



Illinois Department of Transportation

2300 South Dirksen Parkway / Springfield, Illinois / 62764

January 11, 2005

SUBJECT: FAI Route 90/94
Project ACIM-094-3(367)057
Section 2021-922PT.1-AC
Cook County
Item No. 101, January 21, 2005 Letting
Addendum A

NOTICE TO PROSPECTIVE BIDDERS:

Attached is an addendum to the plans or proposal. This addendum involves revised and/or added material.

1. Revised page v of the Table of Contents.
2. Revised pages 215 – 217 of the Special Provisions.
3. Added pages 259 - 266 to the Special Provisions.
4. Revised pages 1, 2 and 11 of the Schedule of Prices.
5. Revised sheets 7, 8, 12, 15, 228 – 239, 241 – 247, 256, 257, 261, 264, 267, 268, 271, 277, 278, 279, 280, 283, 286, 289, 292 – 295, 305, 315, 316, 324, 326, 327 and 352 of the Plans.

Prime contractors must utilize the enclosed material when preparing their bid and must include any Schedule of Prices changes in their bidding proposal.

Bidders using computer-generated bids are cautioned to reflect any and all Schedule of Prices changes, if involved, into their computer programs.

Very truly yours,

Michael L. Hine
Engineer of Design
and Environment

A handwritten signature in cursive script, reading "Ted B. Walschleger" followed by "P.E." in a smaller font.

By: Ted B. Walschleger, P. E.
Engineer of Project Management

cc: Diane O'Keefe; N. R. Stoner; Roger Driskell; R. E. Anderson; Jim White;
Design & Environment File

TBW:TK;jc

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Delete the fourth paragraph of Article 702.05(a) of the Standard Specifications.

Revise the sixth paragraph of Article 702.05(a) of the Standard Specifications to read:

“When the work operations exceed four days, all signs shall be post mounted unless the signs are located on the pavement or define a moving or intermittent operation. When approved by the Engineer, a temporary sign stand may be used to support a sign at 1.2 m (5 ft) minimum where posts are impractical. Longitudinal dimensions shown on the plans for the placement of signs may be increased up to 30 m (100 ft) to avoid obstacles, hazards or to improve sight distance, when approved by the Engineer. “ROAD CONSTRUCTION AHEAD” signs will also be required on side roads located within the limits of the mainline “ROAD CONSTRUCTION AHEAD” signs.”

Delete all references to “Type 1A barricades” and “wing barricades” throughout Section 702 of the Standard Specifications.

SUBCONTRACTOR MOBILIZATION PAYMENTS (BDE)

Effective: January 2, 2005

To account for the preparatory work and operations necessary for the movement of subcontractor personnel, equipment, supplies, and incidentals to the project site and for all other work or operations that must be performed or costs incurred when beginning work approved for subcontracting in accordance with Article 108.01 of the Standard Specifications, the Contractor shall make a mobilization payment to each subcontractor.

This mobilization payment shall be made at least 14 days prior to the subcontractor starting work. The amount paid shall be equal to 3 percent of the amount of the subcontract reported on form BC 260A submitted for the approval of the subcontractor’s work.

This provision shall be incorporated directly or by reference into each subcontract approved by the Department.

TEMPORARY SHEET PILING INSTALLATION

Description. This work shall consist of installing Temporary Sheet Piling, in accordance with the applicable requirements of the Special Provisions and Section 512 of the Standard Specifications except as amended herein.

Construction.

Impact hammers of any kind will not be allowed to install the sheet piling. All sheet piling shall be installed with a vibratory hammer or any other method such as “press-in method”. Integral augering or jetting may be required to facilitate driving of the sheet piles.

The cost to comply with this requirement will not be paid for separately, but shall be considered as included in the contract unit bid prices for TEMPORARY SHEET PILING.

Revised 01-11-2005

FAI ROUTE 90/94 (DAN RYAN EXPRESSWAY)
SECTION: (2021-922 PT. 1-AC)
COOK COUNTY

Deleted 01-11-2005

FAI ROUTE 90/94 (DAN RYAN EXPRESSWAY)
SECTION: (2021-922 PT. 1-AC)
COOK COUNTY

Deleted 01-11-2005

MICROPILES

Effective: April 19, 1996

Revised: May 11, 2001

Description. This work shall consist of designing, furnishing and installing the proposed micropiles according to the contract plans, approved shop drawings, and the Special Provisions. The term micropiles is used generically in this specification to refer to any proprietary system able to satisfy this specification and the contract plans. Systems that may satisfy this specification are Pin-Piles, Mini-Piles, Root-Piles, Needle-Piles or other systems.

