03-04-2016 LETTING ITEM 109

STATE OF ILLINOIS

DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

PROPOSED HIGHWAY PLANS

FAP ROUTE 777 (IL 185) SECTION 10B-2, 405B-1 PROJECT ACF-0777 (018) **BRIDGE REPLACEMENT MONTGOMERY COUNTY** C-96-109-09

FUNCTIONAL CLASSIFICATION: MINOR ARTERIAL (NON-URBAN)

INDEX OF SHEETS

SEE SHEET NO. 2

HIGHWAY STANDARDS

SEE SHEET NO. 2

DESIGN SPEED = 60 MPH POSTED SPEED = 55 MPH

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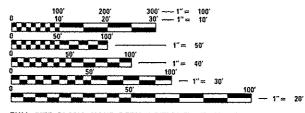
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MCDAVID BRANCH ADT: 3,000 (2013); 2,982 (2032) PV 90.16% SU = 6.17% MU = 3.67%

BAYOU CREEK ADT: 1,850 (2013); 2,183 (2032) PV 89.18% SU = 5.41% MU = 5.41%



ENGINEERING SCALES. REDUCED SIZED PLANS WILL NOT CONFORM TO STANDARD SCALES. IN MAKING MEASUREMENTS ON REDUCED PLANS, THE ABOVE SCALES MAY BE USED.

JOINT UTILITY LOCATION INFORMATION FOR EXCAVATION 1-800-892-0123

TEAM LEADER: GARY TURNER (217) 524-7940 PROJECT ENGINEER: KEITH DONOVAN (217) 782-4761

3RD P.M. **BEGIN PROJECT** STA. 307 + 05**BRIDGE REPLACEMENT** OVER MCDAVID BRANCH STA. 310 + 30.70 EXIST SN 068-0026 PROP SN 068-0512 **END PROJECT** STA. 314+00

LOCATION MAP

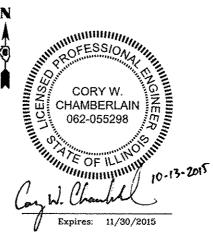
NOT TO SCALE GROSS LENGTH = 2,020 FT. = 0.383 MILE

NET LENGTH = 2,020 FT. = 0.383 MILE

BEGIN PROJECT STA.485 + 60

BRIDGE REPLACEMENT OVER BAYOU CREEK STA. 492 + 70.00 EXIST SN 068-0027 PROP SN 068-0513

> **END PROJECT** STA. 497 + 30



D-96-109-09

108-2, 4058-1

MONTGOMERY 121 1 ILLINOIS CONTRACT NO. 72DO8



STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

PRINTED BY THE AUTHORITY OF THE STATE OF ILLINOIS

CONTRACT NO. 72D08

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- 77 96 BRIDGE PLANS BAYOU CREEK
- 97 105 CROSS SECTIONS MCDAVID BRANCH
- 106 121 CROSS SECTIONS BAYOU CREEK

HIGHWAY STANDARDS

- 000001-06 STANDARD SYMBOLS, ABBREVIATIONS & PATTERNS
- 001001-02 AREAS OF REINFORCEMENT BARS
- 001006 DECIMAL OF AN INCH AND OF A FOOT
- 280001-07 TEMPORARY EROSION CONTROL SYSTEMS
- 406201-01 MAILBOX TURNOUT
- 420401-11 BRIDGE APPROACH PAVEMENT CONNECTOR
- 482001-02 HMA SHOULDER ADJACENT TO FLEXIBLE PAVEMENT
- 515001-03 NAME PLATE FOR BRIDGES
- 542401-01 METAL END SECTION FOR PIPE CULVERTS
- 601101-01 CONCRETE HEADWALL FOR PIPE DRAIN
- 610001-06 SHOULDER INLET WITH CURB
- 630001-10 STEEL PLATE BEAM GUARDRAIL
- 630301-06 SHOULDER WIDENING FOR TYPE 1 (SPECIAL) GUARDRAIL TERMINALS
- 631031-13 TRAFFIC BARRIER TERMINAL, TYPE 6
- 635006-03 REFLECTOR AND TERMINAL MARKER PLACEMENT
- 635011-02 REFLECTOR MARKER AND MOUNTING DETAILS
- 666001-01 RIGHT OF WAY MARKERS
- 701006-05 OFF-ROAD OPERATIONS, 2L, 2W, 15' (4.5 M) TO 24" (600 MM) FROM PAVEMENT EDGE
- 701011-04 OFF-ROAD MOVING OPERATIONS, 2L, 2W, DAY ONLY
- 701201-04 LANE CLOSURE, 2L, 2W, DAY ONLY, FOR SPEEDS > 45 MPH
- 701301-04 LANE CLOSURE, 2L, 2W, SHORT TIME OPERATIONS
- 701306-03 LANE CLOSURE, 2L, 2W, SLOW MOVING OPERATIONS DAY ONLY, FOR SPEEDS >= 45 MPH
- 701311-03 LANE CLOSURE, 2L, 2W MOVING OPERATIONS DAY ONLY
- 701321-14 LANE CLOSURE, 2L, 2W, BRIDGE REPAIR WITH BARRIER
- 701326-04 LANE CLOSURE, 2L, 2W, PAVEMENT WIDENING, FOR SPEEDS >= 45 MPH
- 701901-04 TRAFFIC CONTROL DEVICES
- 704001-07 TEMPORARY CONCRETE BARRIER
- 720001-01 SIGN PANEL MOUNTING DETAILS
- 720006-04 SIGN PANEL ERECTION DETAILS
- 720011-01 METAL POSTS FOR SIGNS, MARKERS & DELINEATORS
- 729001-01 APPLICATIONS OF TYPES A & B METAL POSTS (FOR SIGNS AND MARKERS)
- 780001-05 TYPICAL PAVEMENT MARKINGS
- 781001-03 TYPICAL APPLICATIONS RAISED REFLECTIVE PAVEMENT MARKERS
- 886001-01 DETECTOR LOOP INSTALLATIONS
- 886006-01 TYPICAL LAYOUTS FOR DETECTION LOOPS
- BLR 21-9 TYPICAL APPLICATION OF TRAFFIC CONTROL DEVICES FOR CONSTRUCTION

ON RURAL LOCAL HIGHWAYS

DISTRICT SIX OPERATIONS ENGINEEL EXAMINED October 7 20 15 20 15 EXAMINED October PROGRAM DEVELOPMENT ENGINEER PROJECT IMPLEMENTATION ENGINEER

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STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

INDEX OF OURTE AND INCURSIAN GRANDABING	RIE.	SECTION
INDEX OF SHEETS AND HIGHWAY STANDARDS	777	108-2, 4058-1
SCALE: NTS SHEET NO. 1 OF 1 SHEETS STA. TO STA.	<u> </u>	ILLINOIS

COUNTY TOTAL SHEE

MONTGOMERY 121 2

CONTRACT NO. 72008

 THE CONTRACTOR SHALL PROTECT UTILITY PROPERTY FROM CONSTRUCTION OPERATIONS AS OUTLINED IN ARTICLE 107.31 OF THE STANDARD SPECIFICATION. THE J.U.L.I.E. NUMBER IS 1-800-892-0123.

ILLINOIS STATE LAW REQUIRES A 48-HOUR NOTICE BE GIVEN TO ALL UTILITIES BEFORE DIGGING. FIELD MARKING OF FACILITIES MAY BE OBTAINED BY CONTACTING J.U.L.I.E. OR FOR NON-MEMBERS, BY CONTACTING THE UTILITY COMPANY DIRECTLY.

IT IS UNDERSTOOD AND AGREED THAT THE CONTRACTOR HAS TAKEN THE FOREGOING INTO CONSIDERATION IN SUBMITTING HIS BID, AND NO ADDITIONAL COMPENSATION WILL BE ALLOWED FOR ANY DELAYS OR INCONVENIENCES CAUSED BY THE SAME.

THE INFORMATION AND DATA SHOWN OR INDICATED ON THESE IMPROVEMENT PLANS WITH RESPECT TO EXISTING UNDERGROUND FACILITIES AND UTILITIES AT OR CONTIQUOUS TO THE SITE IS BASED ON INFORMATION AND DATA FURNISHED BY THE OWNERS OF SUCH FACILITIES AND UTILITIES OR BY OTHERS. FIELD MARKINGS OF FACILITIES IN CRITICAL AREAS MAY BE OBTAINED BY PROVIDING A MINIMUM OF 96 HOURS ADVANCE NOTICE TO THE RESIDENT ENGINEER SO THAT UTILITIES CAN BE GIVEN NOTICE. NO GUARANTEE IS IMPLIED AS TO THE ACCURACY OR COMPLETENESS OF ANY SUCH INFORMATION OR DATA; AND CONTRACTOR SHALL HAVE FULL RESPONSIBILITY FOR (I) REVIEWING AND CHECKING ALL SUCH INFORMATION AND DATA, (II) VERIFYING IF ANY CONFLICTS EXIST WITH THE PROPOSED WORKED AND UNDERGROUND FACILITIES AND UTILITIES SHOWN OR INDICATED ON THE IMPROVEMENT PLANS; (III) COORDINATION OF THE WORK WITH THE OWNERS OF SUCH UNDERGROUND FACILITIES AND UTILITIES DURING THE CONSTRUCTION, AND (IV) THE SAFETY AND PROTECTION OF ALL SUCH UNDERGROUND FACILITIES AND UTILITIES AND REPAIR OF ANY DAMAGE THERE TO RESULTING FROM THE WORK AT HIS EXPENSE

- 2. IN ADDITION TO FIELD SURVEYS AND AERIAL SURVEYS, PLAN DIMENSIONS AND DETAILS RELATIVE TO EXISTING FACILITIES HAVE BEEN TAKEN FROM EXISTING PLANS AND ARE SUBJECT TO CONSTRUCTION VARIATIONS. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO VERIEY SUCH DIMENSIONS AND DETAILS IN THE FIELD. SUCH VARIATIONS SHALL NOT BE CAUSE FOR ADDITIONAL COMPENSATION DUE TO A CHANGE IN THE SCOPE OF WORK. HOWEVER, THE CONTRACTOR WILL BE PAID FOR THE QUANTITY ACTUALLY FURNISHED AT THE AGREED UNIT PRICE BID FOR THE WORK.
- 3. ALL STATION AND OFFSET REFERENCES ARE TO THE ROADWAY CENTERLINE FOR IL185 OVER BAYOU CREEK UNLESS OTHERWISE NOTED. ALL STATION AND OFFSET REFERENCES ARE TO THE BASELINE OF CONSTRUCTION FOR IL185 OVER MCDAVID CREEK UNLESS OTHERWISE NOTED. THE STATE PLANE COORDINATE SYSTEM HAS BEEN USED FOR THE HORIZONTAL CONTROL.
- 4. ALL ELEVATIONS SHOWN ON THE PLANS ARE BASED ON NAVD 88.
- ANY REFERENCE WITHIN THESE PLANS TO A STANDARD SHALL BE INTERPRETED TO MEAN THE LATEST STANDARDS OF THE DEPARTMENT AS SHOWN IN THE PLANS.
- THE ENGINEER SHALL BE THE SOLE JUDGE CONCERNING CURING TIME FOR THE VARIOUS BITUMINOUS LIFTS.
- IF SO DIRECTED BY THE ENGINEER, DITCHES ADJACENT TO EMBANKMENTS SHALL BE CONSTRUCTED PRIOR TO STARTING THE CONSTRUCTION OF THE EMBANKMENT FILL.
- 8. GRADING SHALL BE DONE BY HAND AROUND LIGHT POLES, UTILITY POLES, SIGN POSTS, SHRUBS, TREES OR OTHER NATURAL OR MAN-MADE OBJECTS WHERE SHALLOW FILLS OR CUTS ARE ADJACENT TO THE ITEMS. IT IS THE INTENT THAT THE LIMITS OF CONSTRUCTION BE SUCH AS TO PRESERVE IN THE ORIGINAL STATE AS MUCH AREA OF TEMPORARY EASEMENTS AS POSSIBLE. THE DECISION AS TO ITEMS TO REMAIN IN PLACE SHALL BE DIRECTED BY THE ENGINEER. THIS WORK WILL NOT BE PAID FOR SEPARATELY, BUT SHALL BE CONSIDERED INCLUDED IN THE CONTRACT UNIT PRICE PER CUBIC YARD FOR EARTH EXCAVATION, AND NO ADDITIONAL COMPENSATION WILL BE ALLOWED.

GENERAL NOTES

- 9. SEEDING SHALL BE DONE ON ALL AREAS THAT ARE DISTURBED BY CONSTRUCTION OPERATIONS AS DIRECTED BY THE ENGINEER. SEEDING WILL NOT BE PERMITTED AT ANY TIME WHEN THE GROUND IS FROZEN, WET OR IN AN UNTILLABLE CONDITION. SEEDING SHALL BE PAID FOR ONLY WITHIN THE PROPOSED RIGHT-OF-WAY OR EASEMENT LIMITS. ALL AREAS DISTURBED BY THE CONTRACTOR OUTSIDE THE PROPOSED RIGHT-OF-WAY OR EASEMENT LIMITS SHALL BE SEEDED, AS DIRECTED BY THE ENGINEER, AT THE CONTRACTOR'S EXPENSE.
- FACTORS USED FOR ESTIMATING PLAN QUANTITIES OR USAGE ARE AS FOLLOWS AND SHALL NOT BE USED FOR THE BASIS OF FINAL QUANTITIES:

HMA BINDER COURSE	112 LBS/SQ YD-IN
HMA SURFACE COURSE	112 L8S/SQ YD-IN
HMA SHOULDERS	112 LBS/SQ YD-IN
INCIDENTAL HMA SURFACING	112 LBS/SQ YD-IN
BITUMINOUS MATERIALS (RESIDUAL RATE):	
ON PAVEMENT	0.05 LBS/SQ FT
BETWEEN HMA LIFTS (FOG COAT)	0.025 LBS/SQ FT
ON AGGREGATE SURFACE	0.25 LBS/SQ FT
AGGREGATE (PRIME COAT)	3 LBS/SQ YD
AGGREGATE ITEMS EXCEPT RIPRAP	2.05 TONS/CU YD
SEEDING FERTILIZER RATIO (NIT:PHOS:POT)	90:90:90 LB\$/ACRE
AGRICULTURAL GROUND LIMESTONE	2 TONS/ACRE
MHICHRATE	2 TONS/ACRE

- THE QUANTITY OF SHORT TERM PAVEMENT MARKING SHOWN IN THE PLANS IS BASED ON ONE APPLICATION EACH FOR THE BINDER COURSE AND SURFACE COURSE.
- 12. ACCESS TO ALL ENTRANCES SHALL BE MAINTAINED AT ALL TIMES. QUANTITIES FOR AGGREGATE FOR TEMPORARY ACCESS HAVE BEEN ESTIMATED FOR THIS WORK. THESE QUANTITIES SHALL BE USED AS DIRECTED BY THE ENGINEER FOR MAINTAINING ACCESS.
- THE DISTRICT BUREAU OF OPERATIONS SHALL BE CONTACTED AT LEAST 21 DAYS PRIOR TO IMPLEMENTING ANY TRAFFIC CONTROL.
- 4. THE DISTRICT BUREAU OF OPERATIONS SHALL BE CONTACTED AT LEAST 14 DAYS PRIOR TO PLACING ANY FINAL AND TEMPORARY PAVEMENT MARKINGS. THE BUREAU OF OPERATIONS WILL APPROVE THE MARKINGS AND ALSO DETERMINE THE ACTUAL LIMITS TO BE STRIPED AS "NO PASSING ZONES."
- ONLY THOSE TREES WITHIN THE CONSTRUCTION LIMITS OR CLEAR ZONE SHALL BE REMOVED. THE CONTRACTOR SHALL PROTECT ALL REMAINING TREES, PLANTS, AND WETLANDS FROM DAMAGE. ALL TREES AND STUMPS INDICATED ON THE PLANS FOR REMOVAL SHALL BE REMOVED AND DISPOSED OF BY THE CONTRACTOR.
- 16. WHERE PROPOSED CONSTRUCTION ABUTS EXISTING APPURTENANCES, A FULL DEPTH SAW CUT ACCORDING TO SECTION 440 SHALL BE MADE TO ACHIEVE A NEAT JOINT. SAW CUTS WILL NOT BE MEASURED FOR PAYMENT. SAW CUTS FOR REMOVAL ITEMS AND HMA SURFACE REMOVAL SHALL BE CONSIDERED INCLUDED IN THE ITEM BEING REMOVED OR CONSTRUCTED.
- THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS IN THE FIELD PRIOR TO CONSTRUCTION AND ORDERING OF MATERIALS.
- ALL OPENINGS IN PRECAST STRUCTURES, INCLUDING BOX CULVERTS, SHALL BE PRECAST TO THE PROPER SIZE. THIS INCLUDES OPENINGS FOR PIPE STRUCTURES, MANHOLE OPENINGS AND OPENINGS FOR PIPE UNDERDRAINS, COSTS FOR THESE OPENINGS AND THE CONNECTIONS SHALL BE CONSIDERED INCLUDED IN THE VARIOUS PAY ITEMS FOR THE STRUCTURES INVOLVED.
- 19. WHERE SECTION OR SUB-SECTION MARKERS ARE ENCOUNTERED, THE ENGINEER SHALL BE NOTIFIED BEFORE SUCH MONUMENTS ARE REMOVED. THE CONTRACTOR SHALL PROTECT AND CAREFULLY PRESERVE ALL PROPERTY MARKERS AND MONUMENTS UNTIL THE DWNER, AN AUTHORIZED AGENT OR LAND SURVEYOR HAS WITNESSED OR OTHERWISE REFERENCED THEIR LOCATION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR HAVING AN AUTHORIZED SURVEYOR RE-ESTABLISH ANY SECTION OR SUBSECTION MONUMENT DAMAGED BY HIS OPERATIONS.

- EXISTING RAISED REFLECTIVE PAVEMENT MARKERS SHALL BE REMOVED PRIOR TO MILLING HMA SURFACES. RAISED REFLECTIVE PAVEMENT MARKERS REMOVAL WILL NOT BE PAID FOR SEPARATELY BUT SHALL BE CONSIDERED AS INCLUDED WITH THE BITUMINOUS SURFACE REMOVAL.
- 21. THE THICKNESS OF HOT-MIX ASPHALT SHOWN ON THE PLANS IS THE NOMINAL THICKNESS. DEVIATIONS FROM THE NOMINAL THICKNESS WILL BE PERMITTED WHEN SUCH DEVIATIONS OCCUR DUE TO IRREGULARITIES IN THE EXISTING SURFACE OR BASE ON WHICH THE HOT-MIX ASPHALT IS PLACED.
- 22. THE CONTRACTOR WILL BE REQUIRED TO REPAIR AREAS THAT ARE DAMAGED DUE TO THE CONTRACTOR'S OPERATIONS DURING THE EXECUTION OF THIS CONTRACT OR AS DIRECTED BY THE ENGINEER. THIS WORK WILL NOT BE MEASURED FOR PAYMENT AND WILL BE AT THE EXPENSE OF THE CONTRACTOR.
- 23. EXISTING ROAD SIGNS THAT CONFLICT WITH CONSTRUCTION SHALL BE COVERED OR REMOVED AS DIRECTED BY THE ENGINEER. AFTER THE CONSTRUCTION IS COMPLETED, THE CONTRACTOR WILL UNCOVER OR REPLACE THE SIGNS AS DIRECTED BY THE ENGINEER. THIS WORK WILL NOT BE PAID FOR SEPARATELY BUT SHALL BE CONSIDERED INCLUDED IN THE CONTRACT.
- 24. ABANDONED UNDERGROUND UTILITIES THAT CONFLICT WITH CONSTRUCTION SHALL BE DISPOSED OUTSIDE THE LIMITS OF THE RIGHT-OF-WAY ACCORDING TO ARTICLE 202.03 OF THE STANDARD SPECIFICATIONS AND AS DIRECTED BY THE ENGINEER. THIS WORK WILL NOT BE PAID FOR SEPARATELY BUT SHALL BE CONSIDERED INCLUDED IN THE COST OF EARTH EXCAVATION AND NO ADDITIONAL COMPENSATION WILL BE ALLOWED.
- 25. THE REPLACEMENT OF DAMAGED TEMPORARY PAVEMENT MARKING FOR ANY OF THE TRAFFIC CONTROL STANDARDS WILL NOT BE PAID FOR SEPARATELY. THE REPLACEMENT COST SHALL BE CONSIDERED INCLUDED IN THE COST OF THE TRAFFIC CONTROL STANDARD.
- 26. BARRIER WALL AND GUARDRAIL REFLECTORS SHALL BE INSTALLED PRIOR TO SWITCHING STAGE TRAFFIC. VERTICAL PANELS SHOWN ON STANDARD 701321 AND AS SHOWN WILL STILL BE REQUIRED FOR STAGE II TRAFFIC.
- 27. THE CONTRACTOR SHALL TRIM EXISTING SURFACE AND BASE TO A FIRM, NEAR VERTICAL PLANE BEFORE CONSTRUCTING THE HMA WIDENING. COST INCLUDED WITH HMA WIDENING.
- QUANTITIES SHOWN IN THE PLANS FOR BRIDGE DECK GROOVING AND PROTECTIVE COAT INCLUDE THE BRIDGE AND THE BRIDGE APPROACH SLABS.
- EXCAVATING AND CONSTRUCTING THE BENCH CUTS AS SHOWN ON THE DISTRICT 6 STANDARD (SLOPE STEPS DETAIL) WILL NOT BE PAID FOR SEPARATELY BUT SHALL BE CONSIDERED AS INCLUDED WITH THE CONTRACT UNIT PRICE, CUBIC YARD, FOR EARTH EXCAVATION.
- 30. AGGREGATE DITCH CHECKS SHALL BE OPTION 1 AS INDICATED ON SHEET 41 OR AS DIRECTED BY THE ENGINEER. THE AGGREGATE DITCH CHECKS SHALL BE LEVELED OUT AND BECOME PART OF THE RIPRAP DITCH LINING AS DIRECTED BY THE ENGINEER. COST INCLUDED WITH AGGREGATE DITCH CHECKS.
- 31. PAVED DITCH REMOVAL SHALL INCLUDE BOTH CONCRETE OR RIPRAP DITCH UNING.

COMMITMENTS

THE RESIDENT ENGINEER SHALL CONTACT STUDIES AND PLANS CONCERNING ANY MAJOR PLAN CHANGES TO ENSURE NO PREVIOUS COMMITMENTS (NOT LISTED) WERE MADE AFFECTING THE DESIGN AND TO ALLOW IMPROVEMENTS IN THE DESIGN OF FUTURE PROJECTS.

THERE ARE NO COMMITMENTS AT THIS TIME.

UTILITY COMPANIES

AMEREN ILLINOIS 1915 OLD BUS LINE ROAD P.O. BOX 579 HILLSBORO, IL 62049 ATTN: DEBORAH LEWEY

FRONTIER COMMUNICATIONS 225 NORTH BROAD ROAD CARLINVILLE, IL 62626 ATTN: GREG KING

CITY OF COFFEEN 107 LOCUST STREET COFFEEN, IL 62017 ATTN: JOHN HORSTMANN

MJM ELECTRIC 264 NORTH EAST ST. P.O. BOX 80 CARLINVILLE, IL 62626

SCALE: NTS

MIXTURE REQUIREMENTS

MIXTURE USE(S):	SURFACE COURSE	BINDER COURSE	HMA SHOULDER	HMA SHOULDER	INCIDENTAL HMA	BASE COURSE
			(SURFACE LIFT)	(LOWER LIFTS)	SURFACING	WIDENING
PG:	PG 64-22	PG 64-22	PG 64-22	PG 64-22	PG 64-22	PG 54-22
DESIGN AIR VOIDS:	4.0% @ N=50	4.0% @ N=50	4.0% @ N=50	4.0% o N=50	4.0% @ N=50	4.0% @ N=50
MIXTURE COMPOSITION:	IL 9.5	IL 19.0	IL 9.5	IL 19.0	IL 9.5	IL 19.0
FRICTION AGGREGATE:	MIXTURE C	N.A.	MIXTURE C	N.A.	MIXTURE C	N.A
OUALITY MANAGEMENT:	QC/QA	QC/QA	OC/QA	OC/QA	QC/QA	OC/OA

design firm no. 184001036 Whks engineers + planners + land sureson

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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

GENERAL NOTES AND COMMITMENTS		F.A.P. SECTION		TOTAL SHEETS	SHEET NO.
		108-2, 4058-1	MONTCOMERY	121	3
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		***************************************	QO/WINT,	MCDAVID	BAYOU	MCDAVID	BAYOU
		A CDF			0.1		
20100500	TREE REMOVAL, ACRES	ACRE	0.4	0.3	0.1		
20200100	EARTH EXCAVATION .	CU YD	1,532	660	872		
20300100	CHANNEL EXCAVATION	CUYD	1,065	379	686		
20400800	FURNISHED EXCAVATION	CU YD	1,079	0	1079		
20800150	TRENCH BACKFILL	CU YD	22	0	22		
***************************************			PA CONTRACTOR OF THE CONTRACTO				
25000200	SEEDING, CLASS 2	ACRE	2.1	0.6	1.5		
			AAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAA				
25000400	NITROGEN FERTILIZER NUTRIENT	POUND	306	91	215		
23000 400							
25000500	PHOSPHORUS FERTILIZER NUTRIENT	POUND	306	91	215		
	THOSE FOR THE PARTY OF THE PART						<u> </u>
35000500	POTASSIUM FERTILIZER NUTRIENT	POUND	306	91	215		<u> </u>
25000600	POTASSION FERTILIZER NOTATEIVI	10000	300	J.	***		5
		TON	7.0	20			<u> </u>
25000700	AGRICULTURAL GROUND LIMESTONE	TON	7.0	2.0	5.0		
				-			ļ
25003200	INTERSEEDING, CLASS 2	ACRE	1.3	0.4	0.9		<u> </u>
tatuk kanusus kakasak aa kitokka tilak tilak ki		1					
25100115	MULCH, METHOD 2	ACRE	0.7	0.1	0.6		
		11 17 18 18 18 18 18 18 18 18 18 18 18 18 18					<u> </u>
25100630	EROSION CONTROL BLANKET	SQ YD	7,170	2,412	4,758		<u> </u>

28000250	TEMPORARY EROSION CONTROL SEEDING	POUND	615	156	459		



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STATI	E OI	FILLINOIS	
DEPARTMENT	OF	TRANSPORTATION	

				F.A.P. RTÉ.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
SUMMARY OF QUANTITIES			777	108-2, 4058-1	MONTCOMERY	121	4	
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28000315	AGGREGATE DITCH CHECKS	TON	152	64	88		
28000400	PERIMETER EROSION BARRIER	FOOT	2,573	1,120	1,453		
28000500	INLET AND PIPE PROTECTION	EACH	3	0	3		
28100107	STONE RIPRAP, CLASS A4	SQ YD	3,920	1,568	2,352		

28100109	STONE RIPRAP, CLASS AS	SQ YD	1,770			955	815
							\
28200200	FILTER FABRIC	SQ YD	5,690	1,568	2,352	955	815
35100700	AGGREGATE BASE COURSE, TYPE A, 8"	SQ YD	122	0	122		
35650500	BASE COURSE WIDENING 10"	SQ YD	1,428	709	719		
					ANALONA		
40200800	AGGREGATE SURFACE COURSE, TYPE B	TON	188	55	133		
40701000	ACCRECATE FOR TEATROPARY ACCREC				and the second		
40201000	AGGREGATE FOR TEMPORARY ACCESS	TON	34	0	34		, .
40600275	BITUMINOUS MATERIALS (PRIME COAT)	POUND	,,,,,,,	1,342	3,587		
40000273	BITOVERIOUS WATERIALS (FRINGE COAT)	POONU	4,929	11274	51281		
40600990	TEMPORARY RAMP	SQ YD	438	180	258		***************************************
-0000330	The state of the s	3410	430	100	420		
40600625	LEVELING BINDER (MACHINE METHOD), N50	TON	59	18	41		·
	active amount municipally had	TON	1	10	41		
40603080	HMA BINDER COURSE, IL-19.0, NSO	TON	615	72	543		
-000,000		1014	013	/ 2	243		
							



 USER NAME + gjomeson	DESIGNED -	REVISED
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		ditterant or o			F,A.P. RTE.	SECTION	COUNTY	SHEE
i	Í	SUMMARY OF C	UANTITIES	1	777	108-2, 4058-1	MONTCOMERY	121
1							CONTRACT	NO.
_	NTS	SHEET NO. 2 OF 9 SHEETS	STA.	TO STA.		ILLINDIS FED. A	ID PROJECT	

			FEDERAL FUNDS 80% FEDERAL /20% STATE					
······································			1	RDWY		BRI	DGE	
	ITEM	UNIT	TOTAL QUANTITY		04)11	
			QUARTIT	6-00392-0000 MCDAVID	6-00635-0000 BAYOU	6-00392-0000 MCDAVID	6-00635-0000 BAYOU	
			distance of the second of the					
40603310	HMA SURFACE COURSE, MIX "C", N50	TON	395	137	258			
40800050	INCIDENTAL HMA SURFACING	TON	35	10		Processing A Confedence Annual	A CONTRACTOR OF THE CONTRACTOR	
40000030	THE DEVIAE HIVE SOMEACHO	TON	35	10	25			
42001430	BRIDGE APPROACH CONNECTOR PAVEMENT (FLEXIBLE)	SQ YD	166	87	79			
		-			1	VYONE AND THE		
44000100	PAVEMENT REMOVAL	SQ YD	600	276	324			
44000200	DRIVEWAY PAVEMENT REMOVAL	SQ YD	122	0	122			

44004000	PAVED DITCH REMOVAL	FOOT	430	0	430			
				The state of the s				
44004250	PAVED SHOULDER REMOVAL	SQ YD	734	469	265	-		
48101200	AGGREGATE SHOULDERS, TYPE B	TON	386	222	164		-	
					207	· · · · · · · · · · · · · · · · · · ·		
48102100	AGGREGATE WEDGE SHOULDERS, TYPE 8	TON	66	11	55			
48203100	HOT-MIX ASPHALT SHOULDERS	TON	350	122	-25			
46203100	UO1-INIX ASPIRACT SUCCEDENS	TON	359	133	226			
50100300	REMOVAL OF EXISTING STRUCTURE NO. 1	EACH	1			1		
50100400	REMOVAL OF EXISTING STRUCTURE NO. 2	EACH	1		of Workship and Commission of the Commission of		1	
50100500	REMOVAL OF EXISTING STRUCTURES NO. 3	EACH	1	1				
			~		THE CAMPANA			
50100600	REMOVAL OF EXISTING STRUCTURES NO. 4	EACH	1	1	and the second decay of			
****					endoment (end-			



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PLOT DATE + 10/15/2015	CHECKED -	REVISED

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DEPARTMENT	OF	TRANSPORTATION

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CURRENTLY OF CHARLETTE	RIE.	SECTION	COUNTY	SHEETS	NO.
SUMMARY OF QUANTITIES	777	108-2, 4058-1	MONTCOMERY	121	6
			CONTRACT		72008
SHEET NO. 3 OF 9 SHEETS STA. TO STA.		ILLINOIS FEO. A	O PROJECT		

CONSTRUCTION CODE

				 	FEDERA	L FUNDS							
					80% FEDERA								
		900	1	RD'			DGE						
	ITEM	UNIT	LART	\$ 14 IST	THAIL	2 (4 12~	I (AILT	I IAIT	TOTAL	00			011
	I LEIVI		QUANTITY	6-00392-0000	6-00635-0000	6-00392-0000	6-00635-0000						
			1	MCDAVID	BAYOU	MCDAVID	BAYOU						
50105220	PIPE CULVERT REMOVAL	FOOT	211	80	131	-							
50200100	STRUCTURE EXCAVATION	CU YD	501			266	235						
50300100	FLOOR DRAINS	EACH	5		anning and a state of the state		5						
		***************************************			<u></u>								
50300225	CONCRETE STRUCTURES	CU YD	135.0		-	69.0	66.0						
		And the state of t											
50300255	CONCRETE SUPERSTRUCTURE	CU YD	470.3			241.1	229.2						
				4	***************************************								
50300260	BRIDGE DECK GROOVING	SQ YD	984			509	475						
50300300	PROTECTIVE COAT	SQ YD	1,235			631	604						
50500105	FURNISHING AND ERECTING STRUCTURAL STEEL	L SUM	1			0.45	0.55						
		FACU	2.020	1	· · · · · · · · · · · · · · · · · · ·	4 404	4.50						
50500505	STUD SHEAR CONNECTORS	EACH	2,970	***************************************		1,404	1,566						
50800205	REINFORCEMENT BARS, EPOXY COATED	POUND	122,730			63,700	59,030						
50800515	BAR SPLICERS	EACH	1,011			507	504						
51200959	FURNISHING METAL SHELL PILES 14" X 0.312"	FOOT	695	1		695							
			A destruction with the second	TAXABIR TAXABI	NAME OF THE PROPERTY OF THE PR								
51201600	FURNISHING STEEL PILES HP12X53	FOOT	400	toning and the second s			400						
<u></u>			***************************************			···							
51202305	DRIVING PILES	FOOT	1,095		777	695	400						
					***************************************	·							



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PLOT DATE . 10/15/2015	CHECKED -	REVISED

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DEPARTMENT	0F	TRANSPORTATION

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	F.A.P. RTE.	SECTION	COUNTY TOTAL SHEE SHEETS NO.		
SUMMARY OF QUANTITIES	777	108-2, 405B-1	MONTGOMERY	121	7
······································			CONTRACT	NO.	72008
SHEET NO. 4 OF 9 SHEETS STA. TO STA.		ILLINDIS FED. A	TJ3LBSS G		

CONSTRUCTION CODE

						TION CODE	
					PEUERA	L FUNDS L/20% STATE	
			T				5.55
ļ			TOTAL		WY		DGE
ļ	ITEM	UNIT	1		104		011
1			QUANTITY	6-00392-0000	6-00635-0000	6-00392-0000	6-00635-0000
		<u> </u>	*	MCDAVID	BAYOU	MCDAVID	BAYOU
			ana ana	-	V		
			ļ	-	***		
51203200	TEST PILE METAL SHELLS	EACH-	2			2	
			 	 	<u> </u>		
		1		TAX			
		**	1				
51203600	TEST PILE STEEL HP12X53	EACH	2		-		2
			<u> </u>				
,		3113	4		[
51204650	PILE SHOES	EACH	15				13
31204030	PILE SHUES	CACH	18		<u> </u>	6	12
			Action			Promise	
							<u> </u>
51500100	NAME PLATES	EACH	2		**************************************	1	1
		_	<u> </u>				
				negative supplied to the suppl		TI AL ACIONA	1
			 				
52100520	ANCHOR BOLTS, 1"	EACH	48	1		24	24
			 	 			
9		48 *** ***					
54215547	METAL END SECTIONS 12"	EACH	3	1	1		
34213347	INICIAL END SECTIONS 12	EACH	3	2	1		

			ļ	ļ			ļ
54215550	METAL END SECTIONS 15"	EACH	2	0	2		
			<u> </u>	 			
aparent.							
		- 		<u> </u>			<u> </u>
54215559	METAL END SECTIONS 24"	EACH	2	2	0		
				İ			
1		1					
54215565	METAL END SECTIONS 30"	EACH	2	0	2		
3-7243303	Metal Land Section 35		ļ <u>.</u>				
		 	 	<u> </u>			
54215571	METAL END SECTIONS 36"	EACH	2	0	2		-

5.4350330	DIDE OF THE STATE OF THE 4 45%	50.07					
542D0220	PIPE CULVERTS, CLASS D, TYPE 1 15"	FOOT	77	0	77		
			ļ				
542D0229	PIPE CULVERTS, CLASS D, TYPE 1 24"	FOOT	48	48	0		
			ļ				
		-					#
		1	 				
1	PIPE CULVERTS, CLASS D, TYPE 1 30"	FOOT	66	0	66		
542D0235			 	 			
542D0235			1				
542D0235		apres of the factor of the fac					
	DIDE CHILVERTS CLASS D. TVDE 1 36"	- FOOT	EV.		EA		
	PIPE CULVERTS, CLASS D, TYPE 1 36"	FOOT	50	0	50		
	PIPE CULVERTS, CLASS D, TYPE 1 36"	FOOT	50	0	50		

design firm	11
design firm no. 184001035	whks
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	SUMMARY OF QUANTITIES						RTE.	321	
							777	108-2.	
							[
	SHEET	NO. 5 0	F 9	SHEETS	STA.	TO STA.	Γ		

******	RTE.	SECTION	COUNTY	SHEETS	NO.
	777	108-2, 4058-1	MONTCOMERY	121	8
4			CONTRACT	NO. 7	2008
		ILLINOIS FED. A	ID PROJECT		

						FEDERA	CTION CODE AL FUNDS AL /20% STATE	
					RD			IDGE
		ITEM	UNIT	TOTAL QUANTITY	6-00392-0000	04 6-00635-0000		01 1 6-00635-000
			-		MCDAVID	BAYOU	MCDAVID	BAYOU
	50404400							
	59100100	GEOCOMPOSITE WALL DRAIN	SQ YD	142			74	68
	60100945	PIPE DRAINS 12"	FOOT	30	20	10		
-	61000335	TYPE G INLET BOX, STANDARD 610001	EACH	3	2	1	THE PROPERTY OF THE PROPERTY O	
	63000001	STEEL PLATE BEAM GUARDRAIL, TYPE A, 6-FOOT POSTS	FOOT	612.5	337.5	275.0		
_	63100085	TRAFFIC BARRIER TERMINAL, TYPE 6	EACH	8	4	4	POPPER PROPERTY AND ADDRESS OF THE POPPER POPPER PROPERTY AND ADDRESS OF THE POPPER PO	
	62100167	TO ACTIC DADDIED TEDAMINAL THOSE & COCCUMINATIONS						
	63100167	TRAFFIC BARRIER TERMINAL, TYPE 1 (SPECIAL) TANGENT	EACH	7	4	3		
_	63200310	GUARDRAIL REMOVAL	FOOT	1,863	1,020	843		
	66201120	CONCRETE SHOULDER CURB	FOOT	45	30	15		VA ANALYSIS OF THE PROPERTY OF
_					***************************************		The state of the s	
-	66600105	FURNISHING AND ERECTING ROW MARKERS	EACH	8	4	4		-
_	67000400	ENGINEER'S FIELD OFFICE, TYPE A	CAL MO	8	4	4		
	67100100	MOBILIZATION	Ł SUM	1	0.5	0.5		The state of the s
_						***************************************		-
	70100405	TRAFFIC CONTROL AND PROTECTION, STANDARD 701321	EACH	2	4-4	1		
	70100450	TRAFFIC CONTROL AND PROTECTION, STANDARD 701201	. Ł SUM	1	0.5	0.5		
_	70100460	TRAFFIC CONTROL AND PROTECTION, STANDARD 701306	L SUM	1	0.5	0.5		
								:

* SPECIALTY ITEM



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DEPARTMENT	OF	TRANSPORTATION

CHEERLARY OF CHARITYTE			RTE.	SECTION	COUNTY	SHEETS	NO.
SUMMARY OF QUANTITIES				108-2, 4058-1	MONTGOMERY	121	9
					CONTRACT	NO.	72008
SHEET NO. 6 OF 9 SHEETS	STA.	TO STA.	1	ILLINOIS FED. AID PROJECT			

					FEDERA	TION CODE L FUNDS	
					WY		IDGE
· · · · · · · · · · · · · · · · · · ·	ITEM	UNIT	TOTAL QUANTITY	6-00392-0000 MCDAVID	6-00635-0000 BAYOU	6-00392-0000 MCDAVID	6-00635-0000 BAYOU
70100500	TRAFFIC CONTROL AND PROTECTION, STANDARD 701326	L SUM	1	0.5	0.5		
				To the state of th	,		
70101830	TRAFFIC CONTROL AND PROTECTION, STANDARD BLR 21	LSUM	1	0	1		
70103815	TRAFFIC CONTROL SURVEILLANCE	CAL DA	10	5	5		
70106500	TEMPORARY BRIDGE TRAFFIC SIGNALS	EACH	2	1	1		
70106700	TEMPORARY RUMBLE STRIPS	EACH	24	12	12		
70106800	CHANGEABLE MESSAGE SIGN	CALMO	22			-	
	CHATCHACC MESSAGE SIGN	CAL MO	32	16	16		
70300100	SHORT-TERM PAVEMENT MARKING	FOOT	6,920	2,720	4,200		
70300230	TEMPORARY PAVEMENT MARKING - LINE 5"	FOOT	4,994	2,420	2,574		
					We will be a second of the sec		
70300280	TEMPORARY PAVEMENT MARKING - LINE 24"	FOOT	100	48	52	`	
70301000	WORK ZONE PAVEMENT MARKING REMOVAL	\$Q FT	7,908	3,108	4,800		****
					V		
70400100	TEMPORARY CONCRETE BARRIER	FOOT	875	475	400		
70400200	RELOCATE TEMPORARY CONCRETE BARRIER	FÖOT	825	425	400		
				The state of the s	a special financia		
70500100	TEMPORARY STEEL PLATE BEAM GUARDRAIL, TYPE A	FOOT	612.5	337.5	275.0		
70500665	TEMPORARY TRAFFIC BARRIER TERMINAL, TYPE 6	EACH	8	4	4		
····					W WAS A STATE OF THE STATE OF T		

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DEPARTMENT	0F	TRANSPORTATION

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SUMMARY OF QUANTITIES		SECTION	COUNTY	SHEETS	1
		10B-2, 405B-1	MONTCOMERY	121	T
SHEET NO. 7 OF 9 SHEETS STA. TO STA.	 -	ILLINOIS FEO. A	CONTRACT	NO.	72
					

					 		CTION CODE AL FUNDS	
							L /20% STATE	······································
				1	RU	WY		DGE
			****	TOTAL		04 		
		ITEM	UNIT	QUANTITY	6-00392-0000	6-00635-0000	0011 6-00392-0000 6-00635-0000	
				QUANTIT	MCDAVID	BAYOU	MCDAVID	BAYOU

	70600260	IMPACT ATTENUATORS TEMPORARY (FULLY REDIRECTIVE, NARROW) TEST LEVEL 3	EACH	6	2	4		
	70600332	IMPACT ATTENUATORS, RELOCATE (FULLY REDIRECTIVE) TEST LEVEL 3	EACH	5	2	4		
	78001120	PAINT PAVEMENT MARKING - LINE 5"	FOOT	7,626	2,526	5,100		
f	78100100	RAISED REFLECTIVE PAVEMENT MARKERS	EACH	24	9	15		
	78200410	GUARDRAIL MARKERS, TYPE A	EACH	23	11	12		
ĸ	78200530	BARRIER WALL MARKERS, TYPE C	EACH	8	4	4		
	78201000	TERMINAL MARKER - DIRECT APPLIED	EACH	7	4	3	THE COLUMN TWO IS NOT	
	78300100	PAVEMENT MARKING REMOVAL	SQ FT	3,053	1,053	2,000	Annalis and Annali	
	Z0001900	ASBESTOS BEARING PAD REMOVAL	EACH	46			26	20
	Z0004552	APPROACH SLAB REMOVAL	SQ YD	296	149	147	North Reservoires	
	Z0013798	CONSTRUCTION LAYOUT	L SUM	1	0.5	0.5	TOTAL AND THE STATE OF THE STAT	
a wys saa saa kaa kaa kaa kaa ka	20046304	PIPE UNDERDRAINS FOR STRUCTURES 4"	FOOT	334			168	166
,	£0076600	TRAINEES	Hour	1000			500	500
		TEMPORARY SOIL RETENTION SYSTEM	SQ FT	1,073		 	538	535
5	2 0076604	TRAINEES TRAINING PROGRAM GRADUATE	Hour	1000	, , ,		500	500
**************************************	X0322584	REVETMENT MAT REMOVAL	SQ YD	61	61	0	amount of constitution of the	
· ·				-				***************************************

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stratheers + plantara + land surveyors

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	F.A.P. RTE.	SECTION	COUNTY	ď
SUMMARY OF QUANTITIES	777	108-2, 4058-1	MONTCOMERY	
<u> </u>			CONTRACT	
SHEET NO, B OF 9 SHEETS STA. TO STA.		ILLINOIS FED. A	ID PROJECT	

CONSTRUCTION CODE

						FEDERA	TION CODE L FUNDS L /20% STATE		
				TOTAL	RDWY 0004		BRIC		
		ITEM	UNIT	TOTAL QUANTITY	6-00392-0000	6-00635-0000	6-00392-0000	6-00635-0000	
-					MCDAVID	BAYOU	MCDAVID	BAYOU	
	X4401198	HOT-MIX ASPHALT SURFACE REMOVAL, VAR DEPTH	SQ YD	2,431	1,455	976			
	······					-			
	XS420630	PIPE CULVERTS TO BE CLEANED 30"	FOOT	29	29	0			
	X5860110	GRANULAR BACKFILL FOR STRUCTURES	CU YD	251			131	120	

	X6330725	STEEL PLATE BEAM GUARDRAIL (SHORT RADIUS)	FOOT	87.5		87.5			
	X7050167	TEMPORARY TRAFFIC BARRIER TERMINAL, TYPE 1, SPECIAL (TANGENT)	EACH	6	4	2			
-	X7200201	WIDTH RESTRICTION SIGNING	ŁSUM	1	0.5	0.5			
				Western Western Western					
-									
	······································		11			·			
								.	
					And Andrews			<u> </u>	
			THE STATE OF THE S						

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armening and a second						!			
L		100 /. STATE)	Manage de la constant	***************************************	<u> </u>				

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design firm
no. 184001036

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engheers + planners + band surveyors

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DEPARTMENT	OF '	TRANSPORTATION	

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		F.A.P. RTE.	SECTION	COUNTY	TOTAL	SHEET
SUMMARY OF QUANTITIES		777	108-2, 4058-1	MONTCOMERY	121	12
· · · · · · · · · · · · · · · · · · ·				CONTRACT		72008
SHEET NO. 9 OF 9 SHEETS STA.	TO STA.		ILLINDIS FED.	AID PROJECT		

EROSION CONTROL SCHEDULE									
					28000315	28000400	28000500		
					AGGREGATE	PERIMETER	INLET AND		
		· · · · · · ·	201	DITCH	EROSION	PIPE			
	LC	CATIO	N		CHECKS	BARRIER	PROTECTION		
					TON	FOOT	EACH		
MCDAVID BRA	ANCH								
308+25.00	RT				4				
308+75.00	RT				4				
309+50.00	RT				4				
309+80.00	RT				4				
311+00.00	RT				4				
311+75.00	RT				4				
312+25.00	312+25.00 RT		4						
308+25.00	LT				4				
308+65.00	LT				4				
309+00.00	LT				4				
309+50.00	LT				4				
309+75.00	LT				4				
311+00.00	LT				4				
311+50.00	LT				4				
312+00.00	LT				4				
312+35.00	LT				4				
307+06.00	RT	TO	307+50.00	RT		44			
308+23.00	RT	TO	310+08.00	RT		185			
310+40.00	RT	TO	313+14.00	RT		274			
307+06.00	LT	ТО	307+86.00	LT		80			
307+50.00	LT	ТО	310+46.00	LT		296			
310+73.00	LT	ТО	313+04.00	LT		241			
							-		
TOTAL					64	1,120	0		

1										
TRAFFIC CONTROL SCHEDULE										
			70400100	70400200	70600260	70600332				
			TEMPORARY	RELOCATE	IMPACT	IMPACT				
			CONCRETE	TEMPORARY	ATTENUATORS	ATTENUATORS,				
			BARRIER	CONCRETE	TEMPORARY	RELOCATE				
	LOCATION	ON		BARRIER	(FULLY	(FULLY				
	200,				REDIRECTIVE,	REDIRECTIVE)				
					NARROW)	TEST LEVEL 3				
					TEST LEVEL 3					
			FOOT	FOOT	EACH	EACH				
MCDAVID										
STAGE I										
307+63.00	TO	312+37.20	475		2					
STAGE II										
308+12.80	TO	312+37.20		425		2				
TOTAL			475	425	2	2				
BAYOU					·					
STAGE I										
490+80.50	то	495+79.50	400		4					
STAGE II										
490+72.00	ТО	495+77.00		400		4				
430172.00	10	493.11.00		400		+				
TOTAL			400	400	4	4				

							PAVIN	IG SCHED	ULE					
					35650500	40600275	40600990	40600625	40603080	40603310	42001430	48101200	48102100	48203100
					BASE	BITUMINOUS	TEMPORARY	LEVELING	НМА	HMA	BRIDGE	AGGREGATE	AGGREGATE	HOT-MIX
		O A TIC	DNI.		COURSE	MATERIALS	RAMP	BINDER	BINDER	SURFACE	APPROACH	SHOULDERS,	WEDGE	ASPHALT
	LC	CATIC	N		WIDENING	(PRIME		(MACHINE	COURSE,	COURSE,	CONNECTOR	TYPE B	SHOULDERS,	SHOULDERS
					10"	COAT)		METHOD),	IL-19.0, N50	MIX "C", N50	PAVEMENT		TYPE B	
								N50			(FLEXIBLE)			
					SQ YD	POUND	SQ YD	TON	TON	TON	SQ YD	TON	TON	TON
MCDAVID BRA	ANCH					,			•					
307+05.00		TO	307+40.00			63				10.6				4
307+05.00	LT	TO	308+01.00	LT									8	
307+40.00		TO	307+90.00			135		6						
307+40.00		TO	309+57.70							52.8				38
307+90.00		TO	308+25.00			126	100	8						6
308+25.00		TO	309+57.70			478	120		72					46
309+57.70		то	309+63.70								24			
310+97.70		ТО	311+12.70								63			
310+97.70		TO	311+12.70			114	60	5		10.3	US			10
311+12.70		TO	311+75.00			36	- 50			5.3				4
311+75.00		TO	312+00.00			45				7.6				4
312+00.00		TO	312+35.00			63				9.9				·
312+35.00		TO	313+23.00			158								
312+35.00		то	314+00.00							40.0				
312+60.00	RT	TO	312+91.00	RT									3	
313+23.00		TO	314+00.00			123								
312+00.00	RT	TO	312+35.00	RT										6
312+35.00	RT	TO	312+91.00	RT										4
313+23.00	RT	TO	313+70.00	RT										1
313+70.00	RT	TO	314+00.00	RT										1
312+35.00	LT	TO	313+01.00	LT										4
313+33.00	LT	TO	313+47.00	LT										2
313+47.00	LT	ТО	313+70.00	LT										2
313+70.00	LT	TO	314+00.00	LT										1
PRESTAGE														
307+05.00	LT	TO	307+22.70	LT	8									
307+22.70	LT	TO	309+90.00	LT	149									
310+72.00 STAGE I	LT	ТО	312+98.00	LT	126									
307+05.00	RT	ТО	307+37.00	RT	17									
307+05.00	RT	TO	309+65.00	RT	17							52		
307+37.00	RT	TO	309+63.70	RT	177							52		
308+01.00	LT	TO	308+25.00	LT	.,,							2		
308+25.00	LT	TO	309+65.00	LT								28		
		-												
311+11.50	RT	TO	312+36.00	RT								25		
311+12.70	RT	TO	312+86.80	RT	136									
312+36.00	RT	TO		RT								2		
311+11.50	LT	TO	313+06.00	LT								39		
STAGE II														
307+05.00	RT		309+65.00	RT	_							39		
307+22.70	LT		307+37.00	LT	2									
307+37.00	LT	-	309+63.70	LT	51									
308+01.00	LT	TO	308+25.00	LT								2		
308+25.00	LT	то	309+65.00	LT								21		
311+11.50	RT	TO	312+36.00	RT								5		
311+11.50	RT		312+36.00	RT								0.4		
311+11.50	LT		312+06.00	LT								7		
311+12.70	LT	TO	313+02.00	LT	43							,		
5 12.75			5.5 02.00		1 10				I					
TOTAL					709	1342	180	18	72	137	87	222	11	133
					•							•		



USER NAME = gjameson	DESIGNED -	REVISED
FILE NAME = D672D08-SHT-SCHEDULE.	gGHECKED -	REVISED
PLOT SCALE = 10.00000 ' / IN.	DRAWN -	REVISED
PLOT DATE = 10/21/2015	CHECKED -	REVISED

STATE OI	FILLINOIS
DEPARTMENT OF	TRANSPORTATION

	COUPDING OF CHARTITIES	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
	SCHEDULE OF QUANTITIES	777	10B-2, 405B-1	MONTGOMERY	121	13	
				CONTRACT	NO. 7	72D08	
SCALE: NTS	SHEET NO. 1 OF 6 SHEETS STA.	TO STA.		ILLINOIS FED. A	ID PROJECT		

