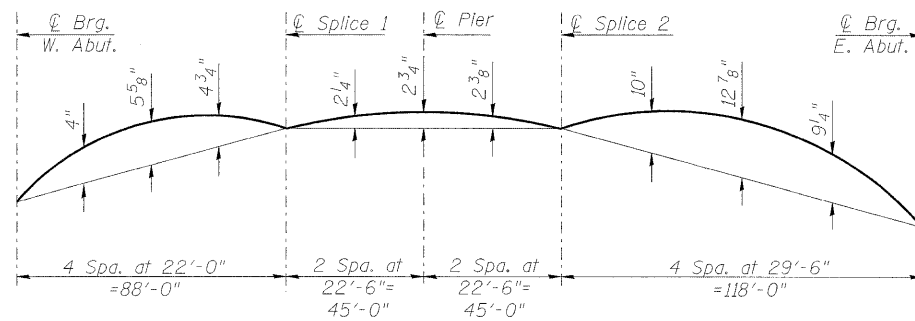


SECTION A-A



CAMBER DIAGRAM

INTERIOR GIRDER MOMENT TABLE				
		0.4 Sp. 1	Pier	0.6 Sp. 2
I_s	(in ⁴)	23819	128987	32825
$I_c(n)$	(in ⁴)	52055	-	81785
$I_c(3n)$	(in ⁴)	39167	-	57563
S_s	(in ³)	95.3	3394	1695
$S_c(n)$	(in ³)	1238	-	2175
$S_c(3n)$	(in ³)	1141	-	2013
DC1	(k/ft)	1.02	1.24	1.09
M_{DC1}	(k)	585	4168	1839
DC2	(k/ft)	0.39	0.39	0.39
M_{DC2}	(k)	299	1318	713
DW	(k/ft)	0.25	0.25	0.25
M_{DW}	(k)	192	847	458
$M_{\xi + IM}$	(k)	1766	2768	2406
M_u (Strength I)	(k)	4484	12972	8088
$\phi_r M_n, \phi_r M_{nc}$	(k)	6333	-	9983
f_s DC1	(ksi)	7.4	14.7	13.0
f_s DC2	(ksi)	3.1	4.7	4.3
f_s DW	(ksi)	2.0	3.0	2.7
f_s 1.3($\xi + IM$)	(ksi)	22.3	12.7	17.3
f_s (Service II)	(ksi)	34.8	35.1	37.3
f_s (Total)(Strength I)	(ksi)	-	45.9	-
V_r	(k)	31.2	-	33.5

* Compact sections
 ** Non-Compact and slender sections

INTERIOR GIRDER REACTION TABLE				
		W. Abut.	Pier	E. Abut.
R_{DC1}	(k)	39	226	65
R_{DC2}	(k)	16	76	24
R_{DW}	(k)	10	49	15
$R_{\xi + IM}$	(k)	95	207	110
R_{Total}	(k)	161	558	214

TOP OF WEB ELEVATIONS*					
Girder	W. Abut.	Splice 1	Pier	Splice 2	E. Abut.
1	428.34	431.21	431.81	431.96	428.17
2	428.21	431.16	431.79	431.98	428.29
3	428.18	431.19	431.87	432.10	428.51
4	428.17	431.25	431.97	432.23	428.74
5	427.98	431.13	431.88	432.18	428.79
6	427.75	430.96	431.75	432.09	428.80

*For fabrication use only.

I_s, S_s : Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total-Strength I, and Service II) due to non-composite dead loads (in⁴ and in³).
 $I_c(n), S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total-Strength I, and Service II) due to short-term composite live loads (in⁴ and in³).
 $I_c(3n), S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total-Strength I, and Service II) due to long-term composite (superimposed) dead loads (in⁴ and in³).
 DC1: Un-factored non-composite dead load (kips/ft.).
 M_{DC1} : Un-factored moment due to non-composite dead load (kip-ft.).
 DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).
 M_{DC2} : Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).
 DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).
 M_{DW} : Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).
 $M_{\xi + IM}$: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).
 M_u (Strength I): Factored design moment (kip-ft.).
 $1.25 (M_{DC1} + M_{DC2}) + 1.5 M_{DW} + 1.75 M_{\xi + IM}$
 $\phi_r M_n$: Compact composite positive moment capacity computed according to Article 6.10.7.1 (kip-ft.).
 $\phi_r M_{nc}$: Compact non-composite negative moment capacity computed according to Article A6.1.1 (kip-ft.).
 f_s (Service II): Sum of stresses as computed from the moments below (ksi).
 $M_{DC1} + M_{DC2} + M_{DW} + 1.3 M_{\xi + IM}$
 f_s (Total)(Strength I): Sum of stresses as computed from the moments below on non-compact section (ksi).
 $1.25 (M_{DC1} + M_{DC2}) + 1.5 M_{DW} + 1.75 M_{\xi + IM}$
 V_r : Maximum factored shear range in composite portion of span computed according to Article 6.10.10.