

{AS-BUILT PLANS}

VOIDS UNDER SLOPENALLS WERE FILLED WITH SLURRY MIX CONCRETE

FA. RTE.	SECTION	COUNTY	TOTAL SHEET
FAI 74	(10-7B) BR	CHAMPAIGN	108 24

SHEET 1 OF 17

TOTAL BILL OF MATERIAL (E.B. & W.B.)

ITEM	UNIT	SUB.		TOTAL
		SUPER	PIERS ABUT.	
Structure Excavation	Cu. Yd.		113 952	1065
Class X Concrete	Cu. Yd.		112 100	212
Class X Concrete Superstructure	Cu. Yd.	425		425
REINFORCEMENT BARS (EPOXY COATED)	LBS.	108,000	9,000 15,000	132,000
REMOVAL OF EXISTING CONCRETE DECK NO. 1	Each	1		1
REMOVAL OF EXISTING CONCRETE DECK NO. 2	Each	1		1
Protective Coat	Sq. Yd.	315		315
Steel Pile HP 12x53	Lin. Ft.		228 394	622
Test Pile Steel HP 12x53	Each	2	2	4
Concrete Removal	Cu. Yd.		10 70	80
Floor Drains	Each	40		40
Name Plates	Each	2		2
Temporary Sheet Piling	Sq. Ft.		650	650
Furnishing & Erecting Precast Prestressed Concrete I Beams (42")	Lin. Ft.	321		321
Slopedwall REPAIR	Sq. Yd.			56
Bridge Seat Sealer	Sq. Ft.		54	54
Repair Concrete Structure	Sq. Ft.	225		225
Epoxy Mortar Repair	Cu. Ft.	10 5		15
Preformed Joint Seal 1 3/4"	Lin. Ft.	200		200
Preformed Joint Seal 2 1/2"	Lin. Ft.	100		100
Preformed Joint Seal 4"	Lin. Ft.	100		100
Furnishing & Erecting Structural Steel	Lbs.	12,000	550 750	13,300
Elastomeric Bearing Assembly, Type I	Each		6	6
Bridge Deck Grooving	Sq. Yds	1,550		1,550
Epoxy Crack Sealing	Lin. Ft.	10	50 5	65

†† INCLUDES TOP AND INSIDE FACES OF PARAPETS ONLY.
 ††† ALL SUPERSTRUCTURE REMOVAL IS INCIDENTAL TO "REMOVAL OF EXISTING CONCRETE DECK", OF THE NO. SPECIFIED. SEE SHEET 7 OF 17.

STATION 690 + 06.00
 REBUILT IN 1999 BY
 STATE OF ILLINOIS
 F.A.I. RTE. 74 SEC. (10-7B) BR
 LOADING HS20 & ALT.
 STR. NO. 010-0028

NAME PLATE

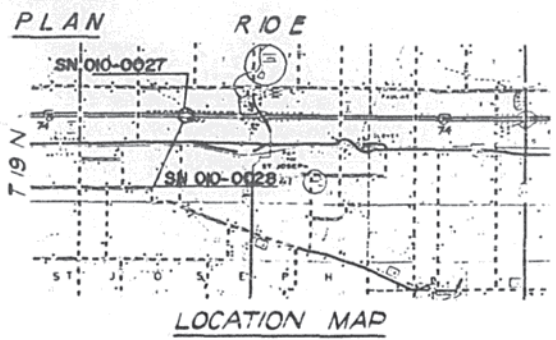
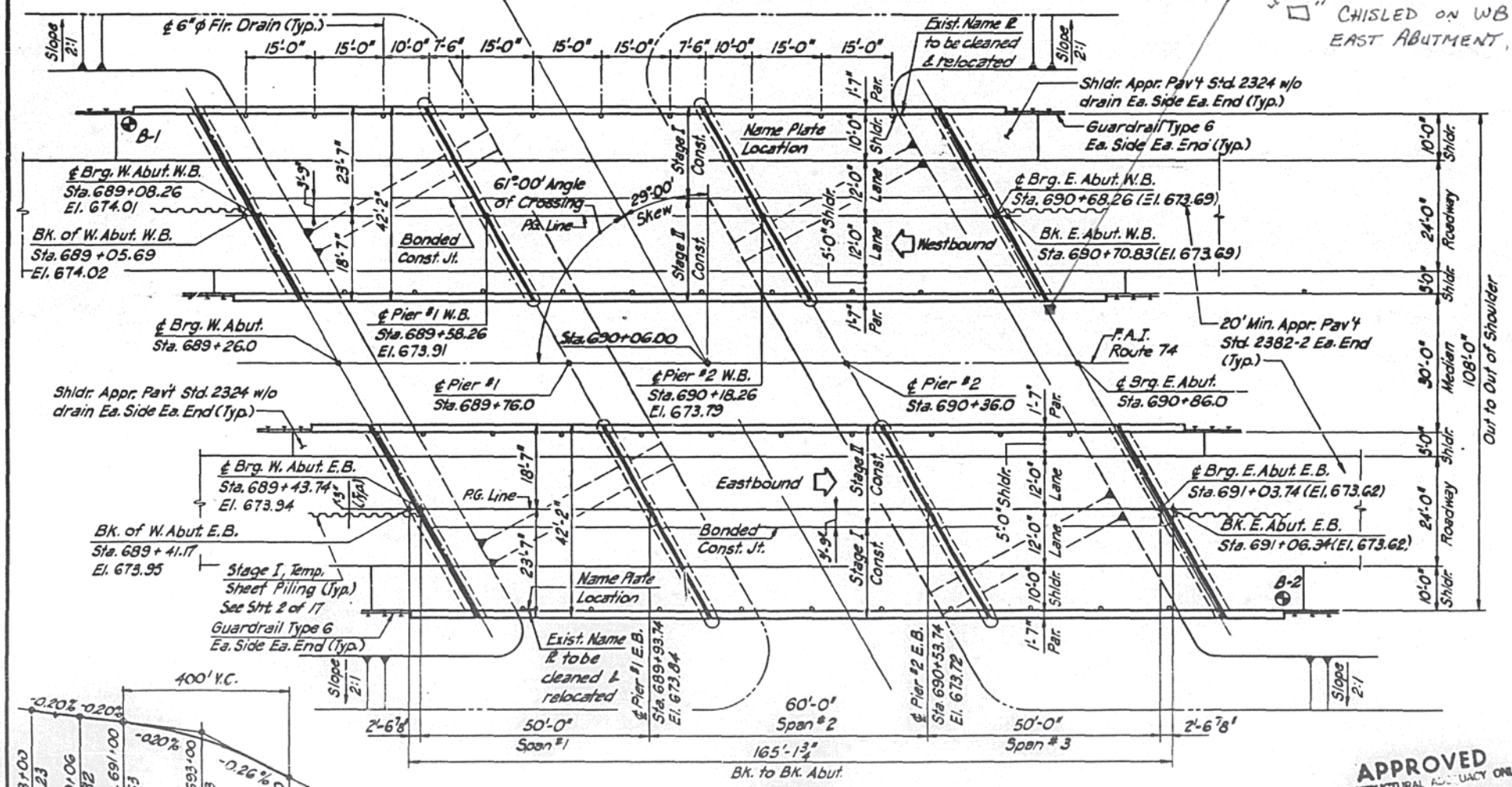
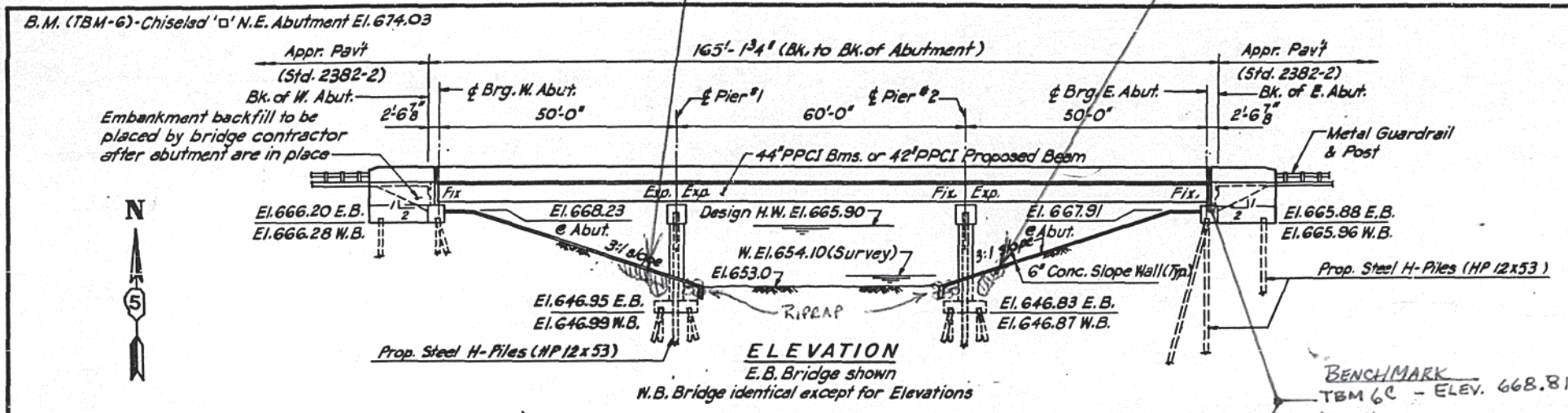
SEE STANDARD 2113
 NOTE: EAST BOUND SHOWN, FOR WEST BOUND
 USE STRUCTURE NUMBER 010-0027

APPROVED
 FOR STRUCTURAL ACCURACY ONLY
Ralph E. Anderson

EXISTING BRIDGE DESCRIPTION
 STRUCTURE NOS. 010-0027, 0028 WERE BUILT IN 1967 CARRYING FAI ROUTE 74 OVER THE SALINE DRAINAGE DITCH, EACH WITH: THREE SIMPLE SPANS 150', 60', 50'. OVERALL LENGTH OF 165' 1 3/4" BACK TO BACK OF ABUTMENTS, 36' 6" OUT TO OUT WIDTH, R.C. DECK ON P.P.C. I-BEAM SUPERSTRUCTURE, SOLID PIERS AND OPEN TYPE ABUTMENTS ON 12" x 42" CAST IN PLACE CONCRETE PILES. THE SUPERSTRUCTURES WILL BE WIDENED TO 42' 2" OUT TO OUT. THE SUBSTRUCTURES AND EXISTING GIRDERS WILL BE REHABILITATED AND REUSED. THE TRAFFIC WILL BE MAINTAINED IN BOTH DIRECTIONS USING STAGE CONSTRUCTION. NO SALVAGE.

GENERAL PLAN
 FAI RTE. 74
 OVER SALINE DRAINAGE DITCH
 SEC. (10-7B) BR
 CHAMPAIGN COUNTY
 STR. NOS. 010-0027, 0028
 STA. 690 + 06.00

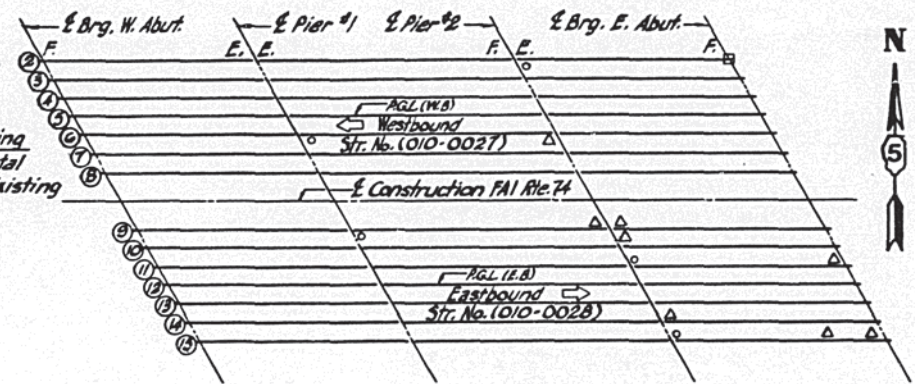
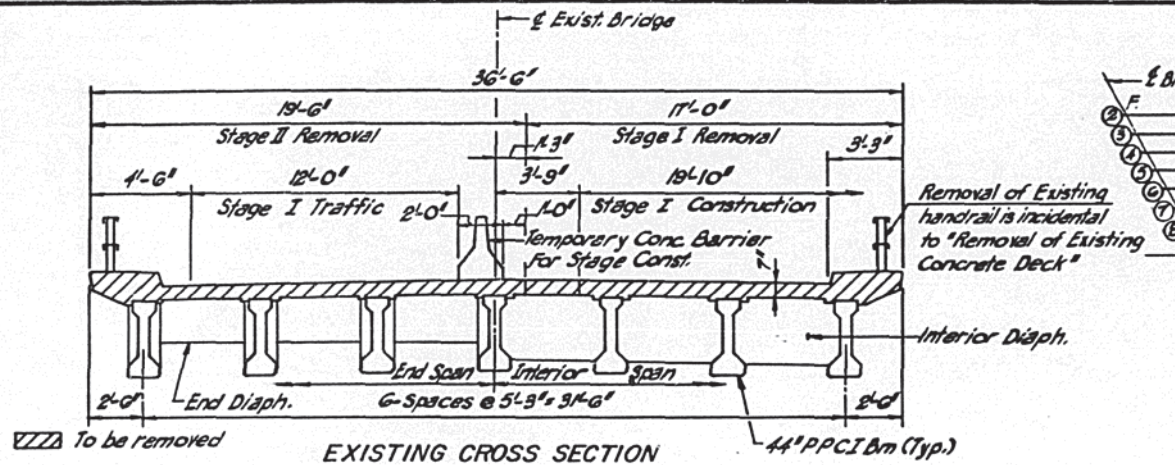
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Expires 04-30-93
Walter Lewis

DESIGNED BY *M. A. F.*
 DRAWN BY *JNK*
 CHECKED BY *M. A. F. QMD*

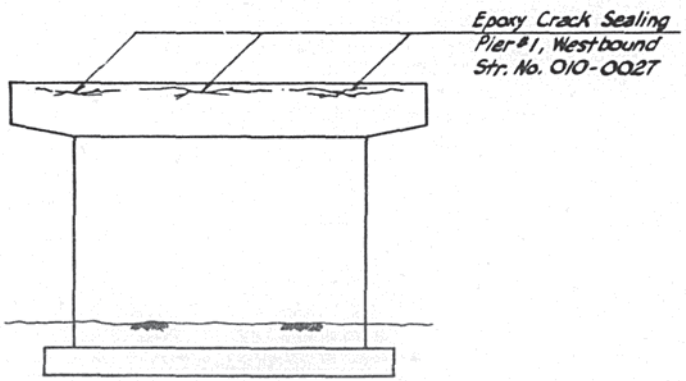
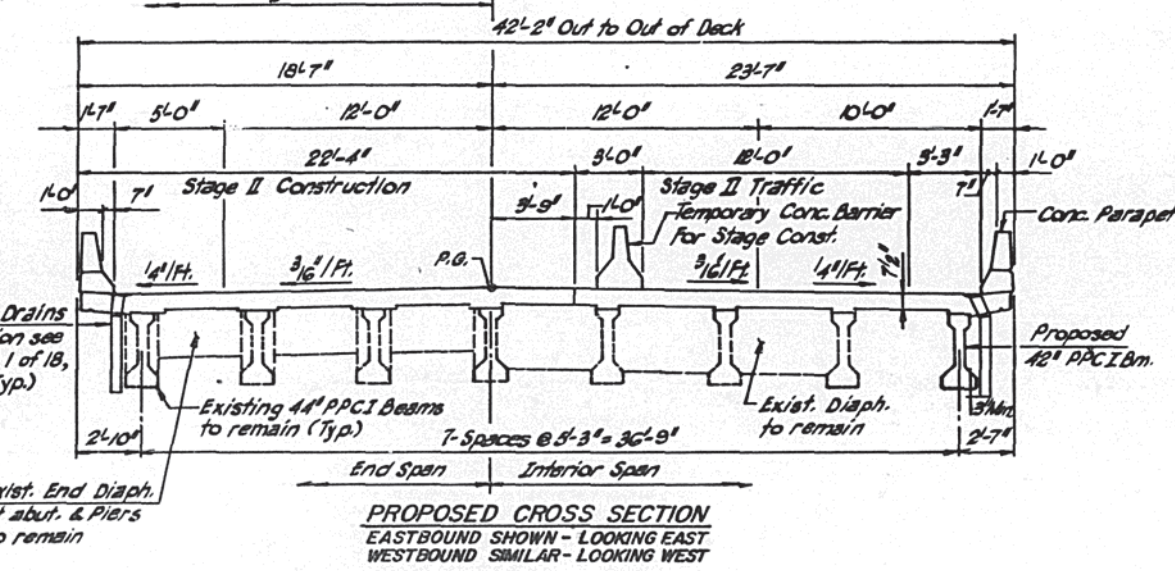
AMIR-FAZLI AND ASSOCIATES, INC.
 CONSULTING ENGINEERS



BILL OF MATERIAL

ITEM	UNIT	QUANTITY
No. of Exp. bearings to be adjusted	Each	5
No. of Fixed bearings to be cleaned or replaced	Each	1

- LEGEND**
- △ Bottom flange of beam to be repaired (Epoxy Mortar Repair)
 - Bearing Plates and bolts to be cleaned or replaced. "See Special Provisions"
 - Exp. Bearing to be adjusted. "See Special Provisions"
 - ▨ Epoxy Mortar Repair
 - ⋈ Epoxy Crack Sealing



BILL OF MATERIAL

ITEM	UNIT	TOTAL
Repair Concrete Structure	Sq. Ft.	5
Epoxy Crack Sealing	Lin. Ft.	60
Epoxy Mortar Repair	Cu. Ft.	15

- LEGEND**
- ⋈ Epoxy Crack Sealing

GENERAL NOTES

CLASS X CONCRETE SHALL BE USED THROUGHOUT.