3AYOU CREEF 485+60.00 485+60.00 486+73.00 485+60.00 488+45.50 485+85.00	LC	САПО			35650500	40600275	40600990	40600625	40603080	40603310	42001430	48101200	48102100	48203100
485+60.00 485+60.00 486+73.00 485+60.00 488+45.50 485+85.00	LC	САПО			D 4 0 E									
485+60.00 485+60.00 486+73.00 485+60.00 488+45.50 485+85.00	LC	САТІО			BASE	BITUMINOUS	TEMPORARY	LEVELING	HMA	HMA	BRIDGE	AGGREGATE	AGGREGATE	HOT-MIX
485+60.00 485+60.00 486+73.00 485+60.00 488+45.50 485+85.00	LC	САТІ			COURSE	MATERIALS	RAMP	BINDER	BINDER	SURFACE	APPROACH	SHOULDERS,	WEDGE	ASPHALT
485+60.00 485+60.00 486+73.00 485+60.00 488+45.50 485+85.00			ON		WIDENING	(PRIME		(MACHINE	COURSE,	COURSE,	CONNECTOR	TYPE B	SHOULDERS,	SHOULDER
485+60.00 485+60.00 486+73.00 485+60.00 488+45.50 485+85.00					10"	COAT)		METHOD),	IL-19.0, N50	MIX "C", N50	PAVEMENT		TYPE B	
485+60.00 485+60.00 486+73.00 485+60.00 488+45.50 485+85.00								N50			(FLEXIBLE)			1
485+60.00 485+60.00 486+73.00 485+60.00 488+45.50 485+85.00					SQ YD	POUND	SQ YD	TON	TON	TON	SQ YD	TON	TON	TON
485+60.00 485+60.00 486+73.00 485+60.00 488+45.50 485+85.00	<u>к</u>													
485+60.00 486+73.00 485+60.00 488+45.50 485+85.00		ТО	485+85.00			40				7.0				2
485+60.00 488+45.50 485+85.00	RT	TO	486+33.00	RT									6	
488+45.50 485+85.00	RT	то	488+83.00	RT									16	
485+85.00	LT	то	488+15.50	LT									20	
	LT	TO	490+16.00	LT									13	
		TO	486+10.00			60		3		5.6				4
486+10.00		TO	486+25.00			48		4		3.4				2
486+25.00		TO	491+93.31			2273	172	-	471	127.3				130
491+93.31		TO	491+99.31			1			1		23			
									<u> </u>					
493+42.00		то	493+57.00						 		56			
493+55.69		TO	494+51.80			365	86		72	21.5				
494+51.80		TO	495+60.00			411	"	26		24.2				26
495+60.00		TO	496+30.37			201		9		15.8				12
496+30.37		TO	497+30.00			189				27.9				18
430130.07		10	437 130.00			100				21.5				10
488+83.00	RT	ТО	489+07.00	RT								2		
489+07.00	RT	TO	492+11.00	RT								61		
490+16.00	LT	TO	490+40.00	LT								2		
490+40.00	LT	TO	492+19.00	LT								36		-
430140.00	L1	10	492119.00									30		
493+22.00	RT	ТО	493+86.21	RT								13		
493+86.21	RT	TO	494+06.00	RT								11		
493+31.00	LT	TO	493+66.28	LT								7		
493+66.28	LT	TO	494+07.75	LT								33		
433100.20		10	434107.73	L'								33		
493+55.69	RT	ТО	494+95.00	RT										17
494+95.00	RT	TO	495+51.80	RT										9
493+55.69	LT	TO	493+69.00	LT										3
PRESTAGE		10	493109.00	L'										
485+60.00	LT	то	485+75.00	LT	3									
485+75.00	LT	TO	485+91.00	LT	10				 					
485+91.00	LT	TO	486+11.00	LT	20				 					
486+11.00	LT	TO	486+34.00	LT	20				 					
486+34.00	LT	TO	492+50.00	LT	377				 					
- 00+34.00	L1	10	+3∠+30.00		311	-			 					
493+81.00	LT	ТО	494+03.00	LT	8				 					
494+27.00	LT	TO	494+45.00	LT	8				-					
STAGE I	L1	10	+3++43.00		0				-					
485+60.00	рт	TO	492+00.00	PT	271				 					
493+87.00	RT RT	TO	494+01.00	RT	271				-					
		10	494701.00	KI					-					
10+13.00	<u> </u>	TO	10 50 00	-		-			-	12.2				
10+13.00		TO	10+50.00	_		-			 	13.3				
		TO	10+42.00	-		-			 	F 0				3
10+50.00		TO	10+80.00							5.9				
10+50.00		то	10+80.00			L			I	5.9				
TOTAL					719	3587	258	41	543	258	79	164	55	226

		ERC	SION CO	NTF	ROL SCHE	DULE	
					28000315	28000400	28000500
					AGGREGATE	PERIMETER	INLET AND
	LC	CATIO	NC		DITCH	EROSION	PIPE
					CHECKS	BARRIER	PROTECTION
					TON	FOOT	EACH
BAYOU CREE	K						
486+28.00	RT						1
487+17.00	RT				4		
487+42.00	RT				4		
487+67.00	RT				4		
487+92.00	RT				4		
488+67.00	RT				4		
489+42.00	RT				4		
490+17.00	RT				4		
491+15.00	RT				4		
493+50.00	RT				4		
494+49.00	RT						1
495+00.00	RT				4		
485+80.00	LT				4		
486+00.00	LT				4		
486+20.00	LT				4		
486+40.00	LT				4		
486+70.00	LT				4		
487+00.00	LT				4		
487+30.00	LT				4		
487+60.00	LT				4		
488+05.00	LT						1
488+85.00	LT				4		
489+70.00	LT				4		
490+45.00	LT				4		
491+50.00	LT				4		
486+59.00	RT	ТО	491+91.00	RT		532	
491+91.00	RT	TO	494+05.00	RT		214	
494+20.00	RT	ТО	494+44.00	RT		24	
484+50.00	LT	TO	488+22.00	LT		372	
488+39.00	LT	TO	490+28.00	LT		189	
493+51.00	LT	TO	494+07.00	LT		78	
494+27.00	LT	TO	494+71.00	LT		44	
TOTAL					00	1 452	•
TOTAL					88	1,453	3

ENTRANCE	Ξ, Ν		TURNOUT IEDULE	, AND SIDE	ROAD
		35100700	40200800	40201000	40800050
		AGGREGATE	AGGREGATE	AGGREGATE	INCIDENTAL
		BASE	SURFACE	FOR	HMA
LOCATION		COURSE,	COURSE,	TEMPORARY	SURFACING
		TYPE A, 8"	TYPE B	ACCESS	
		SQ YD	TON	TON	TON
MCDAVID BRANCH					
313+10.00	RT		17		4
313+30.00	LT		38		6
TOTAL		0	55	0	10
BAYOU CREEK					
486+52.00	RT	122			17
488+31.00 LT			55	34	8
494+15.85 RT			78		
TOTAL		122	133	34	25



JSER NAME = gjameson	DESIGNED -	REVISED
TILE NAME = D672D08-SHT-SCHEDULE.	sg6HECKED -	REVISED
PLOT SCALE = 10.0000 '/ IN.	DRAWN -	REVISED
PLOT DATE = 10/21/2015	CHECKED -	REVISED

				F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	SCHEDULE OF QU	777	10B-2, 405B-1	MONTGOMERY	121	14		
				CONTRACT	NO. 7	72D08		
SCALE: NTS	SHEET NO. 2 OF 6 SHEETS	STA.	TO STA.	ILLINOIS FED. AID PROJECT				

						R	EMOVAL	SCHED	ULE				
					44000100	X4401198	44000200	44004000	44004250	Z0004552	X0322584	50100500	50100600
					PAVEMENT	HOT-MIX	DRIVEWAY	PAVED	PAVED			REMOVAL OF	REMOVAL OF
					REMOVAL	ASPHALT	PAVEMENT	DITCH	SHOULDER	SLAB	MAT	EXISTING	EXISTING
						SURFACE	REMOVAL	REMOVAL	REMOVAL	REMOVAL	REMOVAL	STRUCTURES	STRUCTURES
	LO	CATIO	NC			REMOVAL,						NO. 3	NO. 4
						VAR							
						DEPTH							
					20.1/5	00.1/0	00.1/5		00.1/5	20.1/5	00.1/5	E 1 011	E4011
140D 41/1D DE					SQ YD	SQ YD	SQ YD	FOOT	SQ YD	SQ YD	SQ YD	EACH	EACH
MCDAVID BF 307+05.00		TO	310+10.00	DT					102				
307+05.00	RT LT	TO	310+10.00	RT LT					114				
307+05.00	LI	TO	307+90.00	LI		323			114				
307+86.00	RT	TO	308+28.00	RT		323					42		
307+86.00	RT	10	300+20.00	ΠI							42	1	
309+54.00	LT						1					'	1
309+57.70	LI	TO	310+10.00		128								'
309+89.90		TO	310+10.00		120		+			74			
310+51.70		TO	310+72.40							74			
310+52.00		TO	311+12.70		148					/-			
310+52.00	RT	TO	314+00.00	RT	140				121				
310+52.00	LT	TO	314+00.00	LT					133				
311+12.70		TO	314+00.00			1,132			100				
313+10.00	RT	10	314100.00			1,102							
313+30.00	LT												
313+33.00	LT	TO	313+50.00	LT							19		
TOTAL			010100.00		276	1,455	0	0	469	149	61	1	1
						1,100	<u> </u>		1.00	1.0	ļ		•
BAYOU CREE	ΞK												
485+60.00	LT	TO	488+16.00	LT				256					
485+60.00	LT	TO	492+50.00	LT					100				
487+00.00	RT	TO	492+50.00	RT					127				
485+60.00		TO	486+10.00			186							
486+53.00	RT						122						
486+73.00	RT	TO	488+47.00	RT				174					
488+31.00	LT												
491+93.31		TO	492+50.00		151								
492+27.60		TO	492+49.50							73			
492+92.10		TO	493+14.70							73			
492+91.00	RT	TO	493+38.00	RT					16				
492+91.00		TO	493+55.69		173								
492+91.00	LT	TO	493+59.00	LT					23				
494+15.85	RT												
495+60.00		TO	497+30.00			718							
E. 18TH STR	EET												
10+50.00		TO	10+80.00			72							
TOTAL					324	976	122	430	265	147	0	0	0

ROW M	ARKER	SCI	HEDULE								
			66600105								
LOCA	TION		AND								
LOCA	TION		ERECTING								
			ROW								
			EACH								
MCDAVID BRA											
307+88.42	54.11	RT	1								
310+00.00	70.00	RT	1								
311+00.00	70.00	RT	1								
313+03.05	53.21	RT	1								
TOTAL			4								
BAYOU CREEK											
489+00.00	65.00	RT	1								
490+00.00	70.00	RT	1								
491+95.83	491+95.83 70.00 RT										
492+25.00	1										
TOTAL	4										

TREE REMOVAL SC	HEDULE	
LOCATION	OFFSET	20100500 TREE REMOVAL, ACRES
		ACRE
MCDAVID BRANCH		
STA. 307+05.00 TO STA. 310+10.00	RT	0.12
STA. 310+50.00 TO STA. 312+48.00	RT	0.18
TOTALS		0.3
BAYOU CREEK		
STA. 490+25.00 TO STA. 492+75.00	LT	0.08
STA. 491+85.00 TO STA. 492+40.00	RT	0.02
STA. 492+90.00 TO STA. 493+45.00	RT	0.02
TOTALS		0.1

					P	AVEMENT N	/ARKING S	CHEDULE				
					70300100 SHORT- TERM PAVEMENT MARKING	70300230 TEMPORARY PAVEMENT MARKING - LINE 5"	70300280 TEMPORARY PAVEMENT MARKING - LINE 24"	70301000 WORK ZONE PAVEMENT MARKING REMOVAL	PAINT PA	O1120 AVEMENT G - LINE 5"	78100100 RAISED REFLECTIVE PAVEMENT MARKERS	78300100 PAVEMENT MARKING REMOVAL
					FOOT	FOOT	FOOT	SQ FT	FOOT	FOOT	EACH	SQ FT
MCDAVID					1 001	1001	1001	5411	, , ,	1001	27.011	5411
FINAL 306+40.00	RT	то	314+17.00	RT	777			777	777			324
306+40.00	CL	то	314+17.00	CL	1166			1554		972		405
306+40.00	LT	ТО	314+17.00	LT	777			777	777			324
307+05.00	CL	то	314+00.00	CL							9	
STAGEI												
306+50.00	RT	то	307+75.00	LT		125						
307+75.00	LT	TO	312+00.00	LT		425						
312+00.00 312+57.00	CL	TO	312+57.00 313+57.00	CL LT		57 100						
312+31.00	CL	10	313+37.00			100						
307+37.60	LT	то	307+75.00	LT		37						
307+75.00	LT	то	312+00.00	LT		425						
312+00.00	LT	то	312+57.00	CL		57	40					
306+40.00 314+17.00	RT LT						12					
314+17.00							14					
STAGE II												
307+00.00	LT	то	308+00.00	CL		100						
308+00.00	CL	то	308+50.00	RT		50						
308+50.00 312+00.00	RT	TO TO	312+00.00 314+07.00	RT LT		350 207						
312+00.00	KI	10	314+07.00			207						
307+85.00	RT	то	308+50.00	RT		65						
308+50.00	RT	то	312+00.00	RT		350						
312+00.00	RT	то	312+71.50	RT		72						
306+40.00 314+17.00	RT LT						12					
314+17.00	LI						12					
TOTAL					2720	2420	48	3108	1554	972	9	1,053
BAYOU												
FINAL												
485+60.00	RT	то	497+60.00	RT	1200			1200	1200			500
485+60.00	CL	TO	497+60.00	CL	1800			2400	1000	2700		1,000
485+60.00	LT	то	497+60.00	LT	1200			1200	1200			500
485+60.00	CL	то	497+30.00	CL							15	
STAGEI												
489+06.00	RT	то	491+18.00	LT		212						
491+18.00	LT	TO	495+43.50	LT		426						
495+43.50	LT	то	496+00.00	CL		57						
496+00.00	CL	TO	497+00.00	LT		100						
490+86.00 491+18.00	LT	TO	491+18.00 495+43.50	LT		32 426					-	
491+18.00	LT	TO	495+43.50	CL		38					1	
488+96.00	RT	· •		† <u> </u>			13					
497+60.00	LT						13					
STAGE II												
489+56.00	LT	то	490+56.00	CL		100						
490+56.00	CL	TO	491+10.00	RT		54						
491+10.00 495+40.00	RT	TO	495+40.00	RT		430 210						
495+40.00	RT	TO	497+50.00 491+10.00	RT		30						
491+10.00	RT	TO	495+40.00	RT		430						
495+40.00	RT	ТО	495+70.00	RT		30						
488+96.00	RT						13					
497+60.00	LT						13					
TOTAL		<u> </u>			4200	2574	52	4800	2400	2700	15	2,000



ı	USER NAME = gjames	on DESIGNED	_	REVISED
	FILE NAME = D672D0	8-SHT-SCHEDULE.	-	REVISED
	PLOT SCALE = 10.0000	'/ IN. DRAWN	-	REVISED
	PLOT DATE = 10/15/2	Ø15 CHECKED	-	REVISED

		F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.		
	SCHEDULE OF QUA	ANTITIES		777	10B-2, 405B-1	MONTGOMERY	121	15
						CONTRACT	NO.	72D08
SCALE: NTS	SHEET NO. 3 OF 6 SHEETS	STA.	TO STA.		ILLINOIS FED. A	D PROJECT		

in firm 34001036	whks	_
	engineers + planners + land surveyors	

USER NAME = gjameson	DESIGNED -	REVISED
FILE NAME = D672D08-SHT-SCHEDULE.	g6HECKED -	REVISED
PLOT SCALE = 10.00000 ' / IN.	DRAWN -	REVISED
PLOT DATE = 10/15/2015	CHECKED -	REVISED

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

			F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
ı		SCHEDULE OF QUANTITIES		777	10B-2, 405B-1	MONTGOMERY	121	16
						CONTRACT	NO.	72D08
	SCALE: NTS	SHEET NO. 4 OF 6 SHEETS STA.		ILLINOIS FED. A	D PROJECT			

	485+60.	00 TC	497+30.0	00			215	215	2	15		5			
	485+60.	00 RT TC	497+30.0	00 RT										278	1
	485+60.	00 LT TC	497+30.0	00 LT										181	1
					'		•		'	'		'	'		1
	TOTAL	_			1.5	0.9	215	215	2	15	0.6	5	4,758	459	
					•				·				•		
							DRAINAC	GE SCHED	ULE						
		20800150	50105220	54215547	54215550	54215559	54215565	54215571	60100945	542D0220	542D0229	542D0235	542D0241	61000335	X5420630
		TRENCH	PIPE	METAL	METAL	METAL	METAL	METAL	PIPE	PIPE	PIPE	PIPE	PIPE	TYPE G	PIPE
LOCAT	ION	BACKFILL	CULVERT	END	END	END	END	END	DRAINS	CULVERTS	CULVERTS	CULVERTS	CULVERTS	INLET BOX,	CULVERTS
LOCAT	ION	BACKI ILL	REMOVAL	SECTIONS	SECTIONS	SECTIONS	SECTIONS	SECTIONS	12"	, CLASS D	, CLASS D,	, CLASS D,	, CLASS D,	STANDARD	TO BE
				12"	15"	24"	30"	36"			TYPE 1 24"	TYPE 1 30"	TYPE 1 36"	610001	CLEANED
		CU YD	FOOT	EACH	EACH	EACH	EACH	EACH	FOOT	FOOT	FOOT	FOOT	FOOT	EACH	FOOT
MCDAVID BI	RANCH														
309+89.00	RT		14												
310+72.00	LT		13												
310+74.00	LT		20												
311+05.00	RT			1					10					1	
311+05.00	LT			1					10					1	
312+91.00	RT					1									
313+10.00	RT														29
313+16.00	LT		33								48				
313+39.00	LT					1									
TOTAL			80	2	0	2	0	0	20	0	48	0	0	2	29
BAYOU CREE															
486+28.00	RT		45				1								
486+45.00 486+53.00	RT	20	45												
486+53.00 486+92.00	LT	22					1					66			
486+92.00 488+05.00	RT LT						1	1							
488+31.00	LT		34					<u>'</u>					50		
488+56.00	LT		34					1					30		
493+54.00	LT			1				 '	10					1	
493+71.00	RT			1	1				10						
494+14.00	RT		52		Į.					77					
494+48.00	RT		UZ		1					11					
.5 11 15.00	1				'										
TOTAL		22	131	1	2	0	2	2	10	77	0	66	50	1	0

SEEDING SCHEDULE

NUTRIENT

POUND

91

91

25000400 25000500 25000600 25100115

FERTILIZER FERTILIZER METHOD 2

NUTRIENT

POUND

91

91

SEEDING, INTERSEEDING, NITROGEN PHOSPHORUS POTASSIUM MULCH, AGRICULTURAL EROSION TEMPORARY

NUTRIENT

POUND

91

91

ACRE

0.1

0.1

0.4

25000700

GROUND

LIMESTONE

TON

25100630 28000250

EROSION

CONTROL SEEDING

POUND

88

68

156

CONTROL

BLANKET

SQ YD

713

521

701 477

2,412

1,922

1,705

942

189

25000200

CLASS 2

ACRE

0.6

0.53

LOCATION

307+05.00 RT TO 310+15.00 RT 0.15

307+05.00 LT TO 310+45.00 LT 0.13

310+40.00 RT TO 313+04.00 RT 0.15 310+85.00 LT TO 313+06.00 LT 0.10

485+60.00 RT TO 492+36.00 RT 0.62

492+82.00 RT TO 497+30.00 RT 0.31

493+11.00 LT TO 497+30.00 LT 0.07

TO 490+50.00 TO 497+30.00

307+05.00 LT TO 308+35.00 LT

307+05.00 RT TO 314+00.00 RT

307+05.00 LT TO 314+00.00 LT

485+60.00 LT TO 492+91.00 LT

307+05.00 TO 314+00.00

MCDAVID BRANCH

BAYOU CREEK

485+60.00

25003200

CLASS 2

ACRE

0.10

0.11

0.12

0.08

0.4

0.10

0.19

0.10

0.49

							GUADO	DAII 66115						
								RAIL SCHE						
					63000001	70500100	63200310	63100167	X7050167	63100085	70500665	78200410	78200530	78201000
					STEEL PLATE		GUARDRAIL	TRAFFIC	TEMPORARY	TRAFFIC	TEMPORARY	GUARDRAIL	BARRIER	TERMINAL
					BEAM	STEEL PLATE	REMOVAL	BARRIER	TRAFFIC	BARRIER	TRAFFIC	MARKERS,	WALL	MARKER -
					GUARDRAIL,	BEAM		TERMINAL,	BARRIER	TERMINAL,	BARRIER	TYPE A	MARKERS,	DIRECT
	LC	CATIC	N		TYPE A, 6-	GUARDRAIL,		TYPE 1	TERMINAL,	TYPE 6	TERMINAL,		TYPE C	APPLIED
					FOOT POSTS	TYPE A		(SPECIAL)	TYPE 1,		TYPE 6			
								TANGENT	SPECIAL					
									(TANGENT)					
					FOOT	FOOT	FOOT	EACH	EACH	EACH	EACH	EACH	EACH	EACH
MCDAVID BRA	ANCH													
307+60.55	RT	ТО	308+10.55	RT				1	1					1
307+60.55	RT	ТО	312+25.85	RT								6		
307+69.70	RT	ТО	309+89.70	RT			220.0							
308+10.55	RT	то	309+35.55	RT	125.0	125.0								
309+35.55	RT	ТО	309+81.20	RT						1	1			
309+77.70	RT	то	310+83.70	RT									2	
310+72.00	RT	то	312+80.00	RT			208.0							
310+80.20	RT	то	311+25.85	RT						1	1			
311+25.85	RT	то	311+75.85	RT	50.0	50.0								
311+75.85	RT	ТО	312+25.85	RT				1	1					1
307+19.00	LT	то	309+89.70	LT			271.0							
308+35.55	LT	то	312+88.35	LT								5		
308+35.55	LT	то	308+85.55	LT				1	1					1
308+85.55	LT	то	309+35.55	LT	50.0	50.0								
309+35.55	LT	то	309+81.20	LT						1	1			
309+77.70	LT	ТО	310+83.70	LT									2	
		T = 0 1	0.10.00.00				201.0							
310+72.00	LT	ТО	313+93.00	LT			321.0							
310+80.20	LT	ТО	311+25.85	LT						1	1			
311+25.85	LT	ТО	312+38.35	LT	112.5	112.5								
312+38.35	LT	то	312+88.35	LT				1	1					1
TOTAL					227 5	227 5	4 000 0	4				44		
IOIAL					337.5	337.5	1,020.0	4	4	4	4	11	4	4

RIPRAP SO	HEDUL	E	
LOCATION	OFFSET	28100107 STONE RIPRAP,	28200200 FILTER FABRIC
200,111011		CLASS A4	SQ YD
MCDAVID BRANCH		54.5	54.5
STA. 307+50 TO STA. 310+00	RT	408	408
STA. 310+40 TO STA. 312+95	RT	416	416
STA. 307+85 TO STA. 310+00	LT	350	350
STA. 311+00 TO STA. 312+90	LT	310	310
STA. 311+05	RT	46	46
STA. 311+05	LT	39	39
TOTAL		1,568	1,568
BAYOU CREEK			
STA. 485+60 TO STA. 488+10	LT	408	408
STA. 486+82 TO STA. 492+30	RT	893	893
STA. 488+44 TO STA. 492+30	LT	629	629
STA. 493+00 TO STA. 493+75	RT	122	122
STA. 493+54	LT	40	40
STA. 494+41 TO STA. 496+00	RT	259	259
TOTAL		2,352	2,352

SEE BRIDGE PLANS FOR DETAIL OF STONE RIPRAP, CLASS A5.



USER NAME = gjameson	DESIGNED	-	REVISED
FILE NAME = D672D08-SHT-SCHEDULE.	ag6HECKED	-	REVISED
PLOT SCALE = 10.00000 ' / IN.	DRAWN	-	REVISED
PLOT DATE = 10/15/2015	CHECKED	-	REVISED

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
SCHEDULE OF QUANTITIES	777	10B-2, 405B-1	MONTGOMERY	121	17
			CONTRACT	NO. 7	2D08
CALE: NTS SHEET NO. 5 OF 6 SHEETS STA. TO STA.		ILLINOIS FED. A	ID PROJECT		

	EARTHV	VORK SCHE	DULE		
LOCATION	20200100 EARTH EXCAVATION	20300100 CHANNEL EXCAVATION	EARTH EXCAVATION ADJUSTED FOR SHRINKAGE	EMBANKMENT (FILL)	20400800 EARTHWORK BALANCE WASTE(+) AND SHORTAGE (-) FURNISHED EXCAVATION
	(CU YD)	(CU YD)	(CU YD)	(CU YD)	(CU YD)
MCDAVID BRANCH	(== /	(== /	(,	, , ,	(== /
PRESTAGE					
STA. 307+50.00 TO STA. 309+93.70	32		24	0	24
STA. 310+67.70 TO STA. 314+00.00	16		12	0	12
STAGE I					
STA. 307+50.00 TO STA. 309+93.70	207		156	31	125
STA. 310+67.70 TO STA. 314+00.00	145		109	160	-51
STAGE II					
STA. 307+50.00 TO STA. 309+93.70	103		78	91	-13
STA. 310+67.70 TO STA. 314+00.00	157		118	129	-11
STAGE III					
STA. 307+50.00 TO STA. 309+93.70	0		0	10	-10
STA. 310+67.70 TO STA. 314+00.00	0		0	0	0
CHANNEL EXCAVATION					
STA. 309+93.70 TO STA. 310+67.70		379			
TOTAL	660	379	497	421	76
BAYOU CREEK					
PRESTAGE					
STA. 485+60.00 TO STA. 492+28.27	27		21	4	17
STA. 493+11.73 TO STA. 497+30.00	0		0	0	0
31A. 493+11.73 TO 31A. 497+30.00			0	0	0
STAGE I					
STA. 485+60.00 TO STA. 492+28.27	323		243	506	-263
STA. 493+11.73 TO STA. 497+30.00	86		65	292	-227
STAGE II					
STA. 485+60.00 TO STA. 492+28.27	303		228	493	-265
STA. 493+11.73 TO STA. 497+30.00	99		75	74	1
STAGE III					
STA. 485+60.00 TO STA. 492+28.27	11		9	417	-408
STA. 493+11.73 TO STA. 497+30.00	23		18	28	-10
CHANNEL EXCAVATION					
STA. 492+28.27 TO STA. 493+11.73		686			
STA. 482+20.21 TO STA. 483+11.13		000			
TOTAL	872	686	659	1,814	-1,155
					· ·

SHRINKAGE FACTOR FOR EARTH EXCAVATION = 25%
CHANNEL EXCAVATION ASSUMED AS UNSUITABLE MATERIAL FOR EARTHWORK SCHEDULE AND QUANTITIES

							GI	JARDRAIL	SCHEDULE						
	LC	OCATIO	NO		6300001 STEEL PLATE BEAM GUARDRAIL, TYPE A, 6- FOOT POSTS	70500100 TEMPORARY STEEL PLATE BEAM GUARDRAIL,	X6330725 STEEL PLATE BEAM GUARDRAIL (SHORT RADIUS)	63200310 GUARDRAIL REMOVAL	63100167 TRAFFIC BARRIER TERMINAL, TYPE 1 (SPECIAL) TANGENT	X7050167 TEMPORARY TRAFFIC BARRIER TERMINAL, TYPE 1, SPECIAL (TANGENT)	63100045 TRAFFIC BARRIER TERMINAL, TYPE 6	70500665 TEMPORARY TRAFFIC BARRIER TERMINAL, TYPE 6	78200410 GUARDRAIL MARKERS, TYPE A	78200530 BARRIER WALL MARKERS, TYPE C	78201000 TERMINA MARKER DIRECT APPLIED
					FOOT	FOOT	FOOT	FOOT	EACH	EACH	EACH	EACH	EACH	EACH	EACH
BAYOU CREEK	(
489+16.87	RT	ТО	489+66.87	RT					1	1					1
489+16.87	RT		494+04.21	RT									6		
489+41.00	RT	то	492+27.70	RT				287.0							
489+66.87	RT	то	491+66.87	RT	200.0	200.0									
491+66.87	RT	то	492+12.52	RT							1	1			
492+10.00	RT	то	493+22.00	RT										2	
493+14.70	RT	ТО	493+88.00	RT				74.0							
493+18.91	RT	TO	493+64.56	RT			50.0				1	1			
493+64.56	RT	то	494+04.21	RT			50.0	44.0							
494+14.21	RI							41.0							
490+48.70	LT	то	492+27.70	LT				179.0		-					
490+46.70	LT	TO	492+27.70	LT				179.0	1	1					1
490+50.44	LT	10	491+00.44	LT					'	 ' 			6		<u> </u>
491+00.44	LT	то	491+75.44	LT	75.0	75.0							U		
491+75.44	LT	10	492+21.09	LT	73.0	75.0					1	1			
491173.44		10	492121.09								'	'			
492+18.00	LT	ТО	493+30.50	LT						+				2	
.52 .5.50	·														
493+14.70	LT	то	493+78.00	LT				64.0							
493+27.48	LT	ТО	493+73.09	LT							1	1			
493+73.09	LT	ТО	493+99.68	LT			37.5								
493+99.68	LT	то	494+02.92	LT					1						1
494+01.59	LT	то	494+01.85	LT											
494+15.85	LT							56.0							
494+40.00	LT	то	495+82.00	LT				142.0							
TOTAL					275.0	275.0	87.5	843.0	3	2	4	4	12	4	3



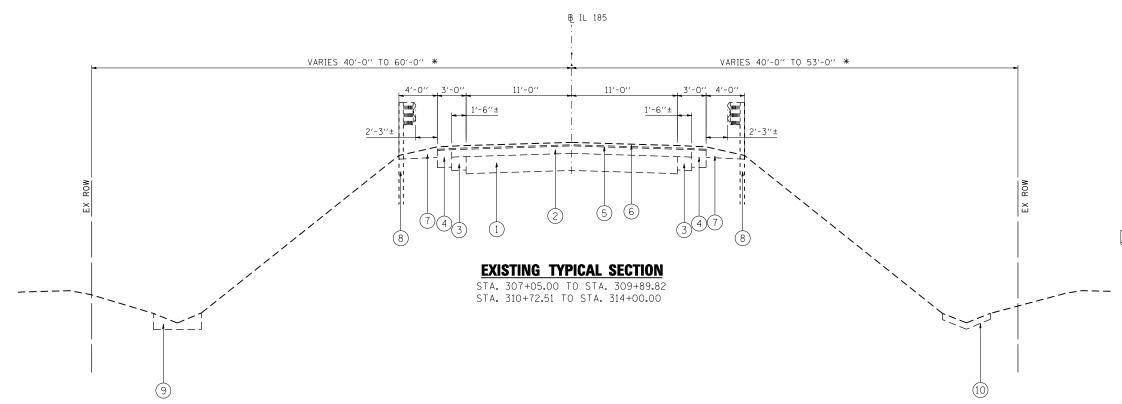
USER NAME = gjameson	DESIGNED	-	REVISED
FILE NAME = D672D08-SHT-SCHEDULE.	ag6HECKED	-	REVISED
PLOT SCALE = 10.0000 '/ IN.	DRAWN	-	REVISED
PLOT DATE = 10/15/2015	CHECKED	-	REVISED

STATE	OF ILLINOIS
DEPARTMENT (OF TRANSPORTATION

	SCHEDULE OF QUANTITIES			SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
				10B-2, 405B-1	MONTGOMERY	121	18
					CONTRACT	NO.	72D08
	SCALE: NTS	SHEET NO. 6 OF 6 SHEETS STA. TO STA.		ILLINOIS FED. A	D PROJECT		
_		· · · · · · · · · · · · · · · · · · ·	•				

LEGEND

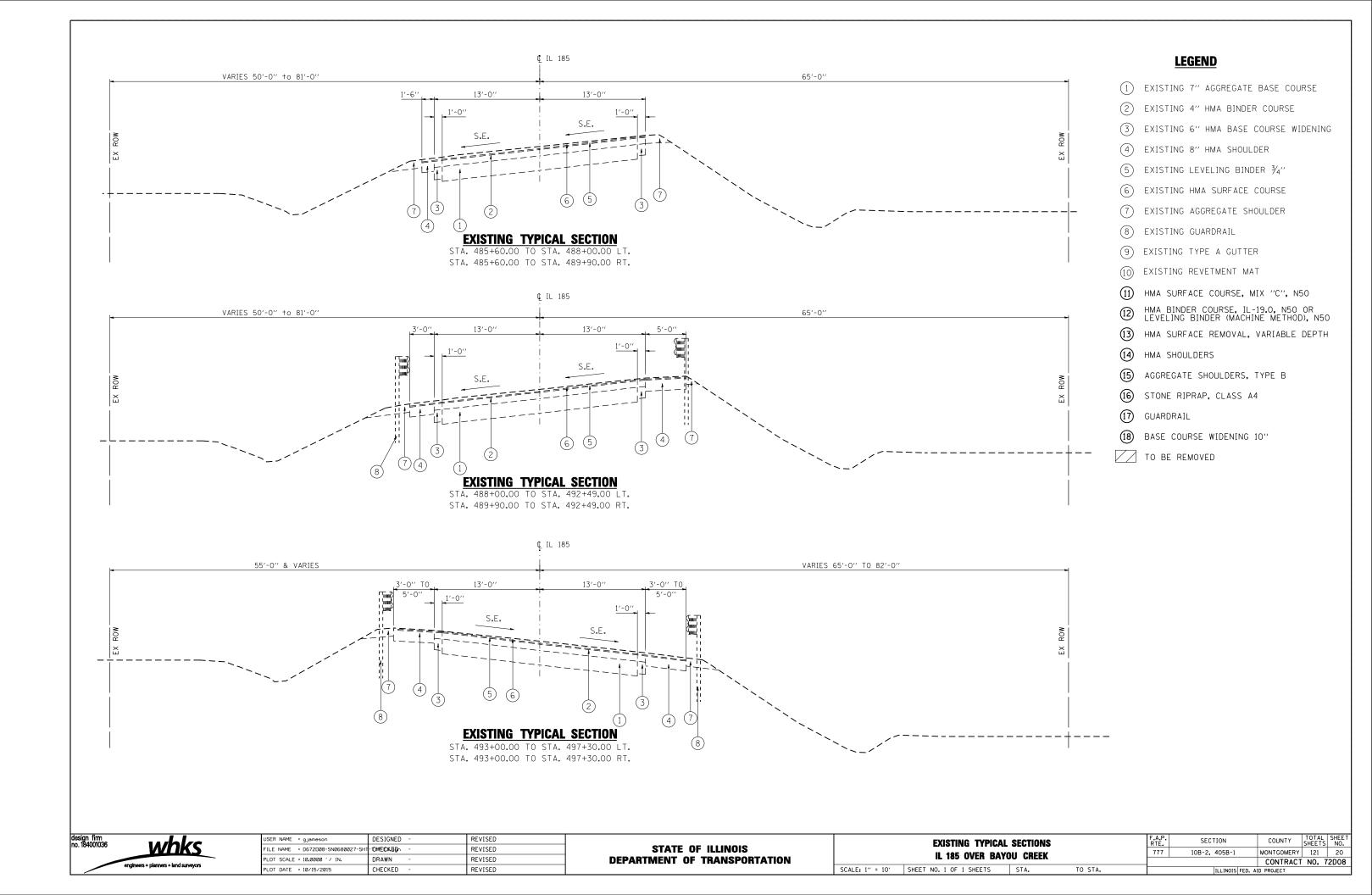
- 1) EXISTING 7" AGGREGATE BASE COURSE
- (2) EXISTING 4" HMA BINDER COURSE
- 3 EXISTING 6" HMA BASE COURSE WIDENING
- (4) EXISTING 8" HMA SHOULDER
- (5) EXISTING LEVELING BINDER 3/4"
- (6) EXISTING HMA SURFACE COURSE
- (7) EXISTING AGGREGATE SHOULDER
- 8 EXISTING GUARDRAIL
- 9 EXISTING TYPE A GUTTER
- (10) EXISTING REVETMENT MAT
- (1) HMA SURFACE COURSE, MIX "C", N50
- (2) HMA BINDER COURSE, IL-19.0, N50 OR LEVELING BINDER (MACHINE METHOD), N50
- (13) HMA SURFACE REMOVAL, VARIABLE DEPTH
- 14) HMA SHOULDERS
- (15) AGGREGATE SHOULDERS, TYPE B
- (16) STONE RIPRAP, CLASS A4
- (17) GUARDRAIL
- (18) BASE COURSE WIDENING 10"
- TO BE REMOVED

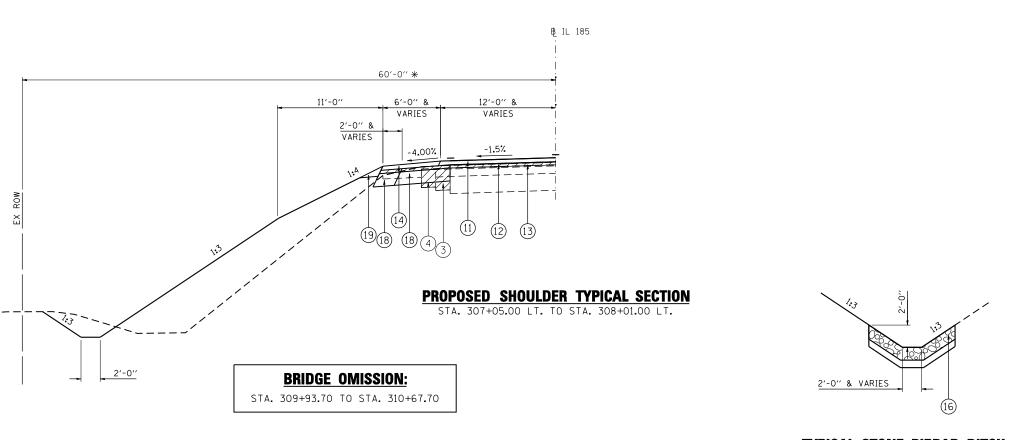


* EXISTING AND PROPOSED ROW IS MEASURED TO BASELINE OF IL 185.

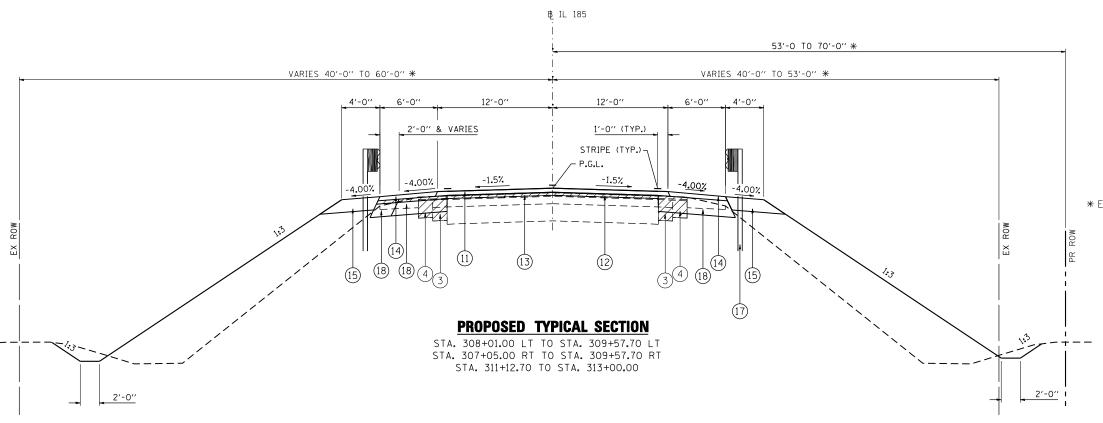


USER NAME = gjameson	DESIGNED -	REVISED
FILE NAME = D672D08-SN0680026-SHT	-TOMPEICKEBBn -	REVISED
PLOT SCALE = 10.0000 '/ IN.	DRAWN -	REVISED
PLOT DATE = 10/15/2015	CHECKED -	REVISED





TYPICAL STONE RIPRAP DITCH



LEGEND

- 1) EXISTING 7" AGGREGATE BASE COURSE
- 2) EXISTING 4" HMA BINDER COURSE
- (3) EXISTING 6" HMA BASE COURSE WIDENING
- 4 EXISTING 8" HMA SHOULDER
- (5) EXISTING LEVELING BINDER 3/4"
- (6) EXISTING HMA SURFACE COURSE
- (7) EXISTING AGGREGATE SHOULDER
- 8 EXISTING GUARDRAIL
- 9 EXISTING TYPE A GUTTER
- (10) EXISTING REVETMENT MAT
- 11) HMA SURFACE COURSE, MIX "C", N50
- 12 HMA BINDER COURSE, IL-19.0, N50 OR LEVELING BINDER (MACHINE METHOD), N50
- (13) HMA SURFACE REMOVAL, VARIABLE DEPTH
- (14) HMA SHOULDERS
- 15) AGGREGATE SHOULDERS, TYPE B
- (16) STONE RIPRAP, CLASS A4
- (17) GUARDRAIL
- 18) BASE COURSE WIDENING 10"
- (19) AGGREGATE WEDGE SHOULDERS, TYPE B

TO BE REMOVED

* EXISTING AND PROPOSED ROW IS MEASURED TO BASELINE OF IL 185.

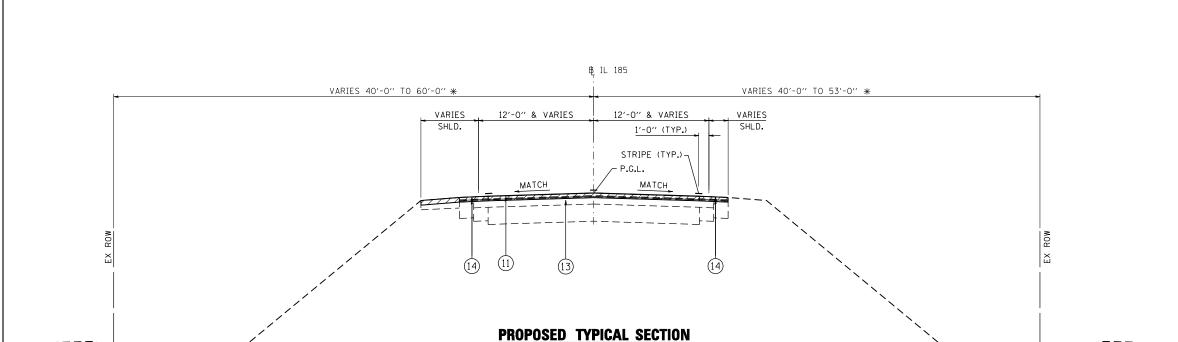
whks

USER NAME = gjameson	DESIGNED -	REVISED
FILE NAME = D672D08-SN0680026-SHT	- CHREIOK.B.D	REVISED
PLOT SCALE = 10.0000 ' / IN.	DRAWN -	REVISED
PLOT DATE = 10/15/2015	CHECKED -	REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

PROPOSED TYPICAL SECTIONS				
	IL 185 OVER MCDAV	ID BRANCH		
SCALE: 1" = 10'	SHEET NO. 1 OF 2 SHEETS	STA.	TO STA.	

		THE INOIS FED. AT	ID PROJECT		
_			CONTRACT	NO. 7	'2D0
	777	10B-2, 405B-1	MONTGOMERY	121	21
	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHE



STA. 313+00.00 TO STA. 314+00.00

LEGEND

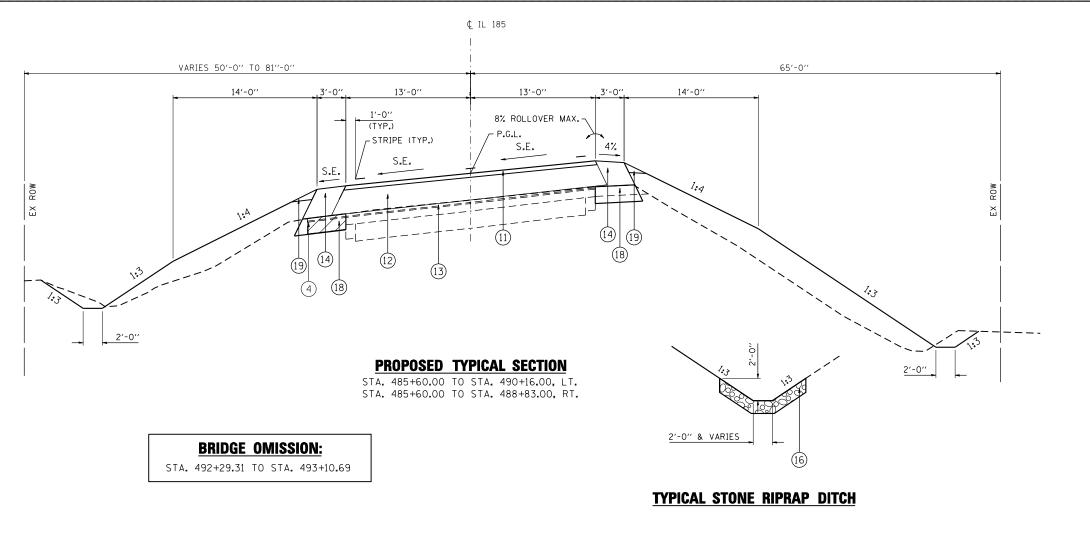
- 1) EXISTING 7" AGGREGATE BASE COURSE
- (2) EXISTING 4" HMA BINDER COURSE
- (3) EXISTING 6" HMA BASE COURSE WIDENING
- 4 EXISTING 8" HMA SHOULDER
- (5) EXISTING LEVELING BINDER 3/4"
- (6) EXISTING HMA SURFACE COURSE
- (7) EXISTING AGGREGATE SHOULDER
- 8 EXISTING GUARDRAIL
- (9) EXISTING TYPE A GUTTER
- (10) EXISTING REVETMENT MAT
- 11) HMA SURFACE COURSE, MIX "C", N50
- 12 HMA BINDER COURSE, IL-19.0, N50 OR LEVELING BINDER (MACHINE METHOD), N50
- (13) HMA SURFACE REMOVAL, VARIABLE DEPTH
- 14) HMA SHOULDERS
- 15) AGGREGATE SHOULDERS, TYPE B
- (16) STONE RIPRAP, CLASS A4
- (17) GUARDRAIL
- 18) BASE COURSE WIDENING 10"
- 19 AGGREGATE WEDGE SHOULDERS, TYPE B

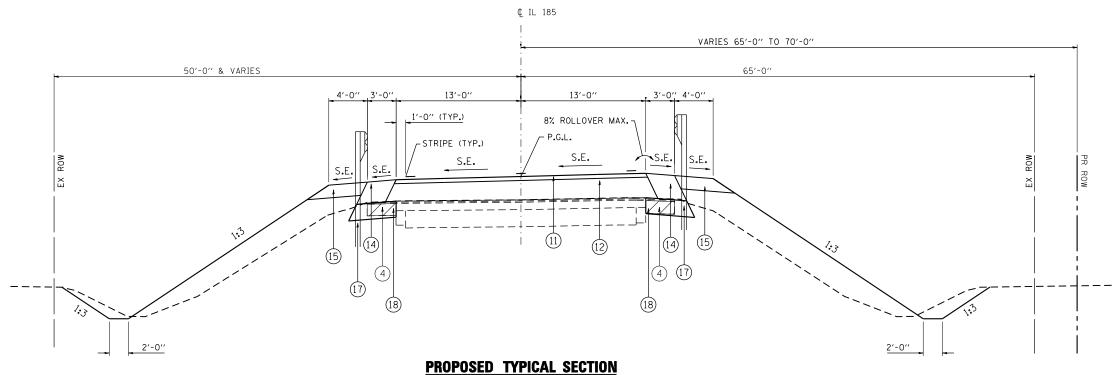
TO BE REMOVED

* EXISTING AND PROPOSED ROW IS MEASURED TO BASELINE OF IL 185.



ISER NAME = gjameson	DESIGNED -	REVISED
ILE NAME = D672D08-SN0680026-SHT	-10/HE101K.ELGh -	REVISED
LOT SCALE = 10.0000 '/ IN.	DRAWN -	REVISED
LOT DATE = 10/15/2015	CHECKED -	REVISED





LEGEND

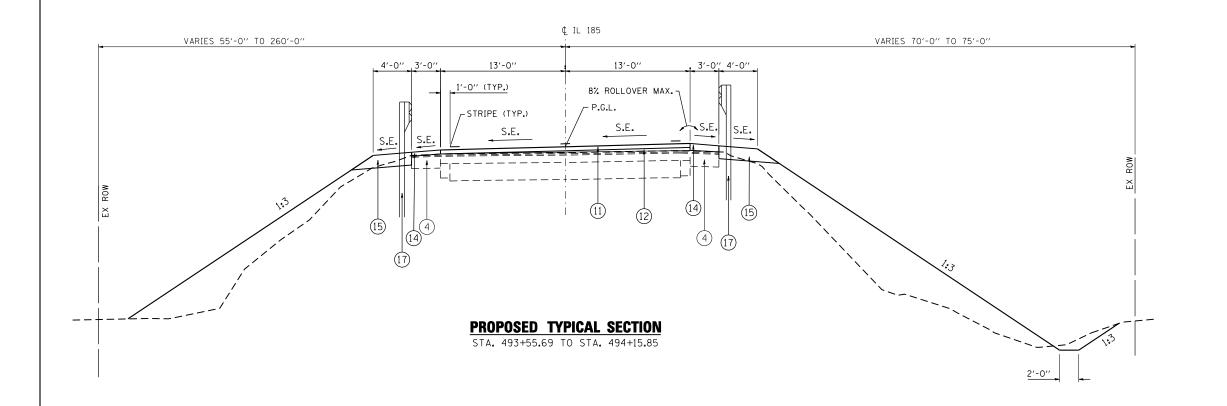
- 1 EXISTING 7" AGGREGATE BASE COURSE
- (2) EXISTING 4" HMA BINDER COURSE
- (3) EXISTING 6" HMA BASE COURSE WIDENING
- 4 EXISTING 8" HMA SHOULDER
- 5 EXISTING LEVELING BINDER 3/4"
- (6) EXISTING HMA SURFACE COURSE
- (7) EXISTING AGGREGATE SHOULDER
- 8 EXISTING GUARDRAIL
- 9 EXISTING TYPE A GUTTER
- (10) EXISTING REVETMENT MAT
- 11) HMA SURFACE COURSE, MIX "C", N50
- (12) HMA BINDER COURSE, IL-19.0, N50 OR LEVELING BINDER (MACHINE METHOD), N50
- (13) HMA SURFACE REMOVAL, VARIABLE DEPTH
- (14) HMA SHOULDERS
- (15) AGGREGATE SHOULDERS, TYPE B
- (16) STONE RIPRAP, CLASS A4
- (17) GUARDRAIL
- 18 BASE COURSE WIDENING 10"
- (19) AGGREGATE WEDGE SHOULDERS, TYPE B
- TO BE REMOVED

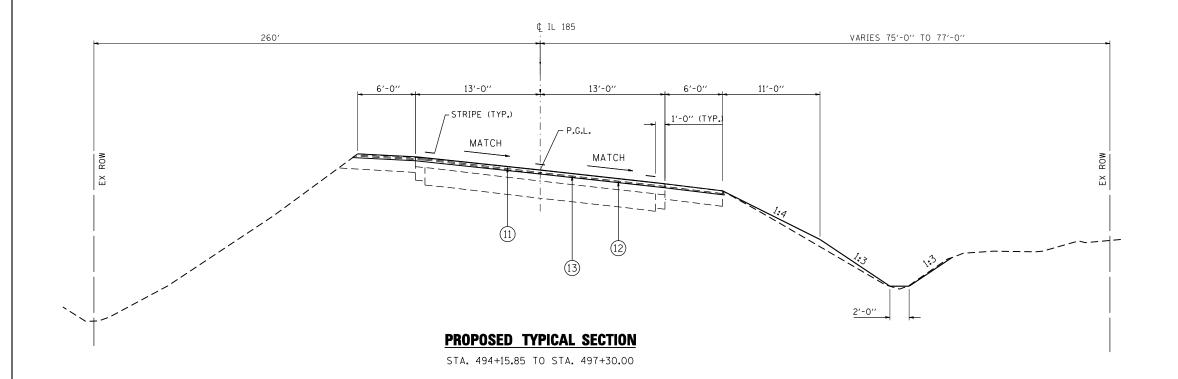
STA. 490+40.00 TO STA. 491+93.31, LT. STA. 489+07.00 TO STA. 491+93.31, RT.

whks
engineers + planners + land surveyors

JSER NAME = gjameson	DESIGNED -	REVISED
FILE NAME = D672D08-SN0680027-SHT	- TOMPEROKEBON -	REVISED
PLOT SCALE = 10.0000 '/ IN.	DRAWN -	REVISED
PLOT DATE = 10/15/2015	CHECKED -	REVISED

PROPOSED TYPICA	AL SECTION	IS	RTE.	
IL 185 OVER BAY	OU CREEK	(777	1
SCALE: 1" = 10' SHEET NO. 1 OF 2 SHEETS	STA.	TO STA.		





