

September 17, 2019

Mr. Amish T. Bhatt, S.E, P.E **AECOM**303 East Wacker Drive, Suite 1400
Chicago, IL 60601

Re: Geotechnical Letter Report
Overhead Sign Structures and High Mast Towers
Jane Byrne Interchange, Contract 62A76
Cook County, Illinois
Wang No. 1100-04-01

Dear Mr. Bhatt,

Wang Engineering Inc. (Wang) is pleased to present our geotechnical analysis and recommendations for the design of seven overhead sign and five high mast tower structures along the northbound (NB) I-90/94 as part of Jane Byrne Interchange Reconstruction, Contract 62A76, in Chicago, Cook County, Illinois. Based on the information provided by AECOM, Wang understands the proposed overhead signs and high mast towers will be located between West Roosevelt Road Bridge and West Madison Street Bridge crossings.

The purpose of the investigation was to characterize the subsurface conditions and provide geotechnical analyses and recommendations for the design and construction of the proposed structures.

Subsurface Investigation and Laboratory Testing

The project site is located in the W ½ of Section 16 and NW ½ of Section 21, T39N, R14E of the third Principal Meridian. A *Site Location Map* is presented as Exhibit 1. The subsurface investigation consisted of ten soil borings designated as NB for sign structure borings and LTB for high mast tower borings. The subsurface investigation was carried out between July 5 and August 26, 2019. The borings were drilled to depths ranging from 40.0 to 60.0 feet below ground surface (bgs). In addition, two existing borings designated as boring 02-RWB-06 and 27-RWB-01 were



used to supplement this report. The as-drilled northing and easting coordinates were surveyed by Wang using a mapping grade GPS unit. The sign structures and high mast towers locations, as provided by AECOM/ TranSystems, with corresponding reference borings are shown in Table 1 and Table 2, respectively.

Table 1: Overhead Sign Structure Locations and Reference Borings along NB I-90/94

Overhead Sign Structure ID	Approximate Station	Reference Borings ⁽¹⁾	Termination Depth (ft.)
NB-10	6105+20 (NB I-90/94)	NB-10	50
NB-11	6108+25 (NB I-90/94)	NB-11	60
NB-13	6119+45 (NB I-90/94)	NB-13	45
NB-14	6124+75 (NB I-90/94)	02-RWB-06	112
NB-15	6128+37 (NB I-90/94)	NB-15	50
NB-21	6156+60 (NB I-90/94)	NB-21	50
NB-22	6158+80 (NB I-90/94)	NB-22	45

^{(1) 02-}RWB-06 boring was performed by Wang on 06/17/2013.

Table 2: High Mast Tower Structure Locations and Reference Borings along NB I-90/94

High Mast Tower Structure ID	Approximate Station	Approximate Offset	Reference Borings ⁽¹⁾	Termination Depth (ft.)
8 CCD2	6108+00 (NB I-90/94)	141.2 RT	LTB-01	60
7 CCD3	6112+50 (NB I-90/94)	140.0 RT	LTB-04	45
7 DAB1	6122+85 (NB I-90/94)	117.5 RT	LTB-06	40
7 DAB3	6118+54 (NB I-90/94)	128.3 RT	LTB-07	60
5 VCD3	6343+85 (NB C-D Rd)	54.0 RT	27-RWB-01	96

^{(1) 27-}RWB-01 boring was performed by Wang on 06/23/2014.

Truck-mounted drilling rigs, equipped with hollow stem augers, were used to advance and maintain an open borehole. Soil sampling was performed in accordance with AASHTO T 206, "*Penetration Test and Split Barrel Sampling of Soils*." The soil was sampled at 2.5-foot intervals to 30 feet bgs and at 5.0-foot intervals thereafter to the boring termination depth. Soil samples collected from each sampling interval were placed in sealed jars for further examination and laboratory testing.

Sign Structures and High Mast Towers Contract 62A76

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Field boring logs were prepared and maintained by a Wang engineer, and included lithologic descriptions, visual-manual soil classifications, penetrometer or Rimac unconfined compressive strength tests, and results of standard penetration tests recorded as blows per 6-inches of penetration.

Ground water level was measured during drilling and at completion of each boring. The boreholes were backfilled with bentonite chips after completion, and the surface was restored as close as possible to its original condition.

Soil samples were tested in the laboratory for moisture content (AASHTO T 265). Field visual descriptions of the soil samples were verified in the laboratory and classified according to IDH Soil Classification System.

The as-drilled boring locations were surveyed by Wang and station and offset information for each boring were provided by AECOM/ TranSystems. It is noted that LTB boring locations prepared during OUC submittal process are located 30 to 80 feet away from the latest high mast tower locations provided by AECOM. Most of boring elevations were provided by AECOM/ TranSystems from topo drawings. Where information was not available, the boring elevations were estimated either from cross section drawings or from layout survey. Boring location data are presented in the Boring Logs (Appendix A) and the boring locations are shown in Exhibit 2. Please note that the sign structure locations are not shown in the Exhibit 2 because the actual sign locations are not available during the report preparation.

Subsurface Conditions

Detailed descriptions of the soil conditions encountered are presented in the attached *Boring Logs* (Appendix A). Please note the lithological boundaries shown on the logs and profiles (Exhibit 3) represent approximate boundaries between the soil types. In the field, the actual transition between soil types might be different in horizontal and vertical directions.

Below the pavement, the borings encountered one to six feet of fill materials. The fill consists of hard silty clay loam with unconfined compressive strength (Q_u) values of up to 4.1 tsf or medium dense to dense sandy gravel. Please note that Boring LTB-07 encountered 11-foot thick lightweight concrete fill. At elevations of 565 to 581 (2 to 15 feet bgs), the borings advanced through up to 36 feet of very soft to medium stiff clay to silty clay. Beneath the very soft to medium stiff clay to silty



clay, the borings encountered stiff to hard silty clay to silty loam. In deeper borings, the stiff to hard silty clay to silty loam is followed by medium dense to very dense silt to silty loam and sand. Boring 02-RWB-06 encountered Dolostone bedrock at 92 feet bgs (490 feet elevation).

The design and construction of drilled shaft foundation should consider the groundwater in granular fill. Moreover, the granular soil layers within and below the clay layers are expected to be saturated; groundwater in granular soils above the bedrock is expected to be under hydrostatic pressure.

Engineering Analyses and Recommendations

Our evaluation showed at sign structure and High mast tower locations, soft to medium stiff clay to silty clay with Q_u values less than 1.0 tsf extending to about 35 feet below ground surface or about elevation of 540 feet. Therefore, the standard foundation dimensions criteria were not met. Accordingly, the sign structure foundations will require site specific design as per IDOT Sign Structure Manual (IDOT 2012).

Lateral loads on drilled shafts should be analyzed for maximum moments and lateral deflections. The lateral load capacity analysis can be performed using computer program such as COMP 624P, LPILE, LATPILE, or any other similar programs. The estimated soil parameters that may be used to analyze stresses and deflections of drilled shafts sign structure and high mast tower foundations under lateral loads are presented in Table 3 through Table 14. The Qu values for the soft silty clay were obtained from the closest vane shear testing conducted near the structures. Information on the vane shear testing is provided in Appendix A for reference.

Table 3: Recommended Parameters for Lateral Load Analysis of Sign Structure at NB-10 (Reference Borings: NB-10 and VST-07)

	Unit	Undrained	Estimated	Estimated Lateral	Estimated Soil
Soil Type (Layer)	Weight, γ ⁽¹⁾ (pcf)	Shear Strength, c _u (psf)	Friction Angle, Φ ⁽¹⁾ (°)	Soil Modulus Parameter, k ⁽²⁾ (pci)	Strain Parameter, ε_{50} (%)
Existing FILL Below pavement to EL 575 feet	120	0	30	90	
Stiff SILTY CLAY	115	1000	0	500	0.7
EL 575 to 572 feet Soft to M Stiff CLAY	110	900	0	100	1.0
EL 572 to 567 feet	110	800	0	100	1.0
Soft to M Stiff CLAY EL 567 to 557 feet	110	600	0	100	1.0
Soft to M Stiff CLAY EL 557 to 550 feet	110	780	0	100	1.0



Soil Type (Layer)	Unit Weight, $\gamma^{(1)}$ (pcf)	Undrained Shear Strength, c _u (psf)	Estimated Friction Angle, Φ (1) (°)	Estimated Lateral Soil Modulus Parameter, k ⁽²⁾ (pci)	Estimated Soil Strain Parameter, ε ₅₀ (2) (%)
Stiff SILTY CLAY EL 550 to 544 feet	115	1000	0	500	0.7
Medium dense SILT EL 544 to 539 feet	120	0	32	90	
Very Stiff to Hard SILTY CLAY EL 539 to 530 feet	120	3900	0	1000	0.5

⁽¹⁾ Based on Naval Facilities Engineering Command, Design Manual 7.1 (1996)

Table 4: Recommended Parameters for Lateral Load Analysis of Sign Structure at NB-11 (Reference Borings: NB-11 and VST-07)

Soil Type (Layer)	Unit Weight, γ (pcf)	Undrained Shear Strength, c _u (psf)	Estimated Friction Angle, Φ (°)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ε ₅₀ (%)
Existing FILL Below pavement to EL 587 feet	120	0	34	225	
V Stiff SILTY CLAY EL 587 to 578 feet	115	2500	0	100	0.5
Stiff SILTY CLAY EL 578 to 572 feet	115	1000	0	500	0.7
Soft to M Stiff CLAY EL 572 to 567 feet	110	800	0	100	1.0
Soft to M Stiff CLAY EL 567 to 557 feet	110	600	0	100	1.0
Soft to M Stiff CLAY EL 557 to 550 feet	110	780	0	100	1.0
Stiff SILTY CLAY EL 550 to 544 feet	115	1000	0	500	0.7
Stiff SILTY CLAY EL 544 to 540 feet	120	1100	0	500	0.7
Hard SILTY LOAM to SI CL LOAM EL 540 to 531 feet	120	4600	0	2000	0.4

⁽²⁾ Based on L-Pile Technical Manual 2012



Table 5: Recommended Parameters for Lateral Load Analysis of Sign Structure at NB-13 (Reference Borings: NB-13 and VST-07)

(Reference Borings: NB-13 and VS1-07)						
	T.T. *:	Undrained	Estimated	Estimated Lateral	Estimated Soil	
G 11 TF (T)	Unit	Shear	Friction	Soil Modulus	Strain	
Soil Type (Layer)	Weight, γ	Strength, c _u	Angle, Φ	Parameter, k	Parameter, ε_{50}	
	(pcf)	(psf)	(°)	(pci)	(%)	
Existing FILL	120	4000	0	2000	0.4	
Below pavement to EL 571 feet	120					
Stiff SILTY CLAY	115	1000	0	500	0.7	
EL 575 to 572 feet	115	1000	Ů	300	0.7	
Soft to M Stiff CLAY	110	800	0	100	1.0	
EL 572 to 567 feet	110	300	Ů	100		
Soft to M Stiff CLAY	110	600	0	100	1.0	
EL 567 to 557 feet	110	000		100	1.0	
Soft to M Stiff CLAY	110	780	0	100	1.0	
EL 557 to 550 feet	110	700		100	1.0	
Stiff SILTY CLAY	115	1000	0	500	0.7	
EL 550 to 545 feet	113	1000	O	300	0.7	
Very Stiff SILTY CLAY	120	3500	0	1000	0.5	
EL 545 to 535 feet	120	3300	U	1000	0.5	
Stiff SILTY CLAY	120	1500	0	500	0.7	
EL 535 to 531 feet	120	1500	O	300	0.7	

Table 6: Recommended Parameters for Lateral Load Analysis of Sign Structure at NB-14 (Reference Borings: 02-RWB-06 and DBT-VST-01)

		Undrained	Estimated	Estimated Lateral	Estimated Soil
	Unit	Shear	Friction	Soil Modulus	Strain
Soil Type (Layer)	Weight, γ	Strength, c _u	Angle, Φ	Parameter, k	Parameter, ε_{50}
	(pcf)	(psf)	(°)	(pci)	(%)
Existing FILL	120	2800	0	1000	0.5
Below pavement to EL 578 feet					
Stiff SILTY CLAY	115	1250	0	500	0.7
EL 578 to 568 feet	_				V 1,
Soft to M Stiff CLAY	110	800	0	100	1.0
EL 568 to 553 feet					
Stiff CLAY	115	1100	0	500	0.7
EL 553 to 540 feet			Ü		0.7
Stiff SILTY CLAY LOAM	115	1200	0	500	0.7
EL 540 to 535 feet	110	1200	ŭ		
V Stiff SILTY CLAY LOAM	120	3100	0	1000	0.5
EL 535 to 527 feet	120	3100	V	1000	0.5
Hard SILTY CLAY	120	6100	0	2000	0.4
EL 527 to 520 feet	120	0100	U	2000	0.4



Soil Type (Layer)	Unit Weight, γ (pcf)	Undrained Shear Strength, c _u (psf)	Estimated Friction Angle, Φ (°)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ε ₅₀ (%)
M Dense GRAVELLY					
SANDY LOAM	125	0	34	60	
EL 520 to 512 feet					
Hard SILTY CLAY	125	6000	0	2000	0.4
EL 512 to 491 feet	123	0000	U	2000	0.4

Table 7: Recommended Parameters for Lateral Load Analysis of Sign Structure at NB-15 (Reference Borings: NB-15 and VST-01)

Soil Type (Layer)	Unit Weight, γ (pcf)	Undrained Shear Strength, c _u (psf)	Estimated Friction Angle, Φ (°)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ε_{50} (%)
Existing FILL Below pavement to EL 578 feet	120	0	30	25	
Stiff to V Stiff SILTY CLAY EL 578 to 574 feet	115	1500	0	500	0.7
Soft to M Stiff SILTY CLAY EL 574 to 570 feet	110	750	0	100	1.0
Soft to M Stiff CLAY EL 570 to 562 feet	110	650	0	100	1.0
Soft to M Stiff CLAY EL 562 to 558 feet	110	950	0	100	1.0
Soft to M Stiff CLAY EL 558 to 540 feet	115	1100	0	500	0.7
Dense SILTY LOAM EL 540 to 533 feet	120	0	36	125	
V Stiff SILTY CLAY LOAM EL 533 to 532 feet	120	2800	0	1000	0.5

Table 8: Recommended Parameters for Lateral Load Analysis of Sign Structure at NB-21 (Reference Borings: NB-21 and VST-03)

Soil Type (Layer)	Unit Weight, γ	Undrained Shear Strength, c _u	Estimated Friction Angle, Φ	Estimated Lateral Soil Modulus Parameter, k	Estimated Soil Strain Parameter, ε_{50}
Existing FILL	(pcf)	(psf)	()	(pci)	(%)
Below pavement to EL 575 feet	120	3000	0	1000	0.5
Soft to M Stiff CLAY	110	400	0	30	2.0
EL 575 to 565 feet	110	100	<u> </u>	50	2.0



