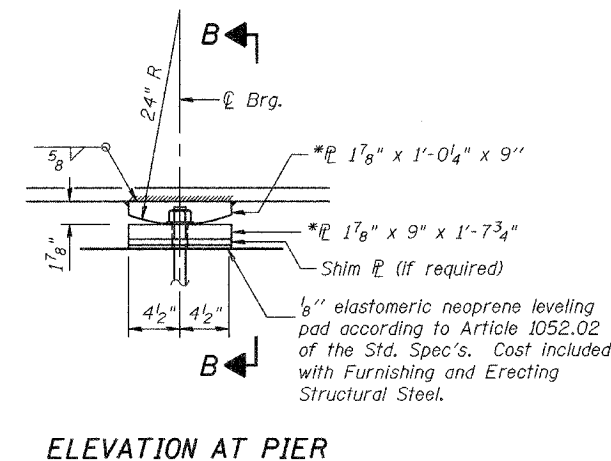
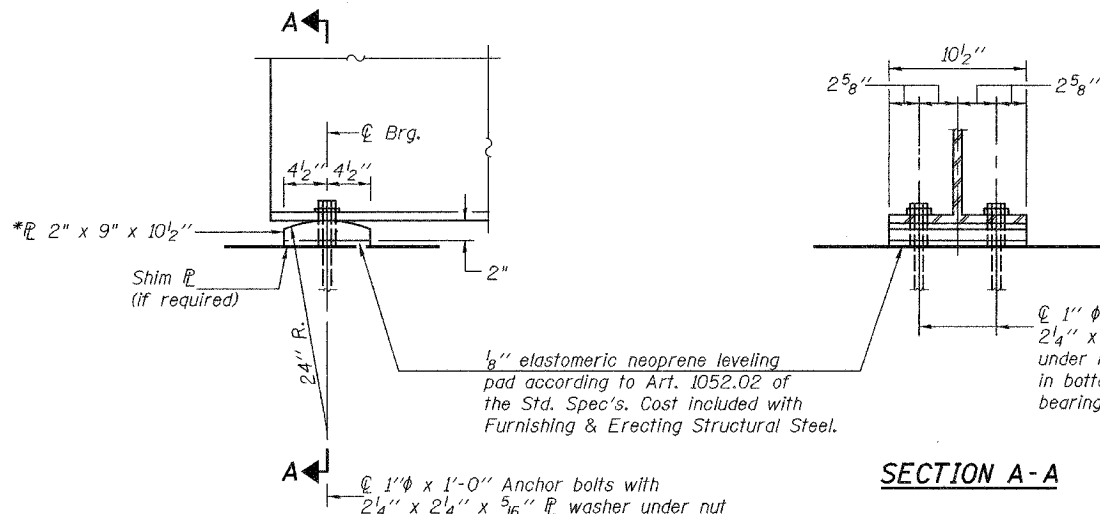


STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
FAP 314	108BR-1	MADISON	123	41
FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT		

SHEET NO. 13  
29 SHEETS

Contract #76454



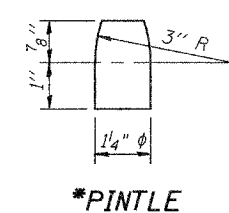
ELEVATION AT ABUTMENTS

ABUTMENT BEARING  
(12 Required)

ELEVATION AT PIER

FIXED BEARING  
(24 Required)

SECTION B-B



	0.4 Sp. 1 & 0.6 Sp. 5	Pier 1 & Pier 4	0.5 Sp. 2 & Sp. 4	Pier 2 & Pier 3	0.5 Sp. 3
$I_s$	6680	6680	6680	6680	6680
$I_c$	17051	17051	17051	17051	17051
$I_{c(3n)}$	12386	12386	12386	12386	12386
$S_s$	436	436	436	436	436
$S_{c(n)}$	628	628	628	628	628
$S_{c(3n)}$	565	565	565	565	565
DC1	0.800	0.800	0.800	0.800	0.800
M DC1	128	323	200	374	174
DC2	0.150	0.150	0.150	0.150	0.150
M DC2	29	48	50	57	45
DW	0.329	0.329	0.329	0.329	0.329
M DW	64	104	110	126	100
M $\pm$ Imp	556	417	712	503	715
M $\alpha$ (Strength I)	1265	1350	1724	1608	1675
$\phi$ M $\alpha$	3177	3177	3177	3177	3177
$f_s$ DC1	3.5	8.9	5.5	10.3	4.8
$f_s$ DC2	0.6	1.3	1.1	1.6	1.0
$f_s$ DW	1.4	2.9	2.3	3.5	2.1
$f_s$ L3 ( $\pm$ +I)	13.8	14.9	17.7	18.0	17.8
$f_s$ (Service II)	19.3	28.0	26.6	33.4	25.7
$f_s$ (Total)(Strength I)		37.2		44.4	
$\phi$ F $\alpha$		50.0		50.0	
V $\alpha$	22.3	26.5	26.5	26.3	26.3

$I_s$  and  $S_s$  are the moment of inertia and section modulus of the steel section used in computing  $f_s$  due to non-composite loads.