REINFORCEMENT BARS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M-31, M-42 OR M-53 GRADE 60.

THE EXISTING EMBANKMENT CONFIGURATION IS THE MINIMUM TO BE MAINTAINED BY THE CONTRACTOR. ADDITIONAL MATERIAL SHALL BE PLACED ON TOP AS DIRECTED BY THE ENGINEER. COST INCIDENTAL TO "SUB-BASE GRANULAR MATERIAL, TYPE B-6".

PLAN DIMENSIONS AND DETAILS RELATIVE TO EXISTING STRUCTURE HAVE BEEN TAKEN FROM EXISTING PLANS AND ARE SUBJECT TO NOMINAL CONSTRUCTION VARIATIONS. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO VERIFY SUCH DIMENSIONS AND DETAILS IN THE FIELD AND MAKE NECESSARY APPROVED ADJUSTMENTS PRIOR TO CONSTRUCTION OR ORDERING OF MATERIALS. SUCH VARIATIONS SHALL NOT BE CAUSE FOR ADDITIONAL COMPENSATION FOR A CHANGE IN THE SCOPE OF THE WORK. HOWEVER, THE CONTRACTOR WILL BE PAID FOR THE QUANTITY ACTUALLY FURNISHED AT THE UNIT PRICE BID FOR THE WORK.

THE ZINC-SILICATE AND VINYL PAINT SYSTEM SHALL BE USED FOR SHOP AND FIELD PAINTING OF STRUCTURAL STEEL. THE COLOR OF THE VINYL FINISH COATS SHALL BE MUNSELL NO. 10Y 7/1 LIGHT GREY.

ALL BARRICADES AND TRAFFIC CONTROL DEVICES NOTED IN THE CONTRACT DOCUMENTS AND DEEMED NECESSARY BY THE ENGINEER SHALL BE PLACED PRIOR TO THE START OF CONSTRUCTION.

THE LOCATION, QUANTITIES AND EXTENT OF THE REPAIRS AND/OR REPLACEMENT SHOWN ON THE PLANS ARE BASED ON VISUAL OBSERVATIONS MADE DURING THE FIELD INSPECTION. ADDITIONAL DETERIORATION WHICH COULD NOT BE DETERMINED VISUALLY OR WHICH MAY HAVE OCCURRED AFTER THE INSPECTION SHALL ALSO BE REPAIRED AS DIRECTED BY THE ENGINEER.

ALL MINOR DETERIORATION AND SPALLS AT ABUTMENTS AND PIERS SHALL BE REPAIRED USING PNEUMATIC APPLIED CONCRETE AND SHALL BE PAID FOR AT THE CONTRACT UNIT PRICE PER SQ. FT. FOR "REPAIR CONCRETE STRUCTURES". SEE SPECIAL PROVISION.

ALL CRACKS IN THE CONCRETE FACES OF ABUTMENTS AND PIERS SHALL BE FILLED IN WITH EPOXY CRACK SEALING. SEE SPECIAL PROVISION.

FORMED CONCRETE REPAIR LESS THAN OR EQUAL TO 5' SHALL BE PERFORMED AT THE EXISTING INTERMEDIATE DIAPHRAGMS (SEE SHEET 7 OF 17) AND PIERS AS SHOWN ON PLANS AND SHALL BE PAID FOR AT THE CONTRACT UNIT PRICE PER SQ. FT. FOR "FORMED CONCRETE REPAIR LESS THAN OR EQUAL TO 5'".

REMOVE INTERMEDIATE DIAPHRAGMS BETWEEN LINES "C" AND "D" AS SHOWN ON SHEET 7 OF 17 BEFORE CONSTRUCTING THE NEW DECK.

THE CONTRACTOR WILL BE REQUIRED TO MARK ON TOP OF THE CONCRETE DECK THE LOCATIONS OF THE TOP FLANGE OF ALL THE CONCRETE BEAMS. PRIOR TO ANY REMOVAL OF THE BRIDGE CONCRETE DECK, SAW CUTTING DIRECTLY OVER THE TOP OF THE BEAM FLANGES IS NOT PERMITTED.

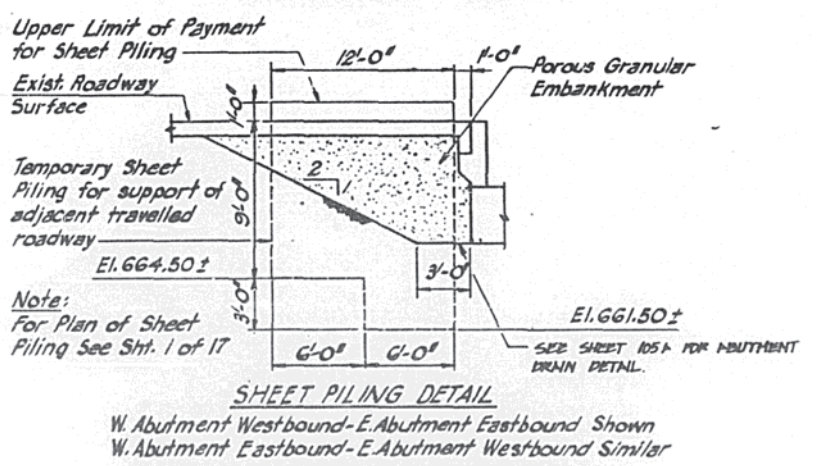
ALL SAWCUTTING SHALL BE CONSIDERED AS INCIDENTAL TO THE REMOVAL ITEM INVOLVED.

SLOPE WALLS SHALL BE REINFORCED WITH WELDED WIRE FABRIC, 6" X 6" - W4.0 X W4.0, WEIGHING 58 LBS. PER 100 SQ. FT.

BRIDGE SEAT SEALER SHALL BE APPLIED TO THE TOP SURFACES OF THE PIERS AND ABUTMENT SEATS.

ESTIMATED BRIDGE SEAT SEALER QUANTITY = 54 SQ. FT.

The Contractor shall drive in permanent locations 4 Steel (HP 12 x 53) test piles 1 Ea. of the W. Abut. and E. Abut. of the E. Bd. Lanes and 1 Ea. of Piers #1 & #2 of the W. Bd. Lanes as directed by the Engineer before ordering the remainder of the Piles.



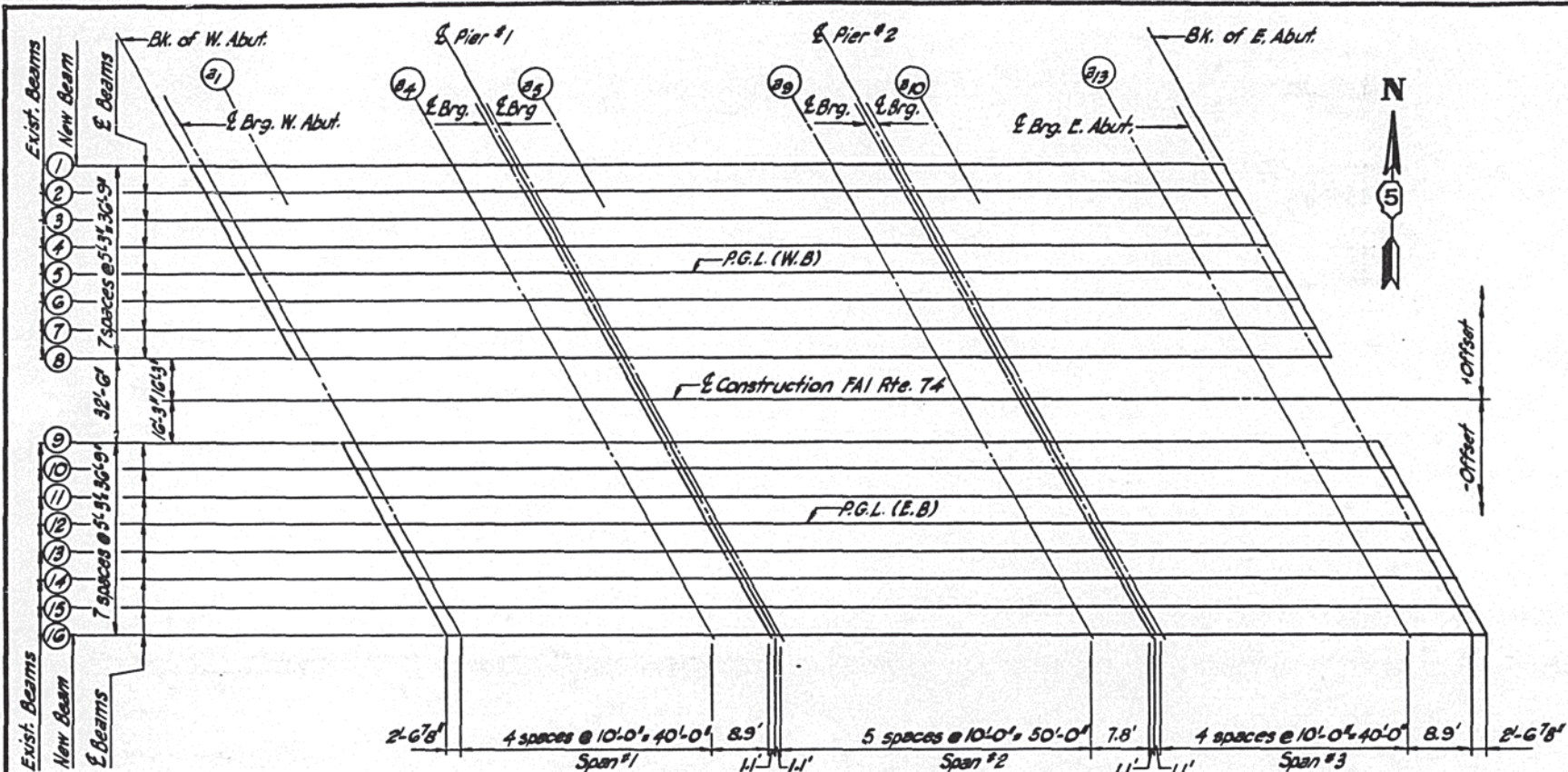
THE INFORMATION SHOWN FOR THE TEMPORARY SHEET PILING IS ESTIMATED. IT IS THE CONTRACTOR'S RESPONSIBILITY TO PROVIDE A DESIGN AND COMPUTATIONS OF THE TEMPORARY SHEET PILING AND ASSOCIATED MEMBERS, IF REQUIRED, SUBJECT TO THE APPROVAL OF THE ENGINEER.

DESIGNED BY N.A.F.
DRAWN BY D.A.
CHECKED BY O.M.D./A.A.

AMIR-FAZLI AND ASSOCIATES, INC.
CONSULTING ENGINEERS

STAGING DETAILS & GENERAL NOTES
FAI RTE. 74
OVER SALINE DRAINAGE DITCH
SEC. (10-7B) BR
CHAMPAIGN COUNTY
STR. NOS. 010-0027,0028
STA. 690 + 06.00

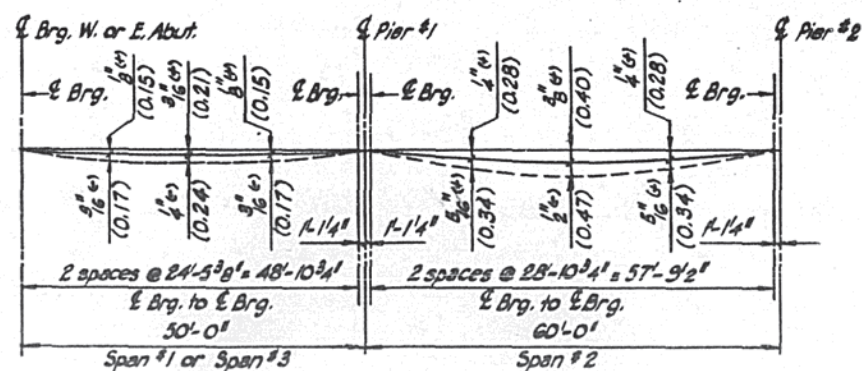
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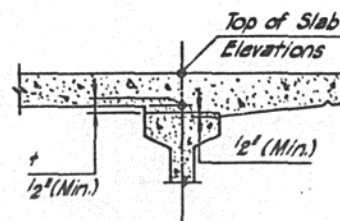
PLAN

BEAM 1

LOCATION	STATION	OFFSET	THEORETICAL GRADE ELEVATION	THEORETICAL GRADE ELEVATION ADJUSTED FOR D.L. DEF.
Bk. W. Abut.	688+94.02	53.00	673.67	673.67
C.L. Brg. W. Abut.	688+96.62	53.00	673.66	673.66
a1	689+06.62	53.00	673.64	673.64
a2	689+16.62	53.00	673.62	673.62
a3	689+26.62	53.00	673.60	673.60
a4	689+36.62	53.00	673.58	673.58
C.L. Brg.	689+45.52	53.00	673.56	673.56
C.L. Pier 1	689+46.62	53.00	673.56	673.56
C.L. Brg.	689+47.72	53.00	673.56	673.56
a5	689+57.72	53.00	673.54	673.53
a6	689+67.72	53.00	673.52	673.51
a7	689+77.72	53.00	673.50	673.48
a8	689+87.72	53.00	673.48	673.47
a9	689+97.72	53.00	673.46	673.45
C.L. Brg.	690+05.52	53.00	673.44	673.44
C.L. Pier 2	690+06.62	53.00	673.44	673.44
C.L. Brg.	690+07.72	53.00	673.44	673.44
a10	690+17.72	53.00	673.42	673.42
a11	690+27.72	53.00	673.40	673.40
a12	690+37.72	53.00	673.38	673.38
a13	690+47.72	53.00	673.36	673.36
C.L. Brg. E. Abut.	690+56.62	53.00	673.34	673.34
Bk. E. Abut.	690+59.22	53.00	673.34	673.34



DEAD LOAD DEFLECTION DIAGRAM
(Includes weight of Concrete only)



FILLET HEIGHTS

TO DETERMINE "t" AFTER ALL PRESTRESSED BEAMS HAVE BEEN ERRECTED, ELEVATIONS OF THE TOP OF THE BEAMS SHALL BE TAKEN AT THE INTERVAL SHOWN IN THE PLAN. THESE ELEVATIONS SUBTRACTED FROM THE "THEORETICAL GRADE ELEVATIONS ADJUSTED FOR DEAD LOAD DEFLECTIONS" SHOWN ON THE FOLLOWING SHEETS, MINUS SLAB (7 1/2") EQUALS THE FILLET HEIGHTS "t" ABOVE THE TOP OF THE BEAMS.

NOTE: THE ABOVE DEFLECTIONS ARE NOT TO BE USED IN THE FIELD IF THE ENGINEER IS WORKING FROM THE GRADE ELEVATIONS ADJUSTED FOR DEAD LOAD DEFLECTIONS AS SHOWN.