The Contractor shall be responsible for selecting the micropile type, installation method, bond lengths, grout pressures, etc., such that the micropiles will satisfy the compressive and tension design loads indicated on the contract plans. The Contractor shall demonstrate the micropile capacities by performing verification pile load test(s) and micropile proof test loadings that satisfy the acceptance criteria of this Special Provision.

Submittals. The Contractor selected to perform this work shall satisfy the qualification requirements and will be required to provide shop drawings for the proposed micropile installation.

- (a) Qualifications: The Contractor performing the work shall be experienced in the design, construction and testing of micropiles. The Contractor shall have successfully installed a total of at least 100 micropiles on no less than five (5) different projects completed within the last five (5) years of similar project conditions and capacities to those required on this project.

The Contractor shall assign a field supervisor with experience on at least three (3) projects of similar scope to this project, completed over the past five (5) years. The on-site foreman and drill rig operators must have completed three (3) projects within the last five (5) years involving micropiles of equal or greater capacity than required on this project. The Department may suspend the micropile work if the Contractor substitutes unqualified personnel and the Contractor shall be liable for additional costs resulting from the suspension.

The above experience qualifications list and personnel list shall be submitted for approval at the time of bidding. **The award of the contract will be contingent upon the Department's approval of these qualifications.**

- (b) Shop Drawings: At least five weeks before work is to begin, the Contractor shall submit to the Engineer for review and approval, complete shop drawings and design calculations describing the micropile system, or systems, intended for use. The micropiles and anchorage head assembly shall be designed and detailed to carry the tension and compression loadings indicated on the contract plans. The submittal shall be sealed by an Illinois Licensed Structural Engineer and include (as a minimum) the following:

- 1) Design Calculations:
 - a) A written summary report that describes the overall micropile design.
 - b) Applicable code requirements and design reference literature used.
 - c) Micropile design cross-section(s) geometry including casing, soil/rock strata, piezometric levels, and magnitude as well as direction of design loadings.

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- d) Design criteria including soil/rock shear strengths (friction angle and cohesion), unit weights, ground/grout bond values, micropile drilled hole diameter, and assumptions for each soil/rock strata.
 - e) Factors of safety used on the ground to grout bond.
 - f) Structural design calculations sizing the verification micropile section, verification and proof testing load frame, reaction piles and connections to both the reaction piles and micropiles. Geotechnical calculations shall also be submitted to indicate that a minimum factor of safety of 2.5 against failure exists for the reaction piling.
- 2) Shop Drawings including the following:
- a) Plan view of the project showing:
 - 1. All proposed micropiles with each labeled with a unique identification number.
 - 2. Locations of subsurface exploration borings plotted and labeled.
 - 3. Proposed overall sequence of construction.
 - 4. Locations of micropiles to be proof tested and station/offset of the micropile verification load test(s).
 - b) Elevation view of project showing:
 - 1. The location of the existing substructures and all soil boring data plotted with all major changes in soil type or stratification identified.
 - 2. The proposed micropile lengths plotted at each substructure as well as the bottom of casing, top of bonded length, total length and final tip elevations indicated.
 - 3. All general notes for constructing the micropiles.
 - c) Micropile typical section showing:
 - 1. The proposed typical micropile configuration(s) including steel casing, tension reinforcement sizes, and average grouted diameters (in both the cased and bonded lengths).
 - 2. Step by step installation procedure(s) including casing advancement, grouting elevations, re-grouting, etc.
 - 3. Tension reinforcement centralizers and spacer locations and details.
 - 4. Casing splice details.
 - d) Anchorage head assembly detail including tension reinforcement connection and required weld sizes.
 - e) Any revisions to details shown on the contract plans necessary to accommodate the micropile system intended for use.
 - f) Micropile verification and proof testing sheet showing:
 - 1. Load frame and anchor pile details for verification tests.
 - 2. Load frame and reaction pile connection for proof testing production piles.
 - 3. Additional tension reinforcement and grout strength required in the verification micropiles to permit testing piles to 2.5 times the design loadings.
 - 4. Jack, pressure gauge and load cell calibration curves.
 - g) The grout mix design and procedures for monitoring and recording the grout depth, volume and pressure during the grouting process.