Soil Type (Layer)	Unit Weight, γ (pcf)	Undrained Shear Strength, c _u (psf)	Estimated Friction Angle, Φ (°)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ε ₅₀ (%)
Soft to M Stiff CLAY EL 565 to 552 feet	110	600	0	100	1.0
Soft to M Stiff CLAY EL 552 to 547 feet	110	950	0	100	1.0
Soft to M Stiff CLAY EL 547 to 541 feet	115	1400	0	500	0.7
V Stiff SILTY CLAY EL 541 to 536 feet	120	2100	0	1000	0.5
Hard Stiff CLAY LOAM EL 536 to 531 feet	120	5200	0	2000	0.4
Very Stiff CLAY EL 531 to 528 feet	120	3300	0	1000	0.5

Table 9: Recommended Parameters for Lateral Load Analysis of Sign Structure at NB-22 (Reference Borings: NB-22 and VST-03)

		Undrained	Estimated	Estimated Lateral	Estimated Soil
	Unit	Shear	Friction	Soil Modulus	Strain
Soil Type (Layer)	Weight, γ	Strength, c _u	Angle, Φ	Parameter, k	Parameter, ε_{50}
	(pcf)	(psf)	(°)	(pci)	(%)
Existing FILL	120	0	30	20	
Below pavement to EL 570 feet	120				
M Stiff to V Stiff SILTY					
CLAY LOAM	120	1600	0	500	0.7
EL 570 to 565 feet					
Soft to M Stiff CLAY	110	600	0	100	1.0
EL 565 to 552 feet		000	Ů	100	1.0
Soft to M Stiff CLAY	110	950	0	100	1.0
EL 552 to 547 feet	110	750	· ·		
Soft to M Stiff CLAY	115	1400	0	500	0.7
EL 547 to 544 feet	113	1400	U	300	0.7
V Stiff SILTY CLAY	120	1500	0	500	0.7
EL 544 to 531 feet	120	1500	0	500	0.7



Table 10: Recommended Parameters for Lateral Load Analysis of High Mast Tower 8 CCD2 (Reference Borings: LTB-01 and VST-07)

	(Reference Borings: L1B-01 and VS1-07)											
		Undrained	Estimated	Estimated Lateral	Estimated Soil							
a 11 =	Unit	Shear	Friction	Soil Modulus	Strain							
Soil Type (Layer)	Weight, γ	Strength, c _u	Angle, Φ	Parameter, k	Parameter, ε_{50}							
	(pcf)	(psf)	(°)	(pci)	(%)							
Existing FILL	120	0	30	90								
Below pavement to EL 585 feet	120		30									
V Stiff SILTY CLAY	120	2300	0	100	0.5							
EL 585 to 581 feet	120	2300		100	0. 5							
Soft to M Stiff CLAY	110	800	0	100	1.0							
EL 581 to 567 feet	110	800	<u> </u>	100	1.0							
Soft to M Stiff CLAY	110	600	0	100	1.0							
EL 567 to 557 feet	110		0	100								
Soft to M Stiff CLAY	110	780	0	100	1.0							
EL 557 to 550 feet	110	700	<u> </u>	100	1.0							
Stiff SILTY CLAY	115	1000	0	500	0.7							
EL 550 to 545 feet	113	1000	<u> </u>	300	0.7							
V Stiff SILTY CLAY	120	2000	0	1000	0.5							
EL 545 to 540 feet	120	2000	U	1000	0.5							
Hard SILTY CLAY	125	6600	0	2000	0.4							
EL 540 to 531 feet	123	0000	0	2000	U.T							

Table 11: Recommended Parameters for Lateral Load Analysis of High Mast Tower 7 CCD3 (Reference Borings: LTB-04 and VST-07)

Soil Type (Layer)	Unit Weight, γ (pcf)	Undrained Shear Strength, c _u (psf)	Estimated Friction Angle, Φ (°)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ε ₅₀ (%)
Soft to M Stiff CLAY Below Pavement to 567 feet	110	800	0	100	1.0
Soft to M Stiff CLAY EL 567 to 557 feet	110	600	0	100	1.0
Soft to M Stiff CLAY EL 557 to 550 feet	110	780	0	100	1.0
Stiff SILTY CLAY EL 550 to 544 feet	115	1000	0	500	0.7
M Dense SILTY LOAM EL 544 to 534 feet	120	0	34	125	
Hard SILTY CLAY LOAM EL 534 to 531 feet	125	6000	0	2000	0.4



Table 12: Recommended Parameters for Lateral Load Analysis of High Mast Tower 7 DAB3 (Reference Borings: LTB-06 and VST-07)

(Reference Borings: L1B-06 and VS1-07)												
	Unit	Undrained Shear	Estimated Friction	Estimated Lateral Soil Modulus	Estimated Soil Strain							
Soil Type (Layer)	Weight, γ	Strength, c _u	Angle, Φ	Parameter, k	Parameter, ε_{50}							
	(pcf)	(psf)	(°)	(pci)	(%)							
Existing FILL	120	0	34	225								
Below pavement to EL 567 feet	120	<u> </u>	J4	223								
Soft to M Stiff CLAY	110	600	0	100	1.0							
EL 567 to 557 feet	110	000	0	100	1.0							
Soft to M Stiff CLAY	110	780	0	100	1.0							
EL 557 to 550 feet	110	700	<u> </u>	100	1.0							
Stiff SILTY CLAY	115	1000	0	500	0.7							
EL 550 to 547 feet	113	1000		300	0.7							
Stiff SILTY CLAY	120	1100	0	500	0.7							
EL 547 to 541 feet	120	1100	0	300	0.7							
Hard SILTY CLAY	125	6000	0	2000	0.4							
EL 541 to 532 feet	123	0000	J	2000	0.4							

Table 13: Recommended Parameters for Lateral Load Analysis of High Mast Tower 7 DAB1 (Reference Borings: LTB-07 and DBT-VST-01)

Soil Type (Layer)	Unit Weight, γ (pcf)	Undrained Shear Strength, c _u (psf)	Estimated Friction Angle, Φ (°)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ε ₅₀ (%)
Existing Lightweight Concrete	40	22 200		2000	0.4
FILL	48	22,300	0	2000	0.4
Below pavement to EL 582 feet					
M Stiff SILTY CLAY LOAM	115	650	0	100	1.0
EL 582 to 577 feet	113	050		100	1.0
Stiff SILTY CLAY	115	1200	0	500	0.7
EL 577 to 574 feet	113	1200	0	300	0.7
Stiff SILTY CLAY	115	1300	0	500	0.7
EL 574 to 568 feet	113	1300	U	300	0.7
Soft to M Stiff CLAY	110	800	0	100	1.0
EL 568 to 553 feet	110	800	U	100	1.0
Stiff CLAY	115	1100	0	500	0.7
EL 553 to 546 feet	113	1100	U	300	0.7
V Stiff SILTY CLAY LOAM	120	3300	0	1000	0.5
EL 546 to 540 feet	120	3300	U	1000	0.5
Hard SILTY CLAY	125	6900	0	2000	0.4
EL 540 to 537 feet	123	0900	0	2000	0.4



Table 14: Recommended Parameters for Lateral Load Analysis of High Mast Tower 5VCD3
(Reference Borings: 27-RWB-01 and VST-02)

Soil Type (Layer)	Unit Weight, γ (pcf)	Undrained Shear Strength, c _u (psf)	Estimated Friction Angle, Φ (°)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ε_{50} (%)
Existing FILL Below pavement to EL 576 feet	120	4100	0	2000	0.4
Soft to M Stiff CLAY EL 576 to 566 feet	115	500	0	100	1.0
Soft to M Stiff CLAY EL 566 to 561 feet	115	900	0	100	1.0
Soft to M Stiff CLAY EL 561 to 553 feet	115	700	0	100	1.0
Soft to M Stiff CLAY EL 553 to 548 feet	115	900	0	100	1.0
Stiff SILTY CLAY EL 548 to 537 feet	120	1100	0	500	0.7
V Stiff to Hard SILTY CLAY EL 537 to 527 feet	120	3400	0	1000	0.5
Stiff SILTY CLAY EL 527 to 517 feet	120	1400	0	500	0.7
M Dense to V Dense SILTY LOAM EL 517 to 507 feet	125	0	34	125	
Dense to V Dense SAND to SILTY LOAM EL 507 to 483 feet	130	0	36	125	

Construction Considerations

Excavation

Foundations excavation should be performed in accordance with local, state, and federal regulations including current OSHA regulations. The potential effect of ground movements upon nearby structures and utilities should be considered.

Drilled Shafts Construction

The drilled shafts should be constructed in accordance with IDOT Standard Specification Section 516, *Drilled Shafts*. The soft soil layer with Q_u less than 0.5 tsf is prone to squeeze if left open for long period of time. Therefore, to minimize the squeeze potential, casing should be provided. Due

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Wang Engineering

to high squeeze and water bearing in granular layer potential, the following note should appear on the final plans:

Due to the squeeze potential of the clay soils and the presence of water bearing layers, the use of temporary casing may be required to properly construct the shafts. Casing may be pulled or remain in place, as determined by the Contractor at no cost to the Department.'

Qualifications

The analyses and recommendations contained in this letter report are based on data obtained at the boring locations shown in Exhibit 2 and do not reflect any variations that may occur elsewhere on the site, variations whose nature and extent may not become obvious until late in the construction phase. Should subsurface conditions encountered during construction differ from those encountered in the borings or if any change in the location of the overhead signs or high mast towers is planned, Wang should be timely notified so that our recommendations could be reviewed and revised as necessary.

It has been a pleasure to assist AECOM and the Illinois Department of Transportation on this project. Please contact us if you have any questions or if we can be of further assistance.

Respectfully Submitted,

WANG ENGINEERING, INC.

Andri A. Kurnia, P.E. Sr. Geotechnical Engineer

Edwin Greenwood Engineering Geologist

Corina T. Farez, P.E., P.G.

Vice President

Attachments: Exhibit 1: Site Location Map

Exhibit 2: Boring Location Plan

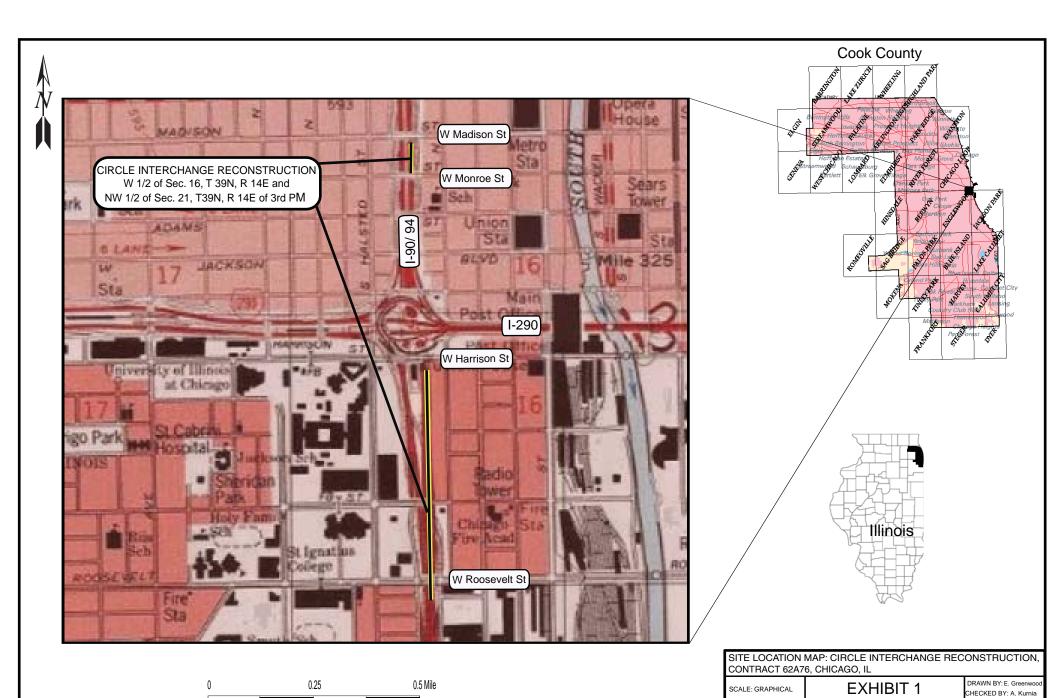
Exhibit 3: Subsurface Soil Data Profile

Appendix A: Boring Logs



EXHIBITS

Exhibit 1: Site Location Map Exhibit 2: Boring Location Plan Exhibit 3: Subsurface Soil Data Profile



