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06-16-2023 LETTING ITEM 094

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SEE SHEET 5 OF 65 FOR HIGHWAY STANDARDS

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

**PLANS FOR PROPOSED
ILLINOIS SPECIAL BRIDGE PROGRAM**

**PROJECT NO. ZHOY(896)
SECTION 18-00166-00-BR
WOODFORD COUNTY
F.A.S. 363 / C.H. 7
STRUCTURE NO. 102-3130 & 102-3131
C-94-045-20**

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
F.A.S. 363	18-00166-00-BR	WOODFORD	65	1
FED. ROAD DIST. NO. 7	ILLINOIS	CONTRACT NO. 89779		

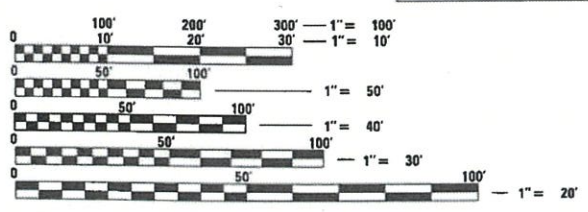


LOCATION OF SECTION INDICATED THUS: -

UTILITIES

- FRONTIER COMMUNICATIONS
109 E. MARKET STREET
BLOOMINGTON, IL 61701
- CORN BELT ENERGY CO-OP
1 ENERGY WAY
BLOOMINGTON, IL 61705
- METRO COMMUNICATIONS COMPANY
8 S. WASHINGTON ST #200
SULLIVAN, IL 61951

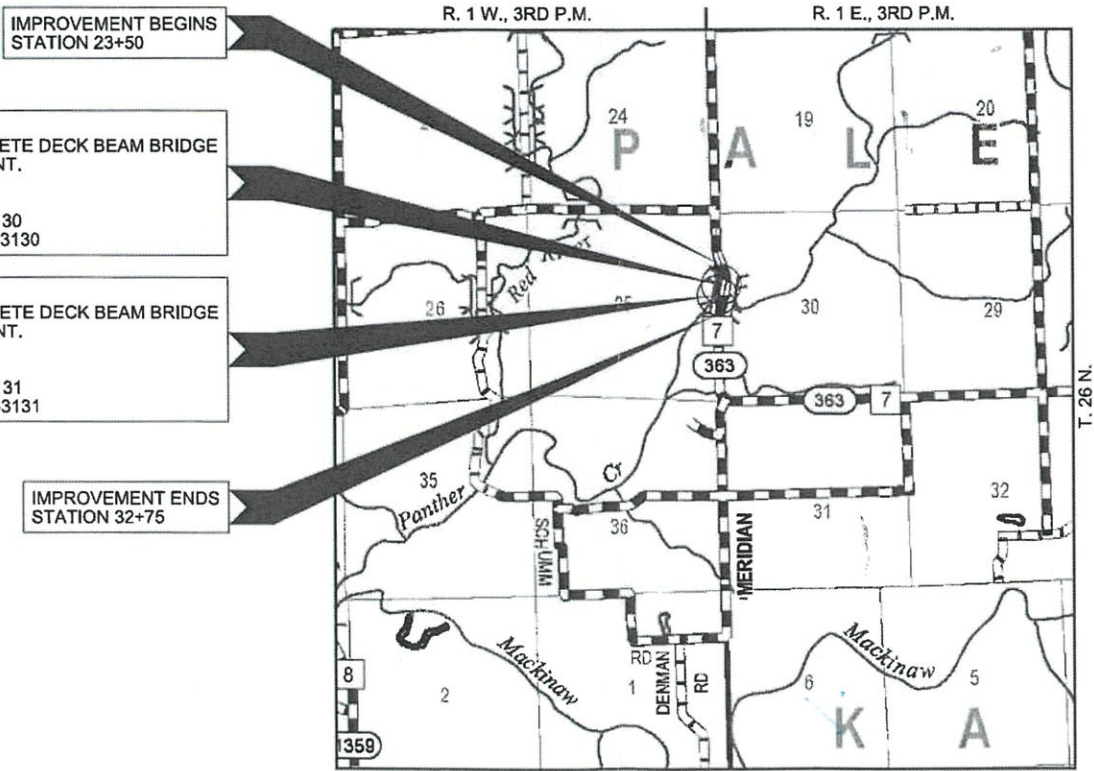
- STA. 26+04
PRECAST PRESTRESSED CONCRETE DECK BEAM BRIDGE
SUPERSTRUCTURE REPLACEMENT.
THREE SPANS AT 47'-10"
30'-0" RDWY., SKEW = 0°
EXISTING STRUCTURE NO. 102-3130
PROPOSED STRUCTURE NO. 102-3130
- STA. 29+40
PRECAST PRESTRESSED CONCRETE DECK BEAM BRIDGE
SUPERSTRUCTURE REPLACEMENT.
FOUR SPANS AT 39'-8"
30'-0" RDWY., SKEW = 0°
EXISTING STRUCTURE NO. 102-3131
PROPOSED STRUCTURE NO. 102-3131



