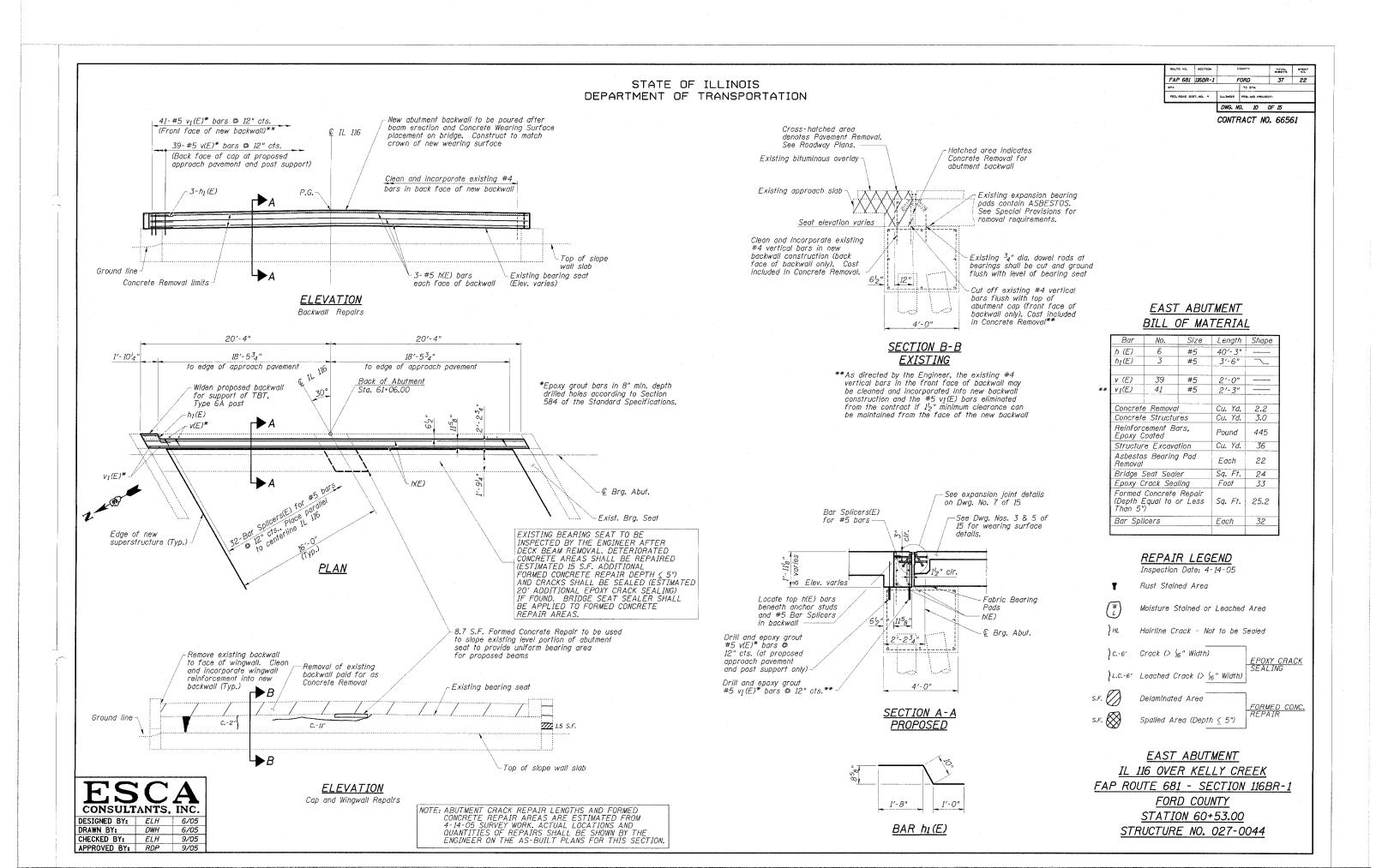
The anchor bolts, furnished and installed and including the epoxy grout or capsules shall not be paid for separately but shall be included in the unit bid price for Precast Prestressed Concrete Deck Beams.