Wang Engineering

FOR AECOM

1100-04-01

1145 N. Main Street Lombard, IL 60148 www.wangeng.com



Legend

Boring Location

Proposed HMLT 8 CCD2



SITE AND BORING LOCATION MAP: CIRCLE INTERCHANGE RECONSTRUCTION, CONTRACT 62A76, CHICAGO, ILLINOIS

SCALE: GRAPHICAL

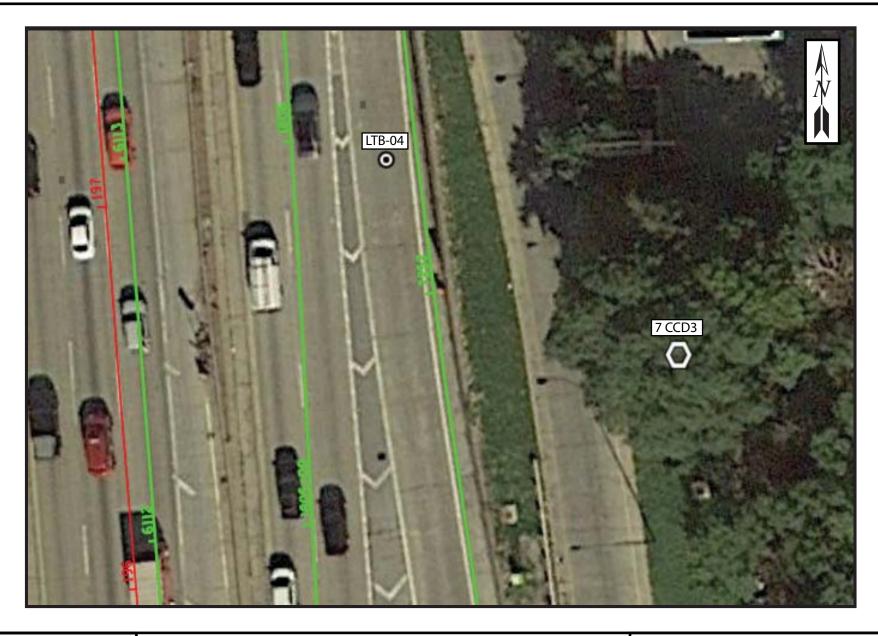
EXHIBIT 2-1

DRAWN BY: E. Greenwood CHECKED BY: A. Kurnia



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FOR AECOM





Proposed HMLT 7 CCD3



BORING LOCATION PLAN: CIRCLE INTERCHANGE RECONSTRUCTION, CONTRACT 62A76, CHICAGO, ILLINOIS

SCALE: GRAPHICAL

EXHIBIT 2-2

DRAWN BY: E. Greenwood CHECKED BY: A. Kurnia



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FOR AECOM





Proposed HMLT 7 DAB1



BORING LOCATION PLAN: CIRCLE INTERCHANGE RECONSTRUCTION, CONTRACT 62A76, CHICAGO, ILLINOIS

SCALE: GRAPHICAL EX

EXHIBIT 2-3 DRAWN BY: E. Greenwood CHECKED BY: A. Kurnia



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FOR AECOM



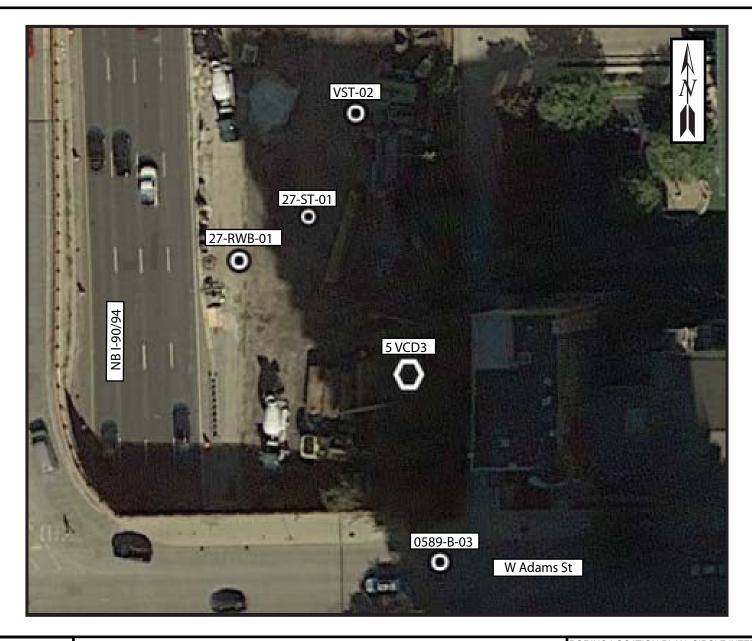






1100-04-01

FOR AECOM

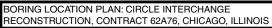


Legend

Boring Location

Proposed HMLT 5 VCD3





SCALE: GRAPHICAL EXHIBIT 2

EXHIBIT 2-5 DRAWN BY: E. Greenwood CHECKED BY: A. Kurnia



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FOR AECOM







BORING LOCATION PLAN: CIRCLE INTERCHANGE RECONSTRUCTION, CONTRACT 62A76, CHICAGO, ILLINOIS

SCALE: GRAPHICAL EXHIBIT 2-6

T 2-6

DRAWN BY: E. Greenwood
CHECKED BY: A. Kurnia



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Legend

Boring Location



SITE AND BORING LOCATION MAP: CIRCLE INTERCHANGE RECONSTRUCTION, CONTRACT 62A76, CHICAGO, ILLINOIS

SCALE: GRAPHIC

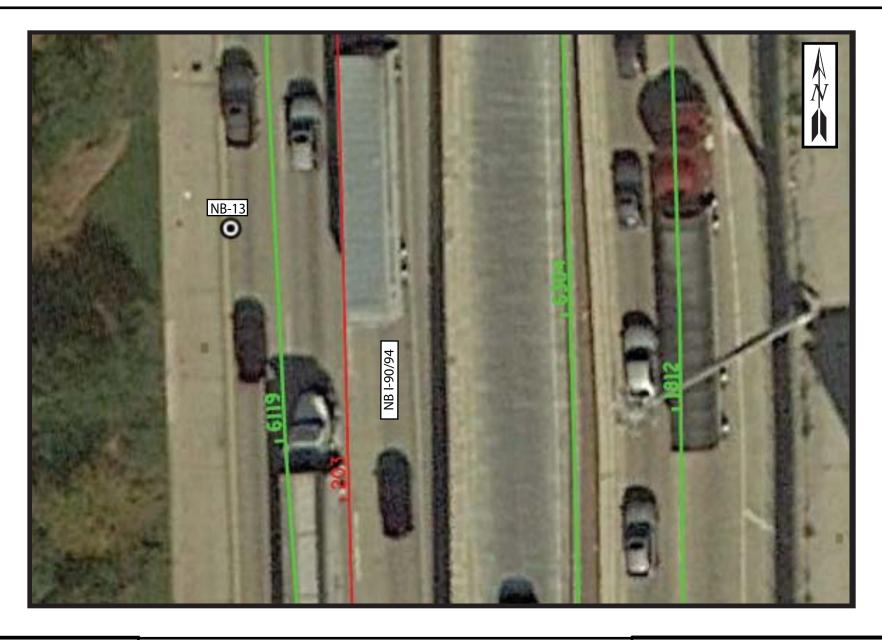
EXHIBIT 2-7

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BORING LOCATION PLAN: CIRCLE INTERCHANGE RECONSTRUCTION, CONTRACT 62A76, CHICAGO, ILLINOIS

SCALE: GRAPHICAL EXHIBIT 2-8

DRAWN BY: E. Greenwood CHECKED BY: A. Kurnia



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Proposed NB-14



BORING LOCATION PLAN: CIRCLE INTERCHANGE RECONSTRUCTION, CONTRACT 62A76, CHICAGO, ILLINOIS

SCALE: GRAPHICAL

EXHIBIT 2-9

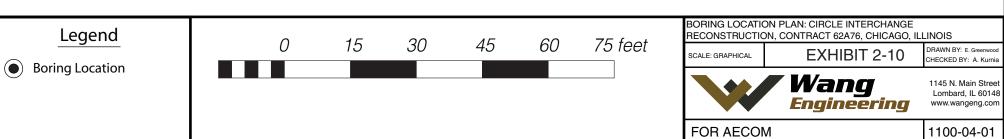
DRAWN BY: E. Greenwood CHECKED BY: A. Kurnia

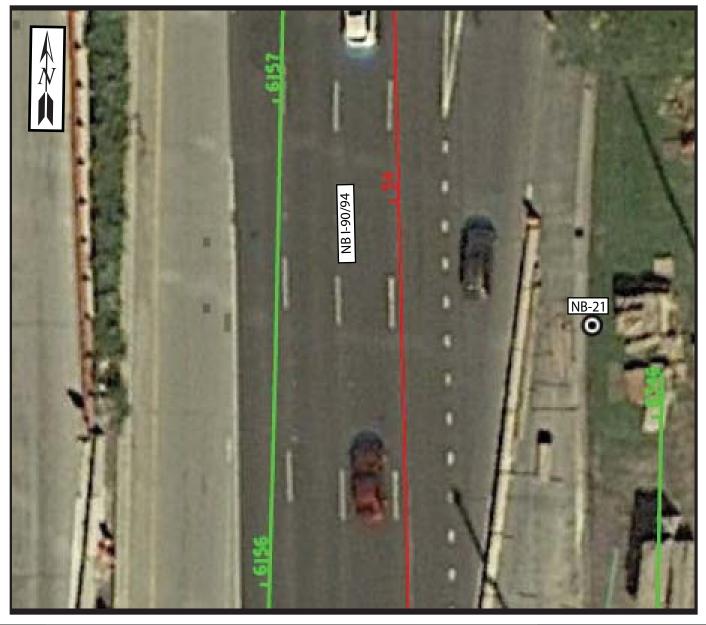


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BORING LOCATION PLAN: CIRCLE INTERCHANGE RECONSTRUCTION, CONTRACT 62A76, CHICAGO, ILLINOIS

SCALE: GRAPHICAL EXHIBIT 2-11

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BORING LOCATION PLAN: CIRCLE INTERCHANGE RECONSTRUCTION, CONTRACT 62A76, CHICAGO, ILLINOIS

SCALE: GRAPHICAL EXHIBIT 2-12

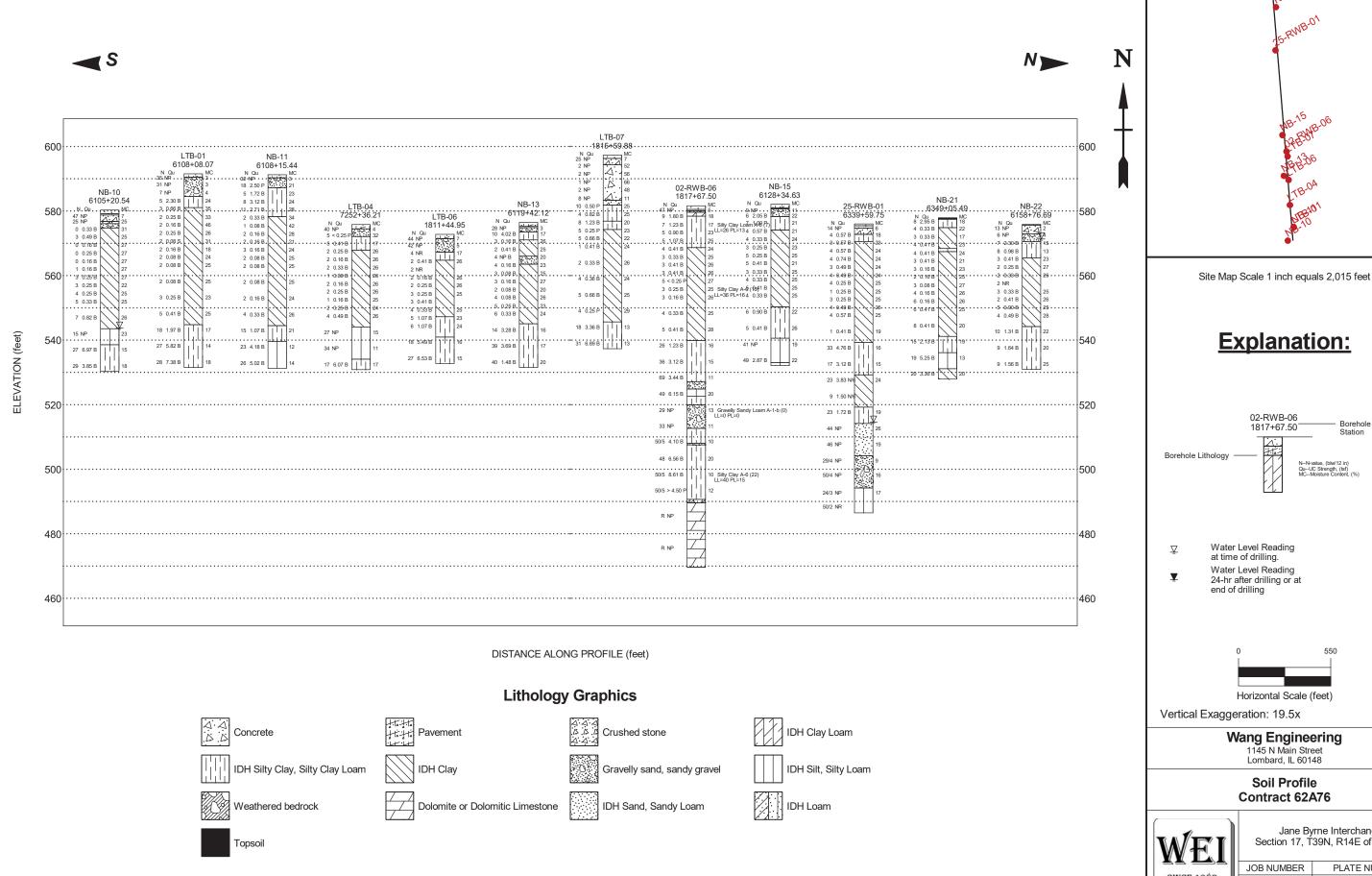


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Jane Byrne Interchange Section 17, T39N, R14E of 3rd PM

JOB NUMBER PLATE NUMBER 1100-04-01 **EXHIBIT 3**



APPENDIX A: BORING LOGS



BORING LOG 02-RWB-06

WEI Job No.: 1100-04-01

Client AECOM

Project Jane Byrne Interchange

Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 581.64 ft North: 1896796.97 ft East: 1171829.83 ft Station: 1817+67.50 Offset: 3.6732 LT

Profile	SOIL AND ROCK DESCRIPTION	Sample Type recovery	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft) Sample Type	sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
4.4	13.75-inch thick CONCRETEPAVEMENT 580.4 550.14-inch thick ASPHALTPAVEMENT 6-inch thick CRUSHED STONE	1	11 20	NP	5					9	1 1 2	0.41 B	26
	BASE COURSE BASE COURSE FILL Medium stiff to stiff, gray and	2	27	1.80 B						10	1 2 3	< 0.25	27
	brown SILTY CLAY LOAM, trace 5-gavelL _L (%)=26, P _L (%)=13	3	3 3 4	1.23 B	17			L _L (%)=36, P _L (%)=16- %Gravel=2.8- %Sand=16.1-	: ¬	11	1	0.25 B	25
	%Silt=49.7 %Clay=23.6 A-6 (7) - - 10_	4	2 2 3	0.90 B	23			%Silt=51.5- %Clay=29.7- A-6 (15)-	- 🚽	12	0 1 2	0.16 B	26
	- - - -	5	2 2 3	1.07 B	25				- - - - -				
	Very soft to soft, gray CLAY to SILTY CLAY, trace gravel	6	1 2 2	0.41 B	24				35_	13	0 2 2	0.33 B	25
9/12/19	-	7	1 1 2	0.33 B	25				- - - -				
WANGENGINC 11000401.GPJ WANGENG.GDT 99	20_	8	1 1 2	0.41 B	26				40	14	3	0.41 B	28
101.GF	GENERAL N	WATER LE											
NGENGINC 110004 Dri Dri	gin Drilling 06-16-2013 Cor illing Contractor Wang Testing Serv iller P&N Logger A. H illing Method 2.25" HSA to 10', mud	While Drilling At Completion of Drilling Time After Drilling Depth to Water Time After Drilling NA NA											
* 	backfilled upon completion	The stratification lines represent the approximate boundary between soil types: the actual transition may be gradual.											