BEAM 2

LOCATION	STATION	OFFSET	THEORETICAL GRADE ELEVATION	THEORETICAL GRADE ELEVATION ADJUSTED FOR D.L. DEF.
Bk. W. Abut.	688+96.93	47.75	673.77	673.77
C.L. Brg. W. Abut.	688+99.53	47.75	673.77	673.77
a1	689+09.53	47.75	673.75	673.76
a2	689+19.53	47.75	673.73	673.75
a3	689+29.53	47.75	673.71	673.73
a4	689+39.53	47.75	673.69	673.70
C.L. Brg.	689+48.43	47.75	673.67	673.67
C.L. Pier 1	689+49.53	47.75	673.67	673.67
C.L. Brg.	689+50.63	47.75	673.66	673.66
a5	689+60.63	47.75	673.64	673.67
a6	689+70.63	47.75	673.62	673.66
a7	689+80.63	47.75	673.60	673.65
a8	689+90.63	47.75	673.58	673.62
a9	690+00.63	47.75	673.56	673.58
C.L. Brg.	690+08.43	47.75	673.55	673.55
C.L. Pier 2	690+09.53	47.75	673.55	673.55
C.L. Brg.	690+10.63	47.75	673.54	673.54
a10	690+20.63	47.75	673.52	673.54
a11	690+30.63	47.75	673.50	673.52
a12	690+40.63	47.75	673.48	673.50
a13	690+50.63	47.75	673.46	673.48
C.L. Brg. E. Abut.	690+59.53	47.75	673.45	673.45
Bk. E. Abut.	690+62.13	47.75	673.44	673.44

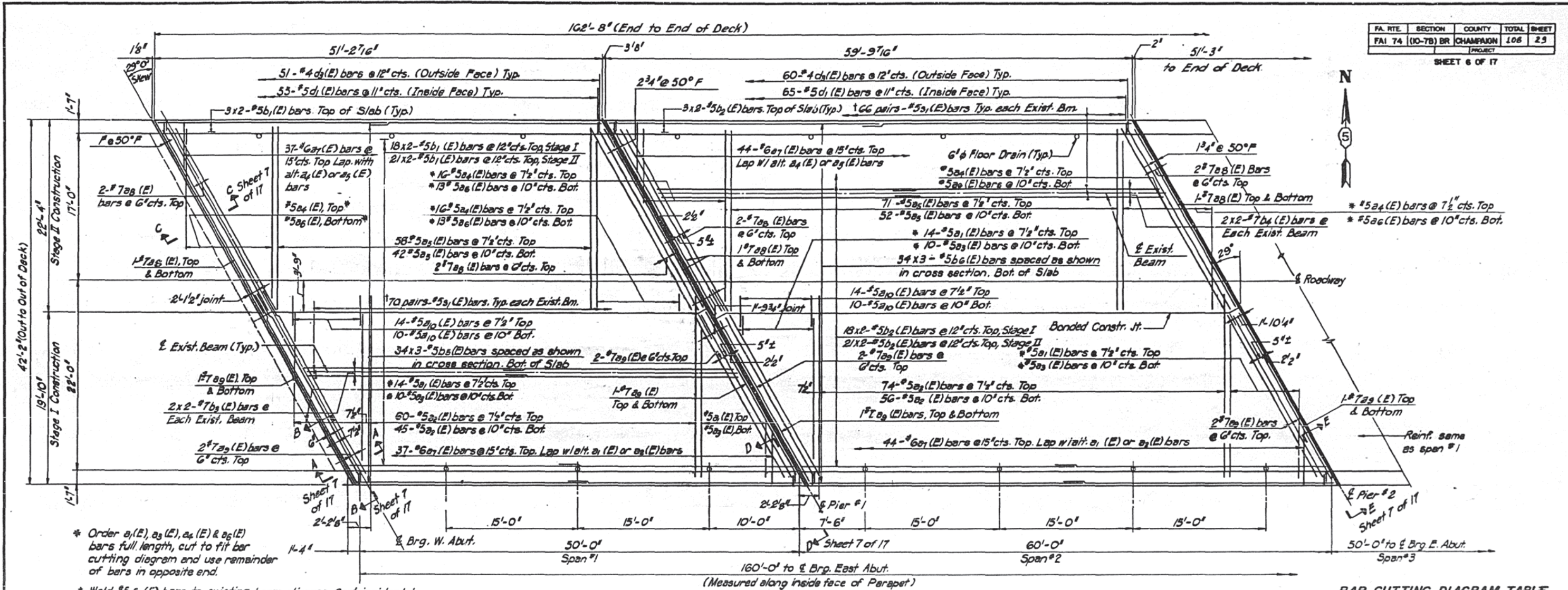
REVISED ELEVATIONS

DESIGNED BY N.A.F.
DRAWN BY b.a.
CHECKED BY O.M.D.

AMIR-FAZLI AND ASSOCIATES, INC.
CONSULTING ENGINEERS

TOP OF SLAB ELEVATIONS
FAI RTE. 74
OVER SALINE DRAINAGE DITCH
SEC. (10-7B) BR
CHAMPAIGN COUNTY
STR. NOS. 010-0027.0028
STA. 690 + 06.00

DATE



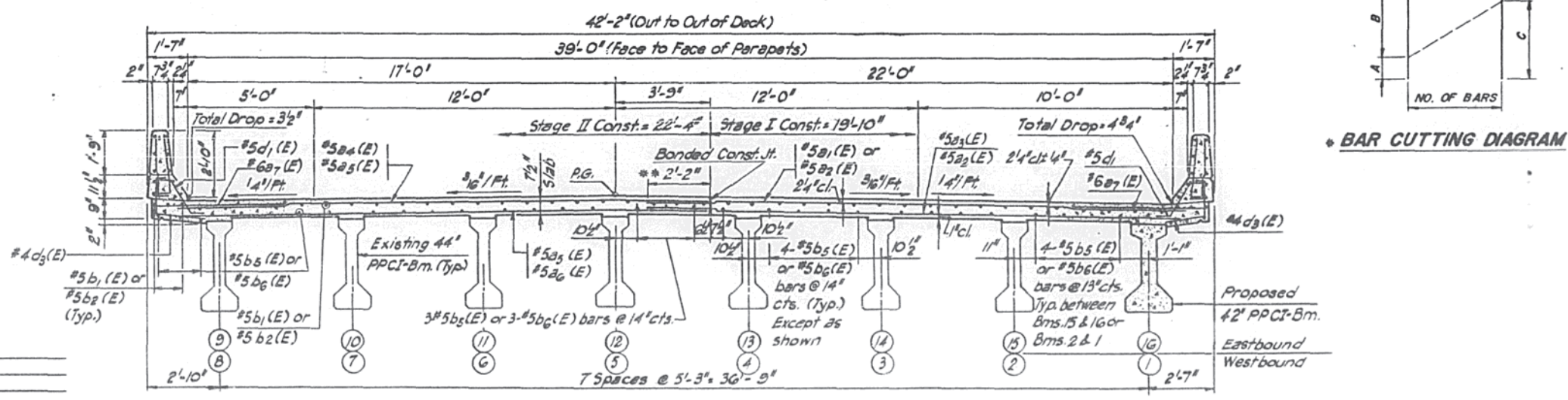
* Order a₁(E), a₂(E), a₃(E) & a₄(E) & a₅(E) bars full length, cut to fit bar cutting diagram and use remainder of bars in opposite end.

† Weld #5 s₁(E) bars to existing beam stirrups. Cost incidental to "Reinforcement Bars, Epoxy Coated"

PARTIAL PLAN
EASTBOUND DECK SHOWN WESTBOUND SIMILAR

BAR CUTTING DIAGRAM TABLE

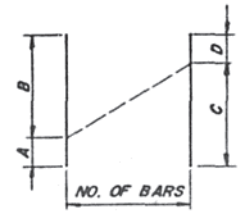
BAR	A	B	C	D	NO. OF BARS & SPACING
a ₁ (E)	2'-4"	17'-0"	17'-0"	2'-4"	14#5 @ 12"
a ₂ (E)	3'-3"	17'-0"	17'-0"	3'-3"	10#5 @ 10"
a ₃ (E)	2'-6"	19'-0"	19'-0"	2'-6"	16#5 @ 12"
a ₄ (E)	1'-6"	19'-0"	19'-0"	1'-6"	13#5 @ 10"



CROSS SECTION
LOOKING EAST FOR EAST BOUND
LOOKING WEST FOR WEST BOUND

** Reinforcement bars in this area only shall have a minimum of 2'-2" lap and be tied with double the number of ties normally used.

BAR CUTTING DIAGRAM



NOTES:

REINFORCEMENT BARS DESIGNATED (E) SHALL BE EPOXY COATED.

BARS INDICATED THUS 29 x 4-#5 etc., INDICATES 29 LINES OF BARS WITH 4 LENGTHS PER LINE.

FOR BILL OF MATERIAL SEE SHEET 8 OF 18.

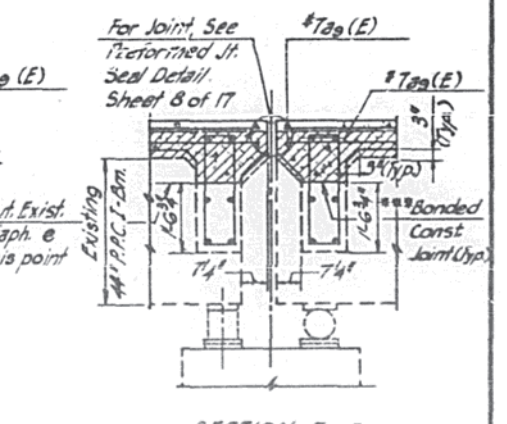
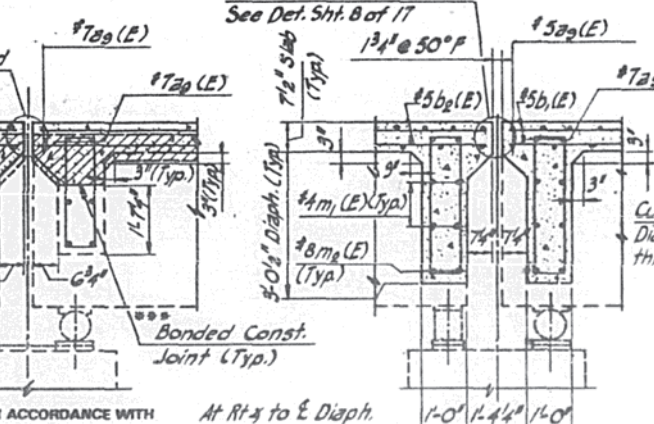
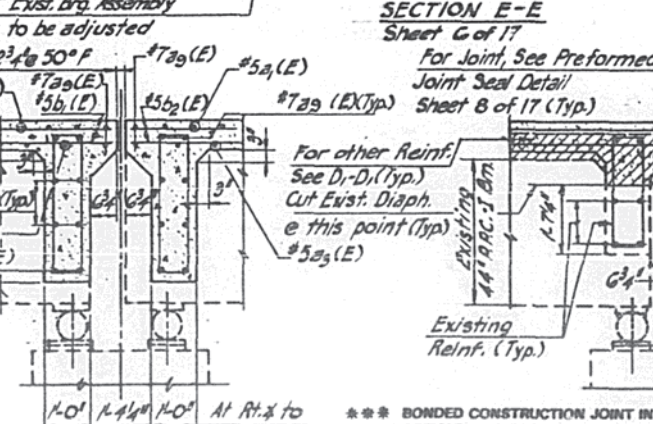
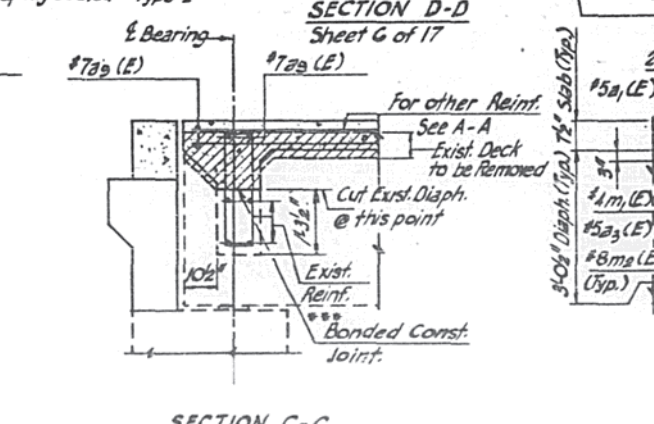
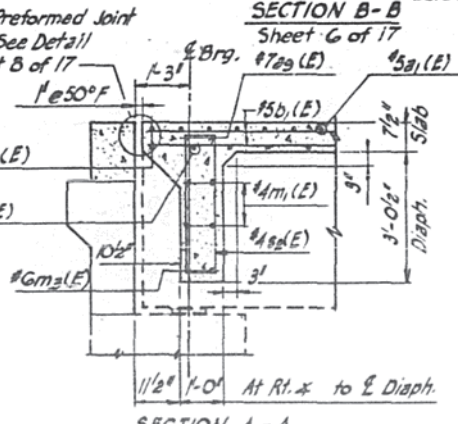
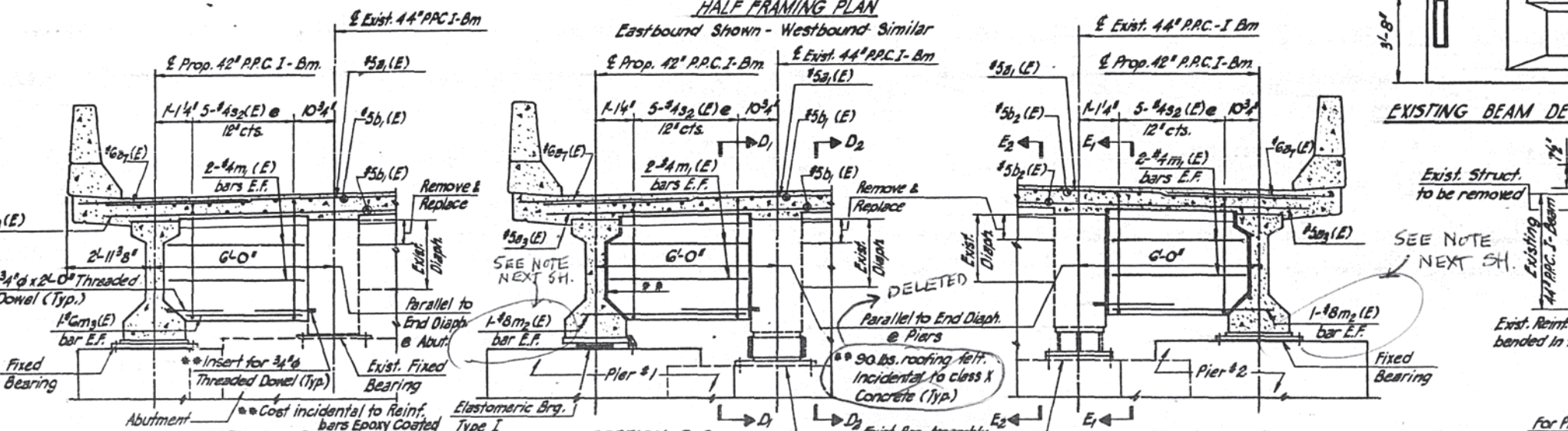
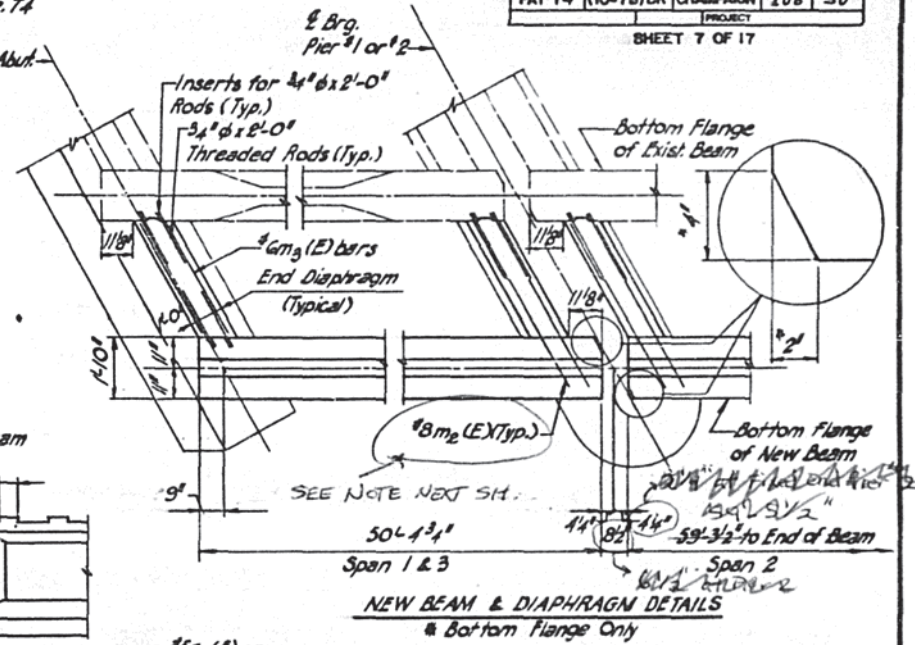
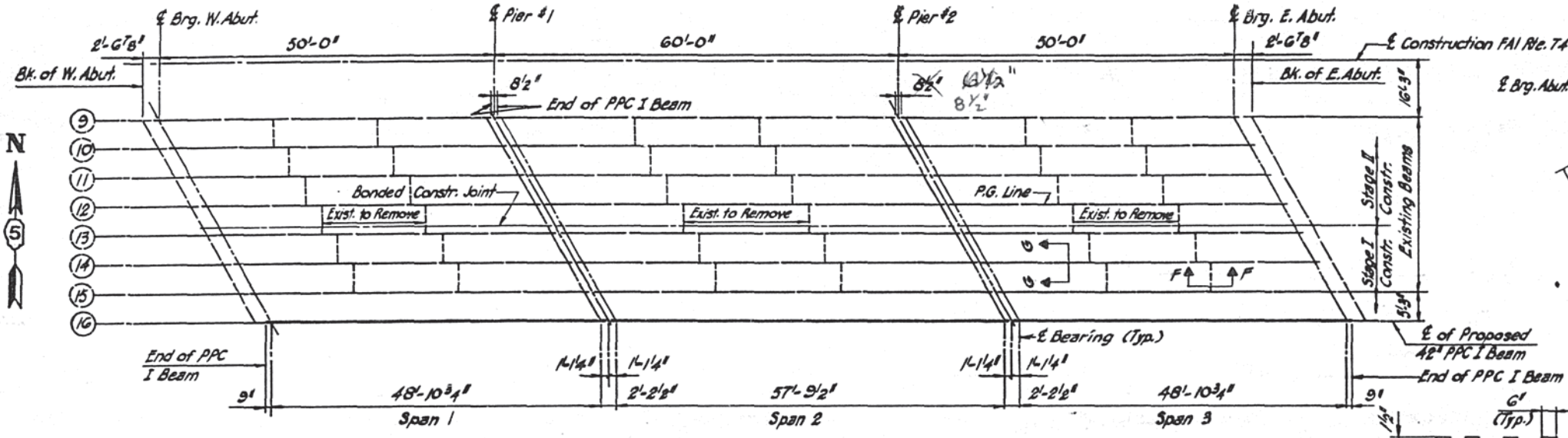
MINIMUM BAR LAPS FOR #5 BARS = 1'-9"
#7 BARS = 2'-8"

DESIGNED BY N.A.F.
DRAWN BY O.S.
CHECKED BY O.M.D.