FULL SIZE PLANS HAVE BEEN PREPARED USING STANDARD ENGINEERING SCALES. REDUCED SIZED PLANS WILL NOT CONFORM TO STANDARD SCALES. IN MAKING MEASUREMENTS ON REDUCED PLANS, THE ABOVE SCALES MAY BE USED.

FUNCTIONAL CLASSIFICATION: MAJOR COLLECTOR
DESIGN SPEED: 40 MPH
DESIGN TRAFFIC: 525 ADT
3R DESIGN GUIDELINES ARE USED

**CATALOG NO. 036018-00
CONTRACT NO. 89779**



LOCATION MAP

APPROXIMATE SCALE: 1" = 1/2 MILE
NET LENGTH OF SECTION = 925 FEET = 0.175 MILES



WARNING

**CALL 811
BEFORE YOU DIG**

DIG NO: X1191503

ILLINOIS DEPARTMENT OF TRANSPORTATION

APPROVED 02/28 2023
CEO
COUNTY ENGINEER

PASSED March 24 2023
Tony Sassine (ARS)
DISTRICT FOUR ENGINEER OF LOCAL ROADS & STREETS

Releasing For Bid Based on Limited Review March 24 2023
Rena A. James
REGION THREE ENGINEER

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

DATE: 02/27/2023

HAMPTON, LENZINI AND RENWICK, INC.
CIVIL ENGINEERS - STRUCTURAL ENGINEERS - LAND SURVEYORS
3085 STEVENSON DRIVE, SUITE 201
SPRINGFIELD, ILLINOIS 62703
217.546.3400 www.hlrengineering.com

184.000959
ILLINOIS PROFESSIONAL DESIGN FIRM L3 / PE / SE CORPORATION

EXPIRES: 11/30/2023 PROJECT NUMBER: 19.0013.130 DATE: 02/27/2023

SUMMARY OF QUANTITIES

CODE NO.	ITEM	UNIT	ROADWAY	SN	SN	TOTAL
			CONSTR. TYPE CODE	102-3130 CONSTR. TYPE CODE	102-3131 CONSTR. TYPE CODE	
			0005	0013	0013	TOTAL
20200100	EARTH EXCAVATION	CU YD	455			455
20300100	CHANNEL EXCAVATION	CU YD		100		100
20400800	FURNISHED EXCAVATION	CU YD	660			660
28100209	STONE RIPRAP, CLASS A5	TON		685		685
28200200	FILTER FABRIC	SQ YD	140	435		575
28300400	AGGREGATE DITCH	TON	100			100
35650600	BASE COURSE WIDENING (VARIABLE DEPTH)	SQ YD	138			138
40200800	AGGREGATE SURFACE COURSE, TYPE B	TON	154			154
40600290	BITUMINOUS MATERIALS (TACK COAT)	POUND	705			705
40604050	HOT-MIX ASPHALT SURFACE COURSE, IL-9.5, MIX "C", N50	TON	333	49	55	437
40800050	INCIDENTAL HOT-MIX ASPHALT SURFACING	TON	6			6
44004000	PAVED DITCH REMOVAL	FOOT	90			90
48101200	AGGREGATE SHOULDERS, TYPE B	TON	288			288
^ 50101500	REMOVAL OF EXISTING SUPERSTRUCTURES	EACH		1	1	2
50102400	CONCRETE REMOVAL	CU YD		4.4	4.7	9.1
50105220	PIPE CULVERT REMOVAL	FOOT	96			96
50300225	CONCRETE STRUCTURES	CU YD		13.1	10.8	23.9
^ 50400305	PRECAST PRESTRESSED CONCRETE DECK BEAMS (17" DEPTH)	SQ FT			4,760	4,760
^ 50400405	PRECAST PRESTRESSED CONCRETE DECK BEAMS (21" DEPTH)	SQ FT		4,305		4,305
50800205	REINFORCEMENT BARS, EPOXY COATED	POUND		1,320	1,400	2,720
* 50900205	STEEL RAILING, TYPE S1	FOOT		294	324	618
^ 542D0220	PIPE CULVERTS, CLASS D, TYPE 1 15"	FOOT	40			40
^ 542D0223	PIPE CULVERTS, CLASS D, TYPE 1 18"	FOOT	80			80
58100200	WATERPROOFING MEMBRANE SYSTEM	SQ YD		484	535	1,019
58300100	PORTLAND CEMENT MORTAR FAIRING COURSE	FOOT		325	360	685
59300100	CONTROLLED LOW-STRENGTH MATERIAL	CU YD		24	20	44