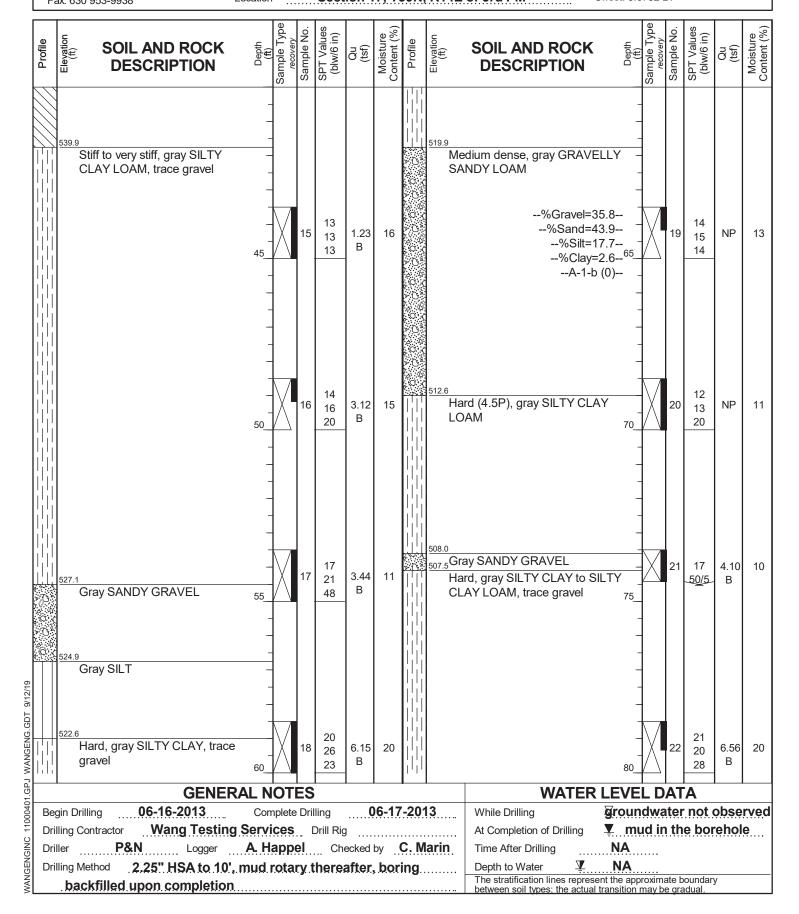


BORING LOG 02-RWB-06

WEI Job No.: 1100-04-01

Client AECOM
Project Jane Byrne Interchange
Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 581.64 ft North: 1896796.97 ft East: 1171829.83 ft Station: 1817+67.50 Offset: 3.6732 LT



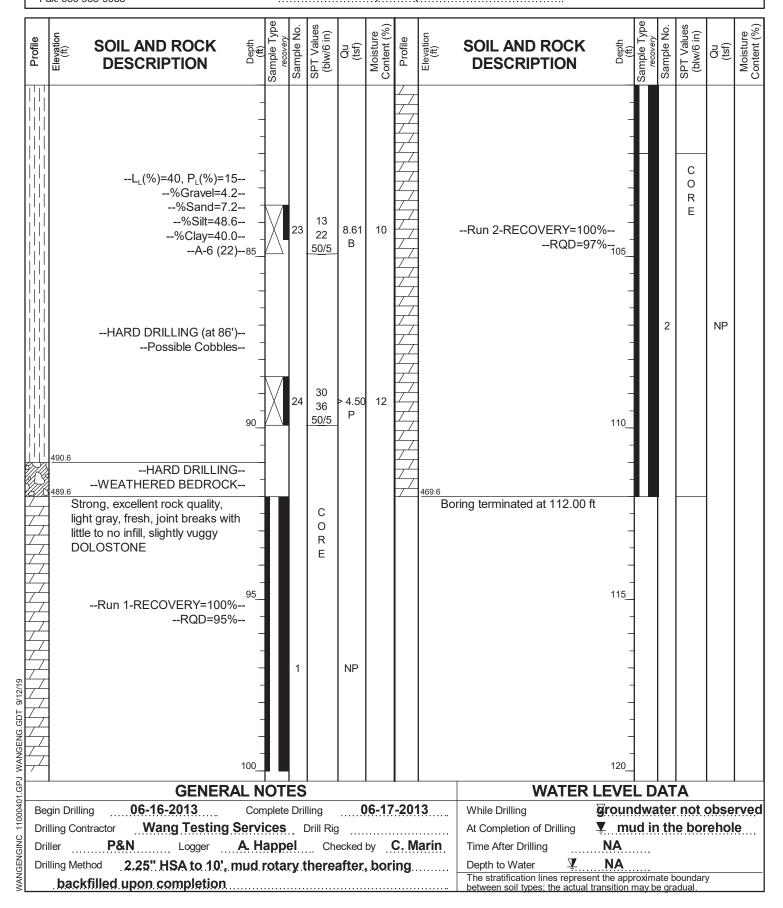


BORING LOG 02-RWB-06

WEI Job No.: 1100-04-01

Client AECOM
Project Jane Byrne Interchange
Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 581.64 ft North: 1896796.97 ft East: 1171829.83 ft Station: 1817+67.50 Offset: 3.6732 LT





BORING LOG 27-RWB-01

WEI Job No.: 1100-04-01

Client AECOM

Project Jane Byrne Interchange

Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 579.17 ft North: 1899481.12 ft East: 1171604.19 ft Station: 6344+30.89 Offset: 14.5751 LT

Profile SOIL AND ROCK DESCRIPTION	Sample Type	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft) Sample Type	Sample No. SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
7-inch thick ASPHALTPAVEMENTPAVEMENTPAVEMENTPAVEMENT	1	4 2 5	4.10 B	15			-	9 0 2 1	0.33 B	27
Very soft to medium stiff, gray CLAY to SILTY CLAY, trace gravel	2	1 1 1	0.16 B	27			25	10 1 2	< 0.25 P	30
	3	0 1 1	0.16 B	25				11 1 1	0.57 B	26
1	4	1 1 1	< 0.25 P	23			30	12 0 2 2	0.57 B	25
	5	0 1 1	0.41 B	25			- - - - -			
1	6	0 0 2	0.49 B	25			35	13 3 3	< 0.25 P	29
9/12/19	7	1 1 2	0.57 B	25			- - - - -			
GENERAL Begin Drilling 06-23-2014 Contractor Wang Testing Sepuration Drilling Method 2.25" SSA to 10', mu backfilled upon completion	NOTES	1 2 4	0.57 B	26		WATER	40 LEVEL DA	14 2 4 6	0.98 B	24
Begin Drilling 06-23-2014 (Complete Dril		0	6-23	3-2014	While Drilling		2.00 ft		
Drilling Contractor Wang Testing Se Driller R&J Logger S. Drilling Method 2.25" SSA to 10', mu backfilled upon completion		MA NA nt the approxima	te boundar		e					

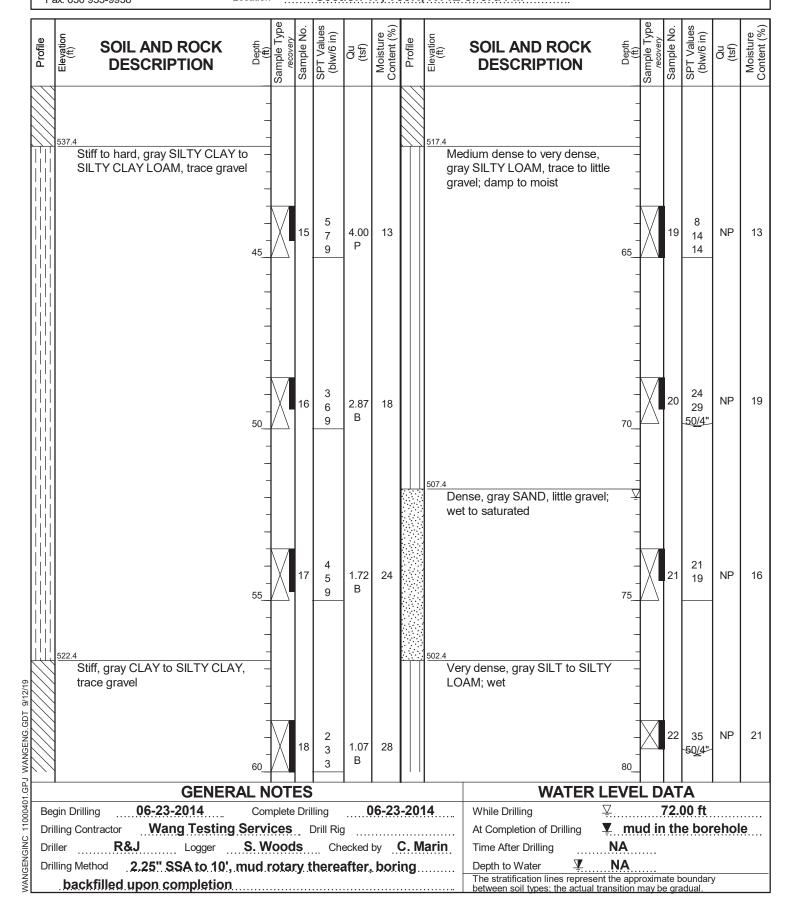


BORING LOG 27-RWB-01

WEI Job No.: 1100-04-01

Client AECOM
Project Jane Byrne Interchange
Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 579.17 ft North: 1899481.12 ft East: 1171604.19 ft Station: 6344+30.89 Offset: 14.5751 LT





BORING LOG 27-RWB-01

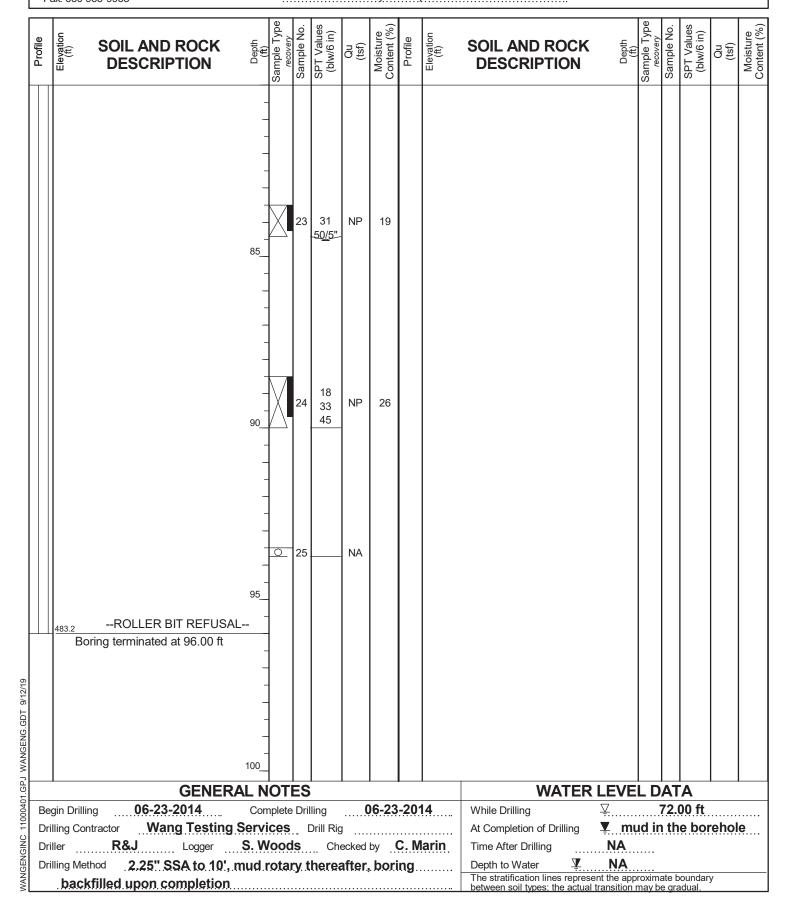
WEI Job No.: 1100-04-01

Client AECOM

Project Jane Byrne Interchange

Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 579.17 ft North: 1899481.12 ft East: 1171604.19 ft Station: 6344+30.89 Offset: 14.5751 LT





BORING LOG DBT-VST-01

WEI Job No.: 1100-04-01

Client AECOM

Project Jane Byrne Interchange

Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 578.78 ft North: 1896835.88 ft East: 1171705.33 ft Station: 6236+46.23 Offset: 93.46' LT

Profile	Elevation	SOIL AND ROCK DESCRIPTION	(ft) Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Sample Type	Sample No.	SP1 ld)	Qu (tsf)	Moisture Content (%)
.Q.:A.	. <u>.</u> . <u></u>	10-inch thick CONCRETE 78.0PAVEMENT Blind drill to 7.5 feet	-							$-S_{u \text{ undis}} = 775.7 -S_{u \text{ remold}} = 409.4 -Sensitivity = 1.89-$	- - -		VS		
			- - - -							In-Situ Vane Shear, 22.0 feet S _{u undis} = 969.6 S _{u remold} = 538.7 Sensitivity = 1.80		6	<u>VS</u>		
			5 - - -							In-Situ Vane Shear, 24.5 feet _{2:} S _{u undis} = 689.5S _{u remold} = 387.8Sensitivity = 1.78		7	V <u>S</u>		
	57′	In-Situ Vane Shear, 7.5 feet S _{u undis} = 1335.9 S _{u remold} = 817.8 Sensitivity = 1.63	_ 							In-Situ Vane Shear, 27.0 feet S _{u undis} = 1012.7 S _{u remold} = 624.9 Sensitivity = 1.63		8	<u>V</u> S		
		1	0	1	V <u>s</u>					In-Situ Vane Shear, 29.5 feet ₃₀ S _{u undis} = 1034.2S _{u remold} = 646.4Sensitivity = 1.60		9	<u>Vs</u>		
		In-Situ Vane Shear, 12.0 feet S _{u undis} = 905.0 S _{u remold} = 603.3 Sensitivity = 1.50	- - -	2	V <u>S</u>					In-Situ Vane Shear, 32.0 feet S _{u undis} = 1206.6 S _{u remold} = 646.4 Sensitivity = 1.87		10	V <u>S</u>		
		In-Situ Vane Shear, 14.5 feet ₁ S _{u undis} = 797.2S _{u remold} = 409.4Sensitivity = 1.95	5 1	3	VS					maxed out vane shear before _{3:} failure	- - -	11	<u>Vs</u>		
WANGENGINC 11000401.GPJ WANGENG.GDT 9/12/19		In-Situ Vane Shear, 17.0 feet S _{u undis} = 883.4 S _{u remold} = 474.0 Sensitivity = 1.86	- - -	4	<u>VS</u>						- - - -				
J WANG		In-Situ Vane Shear, 19.5 feet2		5						40	-				
7.GP.		GENERAL	NOT	ES						WATER LEV	EL D	ΑΤ	Α		
000040 B	egin	n Drilling 06-25-2019	While Drilling ♀												
		ng Contractor Wang Testing Se	At Completion of Drilling DRY												
	riller		Time After Drilling NA												
WANG!		ng Method 3.25" HSA, boring ba	CKTIII	Depth to Water The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.											



BORING LOG DBT-VST-01

WEI Job No.: 1100-04-01

Client AECOM

Project Jane Byrne Interchange

Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 578.78 ft North: 1896835.88 ft East: 1171705.33 ft Station: 6236+46.23 Offset: 93.46' LT

	Profile	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND DESCRIF	ROCK PTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
			-															
			-															
			-															
			-															
			-															
			45															
-		532.8																
		Boring terminated at 46.00 ft	-															
			-															
			-															
			-															
			_ 50_															
			_															
			-															
			-															
			-															
			-															
			-															
			55_															
			_															
			-															
19			-															
F 9/12/			-															
GD.SD			-															
ANGEN			-															
GPJ W												WATER	I EVE	ח ו	∐ ∧T	^		
00401	GENERAL NOTES Begin Drilling 06-25-2019 Complete Drilling 06-25-2019										While Drilling	MIER	Ż			00 ft		
110(lling Contractor Wang Testin	g Servic	ces	. [Orill Rig					At Completion of Drilling DRY							
NGIN	Dri										Time After Drilling NA							
WANGENGINC 11000401.GPJ WANGENG.GDT 9/12/19	Drilling Method 3.25" HSA, boring backfilled upon completion											Depth to Water The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.						