LEGEND

- (1) EXISTING 7" AGGREGATE BASE COURSE
- 2) EXISTING 4" HMA BINDER COURSE
- (3) EXISTING 6" HMA BASE COURSE WIDENING
- 4 EXISTING 8" HMA SHOULDER
- (5) EXISTING LEVELING BINDER 3/4"
- (6) EXISTING HMA SURFACE COURSE
- 7 EXISTING AGGREGATE SHOULDER
- 8 EXISTING GUARDRAIL
- 9 EXISTING TYPE A GUTTER
- (10) EXISTING REVETMENT MAT
- 11) HMA SURFACE COURSE, MIX "C", N50
- 12 HMA BINDER COURSE, IL-19.0, N50 OR LEVELING BINDER (MACHINE METHOD), N50
- (13) HMA SURFACE REMOVAL, VARIABLE DEPTH
- 14 HMA SHOULDERS
- (15) AGGREGATE SHOULDERS, TYPE B
- (16) STONE RIPRAP, CLASS A4
- (17) GUARDRAIL
- 18 BASE COURSE WIDENING 10"
- (19) AGGREGATE WEDGE SHOULDERS, TYPE B

TO BE REMOVED

777

whks

USER NAME = gjameson	DESIGNED -	REVISED
FILE NAME = D672D08-SN0680027-SHT	- TOHFEOKELIGH -	REVISED
PLOT SCALE = 10.0000 '/ IN.	DRAWN -	REVISED
PLOT DATE = 10/15/2015	CHECKED -	REVISED

	PROPOSED TYPICAL	. SECTIONS	
	IL 185 OVER BAYO	U CREEK	
SCALE: 1" = 10'	SHEET NO. 2 OF 2 SHEETS	STA.	TO STA.

BENCHMARK BM-71

STA. 303+10. 57' LT. COTTON PICKER SPINDLE IN SOUTH SIDE OF POWER POLE ELEV. 630.43 (NAVD 88 DATUM)

BENCHMARK BM-72

STA. 313+17. 60' LT. COTTON PICKER SPINDLE IN SOUTH SIDE OF POWER POLE ELEV. 601.39 (NAVD 88 DATUM)

BENCHMARK TJM100

STA. 310+52.11, 16.5' LT. CHISELED "□" IN CONCRETE APPROACH SLAB AT N.E. CORNER OF BRIDGE SN 068-0026 ELEV. 604.18 (NAVD 88 DATUM)

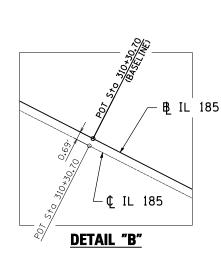
ALIGNMENT COORDINATES CENTERLINE IL 185			
POINT	STA.	N	E
POINT PT C11	304+90.36	888, 102, 614	2,509,376.310
POINT POT 20009	310+30.70	887,857.580	2,509,857.896
POINT PC C12	320+52.14	887, 394. 378	2,510,768.268

ALIGNMENT COORDINATES BASELINE IL 185				
POINT STA. N E			E	
РΤ	POT BASE001	305+00.00	888,099.678	2,509,385.634
РΤ	POT BASE002	310+30.70	887, 858. 191	2,509,858.209
РΤ	POT BASE003	314+24.35	887,679.069	2,510,208.739

EAST OF END -OF BRIDGE

-WEST END OF BRIDGE

CENTER OF DROP INLET



			GRAPHI	C SCAL	Ε.		
			MET	ERS			
80	0	80	16	0	240	320	400
200	Ö	200	400 FEET	600	800	1000	
	00/ 50/ 50/	STA. 3	305+00.00 305+00.00 ETAIL "A	0.00		TING CENTEF ELINE)	RLINE)

B IL 185

CONTROL POINT 20,001

POINT POT BASEOO1

BENCHMARK TJM100

BENCHMARK BM-72

¢ IL 185

CONTROL POINT 1,000-

POINT PC C12-

STA. 314+24.35, 0.00' LT. (EXISTING CENTERLINE) STA. 314+24.34, 0.00' LT. (BASELINE)

-POINT POT BASE003

EXIST. CURVE 11 PI STA. = 302+87.14 Δ = 4° 05′ 32′′ (LT) D = 1° 00′ 23′′ R = 5,692.98'T = 203.39'

L = 406.61'

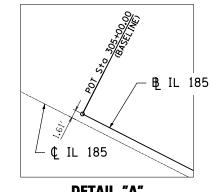
E = 3.63'

PI STA. = 322+45.33 Δ = 7° 43′ 23″ (RT) D = 2° 00′ 07′′ R = 2,862.16' T = 193.19' L = 385.79'

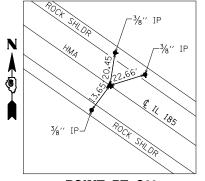
E = 6.51'

EXIST. CURVE 12

P.C. STA. = 300+83.75 P.C. STA. = 320+52.14 P.T. STA. = 304+90.36 P.T. STA. = 324+37.93

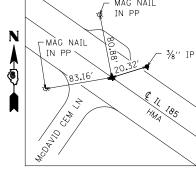


DETAIL "A"



POINT PT C11

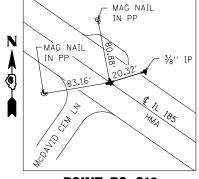
PK NAIL IN CHSLD "X" STA. 304+90.36, 0.00' RT N 888,102.614 E 2,509,376,310



POINT PC C12

PK NAIL IN HMA STA. 320+52.14, 0.00' RT N 887,394.378 E 2,510,768.268

185



185

BENCHMARK BM-71

POINT PT C11

STA. 310+30.70, 0.69' LT. (EXISTING CENTERLINE) STA. 310+30.70, 0.00' LT. (BASELINE)

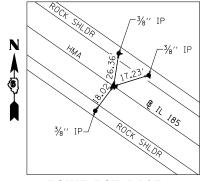
CONTROL POINT 20,000-

EXISTING AND PROPOSED ROW IS MEASURED TO BASELINE OF IL 185. PROPOSED BASELINE AND EXISTING CENTERLINE ARE NOT COINCIDENT. SEE DETAIL "A" AND DETAIL "B".

45/

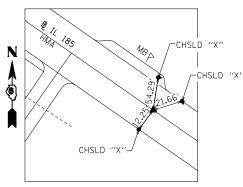
CONTROL POINT 1,000

4 REBAR W. IDOT CAP STA. 319+81.63, 23.05' RT N 887,405.809 E 2,510,694.979



POINT POT BASE001

SET PK N&BC W/CHSLD "X" STA. 305+00.00, 1.61' LT. OF EXIST. ¢ N 888,099.678 E 2,509,385.634



POINT POT BASE003

SET PK N&BC W/CHSLD "X" STA. 314+24.35, 0.00' RT. OF EXIST. ¢ N 887,679.069 E 2,510,208.739

COUNTY

MONTGOMERY 121 25

CONTRACT NO. 72DO8

design firm no. 184001036



N 888,076.346

E 2,509,474.453

CONTROL POINT 20,000

STA. 305+89.74, 21.09' LT

4 REBAR W. IDOT CAP

USER NAME = gjameson	DESIGNED -	REVISED
FILE NAME = D672D08-SHT-ATB.dgn	CHECKED -	REVISED
PLOT SCALE = 400.0000 '/ IN.	DRAWN -	REVISED
PLOT DATE = 10/15/2015	CHECKED -	REVISED

N 887,701.802

E 2,510,119.477

CONTROL POINT 20,001

STA. 313+34.48, 20.22' RT

4 REBAR W. IDOT CAP

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

	ALIGNMENT, TIES AND BENCHMARKS IL 185 OVER MCDAVID BRANCH		F.A.P. RTE.	SECTION	COUNTY	
			777	10B-2, 405B-1	MONTGOME	
	IL 103 OVER MICDAVID DRANGII					CONTRA
	SCALE: 1" = 200"	SHEET NO. 1 OF 1 SHEETS	STA. 307+05.00 TO STA. 314+00.00		ILLINOIS FED. A	ID PROJECT

BENCHMARK BM 88

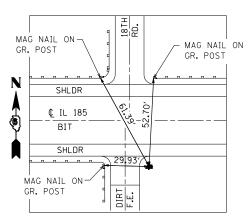
STA. 480+98, 59' LT. COTTON PICKER SPINDLE IN NORTH SIDE OF POWER POLE ELEV. 607.07 (NAVD 88 DATUM)

BENCHMARK BM 89

STA. 492+95, 17' LT. CHSLD "□" ON NE WINGWALL SN 068-0027 ELEV. 558.70 (NAVD 88 DATUM)

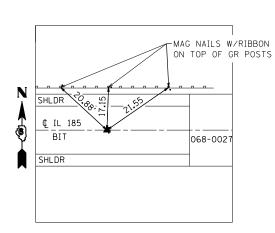
	ALIGNMENT	COORDINATES IL 1	85
POINT	STA.	N	E
PT C5	491+95.83	877,658.762	2,524,692.361
PC C6	495+89.59	877, 667. 536	2,525,086.025
PI C6	497+90.25	877,672.007	2, 525, 286, 631
PT C6	499+84.98	877, 594, 230	2,525,471.600

ALIGNMENT COORDINATES EAST 18TH ROAD			H ROAD	
	POINT	STA.	N	E
	EAST001	10+00.00	877, 663. 665	2,524,912.320
	EAST002	11+53.61	877, 817. 279	2,524,911.905



CONTROL POINT 3000

#4 REBAR W/CAP FLUSH STA. 494+23.29, 29.77' RT N 877,634.063 E 2,524,920.422



POINT PT C5

MAG NAIL W/CHSLD. "X" STA. 491+95.83, 0.00' RT N 877,658.762 E 2,524,692.361

BENCHMARK ZP-1

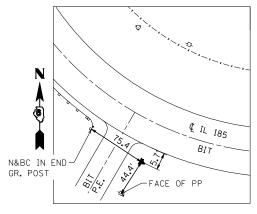
STA. 501+25, 20' LT. CHSLD "□" ON NE WINGWALL SN 068-0505

ELEV. 562.11 (NAVD 88 DATUM)

EXIST. CURVE 5 PI STA. = 486+22.05 $\Delta = 56^{\circ} 52' 53'' \text{ (LT)}$ D = 4° 30′ 18′′ R = 1,271.81' T = 688.83' L = 1,262.61 E = 174.56' e = 5.0% T.R. = 53 S.E. RUN = 177 P.C. STA. = 479+33.22 P.T. STA. = 491+95.83

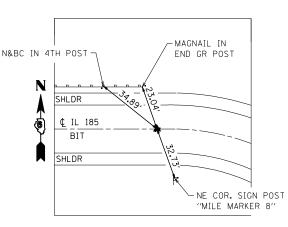
END FULL SE AT STA. 490+81 ATTAIN 1.5% SE AT STA. 492+05 MAINTAIN 1.5% SE TO STA. 493+43 END SE TRANSITION (0.0%) AT STA. 493+96

¢ IL 185



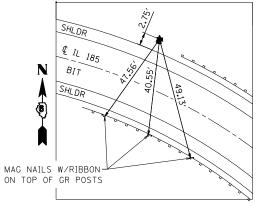
CONTROL POINT 3001

#4 REBAR W/CAP FLUSH STA. 486+75.16, 21.29' RT N 877,732.847 E 2,524,175.857



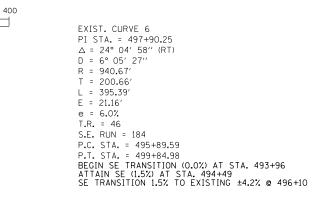
POINT PC C6

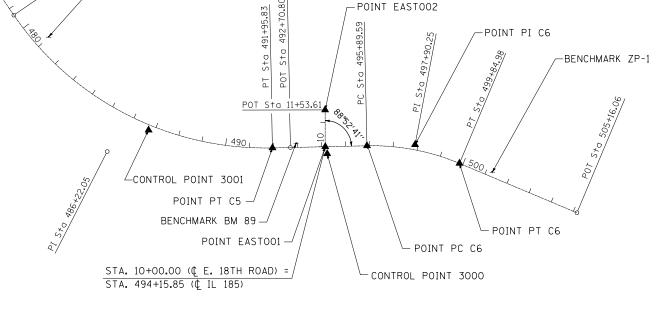
PK NAIL W/CHSLD. "X" STA. 495+89.59, 0.00' RT N 877,667.536 E 2,525,086.025



POINT PI C6

#5 REBAR W/CAP STA. 497+90.25, 21.16' LT N 877,672.007 E 2,525,286.631





GRAPHIC SCALE METERS 160

600

800

1000

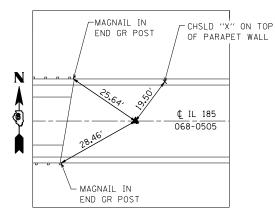
400

FEET

200

BENCHMARK BM 88

200



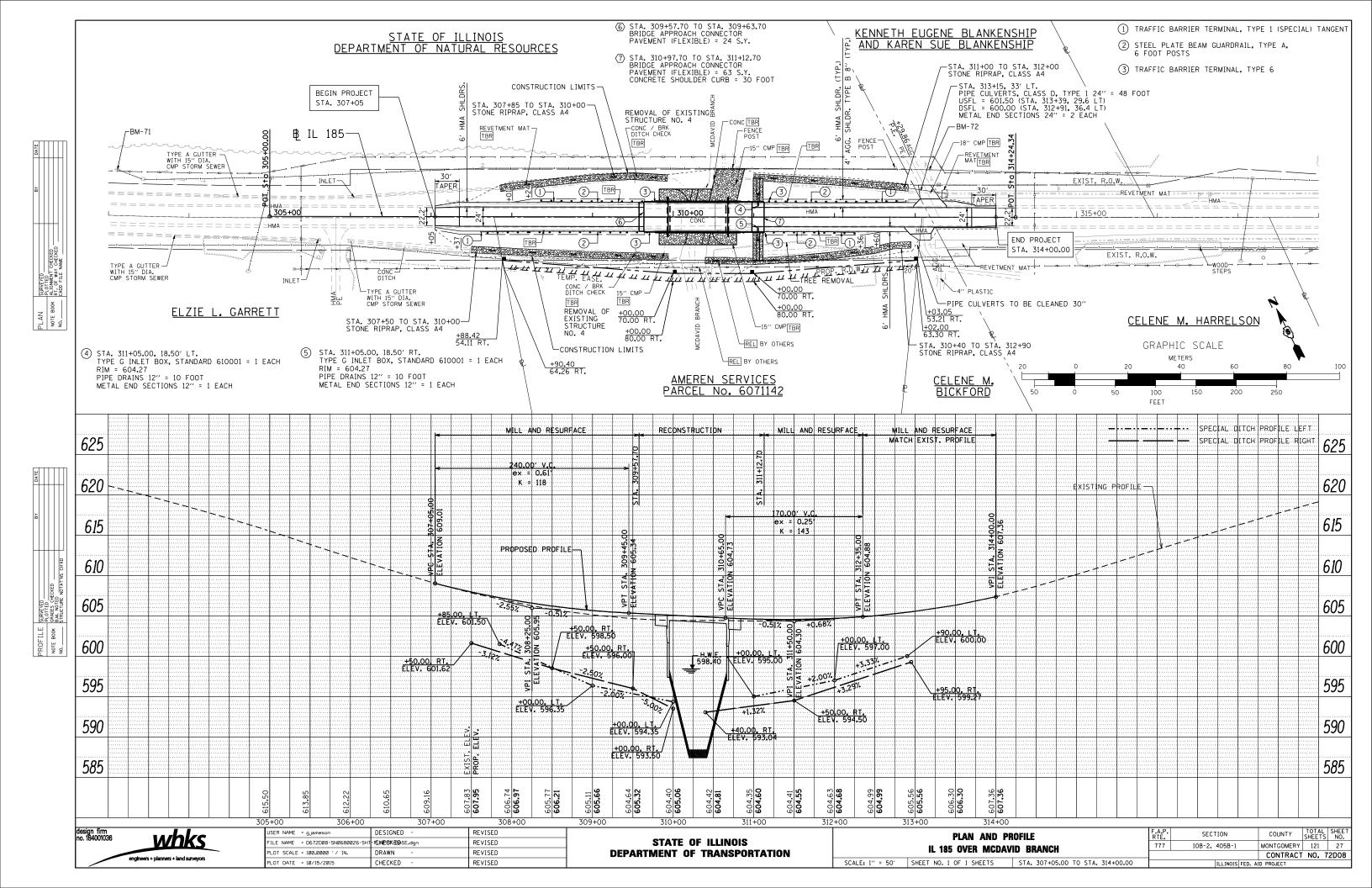
POINT PT C6

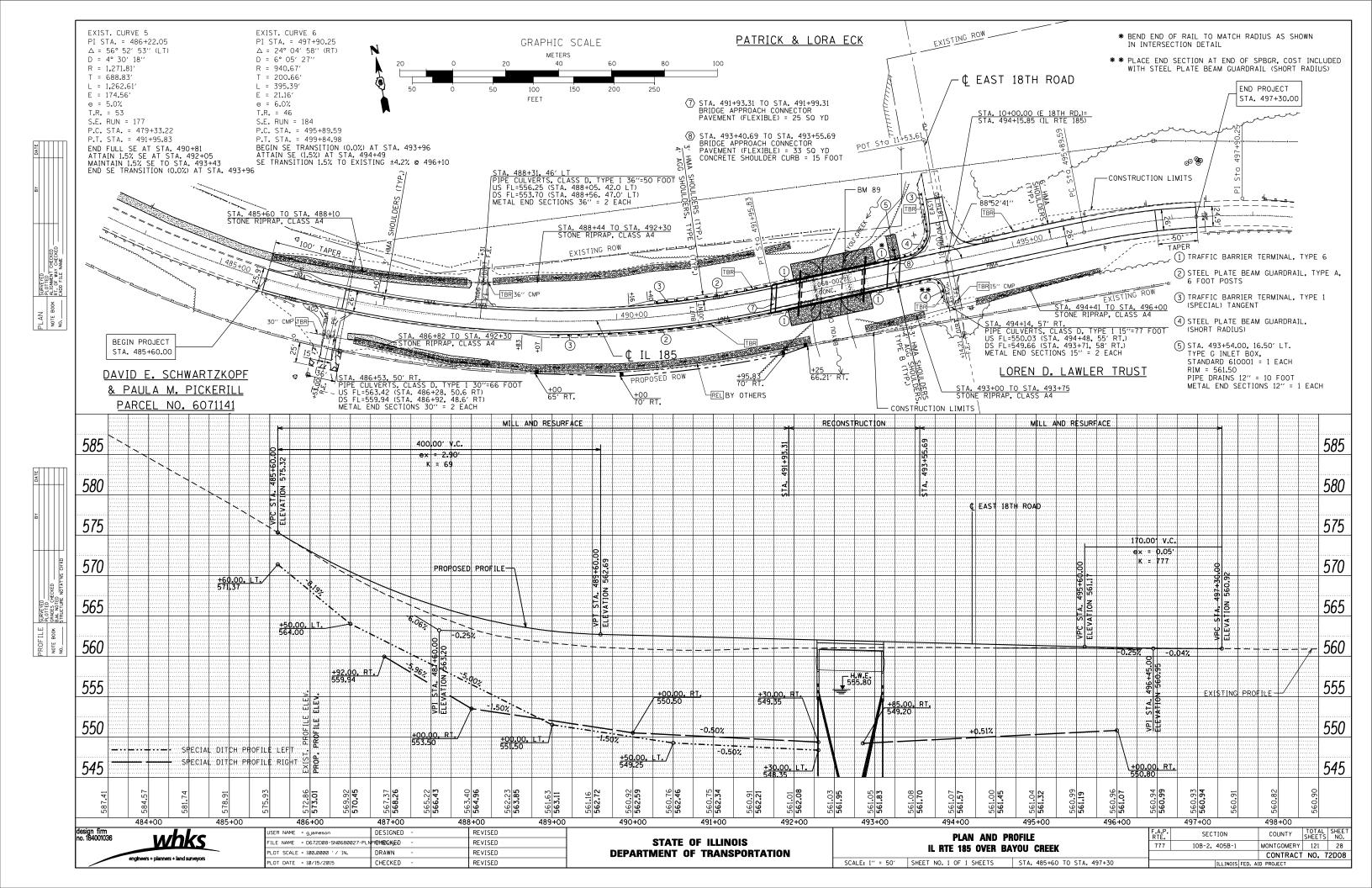
STAR DRILL W/ CHSLD. "X" STA. 499+84.98, 0.00' RT N 877,594.230 E 2,525,471.600

whks engineers + planners + land surveyors

JSER NAME = gjameson	DESIGNED -	REVISED
FILE NAME = D672DØ8-SHT-ATB.dgn	CHECKED -	REVISED
PLOT SCALE = 400.0000 '/ IN.	DRAWN -	REVISED
PLOT DATE = 10/15/2015	CHECKED -	REVISED

ALIGNMENT, TIES AND BENCHMARKS	F.A.P. RTE.	SECTION
IL RTE 185 OVER BAYOU CREEK	777	10B-2, 405
II III 100 OVER BATOO ONEEK		
SHEET NO 1 OF 1 SHEETS STA ABELEO TO STA AB7130		71.17





PRE-STAGE CONSTRUCTION:

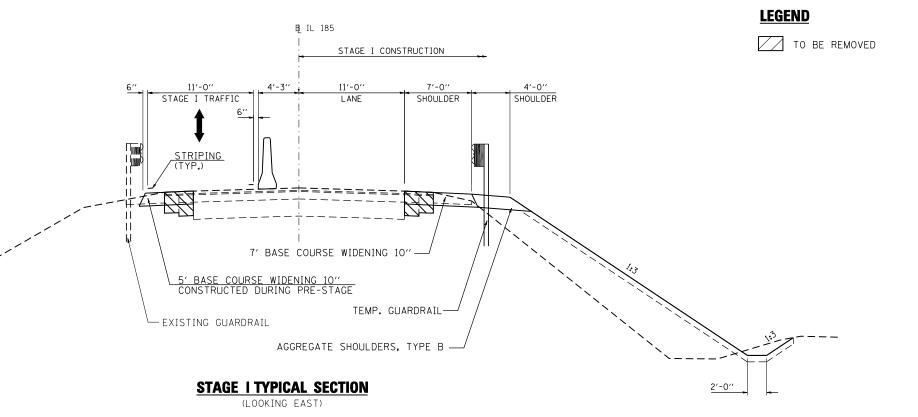
- 1. TRAFFIC REMAINS ON EXISTING PAVEMENT.
- 2. REMOVE THE EXISTING HMA SHOULDERS/WIDENING ALONG THE NORTH SIDE OF IL 185 AS SHOWN IN THE PLANS.
- CONSTRUCT THE WIDENING ALONG THE NORTH SIDE OF IL 185. USE HIGHWAY STANDARD 701326.
- 4. INSTALL WIDTH RESTRICTION SIGNING.
- 5. INSTALL SIGNS AND TEMPORARY BRIDGE TRAFFIC SIGNAL LOOP DETECTORS. USE HIGHWAY STANDARD 701201 OR OTHER APPLICABLE STANDARDS.

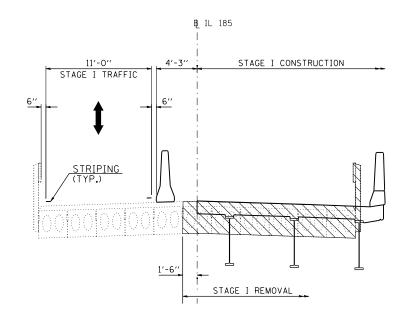
STAGE I CONSTRUCTION:

- REMOVE CONFLICTING STRIPING AND INSTALL TEMPORARY CONCRETE BARRIERS, TEMPORARY IMPACT ATTENUATORS, TEMPORARY STRIPING AND TEMPORARY BRIDGE TRAFFIC SIGNALS AS SHOWN ON THE PLANS. INSTALL OTHER TRAFFIC CONTROL AND PROTECTION USING HIGHWAY STANDARD 701321.
- 2. SHIFT TRAFFIC TO STAGE I LANE.
- 3. REMOVE THE SOUTH HALF OF EXISTING BRIDGE.
- 4. REMOVE EXISTING GUARDRAIL AND SHOULDERS/WIDENING ALONG THE SOUTH SIDE OF IL 185
- 5. CONSTRUCT SOUTH HALF OF PROPOSED BRIDGE, CONNECTOR PAVEMENT AND EARTHWORK.
- 6. CONSTRUCT WIDENING, TEMPORARY RAMPS, AGGREGATE SHOULDERS AND TEMPORARY GUARDRAIL ALONG THE SOUTH SIDE OF IL 185.

GENERAL STAGING NOTES

- 1. REFER TO THE PLAN AND PROFILE SHEETS AND BRIDGE PLANS FOR DRAINAGE ITEMS AND IN-STREAM WORK.
- 2. THE CONTRACTOR SHALL MAINTAIN ACCESS TO ALL ENTRANCES AT ALL TIMES DURING CONSTRUCTION.
- 3. THE TRAFFIC CONTROL PLANS SHALL BE USED IN CONJUNCTION WITH APPLICABLE HIGHWAY STANDARDS.
- 4. VERTICAL PANELS, DRUMS WITH STEADY BURNING LIGHTS, TYPE III BARRICADES, SIGNS, MICROWAVE DETECTOR SYSTEMS, DETECTOR LOOPS AND TYPE C BIDIRECTIONAL REFLECTORS SHALL BE INCLUDED IN THE COST FOR TRAFFIC CONTROL AND PROTECTION, STANDARD 701321. TEMPORARY PAVEMENT MARKINGS, TEMPORARY BRIDGE TRAFFIC SIGNALS, CHANGEABLE MESSAGE SIGNS AND TEMPORARY RUMBLE STRIPS SHALL BE PAID FOR SEPARATELY.
- THE CONTRACTOR SHALL PROVIDE AND ERECT LANE WIDTH RESTRICTION SIGNING.
 THESE SIGNS SHALL BE PLACED AS DIRECTED BY THE ENGINEER BEFORE
 IMPLEMENTING ANY STAGE I TRAFFIC CONTROL. SEE SPECIAL PROVISIONS.
- THE CONTRACTOR SHALL NOTIFY THE DISTRICT 6 TRAFFIC SECTION OF THE BUREAU
 OF OPERATIONS, PH: (217) 785-7314, AT LEAST 21 DAYS PRIOR TO IMPLEMENTING
 STAGE I TRAFFIC AND WHEN A SWITCH IN STAGING IS MADE.
- THE CONTRACTOR SHALL NOTIFY THE DISTRICT 6 TRAFFIC SECTION OF THE BUREAU
 OF OPERATIONS AT LEAST THREE (3) DAYS PRIOR TO ACTIVATING THE TEMPORARY
 TRAFFIC SIGNALS. SEE SPECIAL PROVISIONS FOR TEMPORARY BRIDGE TRAFFIC
 SIGNALS.
- 8. UNLESS OTHERWISE INDICATED, TEMPORARY BRIDGE TRAFFIC SIGNALS, STOP BARS, TEMPORARY RUMBLE STRIPS, DETECTOR LOOPS AND ADVANCE SIGN LOCATIONS TO REMAIN IN PLACE THROUGHOUT THE DURATION OF STAGES I AND II AS PER HIGHWAY STANDARD 701321.
- 9. ALL TEMPORARY TRAFFIC SIGNALS REQUIRED FOR BRIDGE AND ENTRANCES FOR BOTH STAGES OF CONSTRUCTION WILL BE MEASURED AS 1 UNIT.
- 10. THE INTENT OF THE TEMPORARY GUARDRAIL AND TERMINALS SECTIONS IS TO ALLOW FOR SAFE PLACEMENT (CORRECT ELEVATION AND OFFSET) DURING TEMPORARY OR STAGE CONDITIONS. THE COST FOR SUPPLYING THE TEMPORARY GUARDRAIL AND TERMINAL SECTIONS AND ADJUSTING TO PERMANENT CONDITIONS SHALL BE CONSIDERED IN THE TEMPORARY PAY ITEMS INCLUDED IN THE CONTRACT. THIS WORK SHALL INCLUDE ANY EXTRA MATERIAL, CORING OR FILLING HOLES FOR POST INSTALLATION, LABOR AND MATERIALS FOR ALL ADJUSTMENTS AND ANY ADDITIONAL WORK NECESSARY TO PROVIDE SAFE GUARDRAIL CONDITIONS FOR TEMPORARY AND PERMANENT CONDITIONS. ADJUSTMENTS MAY BE MADE BY THE DIRECTION OF THE ENGINEER.





STAGE I BRIDGE TYPICAL

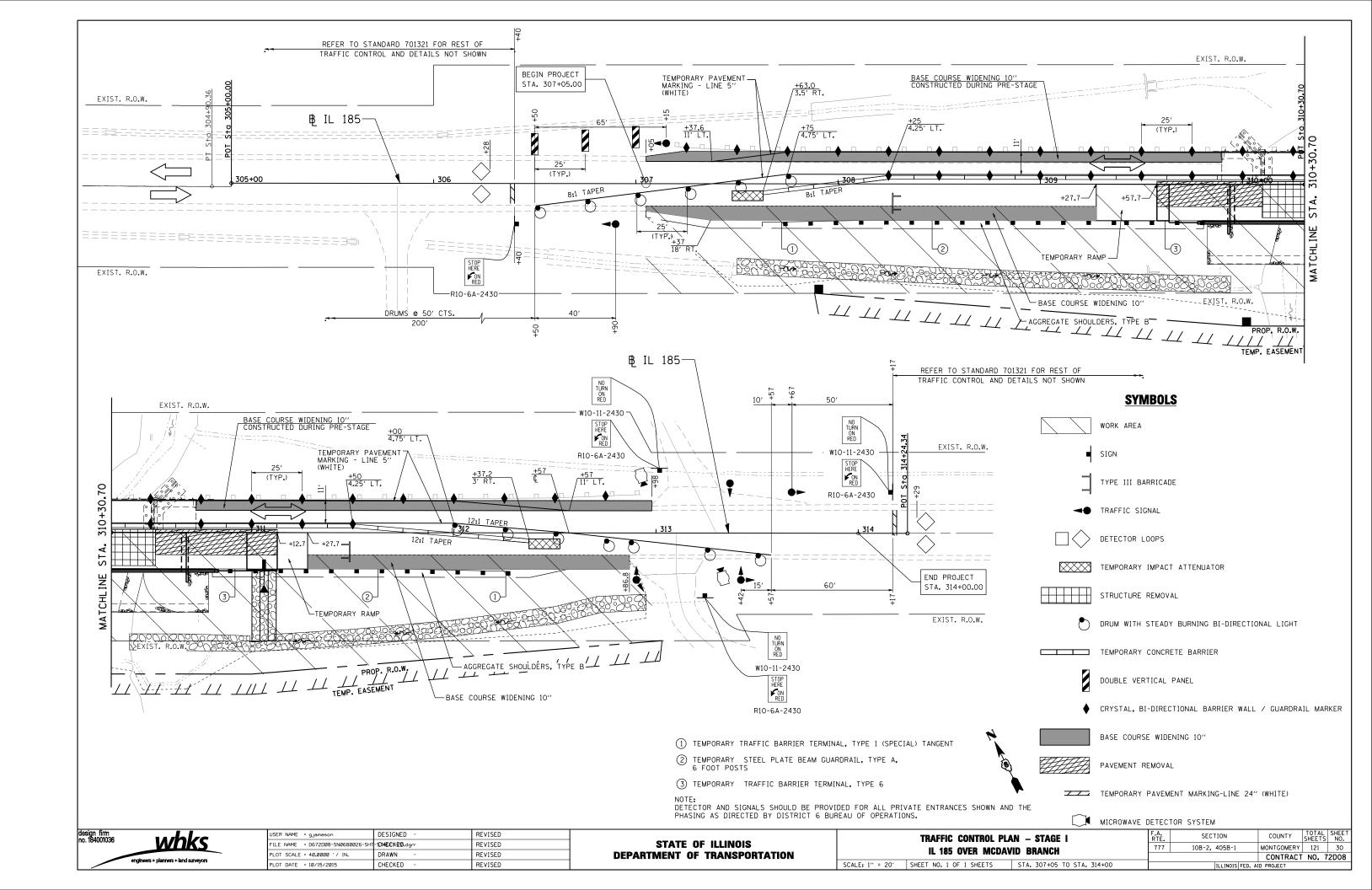
(LOOKING EAST)



USER NAME	= gjameson	DESIGNED -	REVISED
FILE NAME	= D672DØ8-SNØ68ØØ26-SHT	- CHESKEDI.dgn	REVISED
PLOT SCALE	= 10.0000 '/ IN.	DRAWN -	REVISED
PLOT DATE	= 10/15/2015	CHECKED -	REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TRAFFIC CONTROL TYPICAL SECTIONS - STAGE I				F.A. RTE.		
IL 185 OVER MCDAVID BRANCH				777		
IL 103 OVER MICDAVID BRANCH						
SCALE: 1" = 10'	SHEET NO. 1 OF 1 SHEETS	STA.	TO STA.			

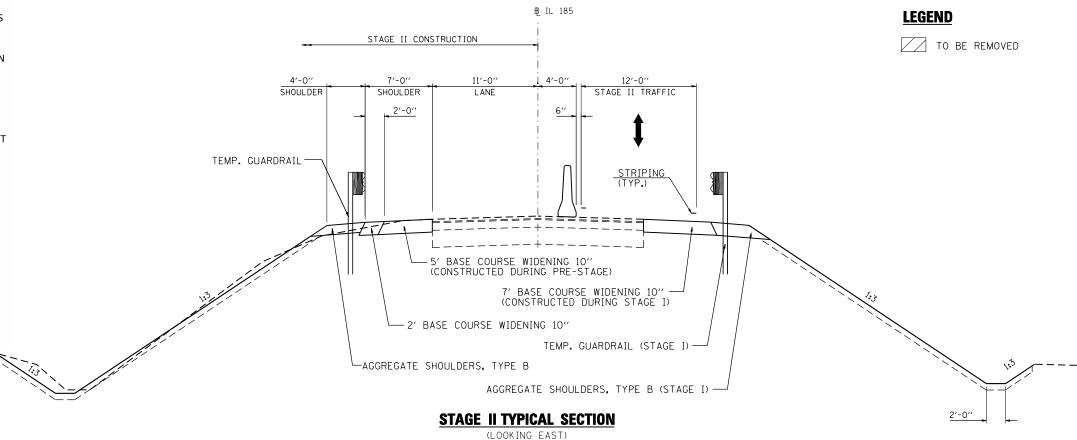


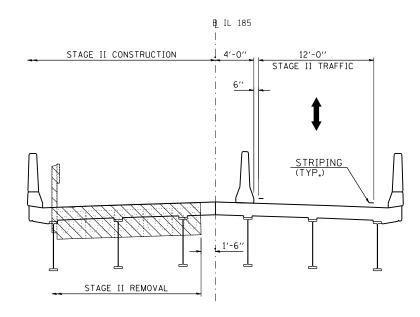
STAGE II CONSTRUCTION:

- 1. RELOCATE TEMPORARY CONCRETE BARRIERS, TEMPORARY IMPACT ATTENUATORS AND TEMPORARY BRIDGE TRAFFIC SIGNALS AS SHOWN ON THE PLANS. USE HIGHWAY STANDARD 701321.
- 2. REMOVE CONFLICTING STRIPING AND PLACE TEMPORARY STRIPING AS SHOWN ON THE PLANS.
- 3. SHIFT TRAFFIC TO STAGE II LANE.
- 4. REMOVE THE NORTH HALF OF THE EXISTING BRIDGE.
- 5. REMOVE EXISTING GUARDRAIL AND SHOULDERS/WIDENING ALONG THE NORTH SIDE OF IL 185.
- 6. CONSTRUCT THE NORTH HALF OF THE PROPOSED BRIDGE, CONNECTOR PAVEMENT EARTHWORK.
- 7. CONSTRUCT WIDENING, TEMPORARY RAMPS, AGGREGATE SHOULDERS AND TEMPORARY GUARDRAIL ALONG THE NORTH SIDE OF IL 185.

STAGE III CONSTRUCTION:

- 1. REMOVE TEMPORARY CONCRETE BARRIERS, TEMPORARY IMPACT ATTENUATORS, TEMPORARY TRAFFIC SIGNALS AND WIDTH RESTRICTION SIGNING.
- 2. REMOVE CONFLICTING STRIPING AND PLACE TEMPORARY STRIPING.
- 3. SHIFT TRAFFIC TO THE EXISTING LANES.
- 4. REMOVE TEMPORARY RAMPS, MILL AND CONSTRUCT THE HMA BINDER AND SURFACE COURSES USING HIGHWAY STANDARD 701306.
- 5. CONSTRUCT HMA AND AGGREGATE SHOULDERS AND PERMANENT GUARDRAIL USING HIGHWAY STANDARD 701326.
- 6. FINISH EARTHWORK AND GRADING AND SEED DISTURBED AREAS.
- 7. PLACE FINAL STRIPING AND CLEANUP.





STAGE II BRIDGE TYPICAL

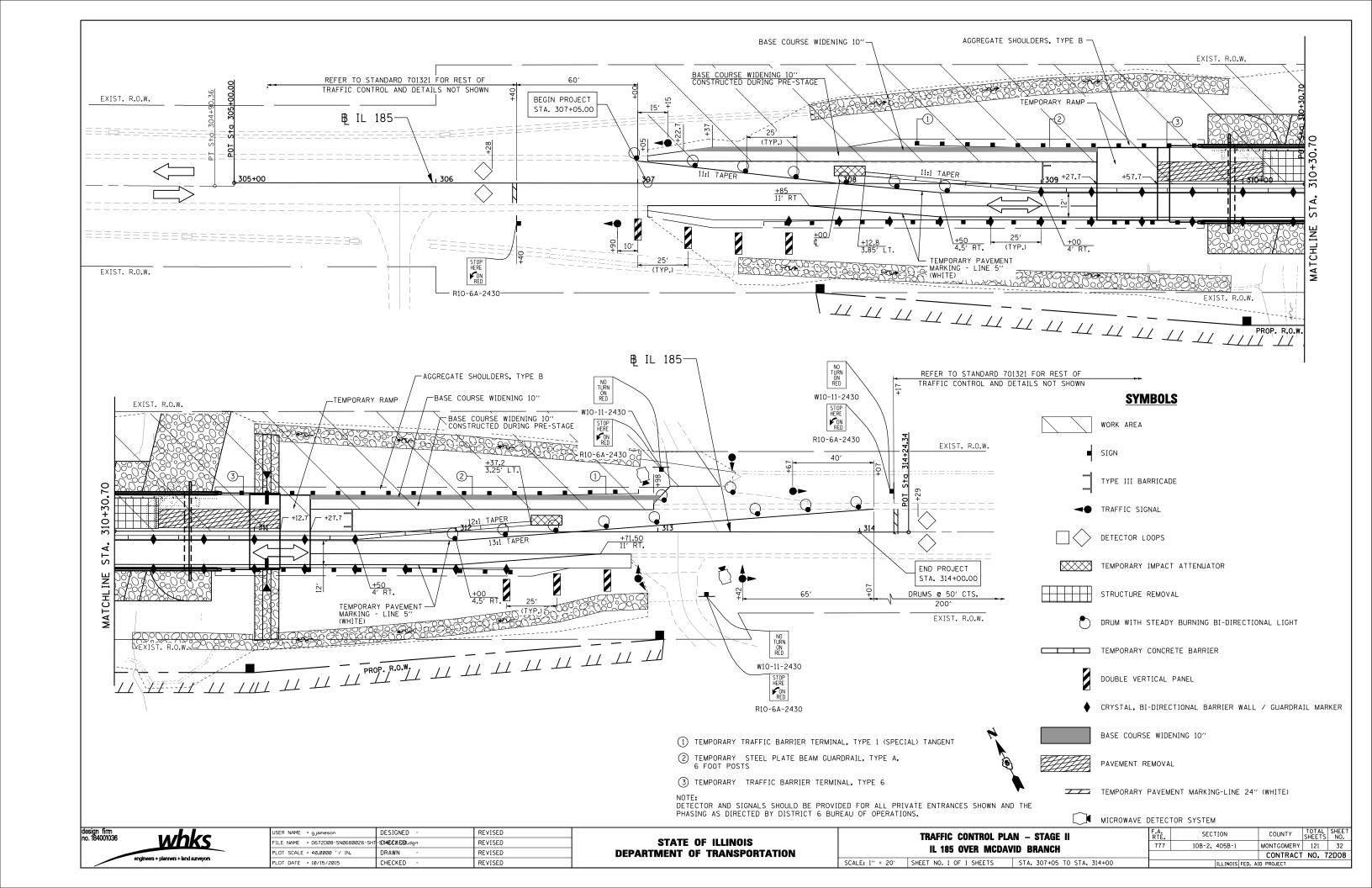
(LOOKING EAST)



USER NAME = gjameson	DESIGNED -	REVISED
FILE NAME = D672D08-SN	0680026-SHT-1 CHFE:\$K4E0 £2.dgn	REVISED
PLOT SCALE = 10.0000 '/	IN. DRAWN -	REVISED
PLOT DATE = 10/15/2015	CHECKED -	REVISED

STATE OI	- ILLINOIS
DEPARTMENT OF	TRANSPORTATION

			F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEE.
			777	10B-2, 405B-1	MONTGOMERY	121	31
	IL 103 OVER MODAVID BRANCOI				CONTRACT	NO.	72D08
SCALE: 1" = 10' SHEET NO. 1 OF 1 SHEETS STA. TO STA.			ILLINOIS FED. AID PROJECT				



PRE-STAGE CONSTRUCTION:

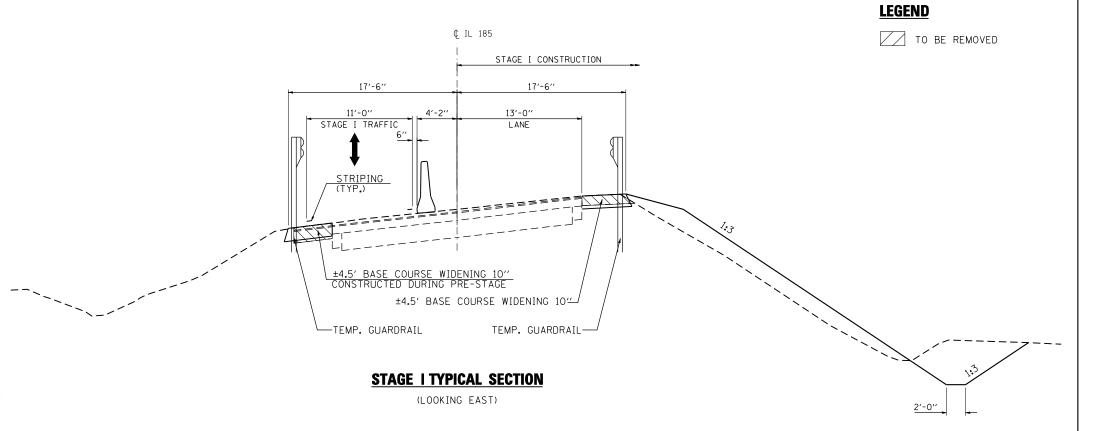
- 1. TRAFFIC REMAINS ON EXISTING PAVEMENT.
- 2. REMOVE THE EXISTING GUARDRAIL (NW ONLY), HMA SHOULDERS/WIDENING ALONG THE NORTH SIDE OF IL 185 AS SHOWN IN THE PLANS.
- CONSTRUCT THE WIDENING ALONG THE NORTH SIDE OF IL 185. USE HIGHWAY STANDARD 701326.
- 4. INSTALL TEMPORARY GUARDRAIL (NW ONLY).
- 5. INSTALL WIDTH RESTRICTION SIGNING.
- 6. INSTALL SIGNS AND TEMPORARY BRIDGE TRAFFIC SIGNAL LOOP DETECTORS. USE HIGHWAY STANDARD 701201 OR OTHER APPLICABLE STANDARDS.

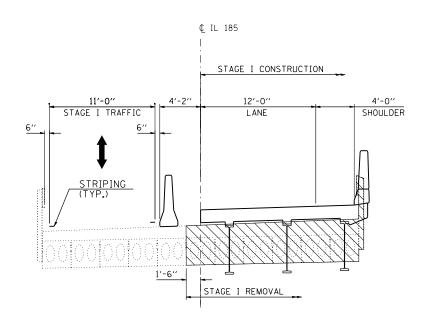
STAGE I CONSTRUCTION:

- REMOVE CONFLICTING STRIPING AND INSTALL TEMPORARY CONCRETE BARRIERS, TEMPORARY IMPACT ATTENUATORS, TEMPORARY STRIPING AND TEMPORARY BRIDGE TRAFFIC SIGNALS AS SHOWN ON THE PLANS. INSTALL OTHER TRAFFIC CONTROL AND PROTECTION USING HIGHWAY STANDARD 701321.
- 2. SHIFT TRAFFIC TO STAGE I LANE.
- 3. REMOVE THE SOUTH HALF OF EXISTING BRIDGE.
- 4. REMOVE EXISTING GUARDRAIL, SHOULDERS/WIDENING AND PIPE CULVERTS ALONG THE SOUTH SIDE OF IL 185.
- CONSTRUCT SOUTH HALF OF PROPOSED BRIDGE, CONNECTOR PAVEMENT AND EARTHWORK.
- CONSTRUCT WIDENING, TEMPORARY RAMPS, AGGREGATE SHOULDERS, PIPE CULVERTS AND TEMPORARY GUARDRAIL ALONG THE SOUTH SIDE OF IL 185.

GENERAL STAGING NOTES

- 1. REFER TO THE PLAN AND PROFILE SHEETS AND BRIDGE PLANS FOR DRAINAGE ITEMS AND IN-STREAM WORK.
- 2. THE CONTRACTOR SHALL MAINTAIN ACCESS TO ALL ENTRANCES AT ALL TIMES DURING CONSTRUCTION.
- 3. THE TRAFFIC CONTROL PLANS SHALL BE USED IN CONJUNCTION WITH APPLICABLE HIGHWAY STANDARDS.
- 4. VERTICAL PANELS, DRUMS WITH STEADY BURNING LIGHTS, TYPE III BARRICADES, SIGNS, MICROWAVE DETECTOR SYSTEMS, DETECTOR LOOPS AND TYPE C BIDIRECTIONAL REFLECTORS SHALL BE INCLUDED IN THE COST FOR TRAFFIC CONTROL AND PROTECTION, STANDARD 701321. TEMPORARY PAVEMENT MARKINGS, TEMPORARY BRIDGE TRAFFIC SIGNALS, CHANGEABLE MESSAGE SIGNS AND TEMPORARY RUMBLE STRIPS SHALL BE PAID FOR SEPARATELY.
- 5. THE CONTRACTOR SHALL PROVIDE AND ERECT LANE WIDTH RESTRICTION SIGNING. THESE SIGNS SHALL BE PLACED AS DIRECTED BY THE ENGINEER BEFORE IMPLEMENTING ANY STAGE I TRAFFIC CONTROL. SEE SPECIAL PROVISIONS.
- 6. THE CONTRACTOR SHALL NOTIFY THE DISTRICT 6 TRAFFIC SECTION OF THE BUREAU OF OPERATIONS, PH: (217) 785-7314, AT LEAST 21 DAYS PRIOR TO IMPLEMENTING STAGE I TRAFFIC AND WHEN A SWITCH IN STAGING IS MADE.
- 7. THE CONTRACTOR SHALL NOTIFY THE DISTRICT 6 TRAFFIC SECTION OF THE BUREAU OF OPERATIONS AT LEAST THREE (3) DAYS PRIOR TO ACTIVATING THE TEMPORARY TRAFFIC SIGNALS. SEE SPECIAL PROVISIONS FOR TEMPORARY BRIDGE TRAFFIC SIGNALS.
- 8. UNLESS OTHERWISE INDICATED, TEMPORARY BRIDGE TRAFFIC SIGNALS, STOP BARS, TEMPORARY RUMBLE STRIPS, DETECTOR LOOPS AND ADVANCE SIGN LOCATIONS TO REMAIN IN PLACE THROUGHOUT THE DURATION OF STAGES I AND II AS PER HIGHWAY STANDARD 701321.
- 9. ALL TEMPORARY TRAFFIC SIGNALS REQUIRED FOR BRIDGE AND ENTRANCES FOR BOTH STAGES OF CONSTRUCTION WILL BE MEASURED AS 1 UNIT.
- 10. THE INTENT OF THE TEMPORARY GUARDRAIL AND TERMINALS SECTIONS IS TO ALLOW FOR SAFE PLACEMENT (CORRECT ELEVATION AND OFFSET) DURING TEMPORARY OR STAGE CONDITIONS. THE COST FOR SUPPLYING THE TEMPORARY GUARDRAIL AND TERMINAL SECTIONS AND ADJUSTING TO PERMANENT CONDITIONS SHALL BE CONSIDERED IN THE TEMPORARY PAY ITEMS INCLUDED IN THE CONTRACT. THIS WORK SHALL INCLUDE ANY EXTRA MATERIAL, CORING OR FILLING HOLES FOR POST INSTALLATION, LABOR AND MATERIALS FOR ALL ADJUSTMENTS AND ANY ADDITIONAL WORK NECESSARY TO PROVIDE SAFE GUARDRAIL CONDITIONS FOR TEMPORARY AND PERMANENT CONDITIONS. ADJUSTMENTS MAY BE MADE BY THE DIRECTION OF THE ENGINEER.