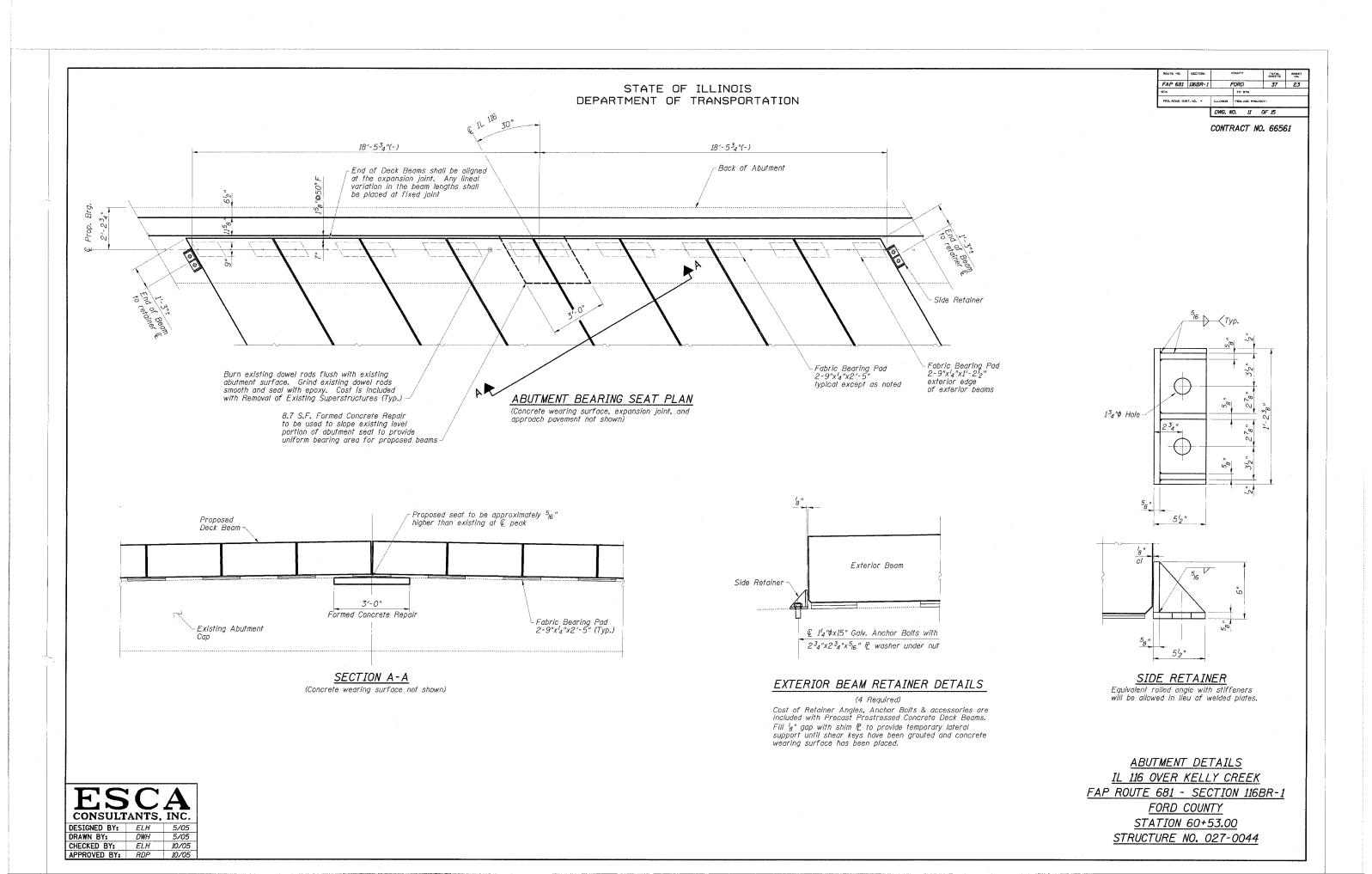
> ANCHOR BOLT DETAILS IL 116 OVER KELLY CREEK FAP ROUTE 681 - SECTION 116BR-1 FORD COUNTY STATION 60+53.00 STRUCTURE NO. 027-0044

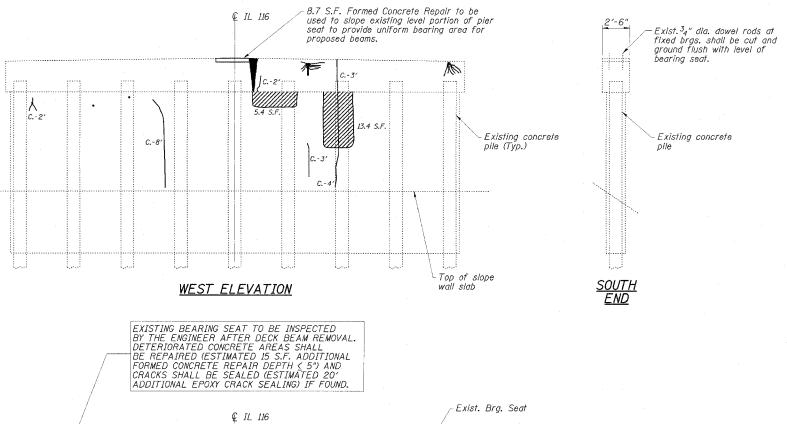
DESIGNED BY: ELH 6/05 DRAWN BY: CJG 6/05 CHECKED BY: ELH 9/05 APPROVED BY: RDP 9/05

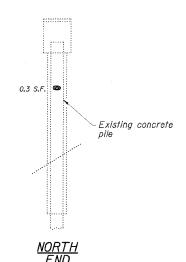
ABB-1 10-22-04











ROUTE NO.	SECTION	۱ '	OUNTY	SHEETS	SHEE NO.
FAP 681	116BR-1 FORD 37	FORD		37	24
STA			TO STA		
FED. ROAD DS	ST. NO. *	ILLENOIS	FED. ALD PROJ	ECT-	

CONTRACT NO. 66561

<u>PIER 1</u> BILL OF MATERIAL

ITEM	UNIT	TOTAL
Epoxy Crack Sealing	Foot	88
Formed Concrete Repair (Depth Equal to or Less Than 5")	Sq. Ft.	135.8
WVVL.N.L.		