AMIR-FAZLI AND ASSOCIATES, INC.
CONSULTING ENGINEERS

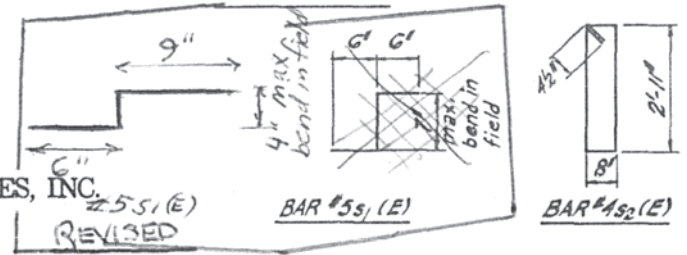
DECK PLAN AND SECTION
FAI RTE. 74
OVER SALINE DRAINAGE DITCH
SEC. (10-7B) BR
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DESIGNED BY N.A.F.
 DRAWN BY b.a.
 CHECKED BY O.M.D.

AMIR-FAZLI AND ASSOCIATES, INC.
 CONSULTING ENGINEERS



DIAPHRAGM DETAILS
 BILL OF MATERIAL

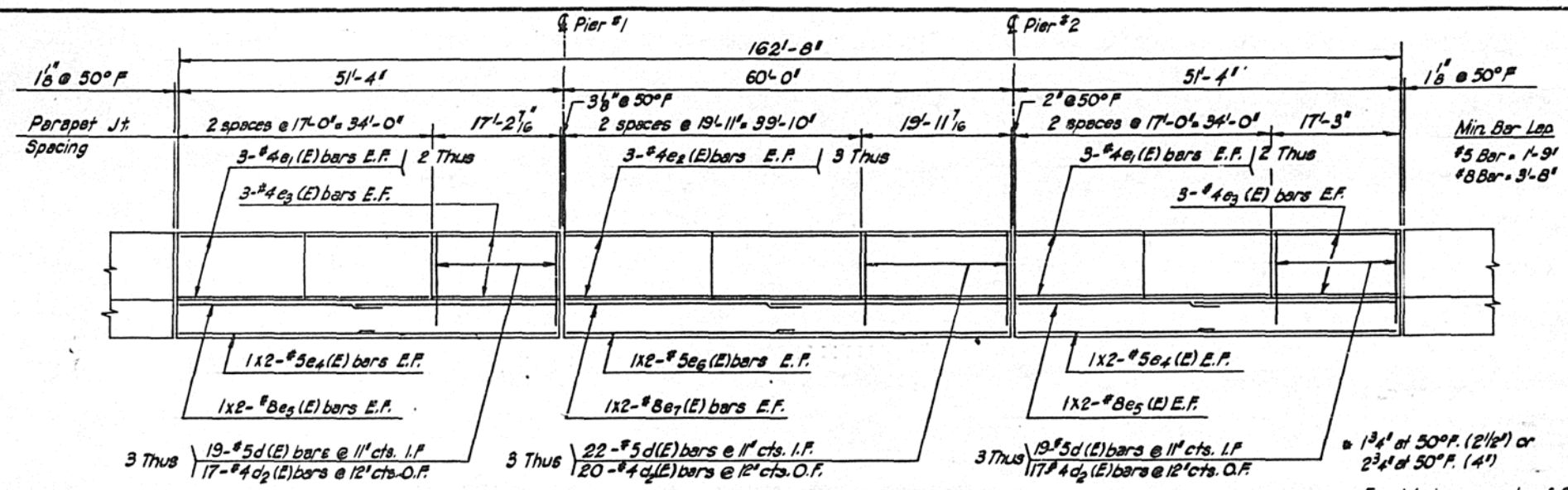
ITEM	UNIT	TOTAL
Repair Concrete Structure	Sq.Ft.	220

NOTE
 EXISTING REINFORCEMENT BARS TO BE CLEANED, STRAIGHTENED AND INCORPORATED INTO NEW CONSTRUCTION. COST INCIDENTAL TO "CLASS X CONCRETE SUPERSTRUCTURE".

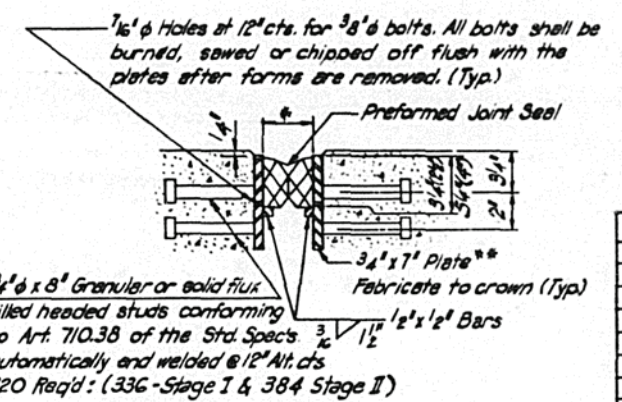
*** WELD #5 S1E BARS TO EXISTING BEAM STIRRUPS. COST INCIDENTAL TO "REINFORCEMENT BARS, EPOXY COATED".

HATCHED PORTIONS INDICATE CONCRETE TO BE REMOVED.

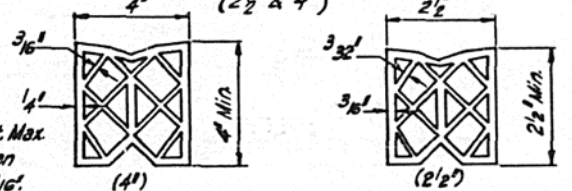
FRAMING PLAN & DIAPHRAGM DETAILS
 FAI RTE. 74
 OVER SALINE DRAINAGE DITCH
 SEC. (10-7B) BR
 CHAMPAIGN COUNTY
 STR. NOS. 010-0027,0028
 STA. 690 + 06.00 DATE



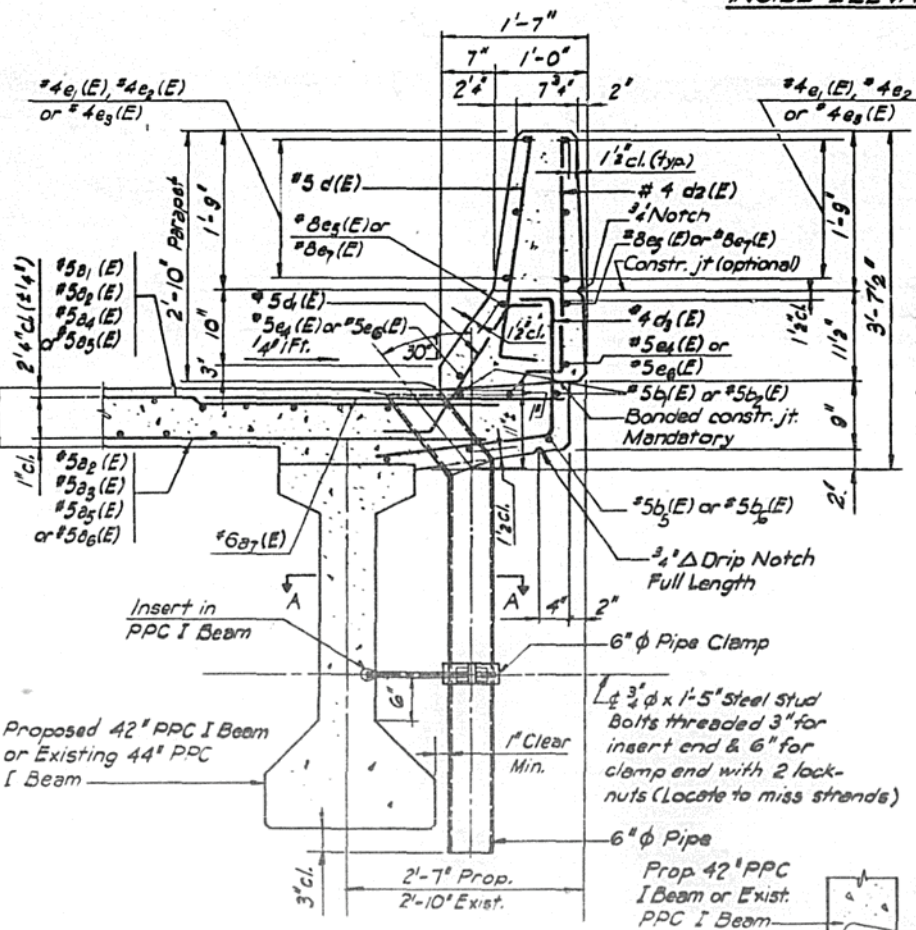
INSIDE ELEVATION OF PARAPET



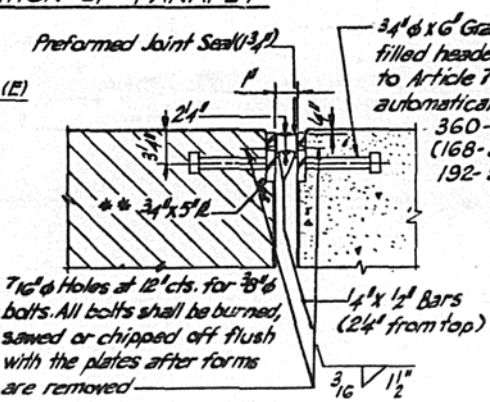
PREFORMED JOINT SEAL DETAIL



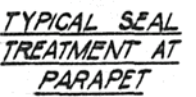
PREFORMED JOINT SEAL



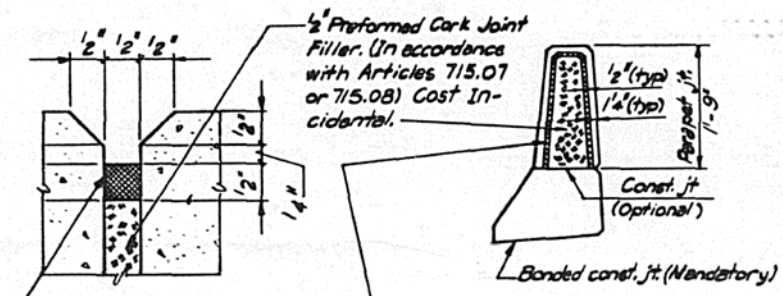
SECTION THRU PARAPET



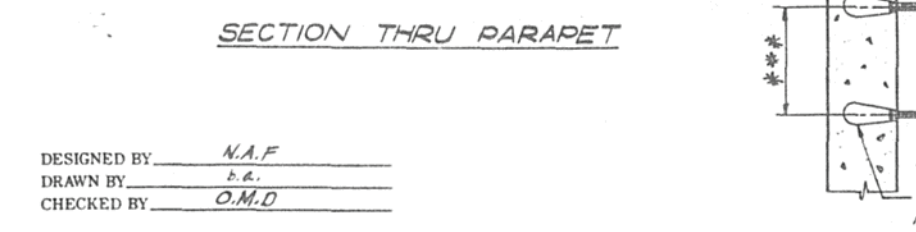
PREFORMED JOINT SEAL DETAIL (1 3/4")



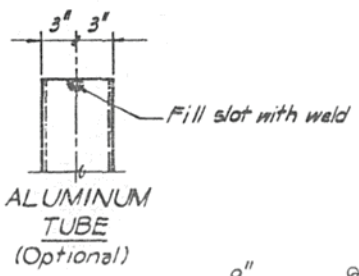
TYPICAL SEAL TREATMENT AT PARAPET



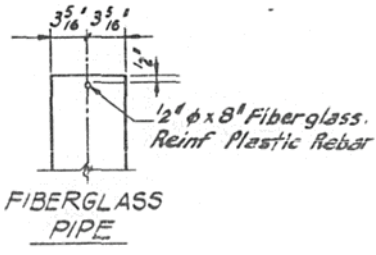
DETAILS OF PARAPET JOINT



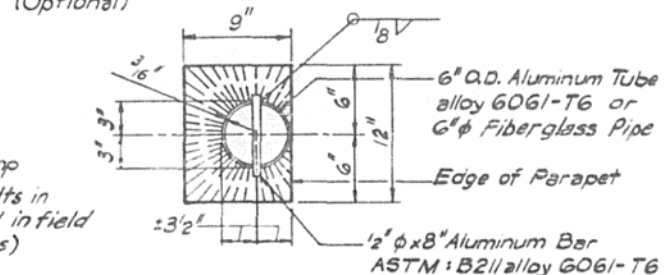
SECTION A-A



ALUMINUM TUBE (Optional)

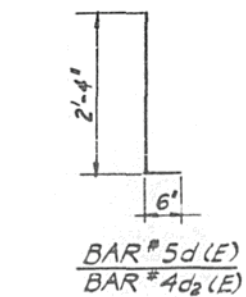


FIBERGLASS PIPE

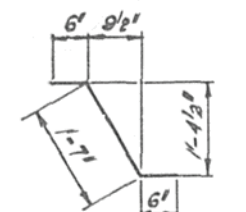


TOP PLAN (Showing Aluminum Tube)

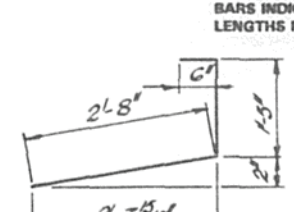
6" DIA. FLOOR DRAIN DETAIL (40 Required)



BAR #5d(E) BAR #4d2(E)



BAR #5d(E)



BAR #4d3(E)

SUPERSTRUCTURE BILL OF MATERIAL

BAR	Nº	SIZE	LENGTH	SHAPE
a1(E)	84	#5	19'-4"	
a2(E)	680	#5	21'-0"	
a3(E)	60	#5	20'-5"	
a4(E)	96	#5	22'-0"	
a5(E)	646	#5	21'-0"	
a6(E)	78	#5	21'-0"	
a7(E)	472	#6	3'-0"	
a8(E)	48	#7	27'-4"	
a9(E)	48	#7	22'-6"	
a10(E)	144	#5	3'-0"	
b1(E)	360	#5	26'-4"	
b2(E)	180	#5	30'-8"	
b3(E)	112	#7	26'-10"	
b4(E)	56	#7	31'-2"	
b5(E)	408	#5	18'-1"	
b6(E)	204	#5	21'-0"	
c1(E)	720	#5	2'-10"	
d1(E)	700	#5	2'-7"	
d2(E)	648	#4	2'-10"	
d3(E)	648	#4	4'-7"	
e1(E)	96	#4	16'-9"	
e2(E)	72	#4	19'-8"	
e3(E)	48	#4	17'-0"	
e4(E)	32	#5	26'-5"	
e5(E)	32	#8	27'-4"	
e6(E)	16	#5	30'-3"	
e7(E)	16	#8	31'-8"	
m1(E)	48	#4	4'-6"	
m2(E)	16	#8	5'-2"	
m3(E)	8	#6	4'-3"	
s1(E)	5768	#5	1'-7"	J
s2(E)	60	#4	7'-11"	B
				ITEM
				UNIT
				QUANTITY
Class X Concrete Superstructure				Cu.Yd. 425
Reinforcement Bars (Epoxy Coated)				Lbs. 108,000

NOTES
FIBERGLASS PIPE SHALL CONFORM TO ASTM D2996, WITH SHORT-TIME RUPTURE STRENGTH HOOP TENSILE STRESS OF 30,000 psi MINIMUM. THE SURFACE OF THE FIBERGLASS PIPE SHALL BE FREE OF BOND INHIBITING AGENTS.

THE EXTERIOR SURFACES OF THE FIBERGLASS FLOOR DRAINS SHALL BE PAINTED WITH ONE COAT OF ALUMINUM PAINT. PAINTING OF THE FIBERGLASS FLOOR DRAINS WILL NOT BE REQUIRED WHEN THE EXTERIOR SURFACES OF THE FURNISHED DRAINS ARE COATED BY THE MANUFACTURER WITH SILVER PIGMENT OR A PIGMENT THAT MATCHES THE COLOR OF THE CONCRETE BEAM.

THE CLAMPING DEVICE AND INSERTS SHALL BE GALVANIZED IN ACCORDANCE WITH AASHTO M-232.

BARS DESIGNATED (E) SHALL BE EPOXY COATED.