Work shall not start on any micropile, nor shall materials be ordered, until the shop drawings have been approved in writing by the Engineer. Such approval shall not relieve the Contractor of any responsibility under this contract for the successful completion of the work.

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Materials. The materials used for the construction of the micropiles shall satisfy the following requirements:

- (a) Reinforcement Steel: Micropiles reinforcement shall consist of single or multiple elements of 1034 MPa (150 ksi) (f_u) high strength threadbars or deformed bars conforming to ASTM A722.
- (b) Steel Couplers: Prestressing steel couplers shall be capable of developing 95 percent of the minimum specified ultimate tensile strength of the tension reinforcement steel.
- (c) Grout: The grout shall consist of a neat cement or sand cement mixture of Type II, III or V portland cement conforming to Section 1020 of the Standard Specifications. Expansive admixtures may not be used except to seal the encapsulations and anchorage covers. Admixtures shall be to control bleed, improve flowability, reduce water content, and retard set may be used if approved by the Engineer. Accelerators and admixtures containing chlorides are not permitted.
- (d) Fine Aggregate: If sand-cement grout is used, sand shall conform to the requirements for fine aggregates according to Section 1003 of the Standard Specifications.
- (e) Spacers: Spacers for separation of elements of a multi-element tension reinforcement shall permit the free flow of grout. They shall be fabricated from plastic, steel or material which is not detrimental to the reinforcement. Wood shall not be used. Spacers shall be placed along the total length of the micropile so that the steel will bond to the grout. They shall be located at 3 m (10 ft) maximum centers with the upper one located a maximum of 1.5 m (5 ft) from the top of the micropile and the lower one located a maximum of 1.5 m (5 ft) from the bottom of the bonded length.
- (f) Centralizers: Centralizers shall be fabricated from plastic, steel or material which is not detrimental to the reinforcing steel. Wood shall not be used. Centralizers shall be able to maintain the reinforcement position and alignment so that a minimum of 38 mm (1.5 inches) of grout cover is obtained at all locations along micropile length. They shall be located at 1.5 m (5 ft) maximum centers with the lower one located one foot from the bottom of the bonded length.
- (g) Anchorage head assembly: The materials properties, dimensions, and design details for the micropile anchorage head assembly components proposed by the Contractor to transfer the tension and compression design loads from the micropile to the footing shall be submitted for approval as part of the shop drawings. Anchorage components may include bearing plates, shear studs, anchorage rebars, and other approved components.
- (h) Permanent steel casing: Permanent steel casing shown on the contract plans has been designed to withstand lateral and vertical forces imposed by the structure during a seismic event. Any changes to this casing shall be submitted to the Department for review and approval. Spiral weld pipe will not be permitted.

Construction Requirements. The soil conditions for this project are represented by the boring information shown on the plans. The Contractor, utilizing his/her expertise, shall be responsible for interpreting the data, as he/she feels necessary to be fully familiar with the existing conditions in order to design, install and successfully test the micropiles as specified. Variations

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in geologic deposits, overburden materials, ground water elevations, etc., may occur between borings and may not necessary be considered a change in site conditions as defined by Article 104.03 of the Standard Specifications.

The drilling method used may be rotary drilling, percussion drilling or an approved alternate. The method of installation used shall be that which prevents loss of ground around the drilled hole that may be detrimental to the structure. The drillhole must be open along its full length at the design minimum drillhole diameter prior to placing reinforcement and grout. Temporary casing or other approved method of micropile drillhole support will be required in caving or unstable ground to permit the micropile shaft to be formed to the minimum design drillhole diameter.

The Contractor shall notify the Engineer if an obstruction is encountered. An obstruction is defined as any object (such as but not limited to, boulders, logs, old foundations etc.) that cannot be drilled through using normal casing advancement techniques. Upon concurrence of the Engineer, the Contractor shall begin working to core, break up, push aside, or remove the obstruction unless relocating the micropile would be less expensive. Tools or equipment lost below grade by the Contractor shall not be defined as obstructions. If an existing pile is encountered, the borehole shall be backfilled with grout, and the micropile relocated to an alternate location as determined by the Engineer. Obstruction mitigation shall be paid for according to Article 109.04 of the Standard Specifications.