<u>REPAIR LEGEND</u>

Inspection Date: 4-14-05

T Rust Stained Area

(W) Moisture Stained or Leached Area

HL Hairline Crack - Not to be Sealed

c.-6' Crack (> 1/6" Width)

L.C.-6' Leached Crack (> 1/6" Width)

EPOXY CRACK
SEALING

.F. Delaminated Area

FORMED CONC.
REPAIR

Spalled Area (Dep†h ≤ 5")

PIER 1
IL 116 OVER KELLY CREEK

FAP ROUTE 681 - SECTION 116BR-1

FORD COUNTY

STATION 60+53.00

STRUCTURE NO. 027-0044

ESCA
CONSULTANTS, INC.
DESIGNED BY: ELH 5/05
DRAWN BY: DWH 5/05
CHECKED BY: ELH 9/05
APPROVED BY: RDP 9/05

6.0 S.F

NOTE: PIER CRACK REPAIR LENGTHS AND FORMED CONCRETE REPAIR AREAS ARE ESTIMATED FROM 4-14-05 SURVEY WORK. ACTUAL LOCATIONS AND QUANTITIES OF REPAIRS SHALL BE SHOWN BY THE ENGINEER ON THE AS-BUILT PLANS FOR THIS SECTION.

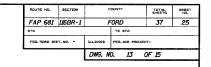
Top of slope

wall slab

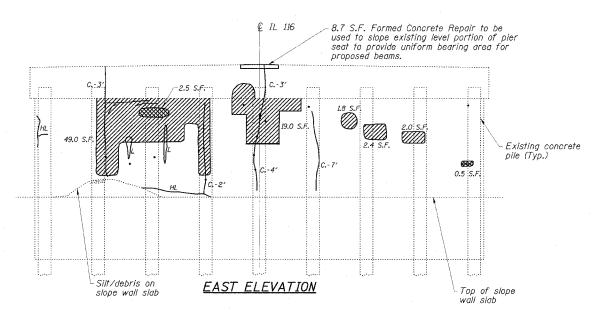
C.-7

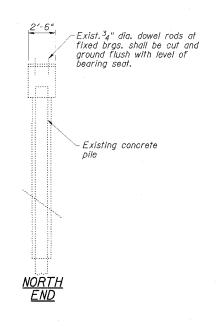
87.0 S.F.

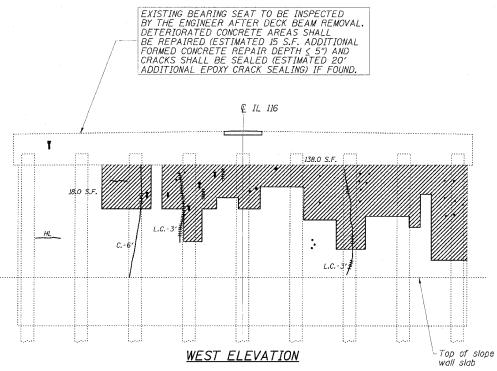
EAST ELEVATION

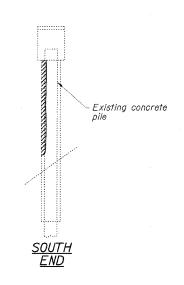


CONTRACT NO. 66561









PIER 2 BILL OF MATERIAL

ITEM	UNIT	TOTAL	
Epoxy Crack Sealing	Foot	51	
Formed Concrete Repair (Depth Equal to or Less Than 5")	Sq. Ft.	256.9	

REPAIR LEGEND

Inspection Date: 4-14-05

Rust Stained Area

Moisture Stained or Leached Area

Hairline Crack - Not to be Sealed

Crack (> 1/6" Width)

EPOXY CRACK SEALING \L.C.-6' Leached Crack (> 16" Width)

Delaminated Area

Spalled Area (Depth ≤ 5")