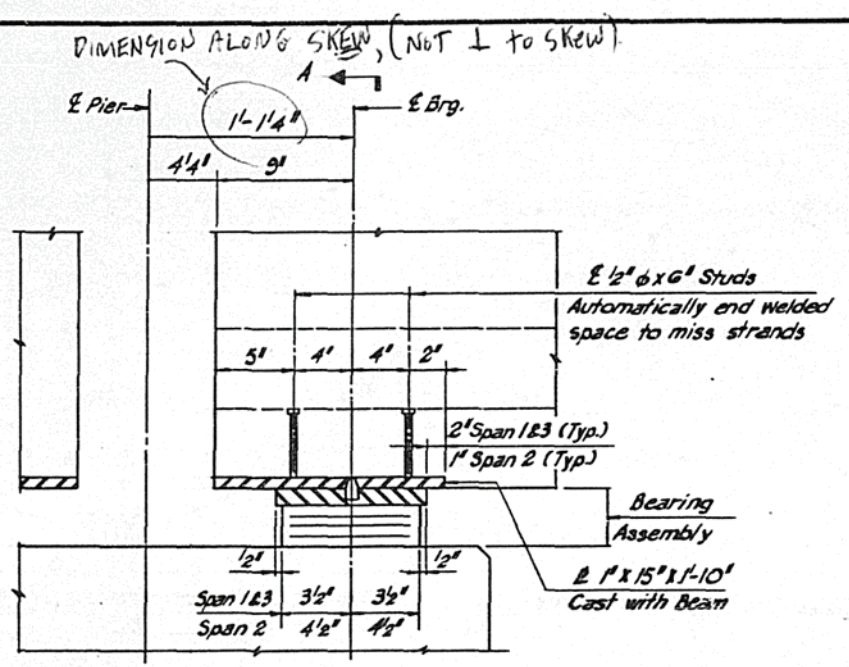
BARS INDICATED THUS 1 X 2 #5 etc., INDICATES ONE LINE OF BARS WITH TWO LENGTHS PER LINE.

PARAPET DETAILS
FAI RTE. 74
OVER SALINE DRAINAGE DITCH
SEC. (10-7B) BR
CHAMPAIGN COUNTY
STR. NOS. 010-0027,0028
STA. 690 + 06.00

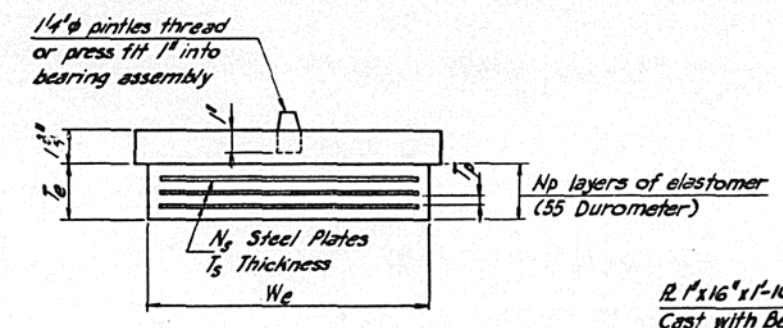
DESIGNED BY N.A.F.
DRAWN BY b.a.
CHECKED BY O.M.D.

AMIR-FAZLI AND ASSOCIATES, INC.
CONSULTING ENGINEERS

DATE



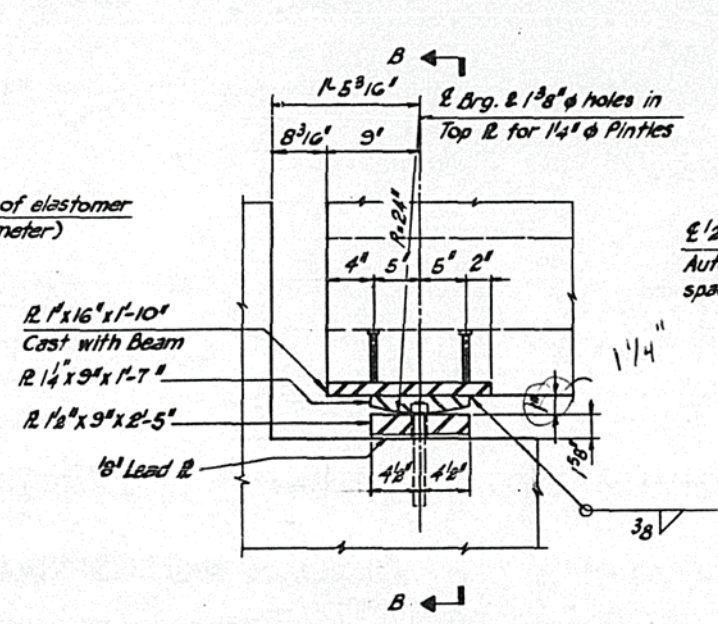
SECTION AT PIER #1 - SPAN 2
Type I Elastomeric Exp. Bearing
Details for Span 1 Expansion Brg. Similar by Rotation
Details for Expansion Brg. e Pier #2 are Similar



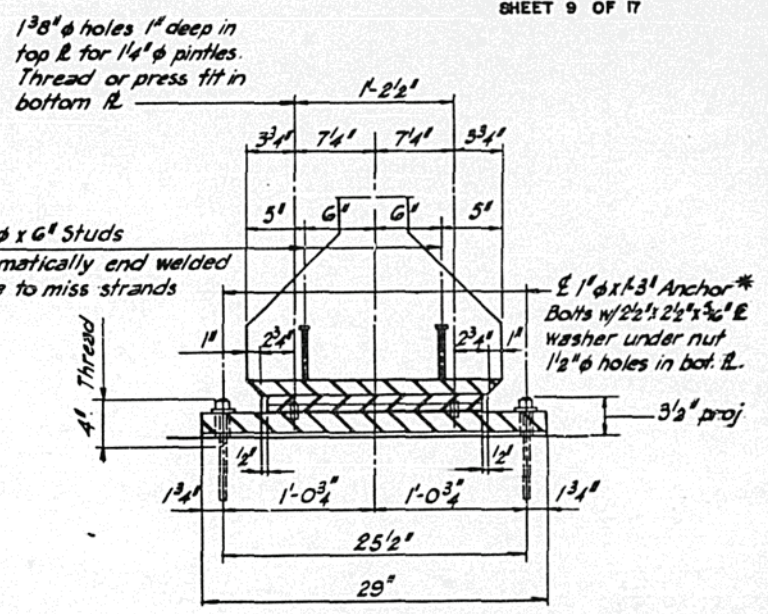
BEARING ASSEMBLY
Note: Shim plates shall not be placed under bearing Assembly

TYPE I - BEARING

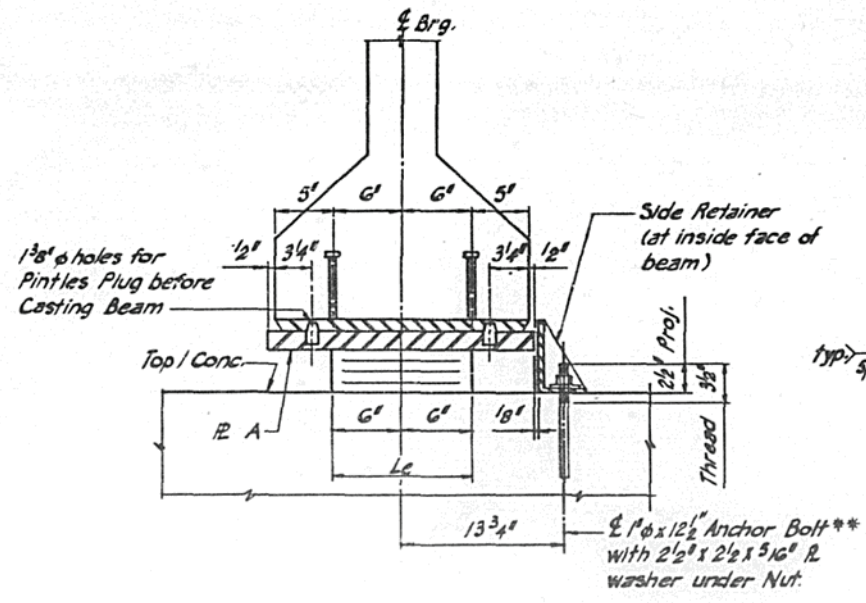
Span	W _e	L _e	T _p	N _p	T _s	N _s	T _e	A
1 & 3	7'	12'	3/8"	4	3/32"	3	1 3/4"	1 3/4" x 8" x 23"
2	9'	12'	3/8"	5	3/32"	4	2 1/4"	1 3/4" x 10" x 23"



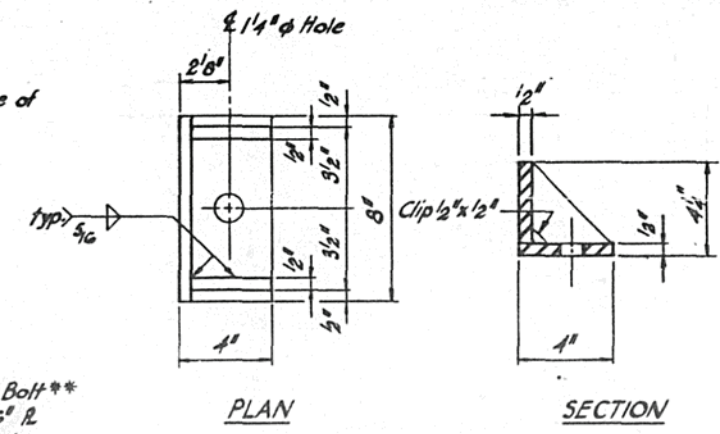
SECTION AT ABUTMENTS
(Fixed Bearing)



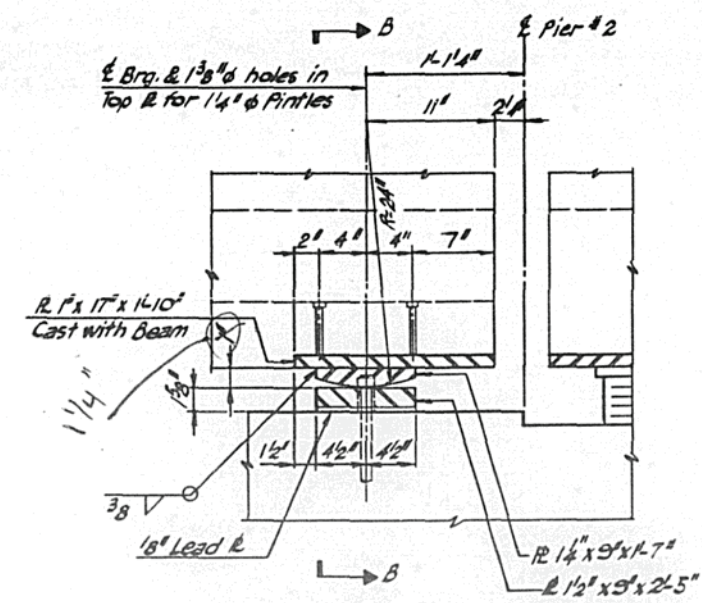
SECTION B-B
Total Number of Fixed Bearing 4



SECTION A-A



SIDE RETAINER
G Required

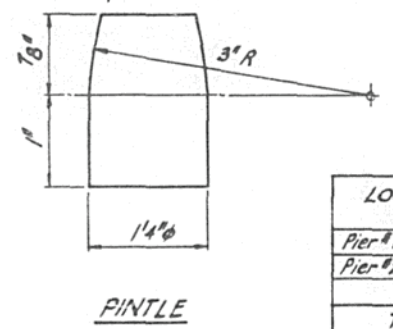


SECTION AT PIER #2 - SPAN 2
(Fixed Bearing)

- NOTES**
- THE STRUCTURAL STEEL BEARING PLATES OF ELASTOMERIC BEARING ASSEMBLIES AND FIXED BEARING ASSEMBLIES SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M223, GRADE 50.
 - ALL EXPOSED SHIM PLATES AND BEARING PLATES SHALL BE AASHTO M223, GRADE 50. COST INCIDENTAL TO THE ITEM INVOLVED.
 - EQUIVALENT ROLLED ANGLE WITH STIFFENERS WILL BE ALLOWED IN LIEU OF WELDED PLATES FOR SIDE RETAINER.
 - ALL ANCHORED BEARING PLATES IN PRESTRESSED CONCRETE BEAMS ARE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE BEAMS (3'-6" DEPTH).
 - ANCHOR BOLTS MAY BE BUILT INTO THE MASONRY OR DRILLED AND GROUTED IN PLACE AFTER BEAMS HAVE BEEN ERECTED. SEE SHEET 10 OF 17 FOR INSTALLATION.
 - AFTER BEAMS HAVE BEEN ERECTED, HOLES SHALL BE DRILLED AND ANCHOR BOLTS GROUTED IN PLACE. SEE SHEET 10 OF 17 FOR ANCHOR BOLT INSTALLATION.

DESIGNED BY N.A.F. A.A.
DRAWN BY b.a.
CHECKED BY O.M.D.

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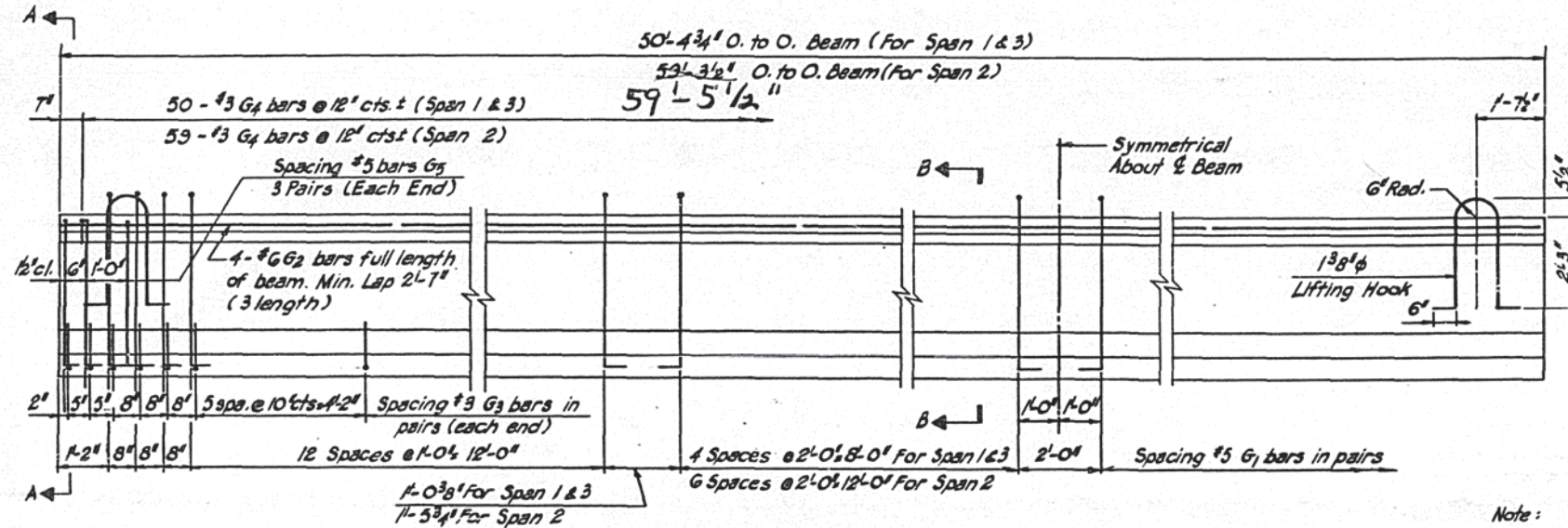


PINTLE

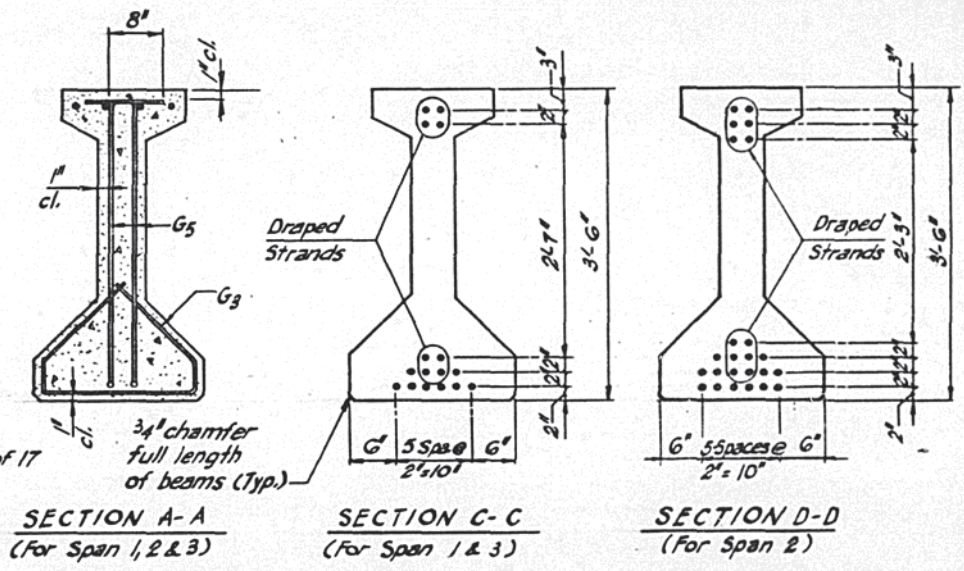
LOCATION	No. of Elast. Brg. Type I
Pier #1 (E.B. & W.B.)	4
Pier #2 (E.B. & W.B.)	2
Total	6

ELASTOMERIC AND FIXED BEARINGS
FAI RTE. 74
OVER SALINE DRAINAGE DITCH
SEC. (10-7B) BR
CHAMPAIGN COUNTY
STR. NOS. 010-0027,0028
STA. 690 + 06.00