^ SEE SPECIAL PROVISIONS

* SPECIALTY ITEMS

SUMMARY OF QUANTITIES

CODE NO.	ITEM	UNIT	ROADWAY	SN	SN	TOTAL
			CONSTR. TYPE CODE	102-3130 CONSTR. TYPE CODE	102-3131 CONSTR. TYPE CODE	
			0005	0013	0013	TOTAL
* 63000001	STEEL PLATE BEAM GUARDRAIL, TYPE A, 6 FOOT POSTS	FOOT	25			25
* 63100075	TRAFFIC BARRIER TERMINAL, TYPE 5A	EACH	8			8
* 63100167	TRAFFIC BARRIER TERMINAL, TYPE 1 (SPECIAL) TANGENT	EACH	8			8
63200310	GUARDRAIL REMOVAL	FOOT	416			416
67100100	MOBILIZATION	L SUM	1			1
70300150	SHORT TERM PAVEMENT MARKING REMOVAL	SQ FT	104			104
70306120	TEMPORARY PAVEMENT MARKING - LINE 4" - TYPE III TAPE	FOOT	3,700			3,700
* 72501000	TERMINAL MARKER - DIRECT APPLIED	EACH	8			8
* 78001110	PAINT PAVEMENT MARKING - LINE 4"	FOOT	3,700			3,700
^ X2501000	SEEDING, CLASS 2 (SPECIAL)	ACRE	0.60			0.6
^ X4401198	HOT-MIX ASPHALT SURFACE REMOVAL, VARIABLE DEPTH	SQ YD	350			350
^ X5150110	NAME PLATES (SPECIAL)	EACH		1	1	2
^ Z0001900	ASBESTOS BEARING PAD REMOVAL	EACH		66	88	154
^ Z0012754	STRUCTURAL REPAIR OF CONCRETE (DEPTH ≤ THAN 5 INCHES)	SQ FT		120	5	125
^ Z0013798	CONSTRUCTION LAYOUT	L SUM	1			1

^ SEE SPECIAL PROVISIONS

* SPECIALTY ITEMS

EARTHWORK SCHEDULE							
LOCATION	EARTH EXCAVATION	CHANNEL EXCAVATION	SHRINKAGE FACTOR	PERCENT USED	EXCAVATION ADJUSTED FOR SHRINKAGE	EMBANKMENT REQUIRED	EARTHWORK BALANCE
	CU.YD.	CU.YD.			CU.YD.	CU.YD.	CU.YD.
FAS 363 / CH 7							
STA. 23+50.00 TO STA. 25+31.47	85		25.00%	100.00%	63	72	-9
STA. 25+31.47 TO STA. 26+76.53		100	25.00%	70.00%	53	0	53
STA. 26+76.53 TO STA. 28+59.84	49		25.00%	100.00%	37	484	-447
STA. 28+59.84 TO STA. 30+20.16		0	25.00%	70.00%	0	0	0
STA. 30+20.16 TO STA. 32+75.00	319		25.00%	100.00%	239	332	-93
ENTRANCES						166	-166
TOTAL	453	100			392	1054	-662
USE	455	100					660