STAGE I TYPICAL SECTION

(LOOKING EAST)



USER NAME = gjameson	DESIGNED -	REVISED
FILE NAME = D672D08-SN0680027-SHT	- CHECKADEL.dgn	REVISED
PLOT SCALE = 10.0000 '/ IN.	DRAWN -	REVISED
PLOT DATE = 10/15/2015	CHECKED -	REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

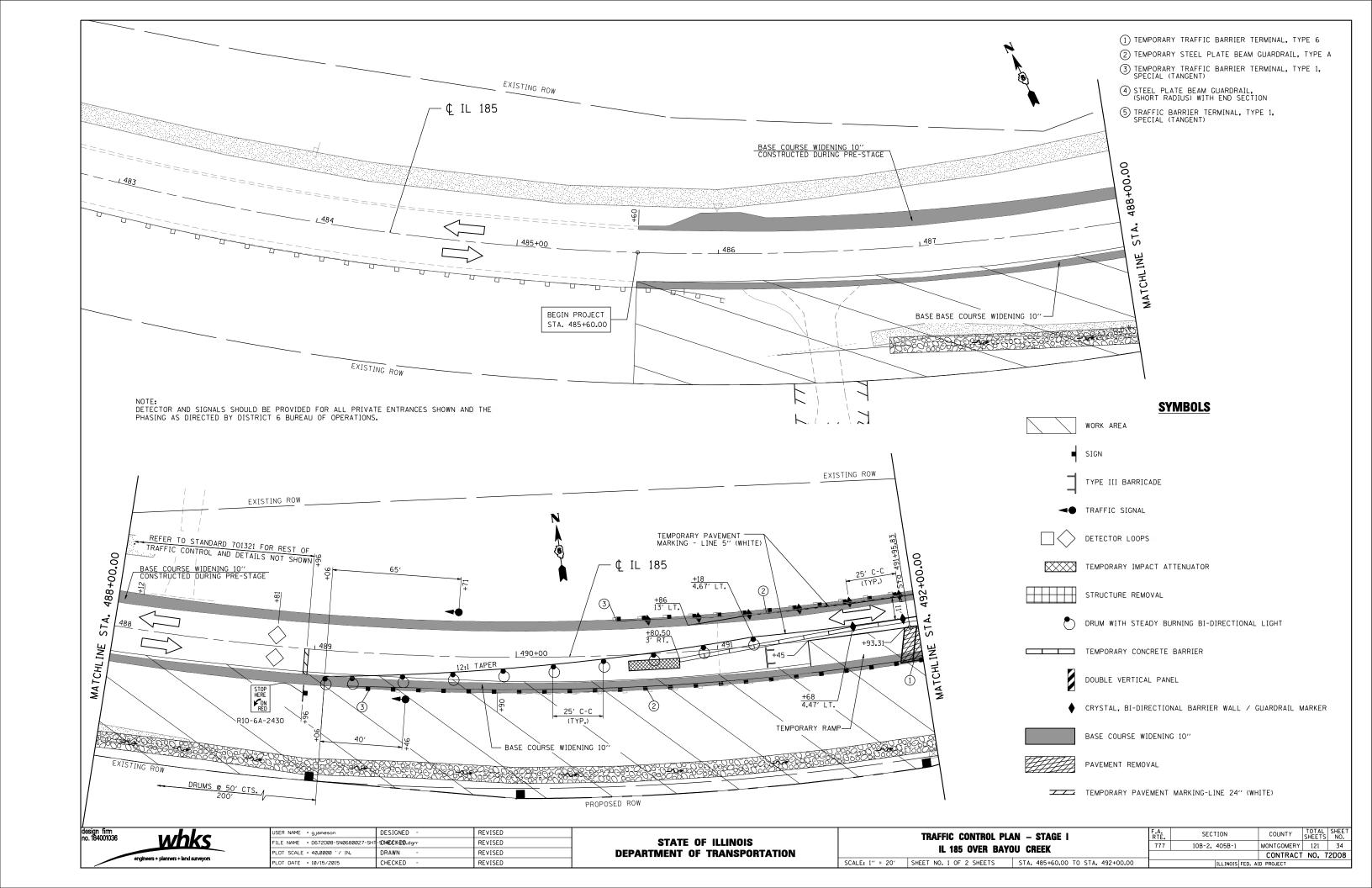
TRAFFIC CONTROL TYPICAL SECTIONS – STAGE I			F.A. RTE.	SECTION	
	IL 185 OVER BAYOU CREEK				10B-2, 405B-1
	IL 103 OVER BATOU GREEK				
,	CHEET NO 1 OF 1 CHEETS	l cta	TO CTA		

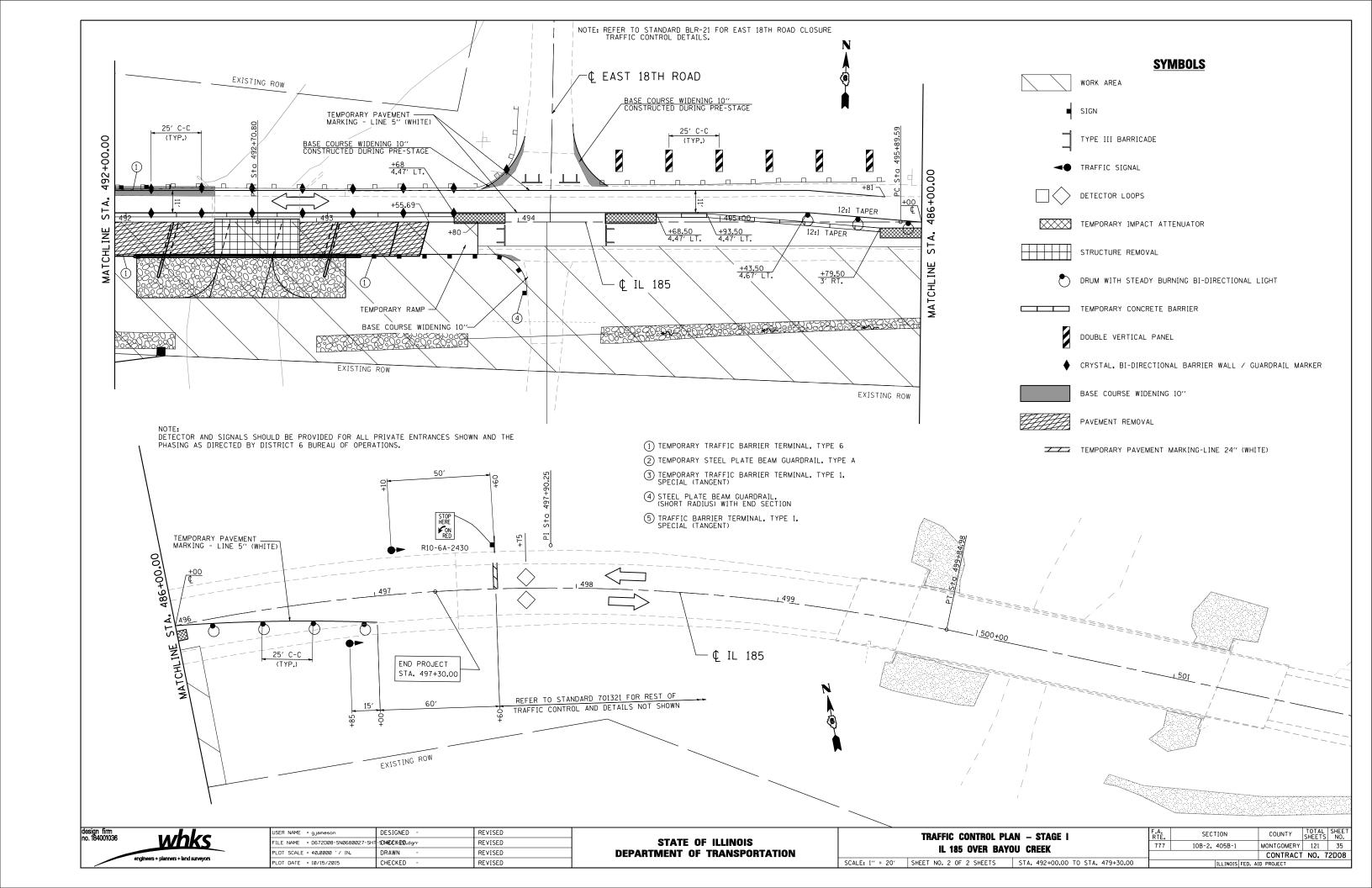
COUNTY

MONTGOMERY

121 33

CONTRACT NO. 72D08





STAGE II CONSTRUCTION:

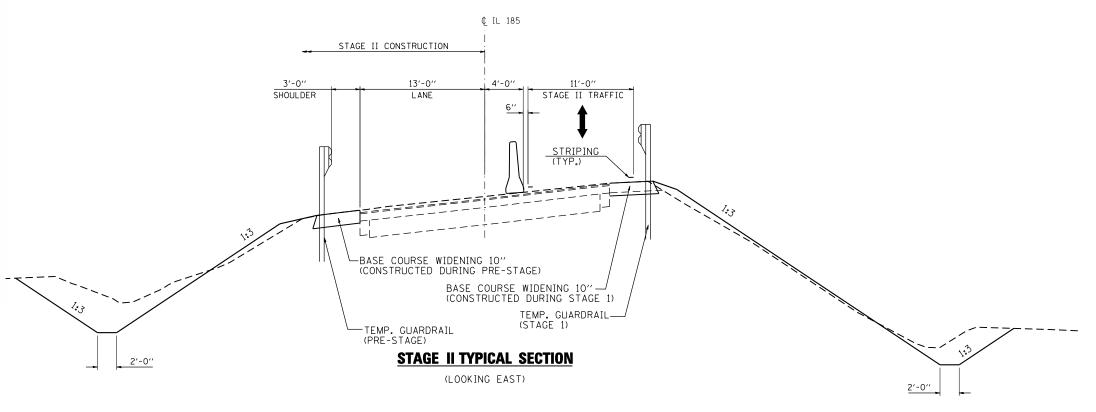
- RELOCATE TEMPORARY CONCRETE BARRIERS, TEMPORARY IMPACT ATTENUATORS AND TEMPORARY BRIDGE TRAFFIC SIGNALS AS SHOWN ON THE PLANS. USE HIGHWAY STANDARD 701321.
- 2. REMOVE CONFLICTING STRIPING AND PLACE TEMPORARY STRIPING AS SHOWN ON THE PLANS.
- 3. SHIFT TRAFFIC TO STAGE II LANE.
- 4. REMOVE THE NORTH HALF OF THE EXISTING BRIDGE.
- 5. REMOVE EXISTING PIPE CULVERTS ALONG THE NORTH SIDE OF IL 185.
- 6. CONSTRUCT THE NORTH HALF OF THE PROPOSED BRIDGE, CONNECTOR PAVEMENT AND EARTHWORK.
- 7. CONSTRUCT WIDENING, TEMPORARY RAMPS, AGGREGATE SHOULDERS, PIPE CULVERTS AND INSTALL OR ADJUST TEMPORARY GUARDRAIL ALONG THE NORTH SIDE OF IL 185.

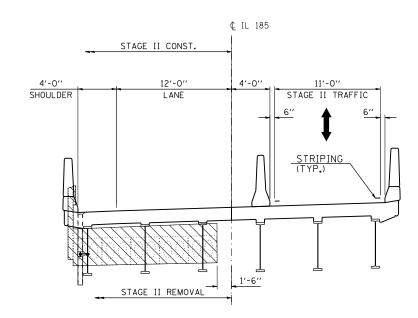
STAGE III CONSTRUCTION:

- 1. REMOVE TEMPORARY CONCRETE BARRIERS, TEMPORARY IMPACT ATTENUATORS, TEMPORARY TRAFFIC SIGNALS AND WIDTH RESTRICTION SIGNING.
- 2. REMOVE CONFLICTING STRIPING AND PLACE TEMPORARY STRIPING.
- 3. SHIFT TRAFFIC TO THE EXISTING LANES.
- 4. REMOVE TEMPORARY RAMPS, MILL AND CONSTRUCT THE HMA BINDER AND SURFACE COURSES USING HIGHWAY STANDARD 701306.
- CONSTRUCT HMA AND AGGREGATE SHOULDERS AND PERMANENT GUARDRAIL USING HIGHWAY STANDARD 701326.
- 6. FINISH EARTHWORK AND GRADING AND SEED DISTURBED AREAS.
- 7. PLACE FINAL STRIPING AND CLEANUP.

<u>LEGEND</u>

TO BE REMOVED





STAGE II TYPICAL SECTION

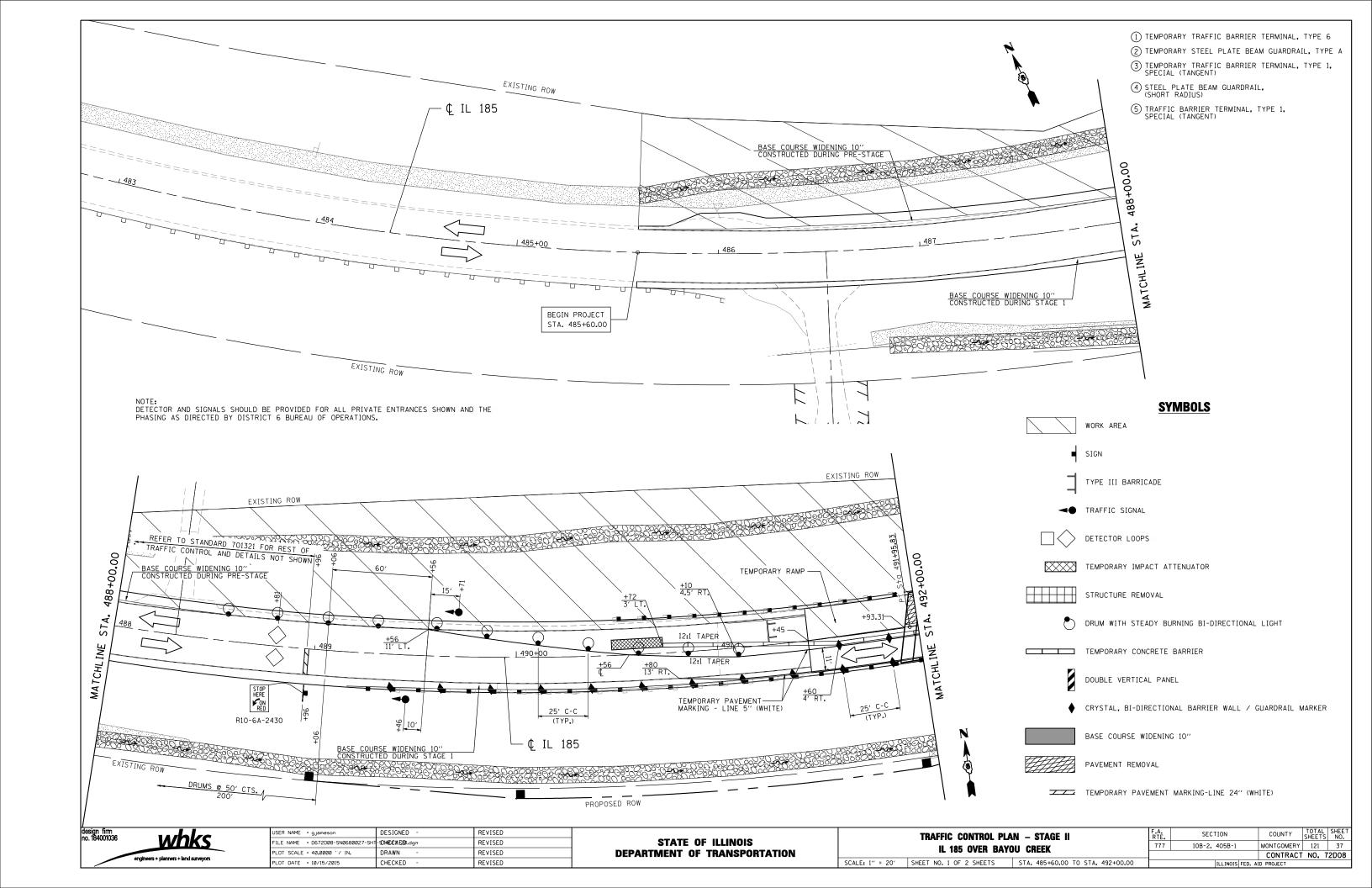
(LOOKING EAST)

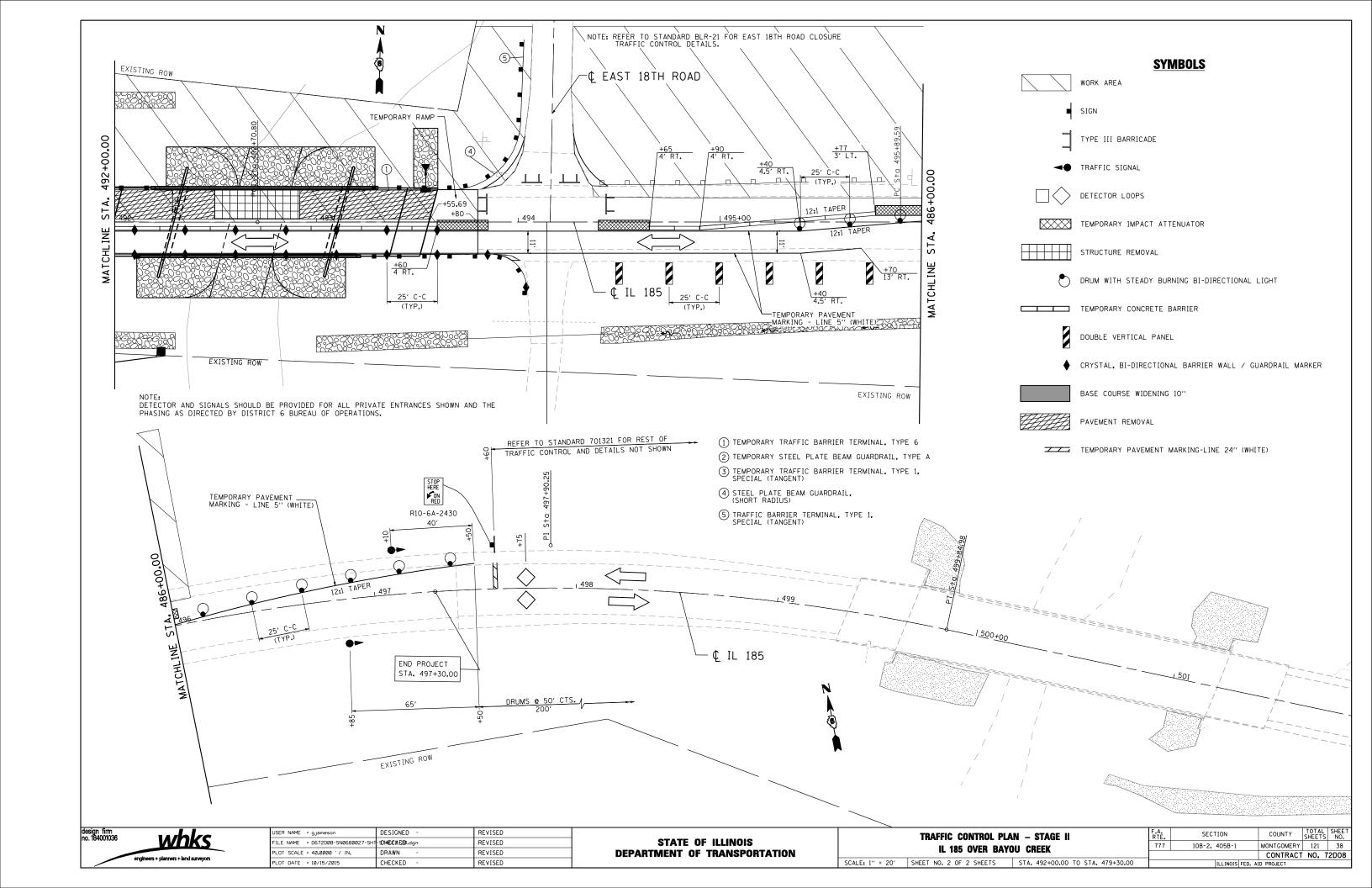


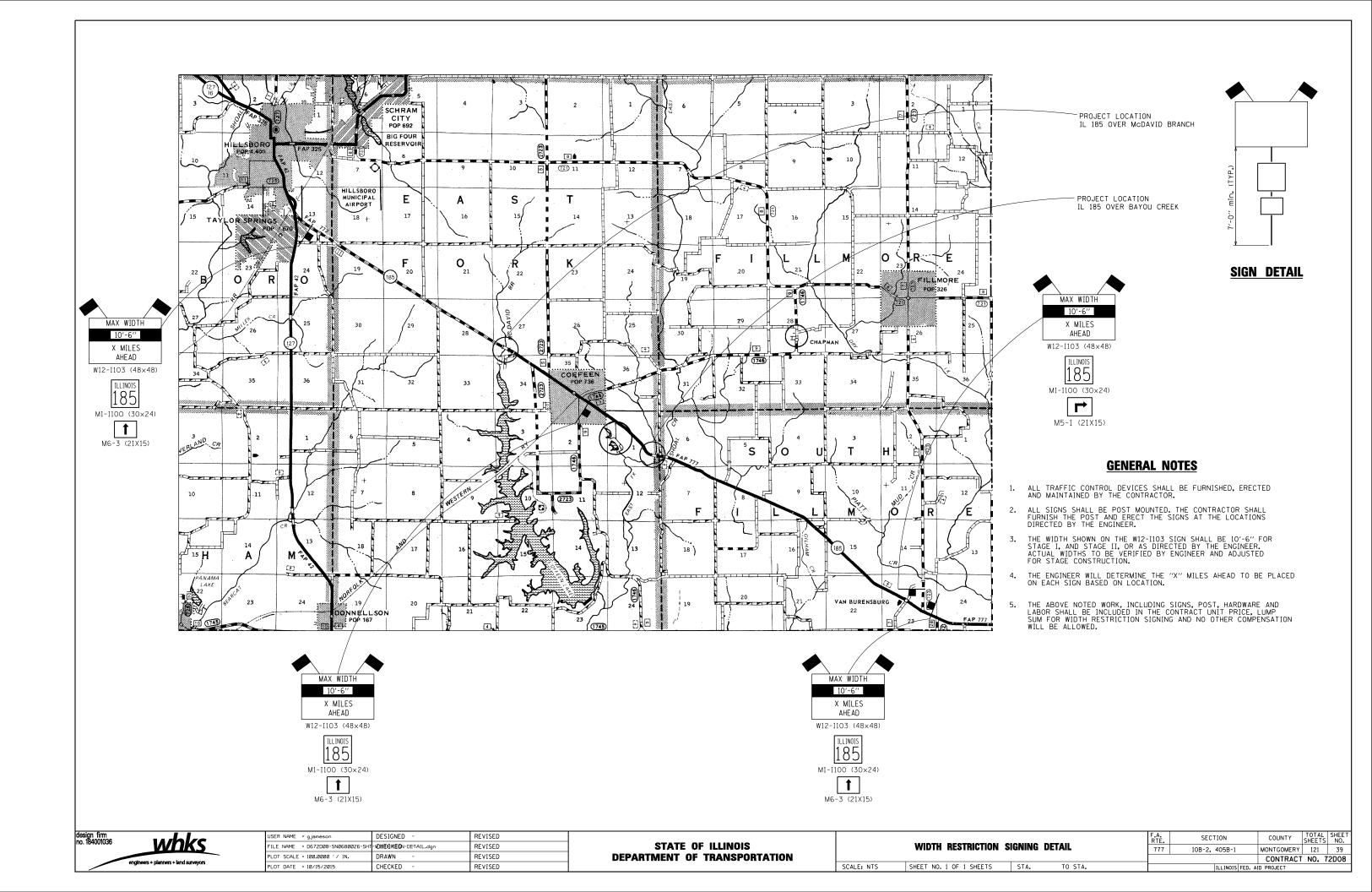
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

	TRAFFIC CONTROL TYPICAL S	SECTIONS -	STAGE II	F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
ı	IL 185 OVER BAYOU CREEK			777	10B-2, 405B-1	MONTGOMERY	121	36
ı						CONTRACT	NO. 7	2D08
	SCALE: 1" = 10' SHEET NO. 1 OF 1 SHEETS	STA.	TO STA.		ILLINOIS FED. AID PROJECT			







STORM WATER POLLUTION PREVENTION PLAN

	Route: FAP 777	Marked: IL 185
	Section: 108-2, 4058-1	Project No.:
McDavid Branch	County: Montgomery	Contract No.: 72D08
Starting Station: Sto. 307+05	(Longitude: 888.099.6781	Latitude: 2.509.385.6341)
Ending Station: Sta. 314+00	(Langitude: 887,690.1471	Latitude: 2,510,187,0605)
Bayou Creek		
Storting Station: Sta. 485+60	(Longitude: 877,800,7960	Lot1tude: 2,524,079,3696)
Ending Station: Sta. 497+30	(Longitude: 887, 660, 1767	Latitude: 2,525.226.1059)

This plan has been prepared to comply with the provision of the NPDES Permit Number ILR10 ______ issued by the illinois Environmental Protection Agency for storm water discharges from construction site activities.

I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel gathered and evaluated the information submitted. Based on my inquire of the person or persons who manage the system, or those persons directly responsible for gathering the Information submitted. Is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.

Note: The above boxed in area will be filled out by IDOT - Construction after the award of the contract to obtain the required NPDES permit.

The following pian was established and included in these plans to direct the Contractor in the placement of temporary erosion control systems and to provide a storm water pollution prevention plan for compliance under NPDES. The Contractor shall abide to all requirements within this plan as part of the contract.

The purpose of this pian is to prevent / minimize slitation within the construction zone and to eliminate sediments from entering and leaving the construction zone by utilizing proper temporary erosion control systems and providing ground cover within a reasonable time.

Certain items, as shown in this pion and referenced by the legend, shall be placed by the Contractor at the beginning of construction. Other items shall be placed by the Contractor as directed by the Engineer on a case by case situation resulting from the Contractor's sequence of activities, time of the year, and expected weather conditions.

The Contractor shall place permanent erosion control systems and seeding within a reasonable amount of time; therefore, reducing the amount of area being open to the possibility of erosion and reducing the amount of temporary erosion control systems and temporary seeding. The Resident Engineer will determine if temporary erosion control systems shown in the plan can be deleted, the size of the proposed ditch checks, the proper method of installation, and if any additional temporary erosion control systems shall be added which are not included in this plan. The Contractor shall perform all work as directed by the Engineer and as shown in special details and in Standard 280001 of the plans.

All disturbed areas having high potential for erosion, as determined by the Engineer, shall be temporarily seeded or permanently seeded by October 1st of each construction year and shall not be reopened until after the winter shutdown period.

SITE DESCRIPTION

- Description of Construction Activity:

 1. The proposed project consists of the removal and replacement of the bridge carrying IL 185 ever McDavid Branch, approximately 0.5 miles east of Coffeen and the bridge carrying IL 185 ever Bayou Creek approximately 1.0 miles west of Coffeen in Montgomery County. The project will be constructed on the existing alignment and will include reconstruction and resurfacing of approximately 0.38 miles of IL 185.
- 2. Additional construction activities consist of HMA bose course widening, HMA resurfacing, HMA and aggregate shoulders, ditch grading, embonkment, topsoil placement, channel grading and other miscellaneous work to complete improvements to the proposed roadways.

Description of Intended Sequence of Major Construction Activities Which Will Disturb Earth and Lead to Possible Erosion for Major Portions of the Construction Site:

- 1. Tree removal will be completed to clear approximately 0.2 acres of wooded land.
- Excavation will be completed along the entire length of the project to grade out for proposed roadway ditches and waterways.
- Embankment will be completed in fill areas to raise the existing ground elevation to meet the proposed roadway foreslopes and backslopes.
- 4. Oralnage structures will be installed before and/or during the construction of the excavation and embankment to allow proper drainage along IL 185.
- Placement, maintenance, removal and proper clean-up of temporary erosion control items, such as erosion control fence, temporary ditch checks and temporary seeding.
- Placement of permanent erosion control items, such as riprop ditch lining, erosion control blanket, mulch method II and seeding.
- 7. Final grading, paving and other miscellaneous Items.

SCALE: NTS

Area of Construction Site:

McDavid Branch
The total drainage area entering and including the construction site, at McDavid Branch, is estimated to be approximately 4.8 square miles in which 0.8 acres will be disturbed by excavation, grading or other activities.

<u>Bayou Creek</u>
The total drainage area entering and including the construction site, at Bayou Creek, is estimated to be approximately 5.54 square miles. In which 1.7 acres will be disturbed by excavation, grading or other activities.

Other Reports. Studies and Plans which Aid in the Development of this Storm Water Pollution Prevention Plan as Referenced Documents:

- Estimated run-off coefficients are contained in the project drainage study which were utilized for proposed placement of the temporary erosion control systems.
- Information on the soils within the site was obtained from field reviews which were utilized for proposed placement of the temporary erosion control systems.
- Site maps indicating drainage patterns and approximate slopes were contained in the project design report, USOS drainage maps, project drainage study and project plan documents were all utilized for proposed placement of the temporary erosian control systems.

Drainage Tributaries Receiving Water from this Construction Site:
The receiving waters are McDavid Branch and Bayou Creek. McDavid Branch flows into Coffeen Lake and into the East Fork Shoai Creek. Bayou Creek flows into the East Fork Shoai Creek, then through Shoai Creek and the Kaskoskia River and eventually into the Mississippi River.



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STORM WATER POLLUTION		F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
PREVENTION PLAN		777	10B-2, 405B-1	MONTCOMERY	121	40
LUTATION LTWIA				CONTRACT	NO. 7	2008
SHEET NO. 1 OF 4 SHEETS STA.	O STA.		ILLINOIS FED. A	D PROJECT		

CONTROLS - EROSION CONTROLS AND SEDIMENT CONTROLS

Description of Stabilization Practices at the Beginning of Construction:

- . The area between the existing and proposed right-of-way/temporary easement boundaries and limits of the project will be improved and managed for the purposes of controlling erosion within the area, reducing water flow by temporary diversion and minimizing siltation into the construction zone, and establishing vegetative cover which will become permanent vegetation and act as an erosion barrier. Work at the beginning of construction will consist of the following:
 - (a) Areas of existing vegetation (woods and grasslands) outside the proposed construction slope limits shall be identified for preserving and shall be protected from mowing, brush cutting, tree removal and other activities which would be detrimental to their maintenance and development.
 - (b) Dead, diseased, or unsuitable vegetation within the site shall be removed as directed by the Engineer, along with required tree removal.
 - (c) As soon as reasonable access is available (such as trees cleared) to all locations where water drains away from the project, sediment basins, riprap ditch checks, temporary ditch checks, and/or erosion control fence shall be installed as called out in this plan and directed by the Engineer.
 - (d) Bare and sparsely vegetated ground in highly erodable areas as determined by the Engineer shall be temporarily seeded at the beginning of construction where no construction activities are immediately expected as stated in the special provision "Temporary Erosion and Sediment Control".
 - (e) Immediately after tree removal is completed in certain areas which are highly erodable areas as determined by the Engineer, the areas shall be temporarily seeded where no construction activities are immediately expected as stated in the special provision "Temporary Erosion and Sediment Control".
 - (f) At locations where a significant amount of water drains into the construction zone from outside areas (adjacent landowners), erosion control fence, temporary ditch checks, or riprap ditch checks will be utilized to locally divert water, reduce flow rates, and collect outside siltation inside the right-of-way line. Erosion control items will not be allowed to be installed to cause flooding to upstream private property which could cause crop damages or other undesireable conditions.
- Establishment of these temporary erosion control measures will have additional benefits to the project. Desirable grass seed will become established in these areas and will spread seeds onto the construction site until permanent seeding/mowing and overseeding can be complete.
- 3. A third benefit of these filter areas is that they will begin to provide a screen and buffer. They will help protect the construction site from winds and excess sun and mitigate construction noise and dust.

Description of Stabilization Practices During Construction:

- During roadway construction, areas outside the construction slope limits as outlined
 previous herein shall be protected from damaging effects of construction. The Contractor
 shall not use this area for staging (except as designated on the plans or directed by the
 Engineer), parking of vehicles or construction equipment, storage of materials, or other
 construction related activities.
 - (a) Within the construction zone, critical areas which have high flows of water as determined by the Engineer shall remain undisturbed until full scale construction is underway to prevent unnecessary soil erosion.
 - (b) Top soil and earth stockpiles shall be temporarily seeded if they are to remain unused for more than fourteen days.
 - (c) As the Contractor constructs a portion of roadway in a fill section, he/she shall follow the following steps as directed by the Engineer:
 - i. Place temporary erosion control systems at locations where water leaves and enters the construction zone ${}^{\circ}$
 - ii. Temporary seed highly erodable areas outside the construction slope limits
 - iii. Construct roadside ditches and provide temporary erosion control systems
 - iv. Temporary divert water around proposed culvert locations
 - v. Build necessary embankment at culvert locations and then excavate and place culvert vi. Continue building up the embankment to the proposed grade while at the same time place permanent erosion control such as riprap ditch lining and conduct final shaping to the slopes
 - (d) The Contractor shall immediately follow major earth moving operations with final grading equipment. After the major earth spread operation has moved to a new location, final grading shall be completed within fourteen days. If grading is not completed within fourteen days, all major earth moving operations will be stopped, as directed by the Engineer, until disturbed areas are final graded and seeded.
 - (e) Excavated areas and embankments shall be permanently seeded when final graded. If not, they shall be temporarily seeded as stated in the special provision "Temporary Erosion and Sediment Control.

- (f) Construction equipment shall be stored and fueled only at designated locations. All necessary measures shall be taken to contain any fuel or pollution run-off in compliance with EPA water quality regulations. Leaking equipment or supplies shall be immediately repaired or removed from the site.
- (g) Qualified Personnel shall inspect the project at least every seven days and within 24 hours of the end of a storm that is 0.5 inch or greater as noted in BDE 2342.
- (h) Sediment collected during construction by the various temporary erosion control systems shall be disposed of on the site on a regular basis as directed by the Engineer.
- (i) The temporary erosion control systems shall be removed as directed by the Engineer after use is no longer needed or no longer functioning. The costs of this removal shall be included in the unit bid price for the various temporary erosion control pay items. No additional additional compensation will be allowed.

Description of Structural Practices After Final Grading:

- Temporary erosion control systems shall be left in place with proper maintenance until
 permanent erosion control is in place and working properly and all proposed turf areas
 seeded and established with a proper stand.
- Once permanent erosion control systems as proposed in the plans are functional and established, temporary items shall be removed, cleaned up, and disturbed turf reseeded. Temporary riprap ditch checks will be allowed to remain in place where approved by the Engineer.

Maintenance after Construction:

- 1. Construction is complete after acceptance is received at the final inspection.
- 2. Areas will be inspected on a regular basis by IDOT District 6 Bureau of Operations.
- Maintenance crews will perform regular mowings to aid in keeping weeds down and establishing a good roadside seed stand.
- Maintenance crews will also aid in any ditch lining maintenance or in any drainage problems.
- 5. All maintenance will be conducted at times when weather conditions will not cause site damage.

OCUMENTATION

- 1. A report summarizing the scope of the inspection, name(s) and qualifications of personnel making the inspection, date(s) of the inspection, major observations relating to the implementation of this storm water pollution prevention plan, and actions taken in accordance with Section 4.b. shall be made and retained as part of the plan for at least three years after the date of inspection. The report shall be signed in accordance with part VI.G of the general permit.
- 2. If any violation of the provisions of this plan is identified during the conduct of the construction work covered by this plan, the Resident Engineer or Resident Technician shall complete and file an "Incident of Noncompliance (ION)" report for the identified violation. The Resident Engineer or Resident Technician shall use forms provided by the Illinois Environmental Protection Agency and shall include specific information on the noncompliance, actions which were taken to prevent any further causes of noncompliance, and a statement detailing any environmental impact which may have resulted from the noncompliance. All reports of noncompliance shall be signed by a responsible authority in accordance with Part VI.G. of the general permit. The report of noncompliance shall be mailed to the following address:

Illinois Environmental Protection Agency Division of Water Pollution Control 2200 Churchill Road, P.O. Box 19276 Springfield, IL 62794-9276 Attn: Compliance Assurance Section

SCALE: NTS

design firm no. 184001036



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STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

STORM WATER P	OLLUTION		F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
PREVENTION PLAN		777	10B-2, 405B-1	MONTGOMERY	121	41	
I ILVEITION	LAN				CONTRACT	NO. 7	2D08
SHEET NO. 2 OF 4 SHEETS	STA.	TO STA.		ILLINOIS FED. A	ID PROJECT		

CONTRACTOR CERTIFICATION STATEMENT

	oute:	Marked:
S	ection:	Project No.:
С	ounty:	Contract No.:
ssociated wit n addition, I or the above	h industrial activity from the have read and understand all mentioned project; I have rece	nat authorizes the storm water discharges construction site identified as part of this certification of the information and requirements stated in the SWPPP wed copies of all appropriate maintenance procedures; and, be in compliance with the Permit ILR10 and SWPPP and will
ssociated with addition, I por the above have provide timely	h industrial activity from the have read and understand all mentioned project; I have rece d all documentation required to updates to these documents as	construction site identified as part of this certificatio f the information and requirements stated in the SWPPP ved copies of all appropriate maintenance procedures; and be in compliance with the Permit ILR10 and SWPPP and wilnecessary.
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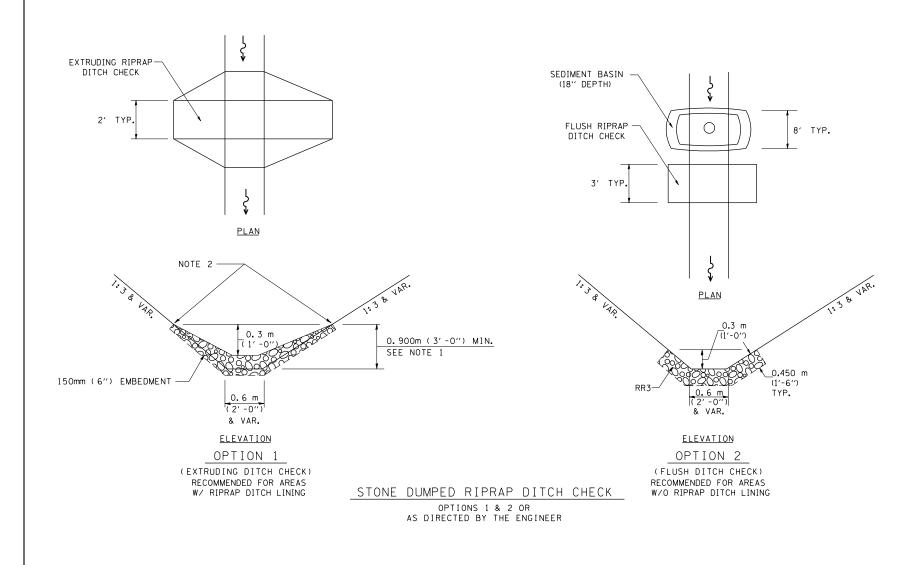
Note: The above boxed in area shall be filled out by the Contractor after the award of the contract to obtain the required NPDES Permit from IEPA. This is a requirement for this contract.

sign firm 184001036	wh

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VVIIAS
engineers + planners + land surveyors

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T				F.A.P.	SECTION	COUNTY	TOTAL	SHEET
STORM	1 WATER F	OLLUTION		RTE.	SECTION	COUNTT	SHEETS	NO.
Dr	REVENTION	DI AN		777	10B-2, 405B-1	MONTGOMERY	121	42
	MEA FIAI I OIA	LEAN				CONTRACT	NO. 7	2D08
SCALE: NTS SHEET NO. 3 OF	4 SHEETS	STA.	TO STA.		ILLINOIS FED. A	ID PROJECT		



NOTE 1: RIPRAP SHALL EXTEND FAR ENOUGH UP THE SLOPES TO ALLOW O.3m (1') OVERTOPPING TO AVOID ERODING AROUND THE EDGES OF THE RIPRAP.

NOTE 2: ENDS SHALL BE TIED INTO SLOPES.

LEGEND FOR STORM WATER POLLUTION PREVENTION PLAN

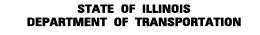
ITEM	SYMBOL
AGGREGATE DITCH CHECKS	—
INLET PIPE PROTECTION	\Leftrightarrow
PERIMETER EROSION BARRIER	
SEDIMENT BASINS EARTH EXCAVATION FOR EROSION CONTROL AGGREGATE (EROSION CONTROL)	(o)
PRESERVE EXISTING TREES, WOODLANDS, AND UNDERSTORY (OUTSIDE CONSTRUCTION LIMITS)	
ITEM PLACED AT BEGINNING OF CONSTRUCTION (Requirement)	* [ITEM] *
ITEM PLACED AS DIRECTED BY ENGINEER (When required by situation)	ITEM 4
DIRECTION OF OVERLAND FLOW	

GENERAL NOTES:

All items shall be constructed as shown on this sheet, on Standard 280001, and as directed by the Engineer.

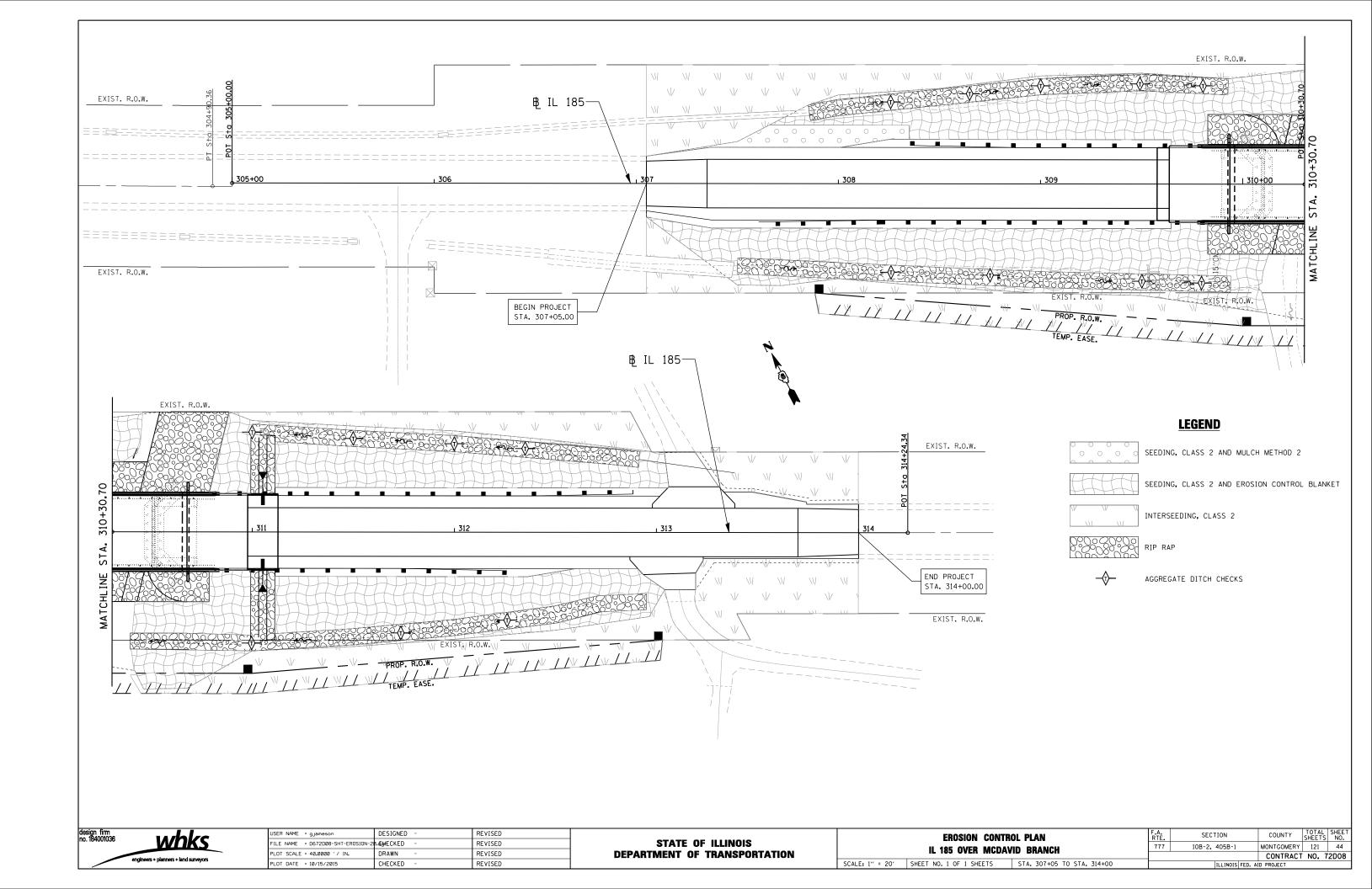


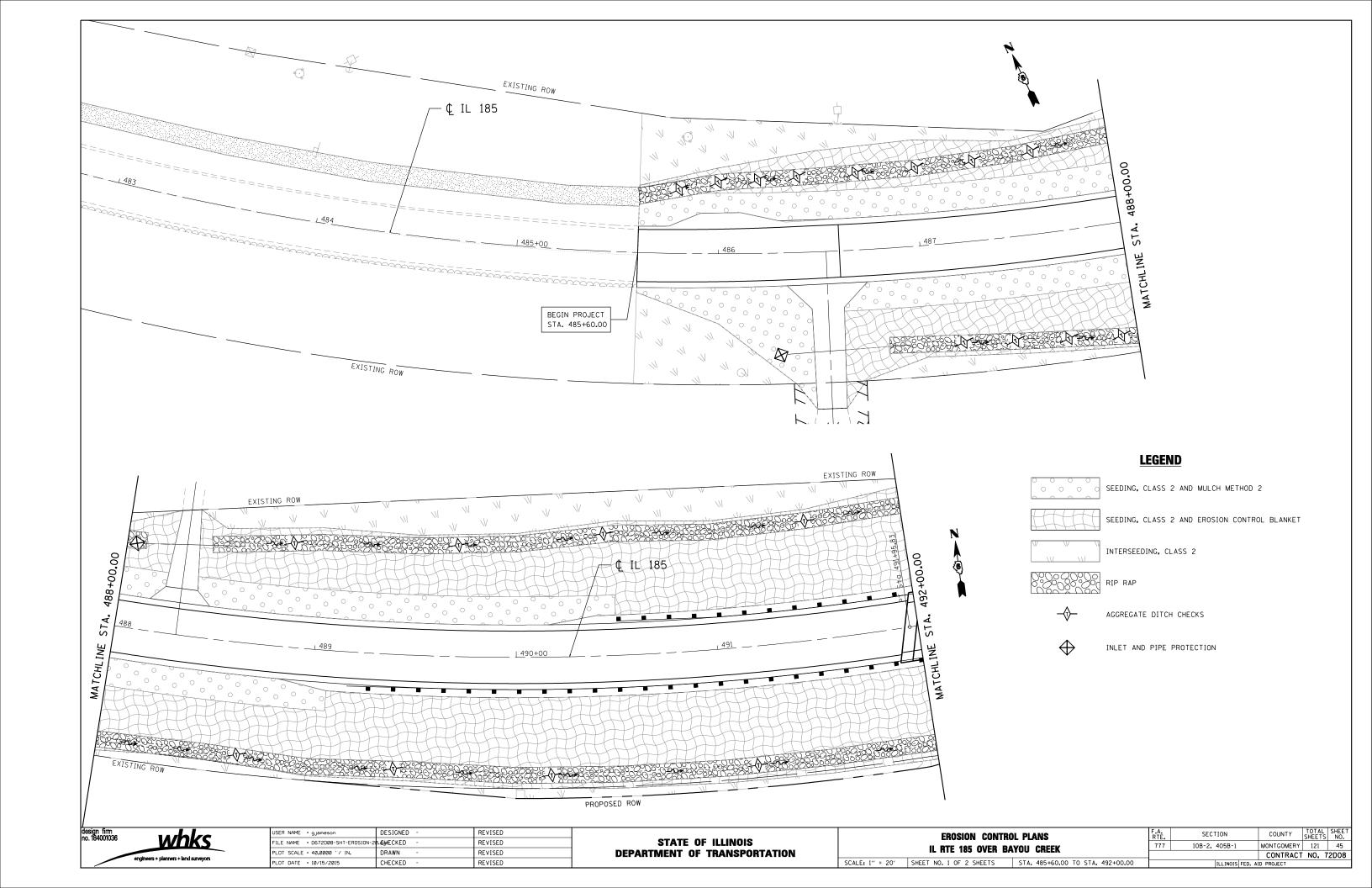
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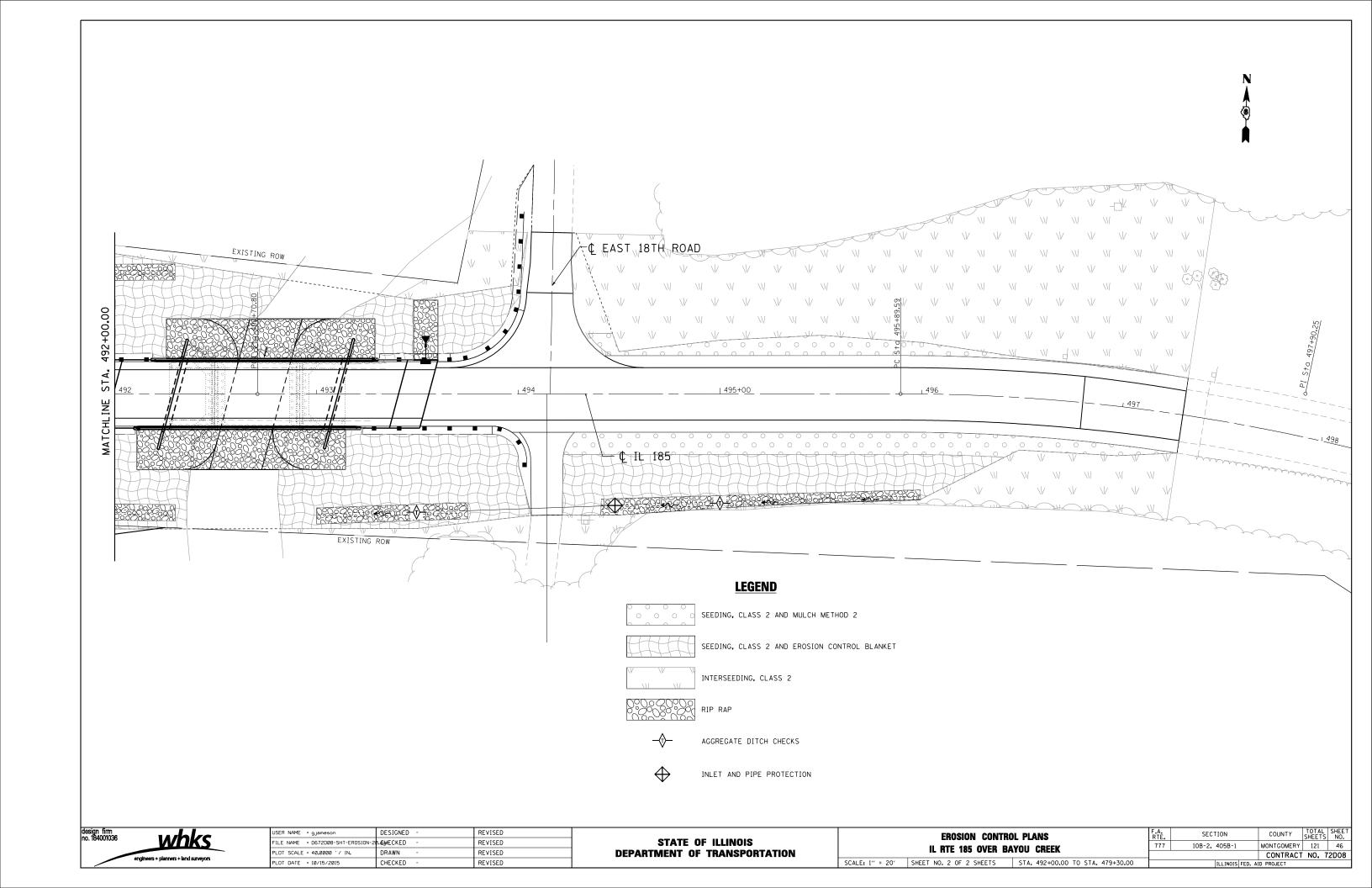


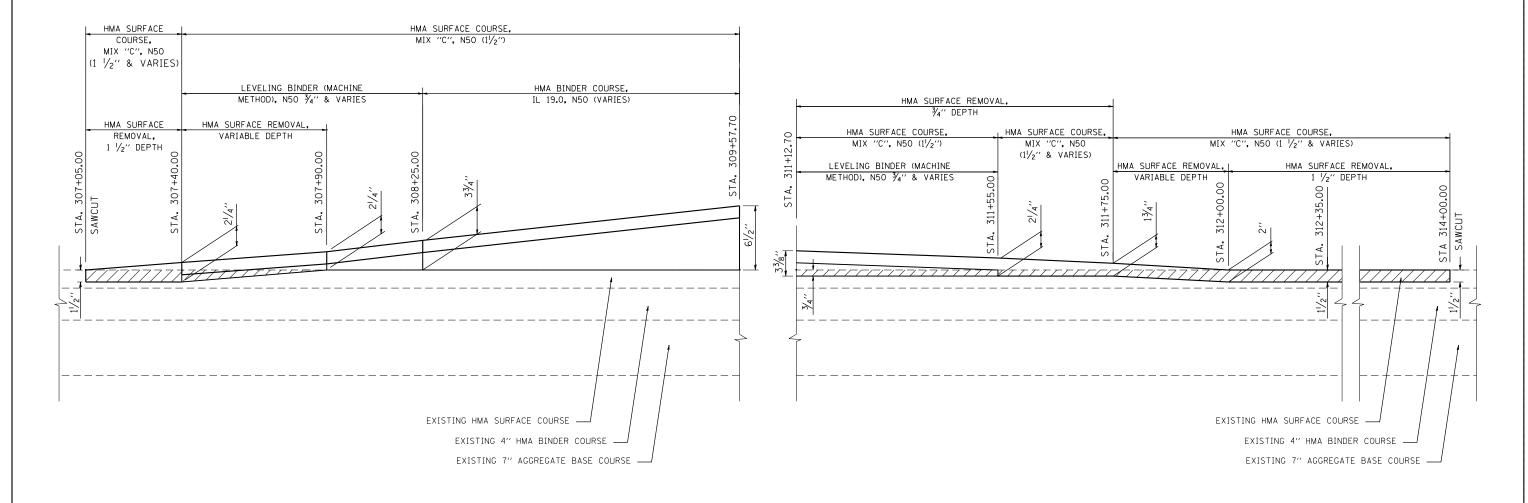
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STORM WATER POLLUTION PREVENTION PLAN	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS			
		777	10B-2, 405B-1	MONTGOMERY	121	43	
PREVENTION PLAN					CONTRACT	NO. 7	2D08
SHEET NO. 4 OF 4 SHEETS	STA.	TO STA.		ILLINOIS FED. A	ID PROJECT		









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STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

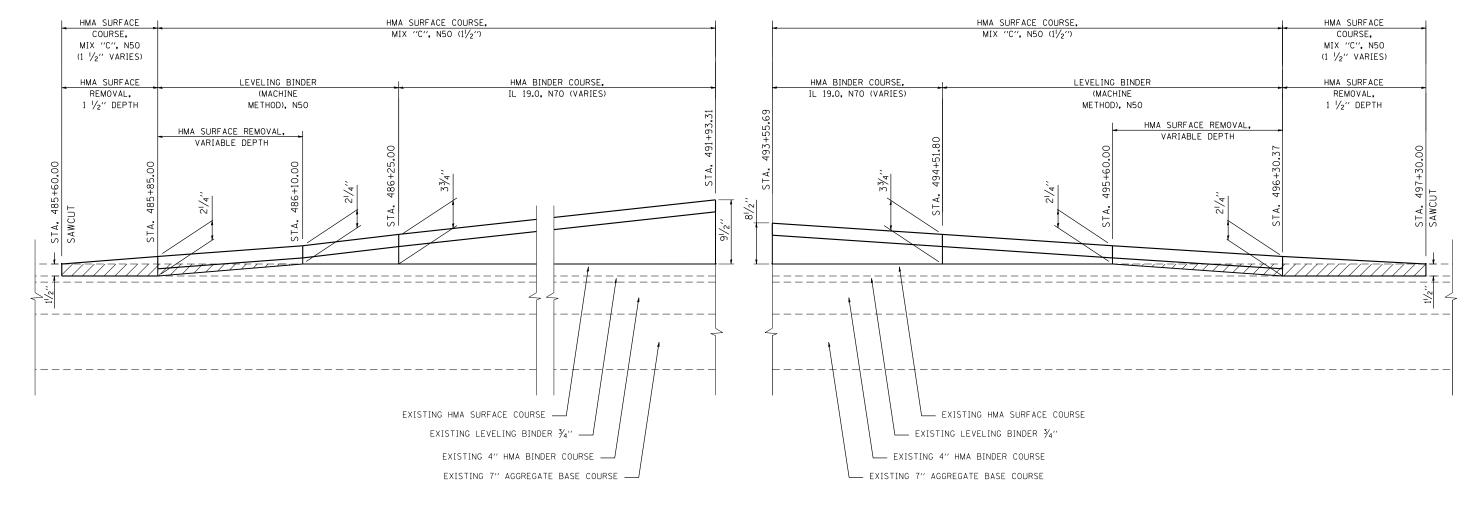
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A.P. SECTION COUNTY TOTAL SHEETS NO.
7777 10B-2, 405B-1 MONTGOMERY 121 47

CONTRACT NO. 72D08

| ILLINOIS FED. AID PROJECT



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PAVEMENT DETAIL

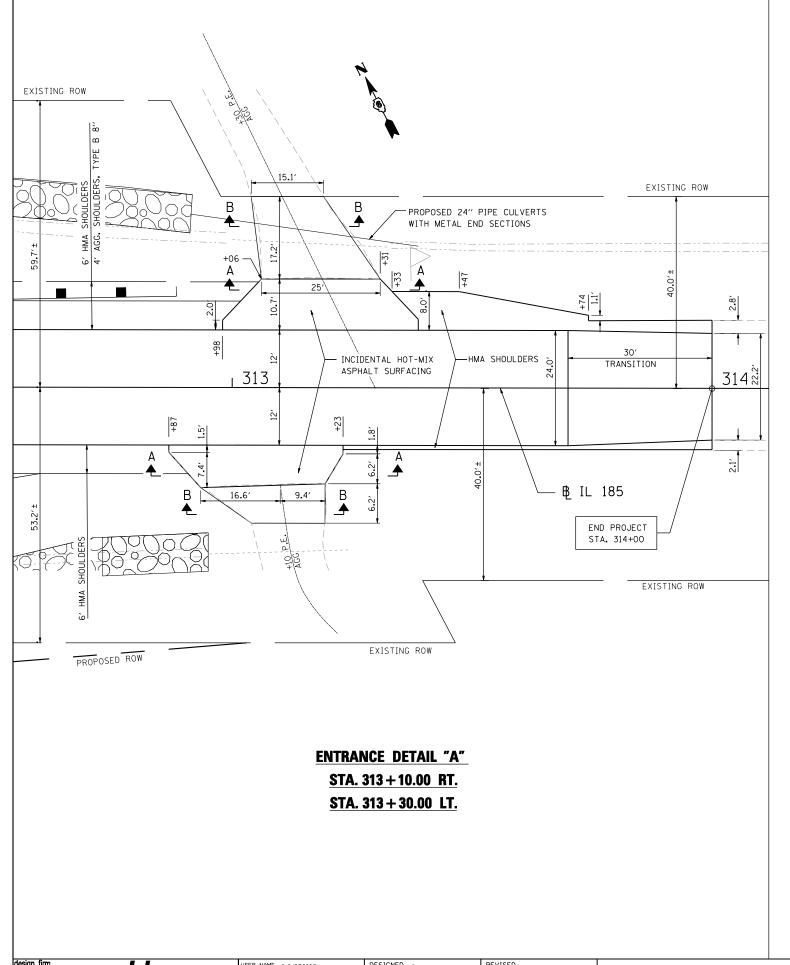
STA. 493+55.69 TO STA. 497+30.00

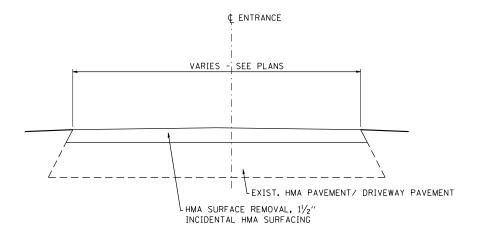
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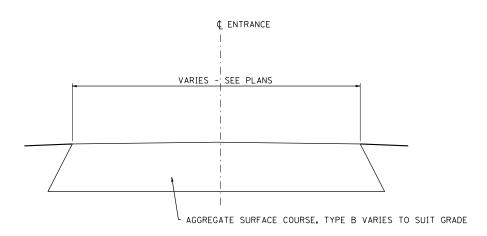
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

	F.A.P. RTE.	SECTION		
	777	10B-2, 405		
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TYPICAL SECTION THRU ENTRANCE SECTION A – A



TYPICAL SECTION THRU ENTRANCE SECTION B - B

IOTES:

THE RESIDENT ENGINEER WILL DETERMINE THE EXACT LAYOUT FOR ALL ENTRANCES, AND MAILBOX TURNOUTS ON THIS PROJECT.