Casing may be any type of flush joint steel of the nominal diameter shown on the plans and appropriate lengths. The casing must be capable of advancing the hole through the soil strata as indicated in the boring data or any other data the Contractor may have obtained.

The reinforcement shall be placed prior to grouting and before the temporary casing is withdrawn. The reinforcement shall be inserted to the desired depth without undue stress or difficulty (not driven or forced). When the reinforcement cannot be completely inserted it shall be removed and the drill hole cleaned or re-drilled to permit insertion. The reinforcement shall be free of soil, grease, or oil that might reduce the grout to bar bond.

The micropiles shall be grouted the same day the load transfer bond length is drilled. Grout used for grouting shall be free of any lumps and undispersed cement. The grout volumes and pressures shall be measured and recorded during the placement operation. The pump shall be equipped with a grout pressure gauge at the pump and a second gauge placed at the point of injection at the top of the casing to monitor grout pressures. The gauges shall be capable of measuring pressures of at least 1.0 MPa (150 psi) or twice the actual grout pressures used, whichever is greater. The grout shall be kept in agitation prior to mixing and place within one hour of mixing. The grouting equipment shall be sized to enable each pile to be grouted in one continuous operation. The grout shall be injected from the lowest point of the drillhole (through grout tubes, casing, drill rods, etc.) and continued until uncontaminated grout flows from the top of the micropile. Temporary casing, if used, shall be extracted in stages ensuring that, after each length of casing is removed, the grout level is brought up to ground level before the next length is removed. The casing or tremie pipe must always extend below the level of the grout in the drillhole. Upon completion of grouting, the grout tube or access valve may remain in the drill hole and anchorage head assembly provided it is filled with grout. The grout take and pressure shall be controlled to prevent any heave of the ground surface or foundations.

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The Contractor shall monitor the existing foundation for movement. If movement is detected, the Contractor shall immediately stop production and notify the Engineer. Work shall not resume until the Contractor's recommendations to remedy the situation are approved by the Engineer.

The following construction tolerances shall apply to all production micropiles:

- (a) The center of the micropile casing shall be within 75 mm (3 inches) of the plan station and offset.
- (b) The out of vertical plumbness of the shaft shall not exceed 2.0%.
- (c) The top of the casing shall be no more than 25 mm (1 inch) above and no more than 50 mm (2 inches) below the plan elevation.

Micropile Load Test and Micropile Proof Load Test. The Contractor shall install and test non-production verification micropile(s) as well as proof load test selected production micropiles. The load testing shall be performed by incrementally loading the micropiles according to ASTM D 1143 for the compression loading and ASTM D 3689 for the tension loading except as modified herein. Testing shall not take place until the grout has acquired sufficient strength to preclude crushing during application of the test loadings.

The structural capacity of the micropiles to be tested will need to be increased such that the peak test tension and compression loadings applied to that micropile do not exceed 80% of its structural capacity, including steel yielding in tension, steel yielding or buckling in compression or grout crushing in compression. All costs associated with the design and materials required to satisfy this shall be included in the bid price for Micropile Load Test (for the verification tested micropiles) and included in the bid item Micropile Proof Load Test (for the proof tested micropiles).

The micropiles tested shall have "tell tale" rods installed in sleeves to the bottom of the casing and to the tip of the micropile allowing readings to be taken to give indications of how much load is resisted by the different segments of the micropile. Other instrumentation such as strain gauges may also be used as an alternative to the tell tales and shall be approved by the Engineer. The micropile movement shall be measured and recorded to the nearest 25 micron (.001 inch) with respect to an independent fixed reference point immediately prior to loading and for each increment of load.