FURNISHED 660 CU YDS

ROADWAY SCHEDULE					
LOCATION	BASE COURSE WIDENING (VARIABLE DEPTH)	BITUMINOUS MATERIALS (TACK COAT)	HOT-MIX ASPHALT SURFACE COURSE, IL-9.5, MIX "C", N50	INCIDENTAL HOT-MIX ASPHALT SURFACING	AGGREGATE SHOULDERS, TYPE B
	SQ YD	POUND	TON	TON	TON
FAS 363 / CH 7					
STA. 23+50.00 TO STA. 25+31.47	40	200	56		82
BRIDGE DECK & BACKFILL - SN 102-3130			64		
STA. 26+76.53 TO STA. 28+59.84	41	202	129		105
BRIDGE DECK & BACKFILL - SN 102-3131			68		
STA. 30+20.16 TO STA. 32+75.00	57	280	120		101
PE ENTR. - STA 32+40 LT		24		6	
TOTAL	138	705	437	6	288

PIPE CULVERTS, CLASS D, TYPE 1			
STATION	SIDE	15"	18"
		FOOT	FOOT
24+56	LT	40	
31+27	LT		40
32+40	LT		40
TOTAL		40	80

PIPE CULVERT REMOVAL		
STATION	SIDE	FOOT
24+53	LT	32
31+26	LT	32
32+43	LT	32
TOTAL		96

TRAFFIC BARRIER TERMINAL, TYPE 1 (SPECIAL) TANGENT		
STATION TO STATION	SIDE	EACH
24+54.75 TO 25+04.75	RT	1
24+67.25 TO 25+17.25	LT	1
26+90.75 TO 27+40.75	RT	1
26+90.75 TO 27+40.75	LT	1
27+95.79 TO 28+45.79	RT	1
27+95.79 TO 28+45.79	LT	1
30+34.21 TO 30+84.21	RT	1
30+46.71 TO 30+96.71	LT	1
TOTAL		8

TRAFFIC BARRIER TERMINAL, TYPE 5A		
STATION TO STATION	SIDE	EACH
25+17.25 TO 25+30.50	RT	1
25+17.25 TO 25+30.50	LT	1
26+77.50 TO 26+90.75	RT	1
26+77.50 TO 26+90.75	LT	1
28+45.79 TO 28+59.04	RT	1
28+45.79 TO 28+59.04	LT	1
30+20.96 TO 30+34.21	RT	1
30+20.96 TO 30+34.21	LT	1
TOTAL		8

STEEL PLATE BEAM GUARDRAIL, TYPE A, 6 FOOT POSTS		
STATION TO STATION	SIDE	FOOT
25+04.75 TO 25+17.25	RT	12.5
30+34.21 TO 30+46.71	LT	12.5
TOTAL		25

GUARDRAIL REMOVAL		
STATION TO STATION	SIDE	FOOT
24+79 TO 25+31	RT	52
24+79 TO 25+31	LT	52
26+75 TO 27+27	RT	52
26+75 TO 27+27	LT	52
28+07 TO 28+59	RT	52
28+07 TO 28+59	LT	52
30+19 TO 30+71	RT	52
30+19 TO 30+71	LT	52
TOTAL		416

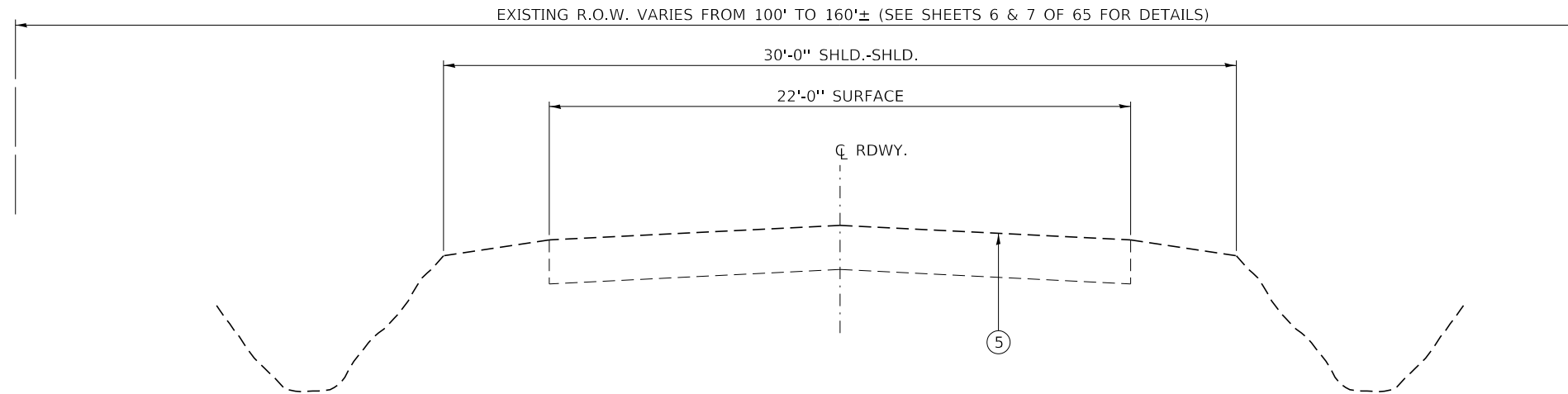
PAVED DITCH REMOVAL		
STATION TO STATION	SIDE	FOOT
24+68 TO 25+58	LT	90
TOTAL		90