BORING LOG LTB-01

WEI Job No.: 1100-04-01

Client AECOM

Project Jane Byrne Interchange

Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 591.52 ft North: 1895140.67 ft East: 1171987.74 ft Station: 6108+08.07 Offset: 118.94 RT

	Profile	SOIL AND ROCK DESCRIPTION	Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND F		Depth (ft)	Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
		12-inch thick, CONCRETE		1	16 18 17	NP	3					- - -		9	1 1 1	0.08 S	31
		5		2	4 13 18	NP	3					- 25		10	1 1 1	0.16 B	18
		584.5 Very stiff, gray SILTY CLAY, trace gravel; dampRDR 2		3	7 5 2	NP	4					-, - -		11	1 1 1	0.08 B	24
c		10 581.0		4	2 2 3	2.30 B	24					30		12	1 1 1	0.08 B	25
		Very soft to medium stiff, gray CLAY to SILTY CLAY, trace gravel; damp to moistRDR 2		5	2 1 2	0.66 B	35					- - - -					
		15		6	0 1 1	0.25 B	33					- 35,		13	1 1 1	0.08 B	25
9/12/19				7	0 1 1	0.16 B	46					- - - -					
WANGENGINC 11000401.GPJ WANGENG.GDT 9/12/19		20		8	1 1 1	0.25 B	26					- - 40		14	1 1 2	0.25 B	23
401.G	Do	GENERAL gin Drilling 07-16-2019 C					7-16	201	10		ATER L	EVEI			A RY		
11000		gin Drilling 07-16-2019 Colling Contractor Wang Testing Ser	omplete vices		-				! !	While Drilling At Completion of I		<u>-</u> <u>7</u>			UD		
GINC		iller N&A Logger M. S								Time After Drilling		NI A	 			• • • • • • •	• • • • •
1GEN	Dri	illing Method 3.25". HSA to 10', muc	l rota	ry.t	here	after	, bor	ing.		Depth to Water		NA					
×		backfilled upon completion								The stratification lir between soil types:	nes represent the actual tra	tne appr nsition n	oxima nay be	te bo gra	oundar <u>ı</u> dual.	/	

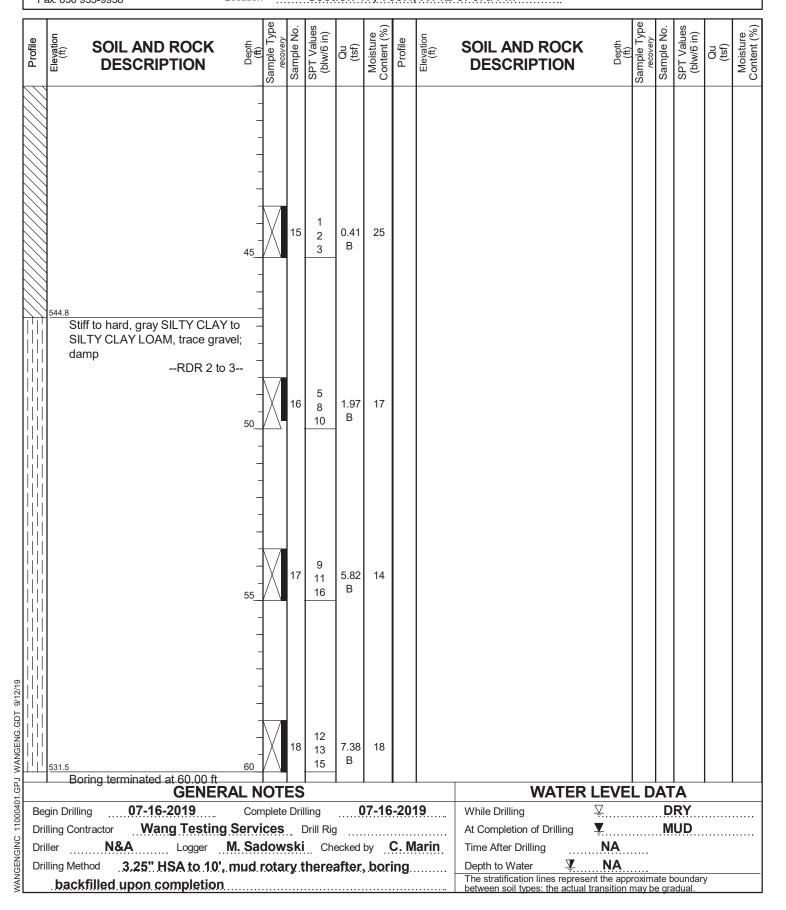


BORING LOG LTB-01

WEI Job No.: 1100-04-01

Client AECOM
Project Jane Byrne Interchange
Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 591.52 ft North: 1895140.67 ft East: 1171987.74 ft Station: 6108+08.07 Offset: 118.94 RT





BORING LOG LTB-04

WEI Job No.: 1100-04-01

Client AECOM

Project Jane Byrne Interchange

Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 575.81 ft North: 1895626.27 ft East: 1171903.85 ft Station: 7252+36.21 Offset: 8.25 LT

Profile		Sample Type	Sample No. SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft) Sample Type Frecovery Sample Type Sample Type Sample No. SPT Values (blw/6 in) Qu	(tsf) Moisture Content (%)
	16-inch thick, CONCRETEPAVEMENT							
	Loose to dense, brown, moist SANDY GRAVELBASE COURSERDR 3 to 4		18 21 19	NP	4			25 26 3
	Very soft to soft, gray SILTY – CLAY; damp to wetRDR 2 to 3 5		2 11 3 2	< 0.25 P	32		10 1 0 E	16 25 3
	- - - - -		3 1 2 3	0.41 B	17		11 0 0 E	25 24 3
	Very soft to soft, gray CLAY, trace gravel; damp to wet RDR 2		4 1	0.25 B	26		12 1 1 3 0. E	49 26 3
			1 5 1 1	0.16 B	26		544.1	
			6 0 1 1	0.33 B	26		35 13 12 N	P 15
STEATS STATES	- - - -		7 0 0 1	0.08 B	26			
WANGENGINC TI000401.GPJ WANGENGING.GDT S	20_		8 1 1	0.16 B	26		14 9 N	P 11
20.00	GENERAL N	OTE	S		•	•	WATER LEVEL DATA	
Be			Orilling	0			_	
Dr	illing Contractor Wang Testing Servi						l l	ole
	iller N&A Logger M. Sac illing Method 2.25" HSA to 10', mud r							
PAN PI DI	illing Method 2.25" HSA to 10', mud r backfilled upon completion	_				_	The stratification lines represent the approximate boundary	



BORING LOG LTB-04

WEI Job No.: 1100-04-01

Client AECOM

Project Jane Byrne Interchange

Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 575.81 ft North: 1895626.27 ft East: 1171903.85 ft Station: 7252+36.21 Offset: 8.25 LT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft) Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROO DESCRIPTION		Sample Type	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
		ard, gray SILTY CLAY LOAM, ace gravel; damp RDR 2-													
	530.8	oring terminated at 45.00 ft	45	15	6 7 10	6.07 B	17								
WANGENGINC 11000401 GPJ WANGENG.GDT 9/12/19	DI	oring terminated at 45.00 it	50												
V [GP]		CENEDA		E						\A/AT	ER LEVE				
0401.C	egin Drill	GENERA ing 08-12-2019	Complete			C	18-12	201	19	While Drilling			AIA er not	ohse	rved
Dr Dr Dr	-	ntractor Wang Testing S N&A Logger M.	ervices Sadow	/ski	Orill Rig	3 ecked	by!	C. M	arin	At Completion of Drillin	_				
WANG	_	kfilled upon completion		-		_		The stratification lines re between soil types; the a	present the app	roximat nay be	e boundar gradual.	у			

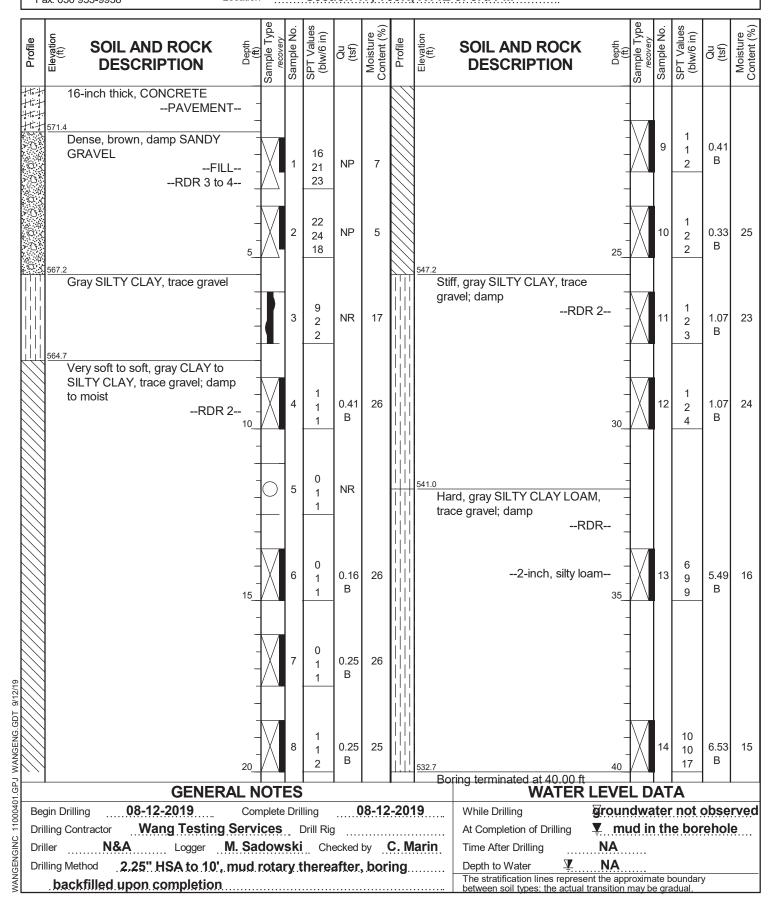


BORING LOG LTB-06

WEI Job No.: 1100-04-01

Client AECOM
Project Jane Byrne Interchange
Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 572.72 ft North: 1896175.31 ft East: 1171874.75 ft Station: 1811+44.95 Offset: 14.92 RT





BORING LOG LTB-07

WEI Job No.: 1100-04-01

Client AECOM
Project Jane Byrne Interchange
Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 597.33 ft North: 1896690.25 ft East: 1171852.45 ft Station: 1815+59.88 Offset: 14.51 RT

Profile	SOIL AND ROCK DESCRIPTION	Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft) Sample Type Recovery Sample No. SPT Values (blw/6 in) (isf) Moisture
	10-inch thick, CONCRETEPAVEMENT Medium dense, gray CRUSHED STONE; moistRDR2FILL		1	16 18 7	NP	7		Stiff, gray SILTY CLAY LOAM, trace gravel; dampRDR 2 9
	Very loose, light gray, lightweight CONCRETE; dryRDR 1 to 2FILL		2	1 1 1	NP	52		Soft to medium stiff, gray CLAY to SILTY CLAY, trace gravel; damp RDR 1 to 2 25 10 23 0.25 P
4.4.4.4.4.4.4.4.4.4.4.4.4.4.4.4.4.4.4.			3	1 1 1	NP	56		11
	10		4	1 0 1	NP	66		12 0 0.41 24 30 1 B
V V V V V V V V V V V V V V V V V V V			5	2 1 1	NP	48		
7.7. 44.4.4.4.4.4.4	Loose, gray CRUSHED STONEFILL 15 881.8 Medium stiff, gray SILTY CLAY		6	3 4 4	NP	11		13 1 0.33 26 35 1 B
	LOAM, trace gravelRDR1		7	8 6 4	0.50 P	25		
Beg Drill	20 GENERAL		8 ES	2 2 2	0.82 B	25		14 0 0.36 24 WATER LEVEL DATA
Beç Dril	gin Drilling 07-05-2019 Colling Contractor Wang Testing Ser	omplet vices	e Dri	lling Drill Rig	J			2019 While Drilling At Completion of Drilling The mude in the borehole The mude in the borehole
Dril	lling Method 2,25" IDA HSA, boring							

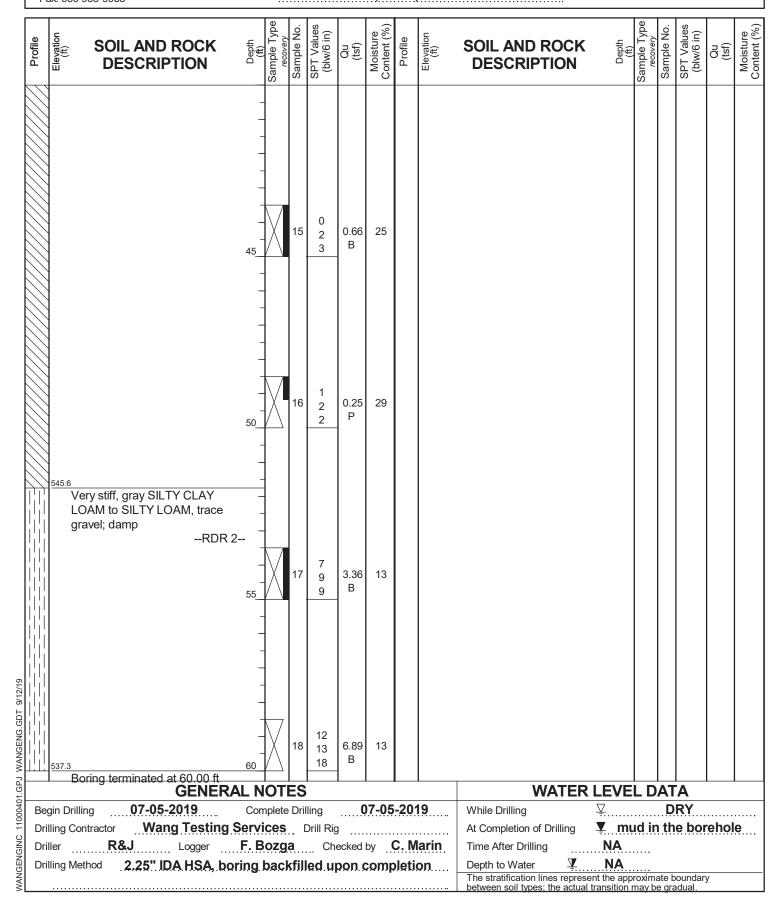


BORING LOG LTB-07

WEI Job No.: 1100-04-01

Client AECOM
Project Jane Byrne Interchange
Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 597.33 ft North: 1896690.25 ft East: 1171852.45 ft Station: 1815+59.88 Offset: 14.51 RT