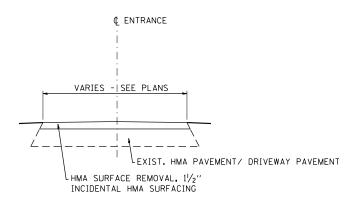
THIS WORK WILL BE PAID FOR AT THE CONTRACT UNIT PRICE TON FOR INCIDENTAL BITUMINOUS SURFACING WHICH SHALL INCLUDE ALL MATERIALS, EQUIPMENT, AND

ANY WORK THE ENGINEER REQUIRES WHICH IS NOT COVERED BY A PAY ITEM CONTAINED IN THE PLANS WILL BE PAID FOR IN ACCORDANCE WITH ARTICLE 109:04 OF THE STANDARD SPECIFICATIONS.

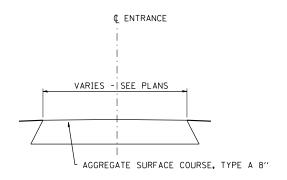


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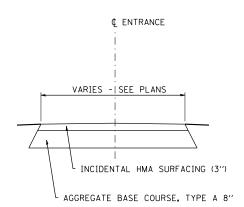
	ENTRANCE DETAILS			F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	
	IL 185 OVER MCDAVID BRANCH				10B-2, 405B-1	MONTGOMERY	121	49
	IL 103 OTEN MODATIO DILATON					CONTRACT	NO. 7	72D08
SCALE: 1" = 10'	SHEET NO. 1 OF 1 SHEETS	STA.	TO STA.		ILLINOIS FED. A	ID PROJECT		



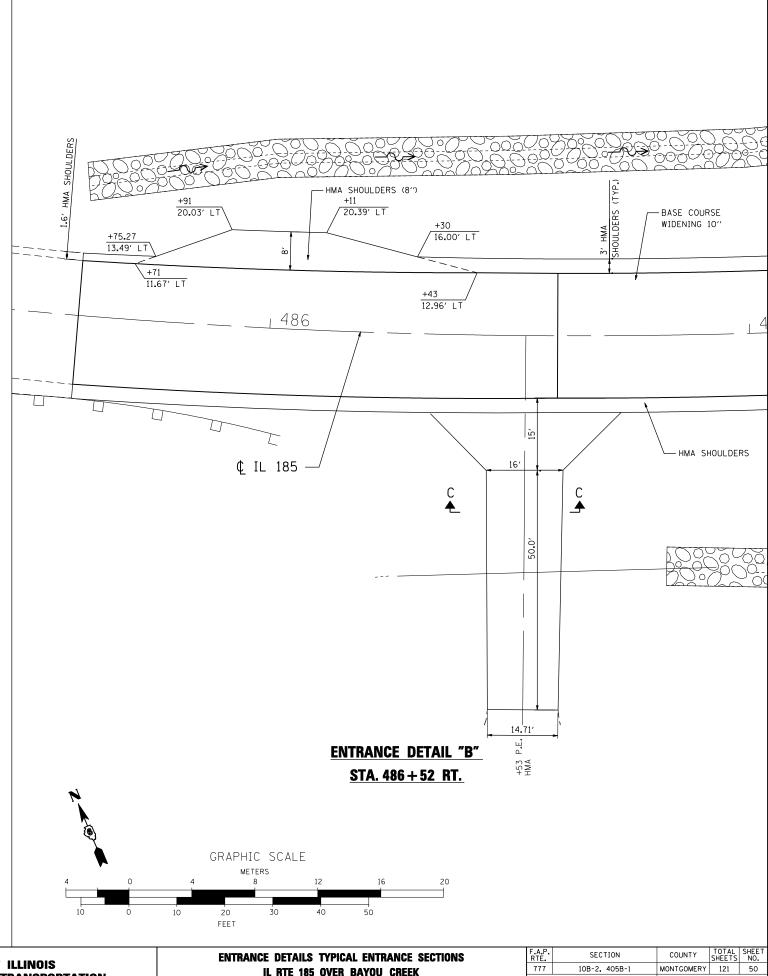
TYPICAL SECTION THRU ENTRANCE SECTION A - A



TYPICAL SECTION THRU ENTRANCE SECTION B - B



TYPICAL SECTION THRU ENTRANCE SECTION C - C



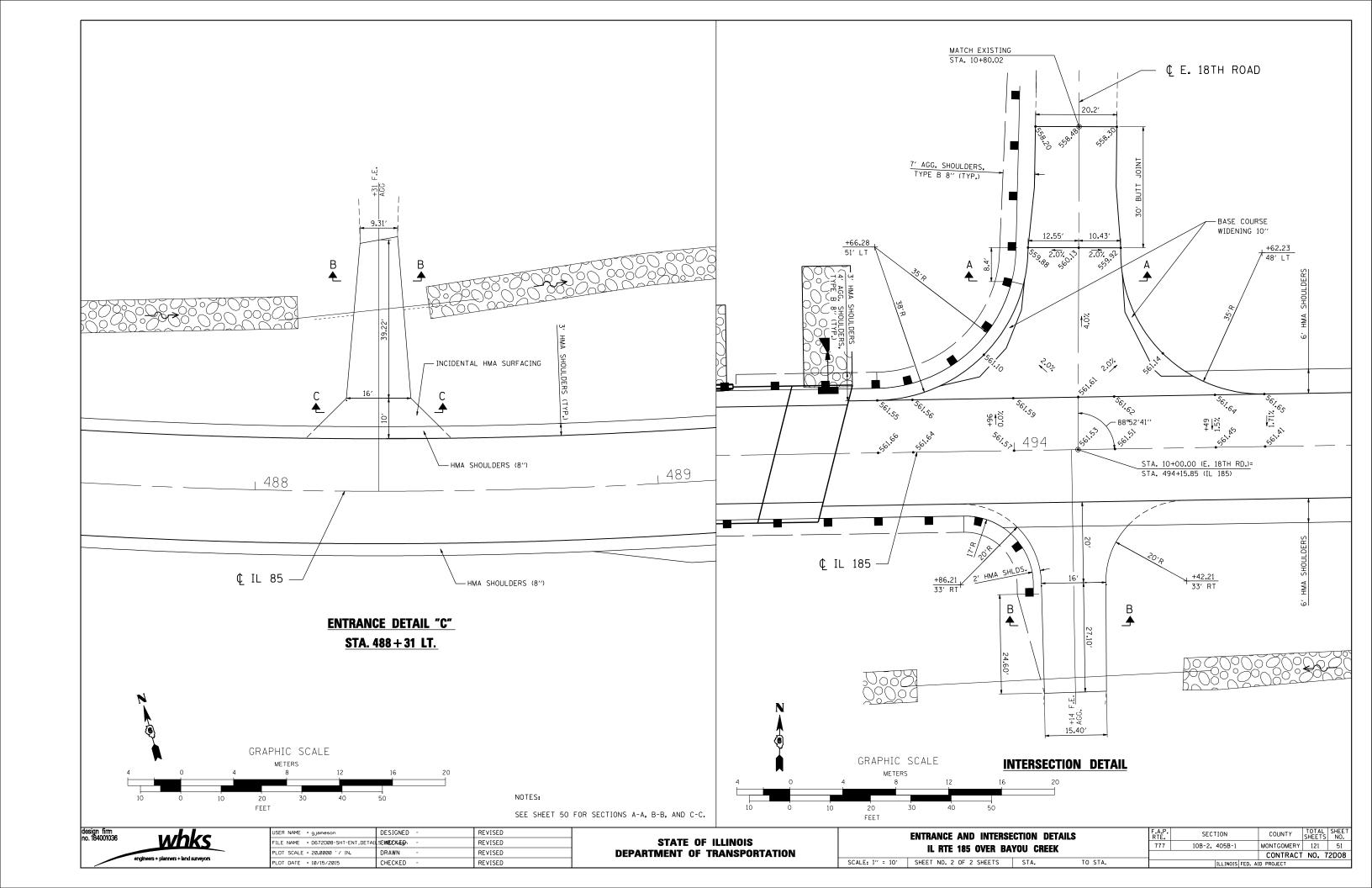


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STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

10B-2, 405B-1 IL RTE 185 OVER BAYOU CREEK SCALE: 1" = 10' SHEET NO. 1 OF 2 SHEETS STA.

CONTRACT NO. 72D08



NOTES

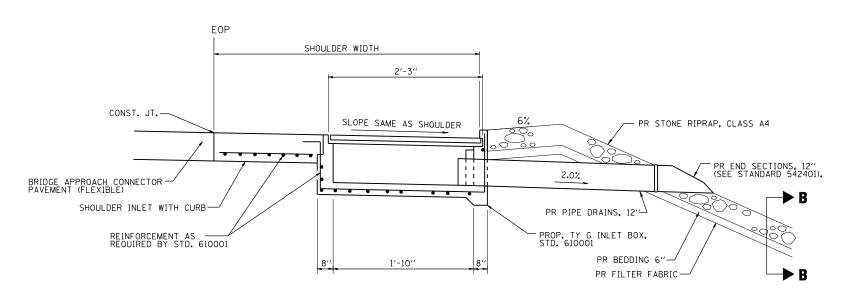
CONSTRUCTION SHALL BE ACCORDING TO STANDARDS 610001 AND 420401 EXCEPT AS MODIFIED.

INLET SHALL BE PLACED SUCH THAT THE OUTLET PIPE WILL MISS THE POST OF THE PROPOSED TYPE 6 TRAFFIC BARRIER TERMINAL. MINOR ADJUSTMENTS TO THE LENGTH AND QUANTITY OF THE CONNECTOR PAVEMENT MAY BE REQUIRED.

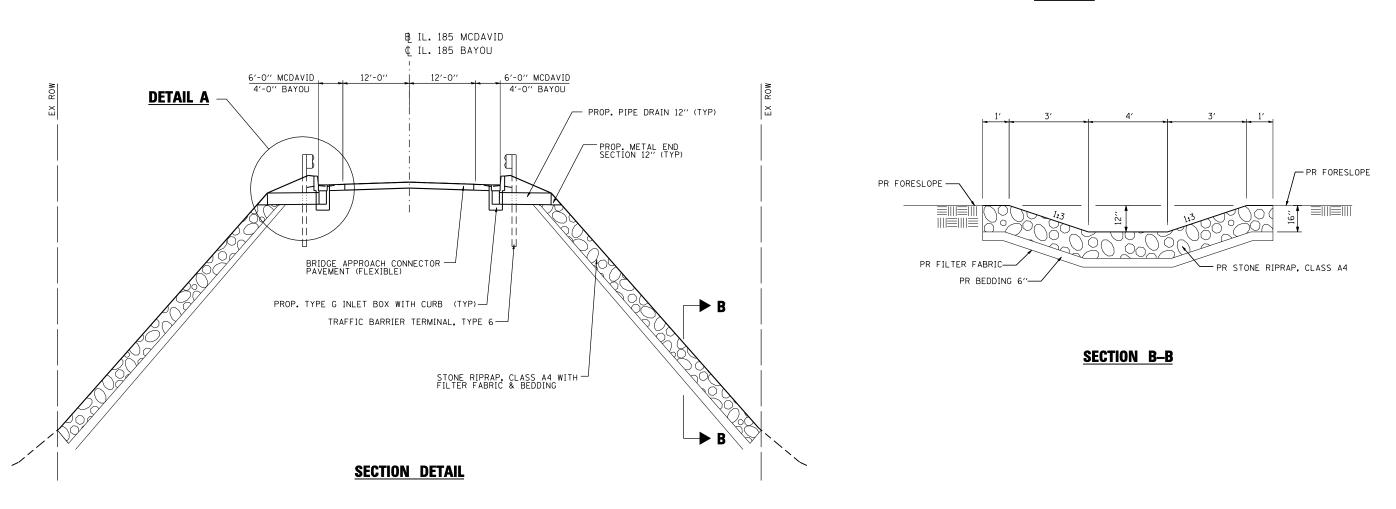
THE ANCHOR BOLTS REQUIRED TO THE THE PIPE DRAIN TO THE CONCRETE COLLAR ACCORDING TO STD. 610001 SHALL BE CONSIDERED INCLUDED IN THE CONTRACT UNIT PRICE, EACH, FOR TYPE G INLET BOX, STD 610001 AND NO ADDITIONAL COMPENSATION WILL BE MADE.

THE REINFORCING AND TIE BARS REQUIRED ACCORDING TO STD. 610001 SHALL BE CONSIDERED INCLUDED IN THE CONTRACT UNIT PRICE, EACH, FOR TYPE G INLET BOX, STD 610001 AND NO ADDITIONAL COMPENSATION WILL BE MADE.

EXCAVATION REQUIRED TO CONSTRUCT THE RIPRAP SWALE AS SHOWN SHALL BE CONSIDERED AS INCLUDED IN THE COST OF RIPRAP, CLASS A4.



DETAIL A



whks

USER NAME	= gjameson	DESIGNED	-	REVISED
FILE NAME	= D672D08-SHT-BOX DETAI	66∯ECKED	-	REVISED
PLOT SCALE	= 10.0000 ' / IN.	DRAWN	-	REVISED
PLOT DATE	= 10/15/2015	CHECKED	-	REVISED

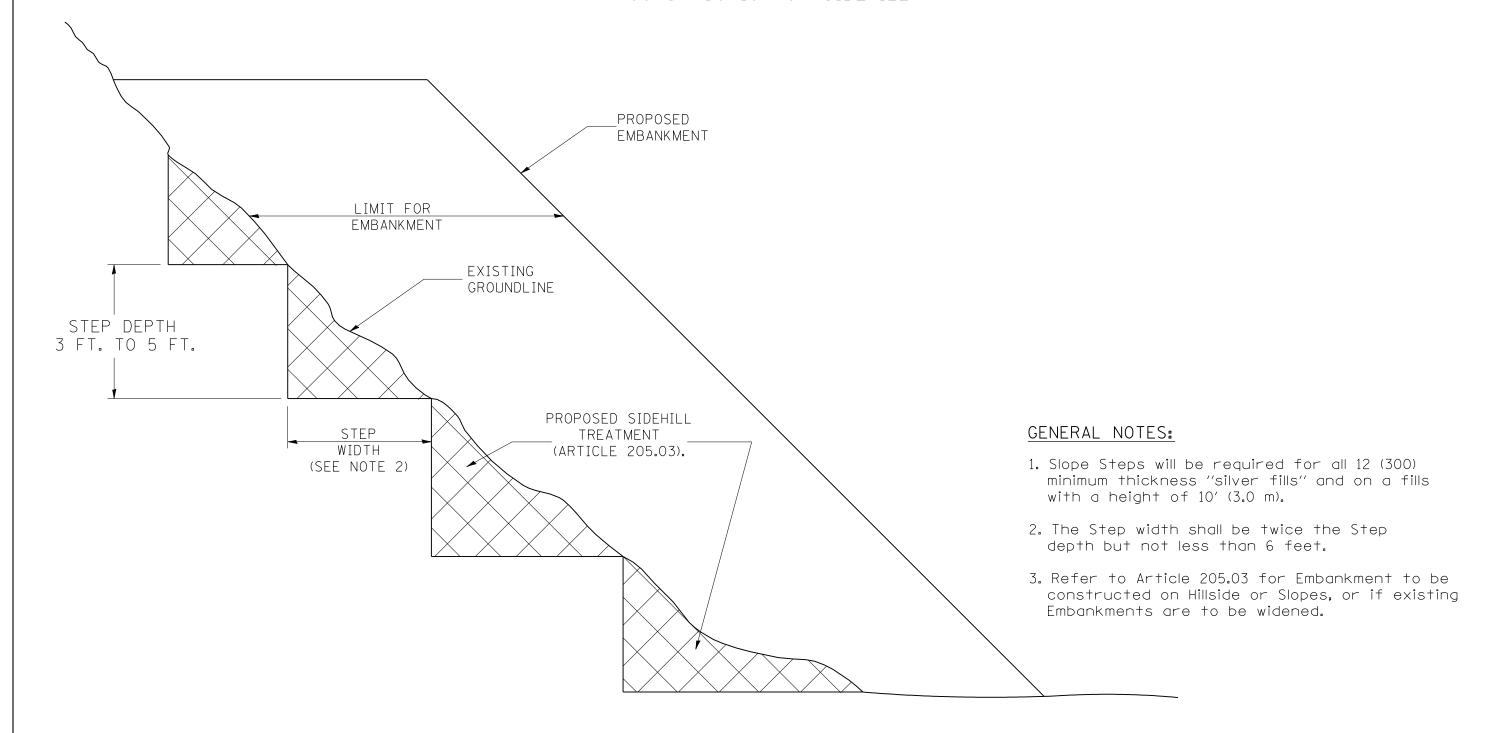
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SCALE: 1" = 10"

	TYPE G INLET BOX DETAILS			F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
				777	10B-2, 405B-1	MONTGOMERY	121	52
						CONTRACT	NO. 7	2D08
	SHEET NO. 1 OF 1 SHEETS	STA.	TO STA.		ILLINOIS FED. A	ID PROJECT		

SLOPE STEPS DETAIL

TYPICAL CROSS-SECTION EMBANKMENT CONSTRUCTION ON SIDEHILL



REPLACEMENT MATERIAL:



STANDARD EMBANKMENT (IN ACCORDANCE WITH 205 OF THE STANDARD SPECIFACATION).

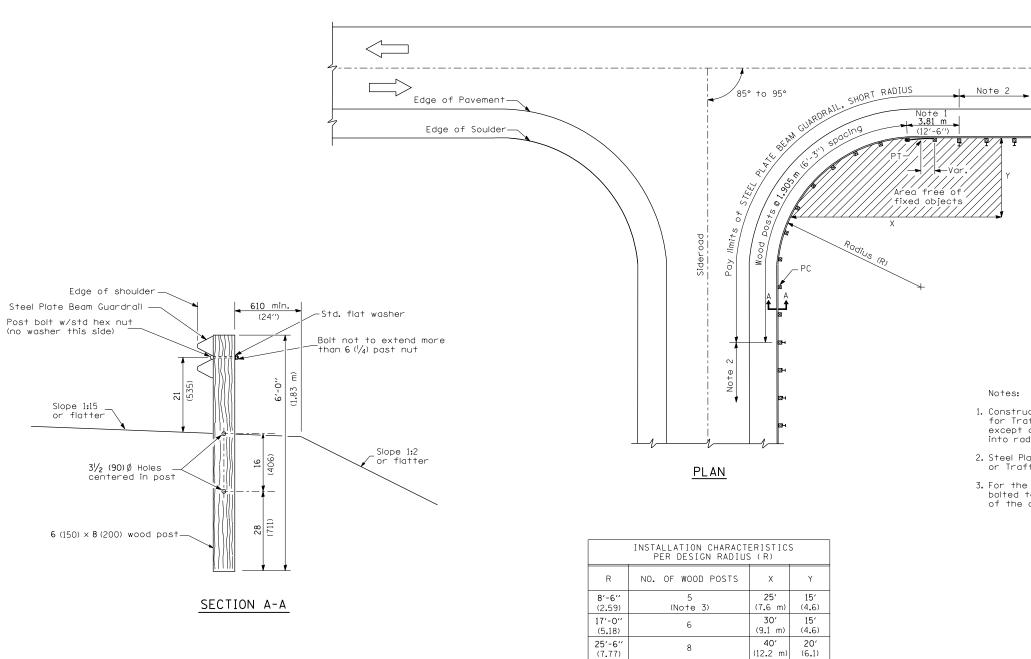
All dimensions are in inches (millimeters) unless otherwise noted.



USER NAME = gjameson	DESIGNED -	REVISED
FILE NAME = D672D08-SN0680026-SHT	-16HECKE8gn -	REVISED
PLOT SCALE = 40.000 '/ IN.	DRAWN -	REVISED
PLOT DATE = 10/15/2015	CHECKED -	REVISED

STATI	OF ILLINOIS	
DEPARTMENT	OF TRANSPORTATION	DN

SLOPE STEPS DETAIL				F.A.P. RTE.	SECTION	COUNTY	TOTAL	SHEET NO.
					10B-2, 405B-1	MONTGOMERY	121	53
						CONTRACT	NO.	72D08
SCALE: NTS	SHEET NO. 1 OF 3 SHEETS	STA.	TO STA.		ILLINOIS FED. A	ID PROJECT		



Notes:

- Construct according to Standard 631011 for Traffic Barrier Terminal Type 2, except delete end section and splice into radius guardrail.
- 2. Steel Plate Beam Guardrail Type A, Type B, or Traffic Barrier Terminal as specified.
- 3. For the 8'-6" (2.59 m) radius, the rail is not bolted to the post located at the midpoint of the curve.

GENERAL NOTES

All slope ratios are expressed as units of vertical displacement to units of horizontal displacement (V:H).

All dimensions are in inches (millimeters) unless otherwise shown.

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USER NAME = gjameson DESIGNED - REVISE	D
FILE NAME = D672D08-SN0680026-SHT-CEHECKEBgn - REVISE	D
PLOT SCALE = 40.000 '/ IN. DRAWN - REVISE	D
PLOT DATE = 10/15/2015 CHECKED - REVISE	D



11

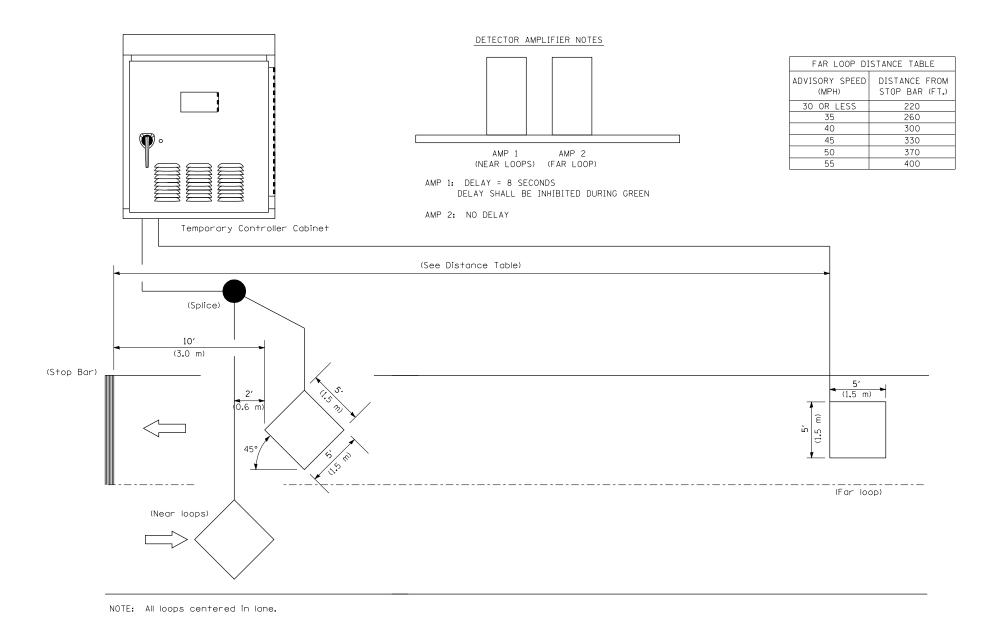
35′-0" (10**.**67)

50′ (15.2 m)

20′ (6.1)

SCALE: NTS

	STEEL PLATE BEAM GUARDRAIL, SHORT RADIUS		F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEE.	
			777	10B-2, 405B-1	MONTGOMERY	121	54	
SHORT RADIOS					CONTRACT	NO. 7	72D08	
	SHEET NO. 2 OF 3 SHEETS	STA.	TO STA.		ILLINOIS FEI	D. AID PROJECT		



INDUCTION LOOP DETECTOR

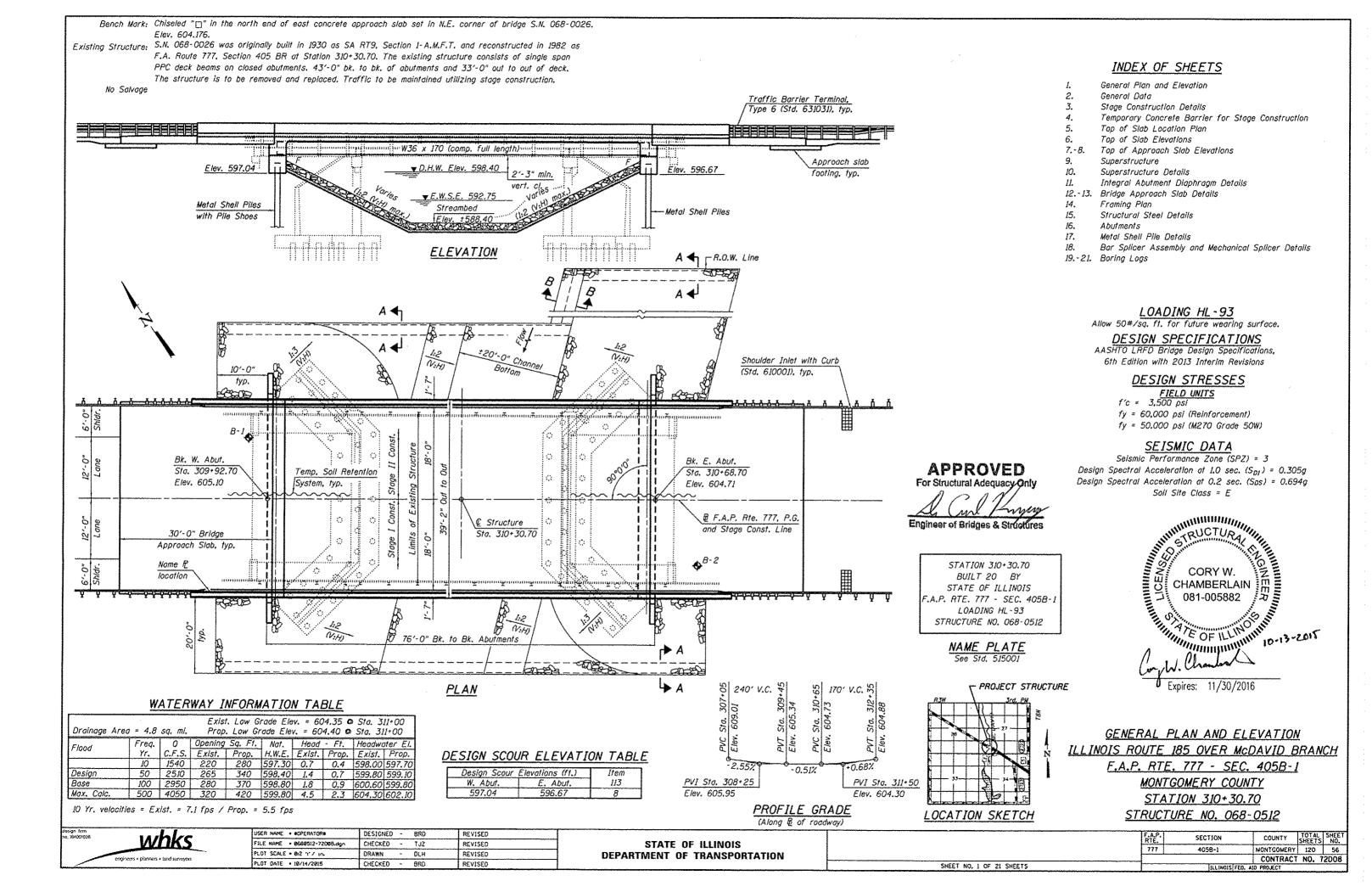


USER NAME	= gjameson	DESIGNED	-	REVISED
FILE NAME	= D672D08-SN0680026-SH1	-00HE101XE899n	-	REVISED
PLOT SCALE	= 40.000 '/ IN.	DRAWN	-	REVISED
PLOT DATE	= 10/15/2015	CHECKED	-	REVISED

STATE	: OF	ILLINOIS
DEPARTMENT	OF	TRANSPORTATION

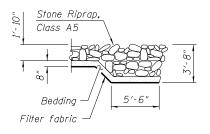
		RIDGE TRAFFIC EMENT DETAIL :	
SCALE: NTS	SHEET NO. 3 OF 3 S	SHEETS STA.	TO STA.
	SCALE: NTS	LOOP PLACE	LOOP PLACEMENT DETAIL

		ILLINOIS	ID PROJECT			
				CONTRACT	NO. 7	2D0
777	10B-2.	405B-1		MONTGOMERY	121	55
F.A.P. RTE.	SEC	TION		COUNTY	SHEETS	SHEE NO.

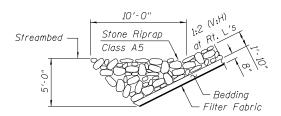


GENERAL NOTES

- 1. Fasteners shall be ASTM A325 Type 3. Bolts $\frac{3}{4}$ " ϕ , holes $\frac{13}{16}$ " ϕ , unless otherwise noted.
- 2. Calculated weight of Structural Steel = 84,310 lbs.
- 3. All structural steel shall be AASHTO M 270 Grade 50W.
- 4. No field welding is permitted except as specified in the contract documents.
- 5. Reinforcement bars designated (E) shall be epoxy coated.
- 6. Structural steel shall only be painted for a distance equal to the depth of embedment into the concrete diaphragm plus 1'-6". Painted areas shall be primed in the shop with a Department approved zinc rich primer. Field painting will not be required.
- 7. Layout of the slope protection system may be varied to suit ground conditions in the field as directed by the Engineer.
- 8. The structure is referenced to a Baseline of Construction.
- 9. Slipforming of the parapet is not allowed.



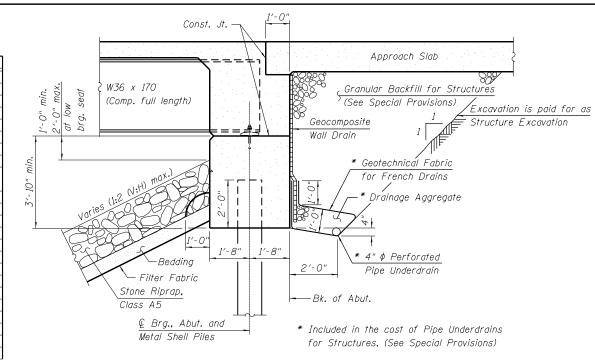
SECTION A-A



SECTION B-B

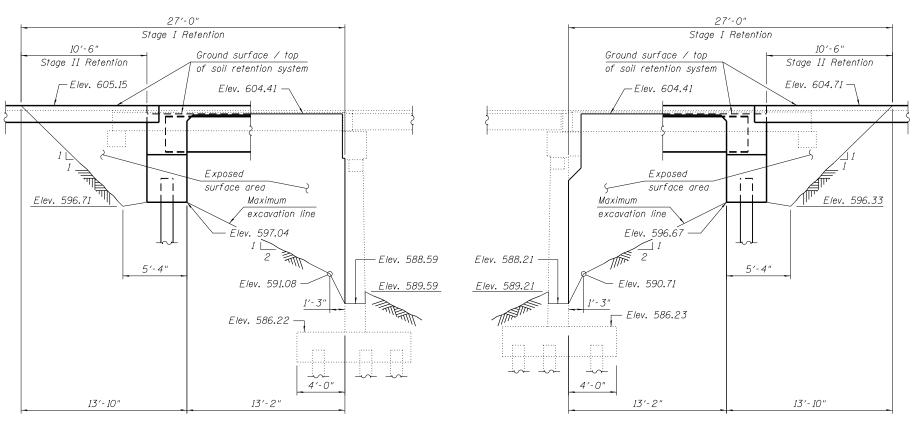
TOTAL BILL OF MATERIAL

Stone Riprap, Class A5 Sq. Yd. 955 955 Filter Fabric Sq. Yd. 955 955 Removal of Existing Structures No. 1 Each 1 1 Structure Excavation Cu. Yd. 266 266 Concrete Structures Cu. Yd. 69.0 69.0 Concrete Superstructure Cu. Yd. 241.1 241.1 Bridge Deck Grooving Sq. Yd. 509 509 Protective Coat Sq. Yd. 631 631 Furnishing and Erecting Structural Steel L. Sum 0.45 0.45 Stud Shear Connectors Each 1,404 1,404 Reinforcement Bars, Epoxy Coated Pound 51,560 12,140 63,700 Bar Splicers Each 399 108 507 Furnishing Metal Pile Shells 14"x0.312" Foot 695 695 Furnishing Piles Each 399 108 507 Furnishing Metal Pile Shells 14"x0.312" Foot 695 695 Fest Pile Metal Shells <	ITEM	UNIT	SUPER	SUB	TOTAL
Filter Fabric Sq. Yd. 955 955 Removal of Existing Structures No. 1 Each 1 1 Structure Excavation Cu. Yd. 266 266 Concrete Structures Cu. Yd. 69.0 69.0 Concrete Superstructure Cu. Yd. 241.1 241.1 Bridge Deck Grooving Sq. Yd. 509 509 Footective Coat Sq. Yd. 631 631 Furnishing and Erecting Structural L. Sum 0.45 0.45 Stud Shear Connectors Each 1,404 1,404 Reinforcement Bars, Epoxy Coated Pound 51,560 12,140 63,700 Bar Splicers Each 399 108 507 Furnishing Metal Pile Shells 14"x0.312" Foot 695 695 Driving Piles Foot 695 695 Test Pile Metal Shells Each 2 2 Pile Shoes Each 6 6 Name Plates Each 1 1 <td< td=""><td>Stone Riprap, Class A5</td><td>Sa. Yd.</td><td></td><td>955</td><td>955</td></td<>	Stone Riprap, Class A5	Sa. Yd.		955	955
Structure Excavation Cu. Yd. 266 266 Concrete Structures Cu. Yd. 69.0 69.0 Concrete Superstructure Cu. Yd. 241.1 241.1 Bridge Deck Grooving Sq. Yd. 509 509 Protective Coat Sq. Yd. 631 631 Furnishing and Erecting Structural Steel L. Sum 0.45 0.45 Stud Shear Connectors Each 1.404 1.404 Reinforcement Bars, Epoxy Coated Pound 51,560 12,140 63,700 Bar Splicers Each 399 108 507 Furnishing Metal Pile Shells 14"x0.312" Foot 695 695 Driving Piles Foot 695 695 Test Pile Metal Shells Each 2 2 Pile Shoes Each 6 6 Name Plates Each 1 1 Anchor Bolts, 1" Each 24 24 Geocomposite Wall Drain Sq. Yd. 74 74 Gra		Sq. Yd.		955	955
Concrete Structures Cu. Yd. 69.0 69.0 Concrete Superstructure Cu. Yd. 241.1 241.1 Bridge Deck Grooving Sq. Yd. 509 509 Protective Coat Sq. Yd. 631 631 Furnishing and Erecting Structural Steel L. Sum 0.45 0.45 Stud Shear Connectors Each 1,404 1.404 Reinforcement Bars, Epoxy Coated Pound 51,560 12,140 63,700 Bar Splicers Each 399 108 507 Furnishing Metal Pile Shells 14"x0.312" Foot 695 695 Driving Piles Foot 695 695 Test Pile Metal Shells Each 2 2 Pile Shoes Each 6 6 Name Plates Each 1 1 Anchor Bolts, 1" Each 24 24 Geocomposite Wall Drain Sq. Yd. 74 74 Granular Backfill for Structures Cu. Yd. 131 131	Removal of Existing Structures No. 1	Each	1		1
Concrete Superstructure Cu. Yd. 241.1 241.1 Bridge Deck Grooving Sq. Yd. 509 509 Protective Coat Sq. Yd. 631 631 Furnishing and Erecting Structural Steel L. Sum 0.45 0.45 Stud Shear Connectors Each 1.404 1.404 Reinforcement Bars, Epoxy Coated Pound 51,560 12,140 63,700 Bar Splicers Each 399 108 507 Furnishing Metal Pile Shells 14"x0.312" Foot 695 695 Driving Piles Foot 695 695 Test Pile Metal Shells Each 2 2 Pile Shoes Each 6 6 Name Plates Each 1 1 Anchor Bolts, 1" Each 24 24 Geocomposite Wall Drain Sq. Yd. 74 74 Granular Backfill for Structures Cu. Yd. 131 131 Asbestos Bearing Pad Removal Each 26 26	Structure Excavation	Cu. Yd.		266	266
Bridge Deck Grooving Sq. Yd. 509 509 Protective Coat Sq. Yd. 631 631 Furnishing and Erecting Structural Steel L. Sum 0.45 0.45 Stud Shear Connectors Each 1.404 1.404 Reinforcement Bars, Epoxy Coated Pound 51,560 12,140 63,700 Bar Splicers Each 399 108 507 Furnishing Metal Pile Shells 14"x0.312" Foot 695 695 Driving Piles Foot 695 695 Test Pile Metal Shells Each 2 2 Pile Shoes Each 6 6 Name Plates Each 1 1 Anchor Bolts, 1" Each 24 24 Geocomposite Wall Drain Sq. Yd. 74 74 Granular Backfill for Structures Cu. Yd. 131 131 Asbestos Bearing Pad Removal Each 26 26 Pipe Underdrains for Structures 4" Foot 168 168 <td>Concrete Structures</td> <td>Cu. Yd.</td> <td></td> <td>69.0</td> <td>69.0</td>	Concrete Structures	Cu. Yd.		69.0	69.0
Protective Coat	Concrete Superstructure	Cu. Yd.	241.1		241.1
Furnishing and Erecting Structural Steel L. Sum 0.45 0.45 Stud Shear Connectors Each 1,404 1,404 Reinforcement Bars, Epoxy Coated Pound 51,560 12,140 63,700 Bar Splicers Each 399 108 507 Furnishing Metal Pile Shells 14"x0.312" Foot 695 695 Driving Piles Foot 695 695 Test Pile Metal Shells Each 2 2 Pile Shoes Each 6 6 Name Plates Each 1 1 Anchor Bolfs, 1" Each 24 24 Geocomposite Wall Drain Sq. Yd. 74 74 Granular Backfill for Structures Cu. Yd. 131 131 Asbestos Bearing Pad Removal Each 26 26 Pipe Underdrains for Structures 4" Foot 168 168	Bridge Deck Grooving	Sq. Yd.	509		509
Steel L. Sum 0.45 0.45 Stud Shear Connectors Each 1,404 1,404 Reinforcement Bars, Epoxy Coated Pound 51,560 12,140 63,700 Bar Splicers Each 399 108 507 Furnishing Metal Pile Shells 14"x0.312" Foot 695 695 Driving Piles Foot 695 695 Test Pile Metal Shells Each 2 2 Pile Shoes Each 6 6 Name Plates Each 1 1 Anchor Bolfs, 1" Each 24 24 Geocomposite Wall Drain Sq. Yd. 74 74 Granular Backfill for Structures Cu. Yd. 131 131 Asbestos Bearing Pad Removal Each 26 26 Pipe Underdrains for Structures 4" Foot 168 168	Protective Coat	Sq. Yd.	631		631
Reinforcement Bars, Epoxy Coated Pound 51,560 12,140 63,700 Bar Splicers Each 399 108 507 Furnishing Metal Pile Shells 14"x0.312" Foot 695 695 Driving Piles Foot 695 695 Test Pile Metal Shells Each 2 2 Pile Shoes Each 6 6 Name Plates Each 1 1 Anchor Bolts, 1" Each 24 24 Geocomposite Wall Drain Sq. Yd. 74 74 Granular Backfill for Structures Cu. Yd. 131 131 Asbestos Bearing Pad Removal Each 26 26 Pipe Underdrains for Structures 4" Foot 168 168	, ,	L. Sum	0.45		0.45
Bar Splicers Each 399 108 507 Furnishing Metal Pile Shells 14"x0.312" Foot 695 695 Driving Piles Foot 695 695 Test Pile Metal Shells Each 2 2 Pile Shoes Each 6 6 Name Plates Each 1 1 Anchor Bolts, 1" Each 24 24 Geocomposite Wall Drain Sq. Yd. 74 74 Granular Backfill for Structures Cu. Yd. 131 131 Asbestos Bearing Pad Removal Each 26 26 Pipe Underdrains for Structures 4" Foot 168 168	Stud Shear Connectors	Each	1,404		1,404
Furnishing Metal Pile Shells 14"x0.312" Foot 695 695 Driving Piles Foot 695 695 Test Pile Metal Shells Each 2 2 Pile Shoes Each 6 6 Name Plates Each 1 1 Anchor Bolts, 1" Each 24 24 Geocomposite Wall Drain Sq. Yd. 74 74 Granular Backfill for Structures Cu. Yd. 131 131 Asbestos Bearing Pad Removal Each 26 26 Pipe Underdrains for Structures 4" Foot 168 168	Reinforcement Bars, Epoxy Coated	Pound	51,560	12,140	63,700
Driving Piles Foot 695 695 Test Pile Metal Shells Each 2 2 Pile Shoes Each 6 6 Name Plates Each 1 1 Anchor Bolts, I" Each 24 24 Geocomposite Wall Drain Sq. Yd. 74 74 Granular Backfill for Structures Cu. Yd. 131 131 Asbestos Bearing Pad Removal Each 26 26 Pipe Underdrains for Structures 4" Foot 168 168	Bar Splicers	Each	399	108	507
Test Pile Metal Shells Each 2 2 Pile Shoes Each 6 6 Name Plates Each 1 1 Anchor Bolts, I" Each 24 24 Geocomposite Wall Drain Sq. Yd. 74 74 Granular Backfill for Structures Cu. Yd. 131 131 Asbestos Bearing Pad Removal Each 26 26 Pipe Underdrains for Structures 4" Foot 168 168	Furnishing Metal Pile Shells 14"x0.312"	Foot		695	695
Pile Shoes Each 6 6 Name Plates Each 1 1 Anchor Bolts, I" Each 24 24 Geocomposite Wall Drain Sq. Yd. 74 74 Granular Backfill for Structures Cu. Yd. 131 131 Asbestos Bearing Pad Removal Each 26 26 Pipe Underdrains for Structures 4" Foot 168 168	Driving Piles	Foot		695	695
Name Plates Each 1 1 Anchor Bolts, I" Each 24 24 Geocomposite Wall Drain Sq. Yd. 74 74 Granular Backfill for Structures Cu. Yd. 131 131 Asbestos Bearing Pad Removal Each 26 26 Pipe Underdrains for Structures 4" Foot 168 168	Test Pile Metal Shells	Each		2	2
Anchor Bolts, I" Each 24 24 Geocomposite Wall Drain Sq. Yd. 74 74 Granular Backfill for Structures Cu. Yd. 131 131 Asbestos Bearing Pad Removal Each 26 26 Pipe Underdrains for Structures 4" Foot 168 168	Pile Shoes	Each		6	6
Geocomposite Wall Drain Sq. Yd. 74 74 Granular Backfill for Structures Cu. Yd. 131 131 Asbestos Bearing Pad Removal Each 26 26 Pipe Underdrains for Structures 4" Foot 168 168	Name Plates	Each	1		1
Granular Backfill for Structures Cu. Yd. 131 131 Asbestos Bearing Pad Removal Each 26 26 Pipe Underdrains for Structures 4" Foot 168 168	Anchor Bolts, 1"	Each		24	24
Asbestos Bearing Pad Removal Each 26 26 Pipe Underdrains for Structures 4" Foot 168 168	Geocomposite Wall Drain	Sq. Yd.		74	74
Pipe Underdrains for Structures 4" Foot 168 168	Granular Backfill for Structures	Cu. Yd.		131	131
	Asbestos Bearing Pad Removal	Each		26	26
Temporary Soil Retention System Sq. Ft. 538 538	Pipe Underdrains for Structures 4"	Foot		168	168
	Temporary Soil Retention System	Sq. Ft.		538	538



SECTION THRU INTEGRAL ABUTMENT

All drainage system components shall extend to 2'-0" from the end of each wingwall except an outlet pipe shall extend until intersecting with the side slopes. The pipes shall drain into concrete headwalls. (See Article 601.05 of the Standard Specifications and Highway Standard 601.101).



TEMPORARY SOIL RETENTION SYSTEM

(Looking North)

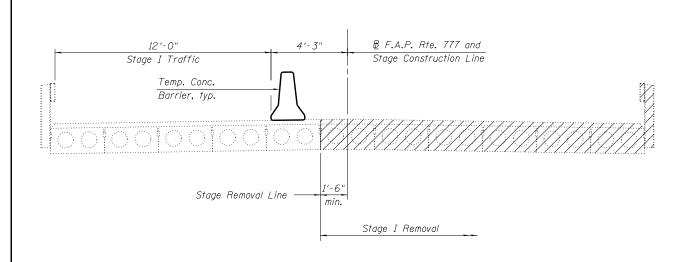
A cantilevered sheet piling design does not appear feasible and additional members or other retention systems may be necessary. The Contractor shall submit a temporary soil retention system design including plan details and calculations for review and acceptance by the Engineer.



USER NAME = dheberling	DESIGNED	-	BRD	REVISED
FILE NAME = 0680512-72D08.dgn	CHECKED	-	TJZ	REVISED
PLOT SCALE = 0:2 ':" / in.	DRAWN	-	DLH	REVISED
PLOT DATE = 10/15/2015	CHECKED	-	BRD	REVISED

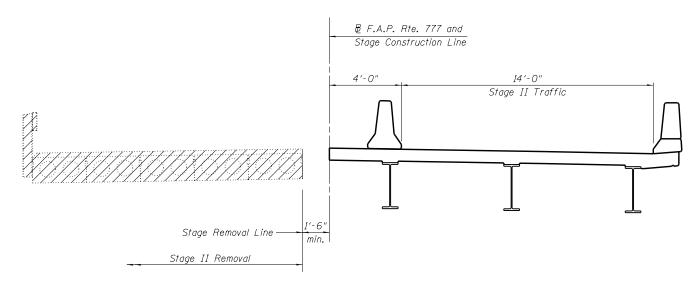
STATE	: OI	FILLINOIS	
DEPARTMENT	0F	TRANSPORTATION	

GENERAL DATA		F.A.P. SECTION C		TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 068-0512	777 405B-1		MONTGOMERY	121	57
			CONTRACT	NO. 7	2D08
SHEET NO. 2 OF 21 SHEETS		ILLINOIS FED.	AID PROJECT		

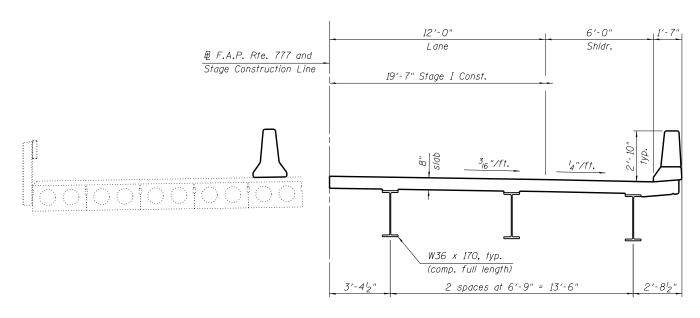


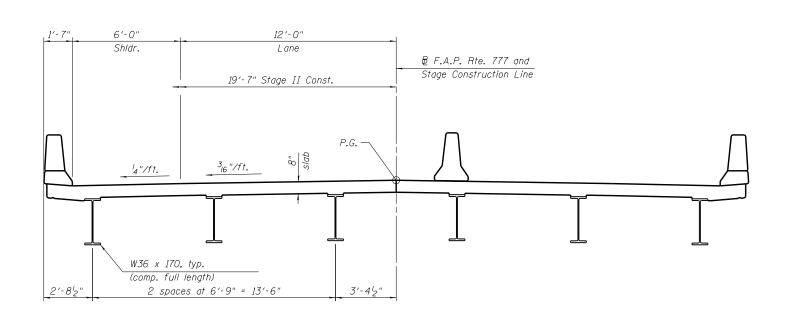
STAGE I REMOVAL

(Looking East)



STAGE II REMOVAL
(Looking East)





STAGE II CONSTRUCTION

(Looking East)

STAGE I CONSTRUCTION

(Looking East)

- 1. Hatched area indicates Removal of Existing Structures.
- 2. For details of Temporary Concrete Barrier, see sheet 4 of 21.
- 3. See roadway plans for quantity of Temporary Concrete Barrier.

