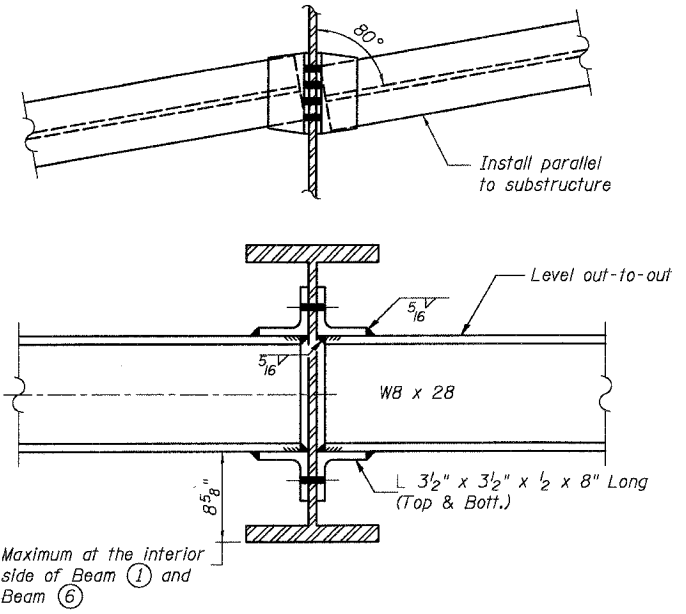


**FRAMING PLAN**  
Not to Scale



**INTERIOR DIAPHRAGM D1**  
Not to Scale

**MOMENT AND REACTION TABLES**

INTERIOR GIRDER MOMENT TABLE		0.5 Span
$I_s$	(in <sup>4</sup> )	3630
$I_c$ (n)	(in <sup>4</sup> )	10,635.1
$I_c$ (3n)	(in <sup>4</sup> )	7,732.86
$S_s$	(in <sup>3</sup> )	329
$S_c$ (n)	(in <sup>3</sup> )	478.38
$S_c$ (3n)	(in <sup>3</sup> )	423.98
DL	(k/ft.)	0.87
$M_{DL}$	(k)	258.3
$f_s$ DL(non-comp)	(k.s.i.)	9.42
$S_{DL}$	(k/ft.)	0.50
$M_{SDL}$	(k)	147.4
$f_s$ SD(comp)	(k.s.i.)	4.17
$M_{LL}$	(k)	416.1
$M_I$ (Impact)	(k)	119.8
$f_s$ [M <sub>LL</sub> + M <sub>Impact</sub> ]	(k.s.i.)	13.44
$f_s$ (Total)	(k.s.i.)	27.04
VR	(k)	59.19

**TOP OF BEAM ELEV. TABLES**

GIRDER #	N. ABUT.	S. ABUT.
1	688.59	688.60
2	688.74	688.75
3	688.85	688.86
4	688.85	688.86
5	688.74	688.75
6	688.59	688.60

For Fabrication use only

INTERIOR GIRDER REACTION TABLE		Abut.
$R_{DL}$	(k)	33.3
$R_{LL}$	(k)	46.0
Imp.	(k)	13.3
$R$ (Total)	(k)	92.6

**NOTES:**

1. Work this sheet with sheet S-07

DESIGNED	MGH
CHECKED	RGD
DRAWN	WJH
CHECKED	NRF

$I_s$  and  $S_s$  are the moment of inertia and section modulus of the steel section used in computing  $f_s$  (Total).  
 $I_c(n)$  and  $S_c(n)$  are the moment of inertia and section modulus of the composite section used in computing stresses due to Live Load.  
 $I_c(3n)$  and  $S_c(3n)$  are the moment of inertia and section modulus of the composite section used in computing stresses due to superimposed dead loads. (see AASHTO 10.38)  
 VR is the maximum Live Load + Impact shear range in span.  
 Live Load and Impact designed for 120,000 lb. permit loading (one lane) as well as HS20-44.

SMITH ENGINEERING CONSULTANTS, INC. CIVIL STRUCTURAL ENGINEERS AND SURVEYORS www.smitheng.com 2601 West Lake Street, Suite 200, Chicago, IL 60659	
REVISIONS	
NAME	DATE

ILLINOIS DEPARTMENT OF TRANSPORTATION

Steel Framing Plan  
Harlem Avenue/Drecksler Road  
Over Black Walnut Creek  
Will County  
Section 01-00139-02-BR  
SN. 099-3091

DATE 5-26-2005