The jack shall be positioned at the beginning of the test so that unloading and repositioning during the test will not be required. When both compression and tension loading is to be performed, it shall be performed on the same micropile and the compression loading shall be conducted first. Dial gauges capable of measuring displacements to 0.025 mm (.001 inch) shall be used to measure micropile movement of the jack from an independent reference point. If the test setup requires reaction against the ground or a single row of reaction piles, two gauges shall be used on either side of the variation micropile. The reaction frame and piles shall be adequately stiff to prevent excessive deformation, misalignment or racking under peak loading. No part of the existing structure shall be used to assist in the load frame and reaction pile design. The stressing equipment shall be placed over the micropile in such a manner that the jack, load cell, and load test reaction frame are axially aligned with the anchorage head assembly reinforcement. Gauges shall have adequate travel so the total micropile and tell tale movements can be measured without resetting the devices.

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Apply and measure the test loads with a hydraulic jack and pressure gauge. The pressure gauge shall be graduated in 500 kPa (72 psi) increments or less. The jack and pressure gauge shall have a pressure range not exceeding twice the anticipated maximum test pressure. Jack ram travel shall be sufficient to allow the test to be done without resetting the equipment. Monitor the creep test load hold during testing with both the pressure gauge and electronic load cell. Use the load cell to accurately maintain a constant load hold during the creep test load hold increment of the testing.

A graph shall be constructed showing a plot of anchorage head assembly movement and both tell tale deflections versus test loading (both tension and compression) for each load increment in the test schedule including the rebound measurements after unloading.

The acceptance criteria, demonstrating a successful test, are as follows:

- (a) The micropile must carry the design compression and tension loading (1.0 Design Load) with a deflection of the anchorage head assembly less than the maximum deflection value shown on the contract plans, as measured from its original unloaded position.
- (b) At the end of the 1.33 Design Load increment, the micropile shall have a creep rate not exceeding 1mm (0.04 inches)/log cycle of time (1 to 10 minutes) or 2mm (0.08 inches)/log cycle of time (6 to 60 minutes). The creep rate shall be linear or decreasing throughout the creep load hold period.
- (c) Failure does not occur at the maximum Design Load increment. Failure is defined as the load at which attempts to further increase the test load simply result in continued micropile movement.

Micropile Load Test. The Contractor shall perform non-production verification micropile load test(s) to verify the design and the construction methods proposed prior to installing production micropiles. The number and general location of the verification test(s) are indicated in the contract plans and shall be constructed and tested according to this specification and the approved shop drawings.

The verification test Design Load shall be taken as the maximum compression and maximum tension design loadings indicated at any substructure shown on the contract plans. The loadings shall be incrementally applied according to the following cyclic load schedule:

Verification Load Test Schedule

Increment	Loading Applied	Increment	Loading Applied
1	0.05 Design Load	13	0.05 Design Load
2	0.25 Design Load	14	0.25 Design Load
3	0.50 Design Load	15	0.50 Design Load
4	0.05 Design Load	16	0.75 Design Load
5	0.25 Design Load	17	1.00 Design Load
6	0.50 Design Load	18	1.33 Design Load
7	0.75 Design Load	19	1.75 Design Load
8	0.05 Design Load	20	2.00 Design Load
9	0.25 Design Load	21	2.25 Design Load
10	0.50 Design Load	22	2.50 Design Load
11	0.75 Design Load	23	0.05 Design Load
12	1.00 Design Load		

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The dial gauges shall be reset to zero after the initial 0.05 Design Load increment is applied. The load holding period shall start as soon as each load increment is fully applied and last for 1 minute for each increment with the exception of the 1.33 and 2.50 load increments which will be held for 60 minutes and 10 minutes respectively. The jack shall be re-pumped as necessary in order to maintain a constant load during this period. The micropile deflections shall be measured and recorded at the end of the load holding period. The 1.33 load hold increment shall be monitored for creep by recording the micropile movements at 1, 2, 3, 5, 6, 10, 20, 30, 50, and 60 minutes during the load hold.

In the event that a verification micropile fails the acceptance criteria, the Contractor shall re-evaluate his/her design and construction procedures, making the necessary changes to install an additional micropile and anchor pile(s) to be verification tested. The above process shall be repeated until a successful micropile passes the verification load test acceptance criteria. Payment for the successful verification load test shall include all work associated with the failed verification pile load test(s).

The Engineer will provide the Contractor with written conformation of the micropile design and construction within 10 working days of the completion of the verification load test(s). This written confirmation shall confirm the bond lengths and tip elevations shown on the Contractor's working drawing or reject the micropiles based on the verification test results.