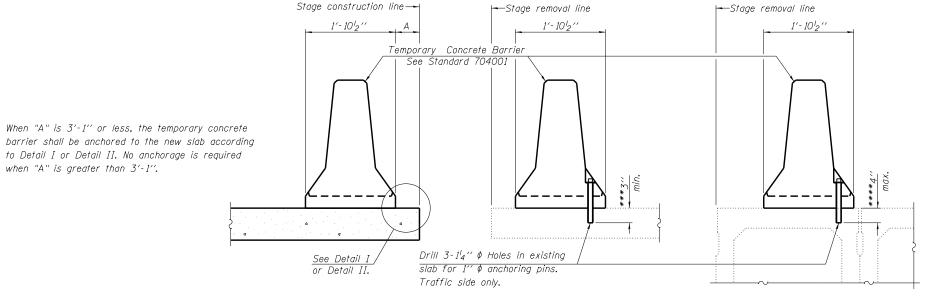
Notes:

USER NAME = dheberling	DESIGNED -	BRD	REVISED
FILE NAME = 0680512-72D08.dgn	CHECKED -	TJZ	REVISED
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PLOT DATE = 10/15/2015	CHECKED -	BRD	REVISED

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

STAGE CONSTRUCTION DETAILS STRUCTURE NO. 068-0512		SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
		777 405B-1		MONTGOMERY	121	58
31NUCTURE NO. 000-0312				CONTRACT	NO. 7	2D08
SHEET NO. 3 OF 21 SHEETS		ILLINOIS F	ED. AII	D PROJECT		

whks



NOTES

Detail I - With Bar Splicer or Couplers:

Connect one (1) 1" x 7" 'x "W" steel P to the top layer of couplers with $2^{-5}8$ " ϕ bolts screwed to coupler at approximate Q of each barrier panel.

Detail II - With Extended Reinforcement Bars:

Connect one (1) I'' x 7'' x ''W'' steel P to the concrete slab or concrete wearing surface with 2-5g'' \$\phi\$

Expansion Anchors or cast in place inserts spaced between the top layer of reinforcement at approximate C of seath barrier and the concrete statement of the control of the contro

at approximate © of each barrier panel.

Cost of retainer assembly is included with Temporary Concrete Barrier.

The I'' x 7'' x ''W'' plate shall not be removed until stage II construction forms and all reinforcement bars are in place and the concrete is ready to be placed.

SECTIONS THRU SLAB OR DECK BEAM

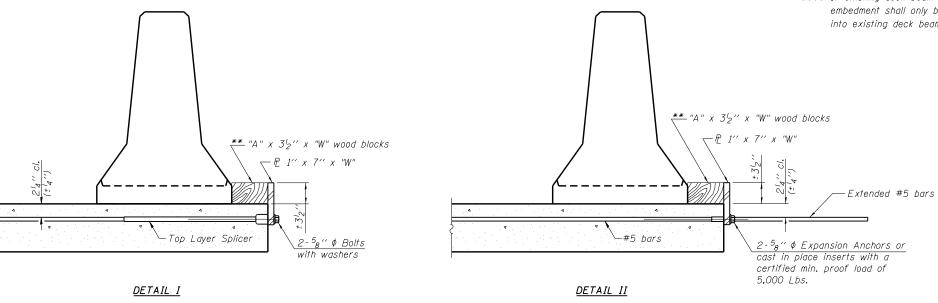
EXISTING SLAB

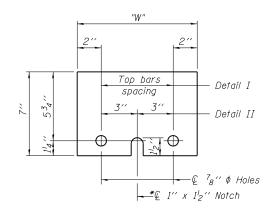
*** Dimension shown is minimum required embedment into concrete.

If hot-mix asphalt wearing surface is present, minimum embedment shall be in addition to wearing surface depth.

EXISTING DECK BEAM

**** If existing deck beam is to remain in place after stage construction, embedment shall only be into wearing surface and not into existing deck beam concrete.





STEEL RETAINER P I'' x 7'' x ''W'' * Required only with Detail II

RETAINER ASSEMBLY

*** Wood blocks may be omitted when required to provide minimum stage traffic lane width. When the wood blocks are omitted, the concrete barrier shall be in direct contact with the steel retainer plate.

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1- 12 - 15

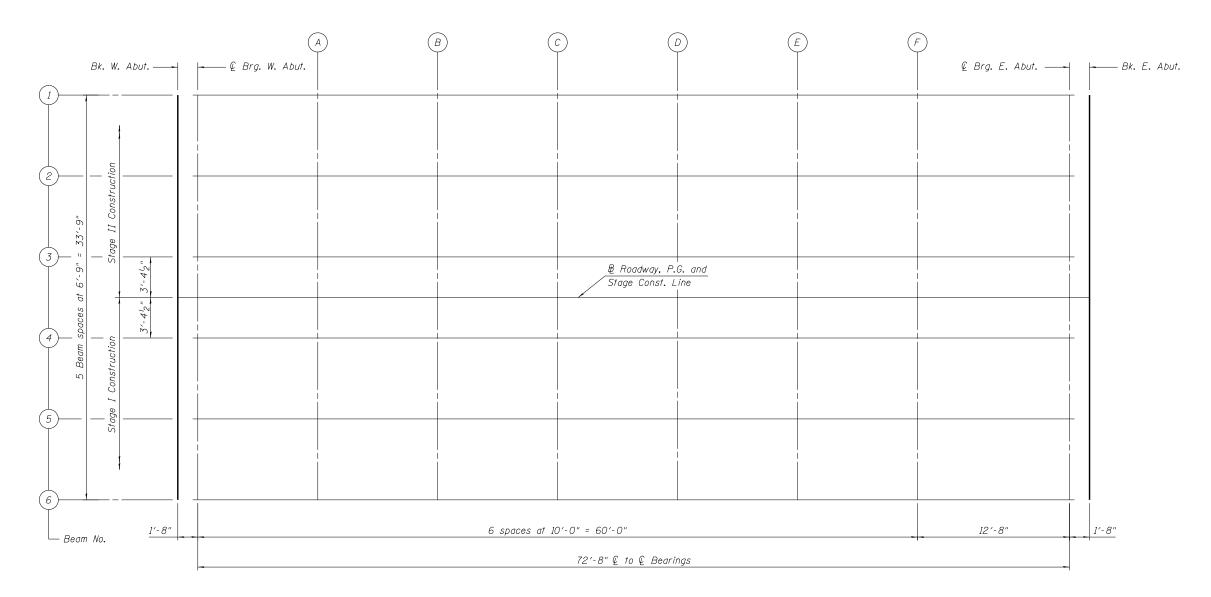
USER NAME = dheberling	DESIGNED	-	BRD	REVISED
FILE NAME = 0680512-72008.dgn	CHECKED	-	TJZ	REVISED
PLOT SCALE = 0:2 ':" / in.	DRAWN	-	DLH	REVISED
PLOT DATE = 10/15/2015	CHECKED	-	BRD	REVISED

NEW SLAB

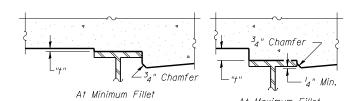
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

TEMPORARY CONCRETE BARRIER FOR STAGE CONSTRUCTION	F.A RT
STRUCTURE NO. 068-0512	77
0111001011E 140: 000-0312	
SHEET NO. 4 OF 21 SHEETS	

	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	777 405B-1		MONTGOMERY	121	59
			CONTRACT	NO. 7	2D08
ILLINOIS FED. AID PROJECT					

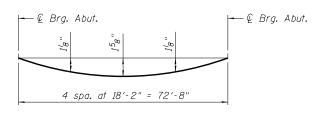


PLAN



To determine "t": After all structural steel has been erected, elevations of the top flanges of the beams shall be taken at intervals shown above. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflection" shown on sheet 6 of 21, minus slab thickness, equals the fillet heights "t" above top flange of beams.

FILLET HEIGHTS



DEAD LOAD DEFLECTION DIAGRAM

(Includes weight of concrete only.)

The above deflections are not to be used in the field if the Engineer is working from the grade elevations adjusted for dead load deflections as shown on sheet 6 of 21.

Note:

Work this sheet with sheet 6 of 21.

design firm no. 184001036 whks engineers + planners + land surveyors

USER NAME = dheberling	DESIGNED	-	BRD	REVISED
FILE NAME = 0680512-72D08.dgn	CHECKED	-	TJZ	REVISED
PLOT SCALE = 0:2 ':" / in.	DRAWN	-	DLH	REVISED
PLOT DATE = 10/15/2015	CHECKED	-	BRD	REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TOP OF SLAB LOCATION PLAN	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 068-0512	777	405B-1	MONTGOMERY	121	60
31NUCTURE NO. 000-0312			CONTRACT	NO. 7	72D08
CUEET NO. F. OF 21 CHEETC					

<u>BEAM 1</u>

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	309+92.70	-16.88	604.81	604.81
© Brg. W. Abut.	309+94.37	-16.88	604.80	604.80
A B C D E F	310+04.37 310+14.37 310+24.37 310+34.37 310+44.37 310+54.37	-16.88 -16.88 -16.88 -16.88 -16.88	604.75 604.70 604.65 604.60 604.55 604.49	604.80 604.80 604.77 604.73 604.66 604.56
© Brg. E. Abut.	310+67.03	-16.88	604.43	604.43
Bk. E. Abut.	310+68.70	-16.88	604.42	604.42

<u>BEAM 2</u>

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	309+92.70	-10.13	604.94	604.94
© Brg. W. Abut.	309+94.37	-10.13	604.93	604.93
A B C D E F	310+04.37 310+14.37 310+24.37 310+34.37 310+44.37 310+54.37	-10.13 -10.13 -10.13 -10.13 -10.13	604.88 604.83 604.78 604.73 604.68 604.63	604.94 604.93 604.91 604.86 604.79 604.69
© Brg. E. Abut.	310+67.03	-10.13	604.56	604.56
Bk. E. Abut.	310+68.70	-10.13	604.55	604.55

<u>BEAM 3</u>

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection		
Bk. W. Abut.	309+92.70	-3.38	605.04	605.04		
€ Brg. W. Abut.	309+94.37	-3.38	605.04	605.04		
A B C D E F	310+04.37 310+14.37 310+24.37 310+34.37 310+44.37 310+54.37	-3.38 -3.38 -3.38 -3.38 -3.38	604.99 604.93 604.88 604.83 604.78	605.04 605.04 605.01 604.96 604.89 604.80		
© Brg. E. Abut.	310+67.03	-3.38	604.67	604.67		
Bk. E. Abut.	310+68.70	-3.38	604.66	604.66		

B ROADWAY, P.G. AND STAGE CONSTRUCTION LINE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	309+92.70	0.00	605.10	605.10
€ Brg. W. Abut.	309+94.37	0.00	605.09	605.09
A B C D E F	310+04.37 310+14.37 310+24.37 310+34.37 310+44.37 310+54.37	0.00 0.00 0.00 0.00 0.00	605.04 604.99 604.94 604.89 604.83 604.78	605.09 605.09 605.06 605.02 604.94 604.85
© Brg. E. Abut.	310+67.03	0.00	604.72	604.72
Bk. E. Abut.	310+68.70	0.00	604.71	604.71

BEAM 4

<u> </u>						
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection		
Bk. W. Abut.	309+92.70	3.38	605.04	605.04		
€ Brg. W. Abut.	309+94.37	3.38	605.04	605.04		
A B C D E F	310+04.37 310+14.37 310+24.37 310+34.37 310+44.37 310+54.37	3.38 3.38 3.38 3.38 3.38 3.38	604.99 604.93 604.88 604.83 604.78	605.04 605.04 605.01 604.96 604.89 604.80		
€ Brg. E. Abut.	310+67.03	3.38	604.67	604.67		
Bk. E. Abut.	310+68.70	3.38	604.66	604.66		

<u>BEAM 5</u>

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	309+92.70	10.13	604.94	604.94
€ Brg. W. Abut.	309+94.37	10.13	604.93	604.93
A B C D E F	310+04.37 310+14.37 310+24.37 310+34.37 310+44.37 310+54.37	10.13 10.13 10.13 10.13 10.13	604.88 604.83 604.78 604.73 604.68 604.63	604.94 604.93 604.91 604.86 604.79 604.69
₽ Brg. E. Abut.	310+67.03	10.13	604.56	604.56
Bk. E. Abut.	310+68.70	10.13	604.55	604.55

<u>BEAM 6</u>

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	309+92.70	16.88	604.81	604.81
₡ Brg. W. Abut.	309+94.37	16.88	604.80	604.80
A B C D E F	310+04.37 310+14.37 310+24.37 310+34.37 310+44.37 310+54.37	16.88 16.88 16.88 16.88 16.88	604.75 604.70 604.65 604.60 604.55 604.49	604.80 604.80 604.77 604.73 604.66 604.56
₡ Brg. E. Abut.	310+67.03	16.88	604.43	604.43
Bk. E. Abut.	310+68.70	16.88	604.42	604.42

Note:

F.A.P. RTE. 777

Work this sheet with sheet 5 of 21.

whks
engineers + planners + land surveyors

USER NAME = dheberling	DESIGNED	-	BRD	REVISED
FILE NAME = 0680512-72D08.dgn	CHECKED	-	TJZ	REVISED
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PLOT DATE = 10/15/2015	CHECKED	-	BRD	REVISED

TOP O	F SLAB ELEVATIONS	
STRUC	CTURE NO. 068-0512	
SHEET	NO 6 OF 21 SHEETS	

NORTH EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations
W. End W. Appr. Pav't.	309+63.70	-18.00	604.93
A 1 A 2	309+73.70 309+83.70	-18.00 -18.00	604.88 604.83
E. End W. Appr. Pav't.	309+93.70	-18.00	604.78

NORTH EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
W. End W. Appr. Pav't.	309+63.70	-12.00	605.06
A 1 A 2	309+73.70 309+83.70	-12.00 -12.00	605.01 604.96
E. End W. Appr. Pav't.	309+93.70	-12.00	604.90

B ROADWAY, PROFILE GRADE AND STAGE CONSTRUCTION LINE

Location	Station	Offset	Theoretical Grade Elevations
W. End W. Appr. Pavít.	309+63.70	0.00	605.24
A1 A2	309+73.70 309+83.70	0.00 0.00	605.19 605.14
E. End W. Appr. Pav't.	309+93.70	0.00	605.09

West End of West Approach Pavement East End of West Approach Pavement

Approach Pavement	North Edge of Shoulder	Approach Paveme
9,-0"	North Edge of Pavement	
Stage II Construction 18'-0"	Roadway, P.G. and Stage Construction Line	
Stage I Construction 18'-0" 12'-0"	South Edge of Pavement	
0-,9	South Edge of Shoulder	
	3 spaces at 10'-0" = 30'-0"	

SOUTH EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
W. End W. Appr. Pav't.	309+63.70	12.00	605.06
A 1 A 2	309+73.70 309+83.70	12.00 12.00	605.01 604.96
E. End W. Appr. Pav't.	309+93.70	12.00	604.90

SOUTH EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations
W. End W. Appr. Pav't.	309+63.70	18.00	604.93
A1 A2	309+73.70 309+83.70	18.00 18.00	604.88 604.83
E. End W. Appr. Pav't.	309+93.70	18.00	604.78

PLAN



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PLOT DATE = 10/15/2015	CHECKED	-	BRD	REVISED

TOP OF WEST APPROACH SLAB ELEVATIONS	F.A.P. RTE.	
STRUCTURE NO. 068-0512	777	ĺ
3111001011L NO. 000-0312		
SHEET NO. 7 OF 21 SHEETS		

NORTH EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations
W. End E. Appr. Pav't.	310+67.70	-18.00	604.40
A3 A4	310+77.70 310+87.70	-18.00 -18.00	604.36 604.32
E. End E. Appr. Pav't.	310+97.70	-18.00	604.29

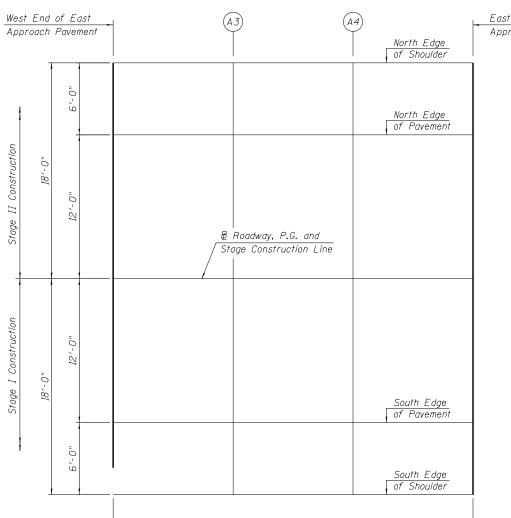
NORTH EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
W. End E. Appr. Pav't.	310+67.70	-12.00	604.53
A3 A4	310+77.70 310+87.70	-12.00 -12.00	604.48 604.45
E. End E. Appr. Pav't.	310+97.70	-12.00	604.41

B ROADWAY, PROFILE GRADE AND STAGE CONSTRUCTION LINE

Location	Station	Offset	Theoretical Grade Elevations
W. End E. Appr. Pav't.	310+67.70	0.00	604.72
A3 A4	310+77.70 310+87.70	0.00 0.00	604.67 604.63
E. End E. Appr. Pav't.	310+97.70	0.00	604.60

West Appr



East End of East
Approach Pavement

SOUTH EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
W. End E. Appr. Pavít.	310+67.70	12.00	604.53
A3 A4	310+77.70 310+87.70	12.00 12.00	604.48 604.45
E. End E. Appr. Pav't.	310+97.70	12.00	604.41

SOUTH EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations
W. End E. Appr. Pav′t.	310+67.70	18.00	604.40
A3 A4	310+77.70 310+87.70	18.00 18.00	604.36 604.32
E. End E. Appr. Pav't.	310+97.70	18.00	604.29

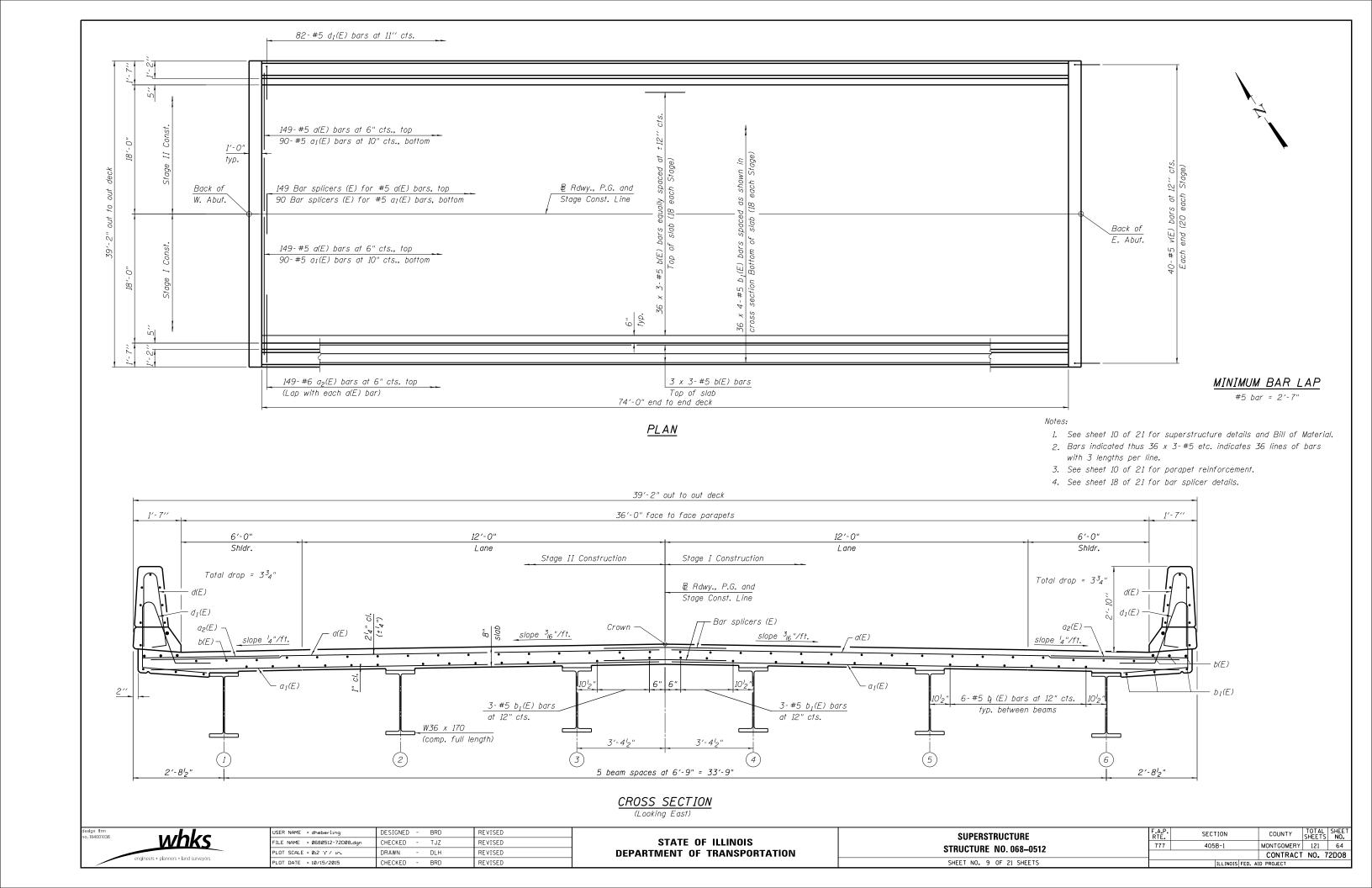
PLAN

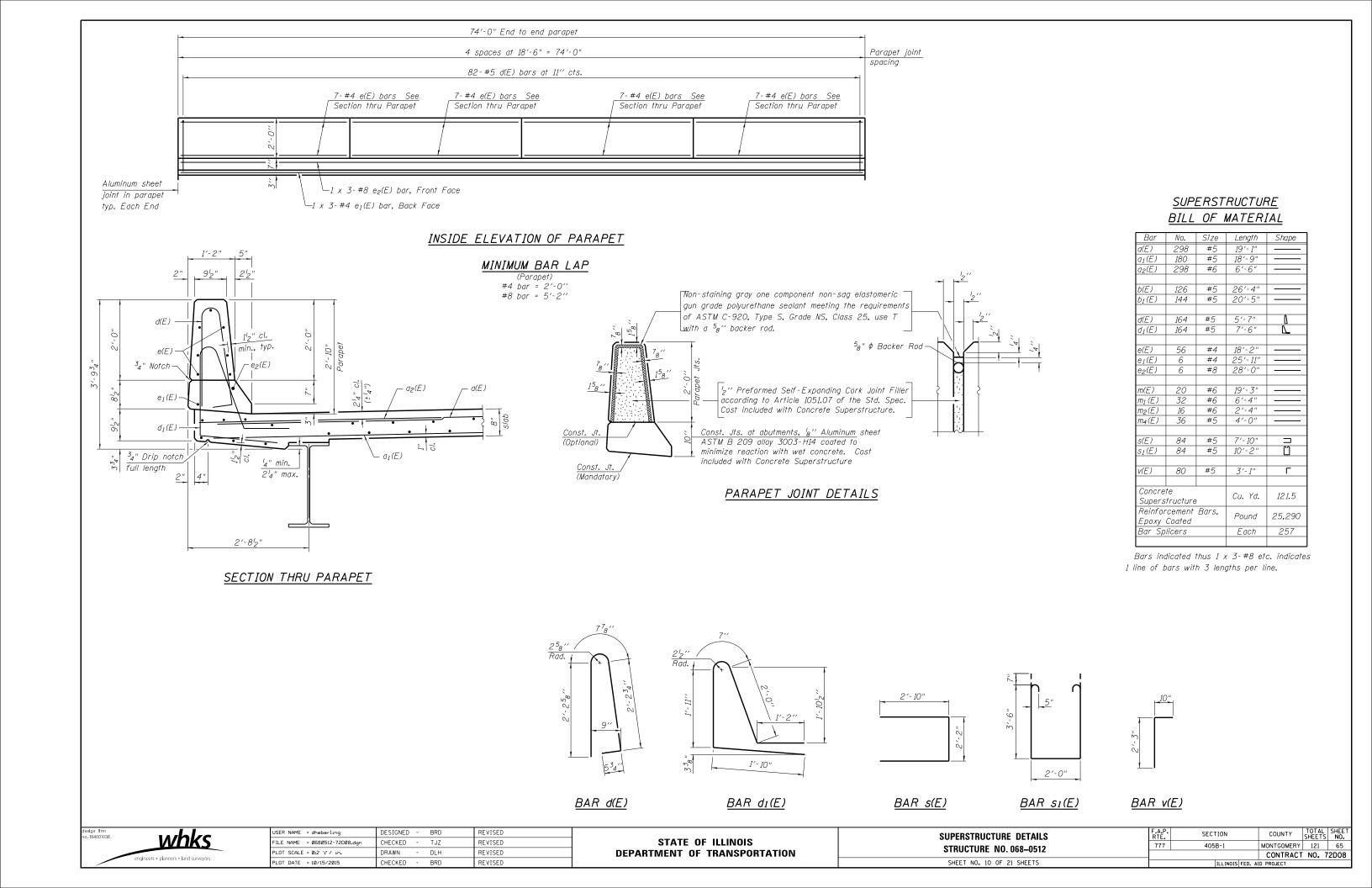
3 spaces at 10'-0" = 30'-0"

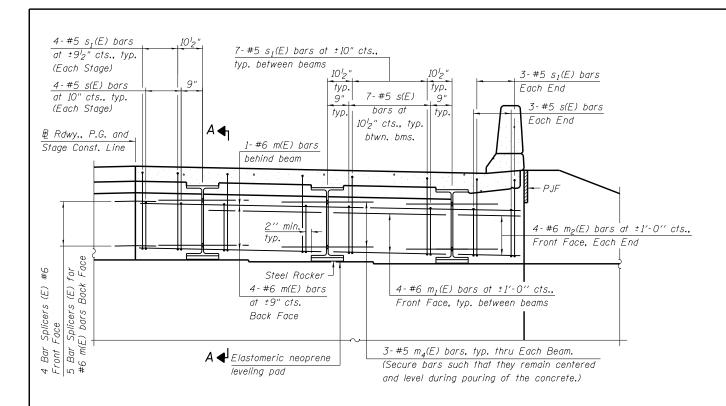


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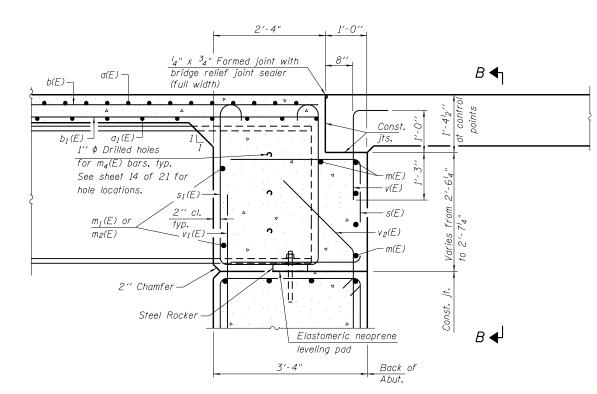
T0P	0F	EAST	A	PPR	0/	\CH	S	LAB	ELEVATIONS	
		STR	UC	TUF	RE	NO	. 0	68–0	512	
		SHF	FΤ	NO.	8	OF	21	SHEE.	rs	







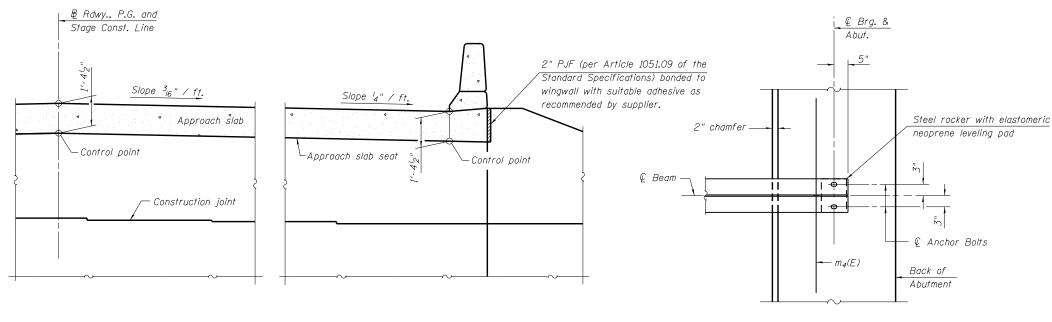
DIAPHRAGM ELEVATION AT ABUTMENT



SECTION A-A

TOP OF CORBEL ELEVATIONS

Abutment	Location	Corbel Elevation
	Inside face of north parapet	603.40
West Abutment	Centerline roadway	603.71
	Inside face of south parapet	603.40
	Inside face of north parapet	603.02
East Abutment	Centerline roadway	603.34
	Inside face of south parapet	603.02



PARTIAL PLAN AT ABUTMENT

(Showing bottom flange of beam)

Notes:

- 1. Reinforcement bars in diaphragm are billed with superstructure on sheet 10 of 21.
- 2. Concrete in diaphragm is included with Concrete Superstructure on sheet 10 of 21.
- 3. For details of bars s(E), $s_1(E)$ and v(E) see sheet 10 of 21.
- 4. The approach slab seat shall have a constant slope determined from the control points shown.
- 5. For bearing details see sheet 15 of 21.

DSI-2440-0 8-31-12

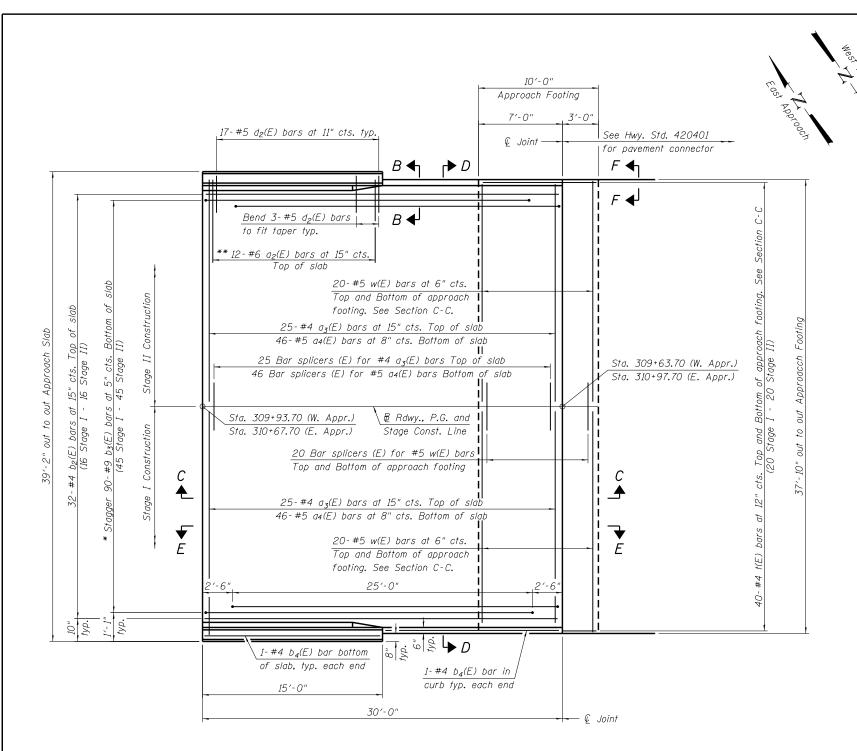
design firm no. 184001036	L	Jea
101104001000	WI	<u>IKS</u>
	engineers + planners + la	ind surveyors

USER NAME = dheberling	DESIGNED	-	BRD	REVISED
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PLOT DATE = 10/15/2015	CHECKED	-	BRD	REVISED

SECTION B-B

NTEGRAL ABUTMENT DIAPHRAGM STRUCTURE NO. 068–0512	
SHEET NO. 11 OF 21 SHEETS	

TE. SECTION COUNTY SHEETS NO. 777 405B-1 MONTGOMERY 121 66						
	A.P. TE.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
	777	405B-1		MONTGOMERY	121	66
CONTRACT NO. 72DOS				CONTRACT	NO. 7	72D08
ILLINOIS FED. AID PROJECT		ILLINOIS FE	ED. AI	D PROJECT		



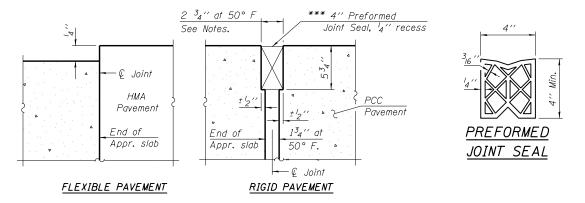
PLAN

(East Approach shown West Approach similar.)

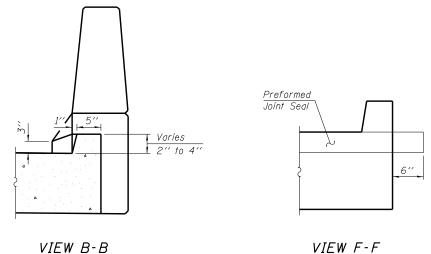
- * Tilt #9 $b_3(E)$ bars as required to maintain clearance.
- ** Space between $a_3(E)$ bars, typ. each parapet.

Notes:

- 1. See sheet 13 of 21 for Sections C-C & D-D and View E-E.
- 2. $a_3(E)$ and $a_4(E)$ bar spacings measured along \cancel{E} Rdwy.
- 3. The joint opening shall be determined per Article 520.04 except that on jointless structures, the distance described as the bridge length between the nearest fixed bearings each way from the joint shall be taken as half the bridge length plus the approach slab length. The minimum dimension shall be 1_2^{l} for installation purposes.
 - *** Cost included with Concrete Superstructure.



<u>DETAIL A</u>



(Sheet 1 of 2)

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STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

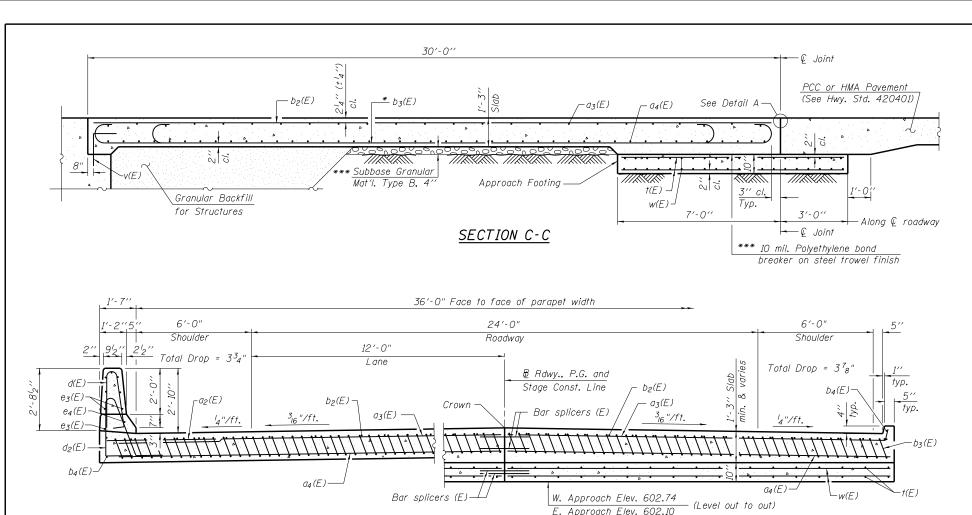
REVISED

BRIDGE APPROACH SLAB DETAILS
STRUCTURE NO. 068-0512
SHEET NO. 12 OF 21 SHEETS

F.A.P. SECTION COUNTY SHEETS NO.
777 405B-1 MONTGOMERY 121 67

CONTRACT NO. 72DO8

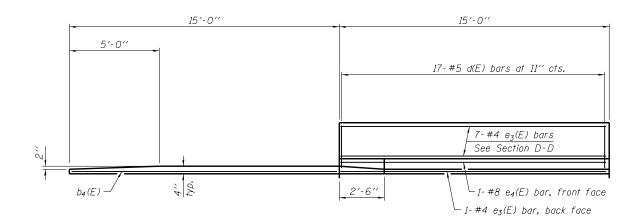
| ILLINOIS | FED. AID | PROJECT



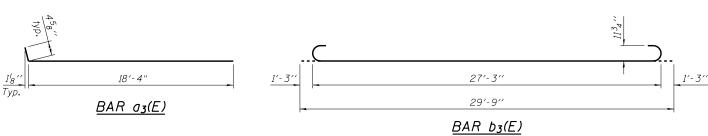
NEAR ABUTMENT

SECTION D-D
(See Plan for dimensions not shown)

AT APPROACH FOOTING



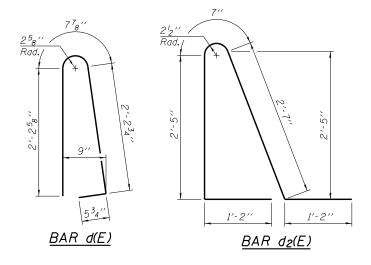
VIEW E-E



(Sheet 2 of 2)

Notes: 1. See sheet 12 of 21 for Detail A and View B-B.

- 2. Approach slab and parapet concrete shall be paid for as Concrete Superstructure.
- 3. Approach footing concrete shall be paid for as Concrete Structures.
- 4. Reinforcement shall be paid for as Reinforcement Bars, Epoxy Coated.
- 5. For v(E) bar details, see sheet 10 of 21.
- 6. For bar splicer details, see sheet 18 of 21.
- 7. Cost of excavation for approach footing included with Concrete Structures.
- 8. For Granular Backfill for Structures and drainage treatment details, see sheet 2 of 21.
- 9. For additional parapet details, see sheet 10 of 21.



- * Tilt #9 $b_3(E)$ bars as required to maintain clearance.
- *** Cost included with Concrete Superstructure.

TWO APPROACHES BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a ₂ (E)	48	#6	6'-6"	
a3(E)	100	#4	18′-9"	
04(E)	184	#5	18'-7"	
b2(E)	64	#4	29'-8"	
b3(E)	180	#9	29'-9"	
b4(E)	8	#4	14′-8"	-
d(E)	68	#5	5′-7"	1
d ₂ (E)	68	#5	5′-7" 7′-11"	Ż
0-(F)	70	#4	14'-8"	
e3(E) e4(E)	32	#4	14'-8"	
C4(L)	7	"0	17 0	
t(E)	160	#4	9′-8"	
w(E)	160	#5	18′-7"	
VV (_ /	100	, ,,	10 7	
Concre	te Struc	ctures	Cu. Yd.	23.4
Concre Supers	ete structure	9	Cu. Yd.	119.6
Reinfo	rcement Coated		Pound	30,410
Bar Sp			Each	222

whks

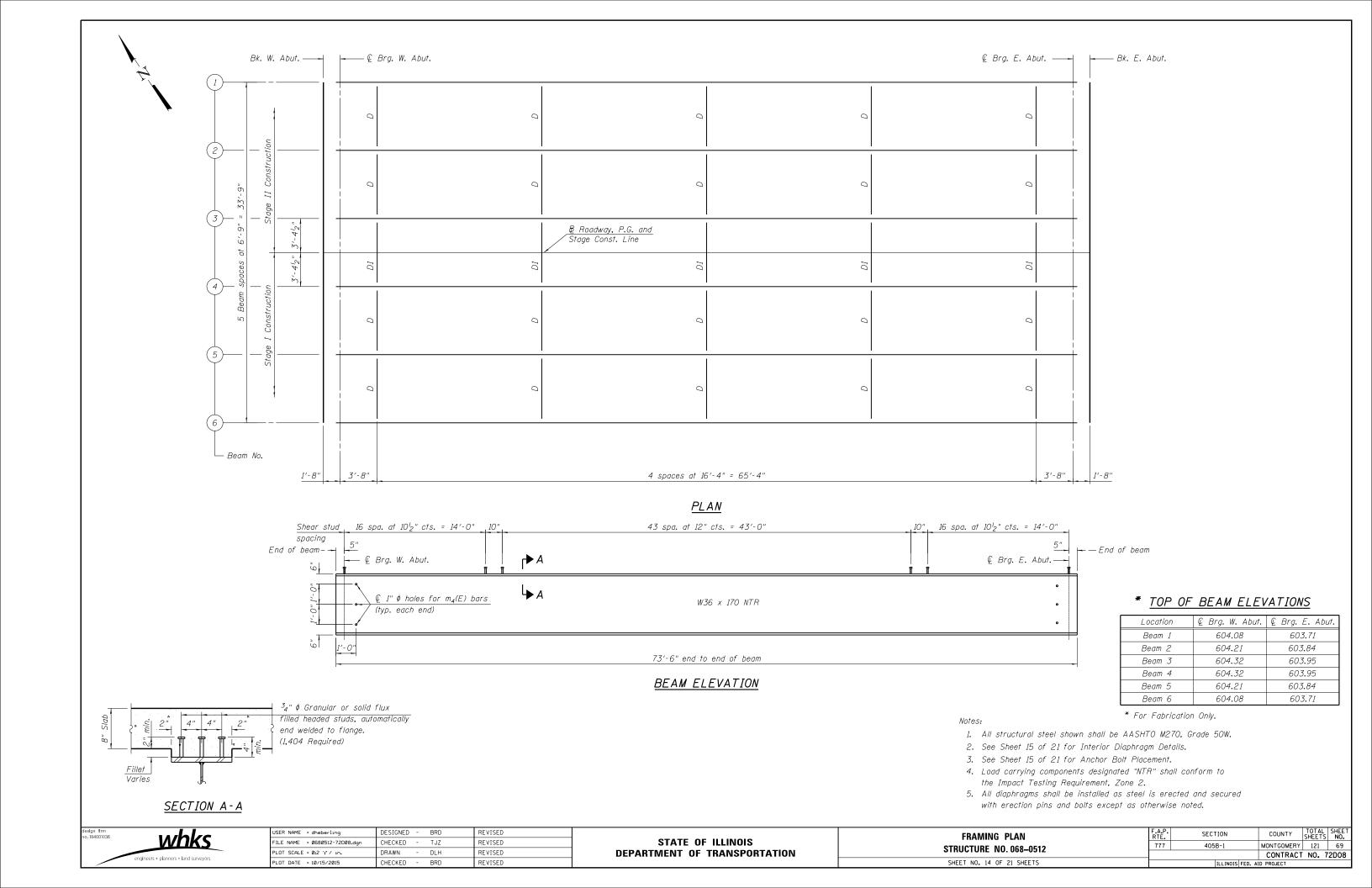
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

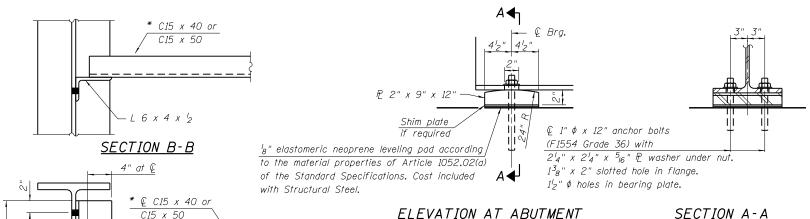
BRIDGE APPROACH SLAB DETAILS
STRUCTURE NO. 068-0512
SHEET NO. 13 OF 21 SHEETS

F.A.P. SECTION COUNTY TOTAL SHEETS NO.

777 405B-1 MONTGOMERY 121 68

CONTRACT NO. 72D08





ELEVATION AT ABUTMENT

SECTION A-A

FIXED BEARING

Notes:

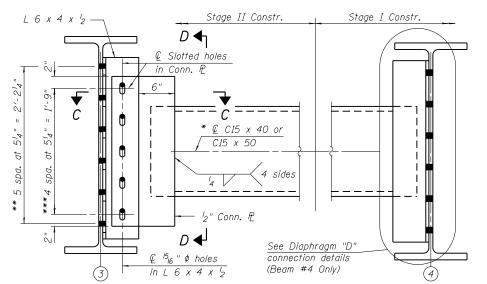
- Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.
- 2. Anchor bolts at fixed bearings may be either cast in place or installed in holes drilled after the supported member is in place.
- 3. Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.
- 4. The bearing plates, diaphragms, and connecting angles shall conform to the requirements of AASHTO M 270 Grade 50W.
- 5. Two hardened washers required for each set of oversized and slotted holes.
- * Alternate channels are permitted to facilitate material acquisition. Calculated weight of structural steel is based on the lighter section. The alternate, if utilized, shall be provided at no additional cost to the Department.
- ** $\frac{3}{4}$ " ϕ HS bolts, $\frac{15}{16}$ " ϕ holes.
- *** 3_4 " 4 H.S. bolts, on the south side of Beam 3 provide $^{13}_{16}$ " x $^{17}_{8}$ " vertical slotted holes in the L 6 x 4 x 1_2 at the web and in the connection P. Bolts in slotted holes shall be finger tightened until Stage II pour is completed. Position slots so bolts move from one end with no concrete load to the opposite end under the deck load. The slotted holes in the L 6 x 4 x $\frac{1}{2}$ and connection $\frac{1}{2}$ shall be positioned as shown to allow the bolts to move to the final erection position under deck load. The holes shall be positioned to allow maximum bolt displacement without laterally stressing the beams.

SECTION C-C

whks

- L 6 x 4 x ½

В



INITIAL BOLT ERECTION POSITION

€ Beam and € C15 x 40 or

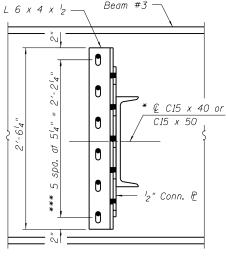
C15 x 50 at end of channel

 $6 \times 4 \times \frac{1}{2}$

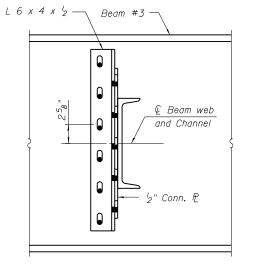
* C15 x 40 or

C15 x 50

INTERIOR DIAPHRAGM "D" DETAILS







INTERIOR BEAM MOMENT TABLE

c(3n)

c(cr)

c(3n)

M DC2

 $M \cap w$

OF Ma

fs DC2

s DW

s (4+IM)

 $0.95R_hF_{Vf}$

fs (Service I.

f_s (Total)(Strength I) (ksi)

Mu (Strength I

 (in^4)

(in4)

(in3 (in^3)

 (in^3)

(in3

(k/')

('k)

(k/')

('k)

(k/')

('k)

('k)

(ksi)

(ksi)

(ksi)

(ksi

(ksi)

(k)

28043

20053

868

774

0.91

597

0.15

0.30

198 1059

3021

4273

12.30 1.50

3.10

14.64

35,98

47.5

FINAL BOLT ERECTION POSITION AFTER STAGE II DECK POUR

INTERIOR DIAPHRAGM "DI" DETAILS

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PLOT DATE = 10/15/2015	CHECKED	-	BRD	REVISED

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

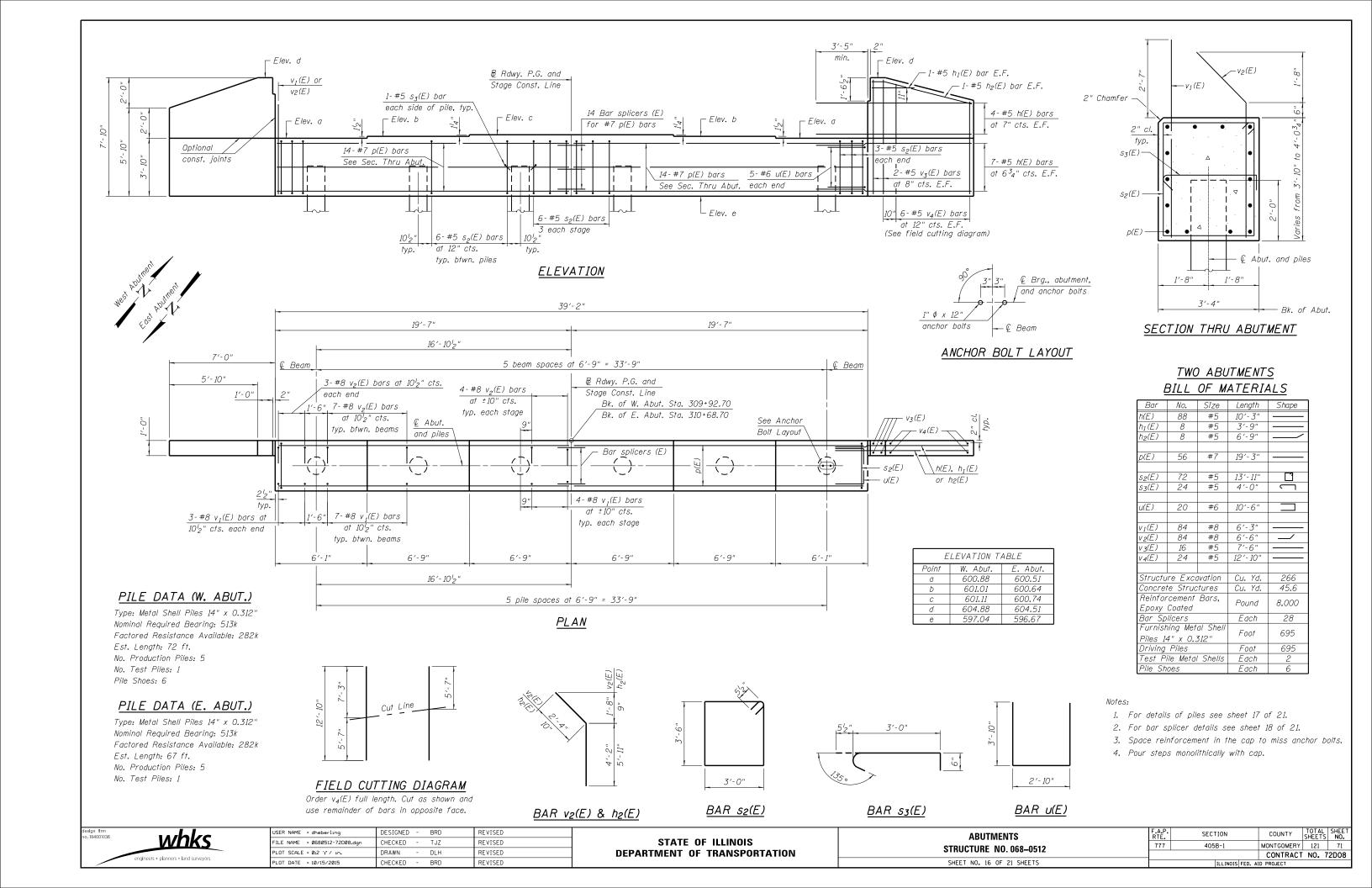
SECTION D-D

		Abut.
Roci	(k)	<i>33.71</i>
RDC1 RDC2 RDW R4 + IM	(k)	5 . 45
Row	(k)	10.90
R4 + IM	(k)	77.37
R Total	(k)	127.42

INTERIOR BEAM REACTION TABLE

- I_{s} , S_{s} : Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total-Strength I, and Service II) due to non-composite dead loads (in.4 and in.3).
- $I_c(n)$, $S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing $f_s(Total-Strength\ I,\ and\ Service\ II)$ in uncracked sections due to short-term composite live loads (in.4 and in.3).
- $I_c(3n)$, $S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections, due to long-term composite (superimposed) dead loads (in.4 and in.3).
- $I_c(cr)$, $S_c(cr)$: Composite moment of inertia and section modulus of the steel and longitudinal deck reinforcement, used for computing f_s (Total-Strength I and Service II) in cracked sections, due to both short-term composite live loads and long-term composite (superimposed) dead loads (in.4 and in.3).
 - DC1: Un-factored non-composite dead load (kips/ft.).
 - M_{DCI}: Un-factored moment due to non-composite dead load (kip-ft.). DC2: Un-factored long-term composite (superimposed excluding future
 - wearing surface) dead load (kips/ft.). M_{DC2}: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).
 - DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).
 - Mpw: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).
 - MŁ + IM: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).
- Mu (Strength I): Factored design moment (kip-ft.).
 - 1.25 (MDC1 + MDC2) + 1.5 MDW + 1.75 M& + IM
 - $\phi_f M_n$: Compact composite positive moment capacity computed according to Article 6.10.7.1 or non-slender negative moment capacity according to Article A6.1.1 or A6.1.2 (kip-ft).
 - $f_{\mathcal{S}}$ DC1: Un-factored stress at edge of flange for controlling steel flange due to vertical non-composite dead loads as calculated below (ksi).
 - fs DC2: Un-factored stress at edge of flange for controlling steel flange due to vertical composite dead loads as calculated below (ksi).
 - $M_{DC2}/S_c(3n)$ or $M_{DC2}/S_c(cr)$ as applicable.
 - fs DW: Un-factored stress at edge of flange for controlling steel flange due to vertical composite future wearing surface loads as calculated below (ksi).
 - M_{DW} / $S_c(3n)$ or M_{DW} / $S_c(cr)$ as applicable.
 - $f_{\it s}$ (4+IM): Un-factored stress at edge of flange for controlling steel flange due to vertical composite live load plus impact loads as calculated below (ksi).
 - $M_4 + IM / S_c(n)$ or $M_4 + IM / S_c(cr)$ as applicable.
- f_s (Service II): Sum of stresses as computed below (ksi).
 - fsDC1 + fs DC2 + fs DW + 1.3 fs (4 + IM) $0.95R_hF_yf$: Composite stress capacity for Service II loading according to Article 6.10.4.2 (ksi).
- $f_{\mathcal{S}}$ (Total)(Strength I): Sum of stresses as computed below on non-compact section (ksi).
 - $1.25 (f_{SDCI} + f_{SDC2}) + 1.5 f_{SDW} + 1.75 f_{S}(L + IM)$
 - $\phi_f F_n$: Non-Compact composite positive or negative stress capacity for Strength I loading according to Article 6.10.7 or 6.10.8 (ksi).
 - V_f : Maximum factored shear range in span computed according to Article 6.10.10.