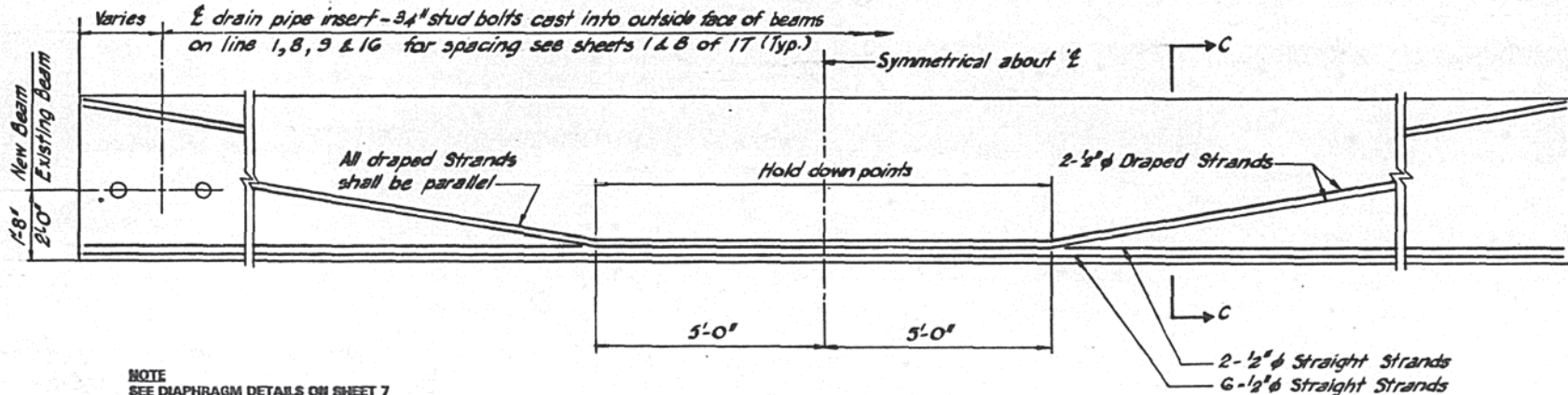
DATE



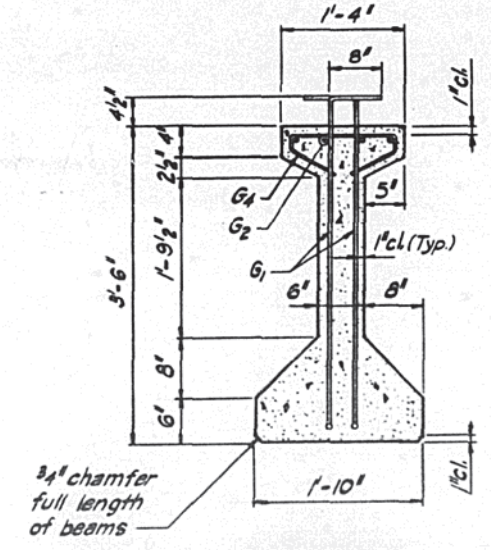
ELEVATION OF BEAM
Showing Reinforcement & Dimensions



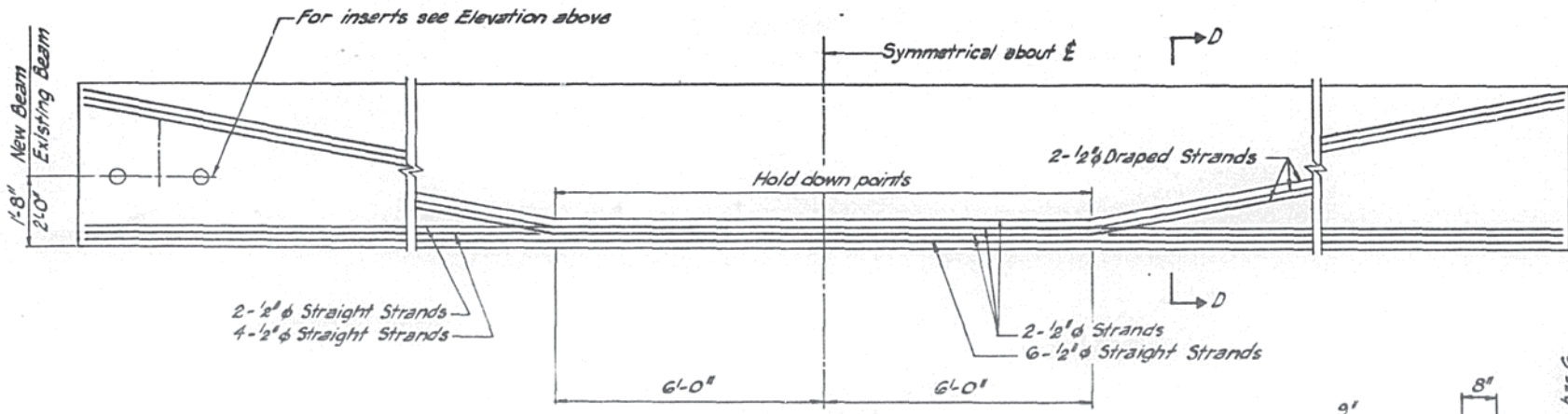
Note:
For diaphragm detail
& Inserts see Sheet 7 of 17



ELEVATION OF BEAM
Showing Prestressing steel
(For Span 1 & 3)



SECTION B-B
(For Span 1, 2 & 3)



ELEVATION OF BEAM
Showing Prestressed Steel
(For Span 2)

BAR LIST (One Beam), for Span 1 & 3

Bar	No.	Size	Length	Shape
G1	84	#5	5'-2"	TL
G2	12	#6	18'-3"	—
G3	44	#3	3'-0"	C
G4	30	#3	2'-6"	C
G5	12	#5	4'-7"	TL

4 Beams Required
BAR LIST (One Beam), for Span 2

Bar	No.	Size	Length	Shape
G1	92	#5	5'-2"	TL
G2	12	#6	21'-9"	—
G3	44	#3	3'-0"	C
G4	39	#3	2'-6"	C
G5	12	#5	4'-7"	TL

2 Beams Required
BILL OF MATERIAL (For Span 1 & 3)

Item	Unit	Total
Furnishing & Erecting Precast Prestressed Concrete I-Beams, 42"	Lin. Ft.	202

BILL OF MATERIAL (For Span 2)

Item	Unit	Total
Furnishing & Erecting Precast Prestressed Concrete I-Beams, 42"	Lin. Ft.	119

NOTES

PRESTRESSING STEEL SHALL BE NON-GALVANIZED HIGH STRENGTH LOW RELAXATION 7 WIRE STRAND, GRADE 270.

ALL INSERTS AND THREADED RODS FOR INSERTS, REINFORCING AND PRESTRESSING STEEL AND OTHER ITEMS WHICH ARE CAST INTO THE PRECAST CONCRETE GIRDERS SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE PER LINEAL FOOT OF "FURNISHING AND ERECTING PRECAST PRESTRESSED CONCRETE I BEAM 42".

PRESTRESSING STEEL SHALL BE A NOMINAL DIAMETER OF 1/2" AND A NOMINAL CROSS-SECTIONAL AREA SHALL BE 0.153 SQ. IN.

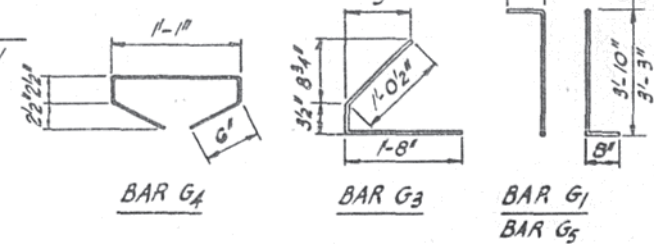
INSERTS FOR 3/4" φ THREADED RODS ARE TO BE TWO TRUST, SINGLE COIL, FLARED LOOP TYPE FOR EXTERIOR GIRDERS.

STEEL FOR LIFTING HOOKS SHALL BE NON-DEFORMED BARS f_y = 40,000 psi.

REQUIRED RELEASE STRENGTH f_{cl} SHALL BE 4,200 psi.

NON-PRESTRESSING STEEL SHALL CONFORM TO AASHTO DESIGNATION M-31, M-42, OR M-52, GRADE 60.

SEE DIAPHRAGM DETAILS ON SHEET 7 OF 17 BEFORE FABRICATING.



DESIGNED BY: N.A.F.
DRAWN BY: E.A.
CHECKED BY: A.A.

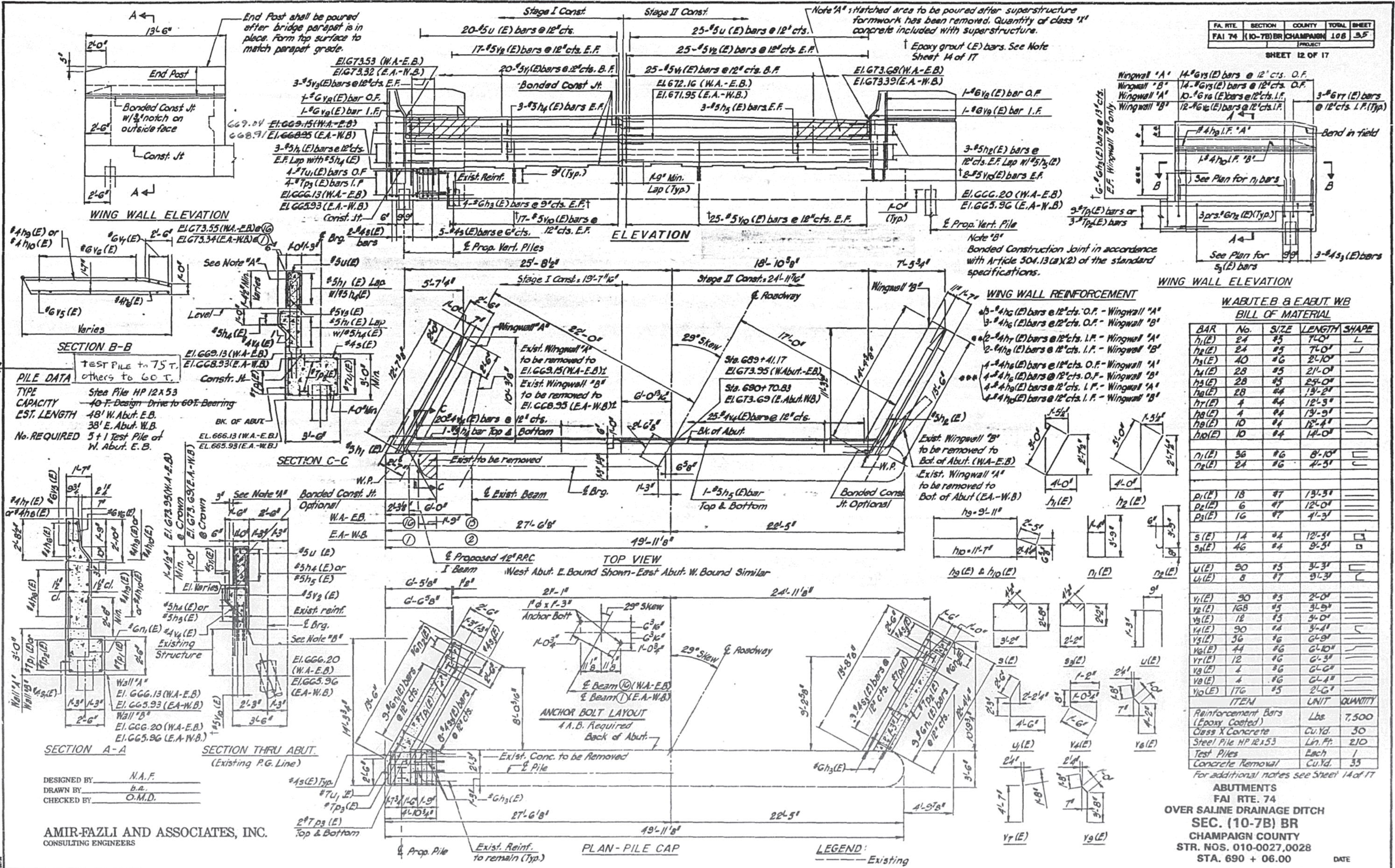
AMIR-FAZLI AND ASSOCIATES, INC.
CONSULTING ENGINEERS

PRESTRESSED BEAMS - SPANS 1, 2 & 3
FAI RTE. 74
OVER SALINE DRAINAGE DITCH
SEC. (10-7B) BR
CHAMPAIGN COUNTY
STR. NOS. 010-0027.0028
STA. 690 + 06.00

DATE

FAI RTE	SECTION	COUNTY	TOTAL SHEET
FAI 74	(10-7B) BR	CHAMPAIGN	108

SHEET 12 OF 17



W.ABUT. E.B. & E.ABUT. W.B. BILL OF MATERIAL

BAR	No.	SIZE	LENGTH	SHAPE
h1(E)	24	#5	7'0"	L
h2(E)	24	#5	7'0"	L
h3(E)	40	#6	2'10"	L
h4(E)	28	#5	2'10"	L
h5(E)	28	#5	2'5"	L
h6(E)	28	#4	1'5"	L
h7(E)	4	#4	12'5"	L
h8(E)	4	#4	1'5"	L
h9(E)	10	#4	1'5"	L
h10(E)	10	#4	1'5"	L
n1(E)	36	#6	8'10"	L
n2(E)	24	#6	4'5"	L
D1(E)	18	#7	1'3'3"	L
D2(E)	6	#7	12'0"	L
D3(E)	16	#7	4'5"	L
S(E)	14	#4	12'5"	L
S2(E)	46	#4	9'5"	L
U(E)	90	#5	3'3"	L
U2(E)	8	#7	9'3"	L
V1(E)	90	#5	2'0"	L
V2(E)	168	#5	3'5"	L
V3(E)	18	#5	5'0"	L
V4(E)	90	#4	3'2"	L
V5(E)	36	#6	6'8"	L
V6(E)	44	#6	6'10"	L
V7(E)	12	#6	6'3"	L
V8(E)	2	#6	6'6"	L
V9(E)	4	#6	6'1"	L
V10(E)	176	#5	2'6"	L
ITEM	UNIT	QUANTITY		
Reinforcement Bars (Epoxy Coated)	Lbs	7,500		
Class X Concrete	Cu. Yd.	30		
Steel Pile HP 12x53	Lin. Ft.	210		
Test Piles	Each	1		
Concrete Removal	Cu. Yd.	33		

For additional notes see Sheet 14 of 17

ABUTMENTS
 FAI RTE. 74
 OVER SALINE DRAINAGE DITCH
 SEC. (10-7B) BR
 CHAMPAIGN COUNTY
 STR. NOS. 010-0027,0028
 STA. 690 + 06.00
 DATE

VIA: AL GOODFIELD
 BRIDGE OFFICE, SPRINGFIELD, ILL.
 12-22-92

DESIGNED BY: N.A.F.
 DRAWN BY: b.a.
 CHECKED BY: O.M.D.