Verification micropiles and reaction piles located in non-production locations shall be cut 600 mm (2 ft.) below finished grade after completion.

Micropile Proof Load Test: The Contractor shall install a set of production micropiles at a substructure unit designated to have a proof test loading, prior to the installation of the remaining production micropiles in that unit or other substructure units covered by that proof test. A set of production micropiles is defined as the number of micropiles required to proof test a production micropile and provide the proof test frame reaction capacity. The substructures that will have a production micropile proof tested and which adjacent substructures are covered by that proof test are indicated on the contract plans.

The proof test Design Load shall be taken as the maximum compression and maximum tension loadings indicated at any substructures covered by that proof test, shown on the contract plans. The loadings shall be incrementally applied according to the following cyclic load schedule shown below:

Proof Test Loading Schedule

Increment	Loading Applied	Increment	Loading Applied
1	0.05 Design Load	5	1.00 Design Load
2	0.25 Design Load	6	1.33 Design Load
3	0.50 Design Load	7	1.67 Design Load
4	0.75 Design Load	8	0.05 Design Load

The Dial gauges shall be reset to zero after the initial 0.05 Design Load increment is applied. The load holding period shall start as soon as each load increment is fully applied and last for 1 minute for each increment with the exception of the 1.33 load increment which shall have a 10 minute load hold. If the top of the micropile movement between the 1 minute and 10 minute time intervals exceeds 1 mm (0.04 inches), the 1.33 load hold shall be maintained for an

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additional 50 minutes. The jack shall be re-pumped as necessary in order to maintain a constant load during this period. The micropile deflections shall be measured and recorded at the end of the load holding period. The 1.33 load hold increment shall be monitored by recording the micropile movements at 1, 2, 3, 5, 6, and 10 minutes and if extended shall be recorded at the 20, 30, 50, and 60 minutes during the load hold.

In the event that a production micropile fails the proof test acceptance criteria, the Contractor shall re-evaluate his/her design and construction procedures, make the necessary changes and install an additional non-production micropile and additional anchor pile(s), outside the proposed footing and proof test the revised micropile. The above process shall be repeated until a successful micropile passes the acceptance criteria. The set of production micropiles installed as part of the failed proof test shall be cut flush with the bottom of the footing and supplemented by micropiles installed using improved design and installation methods adjacent to the failed micropiles. The failed load test(s), any supplemental or additional anchor piles, or micropiles cut flush with the bottom of the footing shall be included with the successful micro pile proof test loading.

Upon the completion of each successful micropile proof load test, the remaining production micropiles in that substructure unit and other substructure units covered by that proof load test can be installed using the same successful design and installation.

Basis of Payment. This work will be paid for at the contract unit price each for MICROPILES, and shall be compensation in full for designing, furnishing and installing the production micropiles incorporated in the final structure, according to the contract plans, approved shop drawings, and the Special Provisions. Verification pile load testing of non-production micropiles passing the verification acceptance criteria shall be paid for at the contract unit price each for MICROPILE LOAD TEST and shall be compensation in full for designing, furnishing and installing the verification micropiles, anchor piles, reaction frame, and applying the test loads. Micropile proof test loading of selected production micropiles shall be paid at the contract unit price each for MICROPILE PROOF LOAD TEST and shall be compensation in full for installing the anchor piles, reaction frame, and applying the test loads.

Obstruction mitigation shall be paid for according to Article 109.04 of the Standard Specifications.

Added 01-11-2005

ILLINOIS DEPARTMENT OF TRANSPORTATION
SCHEDULE OF PRICES

CONTRACT NUMBER - 62693

State Job # - C-91-072-04
PPS NBR - 1-74823-0550
County Name - COOK - -
Code - 31 - -
District - 1 - -
Section Number - 2021-922 PT.1-AC