BORING LOG NB-10

WEI Job No.: 1100-04-01

Client AECOM

Project Jane Byrne Interchange

Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 580.30 ft North: 1894847.01 ft East: 1171858.74 ft Station: 6105+20.54 Offset: 21.05 LT

١	Profile	SOIL AND ROCK Hodel DESCRIPTION	Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROC DESCRIPTION		Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	A	14-inch thick CONCRETEPAVEMENT 579.1 578.84-inch ASPHALT	-								-					
44.4.4.	1. Q. 1. Q. 1. Q. 1. Q.	PAVEMENT/ Dense, gray SANDY GRAVEL			28						-		9	0 1 2	0.25 B	27
. 7 . 7 . 4	1. A. 2. A. 1. A.	ÁGGREGATE BASE 576.8RDR 2		1	33 14	NP	7				_					
		Brown LOAM; damp 576.1FILL Dense, gray SANDY GRAVEL		2	6 16	NP	25				-		10	0 1	0.25 B	22
V		FILL ⁵ RDR 2			9						25 <u> </u>			2		
		Very soft to medium stiff, gray CLAY to SILTY CLAY, trace gravel; damp to moist		3	0 0	0.33	31				-		11	0	0.25	25
		RDR 2			0	В					-			3	В	
					0						-			0		
		10_		4	1 2	0.49 B	25				- 30_		12	1 4	0.33 B	25
		-									-					
		-		5	0 0	0.16 B	27				-					
		- -									-	-				
		- -		6	0 0	0.25	27				-		13	2	0.82	26
		15_			0	В					35_			4	В	
		-			0						_					
		<u>-</u>	$\frac{1}{2}$	7	0	0.16 B	27			dium dense, gray SILT .TY LOAM; wet	to ½					
121 10 1 20		-									RDR 2					
		- 		8	0 0	0.16 B	27				- -		14	9 7 8	NP	23
; 5		GENERAL N	TOL	FS						WATE	40_ ER LEVE		 ΔΤ	-		
5	Be		mplete			0	8-25	-20°	19	While Drilling	<u> </u>			00 ft		
		ling Contractor Wang Testing Serv			-					At Completion of Drilling					ehol	e
	Dri	ler K&A Logger I. N	lenr)	Ch	ecked	by	C. N	larin	Time After Drilling	NA					
	Dri	ling Method 2.25" HSA to 10', mud	rota	ry.1	there	after	, bor	ing.			NA NA		ıte h	oundan	v	
ŀ		backfilled upon completion								The stratification lines rep	tual transition i	may he	ara .	idual	,	



BORING LOG NB-10

WEI Job No.: 1100-04-01

Client AECOM

Project Jane Byrne Interchange

Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 580.30 ft North: 1894847.01 ft East: 1171858.74 ft Station: 6105+20.54 Offset: 21.05 LT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft) Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND DESCRIF		Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
		ery stiff to hard, gray SILTY LAY, trace gravel; damp RDR	2	15	9 12 15	6.97 B	15										
	 530.3 B	oring terminated at 50.00 ft	50	16	9 12 17	3.85 B	18										
GDT 9/12/19			55														
WANGENGINC 11000401.GPJ WANGENG.GDT 9/12/19		OFNED	60_								A/A-TED						
401.G		GENER					0.0		10		WATER						
000 B€	egin Drill		Complete			0				While Drilling	4 D. ''''	<u>Ş.</u>			0 ft	obs!	·····
		ontractor Wang Testing K&A Logger					by		arin	At Completion of Time After Drilli	_	Ψ mι NA		rrie	, nor	enol	₹
	rilling Me									Depth to Water		NA NA	• • • • •				
ANG DI	_	· · · · · · · · · · · · · · · · · · ·		-				_		The stratification	lines repres	ent the app	roxima	ate bo	undary	/	
<i>≊</i> ∟	vac	kfilled upon completion								between soil type	es; the actua	transition	nay be	e grac	<u>dual. </u>		



BORING LOG NB-11

WEI Job No.: 1100-04-01

Client AECOM
Project Jane Byrne Interchange
Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 591.27 ft North: 1895148.03 ft East: 1171985.82 ft Station: 6108+15.44 Offset: 117.37 RT

-	Profile	SOIL AND ROCK definition DESCRIPTION	Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AN DESCR		Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
0.00		12-inch thick, CONCRETE 590.3 Medium dense to dense, brown SANDY GRAVEL FILLRDR 3 to 4		1	17 18 14	NP	3					- - - -		9	1 1 1	0.16 B	21
0		Stiff to very stiff, black and gray SILTY CLAY, trace gravel; damp to moistRDR 2		2	3 10 8	2.50 P	21					- - - 25_		10	1 1 2	0.16 B	24
				3	2 2 3	1.72 B	23					- - - -		11	0 1 1	0.08 B	25
		10_		4	2 4 4	3.12 B	24					- - 30_		12	1 1 1	0.08 B	25
		578.3		5	4 5 6	2.71 B	28					- - -					
		Very soft to soft, gray CLAY to SILTY CLAY, trace gravel; damp to wetRDR 2		6	1 1 1	0.33 B	34					- - - 35_		13	0 1 1	0.08 B	25
9/12/19				7	0 0 1	0.08 B	42					- - - -					
WANGENGINC 11000401.GPJ WANGENG.GDT 9/12/19		20_		8	0 1 1	0.16 B	28					- - 40		14	1 1 1	0.16 B	24
401.G	D-	GENERAL N					17 47	20.	10	Mhile Deillin	WATER						
11000		gin Drilling 07-17-2019 Colling Contractor Wang Testing Serv	mplete v ices			<u>.</u> 0				While Drilling At Completion		<u>Ş</u> ▼ mι		DF		ehol	e
IGINC	Dri									Time After Di	_	NA					•••••
NGEN	Dri	lling Method 3.25" HSA to 10', mud		-				_		Depth to Wat	er <u>¥</u>	NA ent the ann	rovima	ite ho	undar	,	
×		backfilled upon completion								between soil to	on lines represe /pes; the actual	ราน เกษ app transition เ	nav be	e arac	unuary dual.		

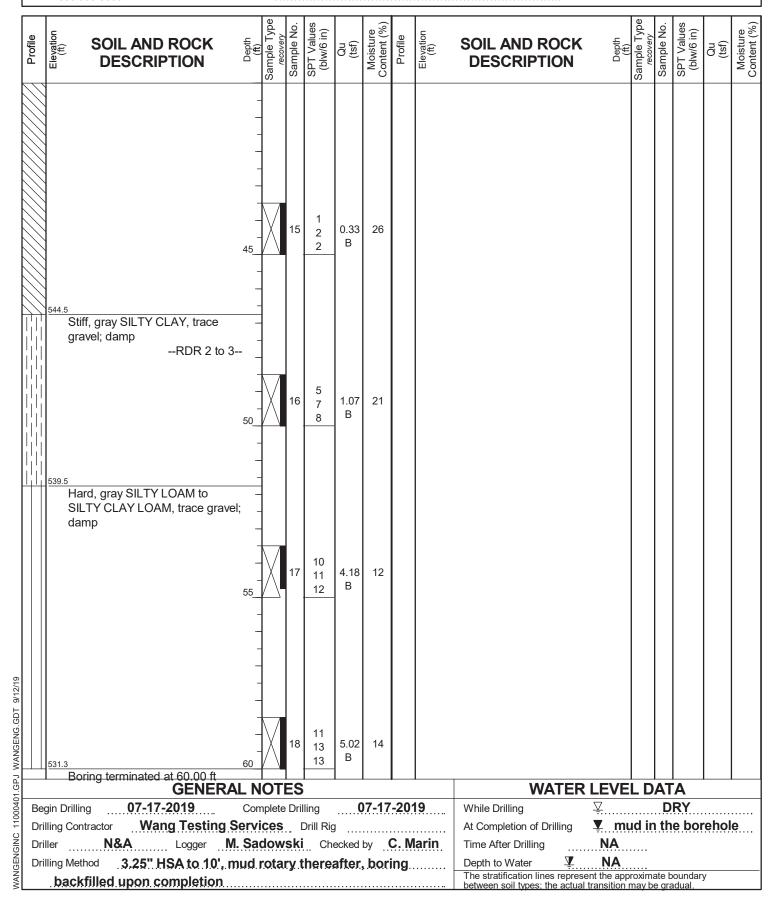


BORING LOG NB-11

WEI Job No.: 1100-04-01

Client AECOM
Project Jane Byrne Interchange
Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 591.27 ft North: 1895148.03 ft East: 1171985.82 ft Station: 6108+15.44 Offset: 117.37 RT





BORING LOG NB-13

WEI Job No.: 1100-04-01

Client AECOM

Project Jane Byrne Interchange

Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 576.50 ft North: 1896264.94 ft East: 1171770.55 ft Station: 6119+42.12 Offset: 8.18 LT

	Profile	SOIL AND ROCK degree DESCRIPTION	Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
10 10 10 10 10 10 00 00 00 00 00 00 00 0		15-inch CONCRETEPAVEMENT 575.2 574.93-inch ASPHALTPAVEMENT Medium dense, gray SANDY -573.5GRAVEL; damp		1	20 16	NP	3				- - - -		9	0 1 1	0.08 B	20
		RDR 2/ Hard, gray SILTY CLAY; dampRDR 2 5_		2	5 5 5 5	4.02 B	17				- - - 25_		10	0 1 3	0.08 B	26
		Soft to very soft, gray CLAY to SILTY CLAY, trace gravel; damp to moist -		3	1 2 1	0.16 B	26				- - -		11	2 2 3	0.25 B	23
///////////////////////////////////////		- - - 10_ 566.0 Loose, gray GRAVEL		4	0 1 1	0.41 B	25				- - 30_ -		12	3 3 3	0.33 B	24
** ***********************************		RDR 2 - - - 563.5		5	1 2 2	NP B	20			ff to very stiff, gray SILTY AY LOAM to SILTY CLAY np RDF	-					
		Very soft to soft, gray CLAY to SILTY CLAY, trace to some gravel; moistRDR 2 15_		6	1 2 2	0.16 B	23				- - 35_ -		13	6 6 8	3.28 B	16
01/21/0		- - - -		7	1 1 2	0.08 B	25				- - - -					
		20_		8	0 1 2	0.16 B	27			WATER	- - 40_		14	15 16 23	3.69 B	17
<u>:</u> -	P~	GENERAL N gin Drilling 08-25-2019 Con					8-25	-20°	10	WATER While Drilling	LEVE Ç			A RY		
		ling Contractor Wang Testing Servi	nplete i ces		-					While Drilling At Completion of Drilling	¥				ehol	e
	Dri									Time After Drilling	NA		: .+1.1	⊼. % ₩!	Y!	. .
	Dri	ling Method 2.25" HSA to 10', mud								Depth to Water $\frac{\Psi}{2}$	NA	roving	ato b	oringo	,	
		backfilled upon completion								The stratification lines represe between soil types: the actual t	nt the app	roxima	ate b e ora	oundar <u>ı</u> dual	/	



BORING LOG NB-13

WEI Job No.: 1100-04-01

Client AECOM

Project Jane Byrne Interchange

Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 576.50 ft North: 1896264.94 ft East: 1171770.55 ft Station: 6119+42.12 Offset: 8.18 LT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft) Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND		Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
WANGENGINC 110000401.GPJ WANGENG.GDT 9/12/19 L	531.5 Bo	ring terminated at 45.00 ft	50	15	13 19 21	1.48 B	20						35	S	8		0
GPJ W,		GENER	60_	F9							WATER LE	=\/E		<u> </u> ДТ	Δ		
1040 Be	gin Drilli		Complete			0	8-25	-201	19	While Drilling	VVATER LE				A RY		
Dr	illing Cor									At Completion			ıd in		e bor	ehol	e
Dr	iller	K&A Logger				ecked	by	C. M	arin	Time After Drill							
[일 Dr	illing Met	· · · · · · · · · · · · · · · · · · ·								Depth to Water		NA he ann	rovim	ate h	ounder	,	
\ \ \ \	back	filled upon completion								i ne stratification between soil type	n lines represent to es; the actual tran	ne app sition r	roxima nay be	ate bo	oundary dual.	/	

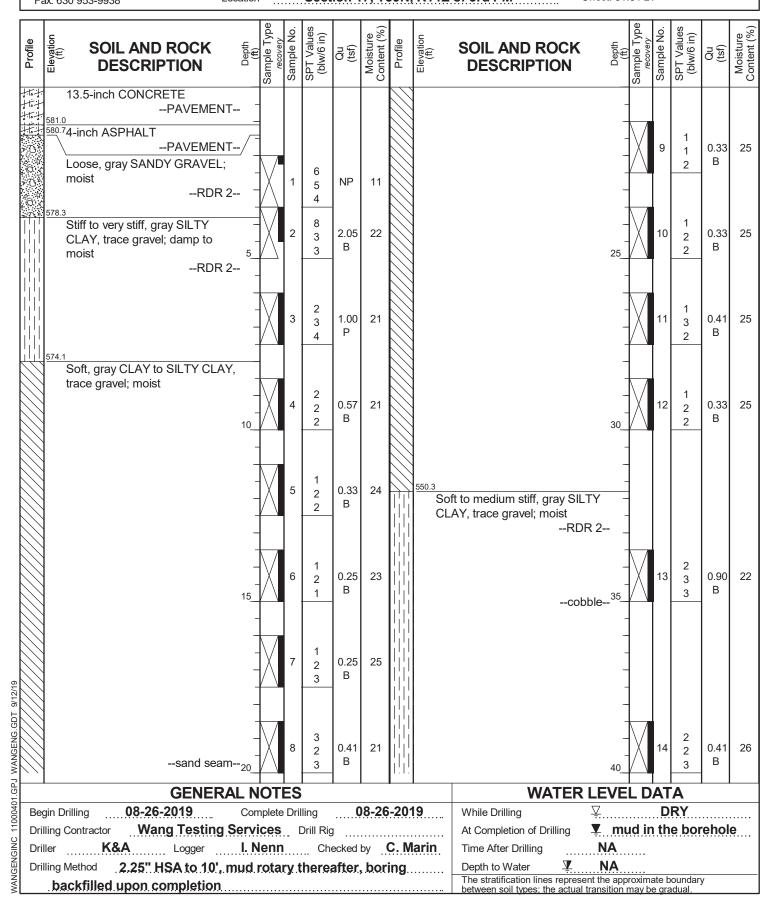


BORING LOG NB-15

WEI Job No.: 1100-04-01

Client AECOM
Project Jane Byrne Interchange
Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 582.07 ft North: 1897156.09 ft East: 1171740.80 ft Station: 6128+34.63 Offset: 31.64 LT