AMIR-FAZLI AND ASSOCIATES, INC.
 CONSULTING ENGINEERS

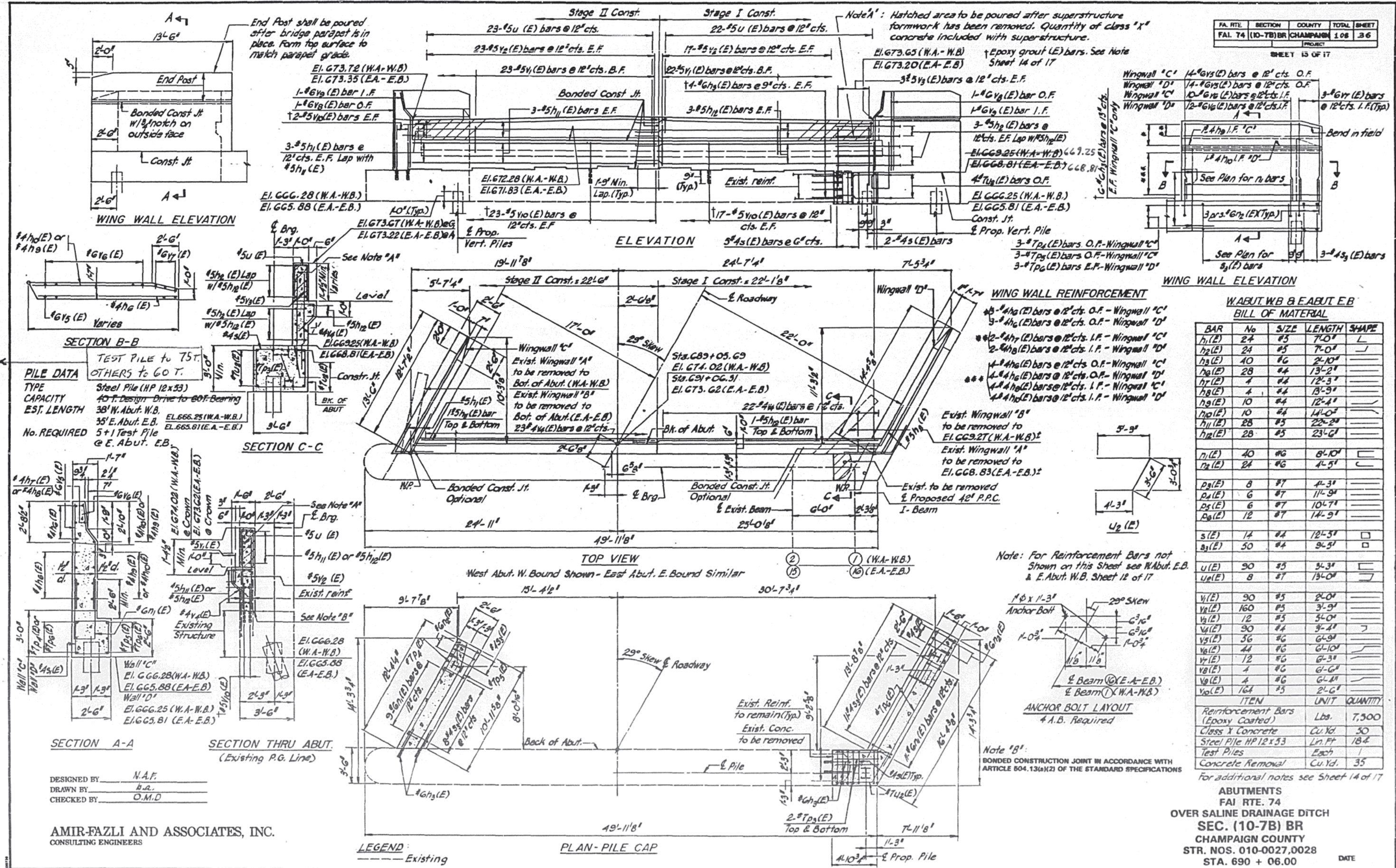
NEW BRG SEAT ELEVS: W. ABUT. -WB → 669.25 ; E. ABUT. -EB → 668.81

← CHANGES MADE DUE TO EFFECT OF BEAM CAMBER

FAI RTE	SECTION	COUNTY	TOTAL SHEET
FAI 74	(10-7B) BR	CHAMPAIGN	108 36

SHEET 15 OF 17

AL GOODFIELD
BRIDGE OFFICE, SPRINGFIELD, ILL.
12-22-92



WABUT.WB & EABUT.EB BILL OF MATERIAL

BAR	No	SIZE	LENGTH	SHAPE
h1(E)	24	#5	7'-0"	L
h2(E)	24	#5	7'-0"	L
h3(E)	40	#6	2'-10"	L
h4(E)	28	#4	13'-2"	L
h7(E)	4	#4	12'-3"	L
h8(E)	4	#4	13'-3"	L
h9(E)	10	#4	12'-4"	L
h10(E)	10	#4	14'-0"	L
h11(E)	28	#5	22'-2"	L
h12(E)	28	#5	23'-0"	L
n1(E)	40	#6	8'-10"	L
n2(E)	24	#6	4'-5"	L
p3(E)	8	#7	4'-3"	L
p4(E)	6	#7	11'-9"	L
p5(E)	6	#7	10'-7"	L
p6(E)	12	#7	14'-9"	L
s(E)	14	#4	12'-3"	L
s2(E)	50	#4	9'-5"	L
u(E)	90	#5	3'-3"	L
u2(E)	8	#7	13'-0"	L
v1(E)	90	#5	2'-0"	L
v2(E)	160	#5	3'-9"	L
v3(E)	12	#5	3'-0"	L
v4(E)	30	#4	3'-4"	L
v5(E)	36	#6	6'-9"	L
v6(E)	44	#6	6'-10"	L
v7(E)	12	#6	6'-3"	L
v8(E)	4	#6	6'-0"	L
v9(E)	4	#6	6'-0"	L
v10(E)	164	#5	2'-6"	L

ITEM	UNIT	QUANTITY
Reinforcement Bars (Epoxy Coated)	Lbs.	7,300
Class X Concrete	Cu.Yd.	50
Steel Pile HP 12x53	Lin.Ft.	184
Test Piles	Each	1
Concrete Removal	Cu.Yd.	35

For additional notes see Sheet 14 of 17

ABUTMENTS
FAI RTE. 74
OVER SALINE DRAINAGE DITCH
SEC. (10-7B) BR
CHAMPAIGN COUNTY
STR. NOS. 010-0027,0028
STA. 690 + 06.00

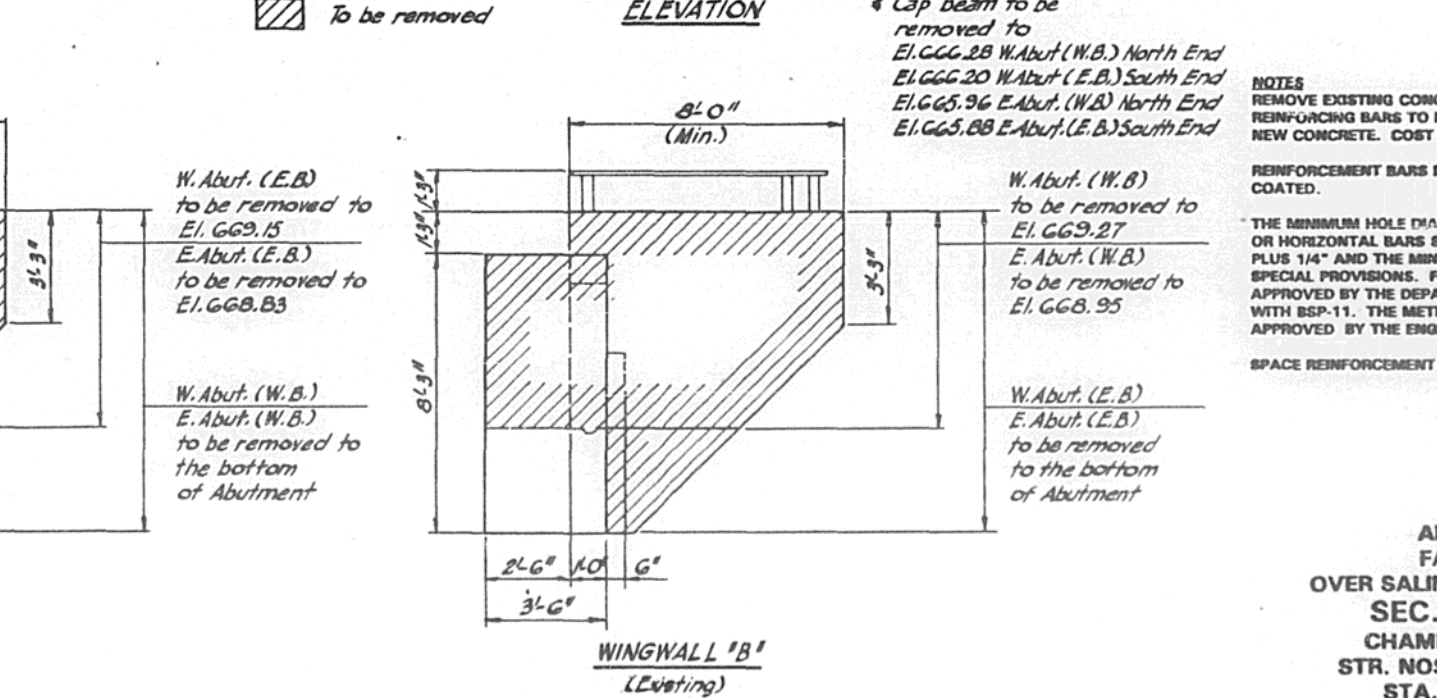
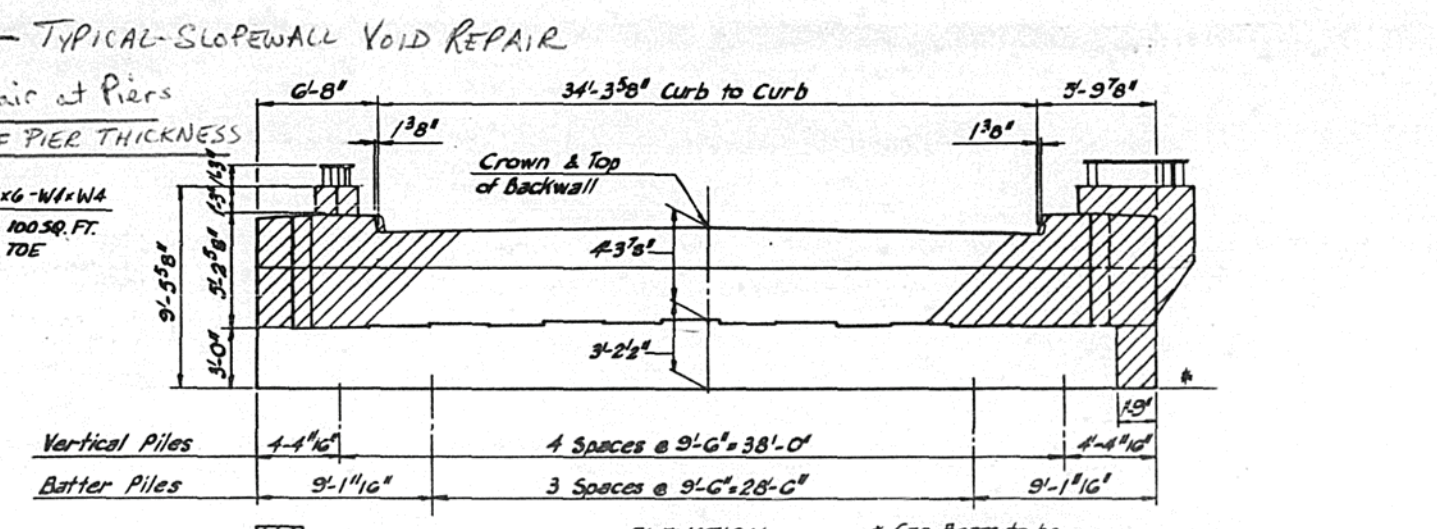
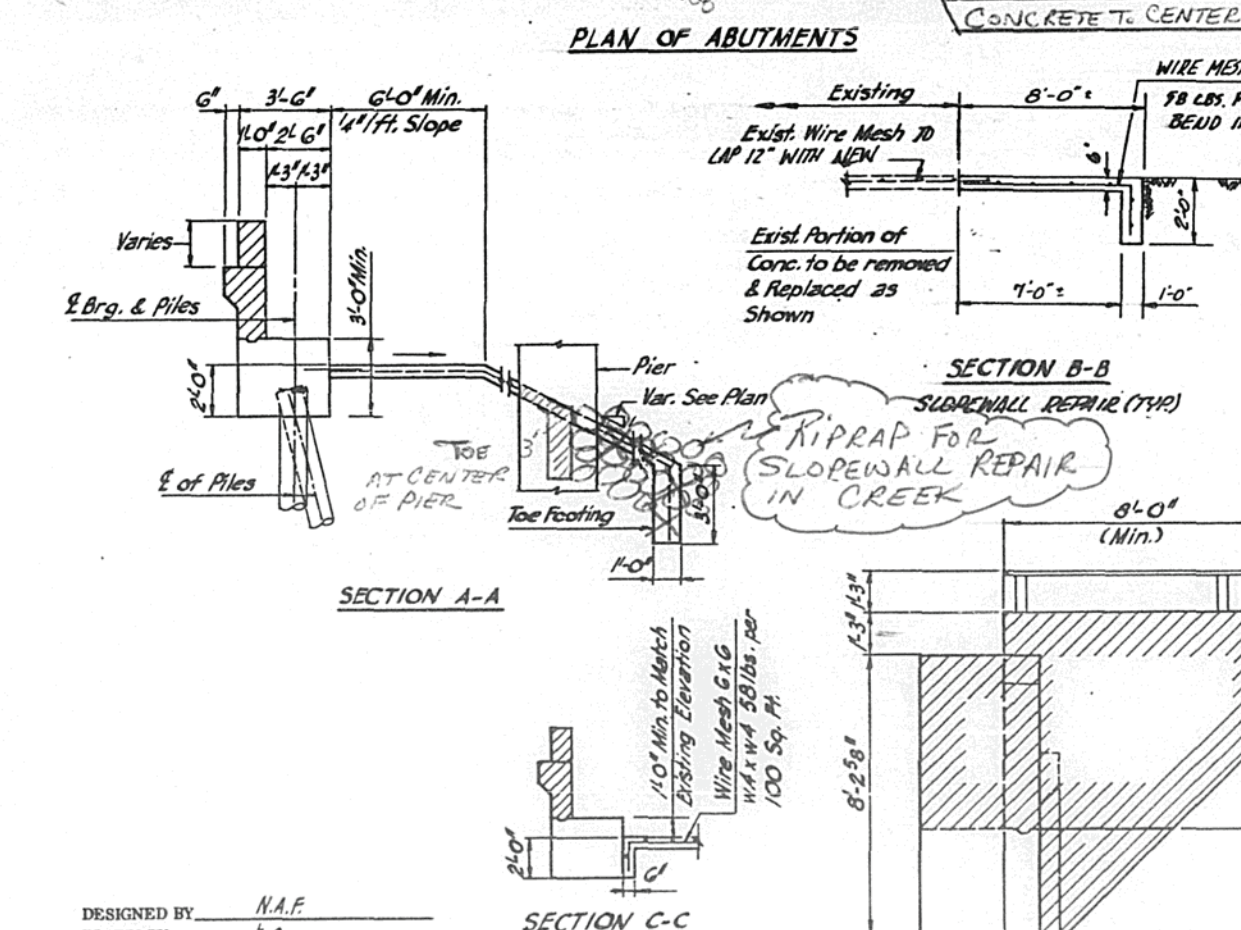
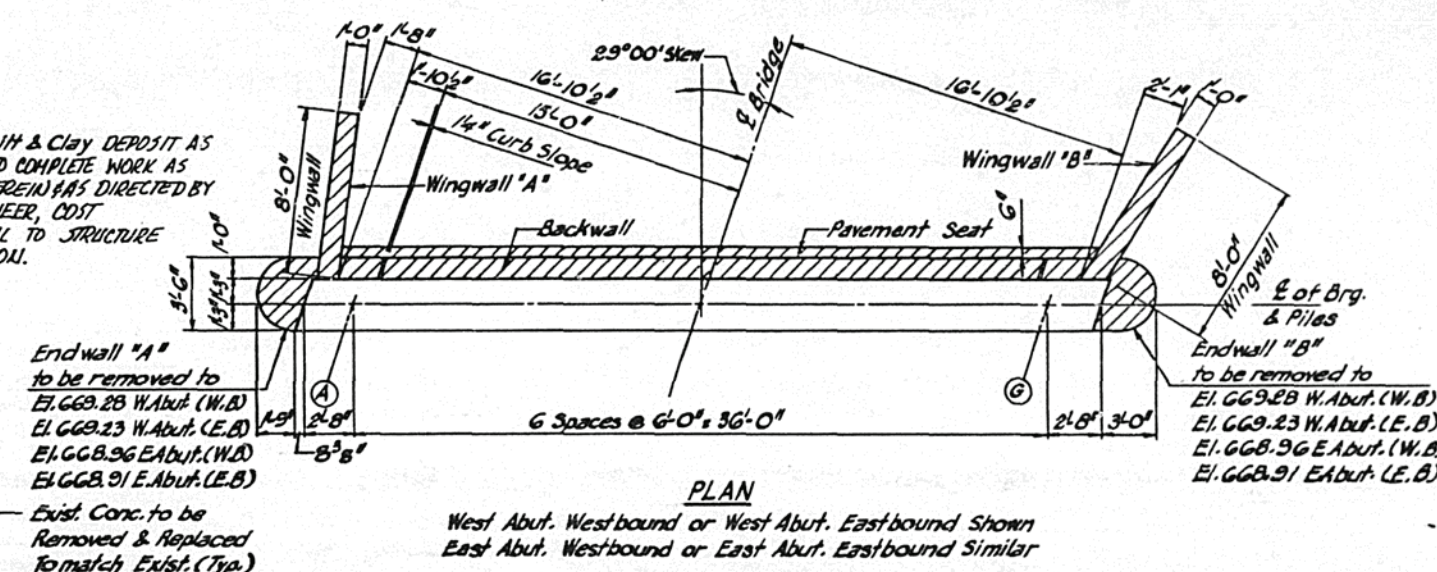
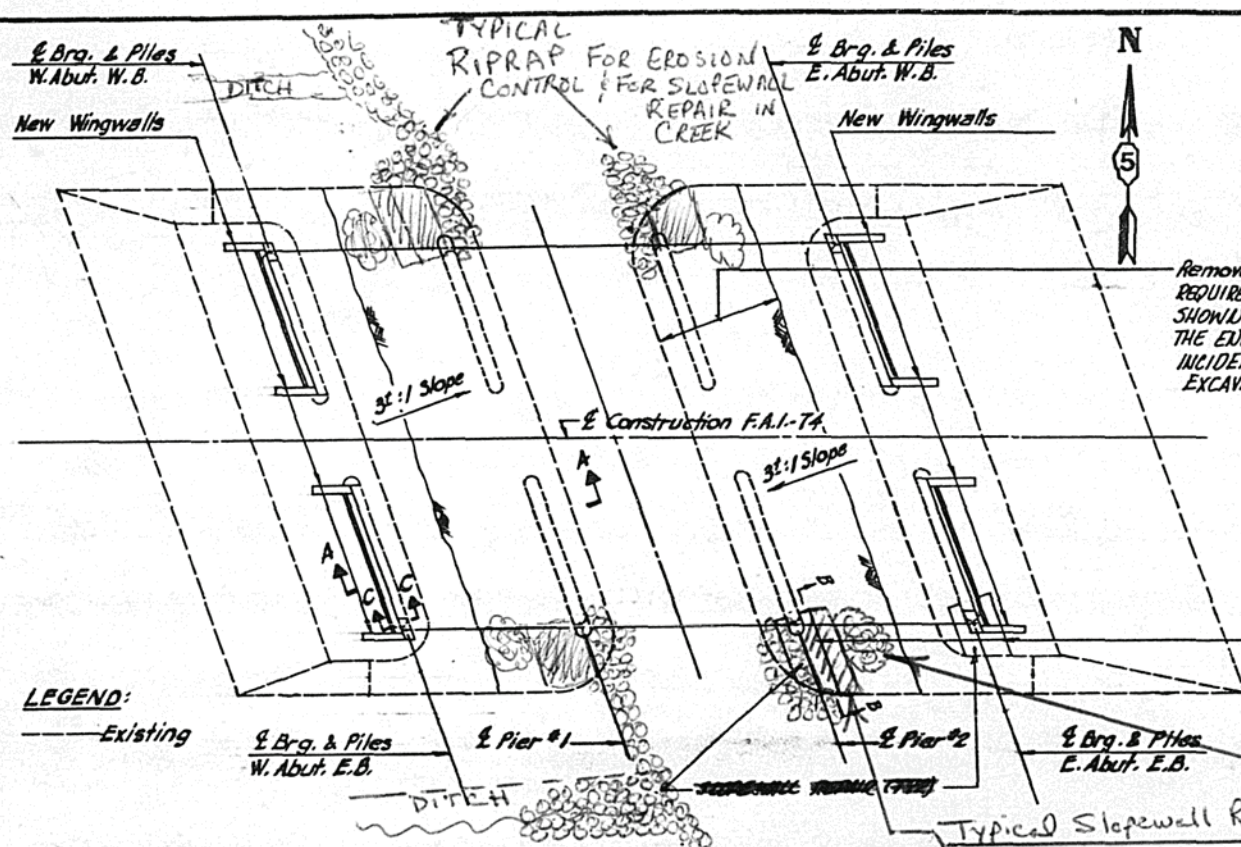
DESIGNED BY: N.A.F.
DRAWN BY: B.R.
CHECKED BY: O.M.D.