FORMED CONC. REPAIR

ESCA DESIGNED BY: ELH 5/05
 DRAWN BY:
 DWH
 5/05

 CHECKED BY:
 ELH
 9/05

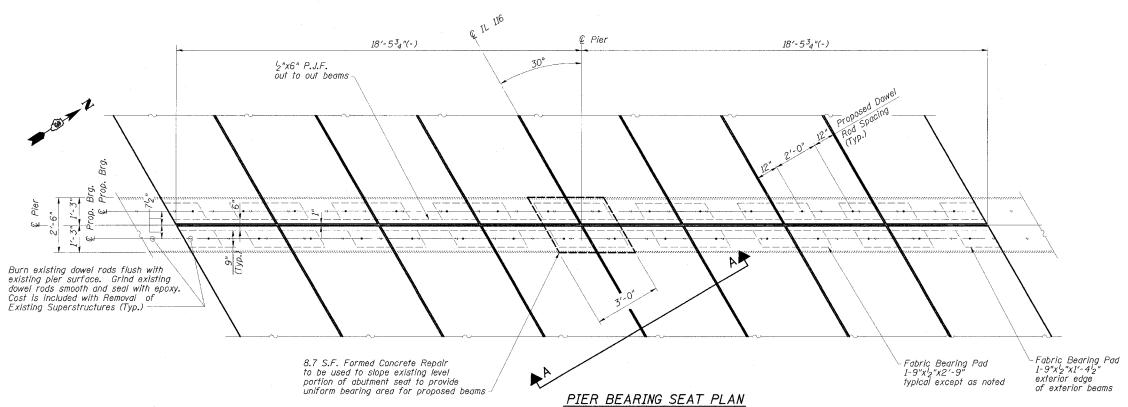
 APPROVED BY:
 RDP
 9/05

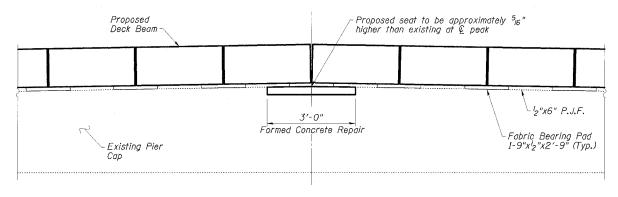
NOTE: PIER CRACK REPAIR LENGTHS AND FORMED CONCRETE REPAIR AREAS ARE ESTIMATED FROM 4-14-05 SURVEY WORK. ACTUAL LOCATIONS AND QUANTITIES OF REPAIRS SHALL BE SHOWN BY THE ENGINEER ON THE AS-BUILT PLANS FOR THIS SECTION.

PIER 2 IL 116 OVER KELLY CREEK FAP ROUTE 681 - SECTION 116BR-1 FORD COUNTY STATION 60+53.00 STRUCTURE NO. 027-0044

(Concrete wearing surface not shown)







SECTION A-A (Concrete wearing surface and dowel rods not shown)

ESCA
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DESIGNED BY: ELH 5/05
DRAWN BY: DWH 5/05
CHECKED BY: ELH 10/05
APPROVED BY: RDP 10/05

PIER DETAILS

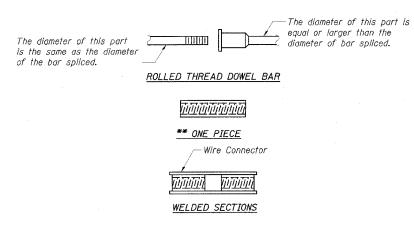
IL 116 OVER KELLY CREEK

FAP ROUTE 681 - SECTION 116BR-1

FORD COUNTY

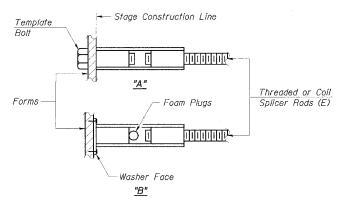
STATION 60+53.00

STRUCTURE NO. 027-0044



BAR SPLICER ASSEMBLY ALTERNATIVES

** Heavy Hex Nuts conforming to ASTM A 563, Grade C, D or DH may be used.



INSTALLATION AND SETTING METHODS

"A": Set bar splicer assembly by means of a template bolt.
"B": Set bar splicer assembly by nailing to wood forms or cementing to steel forms. (E): Indicates epoxy coating.

6'-0"

Threaded or Coil

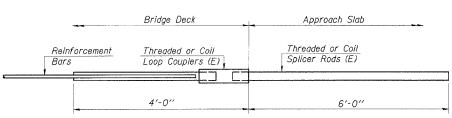
Splicer Rods (E)

Approach slab

Abutment

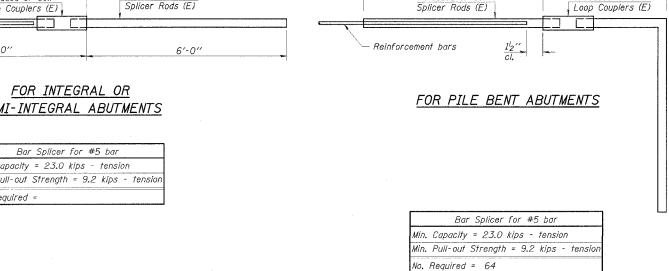
hatch block

Threaded or Coil



FOR INTEGRAL OR SEMI-INTEGRAL ABUTMENTS

Bar Splicer for #5 bar Min. Capacity = 23.0 kips - tension Min. Pull-out Strength = 9.2 kips - tensio Vo. Required =





Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars.

Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length. All reinforcement bars shall be lapped and tied to the splicer rods or dowel bars. Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars.