Project Number
ACIM-0943/367/057

Route
FAI 90/94

Item Number	Pay Item Description	Unit of Measure	Quantity	x	Unit Price	=	Total Price
XX001854	STAB SUB-BASE 6	SQ YD	4,327.000				
XX002082	SAN SEW REMOV 24	FOOT	1,511.000				
XX003704	SS LINING 24 D	FOOT	403.000				
XX004056	MECH ST EARTH RET WL	SQ FT	3,980.000				
XX004201	PAVT REINFORCEMENT 14	SQ YD	2,858.000				
XX004684	REP&REPL DAMAGED COND	FOOT	50.000				
XX104800	COMB CC&G TBV.12	FOOT	6,319.000				
XX152400	SAND CUSHION 3	CU YD	10.000				
XX157200	CITY ELECT MAN ADJ	EACH	26.000				
X0320080	ROD & CL DUCT EX COND	FOOT	1,683.000				
X0320772	WATER MAIN REMOV 12	FOOT	183.000				
* X0321963	MICRO-PILES	EACH	917.000				
X0322124	STORM SEW WM REQ 8	FOOT	390.000				
X0322256	TEMP INFO SIGNING	SQ FT	294.000				
X0323314	MANHOLE TY A (CHGO)	EACH	7.000				
X0323315	MANHOLE TY B (CHGO)	EACH	4.000				
							* REVISED : JANUARY 12, 2005

ILLINOIS DEPARTMENT OF TRANSPORTATION
SCHEDULE OF PRICES
CONTRACT NUMBER - 62693

State Job # - C-91-072-04
 PPS NBR - 1-74823-0550
 County Name - COOK - -
 Code - 31 - -
 District - 1 - -
 Section Number - 2021-922 PT.1-AC

Project Number
ACIM-0943/367/057

Route
FAI 90/94

Item Number	Pay Item Description	Unit of Measure	Quantity	x	Unit Price	=	Total Price
X0323426	SED CONT DR ST INL CL	EACH	374.000				
* X0323432	MICROPILE LOAD TEST	EACH	4.000				
* X0323433	MIC-PII PRF LOAD TEST	EACH	9.000				
X0323768	ELECT HDHOLE 30/24F&L	EACH	7.000				
X0323814	SAN SEW REMOV 18	FOOT	322.000				
X0323900	CONC FDN TY 1	FOOT	4.000				
X0323907	COMMUNICATIONS VAULT	EACH	4.000				
* X0323988	TEMP SOIL RETEN SYSTM	SQ FT	65,060.000				
X0324112	BARRIER BASE	FOOT	466.000				
X0324159	WHITWASH CONC PAVT	SQ YD	9,257.000				
X0324165	COMB SEW ESVCP T2 12	FOOT	10.000				
X0324166	COMB SEW ESVCP T2 15	FOOT	35.000				
X0324347	COMB SEWER RCCP T2 24	FOOT	1,509.000				
X0324350	COMB SEW ESVCP T2 18	FOOT	329.000				
X0324354	TR & BKFIL W SCRNS/ND	FOOT	2,492.000				
X0324415	CON EN RC 1-4 CNC	FOOT	107.000				
X0324420	PVC CON T 4 (S40)	FOOT	3,936.000				
							* REVISED : JANUARY 12, 2005

ILLINOIS DEPARTMENT OF TRANSPORTATION
 SCHEDULE OF PRICES
 CONTRACT NUMBER - 62693

State Job # - C-91-072-04
 PPS NBR - 1-74823-0550
 County Name - COOK - -
 Code - 31 - -
 District - 1 - -
 Section Number - 2021-922 PT.1-AC

Project Number
 ACIM-0943/367/057

Route
 FAI 90/94

Item Number	Pay Item Description	Unit of Measure	Quantity	x	Unit Price	=	Total Price
50102400	CONC REM	CU YD	152.000				
50200100	STRUCTURE EXCAVATION	CU YD	22,627.000				
50300225	CONC STRUCT	CU YD	7,068.000				
50300255	CONC SUP-STR	CU YD	836.000				
50300300	PROTECTIVE COAT	SQ YD	9,740.000				
50300510	RUSTICATION FINISH	SQ FT	42,553.000				
50800105	REINFORCEMENT BARS	POUND	12,370.000				
* 50800205	REINF BARS, EPOXY CTD	POUND	993,160.000				
* DELETED							
* DELETED							
* DELETED							
* 51205200	TEMP SHT PILING	SQ FT	600.000				
54392000	GROUT SLURRY	CU FT	150.000				
550A0330	STORM SEW CL A 2 10	FOOT	39.000				
550A0340	STORM SEW CL A 2 12	FOOT	1,549.000				
* REVISED : JANUARY 12, 2005							