PAVEMENT MARKING SCHEDULE				
LOCATION	TEMPORARY PAVEMENT MARKING - LINE 4" TY III TAPE	SHORT TERM PAVEMENT MARKING REMOVAL	PAINT PAVEMENT MARKING - LINE 4"	
			EDGE LINE WHITE	NO PASSING DOUBLE YELLOW
	FOOT	SQ FT	FOOT	FOOT
FAS 363 / CH 7				
LT. STA. 23+50.00 TO LT. STA. 32+75.00	925	26	925	
CL. STA. 23+50.00 TO CL. STA. 32+75.00	1,850	52		1,850
RT. STA. 23+50.00 TO RT. STA. 32+75.00	925	26	925	
SUBTOTAL	3,700	104	1,850	1,850
TOTAL	3,700	104	3,700	

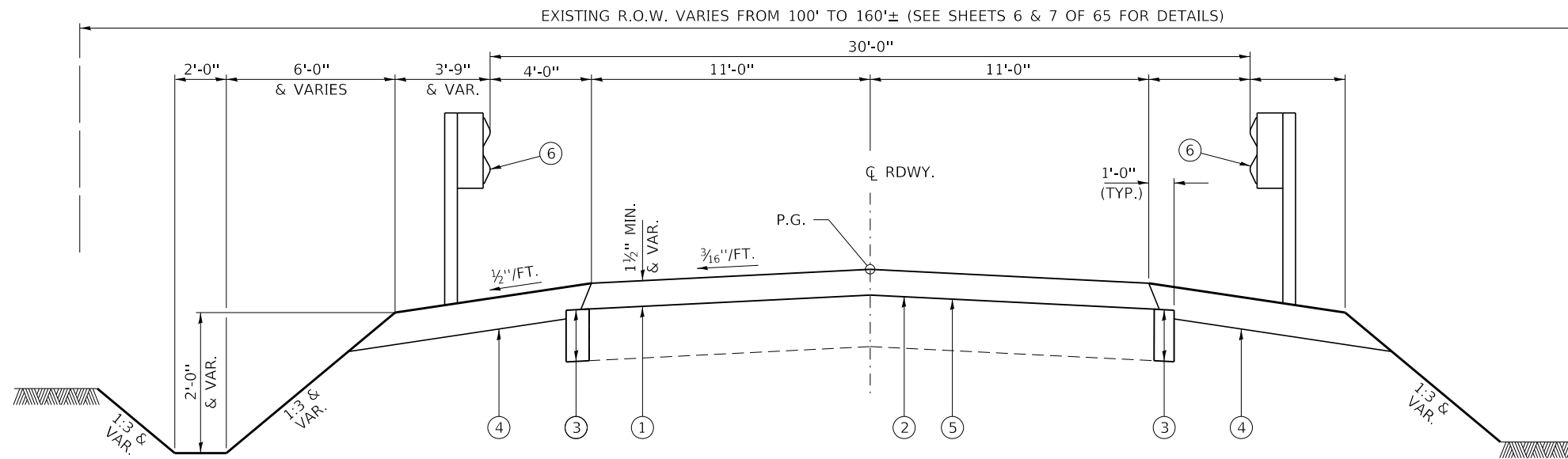
AGGREGATE DITCH				
STATION TO STATION	SIDE	WIDTH	TON	FILTER FABRIC SQ YD
24+75.00 TO 25+78.00	LT	VAR.	100	140
TOTAL			100	140

AGGREGATE SURFACE COURSE, TYPE B				
STATION	THICKNESS	WIDTH	LENGTH	TON
ENTR. - 24+56 LT	6"	16' & VAR.	34.00'	22
ENTR. - 27+51 RT	6"	16' & VAR.	49.00'	32
ENTR. - 27+57 LT	6"	20' & VAR.	29.00'	24
ENTR. - 31+27 LT	6"	20' & VAR.	49.00'	39
ENTR. - 31+27 RT	6"	16' & VAR.	41.00'	27
ENTR. - 32+40 LT	6"	15' & VAR.	20.00'	10
TOTAL				154