Other systems of similar design may be submitted to the Engineer for approval. Approval shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

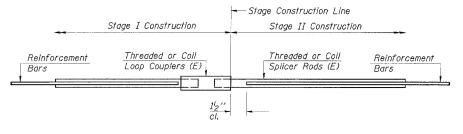
Minimum Capacity = 1.25 x fy x A_t
(Tension in kips) = 1.25 x fy x A_t
Minimum *Pull-out Strength
(Tension in kips) = 1.25 x fs_{allow} x A_t

Where fy = Yield strength of lapped reinforcement bars in ksi. fs_{allow}^{2} Allowable tensile stress in lapped reinforcement bars in ksi (Service Load)

A_t = Tensile stress area of lapped reinforcement bars.
* = 28 day concrete

	BAR SPLIC	ER ASSEMBLI	ES		
Bar Size to be Spliced		Strength Requirements			
	Splicer Rod or Dowel Bar Length		Min. Pull-Out Strength kips - tension		
#4	1'-8''	14.7	5.9		
#5	2'-0''	23.0	9.2		
#6	2'-7"	33.1	13.3		
#7	3′-5″	45.1	18.0		
#8	4'-6"	58.9	23.6		
#9	5′-9′′	75.0	30.0		
#10	7′-3′′	95.0	38.0		
#11	9'-0''	117.4	46.8		

Bar splicer assemblies shall be according to Section 508 of the Standard Specifications, except as noted. The furnishing and installation of bar splicer assemblies will be measured and paid for at the contract unit price each for "BAR SPLICERS."



STANDARD

Bar Size	No. Assemblies Required	Location

BAR SPLICER ASSEMBLY DETAILS IL 116 OVER KELLY CREEK FAP ROUTE 681 - SECTION 116BR-1 FORD COUNTY STATION 60+53.00 STRUCTURE NO. 027-0044

ESCA CONSULTANTS, INC.
 DESIGNED BY:
 ELH
 6/05

 DRAWN BY:
 CJG
 6/05

 CHECKED BY:
 ELH
 9/05
 APPROVED BY: RDP 9/05

BSD-1 10-22-04

CONTRACT NO. 66561 TOTAL SHEET SHEETS NO. RTE SECTION COUNTY 116BR-1 FORD STA. TO STA. FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT - Series bedff - TO Front face of Abutment STATE OF ILLINOIS EN: "6A P. P. Wingwolf of St. Corner of Bridge 115" RT. Sto 60"82" CT. 100.00

Ensting Structure Steel Truss spon @ 70" with reinf. core absed duts.

built as SOI PL. 119", Sec. 116 & (Substructure) Sec. 116 & (Superstructure) in 1959 @ Sto 60" 53 to be removed by Bridge Contractor before 104 44 DEPARTMENT OF PUBLIC WORKS & BUILDINGS SOERILS 10:08R143 DIVISION OF HIGHWAYS * (1582, 11032, 11032(1,1,1,4) 117 88 11 117 W. RS. 1 new construction starts. Welded Wire Fabric

G'16' mesh

Wt.56'/100 sq. ft Note Channel Adjustment to proposed Cross Section under the Dridge by Bridge Contractor All reinforcement bors shall be lapped 24 diameters unless otherwise fast (Nest Dikes North (South of Proposed Bridge to be extended to road shoulder elevation (100 00 at abutments so that trighwater will remain continuously closed between The Basic Lead Silico Chromate paint system shall be used for shop and tied pointing of structural steel.

Expansion guards which are not cost in the precast unit shall be fobricated and erected in occordance with Article 503.08 (c) of the Standard Specifications and are included in quantity of structural steel. SEC · B·B - End of Approach End of Approven Metal Guardrail & Post Meed and length determine by district Slope wall shall be reinforced with welded wire fabric 6"x 6" mash, weighing '58# per (10) sq.ft.
Layout of slope wells may be varied in the field to suit ground conditions as directed by the Engineer. Elev 3413 - 12 __Elev 94.79 Class A (B case to for structures include excovation for slope wall. LOW CONCRETE CHEN. 95.34 . HIM. Elev 96.27 Concrete Piles (Typ)
To be driven to miss the existing abutment feeting on Timber Piles An alternate strond pottern using Extra High Strength Prestressing strand 990m 100erle The contractor shall drive two test piles in the perminnent locations, one at E. Abutment; one at pier L as directed by the Engineer before ordering the remainder of piles. Concrete stepenall (Typ) - Classification Line TATION GO:GS BUILT 197 ET STATE OF ILLIMIS SBI RT.116 SEC:46R 16:6 Prt 15 Libraried area Elcv. 8320 removed by Contractor Bridge chonnel bottom Elev 80.80 SZE VATION WATERWAY INFORMATION LOADING HSEO NAME PLATE Drainage Area Character Level Rolling, Cultivated 300 300 21131 Required Opening (50yr flood) 690 Sq.FI. 630 59 Ft. Present Opening Proposed Opening Ordinary Water Elev. Low Water Elev. 3000 cfs Crepled Head . C7 Boring *2 TOTAL BILL OF MATERIAL Unit Super Sub. Cu. Yds Channel Excavation Cu Yds Class A Escavation for Struct Class B Excavation for Struct. Cu. Yds. Furn. & Erecting RPC Deck Bms. (177) Sq. Ft. Elev 100.25 Cu. Yds. Class & Concrete Lbs. Reinforcement Bars Lbs. 1,680 Lin.Ft 204 Structural Steel | Bailing Type N | Eles | Fles (Concrete) SIN 6013595 Coal Tar Interiayer Pr at 5q. Yds | 388 1 / Name Plates Each 600 600 Slope Wall (6") Removal of Existing Structures
Freshmed Jt. Scaler Lin. Ft Bit. Conc. Surf. Course Class I Tons 42.8 12.0 ICE: O' EX. E BK. Abu DESIGN STRESSES PRESTR. PRECAST UNITS SBI RT 116 OVER KELLY CREEK SBI RT 116 SEC. 116 BR FORD COUNTY FIELD UNITS 111.116 DESIGNED - 1 ... 1 . . . 7 - 3 f'c = 5,000 psi Tel 14000 psi 13 1440,000 psi (Strands 7616) 1si = 173,600 psi (Strands 7616) CHECKED GBMille R=1,000 psi 500 Jo- 75 psi Footing 15-20,000 psi Ren.f STATION 60+55 Allow 85 #1 9. Fl. for liture W.S. LOCATION SKETCH Loading H580-44

