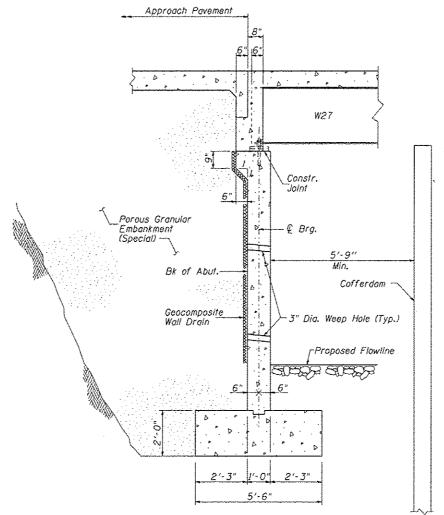
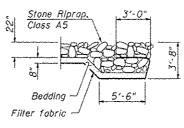
GENERAL NOTES

- Fosteners shall be ASTM A325 Type 1, mechanically galvanized bolts in painted areas and M164 Type 3 in unpainted areas. Bolts 34 in. \$\phi\$, holes \$^{15}_{16}\$ in. \$\phi\$, unless otherwise noted.
- 2. Calculated weight of Structural Steel = 63,520 lb. (AASHTO M270 Grade 50W)
- 3. All structural steel shall be AASHTO M 270 Grade 50W.
- 4. No field welding is permitted except as specified in the contract documents.
- 5. Reinforcement bars designated (E) shall be epoxy coated.
- 6. If the Contractor elects to use cantilever forming brackets on the exterior beams, the brackets shall be placed at the same locations as required for the hardwood blocks in Article 503.06(b) of the Standard Specifications. If additional cantilever forming brackets are required, hardwood blocking shall be wedged between the exterior and first interior beam at each of these additional bracket locations.
- 7. Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.
- Bearing seat surfaces shall be constructed or adjusted to the designated elevations within a tolerance of ¹g inch (0.01 ft.). Adjustment shall be made either by grinding the surface or by shimming the bearings.
- Structural steel shall only be painted for a distance equal to the depth of embedment into the concrete cap plus 3 inches. Painted areas shall be primed in the shop with a Department approved zinc rich primer. Field painting will not required.
- Layout of slope protection system may be varied to suit ground conditions in the field as directed by the Engineer.
- The embankment configuration shown shall be the minimum embankment that must be placed and compacted prior to construction of the abutments.
- 12. The concrete for bridge decks finished according to Article 503.16(a) of the Standard Specifications shall be placed and compacted parallel to the skew in uniform increments along centerline of bridge. The machine used for finishing shall be set parallel to the skew for striking off and screeding the concrete.
- 13. Excavation behind existing abutment walls shall be performed to balance front and back soil pressure before removing the existing superstructure. The Contractor shall sawcut the upper portion of the existing abutment at the stage removal line before Stage I removal to ensure the remaining portion will not be prematurely damaged.
- 14. Slipforming of parapets is not allowed.

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION



SECTION THRU CLOSED ABUTMENT



SECTION A-A
(See Sheet 1 for Plan location)

TOTAL BILL OF MATERIAL

	TOTAL BILL	UI INIM	11 1111	\	
	ITEM	UNIT	SUPER	SUB	TOTAL
Δì	Porous Granular Embankment, Special	Cu. Yd.		5485	5485
		1			
	Stone Riprap. Class A5	Sq. Yd.		885	885
	Filter Fabric	Sq. Yd.		885	885
	Removal of Existing Superstructures	Each	2		2
	Concrete Removal	Cu. Yd.		87.0	87.0
	Structure Excavation	Cu. Yd.	213	2760	2973
A [Cofferdam Excavation	Cu. Yd.		450	450
٠: كلك	COTTO GAME EXCEPTION			700	
	Cofferdam (Type 2) (Location-1)	Each		1	1
	Cofficial Trans (1) (1 1) - (1)	ļ			
	Cofferdam (Type 2) (Location-2)	Each			
	Concrete Structures	Cu. Yd.	96.5	246,6	343.1
	Concrete Superstructure	Cu. Yd.	564.6		564.6
	Bridge Deck Grooving	Sa. Yd.	1284		1284
		1 59. 70.	1201		X201
	Protective Coat	Sq. Yd.	1462		1462
	Cardelia and Cardelia Charles				
	Furnishing and Erecting Structural Steel	L. Sum	1		1
					··
	Stud Shear Connectors	Each	4560		4560
	Davidson Davidson	0	147.000	70.000	100 010
	Reinforcement Bars, Epoxy Coated	Pound	143,690	39,220	182,910
	Bor Splicers	Each	536	44	580
	Temporary Sheet Piling	Sq. F1.		2896	2896
	Name Plates	Each	2	:	2
					5
	Anchor Bolts, I"	Each	64		64
	Geocomposite Wall Drain	E . V.		566	F.C.C
	Geocomposite waii Drain .	Sq. Yd.		566	566

WATERWAY INFORMATION

Drainage Are	a = 2.8	7 sq. mi.			Grade Elev Grade Elev.				
Flood	Freq. 0		Opening Sq. F1.		Nat.	Head - F1.		Headwater El.	
1 1000	Yr.	(C.F.S.)	Exist.	Prop.	H.W.E.	Exist.	Prop.	Exist.	Prop.
	10	980	192.3	213.6	404.4	0.6	0.3	405.0	404.7
Design	50	1470	206.9	228.2	404.9	1.7	1.3	406.6	406.2
Base	100	1680	212.8	234.1	405.1	2.9	1.7	408.0	406.8
Overtopping	-	1 ^	-	-	-	-	-	١	-
Max. Calc.	500	2170	221.5	242.9	405.4	3.4	3.5	408.8	408.9

Exist. 10-Year Velocity Through Bridge=5.0 fps Prop. 10-Year Velocity Through Bridge=4.6 fps

GENERAL DATA
F.A.I. RTE. 57 OVER
LAKE CREEK BRANCH
STATION 228+25.00
STRUCTURE NO. 100-0010 (N.B.)
STRUCTURE NO. 100-0011 (S.B.)

Осмт	SHEET NO. 2	F.A.S. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
CRAWFORD MURPHY & TILLY, INC. CONSULTING ENGINEERS		I-57	(X1-4-1) BR -1	WILLIAMSON	202	85
SPRINGPALD, I # AURORA, II # ST. LOUS, MO REVISED DESIGNED BY: KEH DRAWN BY: GLD A	29 SHEETS			CONTRACT	NO. 78	334
4-16-13 CHECKED BY: KEH ORANN BY: GLD ATE: \$75.60		FED, RO	AD DIST. NO. 9 ILLINOIS FED. A	D PROJECT		