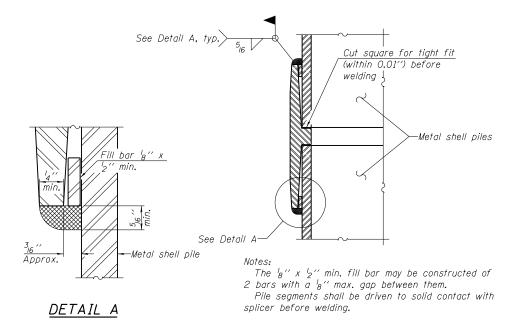
SECTION COUNTY STRUCTURAL STEEL DETAILS 777 405B-1 MONTGOMERY 121 70 **STRUCTURE NO. 068-0512** CONTRACT NO. 72DO8 SHEET NO. 15 OF 21 SHEETS



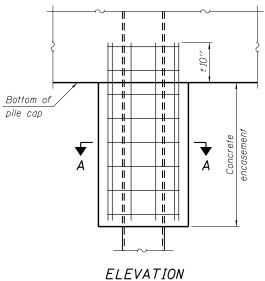


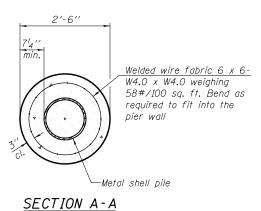
METAL SHELL PILE TABLE

Designation and outside diameter	Wall thickness t	Weight per foot (Lbs./ft.)	Inside volume (yd. ³ /ft.)
PP12	0.179''	22.60	0.0274
PP12	0.250′′	31.37	0.0267
PP14	0.250′′	36.71	0.0368
PP14	0.312"	45.61	0.0361



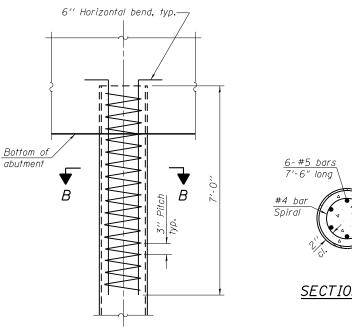
WELDED COMMERCIAL SPLICE

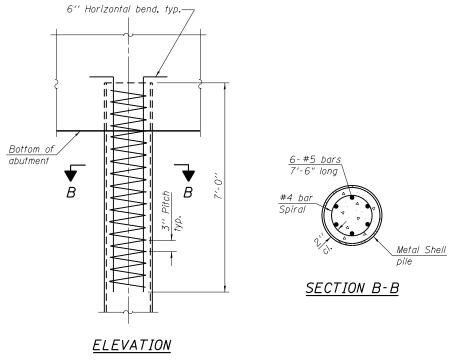




Note: Forms for encasement may be omitted when soil conditions permit.

CONCRETE ENCASEMENT AT PIERS



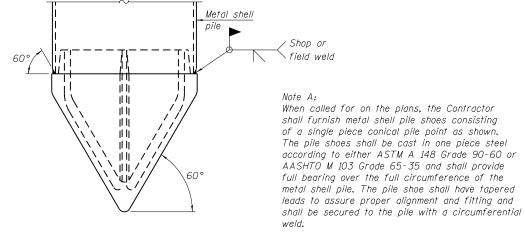


METAL SHELL REINFORCEMENT AT ABUTMENTS

Metal shell pile 3₄'' End plate Shop or field weld

s = t - 16"

END PLATE ATTACHMENT



METAL SHELL PILE SHOE ATTACHMENT

COMPLETE PENETRATION WELD SPLICE

Metal shell

* Field fabricated backing ring may be made from pile shell by removing segment to allow reducing circumference and vertically rejoin with partial joint penetration weld.

Note:

/ * Shop or

∖ field weld

Field fabricated

or commercial

backing ring

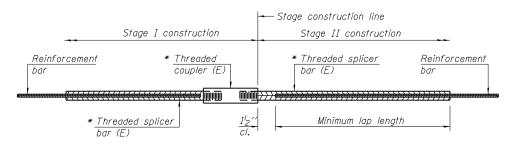
The metal shell piles shall be according to ASTM A 252 Grade 3.

F-MS 1-27-12 whks

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STATE OF	ILLINOIS
DEPARTMENT OF	TRANSPORTATION

METAL SHELL PILE DETAILS	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 068-0512	777	405B-1	MONTGOMERY	121	72
31NOCIONE NO. 000-0312			CONTRACT	NO. 7	2D08
SHEET NO. 17 OF 21 SHEETS	ILLINOIS FED. AID PROJECT				



STANDARD BAR SPLICER ASSEMBLY

Minimum Lap Lengths										
Bar size to be spliced	Table 1	Table 2	Table 3	Table 4	Table 5	Table 6				
3, 4	1'-5''	1'-11''	2'-1''	2'-4''	2'-7''	2'-11''				
5	1'-9''	2'-5"	2'-7''	2'-11''	3'-3''	3'-8''				
6	2'-1''	2'-11''	3'-1''	3′-6′′	3′-10′′	4'-5''				
7	2'-9''	3'-10''	4'-2"	4'-8''	5′-2′′	5′-10′′				
8	3′-8′′	5′-1′′	5′-5′′	6'-2''	6′-9′′	7′-8′′				
9	4'-7''	6′-5′′	6'-10''	7'-9''	8'-7''	9'-8''				

Table 1: Black bar, 0.8 Class C

Table 2: Black bar, Top bar lap, 0.8 Class C

Table 3: Epoxy bar, 0.8 Class C

Table 4: Epoxy bar, Top bar lap, 0.8 Class C

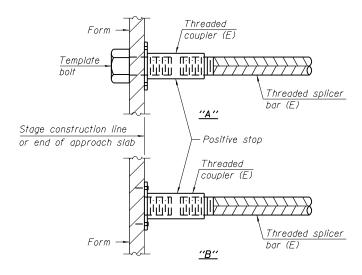
Table 5: Epoxy bar, Class C

Table 6: Epoxy bar, Top bar top, Class C

Threaded splicer bar length = min. lap length + 1^{l_2} " + thread length

* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

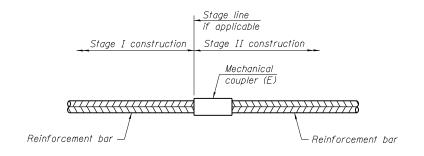
	Bar	No. assemblies	Table for minimum
Location	size	required	lap length
Bridge top of deck	#5	149	3
Bridge bottom of deck	#5	90	3
Integral diaphragm - front face	#6	10	3
Integral diaphragm - back face	#6	8	4
Approach top of deck	#4	50	3
Approach bottom of deck	#5	92	3
Approach footing	#5	80	3
Abutment cap	#7	28	4



INSTALLATION AND SETTING METHODS

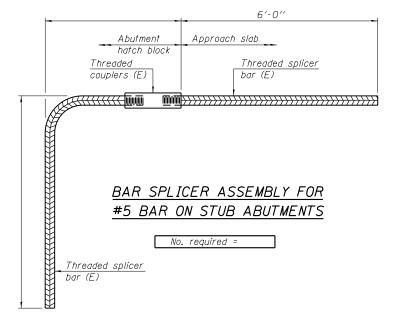
"A": Set bar splicer assembly by means of a template bolt.
"B": Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E): Indicates epoxy coating.



STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required



Notes:

- Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength
- 2. All reinforcement shall be lapped and tied to the splicer bars.
- 3. Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications.
- 4. See approved list of bar splicer assemblies and mechanical splicers for alternatives.

BSD-1

8-31-12

engineers + planners + land surveyors

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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BAR SPLICER ASSEMBLY AND MEC	HANICAL SPLICER DETAILS					
STRUCTURE NO. 068-0512						
SHEET NO. 18 OF 21	SHEETS					

	ILLINOIS	FED. AI	D PROJECT		
			CONTRACT	NO. 7	2D08
777	405B-1		MONTGOMERY	121	73
F.A.P. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.

(A)	Illinois Department of Transportation
	Division of Highways

SOIL BORING LOG

Page $\underline{1}$ of $\underline{2}$

15

	Division of Highw District 6	ays								Date	2/2	1/12
ROUTE	IL-185	DESCRIP	TION _		0	ver Mo	David Branch Creek	LOGO	ED B	Y!	/I. Tap	oan
SECTION _	405B-	1	LOCAT	ION _	SW 1/	4, SEC	2. 27, TWP. 8N, RNG. 3V	N, 3 PM				
COUNTY	Montgomery	DRILI	LING ME	THOD			HSA	HAMMER TYPE		140 #	AUTO)
STRUCT. NO.	068-0	0026	D	В	U	M	Surface Water Elev	590.2 ft	D	В	U	M

310+31 U W S c s 589.2 ft Station Stream Bed Elev. 0 S W 1 W. Abut. BORING NO. Groundwater Elev.: Qи S Qu 591.2 ft Station 309+89 ☑ First Encounter Offset 12.0ft LT ▼Upon Completion ____Washed__ft Ground Surface Elev. 604.2 ft (ft) /6" (tsf) (%) ▼AfterPluggedHrs. /6" (tsf) (%) Brown and Olive Gray Moist SILTY CLAY (Fill) Gray Medium SANDY GRAVEL Washed

Gray Dirty Fine SAND 2 .80 21 Washed 2 2 6 3 Olive Brown and Gray Moist CLAY LOAM (Till) (Fill) Gray Medium SANDY GRAVEL 2 3 6 В 9

3 Washed 17 1.1 4 2 9 Dark Gray Moist LOAM to SILTY CLAY LOAM 1 .40 11

Gray Dirty Medium SAND Washed Brown Medium to Coarse SAND Brown and Gray Moist SILTY CLAY Washed

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer, E-Estimated) Abbreviations W.O.H - Sampler Advanced By Weight of Hammer, W.O.P - Advanced by Weight of Pipe, B.S. - Before Seating The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)

.80 12 B

2



ROUTE | IL-185 | DESCRIPTION |

SOIL BORING LOG

Page <u>2</u> of <u>2</u>

Date 2/21/12

LOGGED BY M. Tappan

CAT	ON _	SW 1/	4, SEC	. 27, TWP. 8N, RNG. 3V	V, 3 PM					
3 ME	THOD			HSA	HAMMER T	YPE		140#	AUTO)
D E P	B L O	C S	M 0 1				D E P	B L O	u c s	M 0 1
Н	w s	Qu					H	w s	Qu	S T
(π)	/6"	(tst)	(%)	Gray Moist SILTY CLA		ft	(π)	16	(tsr)	(%)
	8			(continued) Washed				1		
	15 30	5.4 B	14				-65	6	2.7 B	20
	D E P T H (ft)	D B L P O T W H S (ft) /6"	METHOD D B U C C P O S T W H S Qu (ft) /6" (tsf)	METHOD D B U M E L C O P O S I T W S H S Qu T (ft) /6" (tsf) (%)	B METHOD D B	D B U M Surface Water Elev. 590.2 P O S I T W S Qu T T VIFIS Encounter 591.2 (ft) /6" (tsf) (%) Gray Moist SiLTY CLAY (Till) (continued) 8	B METHOD HSA HAMMER TYPE D B U ME L C O O S I Stream Bed Elev. Stream Bed Elev. Groundwater Elev.: First Encounter S91.2 ft Y Upon Completion Washed ft Y Upon Completion Washed ft (continued) Gray Moist SiLTY CLAY (Till) (continued) Washed Washed	B	D B U M Strface Water Elev. 590.2 ft D B L C O P O S I T W S H S Qu T T W Don Completion Washed ft ft ft C C C C C C C C C	D B U M Strace Water Elev. 590.2 ft D B U C C C C C C C C C

over McDavid Branch Creek

Brown and Gray Moist CLAY LOAM (Till) Washed Washed 12 4.6 15 6 3.0 17 8 Boring Completed Gray Moist SILTY CLAY (Till) Ref, Sta. to Centerline of Ex. Structure = 310+31 Sta. Increase to East

Ref. Elev. to Chsld Square on NE Approach Slab = 604.2 Washed 5 4.5 10 B

File Name S.\SOILS\GINT FILES\MONTGOMERY\\u00f3086.GPJ Dala Template DBT Laffuda 39.05.158\\u00bb Longlude 89.24.915\\u00f3\\u00bb Dalum NAD83 Job Number D-86-109-09 10 Washed 8 5.0 13 B

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer, E-Estimated) Abbreviations W.O.H - Sampler Advanced By Weight of Hammer, W.O.P - Advanced by Weight of Pipe, B.S. - Before Seating The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)

whks

Washed

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PLOT DATE = 10/15/2015	CHECKED	-	BRD	REVISED

BORING LOGS		SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 068-0512	777	405B-1		MONTGOMERY	121	74
				CONTRACT	NO. 7	2D08
SHEET NO. 19 OF 21 SHEETS		ILLINO	IS FED. A	AID PROJECT		

(A)	Illinois Department of Transportation
	District C

SOIL BORING LOG

Page $\underline{1}$ of $\underline{3}$

Date	2/17/12

ROUTE IL-185 DESC	RIPTION	i		0	ver Mc	David Branch Creek	LOGGE	D BY		/l. Tapı	oan
SECTION 405B-1	LO	CATI	ON _	SW 1/	4, SEC	2. 27, TWP. 8N, RNG. 3W, 3 PM					
COUNTY Montgomery D	RILLING	ME	тнор			HSA HAMMER	TYPE		140 #	AUTO)
STRUCT. NO. 068-0026 Station 310+31 BORING NO. 2 E. Abut. Station 310+77 Offset 13.0ft RT Ground Surface Elev. 604.3		DEPTH (ft)	B L O W S	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev. 590.2 Stream Bed Elev. 589.2 Groundwater Elev.: 2 First Encounter 586.3 Jupon Completion Washed After luggedHrs.	_ ft _ ft _ ft	D E P T H	B L O W S	Ou (tsf)	M O I S T (%)
Brown to Black Moist CA-6 with CINDERS (Fill)			1			Gray Wet SANDY CLAY LOAM (continued)	583.30	_	0		
	602.30		3 3			Light Brown Dirty Medium SAND with Some Pea GRAVEL			1		
Olive Brown and Light Blue Gray Moist CLAY LOAM (Till) (Fill)		_	1				580.80	\exists	0		
			1 2	1.0 B	17	Gray Moist LOAM	500.00		0 4	.60 B	20
Gray Very Moist SILTY CLAY (Fill)	599.30)	-5 —	1			Brown Medium SAND	579.30	-25 	1		
			2 2	.70 B	22				1		
Gray Dirty Medium SAND to Gray Moist SILTY CLAY LOAM (Fill)	596.80	_	1 3 2	.80 P	29	Gray Moist LOAM Washed	576.80		4 4 3	1.1 S-10	8
	593.30	-10	0			Tan Medium to Coarse SAND	574.30	-30	0		
Very Dark Gray Moist SILTY CLAY	390.00	_	1	.60 B	27	Washed			1 3		
Gray Moist SILTY CLAY	-	-15	0 1 2	.60 B	23	Washed Tan Fine SAND		-35	4 12 21		
Light Olive Brown and Gray Moist SILTY CLAY with Very Wet Seam at 16.5 - 17.5			0 0 1	.40 B	21						
Gray Wet SANDY CLAY LOAM	586.30		0			Brown Medium to Coarse SAND with Some Medium Pea GRAVEL		_	6		
		亅	0	.20 B	24	Washed		亅	16 15		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer, E-Estimated) Abbreviations W.O.H - Sampler Advanced By Weight of Hammer, W.O.P - Advanced by Weight of Pipe, B.S. - Before Seating The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

Page <u>2</u> of <u>3</u>

Date 2/17/12

5 2.6 23 8 B

9 4.2 13

5 5.3 17 10 B

8 4.9 17 11 B

13 B

LOGGED BY M. Tappan ROUTE <u>IL-185</u> DESCRIPTION ___ over McDavid Branch Creek

SECTION 405B-1 LOCATION SW 1/4, SEC. 27, TWP, 8N, RNG, 3W, 3 PM COUNTY Montgomery DRILLING METHOD HAMMER TYPE

D E Surface Water Elev. 590.2 ft STRUCT. NO. L O W s c s С 0 0 L Stream Bed Elev. 589.2 ft Р O W S BORING NO. 2 E. Abut Qu нѕ Qu T 586.3 ft Station 310+77 ☑First Encounter ft (ft) /6" (tsf) (%) Upon Completion Offset 13.0ft RT ____Washed__ft (ft) /6" (tsf) (%) Ground Surface Elev. 604.3 Tan Medium to Coarse SAND

Washed

(continued) Brown and Olive Gray Moist SILTY CLAY (Till) Brown and Gray Moist CLAY LOAM (Till) Drilled Stiff at 42' Washed

14 5.4 15 15 B

4.3 17

Gray Moist CLAY LOAM (Till)

Washed

10 B Brown and Olive Gray Moist SILTY CLAY (Till)

Washed Washed 5 3.7 10 B

Washed Washed 4 3.0 22 7 B

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Buige, S-Shear, P-Penetrometer, E-Estimated) Abbreviations W.O.H - Sampler Advanced By Weight of Hammer, W.O.P - Advanced by Weight of Pipe, B.S. - Before Seating The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)

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BORING LOGS		SECTIO	NC		COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 068-0512	777	405B-	-1		MONTGOMERY	121	75
51110010HL 140. 000-0312					CONTRACT	NO. 7	2D08
SHEET NO. 20 OF 21 SHEETS		ILI	LINOIS	FED. AI	D PROJECT		

ROUTEIL-185 DESCR	RIPTION		O'	ver Mi	cDavid Branch Creek	LOGGED	Date <u>2/17/1</u> BY M. Tappar
SECTION405B-1	_ LOCATI	ON _	SW 1/	4, SE	C, 27, TWP, 8N, RNG, 3	W, 3 PM	
COUNTY Montgomery DF	RILLING ME	THOD			HSA	_ HAMMER TYPE	140 # AUTO
STRUCT. NO. 068-0026 Station 310+31 BORING NO. 2 E. Abut. Station 310+77 Offset 13.0ft RT Ground Surface Elev. 604.3	D E P T H	B L O W S	U C S Qu (tsf)	M O I S T	Surface Water Elev. Stream Bed Elev. Groundwater Elev.: ▽ First Encounter ▼ Upon Completion ▼ AfterPluggedHrs.		
Brown and Olive Gray Moist SILTY CLAY (Till) (continued) Gray Moist CLAY LOAM (Till) with 6" SAND LOAM Seam Washed	522.30	5 19	5.1	15			
Boring Completed Ref. Sta. to Centerline of Ex. Structure = 310+31 Sta. Increase to East	519.80	18	В		-		
Ref. Elev. to Chsid Square on NE Approach Slab = 604.2							

m whks

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The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer, E-Estimated) Abbreviations W.O.H - Sampler Advanced By Weight of Hammer, W.O.P - Advanced by Weight of Pipe, B.S. - Before Seating The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)

STATE OF	ILLINOIS
DEPARTMENT OF	TRANSPORTATION

BORING LOGS		F.A.P. SECTION		COUNTY	TOTAL SHEETS	SHEET NO.	
STRUCTURE NO. 068-0512	777	405	B-1		MONTGOMERY	121	76
31NUCTURE 140. 000-0312					CONTRACT	NO.	2D08
SHEET NO. 21 OF 21 SHEETS			ILLINOIS	FED. Al	D PROJECT		

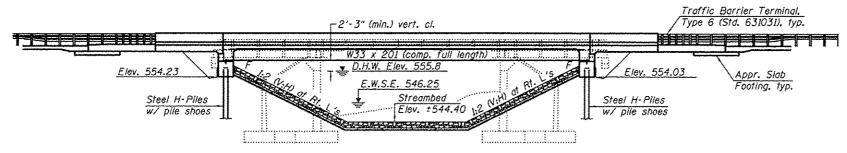
Bench Mark: Chiseled "[]" in the wingwall on the Northeast corner of Bridge S.N. 068-0027. Station 492+94.6: offset 16.6 ft Lt.; Elev. 558.7 (NAVD 88).

Existing Structure: The existing Structure No. 068-0027 is a single span precast prestressed concrete deck beam bridge originally built in 1930 and reconstructed in 1980 as Illinois Route 185 over Bayou Creek. The superstructure consists of Eleven PPC Deck Beams (3' wide by 21" deep). The beams are

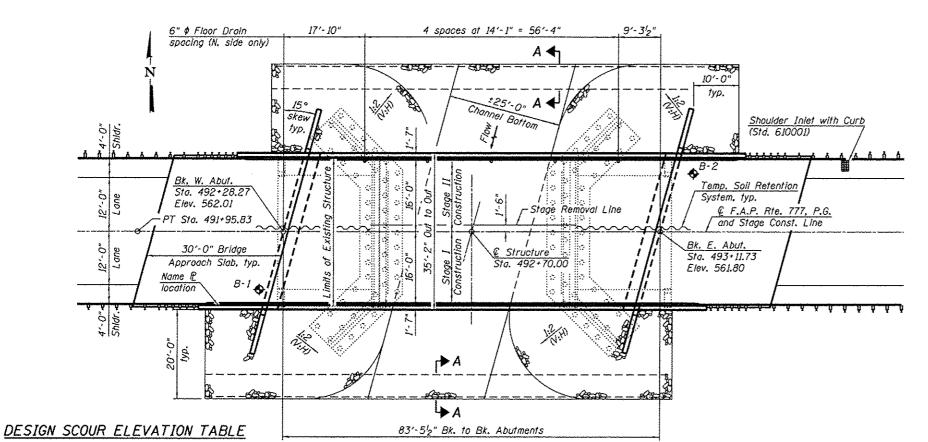
overlaid with a reinforced concrete wearing surface with 7^{l}_{2} " average thickness. The superstructure is supported by closed reinforced concrete abutments on untreated timber piles. The structure length is 43' back to back of abutments and 33' out to out of deck. Structure to be removed

and replaced utilizing stage construction.

Salvage: None



ELEVATION



Design Scour Elevations (ft.) Item W. Abut. E. Abut. II3 554.23 554.03 8

PLAN

WATERWAY INFORMATION TABLE

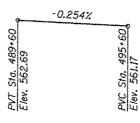
Drainage Are	ea = 5.5	sq. ml.		t. Overto Low G						
Flood	Freq.	Q	Opening	Sq. F1.	Nat.	Head	- Ft.	Headwater El.		
7 7000	Yr.	C.F.S.	Exist.	Prop.	H.W.E.	Exist.	Prop.	Exist.	Prop.	
	10	1560	350	440	554.6	0.4	0.3	555.0	554.9	
Design	50	2560	<i>3</i> 95	530	555.8	0.8	0.5	556.6	556.3	
Base	100	3020	410	560	556.3	1.0	0,6	557.3	556.9	
Max. Calc.	500	4150	450	635	557.3	2.7	1,4	560.0	558.7	

10 Yr. velocities = Exist. = 4.5 fps / Prop. = 3.6 fps

CURVE DATA

T.R. = $53'$ (PC and only)
S.E. Runoff = 177'
△ = 56° 52′ 53" (LT)
D = 4° 30′ 18"
T = 688.83'
L = 1.262.61'
E = 174.56'
R = 1,271,81'
S.E. = 5.0%
P.C. = Sta. 479+33.22
P.T. = Sta. 491+95.83
P.I. = Sta. 486+22.05

See roadway plans for SE transistion details.



PROFILE GRADE

INDEX OF SHEETS

- General Plan and Elevation
- 2, General Data
- . Stage Construction Details
- 4. Temporary Concrete Barrier for Stage Construction
- Top of Slab Location Plan
- 6. Top of Slab Elevations 7.-8. Top of Approach Slab Elevations
- 9. Superstructure
- 10. Superstructure Details
- 11. Integral Abutment Diaphragm Details
- 12.-13. Bridge Approach Slab Details
 14. Framing Plan
- 15. Structural Steel Details
- 16. Abutments

Stone Riprop

Bedding

Filter fabric

5'-6"

STATION 492+70.00
BUILT 20 BY
STATE OF ILLINOIS
F.A.P. RTE. 777 - SEC. 10B-2
LOADING HL-93
STRUCTURE NO. 068-0513

NAME PLATE
See Std. 515001

LOCATION SKETCH

SHEET NO. 1 OF 20 SHEETS

PROJECT STRUCTURE

ROW 3rd, PM

SECTION A-A

APPROVED

For Structural Adequacy Only

Engineer of Bridges & Structures

- 17. HP Pile Details
- 18. Bar Splicer Assembly and Mechanical Splicer Details
- 19.-20. Boring Logs

LOADING HL-93

Allow 50#/sq. ft. for future wearing surface.

DESIGN SPECIFICATIONS

2014 AASHTO LRFD Bridge Design Specifications, 7th Edition

DESIGN STRESSES

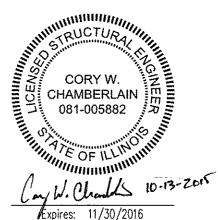
FIELD UNITS

f'c = 3,500 psi fy = 60,000 psi (Reinforcement)

fy = 50,000 psi (M270 Grade 50W)

SEISMIC DATA

Seismic Performance Zone (SPZ) = 2 Design Spectral Acceleration at 1.0 sec. (S_{DI}) = 0.232g Design Spectral Acceleration at 0.2 sec. (S_{DS}) = 0.482g Soil Site Class = D



<u>GEN</u>

GENERAL PLAN AND ELEVATION

ILLINOIS ROUTE 185 OVER BAYOU CREEK

F.A.P. RTE. 777 - SEC. 10B-2

MONTGOMERY COUNTY

STATION 492+70.00

STRUCTURE NO. 068-0513



USER NAME *	DESIGNED	-	TJZ	REVISED
FILE NAME .	CHECKED	~	BRD	REVISED
PLOT DATE .	DRAWN	•	DLH	REVISED
PLOT DATE .	CHECKED	-	TJŽ	REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

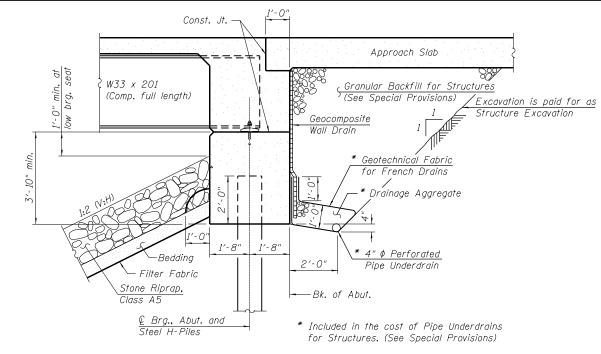
-	F.A.P. RTÉ.	SECTION	COUNTY	TOTAL	SHEET NO.
	777	108-2	MONTGOMERY	120	77
			CONTRACT	NO. 7	2D08
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GENERAL NOTES

- 1. Fasteners shall be ASTM A325 Type 3, bolts. Bolts $\frac{3}{4}$, holes $\frac{13}{16}$, ϕ , unless otherwise noted.
- 2. Calculated weight of Structural Steel = 103,870 lbs.
- 3. All structural steel shall be AASHTO M 270 Grade 50W.
- 4. No field welding is permitted except as specified in the contract documents.
- 5. Reinforcement bars designated (E) shall be epoxy coated.
- Structural steel shall only be painted for a distance equal to the depth of embedment into the concrete diaphragm plus 1'-6". Painted areas shall be primed in the shop with a Department approved zinc rich primer. Field painting will not be required.
- 7. Layout of the slope protection system may be varied to suit ground conditions in the field as directed by the Engineer.
- 8. Slipforming of the parapet is not allowed.

TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Stone Riprap, Class A5	Sq. Yd.		815	815
Filter Fabric	Sq. Yd.		<i>81</i> 5	815
Removal of Existing Structures No. 2	Each			1
Structure Excavation	Cu. Yd.		235	235
Floor Drains	Each	5		5
Concrete Structures	Cu. Yd.		66.0	66.0
Concrete Superstructure	Cu. Yd.	229.2		229.2
Bridge Deck Grooving	Sq. Yd.	475		475
Protective Coat	Sq. Yd.	604		604
Furnishing and Erecting Structural Steel	L. Sum	0.55		0.55
Stud Shear Connectors	Each	1,566		1,566
Reinforcement Bars, Epoxy Coated	Pound	47,390	11,640	59,030
Bar Splicers	Each	396	108	504
Furnishing Steel Piles HP 12x53	Foot		400	400
Driving Piles	Foot		400	400
Test Pile Steel HP 12x53	Each		2	2
Pile Shoes	Each		12	12
Name Plates	Each	1		1
Anchor Bolts, 1"	Each		24	24
Geocomposite Wall Drain	Sq. Yd.		68	68
Granular Backfill for Structures	Cu. Yd.		120	120
Asbestos Bearing Pad Removal	Each		20	20
Pipe Underdrains for Structures 4"	Foot	166		166
Temporary Soil Retention System	Sq. Ft.		535	535



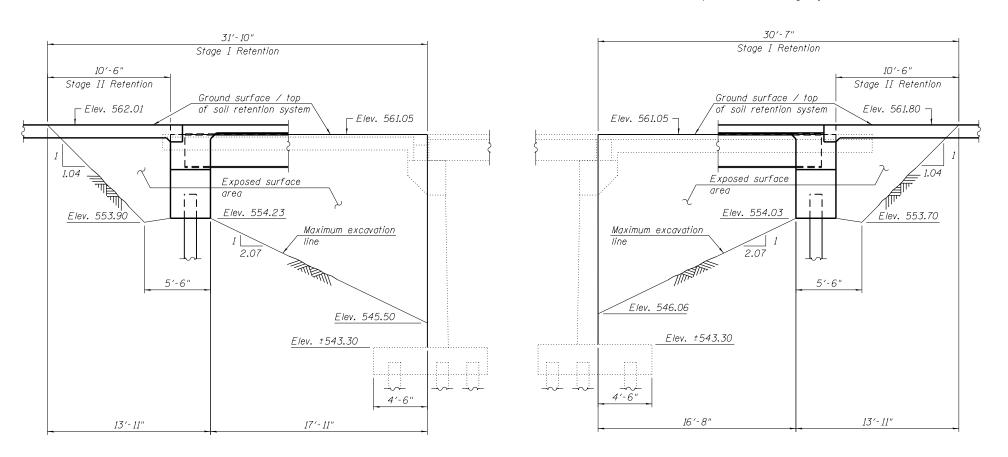
SECTION THRU INTEGRAL ABUTMENT

(Horiz, dimensions are at Rt. L's.)

All drainage system components shall extend to 2'-0" from the end of each wingwall except an outlet pipe shall extend until intersecting with the side slopes. The pipes shall drain into concrete headwalls. (See Article 601.05 of the Standard Specifications and Highway Standard 601101).

MONTGOMERY 121 78

CONTRACT NO. 72D08



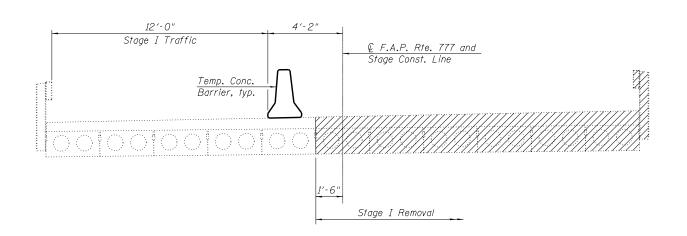
TEMPORARY SOIL RETENTION SYSTEM

(Looking North, Dimensions shown along Stage Const. Line.)

A cantilevered sheet piling design does not appear feasible and additional members or other retention systems may be necessary. The Contractor shall submit a temporary soil retention system design including plan details and calculations for review and acceptance by the Engineer.



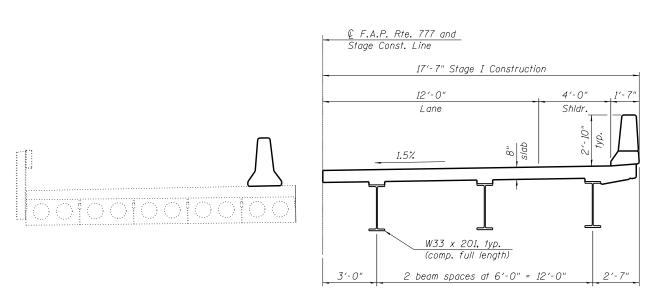
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FILE NAME =	CHECKED - BRD	REVISED	STATE OF ILLINOIS	STRUCTURE NO. 068-0513	777	10B-2	MONTGOMER
PLOT SCALE =	DRAWN - DLH	REVISED	DEPARTMENT OF TRANSPORTATION	STRUCTURE INC. U00-U313			CONTRAC
PLOT DATE =	CHECKED - TJZ	REVISED		SHEET NO. 2 OF 20 SHEETS		ILLINOIS FED.	AID PROJECT



Stage II Removal | C. F.A.P. Rte. 777 and | Stage Const. Line | 4'-0" | 12'-0" | Stage II Traffic | 1'-6"

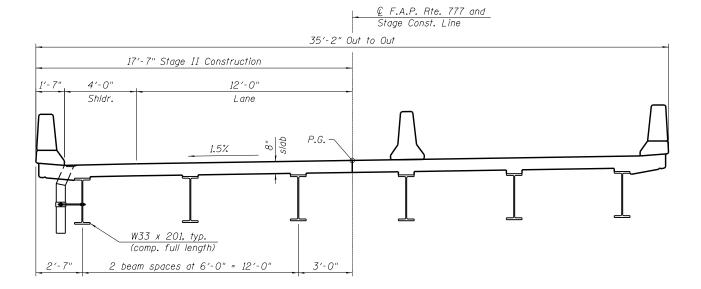
STAGE I REMOVAL

(Looking East)



STAGE II REMOVAL

(Looking East)



STAGE I CONSTRUCTION

(Looking East)

STAGE II CONSTRUCTION

(Looking East)

Notes:

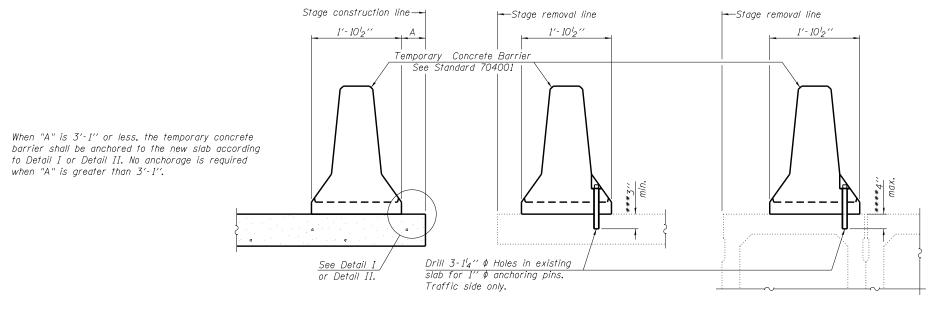
- 1. Hatched area indicates Removal of Existing Structures.
- 2. For details of Temporary Concrete Barrier, see sheet 4 of 20.
- 3. See roadway plans for quantity of Temporary Concrete Barrier.

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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

STAGE CONSTRUCTION DETAILS		SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 068-0513	777	10B-2	MONTGOMERY	121	79
31NOCIONE NO. 000-0313			CONTRACT	NO. 7	2D08
SHEET NO. 3 OF 20 SHEETS		ILLINOIS FED. A	D PROJECT		



NOTES

Detail I - With Bar Splicer or Couplers:

Connect one (1) $1'' \times 7' \times ''W''$ steel ℓ to the top layer of couplers with $2^{-5}8'' \neq bolts$ screwed to coupler at approximate ℓ of each barrier panel.

Detail II - With Extended Reinforcement Bars:

Connect one (1) I'' x 7'' x 'W'' steel P to the concrete slab or concrete wearing surface with 2-58'' \$\phi\$

Expansion Anchors or cast in place inserts spaced between the top layer of reinforcement at approximate \$\mathbb{C}\$ of each barrier panel.

Cost of retainer assembly is included with Temporary Concrete Barrier. The $I^{\prime\prime}$ x $7^{\prime\prime}$ x $^{\prime\prime}$ W $^{\prime\prime}$ plate shall not be removed until stage II construction forms and all reinforcement bars are in place and the concrete is ready to be placed.

SECTIONS THRU SLAB OR DECK BEAM

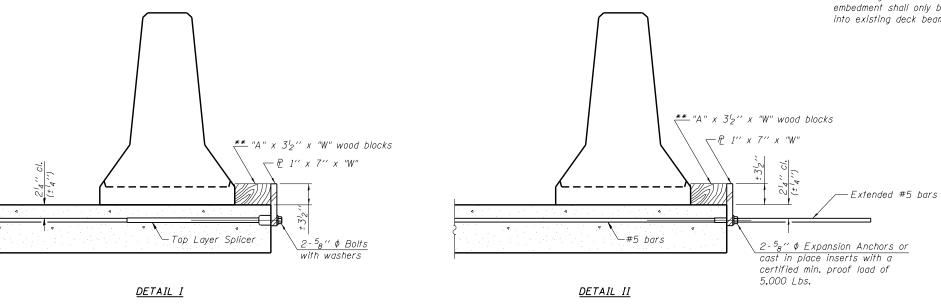
EXISTING SLAB

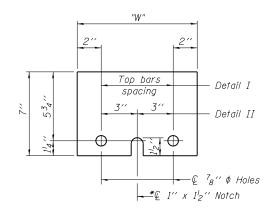
*** Dimension shown is minimum required embedment into concrete.

If hot-mix asphalt wearing surface is present, minimum embedment shall be in addition to wearing surface depth.

EXISTING DECK BEAM

**** If existing deck beam is to remain in place after stage construction, embedment shall only be into wearing surface and not into existing deck beam concrete.





STEEL RETAINER P I'' x 7'' x ''W'' * Required only with Detail II

RETAINER ASSEMBLY

*** Wood blocks may be omitted when required to provide minimum stage traffic lane width. When the wood blocks are omitted, the concrete barrier shall be in direct contact with the steel retainer plate.

R-27

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to 184001036

which
engineers + planners + land surveyors

1- 12 - 15

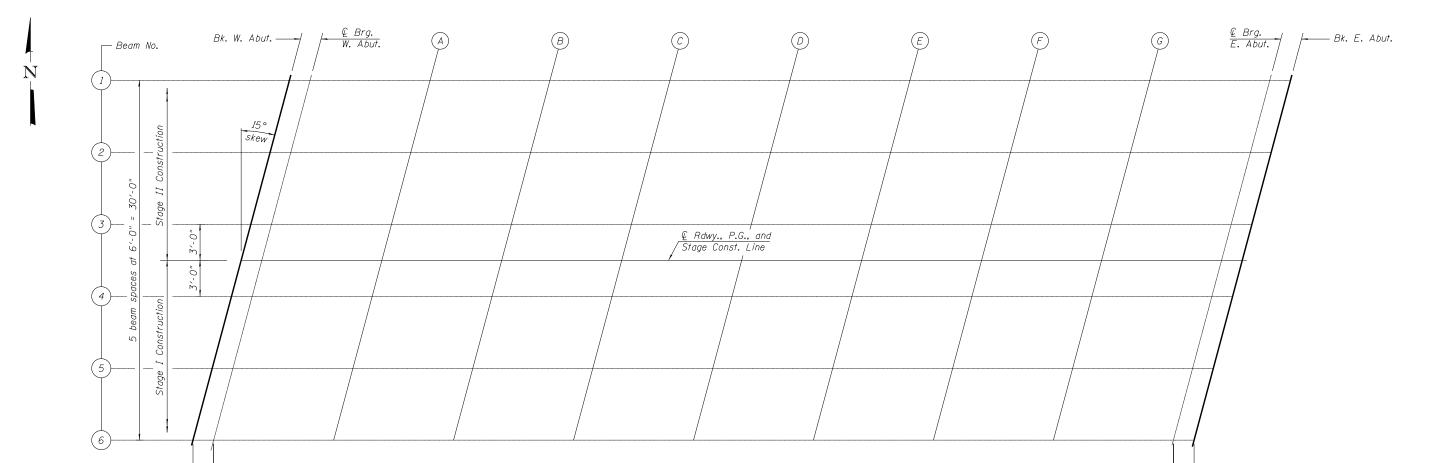
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PLOT DATE =	CHECKED	-	TJZ	REVISED

NEW SLAB

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

TEMPORARY CONCRETE BARRIER FOR STAGE CONSTRUCTION	F.A.P. RTE.	
STRUCTURE NO. 068-0513	777	
31110C1OHL NO. 000-0313		
CHEET NO. 4 OF SO CHEETC		

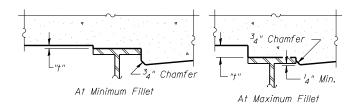
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
777	10B-2	MONTGOMERY	121	80	
		CONTRACT	NO. 7	2D08	
ILLINOIS FED. AID PROJECT					



PLAN

8 spaces at 10'-0" = 80'-0"

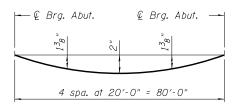
80′-0" € to € Bearings



1'-83_{4"}

To determine "t": After all structural steel has been erected, elevations of the top flanges of the beams shall be taken at intervals shown above. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflection" shown on sheet 6 of 20, minus slab thickness, equals the fillet heights "t" above top flange of beams.

FILLET HEIGHTS



DEAD LOAD DEFLECTION DIAGRAM

(Includes weight of concrete only.)

The above deflections are not to be used in the field if the Engineer is working from the grade elevations adjusted for dead load deflections as shown on sheet 6 of 20.

Note:

1'-834"

Work this sheet with sheet 6 of 20.

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PLOT DATE =	CHECKED	-	TJZ	REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TOP OF SLAB LOCATION PLAN STRUCTURE NO.068-0513	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	777	10B-2	MONTGOMERY	121	81
31NUCTURE NO. 000-0313			CONTRACT	NO. 7	2D08
SHEET NO. 5 OF 20 SHEETS		ti i tuoto eco i	ID DDG IFOT		

<u>BEAM 1</u>

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection	
Bk. W. Abut.	492+32.29	-15.00	561.78	561.78	
© Brg. W. Abut.	492+34.01	-15.00	561.77	561.77	
A B C D E F G	492+44.01 492+54.01 492+64.01 492+74.01 492+84.01 492+94.01 493+04.01	-15.00 -15.00 -15.00 -15.00 -15.00 -15.00 -15.00	561.75 561.72 561.69 561.67 561.64 561.62 561.59	561.81 561.83 561.84 561.83 561.79 561.73 561.66	
© Brg. E. Abut.	493+14.01	-15.00	561.57	561.57	
Bk. E. Abut.	493+15.75	-15.00	561.56	561.56	

<u>BEAM 2</u>

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	492+30.68	-9.00	561.87	561.87
₡ Brg. W. Abut.	492+32.41	-9.00	561.86	561.86
A B C D E F G	492+42.41 492+52.41 492+62.41 492+72.41 492+82.41 492+92.41 493+02.41	-9.00 -9.00 -9.00 -9.00 -9.00 -9.00	561.84 561.81 561.79 561.76 561.74 561.71	561.90 561.93 561.94 561.93 561.89 561.83 561.75
© Brg. E. Abut.	493+12.41	-9.00	561.66	561.66
Bk. E. Abut.	493+14.14	-9.00	561.66	561.66

BEAM 3

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	492+29.07	-3.00	561.96	561.96
€ Brg. W. Abut.	492+30.80	-3.00	561.96	561.96
A B C D E F G	492+40.80 492+50.80 492+60.80 492+70.80 492+80.80 492+90.80 493+00.80	-3.00 -3.00 -3.00 -3.00 -3.00 -3.00 -3.00	561.93 561.91 561.88 561.86 561.83 561.81	562.00 562.02 562.03 562.02 561.98 561.92 561.84
₡ Brg. E. Abut.	493+10.80	-3.00	561.76	561.76
Bk. E. Abut.	493+12.53	-3.00	561.75	561.75

@ ROADWAY, P.G., AND STAGE CONSTRUCTION LINE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	492+28.27	0.00	562.01	562.01
€ Brg. W. Abut.	492+30.00	0.00	562.01	562.01
A B C D E F G	492+40.00 492+50.00 492+60.00 492+70.00 492+80.00 492+90.00 493+00.00	0.00 0.00 0.00 0.00 0.00 0.00	561.98 561.96 561.93 561.90 561.88 561.85	562.04 562.07 562.08 562.07 562.03 561.97 561.89
© Brg. E. Abut.	493+10.00	0.00	561.80	561.80
Bk. E. Abut.	493+11.73	0.00	561.80	561.80

BEAM 4

<u> </u>				
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	492+27.47	3.00	562.06	562.06
© Brg. W. Abut.	492+29.19	3.00	562.05	562.05
A B C D E F G	492+39.19 492+49.19 492+59.19 492+69.19 492+79.19 492+89.19 492+99.19	3.00 3.00 3.00 3.00 3.00 3.00 3.00	562.03 562.00 561.98 561.95 561.93 561.90 561.88	562.09 562.12 562.13 562.11 562.08 562.02 561.94
© Brg. E. Abut.	493+09.19	3.00	561.85	561.85
Bk. E. Abut.	493+10.92	3.00	561.85	561.85

<u>BEAM 5</u>

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	492+25.86	9.00	562.15	562.15
© Brg. W. Abut.	492+27.58	9.00	562.15	562.15
A B C D E F G	492+37.58 492+47.58 492+57.58 492+67.58 492+77.58 492+87.58 492+97.58	9.00 9.00 9.00 9.00 9.00 9.00	562.12 562.10 562.07 562.05 562.02 562.00 561.97	562.18 562.21 562.22 562.21 562.17 562.11 562.03
© Brg. E. Abut.	493+07.58	9.00	561.94	561.94
Bk. E. Abut.	493+09.31	9.00	561.94	561.94

<u>BEAM 6</u>

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut.	492+24.25	15.00	562.25	562.25
€ Brg. W. Abut.	492+25.98	15.00	562.24	562.24
A B C D E F G	492+35.98 492+45.98 492+55.98 492+65.98 492+75.98 492+85.98 492+95.98	15.00 15.00 15.00 15.00 15.00 15.00	562.22 562.19 562.17 562.14 562.11 562.09 562.06	562.28 562.31 562.31 562.30 562.26 562.20 562.13
© Brg. E. Abut.	493+05.98	15.00	562.04	562.04
Bk. E. Abut.	493+07.71	15.00	562.03	562.03

Note:

Work this sheet with sheet 5 of 20.

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PLOT SCALE =	DRAWN	-	DLH	REVISED
PLOT DATE =	CHECKED	-	TJZ	REVISED

NORTH EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations
W. End W. Appr. Pav't.	492+03.59	-16.00	561.83
A 1 A 2	492+13.59 492+23.59	-16.00 -16.00	561.81 561.78
E. End W. Appr. Pav't.	492+33.59	-16.00	561.76

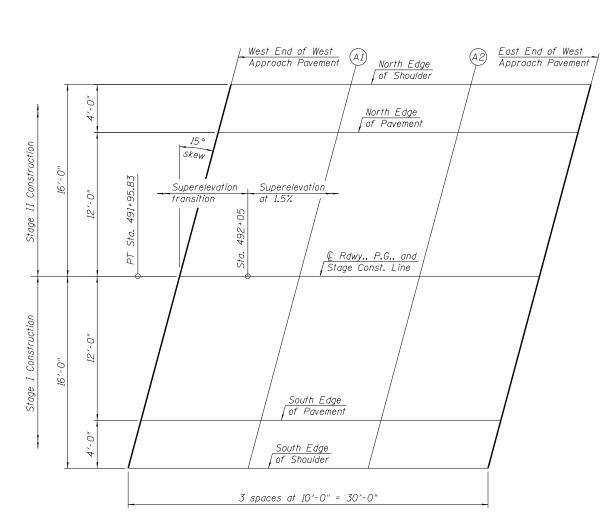
NORTH EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
W. End W. Appr. Pav't.	492+02.52	-12.00	561.90
A 1 A 2	492+12.52 492+22.52	-12.00 -12.00	561.87 561.84
E. End W. Appr. Pav't.	492+32.52	-12.00	561.82

@ ROADWAY, P.G., AND STAGE CONSTRUCTION LINE

Location	Station	Offset	Theoretical Grade Elevations
W. End W. Appr. Pav't.	491+99.31	0.00	562.08
A1 A2	492+09.31 492+19.31	0.00 0.00	562.06 562.03
E. End W. Appr. Pav't.	492+29.31	0.00	562.01

N



<u>PLAN</u>

SOUTH EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
W. End W. Appr. Pav't.	491+96.09	12.00	562.30
A 1 A 2	492+06.09 492+16.09	12.00 12.00	562.25 562.22
E. End W. Appr. Pav't.	492+26.09	12.00	562.20

SOUTH EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations
W. End W. Appr. Pav't.	491+95.02	16.00	562.38
A 1 A2	492+05.02 492+15.02	16.00 16.00	562.31 562.28
E. End W. Appr. Pav't.	492+25.02	16.00	562.26

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LOT SCALE =	DRAWN	-	DLH	REVISED
LOT DATE =	CHECKED	-	TJZ	REVISED

NORTH EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations
W. End E. Appr. Pav't.	493+14.98	-16.00	561.55
A3 A4	493+24.98 493+34.98	-16.00 -16.00	561.53 561.50
E. End E. Appr. Pav't.	493+44.98	-16.00	561.47

NORTH EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
W. End E. Appr. Pav't.	493+13.91	-12.00	561.61
A3 A4	493+23.91 493+33.91	-12.00 -12.00	561.59 561.56
E. End E. Appr. Pav't.	493+43.91	-12.00	561.54

€ ROADWAY, P.G., AND STAGE CONSTRUCTION LINE

Location	Station	Offset	Theoretical Grade Elevations
W. End E. Appr. Pav't.	493+10.69	0.00	561.80
A3 A4	493+20.69 493+30.69	0.00 0.00	561.78 561.75
E. End E. Appr. Pav't.	493+40.69	0.00	561.73

N

| West End of East | Approach Pavement | A | A

<u>PLAN</u>

SOUTH EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
W. End E. Appr. Pavít.	493+07.48	12.00	561.99
A3 A4	493+17.48 493+27.48	12.00 12.00	561.96 561.94
E. End E. Appr. Pav't.	493+37.48	12.00	561.91

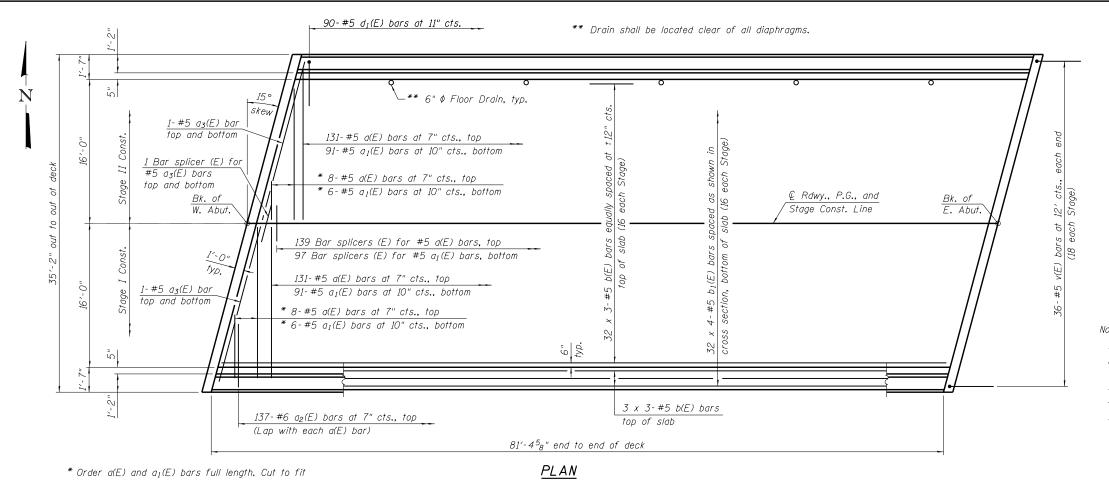
SOUTH EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations
W. End E. Appr. Pav't.	493+06.41	16.00	562.05
A3 A4	493+16.41 493+26.41	16.00 16.00	562.03 562.00
E. End E. Appr. Pav't.	493+36.41	16.00	561.98

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LOT SCALE =	DRAWN	-	DLH	REVISED
LOT DATE =	CHECKED	-	TJZ	REVISED

TOP	0F	EAST	APP	RO/	ACH	S	LAB	ELEVATIONS	
		STR	UCTU	RE	NC	0. 0	68–0	513	
		SHF	FT NO.	8	OF	20	SHEE:	TS	

F.A.P. RTE.	SECT	TION			COUNTY	TOTAL SHEETS	SHEET NO.
777	10E	3-2			MONTGOMERY	121	84
					CONTRACT	NO. 7	2D08
		ILLINOIS	FED.	A)	D PROJECT		

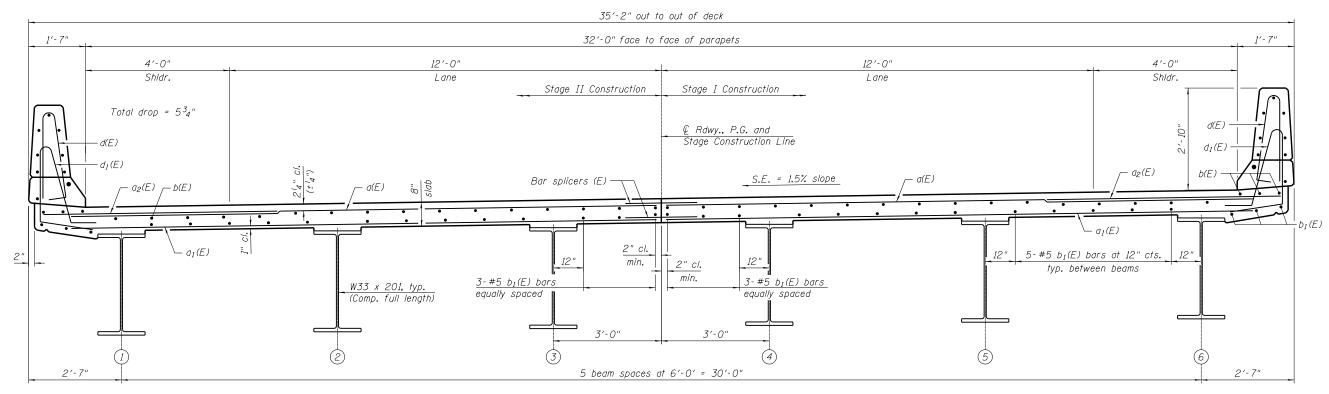


skew and use remainder of bars in opposite end.