CONTRACT NO. 66561 COUNTY TOTAL SHEET NO. SECTION FORD 37 29 STA. TO STA. FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT # Little Love to total Section 1071 Love to tota SHEET NO : 3½ 3½ 156 Holes 3½ STATE OF ILLINOIS DEPARTMENT OF PUBLIC WORKS & BUILDINGS 117 W/ 25-1 _35x35x5 Plate Washers GORequired 104 4 #5 B bars full 2.858; bors, .2 x length of beam Each End length of beam FABRIC BEARING PAD U, BAR 30. 2/47 - 3"/8" Wire Mesh Full length of beam -Earl'ex- Elengtons 0 rmed Jt. Sealer Top of Cl. I Surface] (Thread each end 4")
GORequired Nut for 1" Rod E Drain Top of P.P.C 1:8% 1:8% 45pa 868 54° 54° 24 Deck Broms TYPICAL TRANSVERSE TIE ASSEMBLY GRAPHITED ASBESTOS = 102" Laded 18 12 - 14 Long J. J. P. 35 6 %" BEARING PAD TYPICAL SECTION 14 - 75 Strands Each Strand Stressed to 18,900 lbs. 7-Strands 15 up 7-Strands 35 up 451 At CIC alterate W6 x25 16'-6" + Over run T. Iv I Drop 30 PREE JT. SEALER Syrr m. Rdwy. - 5"1 x 5" CR 1000 Class I & Initial Deck Cost in Conc Anchor Device for Rail Post STL granular or solia Waterproofing (2.85upports) Slope 3/ poitt flux filed rended than autored welded. (50 stude @ 12 allets) Sec Shock 2 DETAIL A Class I /Surfacing (See Sheet S) See Defail 2" Drain Hole Il Precost Prestressed Concrete Beams at 3'-0" each = 33'-0" o to a. HALF CROSS SECTION 4½ 1184 4½ 2154 C-9"x b", 2". 1"
Graphiled Asbes los
Bearing Pads. _3" Typ. £ 600. poured after beam: 1.84 1.6" are in place 34:0"0100 - 248x 11 2" Dowels (2 Ea Bm.) SEC. THRU ABUTS. BILL OF MATERIAL (All Dimensions @ Rt Ls) PLAN No. Size Length Shape Sound Jt bond with Elastic St Filler. Symm about E Pier Class I

