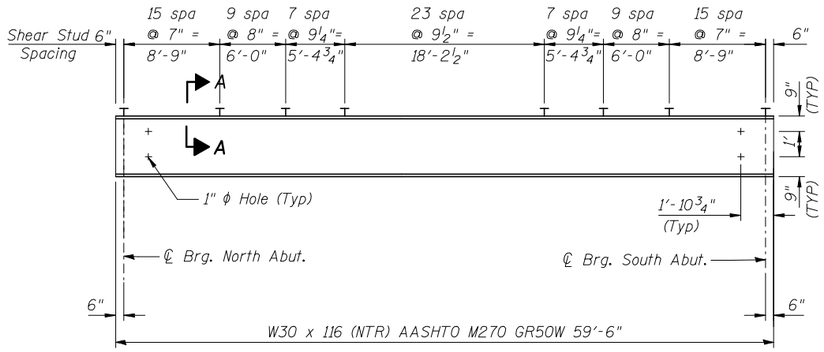
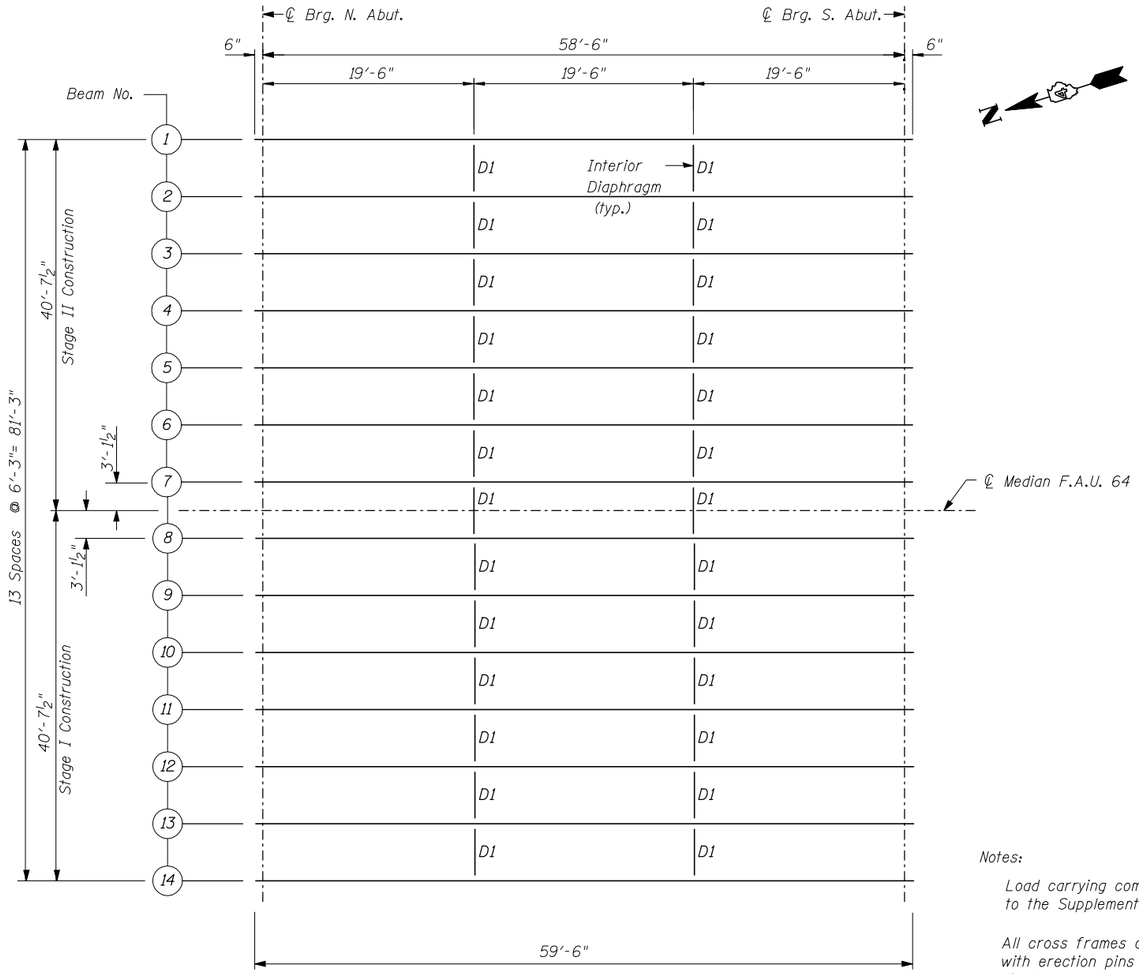


ELEVATION



FRAMING PLAN

INTERIOR GIRDER MOMENT TABLE		
		0.5 Span
I_s	(in^4)	4930
$I_c(n)$	(in^4)	13147
$I_c(3n)$	(in^4)	9909
$I_c(cr)$	(in^4)	-
S_s	(in^3)	329
$S_c(n)$	(in^3)	477
$S_c(3n)$	(in^3)	440
$S_c(cr)$	(in^4)	-
Z	(in^3)	378
DC1	(k/')	0.780
M_{DC1}	('k)	334
DC2	(k/')	0.064
M_{DC2}	('k)	27
DW	(k/')	0.300
M_{DW}	('k)	128
M_{L+IM}	('k)	691
M_u (Strength I)	('k)	1853
$\phi_f M_n$	('k)	2466
f_s DC1	(ksi)	12.2
f_s DC2	(ksi)	0.8
f_s DW	(ksi)	3.4
f_s (L+IM)	(ksi)	17.4
f_s (Service II)	(ksi)	39.0
$0.95R_hF_{yf}$	(ksi)	45.5
f_s (Total)(Strength I)	(ksi)	-
$\phi_f F_n$	(ksi)	-
V_f	(k)	41.1