- 1. See sheet 10 of 20 for superstructure details and Bill of Material.
- 2. Bars indicated thus 32 x 3-#5 etc. indicates 32 lines of bars with 3 lengths per line.
- 3. See sheet 10 of 20 for parapet reinforcement.
- 4. See sheet 18 of 20 for bar splicer details.
- 5. See sheet 1 of 20 for floor drain locations.

MINIMUM BAR LAP

#5 bar = 2'-7"



CROSS SECTION

(Looking East)

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	engineers + planners + land surveyors

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FILE NAME =	CHECKED	-	BRD	REVISED
PLOT SCALE =	DRAWN	-	DLH	REVISED
PLOT DATE =	CHECKED	-	TJZ	REVISED

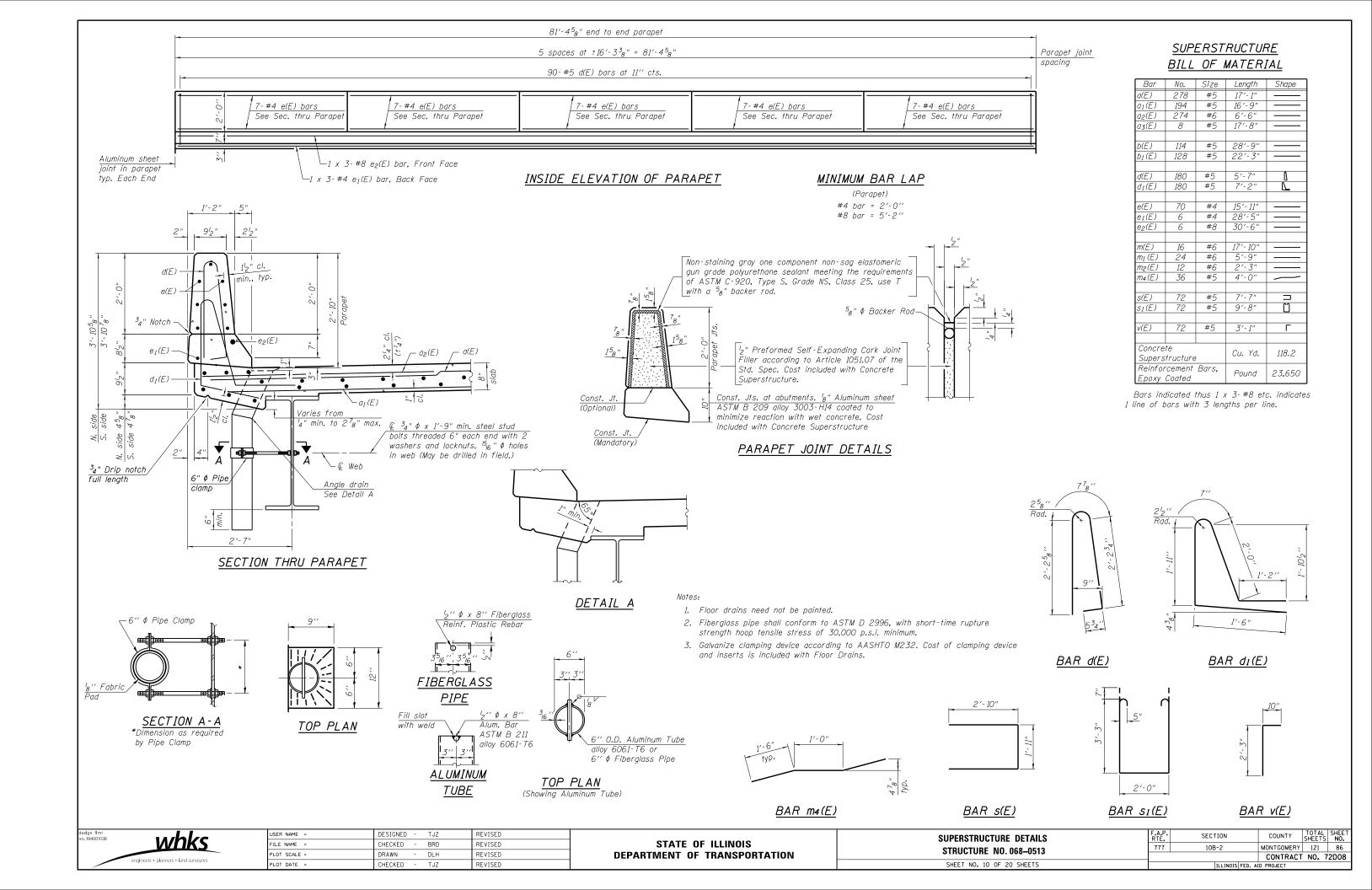
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

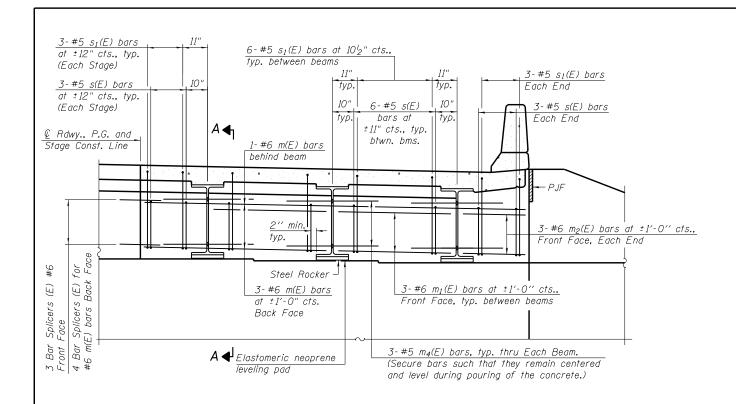
SUPERSTRUCTURE STRUCTURE NO. 068–0513		SECTION		COUNTY	
		10B-2		MONTGOME	
				CONTRA	
SHEET NO. 9 OF 20 SHEETS		ILLINOIS FE	D. AII	D PROJECT	

COUNTY

MONTGOMERY 121 85

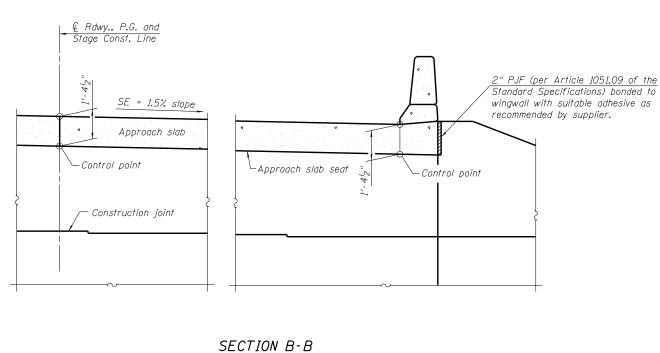
CONTRACT NO. 72DO8



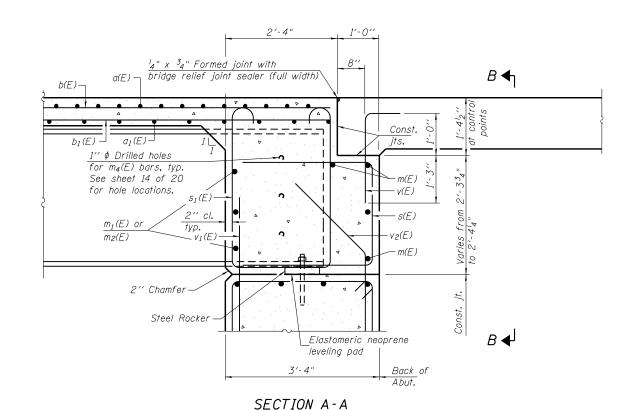


DIAPHRAGM ELEVATION AT ABUTMENT

(Looking West)



(Looking West)



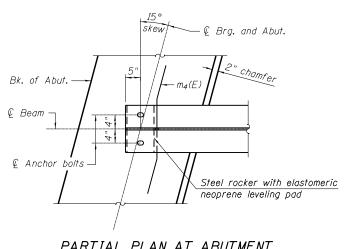
TOP OF CORBEL ELEVATIONS

Abutment	Location	Corbel Elevation
	Inside Face of North Parapet	560.38
West Abutment	Centerline Roadway	560.63
	Inside Face of South Parapet	560 . 88
	Inside Face of North Parapet	560 . 18
East Abutment	Centerline Roadway	560.43
	Inside Face of South Parapet	560.68

Notes:

(at Rt. L's)

- 1. Reinforcement bars in diaphragm are billed with superstructure on sheet 10 of 20.
- 2. Concrete in diaphragm is included with Concrete Superstructure on sheet 10 of 20.
- 3. For details of bars s(E), $s_1(E)$ and v(E) see sheet 10 of 20.
- 4. The approach slab seat shall have a constant slope determined from the control points shown.
- 5. For bearing details see sheet 15 of 20.



PARTIAL PLAN AT ABUTMENT

(Showing bottom flange of beam)

whks

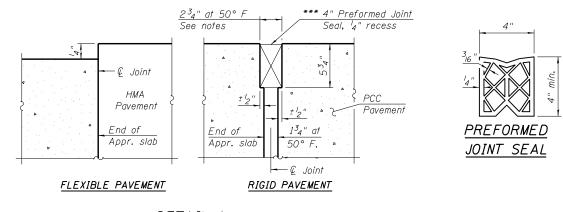
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PLOT DATE =	CHECKED - TJZ	REVISED

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

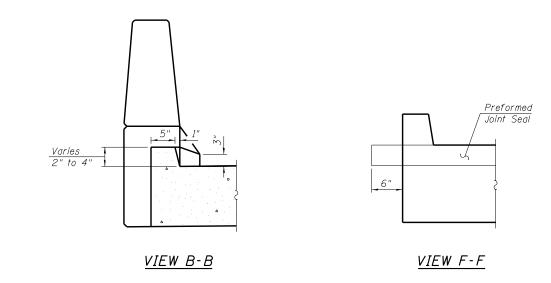
INTEGRAL ABUTMENT DIAPHRAGM DETAILS	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 068-0513	777	10B-2	MONTGOMERY	121	87
31110C1011L 140.000-0313			CONTRACT	NO. 7	2D08
SHEET NO. 11 OF 20 SHEETS		ILLINOIS FED. A	D PROJECT		

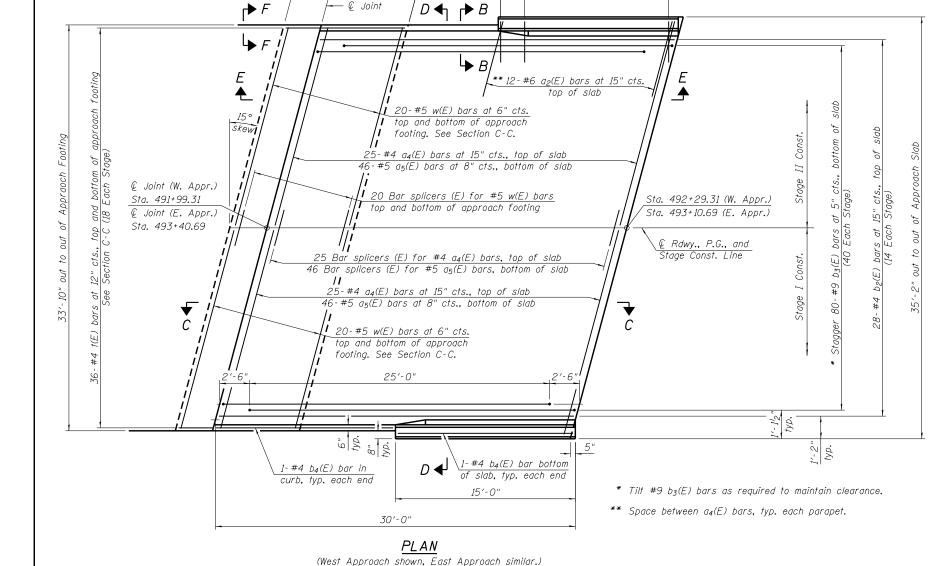


- 1. See sheet 13 of 20 for Sections C-C & D-D and View E-E.
- 2. $a_4(E)$ and $a_5(E)$ bar spacings measured along \mathcal{C} Rdwy.
- 3. The joint opening shall be determined per Article 520.04 except that on jointless structures, the distance described as the bridge length between the nearest fixed bearings each way from the joint shall be taken as half the bridge length plus the approach slab length. The minimum dimension shall be 1_2^{\prime} " for installation purposes.
 - *** Cost included with Concrete Superstructure.



DETAIL A





17-#5 d₂(E) bars at 11" cts., typ.

Bend 3-#5 d₂(E) bars

to fit taper, typ.

Approach Footing

€ Joint

See Hwy. Std. 420401

for pavement connector

(Sheet 1 of 2)

SECTION COUNTY **BRIDGE APPROACH SLAB DETAILS** STATE OF ILLINOIS 777 10B-2 MONTGOMERY 121 88 STRUCTURE NO. 068-0513 **DEPARTMENT OF TRANSPORTATION** CONTRACT NO. 72D08 SHEET NO. 12 OF 20 SHEETS

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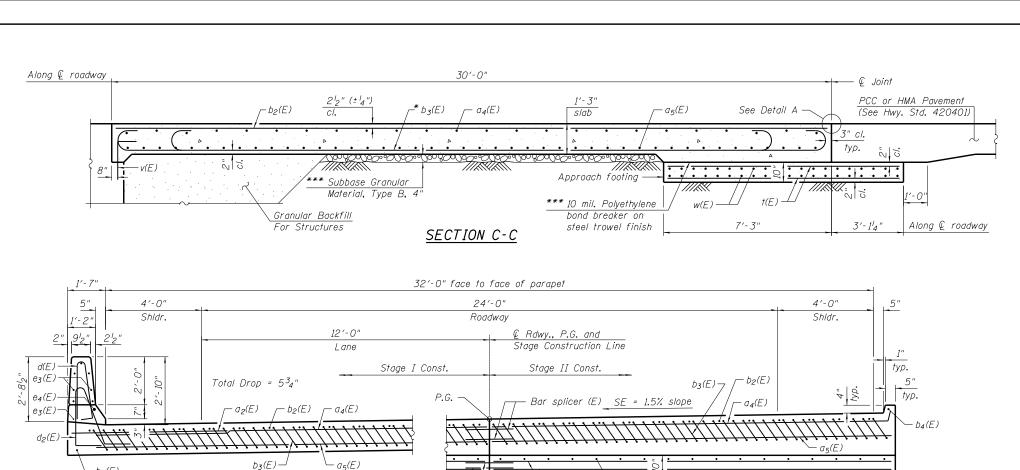
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Appr N



NEAR ABUTMENT

b3(E) -

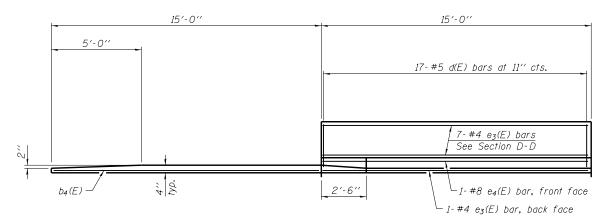
- b4(E)

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SECTION D-D

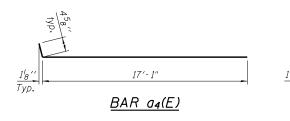
___ t(E)

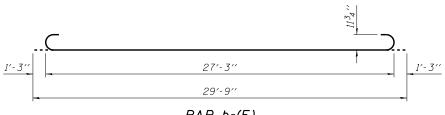
(See plan for dimensions not shown.)



Bar splicer (E)

VIEW E-E

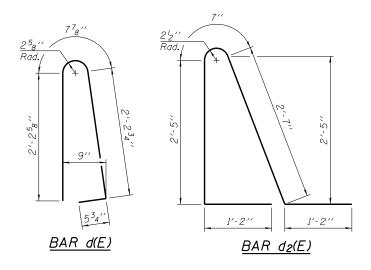




BAR b3(E)

Notes:

- 1. See sheet 12 of 20 for Detail A and View B-B.
- 2. Approach slab and parapet concrete shall be paid for as Concrete Superstructure.
- 3. Approach footing concrete shall be paid for as Concrete Structures.
- 4. Reinforcement shall be paid for as Reinforcement Bars, Epoxy Coated.
- 5. For v(E) bar details, see sheet 10 of 20.
- 6. For bar splicer details, see sheet 18 of 20.
- 7. Cost of excavation for approach footing included with Concrete Structures.
- 8. For Granular Backfill for Structures and drainage treatment details, see sheet 2 of 20.
- 9. For additional parapet details, see sheet 10 of 20.



- * Tilt #9 $b_3(E)$ bars as required to maintain clearance.
- *** Cost included with Concrete Superstructure.

TWO APPROACHES BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a ₂ (E)	48	#6	6'-6"	
a4(E)	100	#4	17′-6"	
a5(E)	184	#5	17'-2"	
b2(E)	56	#4	29′-8"	
b3(E)	160	#9	29'-9"	
b4(E)	8	#4	14'-8"	
d(E)	68	#5	5′-7"	Λ.
	68	#5	7'-11"	17
d2(E)	00	#5	7 - 11	L_
ез(E)	32	#4	14'-8"	
e4(E)	4	#8	14'-8"	
t(E)	144	#4	10'-0"	
(F)	160	#5	17'-2"	
w(E)	100	#5	1/ -2	_
Concre	te Struc	tures	Cu. Yd.	21.8
Concre		Cu. Yd.	111.0	
	structure			
	rcement Coated	Bars,	Pound	27,570
Bar Sp			Each	222

(Sheet 2 of 2)

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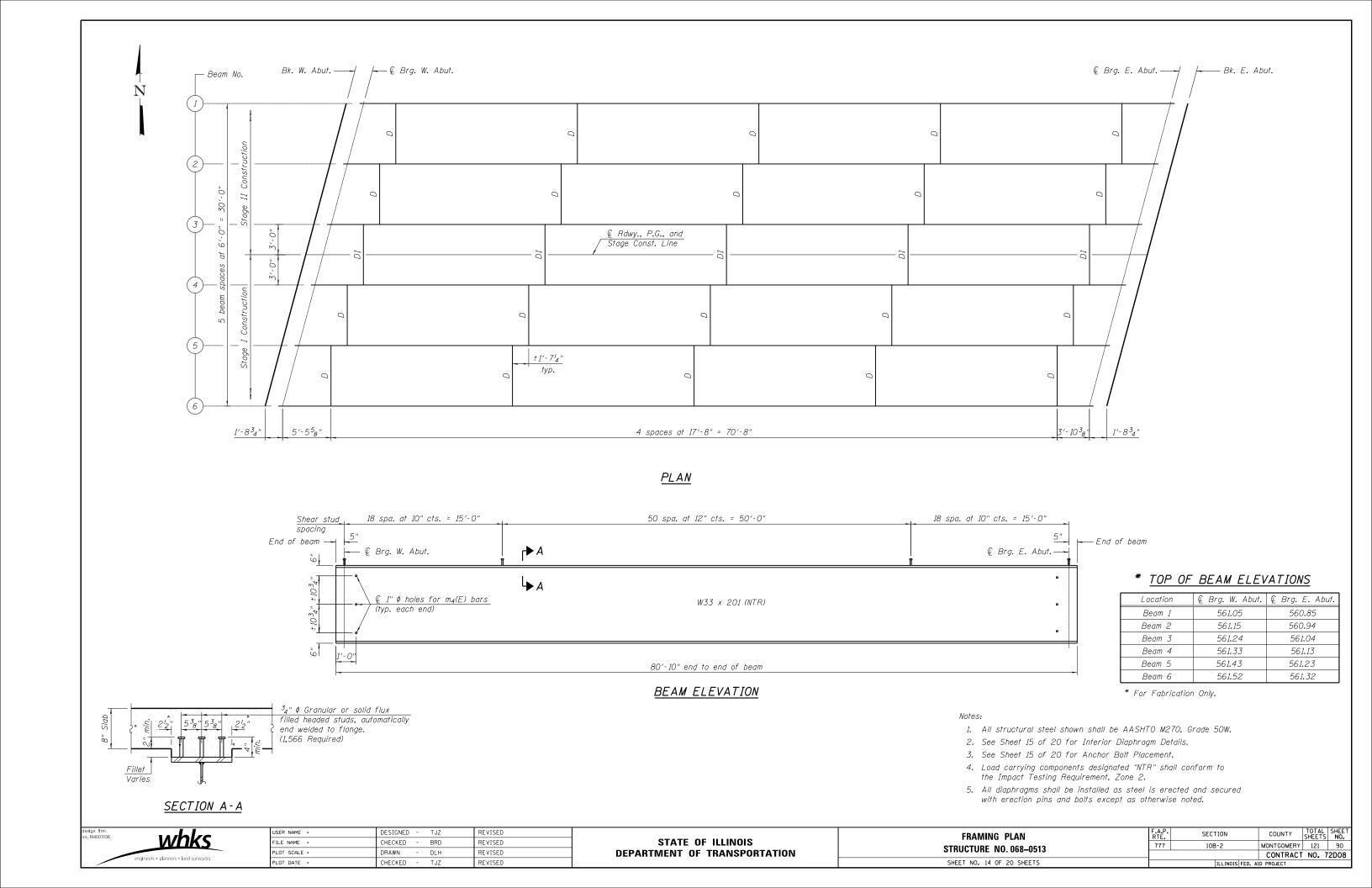
W. Appr. Elev. 559.63 E. Appr. Elev. 559.29

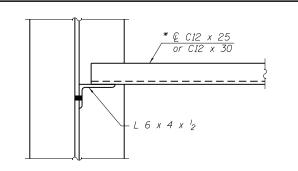
AT APPROACH FOOTING

-(Level out to out)

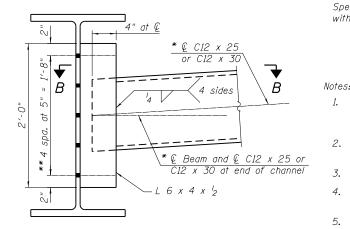
BRIDGE APPROACH SLAB DETAILS STRUCTURE NO. 068-0513 SHEET NO. 13 OF 20 SHEETS

F.A.P. RTE. 777 SECTION COUNTY MONTGOMERY 121 89
CONTRACT NO. 72DO8 10B-2





SECTION B-B



ELEVATION AT ABUTMENT

A◀₁

lil ⊄

⊢ 🖟 Brg.

SECTION A-A

FIXED BEARING

 $2\frac{1}{4}$ " x $2\frac{1}{4}$ " x $\frac{5}{16}$ " P washer under nut.

 1^{3}_{8} " x 2" slotted hole in flange.

 $1_2^{l_2}$ " ϕ holes in bearing plate.

1.	Anchor	bolts	shall	be i	ASTM	F1554	all-	thread	(or	an	Engineer-	approve	ed	alternate	mater	ial) of	the
	grade(s)) and	diame	eter(s	s) spe	cified.	The	corres	spon	nding	specified	d grade	of	AASHTO	M314	ancho	or bolts
	may be	used	in lie	u of	ASTI	1 F155	4.										

 \mathcal{Q} 1" ϕ x 12" anchor bolts

F1554 Grade 36) with

- 2. Anchor bolts at fixed bearings may be either cast in place or installed in holes drilled after the supported member is in place.
- 3. Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.
- 4. The bearing plates, diaphragms, and connecting angles shall conform to the requirements of AASHTO M 270 Grade 50W.
- 5. Two hardened washers required for each set of oversized and slotted holes.
- * Alternate channels are permitted to facilitate material acquisition. Calculated weight of structural steel is based on the lighter section. The alternate, if utilized, shall be provided at no additional cost to the Department.
- ** 3_4 " ϕ HS bolts, 15 16 " ϕ holes.

P 2" x 9" x 1534"

l₈" elastomeric neoprene

with Structural Steel,

Shim plate

if required

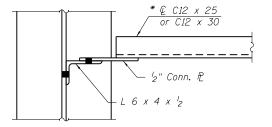
material properties of Article A

leveling pad according to the

1052.02(a) of the Standard

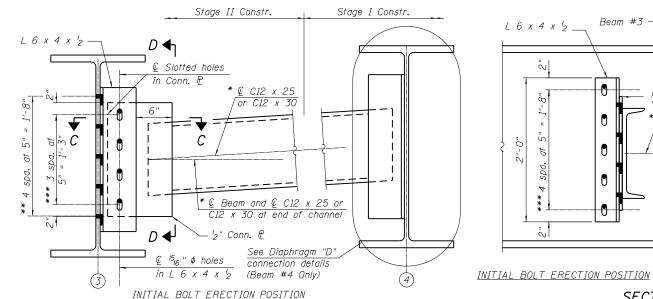
Specifications, Cost included

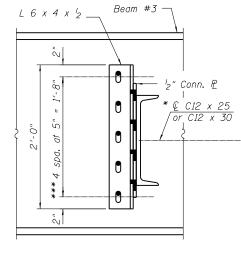
*** $^{3}_{4}$ " ϕ H.S. bolts, on the south side of Beam 3 provide $^{13}_{16}$ " \times 1 $^{7}_{8}$ " vertical slotted holes in the L 6 \times 4 \times $^{1}_{2}$ at the web and in the connection \mathbb{R} . Bolts in slotted holes shall be finger tightened until Stage II pour is completed. Position slots so bolts move from one end with no concrete load to the opposite end under the deck load. The slotted holes in the L 6 x 4 x $\frac{1}{2}$ and connection $\frac{1}{2}$ shall be positioned as shown to allow the bolts to move to the final erection position under deck load. The holes shall be positioned to allow maximum bolt displacement without laterally stressing the beams.

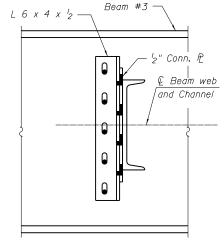


SECTION C-C

INTERIOR DIAPHRAGM "D" DETAILS







FINAL BOLT ERECTION POSITION AFTER STAGE II DECK POUR

- INTERIOR BEAM MOMENT TABLE 11600 (in^4) 19902 (in^4) (in4) 686.0 (in³ 974.9 (in 3) Sc(3n) (in³) 869.0 (in^3) c(cr)0.864 691 Mode ('k) 0.150 (k/') ('k) 120 M DC2 0.267 DW (k/') ('k) 213 1060 3188 M4 + IM ('k) Mu (Strength ('k) ('k) 4424 (ksi) s DC 12.1 (ksi. fs DW (ksi) 13.0 f_s (4+IM) (ksi) s (Service II (ksi) 47.5 0.95R_hFyf (ksi s (Total)(Strength I) (ksi) (ksi) 24.5 (k)
- I_s , S_s : Non-composite moment of inertia and section modulus of the

INTERIOR BEAM REACTION TABLE

₹*DCI*

PDC2

RTotal

74 + IM

Abut.

6.0

10.7

78.2

130.3

35.4

 $I_c(n)$, $S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections due to short-term composite live loads (in.4 and in.3).

steel section used for computing f_s (Total-Strength I, and

Service II) due to non-composite dead loads (in 4 and in 3).

- $I_c(3n)$, $S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections, due to long-term composite (superimposed) dead loads (in.4 and in.3).
- $I_c(cr)$, $S_c(cr)$: Composite moment of inertia and section modulus of the steel and longitudinal deck reinforcement, used for computing f_{s} (Total-Strength I and Service II) in cracked sections, due to both short-term composite live loads and long-term composite (superimposed) dead loads (in.4 and in.3).
 - DC1: Un-factored non-composite dead load (kips/ft.).
 - M_{DCI}: Un-factored moment due to non-composite dead load (kip-ft.). DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).
 - MDC2: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).
 - DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).
 - M_{DW}: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).
 - MŁ + IM: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).
- M_u (Strength I): Factored design moment (kip-ft.).
 - 1.25 (MDC1 + MDC2) + 1.5 MDW + 1.75 M & + IM
 - $\phi_f M_n$: Compact composite positive moment capacity computed according to Article 6.10.7.1 or non-slender negative moment capacity according to Article A6.1.1 or A6.1.2 (kip-ft).
 - fs DC1: Un-factored stress at edge of flange for controlling steel flange due to vertical non-composite dead loads as calculated below (ksi). MDC1 / Snc
 - fs DC2: Un-factored stress at edge of flange for controlling steel flange due to vertical composite dead loads as calculated below (ksi).
 - $M_{DC2}/S_c(3n)$ or $M_{DC2}/S_c(cr)$ as applicable.
 - fs DW: Un-factored stress at edge of flange for controlling steel flange due to vertical composite future wearing surface loads as calculated below (ksi).
 - M_{DW} / $S_c(3n)$ or M_{DW} / $S_c(cr)$ as applicable.
 - fs (4+IM): Un-factored stress at edge of flange for controlling steel flange due to vertical composite live load plus impact loads as calculated below (ksi).
 - $M_4 + IM / S_c(n)$ or $M_4 + IM / S_c(cr)$ as applicable.
- fs (Service II): Sum of stresses as computed below (ksi).
 - fsDC1 + fs DC2 + fs DW + 1.3 fs (4 + IM)
- $0.95R_hF_yf$: Composite stress capacity for Service II loading according to Article 6.10.4.2 (ksi).
- $\it fs$ (Total)(Strength I): Sum of stresses as computed below on non-compact section (ksi).
 - $1.25 (f_{SDC1} + f_{SDC2}) + 1.5 f_{SDW} + 1.75 f_{S}(4 + IM)$
 - $\phi_f F_n$: Non-Compact composite positive or negative stress capacity for Strength I loading according to Article 6.10.7 or 6.10.8 (ksi).
 - V_f : Maximum factored shear range in span computed according to Article 6.10.10.

INTERIOR DIAPHRAGM "D1" DETAILS



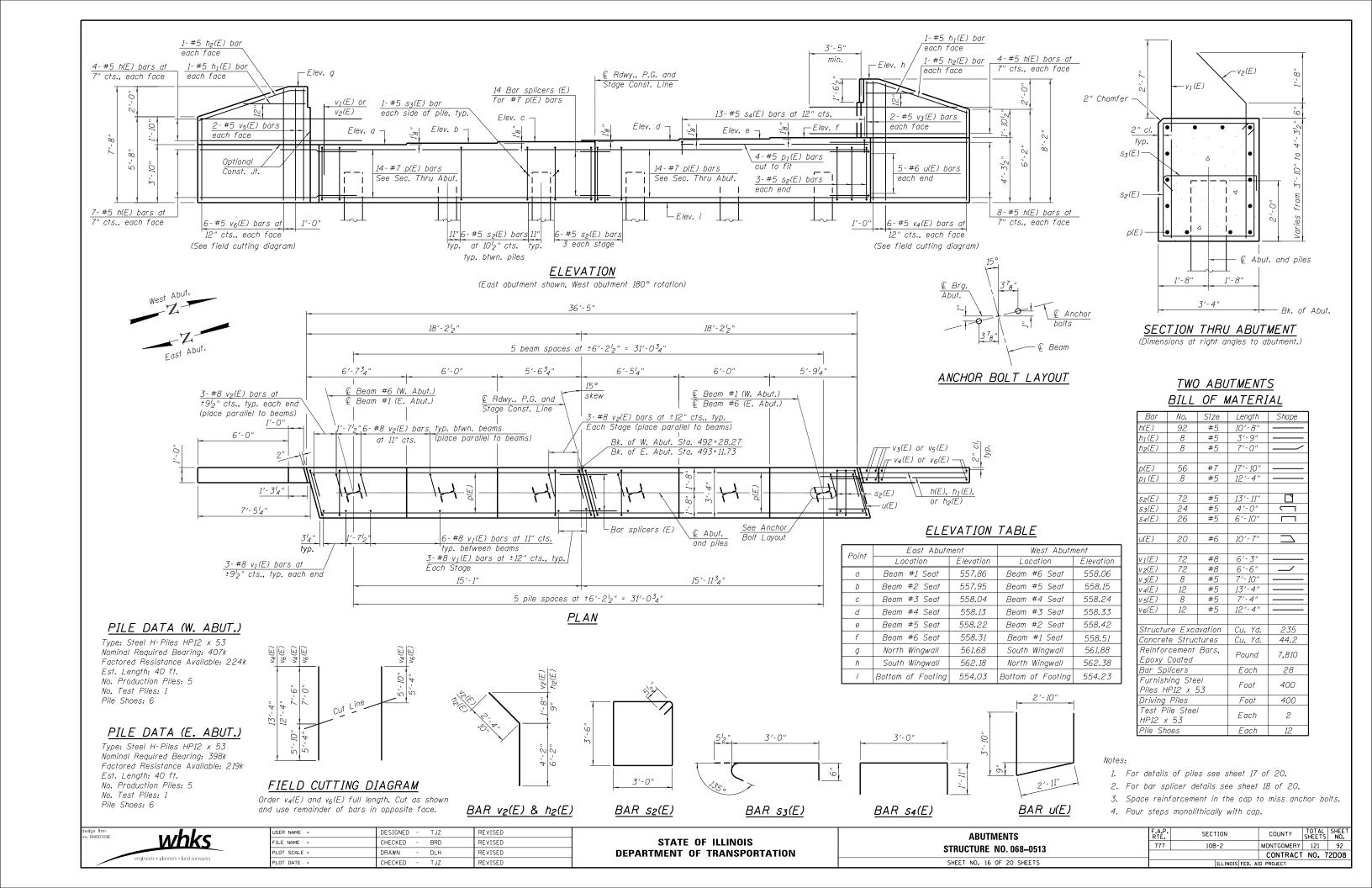
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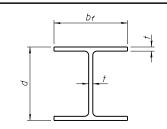
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SECTION D-D

	AL STEEL DETAILS JRE NO. 068–0513	_
SHEET NO	. 15 OF 20 SHEETS	

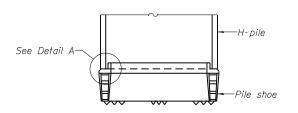
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777	10B-2			MONTGOMERY	121	91
			П	CONTRACT	NO. 7	2D08
	ILLINOIS	FED.	ΑI	D PROJECT		



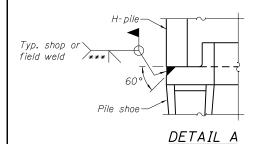


STEEL PILE TABLE

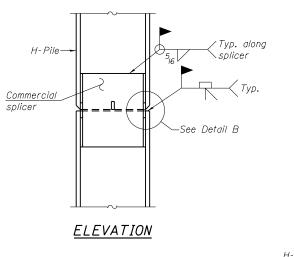
Designation	Depth d	Flange width b _f	Web and Flange thickness t	Encasement diameter A
HP 14x117	14 4 ′′	14 ⁷ 8′′	13 ₁₆ ′′	30′′
x102	14′′	14 ³ 4 ′′	1/16 ′′	30′′
x89	13 ⁷ 8′′	14 ³ 4′′	58′′	30′′
x73	13 ⁵ 8′′	14 ⁵ 8 ′′	2"	30′′
HP 12x84	1214''	1214''	1/16 ′′	24''
x74	12 ¹ 8 ′′	12/4′′	58′′	24''
x63	12''	12 % ′′	2"	24''
x53	11 ³ 4′′	12''	7 ₁₆ ′′	24''
HP 10x57	10′′	1014''	916 ′′	24''
x42	934''	1018′′	7 ₁₆ ′′	24''
HP 8x36	8′′	818''	716 ′′	18′′

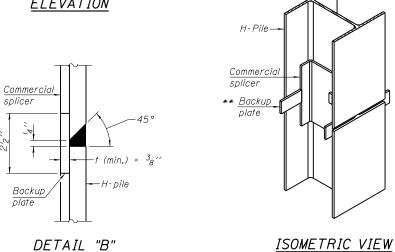


ELEVATION

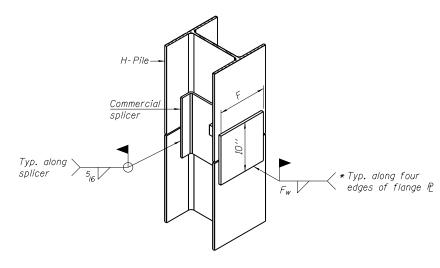


H-PILE SHOE ATTACHMENT

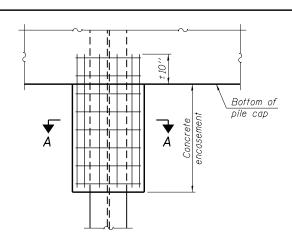


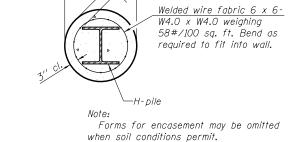


WELDED COMMERCIAL SPLICE



ISOMETRIC VIEW

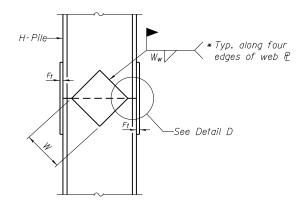


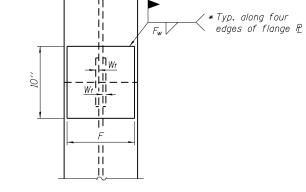


ELEVATION

SECTION A-A

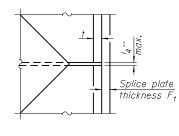
PILE ENCASEMENT





<u>ELEVATION</u>

END VIEW



DETAIL D

Designation	F	F _t	F _w	W	W _f	Ww
HP 14x117	12½''	1''	⁷ 8′′	734''	5 ₈ ′′	2"
x102	1212''	78′′	34''	734''	58′′	2"
x89	1212''	34''	116 ′′	7 ³ 4′′	58′′	2"
x73	1212''	58′′	916 ′′	7 ³ 4′′	58′′	2"
HP 12x84	10′′	78′′	6 ′′	612''	58′′	2"
x74	10′′	78′′	116 ′′	62''	58′′	2"
x63	10′′	58′′	2"	612''	2"	38''
x53	10′′	58′′	2"	612''	2"	38''
HP 10x57	8′′	34''	916 ''	54"	2"	38''
x42	8′′	58′′	916 ′′	514''	2"	38''
HP 8x36	7''	58′′	⁷ 16 ′′	414''	2"	38''

WELDED PLATE FIELD SPLICE

WELDED COMMERCIAL SPLICE ALTERNATE

- * Interrupt welds $^{l}_{4}$ " from end of web and/or each flange.
- ** Remove portions of backup plates that extend outside the flanges.
- *** Weld size per pile shoe manufacturer (${}^{5}_{16}$ " min.).

Note.

The steel H-piles shall be according to AASHTO M270 Grade 50.

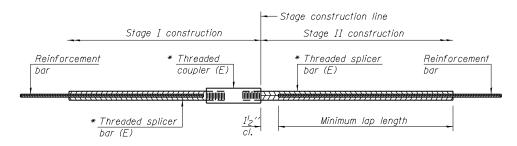
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STATE OF	ILLINOIS
DEPARTMENT OF	TRANSPORTATION

HP PILE DETAILS	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 068-0513	777	10B-2	MONTGOMERY	121	
31110C10HL NO. 000-0313			CONTRACT	NO. 7	2D08
SHEET NO. 17 OF 20 SHEETS		TILL THIOTIC FED. A	ID DDO IECT		



STANDARD BAR SPLICER ASSEMBLY

Minimum Lap Lengths								
Bar size to be spliced	Table 1	Table 2	Table 3	Table 4	Table 5	Table 6		
3, 4	1'-5''	1'-11''	2'-1''	2'-4''	2'-7''	2'-11''		
5	1'-9''	2'-5"	2'-7''	2'-11''	3'-3''	3'-8''		
6	2'-1''	2'-11''	3'-1''	3′-6′′	3′-10′′	4'-5''		
7	2'-9''	3'-10''	4'-2"	4'-8''	5′-2′′	5′-10′′		
8	3′-8′′	5′-1′′	5′-5′′	6'-2''	6'-9''	7′-8′′		
9	4'-7''	6′-5′′	6'-10''	7'-9''	8'-7''	9'-8''		

Table 1: Black bar, 0.8 Class C

Table 2: Black bar, Top bar lap, 0.8 Class C

Table 3: Epoxy bar, 0.8 Class C

Table 4: Epoxy bar, Top bar lap, 0.8 Class C

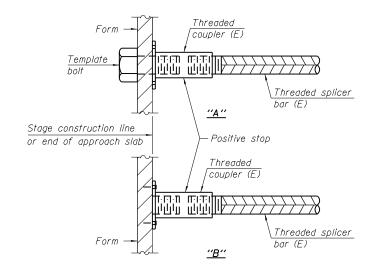
Table 5: Epoxy bar, Class C

Table 6: Epoxy bar, Top bar top, Class C

Threaded splicer bar length = min. lap length + 1^{l_2} " + thread length

* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

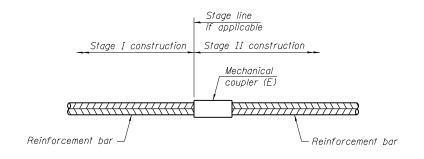
Location	Bar size	No. assemblies required	Table for minimum lap length
Bridge top of deck	#5	141	3
Bridge bottom of deck	#5	99	3
Integral diaphragm - front face	#6	6	2
Integral diaphragm - back face	#6	8	4
Approach top of deck	#4	50	3
Approach bottom of deck	#5	92	3
Approach footing	#5	80	3
Abutment cap	#7	28	4



INSTALLATION AND SETTING METHODS

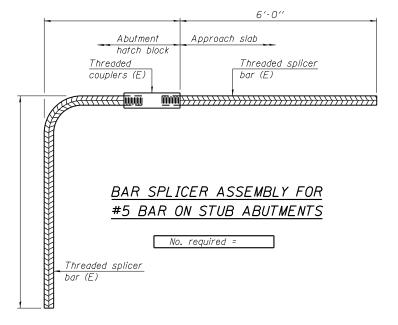
"A": Set bar splicer assembly by means of a template bolt.
"B": Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E): Indicates epoxy coating.



STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required



Notes:

- 1. Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength
- 2. All reinforcement shall be lapped and tied to the splicer bars.
- 3. Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications.
- 4. See approved list of bar splicer assemblies and mechanical splicers for alternatives.

BSD-1

8-31-12

n firm 4001036 whks engineers + planners + land surveyors

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PLOT DATE =	CHECKED	-	TJZ	REVISED

BAR SPLICER ASSEMBLY AND	MECHANICAL SPLICER DETAILS
STRUCTURE	NO. 068–0513
SHEET NO. 1	8 OF 20 SHEETS

F.A.P. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEE NO.
777	10B-2		MONTGOMERY	121	94
			CONTRACT	NO. 7	2D08
	ILLINOIS	FED. Al	D PROJECT		

Illinois Department of Transportation

SOIL PODING LOG

Page <u>1</u> of <u>2</u>

24 5.7 40 60 S-8

(() of I ransport	atı	on			20	JIL BURING LUG			
Division of Highways							Date	2/2	2/12
ROUTEIL 185 DESCRIP	TION	ŧ			ov	er Bayou Creek LOGGED	BY	vi. Tapı	pan
SECTION108-1	LO	CATI	ои _	SE 1/4	4, SEC	. 1, TWP. 7N, RNG. 3W, 3 PM			
COUNTY Montgomery DRIL	LING	ME	THOD			HSA HAMMER TYPE	140	# AUTO)
STRUCT. NO. 068-0513 Station 492+71 BORING NO. 1 W. Abut. Station 492+23 Offset 13.0ft RT Ground Surface Elev. 559.3	-	DEPTH (ft)		U C S Qu (tsf)	S	Stream Bed Elev. 542.5 ft Groundwater Elev.: Very First Encounter 538.3 ft Upon Completion Washed ft	D B L C O W S	Qu	M O I S T (%)
Brown and Gray Moist CLAY LOAM (Fill)			1			Gray Very Moist SILTY CLAY LOAM (continued)			
	0.00		5	2.1 B	17	Gray Dirty Medium to Coarse SAND	1 4		
Brown and Gray moist SILTY CLAY LOAM (Fill)	6.30	-5	1 2 2	.70 B	21	Gray Medium to Coarse SANDY GRAVEL Washed	4 4 25 4		
Gray Very Moist SILTY CLAY LOAM (Fill)			0 1 1	.50 B	28	Gray Medium to Coarse SAND Washed	1 3 3		
		-10	0 1 1	.60 B	27	Gray Dirty Medium SANDY GRAVEL Washed	1 2 2 30 2		
			0	60	28				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer, E-Estimated) Abbreviations W.O.H - Sampler Advanced By Weight of Hammer, W.O.P - Advanced by Weight of Pipe, B.S. - Before Seating The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)

.40 35

.60 29 В

0 .50 30 1 B

Washed

Washed

Brown and Light Gray Moist Clayey SHALE Drilled Stiff at 37.5'



SOIL BORING LOG

Page <u>2</u> of <u>2</u>

Division of Highways IDOT						Date	2/22/12
ROUTE IL 185 DESC	RIPTION		ov	er Bayou Creek	LOGGE	D BY	/I. Tappan
SECTION108-1	LOCATIO	N SE	E 1/4, SEC	2. 1, TWP. 7N, RNG. 3W, 3	PM		
COUNTY Montgomery [RILLING MET	HOD _	****	HSA	HAMMER TYPE	140 #	E AUTO
STRUCT. NO. 068-0513 Station 492+71 BORING NO. 1 W. Abut. Station 492+23 Offset 13.0ft RT		L 0 W	U M C O S I S Qu T	Surface Water Elev. Stream Bed Elev. Groundwater Elev.:	<u>542.5</u> ft	D B E L P O W H S	U M C O S I S Qu T
Ground Surface Elev. 559.3	ft (ft)	/6" (tsf) (%)	▼ AfterPluggedHrs.	ft	(ft) /6"	(tsf) (%)
Brown and Light Gray Moist Clayey SHALE Drilled Stiff at 37.5' <i>(continued)</i>				Ref. Sta. to Centerline of Structure = 492+71 Sta. Increase to East Ref. Elev. to Chsld Squa		__6"_	
Dark Gray Moist Poorly Indurated Clayey SHALE Interbedded with Black Lignite COAL Seams			4.0 20	Approach Slab = 558.7		-65	
Gray Moist Poorly Indurated Clayey SHALE	-50		3.7 13 5-9			-70	
Gray Dry Fissile Clayey SHALE	-	8					
	-55	55 45	11	-		-75 	
	499.30 -60	10 33 67	16			-80	

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer, E-Estimated) Abbreviations W.O.H - Sampler Advanced By Weight of Hammer, W.O.P - Advanced by Weight of Pipe, B.S. - Before Seating The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)

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JSER NAME =	DESIGNED - TJZ	REVISED
FILE NAME =	CHECKED - BRD	REVISED
PLOT SCALE =	DRAWN - DLH	REVISED
PLOT DATE =	CHECKED - TJZ	REVISED

BORING LOGS Structure No. 068-0513	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	777	10B-2	MONTGOMERY	121	95
			CONTRACT	NO. 7	2D08
SHEET NO. 19 OF 20 SHEETS		ILLINOIS FEI	D. AID PROJECT		

545.80

Gray Very Moist SILTY CLAY LOAM

Gray Wet SILTY CLAY LOAM

IDOT						OIL BORING LOG				2/2	
						er Bayou Creek LO					an
SECTION108-1	_ LO	CATI	ON _	SE 1/4	, SEC	. 1, TWP. 7N, RNG. 3W, 3 PM					
COUNTY Montgomery DR	RILLING	ME	THOD			HSA HAMMER TY	PE _		140#	AUTC)
STRUCT. NO. 068-0513 Station 492+71 BORING NO. 2 E. Abut. Station 493+19 Offset 13.0ft LT Ground Surface Elev. 558.8		D E P T H (ft)	U W S	U C S Qu (tsf)	M O I S T	Surface Water Elev. 543 ft Stream Bed Elev. 542.5 ft Groundwater Elev.: ∇ First Encounter 542.8 ft ▼ Upon Completion Washed ft ▼ AfterPluggedHrs. ft	t t	D E P T H	B L O W S /6"	U C S Qu (tsf)	M O I S T (%)
CA-6 and CINDERS (Fill)				-		Gray Fine SAND (continued)					
		_	10 6 5				-	_	0 0 1		
	555.80	_	Ť				_				********
Brown and Gray Moist SILTY CLAY (Fill)		_	1			Gray Medium SANDY GRAVEL	_	\exists	2		
		-5	1 2	.90 B	21	Washed	-	-25	2		
Grayish Brown Very Moist LOAM	553.30	_				0.00			_		
(Fill)			1	.50	18	Gray Dirty Fine SAND Washed		+	1		
		_	1	S-10			30.80	7	2		
O V V V V V V V V V V V V V V V V V V V	550.30					Gray Wet LOAM	30.00	ヿ	0		
Gray Very Moist SILTY CLAY LOAM			0	.30	25	Washed			0	.30	26
		-10	1	В				-30	1	В	
							_				
		-	0	.40	30	52	26.80	-			
		_	1	В		Gray Dirty Medium SANDY GRAVEL		_			
							-				
Dark Gray Very Moist SILTY CLAY LOAM			0	.40	30	Washed	-		6		
		-15		В	<u> </u>		-	-35	13		
Gray Wet LOAM	543.30	-						\dashv			
. <u>X</u>		_	0	.10	25		-	\exists			
			0	В	20		21.80	\exists			
	540.30		MOH	1		Light Olive Brown and Gray Moist Poorly Indurated Clayey SHALE		_			
Gray Fine SAND	34U,3U		0			Washed		_	8		25
		_] 0					-	18 28	4.1 S-8	18

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer, E-Estimated) Abbreviations W.O.H - Sampler Advanced By Weight of Hammer, W.O.P - Advanced by Weight of Pipe, B.S. - Before Seating The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)



Page <u>2</u> of <u>2</u>

of Transportation of Highways	on	SC	OIL BORIN	G LOG	Date _ 2/23/12
ROUTE <u>IL 185</u> DESCRIPTION		ov	er Bayou Creek	LOGGED B	Y M. Tappan
SECTION 108-1 LOC	CATION _	SE 1/4, SEC	2. 1, TWP. 7N, RNG. 3V	/, 3 PM	
COUNTY Montgomery DRILLING	METHOD		HSA	HAMMER TYPE	140 # AUTO
STRUCT. NO. 068-0513 Station 492+71 BORING NO. 2 E. Abut. Station 493+19 Offset 13.0ft LT Ground Surface Elev. 558.8 ft Light Olive Brown and Gray Moist Poorly Indurated Clayey SHALE (continued)	D B L L P O T W H S (ft) //6"	U M C O S I S Qu T (tsf) (%)	Surface Water Elev. Stream Bed Elev. Groundwater Elev.: ☐ First Encounter ☐ Upon Completion ☐ AfterPluggedHrs.	542.8 ft Washed ft	
Dark Gray Dry Fissile Clayey SHALE with 3" Black Lignite COAL Seam	15 43 -45 57 4"	12	-		
Ref. Sta. to Centerline of Ex. Structure = 492+71 Sta. Increase to East	7 22 78 6"	19			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer, E-Estimated) Abbreviations W.O.H - Sampler Advanced By Weight of Hammer, W.O.P - Advanced by Weight of Pipe, B.S. - Before Seating The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, from 137 (Rev. 8-99)

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USER NAME =	DESIGNED	-	TJZ	REVISED
FILE NAME =	CHECKED	-	BRD	REVISED
PLOT SCALE =	DRAWN	-	DLH	REVISED
PLOT DATE =	CHECKED	-	TJZ	REVISED

BORING LOGS Structure No. 068-0513	F.A.P. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
	777	77 10B-2		MONTGOMERY	121	96
31NOCTORE NO. 000-0313				CONTRACT	NO. 7	2D08
SHEET NO. 20 OF 20 SHEETS		ILLINOIS F	ED. AIC	PROJECT		