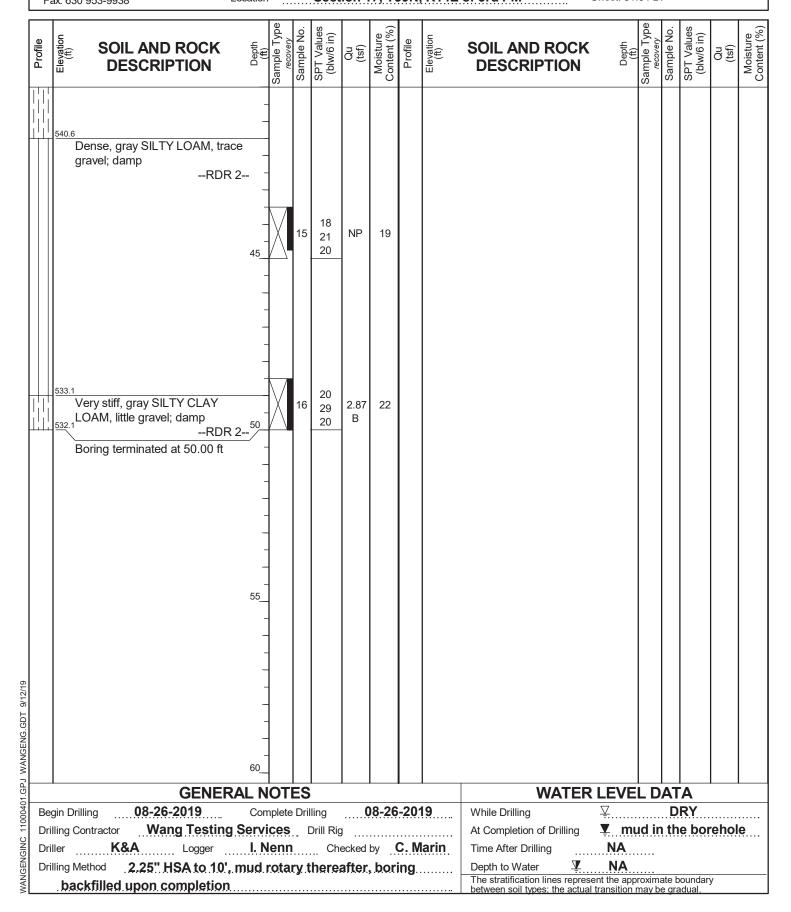


BORING LOG NB-15

WEI Job No.: 1100-04-01

Client AECOM
Project Jane Byrne Interchange
Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 582.07 ft North: 1897156.09 ft East: 1171740.80 ft Station: 6128+34.63 Offset: 31.64 LT





BORING LOG NB-21

WEI Job No.: 1100-04-01

Client AECOM

Project Jane Byrne Interchange

Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 577.89 ft North: 1899954.96 ft East: 1171596.71 ft Station: 6349+05.49 Offset: 26.08 LT

	m DESSIMI HOM	Sample Type	Sample No. SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile Elevation (ft)	SOIL AND ROO DESCRIPTIO	CK thick the contract of the c	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	Very stiff, gray SILTY CLAY to CLAY, trace gravelRDR 2		1 4 4 4	2.95 B	18				9	1 1 2	0.08 B	27
	Soft, gray CLAY, trace gravelRDR 2	- \/■	2 2 1 3	0.33 B	22			25_/	10	1 2 2	0.16 B	26
			3 1 2	0.33 B	17				11	1 3 3	0.16 B	26
	Medium stiff, gray SILTY CLAY LOAM, trace gravel Very soft to soft, gray CLAY to	10	4 1 2 2	0.41 B	23			30	12	3 3 3	0.41 B	25
	SILTY CLAY, trace gravel; damp to wet		5 2 2	0.41 B	24			-				
		15	6 1 1 2	0.41 B	21			35	13	2 3 5	0.41 B	20
			7 1 1 2	0.16 B	23		ry stiff, gray SILTY CL ce gravel; damp -	AY, –				
MANGENGINC 11000401.GPJ WANGENG.GDT	GENERAL	20	8 1 1 1 1 T	0.16 B	25		WAT	40	14 . DAT	9	2.13 B	19
Beg	in Drilling 07-18-2019	Complete [Drilling			3-2019	While Drilling	₹	D	RY		
ANGENGINC TION TO THE COLUMN THE	ing Contractor Wang Testing Se er N&A Logger M. ing Method 2.25" HSA to 10', mu backfilled upon completion	Sadows ud rotary	ki c	hecked e after	by , bor	C. Marin ing	At Completion of Drilling Time After Drilling Depth to Water The stratification lines re between soil types; the a	NA V NA epresent the approx	 ximate l	ooundar		e

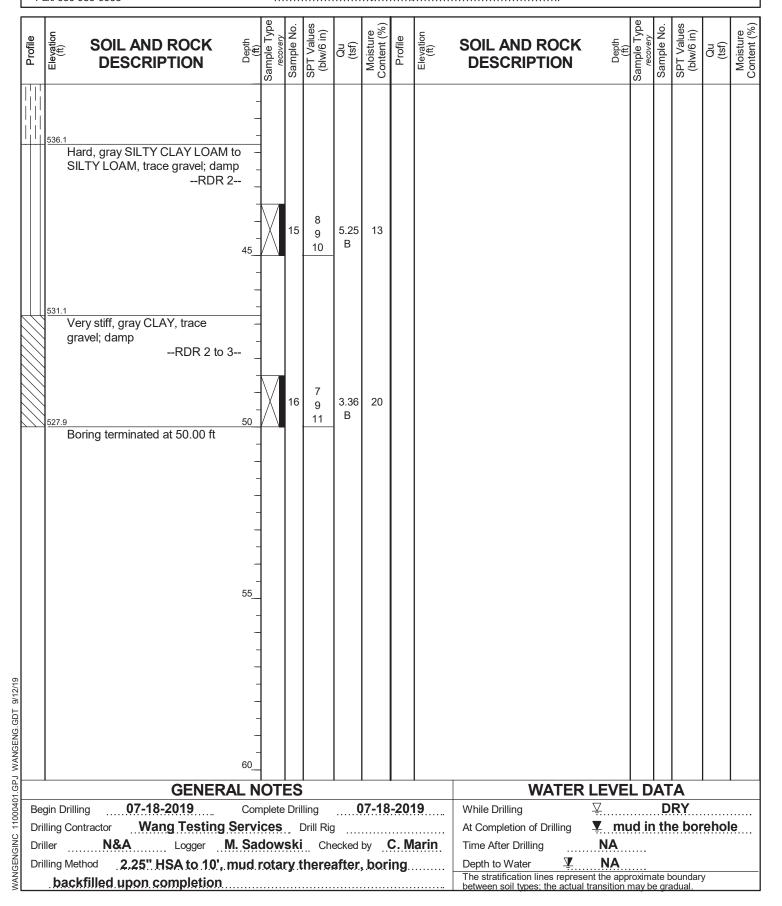


BORING LOG NB-21

WEI Job No.: 1100-04-01

Client AECOM
Project Jane Byrne Interchange
Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 577.89 ft North: 1899954.96 ft East: 1171596.71 ft Station: 6349+05.49 Offset: 26.08 LT





BORING LOG NB-22

WEI Job No.: 1100-04-01

Client AECOM
Project Jane Byrne Interchange
Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 575.93 ft North: 1900177.31 ft East: 1171607.02 ft Station: 6158+76.69 Offset: 74.07 RT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND RO		Depth (ft) Somple Tung	recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
T. 0.4 0.4 0.4 0.4 0.4 0.4 0.4 0.4 0.4 0.	<u> </u>	4-inch thick, ASPHALTPAVEMENT Medium dense, brown to gray CRUSHED STONEAGGREGATE BASEFILL		1	9 7 6	NP	2							9	1 1 2	0.33 B	25
	C 570.4	Loose, brown and gray, damp SANDY GRAVELFILLRDR 2 to 3 5		2	5 3 3	NP	4					25		10	1 1 1	0.41 B	26
	:	Medium stiff to very stiff, gray SILTY CLAY LOAM, little gravel; damp to moistRDR 2		3	2 3 4	2.30 B	15							11	1 2 3	0.90 B	25
	565.4	10		4	3 4 4	0.98 B	13					30		12	1 2 2	0.49 B	28
	\	Soft to medium stiff, gray CLAY to SILTY CLAY, trace gravel; damp to moistRDR 2		5	1 1 2	0.41 B	23			f, gray SILTY CLA` vel; damp	/, trace RDR 2	-					
		15		6	1 1 1	0.25 B	27					35		13	3 4 6	1.31 B	22
9/12/19				7	1 1 1	0.33 B	26					-					
WANGENGINC 11000401.GPJ WANGENG.GDT 9/12/19		20.	1	8	1 1 1	NR						40		14	3 4 5	1.64 B	20
401.GL		GENERAL I					7 40	00	10		TER LE						
) Be	egin Dr rilling C	illing 07-10-2019 Co	mplete		_	0				While Drilling At Completion of Dr		muc		5.00 the		ehol	
ON Dr	riller	N&A Logger M. S								Time After Drilling				ri i G	DOI		۲
Dr GEN	rilling N									Depth to Water	Å N	A					_
WAN	ba	ckfilled upon completion		-				_		The stratification lines between soil types; the	s represent the e actual transi	appro	ximat vy be	e boi grad	undary lual.	′	



BORING LOG NB-22

WEI Job No.: 1100-04-01

Client AECOM
Project Jane Byrne Interchange
Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 575.93 ft North: 1900177.31 ft East: 1171607.02 ft Station: 6158+76.69 Offset: 74.07 RT

Profile	SOIL AND ROCK DESCRIPTION	Depth (ft) Sample Type recovery Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND		Depth (ft)	Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	30.9	15	3 4 5	1.56 B	25										
	Boring terminated at 45.00 ft	-													
]													
		-													
		50													
		-													
		-													
		-													
		55													
		-													
		-													
		-													
		-													
	GENER	AL NOTES							WATER	I FVF	 D	ΔΤ	Δ		
Beair	n Drilling 07-10-2019	Complete Dril		0	7-10	-201	19	While Drilling	VVAI LIN	<u> </u>			0 ft		
_	ng Contractor Wang Testing		_					At Completion	of Drilling	Ψ mι				ehol	e
Drille								Time After Dril							
	ng Method 3,25" HSA to 10',							Depth to Wate		NA ent the app	roxim	ate h	oundan	/	
	backfilled upon completion.							The stratificatio between soil type	es; the actual	transition	may b	e gra	idual.	'	



BORING LOG VST-01

WEI Job No.: 1100-04-01

Client AECOM

Project Jane Byrne Interchange

Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 593.55 ft North: 1897108.36 ft East: 1171435.63 ft Station: 7313+90.47 Offset: 2.00 LT

	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft) Sample Typ	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIT VALUES (FIX) Sample Type (A) SPT Values (B)(W/6 in) Quarter (Esf)	Moisture Content (%)					
			Very stiff, brown SILTY CLAY LOAM, trace gravel FILL		1	3 5	2.75	14			S _{u undis} = 786.3 psf S _{u remold} = 371.3 psf Sensitivity = 2.1						
		590.5 N	Medium dense, fine SAND		/	5	P				In-Situ Vane Shear, 22.0 feet S _{u undis} = 742.6 psf S _{u remold} = 305.8 psf Sensitivity = 2.4						
				5	2	5 6	NP	7			In-Situ Vane Shear, 24.5 feet ₂₅ 3S _{u undis} = 578.8 psfS _{u remold} = 382.2 psfSensitivity = 1.5						
			Medium stiff to stiff, gray SILTY CLAY	-	/	2					In-Situ Vane Shear, 27.0 feet S _{u undis} = 742.6 psf S _{u remold} = 415.0 psf Sensitivity = 1.8						
				10	3	2 3	1.31 B	26			In-Situ Vane Shear, 29.5 feet ₃₀ 5 S _{u undis} = 589.7 psf S _{u remold} = 283.9 psf Sensitivity = 2.1						
					/	2	0.00	00			In-Situ Vane Shear, 32.0 feet S _{u undis} = 1026.6 psf S _{u remold} = 447.8 psf Sensitivity = 2.3						
		578.0	Soft, gray SILTY CLAY	15	4	2 3	0.98 B	28			In-Situ Vane Shear, 34.5 feet ₃₅ 7S _{u undis} = 764.5 psfS _{u remold} = 480.5 psfSensitivity = 1.6						
		575.3			5	1 2 2	0.25 P	29			In-Situ Vane Shear, 37.0 feet S _{u undis} = 1026.6 psf S _{u remold} = 589.7 psf Sensitivity = 1.7						
			In-Situ Vane Shear, 19.5 feet	20 [1						40_						
<u>;</u> [GENERAL	NO	WATER LEVEL DATA												
	Ве	egin Dri	illing 12-01-2015	Comple	While Drilling groundwater not observed												
	Dr	-	Contractor Wang Testing Se								At Completion of Drilling mud in the borehole						
Driller R&N Logger F. Bozga Checked by A. Kurnia																	
Drilling Method 2.25" HSA to 10', mud rotary thereafter, boring											The stratification lines represent the approximate boundary						
1		pag	ckfilled upon completion								between soil types; the actual transition may be gradual.						