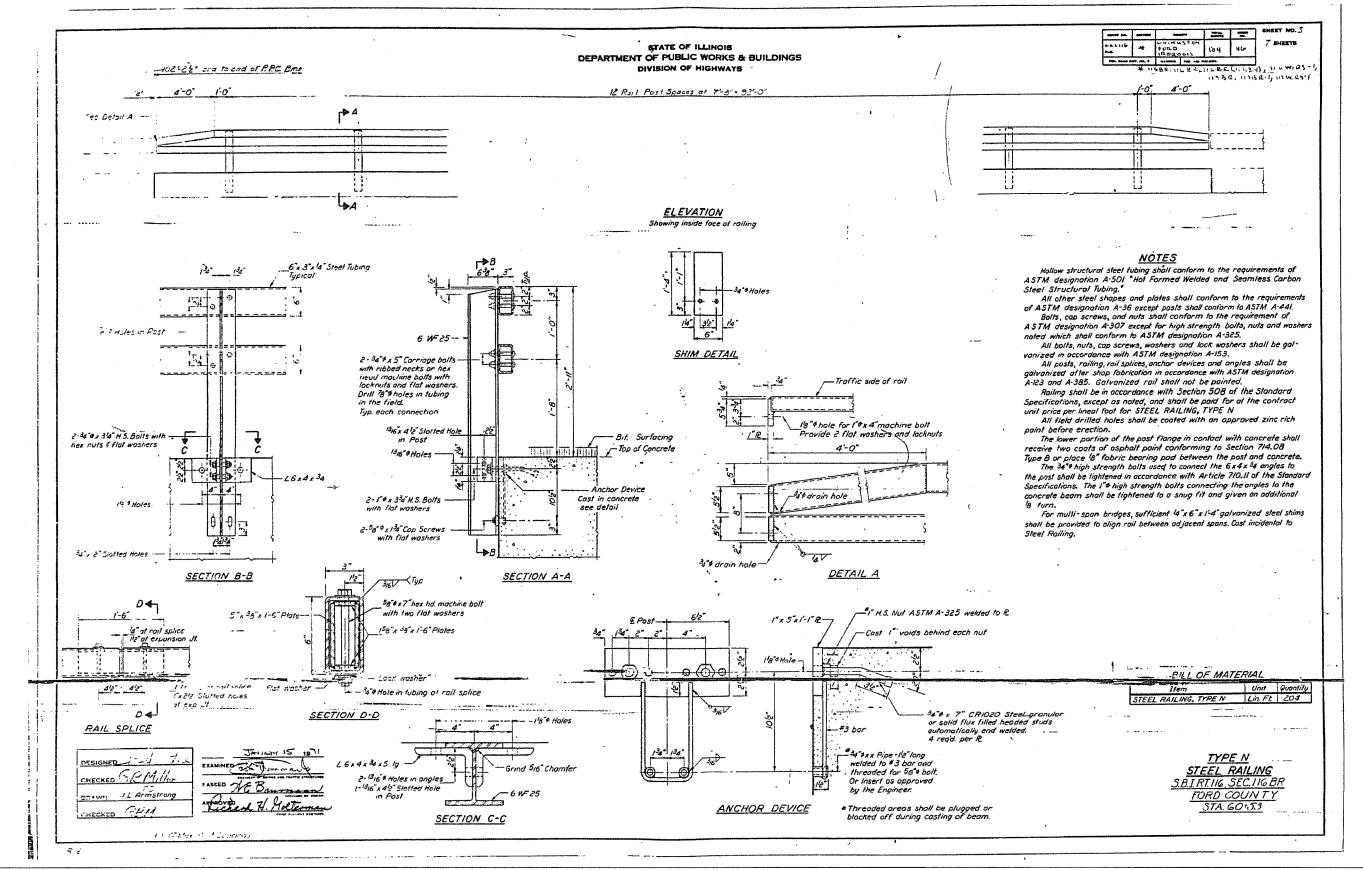
See Special Prov

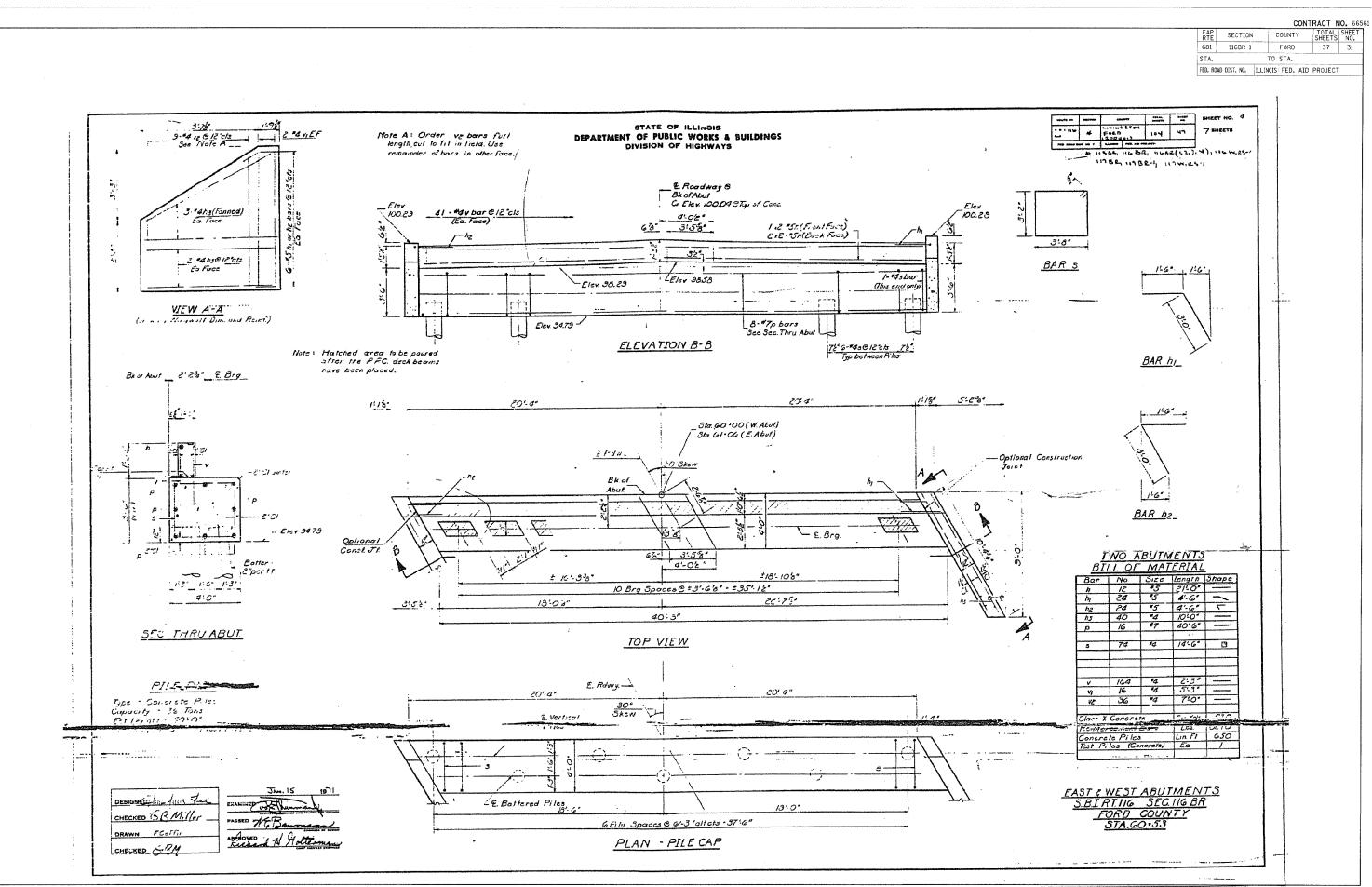
Surfacing Prestressing stee GENERAL NOTES 3-4 U Each Side Precast Prestressed Concrete Deck Beams Sq. Ft. 3,366 END PLAN shai! Le ASTM A. JOG, Grade 70.80. After fatheration the transverse lie assembles (tie rods, nuts, washers and sleeves) shall be hat-dipped, galvanized in occardance with A.S.T.M. Designation: A153.

