

	0.4 Sp. 1 0.6 Sp. 3	Piers	0.5 Sp. 2
I_s	(in ⁴) 50326	81932	50326
I_c (n)	(in ⁴) 100935		100935
I_c (3n)	(in ⁴) 76837		76837
S_s	(in ³) 1448	2292	1448
S_c (n)	(in ³) 1807		1807
S_c (3n)	(in ³) 1677		1677
\bar{D}	(k/ft.) 1.100	1.520	1.100
$M\bar{D}$	(k) 852	2483	829
$s\bar{D}$	(k/ft.) 0.420		0.420
$M_s\bar{D}$	(k) 363		410
$M\bar{L}$	(k) 1007	990	1036
M (Imp)	(k) 215	199	195
$5_s[M\bar{L} + M(\text{Imp})]$	(k) 2037	1982	2052
M_a	(k) 4228	5805	4278
M_u	(k) 4659		4662
$f_s\bar{D}$ non-comp (k.s.i.)	7.1	13.0	6.9
$f_s\bar{D}$ (comp) (k.s.i.)	2.6		3.0
$f_s^{5_3}(\bar{L} + \text{Imp})$ (k.s.i.)	13.6	10.4	13.7
f_s (Overload) (k.s.i.)	23.3	23.4	23.6
f_s (Total) (k.s.i.)		30.4	
VR	(k) 61.2		53.1

	Pier 3	Pier 4 or 5	Abut.
$R\bar{D}$	(k) 63.0	212.5	61.1
$R\bar{L}$	(k) 45.3	83.3	45.3
Imp.	(k) 9.6	11.1	9.6
R (Total)	(k) 117.9	306.9	116.0

I_s and S_s are the moment of inertia and section modulus of the steel section used in computing f_s (Total & Overload).
 I_c and S_c are the moment of inertia and section modulus of the composite section used in computing stresses due to Live Load.
 I_c and S_c are the moment of inertia and section modulus of the composite section used in computing stresses due to superimposed dead loads. (see AASHTO 10.38)
 VR is the maximum Live Load + Impact shear range in span.
 M_a (Applied Moment) = $1.3[M\bar{D} + M_s\bar{D} + 5_3(M\bar{L} + M(\text{Imp}))]$.
 The Plastic Moment capacity (M_u) is computed according to AASHTO 10.48.1 and 10.50.1.1.
 f_s (Overload) is the sum of the stresses due to $M\bar{D} + M_s\bar{D} + 5_3(M\bar{L} + M(\text{Imp}))$.
 f_s (Total) (Non-compact section) is the sum of the stresses due to $1.3[M\bar{D} + M_s\bar{D} + 5_3(M\bar{L} + M(\text{Imp}))]$.
 $R\bar{D}$ at Pier 3 includes Finger Joint weight.

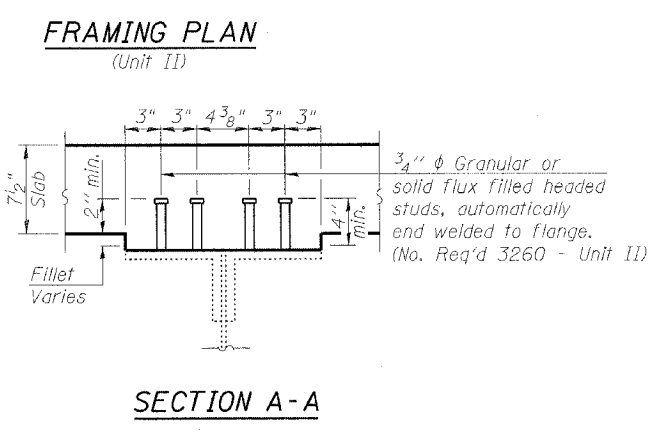
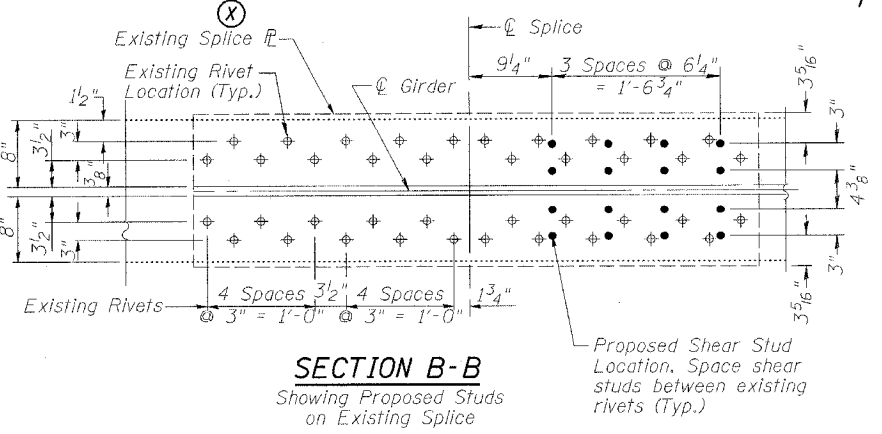
See Sheet 14 of 25 for Section C-C and Structural Steel Repair (3 Loc. Unit II)

See Sheet 15 of 25 for Pier Diaphragm Details

Existing Cross Frames at Pier 3 to be removed, stored, and re-erected upon completion of Structural Repairs. Cost included with Furnishing and Erecting Structural Steel.

See Sheet 16 of 25 for Abutment Diaphragm Details

Remove Existing WF diaphragms at Pier 3 and E. Abutment



NOTES:
 Two hardened washers shall be required over all oversize holes at diaphragms.
 The cost of removing the existing diaphragms and Finger Joint to be included in the cost of Structural Steel Removal.
 The cost of the replacement diaphragms, hardware and painting to be included in the cost of Furnishing and Erecting Structural Steel. Cost of Field Drilling Holes in Beams included in the cost of Furnishing and Erecting Structural Steel.
 Existing dimensions to be field verified prior to ordering of material.
 See Sheet 14 of 25 for Section C-C.
 See Sheet 12 of 25 for Bill of Material

DIAPHRAGM REPLACEMENT:
 1 Bottom L3 1/2"x3 1/2"x5/16"
 2 Bottom L 3 1/2"x3 1/2"x5/16" and bottom 5/16" Gusset Plates.
 (See Sheet 16 of 25 for Diaphragm Details)

ILLINOIS DEPARTMENT OF TRANSPORTATION
FRAMING PLAN UNIT II
 OLD U.S. ROUTE 36 OVER
 SANGAMON RIVER
 F.A.U. ROUTE 7978
 SECTION BR-1
 SANGAMON COUNTY
 STA. 70+00.00
 STRUCTURE NUMBER 084-0052
 DRAWN BY: NJV
 CHECKED BY: PBB
 DATE: JAN. 2005