INTERIOR GIRDER REACTION TABLE		
		Abut.
R_{DC1}	(k)	23.79
R_{DC2}	(k)	1.95
R_{DW}	(k)	9.15
R_{L+IM}	(k)	68.34
R_{Total}	(k)	103.23

I_s, S_s: Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total-Strength I, and Service II) due to non-composite dead loads (in.^{4} and in.^{3}).

$I_c(n)$, $S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections, due to short-term composite live loads (in.^{4} and in.^{3}).

$I_c(3n)$, $S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections, due to long-term composite (superimposed) dead loads (in.^{4} and in.^{3}).

$I_c(cr)$, $S_c(cr)$: Composite moment of inertia and section modulus of the steel and longitudinal deck reinforcement, used for computing f_s (Total-Strength I and Service II) in cracked sections, due to both short-term composite live loads and long-term composite dead loads (in.^{4} and in.^{3}).

DC1: Un-factored non-composite dead load (kips/ft.).

M_{DC1}: Un-factored moment due to non-composite dead load (kip-ft.).

DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).

M_{DC2}: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).

DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).

M_{DW}: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).

M_{L+IM}: Un-factored live load moment plus dynamic load allowance (impact) ((kip-ft.).

M_u (Strength I): Factored design moment (kip-ft.).
$1.25\ (M_{DC1} + M_{DC2}) + 1.5\ M_{DW} + 1.75\ M_{L+IM}$

$\phi_f M_n$: Compact composite positive moment capacity computed according to Article 6.10.7.1 (kip-ft.).

f_s DC1: Un-factored stress at edge of flange for controlling steel flange due to vertical non-composite dead loads as calculated below (ksi).
M_{DC1} / S_{nc}

f_s DC2: Un-factored stress at edge of flange for controlling steel flange due to vertical composite dead loads as calculated below (ksi).
$M_{DC2} / S_c(3n)$ or $M_{DC2} / S_c(cr)$ as applicable.

f_s DW: Un-factored stress at edge of flange for controlling steel flange due to vertical composite future wearing surface loads as calculated below (ksi).
$M_{DW} / S_c(3n)$ or $M_{DW} / S_c(cr)$ as applicable.

f_s (L+IM): Un-factored stress at edge of flange for controlling steel flange due to vertical composite live plus impact loads as calculated below (ksi).
$M_{L+IM} / S_c(n)$ or $M_{L+IM} / S_c(cr)$ as applicable.

f_s (Service II): Sum of stresses as computed below (ksi).
$f_{sDC1} + f_{sDC2} + f_{sDW} + 1.3\ f_s(L+IM)$

$0.95R_hF_yf$: Composite stress capacity for Service II loading according to Article 6.10.4.2 (ksi).

f_s (Total)(Strength I): Sum of stresses as computed below on non-compact section (ksi).
$1.25\ (f_{sDC1} + f_{sDC2}) + 1.5\ f_{sDW} + 1.75\ f_{sL+IM}$

$\phi_f F_n$: Non-Compact composite positive or negative stress capacity for Strength I loading according to Article 6.10.7.2 (ksi).

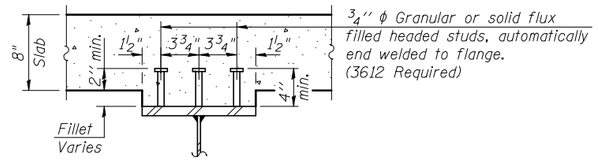
V_f: Maximum factored shear range in composite portion of span computed according to Article 6.10.10.

TOP OF BEAM ELEVATIONS (For Fabrication Only)														
Location	Beam 1	Beam 2	Beam 3	Beam 4	Beam 5	Beam 6	Beam 7	Beam 8	Beam 9	Beam 10	Beam 11	Beam 12	Beam 13	Beam 14
℄ Brg. N. Abut.	474.71	474.84	474.97	475.10	475.19	475.29	475.39	475.39	475.29	475.39	475.19	475.10	474.97	474.84	474.71
℄ Brg. S. Abut.	474.91	475.04	475.17	475.30	475.39	475.49	475.59	475.59	475.49	475.39	475.30	475.17	475.04	474.91

Notes:

Load carrying components designated "NTR" shall conform to the Supplemental Requirements for Notch Toughness Zone 2.

All cross frames or diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted. Individual cross frames or diaphragms at supports may be temporarily disconnected to install bearing anchor rods.



SECTION A-A

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		CHECKED - MJS	REVISED -
	PLOT SCALE = $SCALE$	DRAWN - RMH	REVISED -
THE UPCHURCH GROUP, INC.	PLOT DATE = 12/17/2012 10:01:28 AM	CHECKED - LP	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

FRAMING PLAN
STRUCTURE NO. 072-0226
SHEET NO. 14 OF 27 SHEETS

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
64	(10B) BR	PEORIA	77	42
		CONTRACT NO. 68481		
	ILLINOIS FED. AID PROJECT			