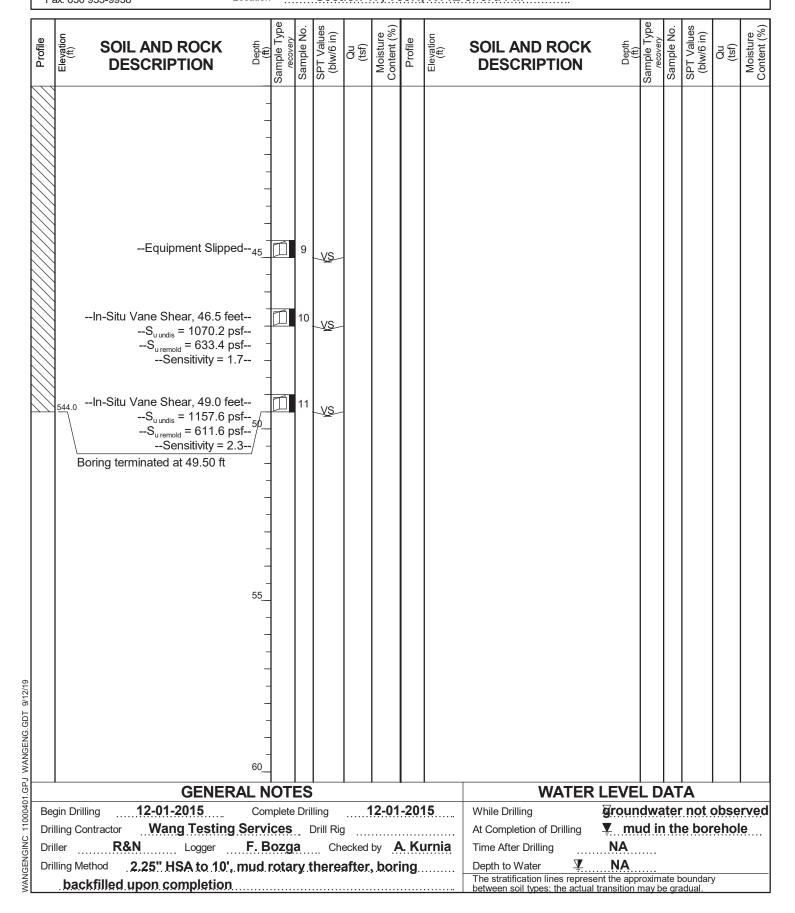


BORING LOG VST-01

WEI Job No.: 1100-04-01

Client AECOM
Project Jane Byrne Interchange
Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 593.55 ft North: 1897108.36 ft East: 1171435.63 ft Station: 7313+90.47 Offset: 2.00 LT



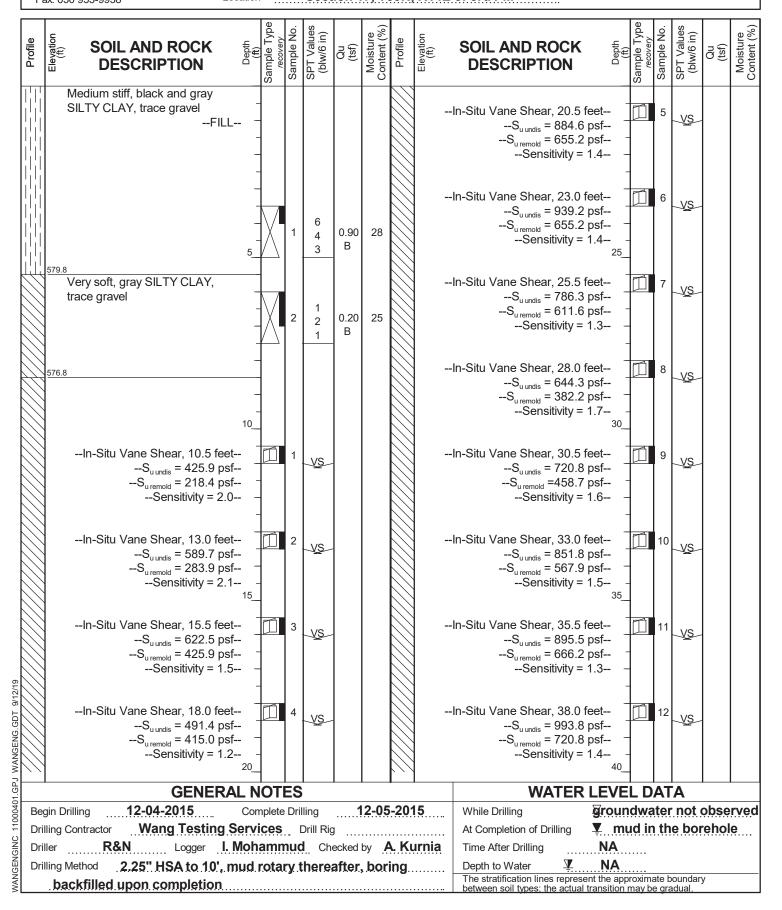


BORING LOG VST-02

WEI Job No.: 1100-04-01

Client AECOM
Project Jane Byrne Interchange
Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 585.26 ft North: 1899543.57 ft East: 1171652.91 ft Station: 8415+02.96 Offset: 258.109 RT



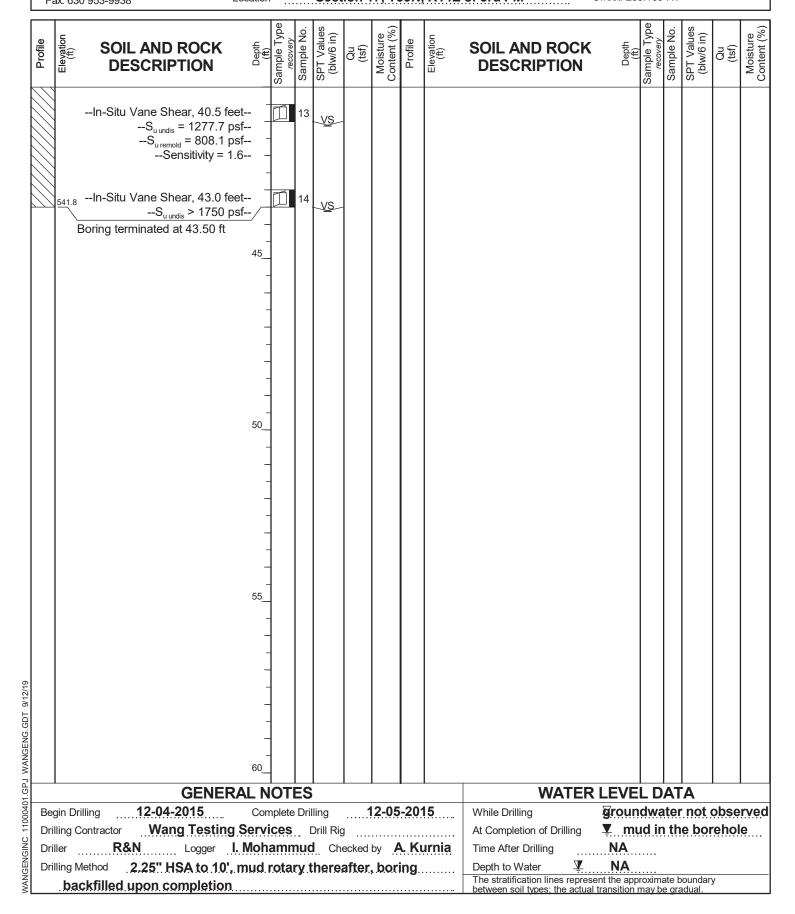


BORING LOG VST-02

WEI Job No.: 1100-04-01

Client AECOM
Project Jane Byrne Interchange
Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 585.26 ft North: 1899543.57 ft East: 1171652.91 ft Station: 8415+02.96 Offset: 258.109 RT



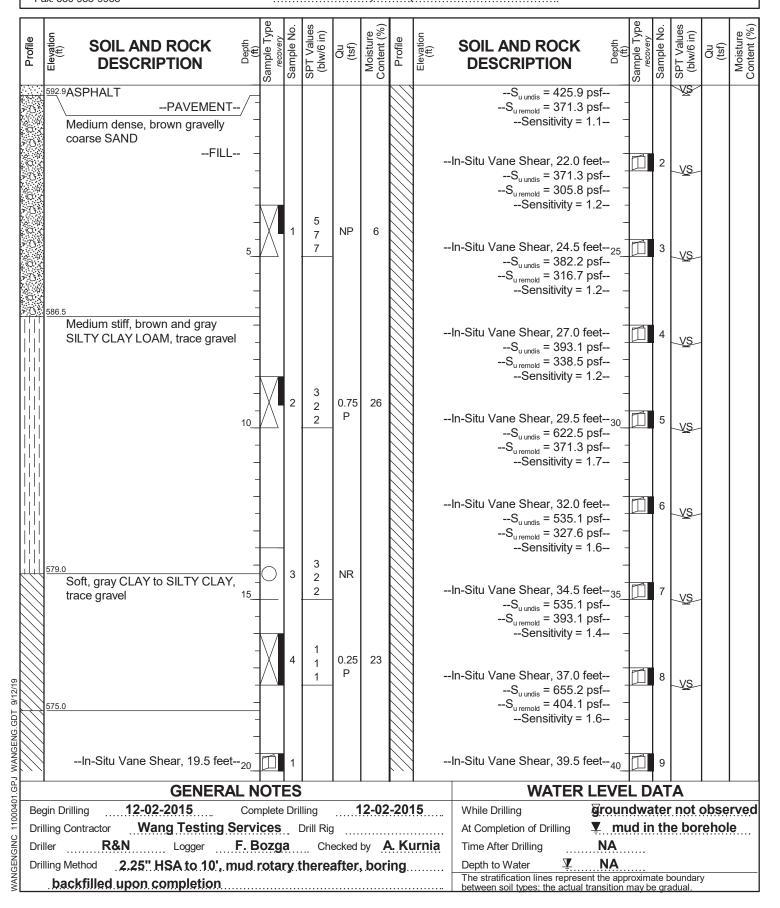


BORING LOG VST-03

WEI Job No.: 1100-04-01

Client AECOM
Project Jane Byrne Interchange
Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 593.21 ft North: 1899985.05 ft East: 1171693.33 ft Station: 8415+53.90 Offset: 182.276 LT





BORING LOG VST-03

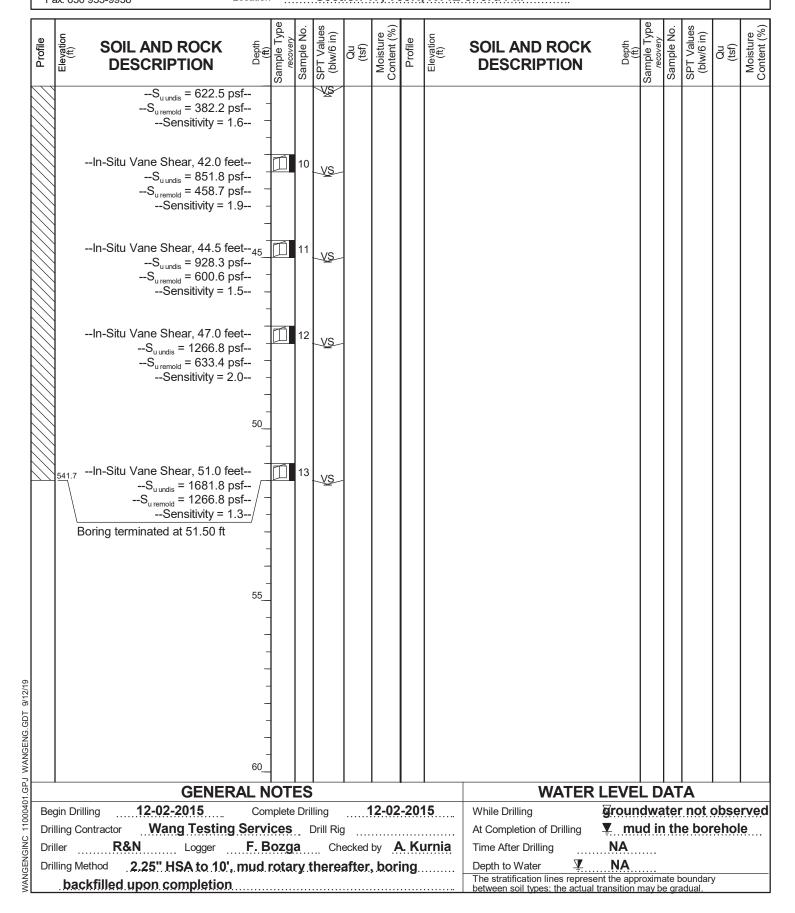
WEI Job No.: 1100-04-01

Client AECOM

Project Jane Byrne Interchange

Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 593.21 ft North: 1899985.05 ft East: 1171693.33 ft Station: 8415+53.90 Offset: 182.276 LT



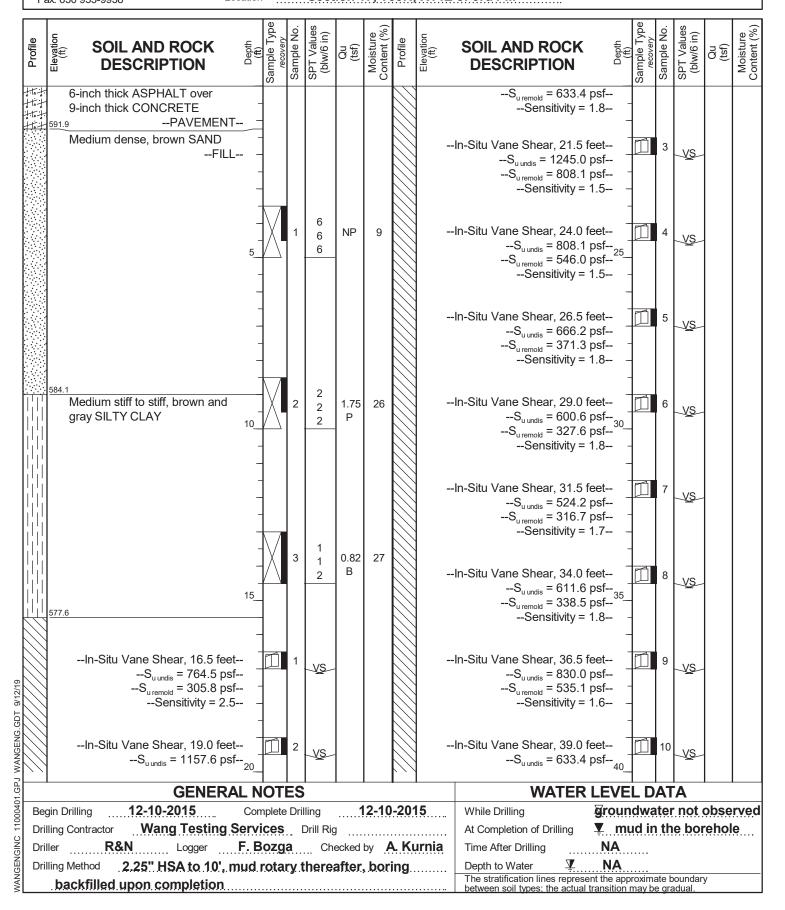


BORING LOG VST-07

WEI Job No.: 1100-04-01

Client AECOM
Project Jane Byrne Interchange
Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 593.11 ft North: 1895740.00 ft East: 1171636.91 ft Station: 6247+22.16 Offset: 105.461 RT





BORING LOG VST-07

WEI Job No.: 1100-04-01

Client AECOM

Project Jane Byrne Interchange

Location Section 17, T39N, R14E of 3rd PM

Datum: NAVD 88 Elevation: 593.11 ft North: 1895740.00 ft East: 1171636.91 ft Station: 6247+22.16 Offset: 105.461 RT

i	Profile	SOIL AND ROCK DESCRIPTION	Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ff)	Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
		S _{u remold} = 393.1 psf Sensitivity = 1.6	-													
		In-Situ Vane Shear, 41.5 feet S _{u undis} = 895.5 psf S _{u remold} = 655.2 psf Sensitivity = 1.4		11	<u>Vs</u>											
		In-Situ Vane Shear, 44.0 feet S _{u undis} = 1026.6 psf S _{u remold} = 698.9 psf Sensitivity = 1.5 Boring terminated at 44.50 ft		12	VS.											
		-	-													
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112/19		<u>-</u>	- - -													
WANGENGINC 11000401.GPJ WANGENG.GDT 9/12/19		60_ 60_								\A/A TES -	F-1 //-					
401.G	Г.	GENERAL N					12-10	204	15	WATER LEVEL DATA						
ENGINC 11000	Dril Dril	gin Drilling 12-10-2015 Colling Contractor Wang Testing Serviller R&N Logger F. Elling Method 2.25" HSA to 10', mud	While Drilling At Completion of Drilling Time After Drilling Depth to Water The stratification lines represent the approximate boundary													
WANG		backfilled upon completion	The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.													