$I_c$  and  $S_c$  are the moment of inertia and section modulus of the composite section used in computing  $f_s$  due to short-term composite loads.

$I_{c(3n)}$  and  $S_{c(3n)}$  are the moment of inertia and section modulus of the composite section used in computing  $f_s$  due to long-term composite loads.

DC1 is the dead load acting on the non-composite section.

DC2 is the dead load acting on the long-term composite section.

DW is the dead load acting on the long-term composite section due to wearing surface.

M $\alpha$  (Strength I) = 1.25(MDC1+DC2)+1.5M(DW)+1.75M(L+Imp).

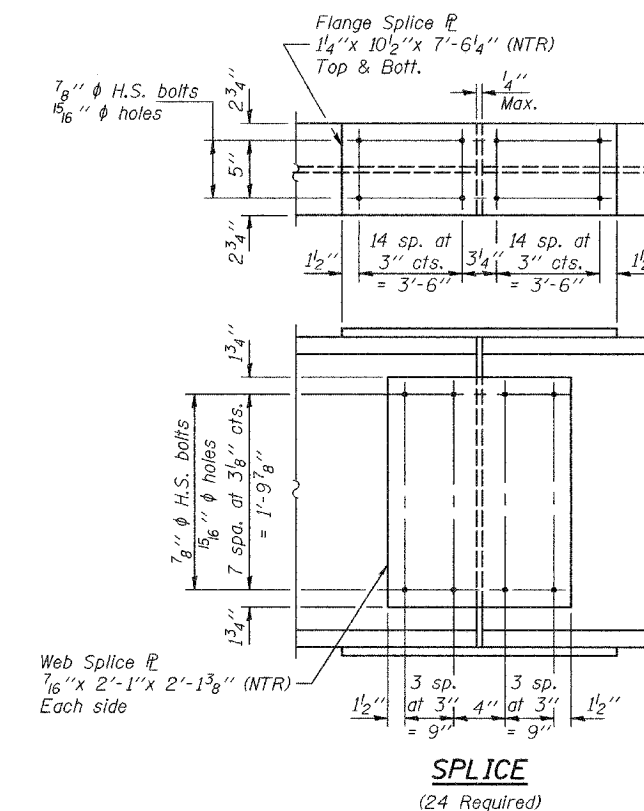
$\phi$  M $\alpha$  is the full plastic moment capacity computed in accordance with Appendix D6.1 and 6.10.7.1.2.

$f_s$  (Service II) is the sum of the stresses due to DC1+DC2+DW+1.3(L+Imp).

$f_s$  (Total) (Strength I) (Non-compact section) is the sum of the stresses due to 1.25(DC1+DC2)+1.5DW+1.75(L+Imp).

V $\alpha$  is the maximum shear range in the span (0.75 L+Imp).

$\phi$  F $\alpha$  is the allowable flexural resistance stress of the compression flange computed in accordance with Art. 6.5.4.2 and 6.10.8.2.



Notes: Two hardened washers shall be required over all  $\frac{1}{16}$ "  $\phi$  holes for diaphragms.

NTR denotes members to which Notch Toughness Requirements are applicable.

Anchor bolts at all bearings may be built into the masonry.

See sheet 14 of 29 for anchor bolt installation.

	N. Abut.	Pier 1 & Pier 4	Pier 2 & Pier 3	S. Abut.
R DC1	14.5	55.8	59.9	14.5
R DC2+DW	9.5	32.6	35.8	9.5
R $\pm$	51.9	82.3	86.5	51.9
Imp.	13.5	16.2	22.5	13.5
R (Total)	89.4	186.9	198.4	89.4

\*\*TOP OF BEAM ELEVATIONS

Location	$\phi$ Brg. N. Abut.	$\phi$ Brg. Pier 1	$\phi$ Splice 1	$\phi$ Brg. Pier 2	$\phi$ Splice 2	$\phi$ Splice 3	$\phi$ Brg. Pier 3	$\phi$ Splice 4	$\phi$ Brg. Pier 4	$\phi$ Brg. S. Abut.
Beam 1	507.583	507.722	507.751	507.886	507.909	507.915	507.894	507.772	507.745	507.617
Beam 2	507.709	507.849	507.878	508.013	508.036	508.041	508.020	507.899	507.872	507.744
Beam 3	507.812	507.952	507.981	508.116	508.139	508.144	508.123	508.002	507.975	507.846
Beam 4	507.812	507.952	507.981	508.116	508.139	508.144	508.123	508.002	507.975	507.846
Beam 5	507.709	507.849	507.878	508.013	508.036	508.041	508.020	507.899	507.872	507.744
Beam 6	507.583	507.722	507.751	507.886	507.909	507.915	507.894	507.772	507.745	507.617

\*\*For fabrication use only.

DESIGNED Curt M. Evoy  
CHECKED Tom L. Kurtenbach  
DRAWN h.t. duong  
CHECKED CME/TLK

November 17, 2005  
EXAMINED Thomas J. Donagallo  
PASSED Ralph E. Anderson

STRUCTURAL STEEL DETAILS  
F.A.P. RTE. 314 - SEC. 108BR-1  
MADISON COUNTY  
STATION 263+51  
STRUCTURE NO. 060-0334