AMIR-FAZLI AND ASSOCIATES, INC.
CONSULTING ENGINEERS

DATE

FA. RTE.	SECTION	COUNTY	TOTAL SHEET
FAI 74	(10-7B) BR	CHAMPAIGN	108 .57
PROJECT			

SHEET 14 OF 17



NOTES
 REMOVE EXISTING CONCRETE AS SHOWN. EXISTING VERTICAL REINFORCING BARS TO BE CLEANED AND INCORPORATED INTO NEW CONCRETE. COST INCIDENTAL TO "CLASS X CONCRETE".

REINFORCEMENT BARS DESIGNATED (E) SHALL BE EPOXY COATED.

THE MINIMUM HOLE DIAMETER FOR EPOXY GROUTED VERTICAL OR HORIZONTAL BARS SHALL BE THE DIAMETER OF THE BAR PLUS 1/4" AND THE MINIMUM DEPTH SHALL BE 9". SEE SPECIAL PROVISIONS. FOR HORIZONTAL BARS USE A GROUT APPROVED BY THE DEPARTMENT OR EPOXY GROUT IN ACCORDANCE WITH BSP-11. THE METHOD OF GROUT APPLICATION SHALL BE APPROVED BY THE ENGINEER. SEE SHEETS 12 AND 13 OF 17.

SPACE REINFORCEMENT IN CAP TO MISS ANCHOR BOLTS.

DESIGNED BY N.A.F.
 DRAWN BY S.A.
 CHECKED BY O.M.D.

AMIR-FAZLI AND ASSOCIATES, INC.
 CONSULTING ENGINEERS

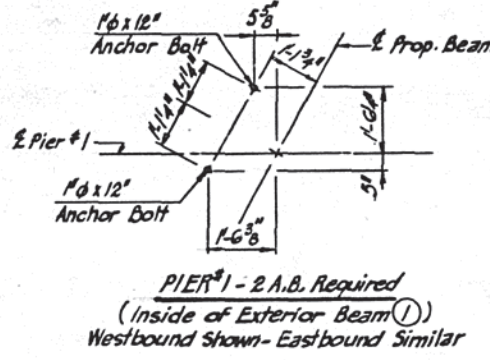
ABUTMENTS
 FAI RTE. 74
 OVER SALINE DRAINAGE DITCH
 SEC. (10-7B) BR
 CHAMPAIGN COUNTY
 STR. NOS. 010-0027.0028
 STA. 690 + 06.00

DATE

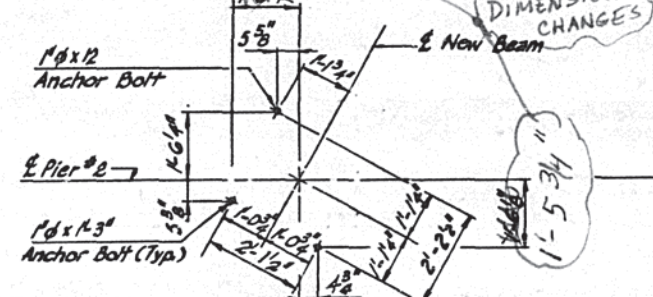
SOME PIER CAP ELEVATIONS WERE CHANGED DURING CONSTRUCTION DUE TO A DESIGNER ERROR IN NOT CALCULATING THE "CAMBER" EFFECT OF BEAMS

FAI RTE	SECTION	COUNTY	TOTAL SHEET
FAI 74	(10-7B) BR	CHAMPAIGN	108
			58

SHEET 15 OF 17



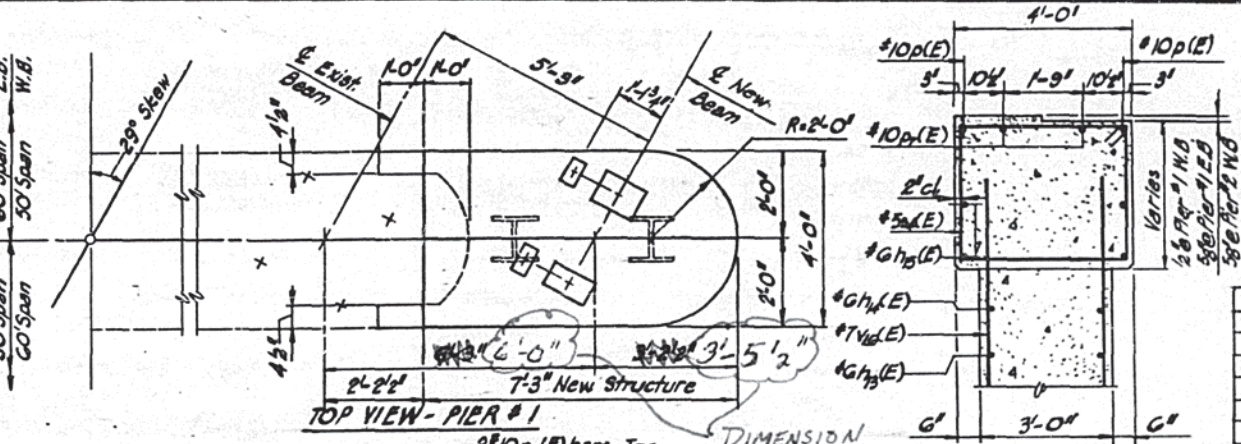
PIER #1 - 2 A.B. Required
(inside of Exterior Beam 1)
Westbound Shown - Eastbound Similar



PIER #2 - 3 A.B. Required
Westbound Shown - Eastbound Similar

DIMENSION CHANGES

1'-5 3/4"



SECTION THRU CAP

4- PIERS
BILL OF MATERIAL

BAR	No.	SIZE	LENGTH	SHAPE
h3(E)	168	#6	2'-10"	
h3(E)	120	#6	12'-6"	
h4(E)	8	#6	3'-9"	
h4(E)	16	#6	6'-3"	
h4(E)	24	#6	7'-9"	
p(E)	8	#10	3'-0"	
p(E)	8	#10	6'-0"	
p(E)	8	#6	7'-11"	
s4(E)	32	#5	13'-2"	
s4(E)	4	#4	12'-9"	
u3(E)	76	#8	7'-11"	
u4(E)	12	#6	9'-0"	
v1(E)	8	#7	14'-7"	
v1(E)	8	#7	14'-9"	
v2(E)	8	#7	14'-11"	
v3(E)	8	#7	17'-1"	
v4(E)	8	#7	17'-6"	
v4(E)	32	#7	20'-0"	

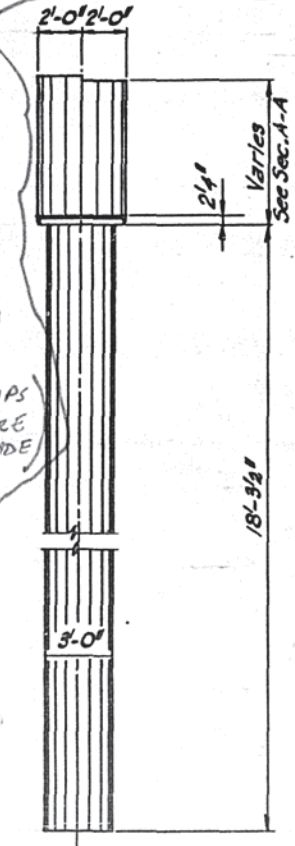
ITEM	UNIT	QUANTITY
Reinforcement Bars (Epoxy Coated)	Lbs.	9,000
Class X Concrete	Cu. Yd.	113
Steel Pile HP 12x53	Ln. Ft.	228
Test Piles	Each	2
Concrete Removal	Cu. Yd.	10

NEW PIER CAP ELEVATIONS FROM BRIDGE OFFICE.

PIER 2 WEST-EB → 668.77
PIER 2 EAST-EB → 668.77

(ALL OTHER PIER CAPS WERE BUILT BEFORE BRIDGE OFFICE MADE CHANGES)

AL GOODFIELD BRIDGE OFFICE, SPRINGFIELD, IL. 12-22-92



END ELEVATION
TEST PILE to 90 T, OTHERS to 75 T.

PILE DATA

TYPE: Steel Pile HP 12x53
CAPACITY: 45 Ton. Design Drive to 60f. Brg.
EST. LENGTH: 38 Ft.
No. REQUIRED: 6 + 2 Test Piles (1 of Pier #1 N.B. & 1 of Pier #2 W.B.)

DESIGNED BY: N.A.F.
DRAWN BY: E.A.
CHECKED BY: O.M.D.

AMIR-FAZLI AND ASSOCIATES, INC.
CONSULTING ENGINEERS

NOTE
EXISTING REINFORCEMENT BARS TO BE CLEANED, STRAIGHTENED AND INCORPORATED INTO NEW CONSTRUCTION. COST INCIDENTAL TO "CLASS X CONCRETE".

ALL EDGES SHALL HAVE STANDARD 3/4" CHAMFER EXCEPT AS NOTED.

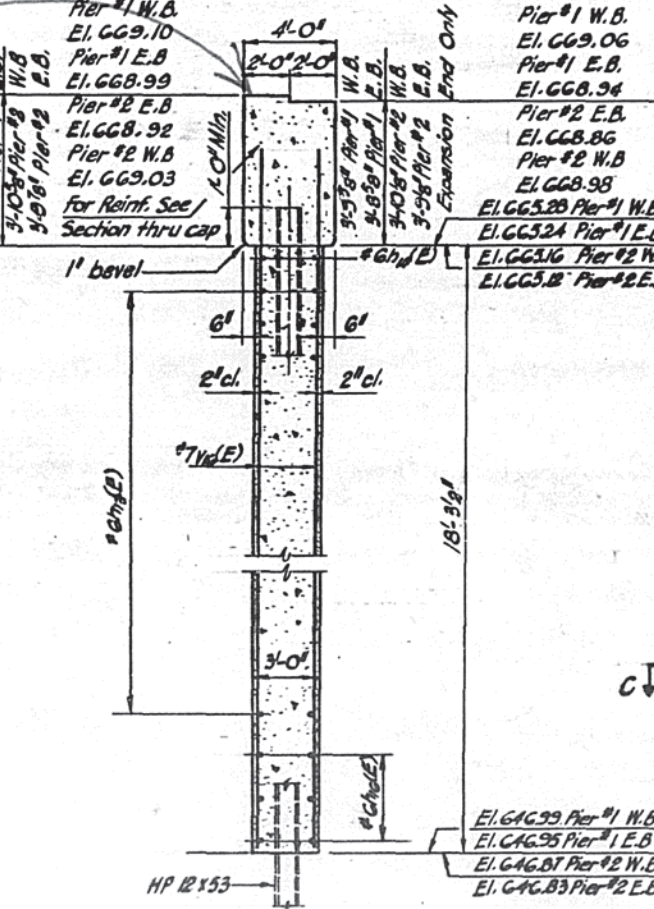
FOUR STEPS MONOLITHICALLY WITH CAP.

SPACE REINFORCEMENT IN CAP TO MISS ANCHOR BOLTS.

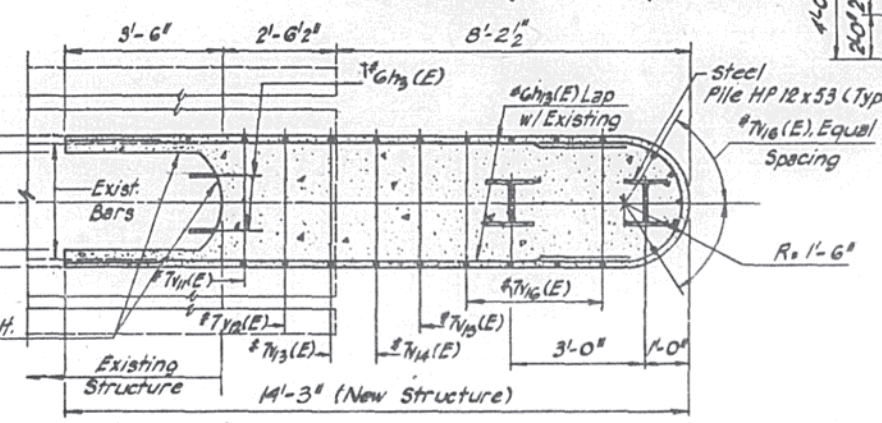
REINFORCEMENT BARS DESIGNATED (E) SHALL BE EPOXY COATED.

BONDED CONSTRUCTION JOINT IN ACCORDANCE WITH ARTICLE 504.13(a)(2) OF THE STANDARD SPECIFICATIONS.

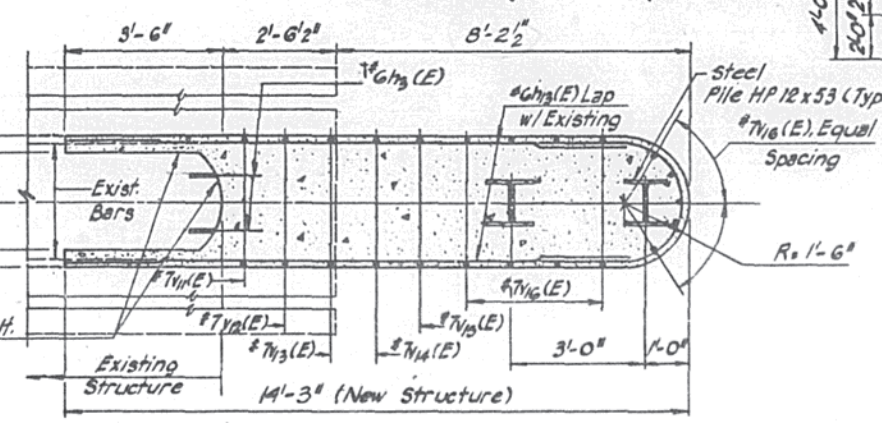
SECTION A-A



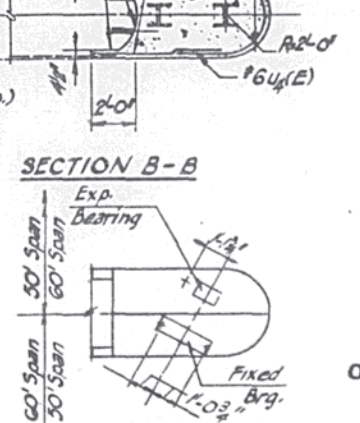
ELEVATION



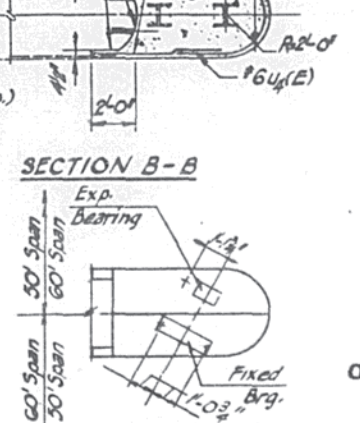
SECTION C-C



SECTION B-B



TOP VIEW - PIER #2



For #Gh3 Epoxy grout (E) bars. See Sheet 14 of 17

PIERS
FAI RTE. 74
OVER SALINE DRAINAGE DITCH
SEC. (10-7B) BR
CHAMPAIGN COUNTY
STR. NOS. 010-0027.0028
STA. 690 + 06.00 DATE