Cost of reinforcement and occessories cast into the beam, of bearing pods, of armor angles, and of grouting longitudinal shear keys is included in unit price bid for "Precast Prestressed Concrete Deck Geoms." DECK BEAM DETAILS B.I. RT. 116 SEC 116 BR SEC. THRU PIERS DRAWN-FORD COUNTY STA GO . S.3 (All Dimensions @RIZS.) LIFTING LOOP DETAIL CHECKED GEM PD-1-L 11-19-65 Per 5-20-68

CONTRACT NO. 66561

FED. RO	AD DIST. NO.	ILLINOIS	FED.	AID	PROJECT	
STA.		T0	STA.			
681	116BR-1		FORD		37	30
FAP RTE	SECTION	C	OUNT	Y	TOTAL SHEETS	SHEET NO.





CONTRACT NO. 66561 FAP RTE SECTION 681 116BR-1 COUNTY TOTAL SHEET NO. FORD 37 32 STA. TO STA. FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT A LONDER 107 18 STATE OF ILLINOIS
DEPARTMENT OF PUBLIC WORKS & BUILDINGS Coal for Emulsion Sturry with O.3 Gal / Sq. Hz incl. (Cure 24 Hz.) DIVISION OF HIGHWAYS + 11502, 1160R, 1168 (1,7,1,4), 116 WIRS-1 Fiberglass Fabric 1.65 Oz./Sq. Yd. 11762, 11762-1, 117W, 25-1 Coal Tar Emulsion with 0.08 to 0.10 Gal / Sq. Yd. Note: Alledges shall have standard 4" chamfers except as noted 30'5kew riberglass Fabric L65 Oz./Sq. Yd. E FOWY Coal For Emulsion with 0.08 to 0.10 Goi/Sq. td. Rad -- E.Brg Pad _Coal for Emulsion with 0.08 to _0.10 GoL/Sq.Yd. (Cure 4 Hr.) Penetrating Primer 0.01
Gol. / Sq Yd. (Cure 24 Mr.) 1\85 PILE DATA Tupe · Concrete Capacity · 35 Tons Est. Length · 40'0" - · No Req'd · 17"1 Test Pile ut Pier 1 108 215% -Clean Concrete Surface 10 Brg. Spaces C ± 3'6'8" TOP PLAN DECK SURFACING 21'-0" 2.61 Elex 9858- ... - Eler 95.30 -3-16 pi Ea Face Symmi about E Pier <u>TWO PIERS</u> BILL OF MATERIAL 5% 5% 4) · 5 bars v.@ 124cts BAR SI B -Elev 3080 Corcrete Piles ELEVATION 102 102 Reinforcement Dimensions 119" ENL VIEW BAR 11/1/2 & Pile Spaces C4-11". 33:4" *30°* E. Rowx NE"H - E. Pier S.B.I. RT. IIG SEC. IIG BR FORD COUNTY Lichent H. Holterne 19:8" 19'8" 5TA. 60+53 CHECKED CHM SEC. B-B