HOT-MIX ASPHALT SURFACE REMOVAL, VARIABLE DEPTH			
STATION TO STATION	WIDTH	LENGTH	SQ YD
23+50.00 TO 24+75.00	22.00' & VAR.	125.00'	277
32+45.00 TO 32+75.00	22.00' AVG.	30.00'	73
TOTAL			350



EXISTING TYPICAL CROSS SECTION
STA. 23+50 TO 32+75



PROPOSED TYPICAL CROSS SECTION

STA. 23+50.00 TO STA. 25+31.47
STA. 26+76.53 TO STA. 28+59.84
STA. 30+20.16 TO STA. 32+75.00

EXCEPT TRANSITIONS AND SUPERELEVATION TRANSITION

BRIDGE OMISSIONS
STA. 25+31.47 TO STA. 26+76.53
STA. 28+59.84 TO STA. 30+20.16

SUGGESTED CUT SECTION
CONSTRUCT AS SHOWN IN
STATION CROSS SECTIONS

SUGGESTED FILL SECTION
CONSTRUCT AS SHOWN IN
STATION CROSS SECTIONS

LEGEND

- ① HMA SURFACE COURSE, IL-9.5, MIX C, N50 (1½" MIN. & VAR.)
- ② BITUMINOUS MATERIALS (TACK COAT)
- ③ BASE COURSE WIDENING (VARIABLE DEPTH) AS DIRECTED BY ENGINEER
- ④ AGGREGATE SHOULDERS, TYPE B (6")
- ⑤ EXISTING HMA SURFACE
- ⑥ TBT TY 5A, SPBGR TY A 6' POSTS, OR TBT TY 1 SPECIAL TANGENT

HOT-MIX ASPHALT MIXTURE REQUIREMENTS	
LOCATIONS(S)	FAS 363 / CH 7
MIXTURE USE(S):	HOT-MIX ASPHALT SURFACE COURSE
PG:	PG 58-28
DESIGN AIR VOIDS:	4% @ 50 Gyr.
MIXTURE COMPOSITION:	IL 9.5
(MIXTURE GRADATION)	
FRICTION AGGREGATE:	MIXTURE C
MIXTURE WEIGHT:	112 LBS \ SY \ INCH THICKNESS
QUALITY MANAGEMENT PROGRAM	QC/QA
MATERIAL TRANSFER DEVICE	NOT REQUIRED

HIGHWAY STANDARDS

- 000001-08 STANDARD SYMBOLS, ABBREVIATIONS, AND PATTERNS
- 280001-07 TEMPORARY EROSION CONTROL SYSTEMS
- 515001-04 NAME PLATE FOR BRIDGES
- 630001-12 STEEL PLATE BEAM GUARDRAIL
- 630301-09 SHOULDER WIDENING FOR TYPE 1, (SPECIAL) GUARDRAIL TERMINALS
- 701901-08 TRAFFIC CONTROL DEVICES
- 725001-01 OBJECT AND TERMINAL MARKERS
- 780001-05 TYPICAL PAVEMENT MARKINGS
- BLR 21-9 TYPICAL APPLICATION OF TRAFFIC CONTROL DEVICES FOR CONSTRUCTION ON RURAL LOCAL HIGHWAYS
- BLR 27-1 TRAFFIC BARRIER TERMINAL TYPE 5A

COMMITMENTS

TREES THREE INCHES OR GREATER IN DIAMETER SHALL NOT BE CLEARED BETWEEN APRIL 1 AND SEPTEMBER 30. THE BRIDGE BAT ASSESSMENT EXPIRES 11/16/2023. A VALID ASSESSMENT IS REQUIRED PRIOR TO PERFORMING ANY WORK BELOW THE EXISTING BRIDGE SURFACE.

ENVIRONMENTAL REVIEWS

PRIOR TO THE USE OF ANY PROPOSED BORROW AREAS, USE AREAS (TEMPORARY ACCESS ROADS, DETOURS, RUN-AROUNDS, ETC.) AND/OR WASTE AREAS, THE CONTRACTOR SHALL FILE THE REQUIRED ENVIRONMENTAL RESOURCE REQUEST SURVEYS ACCORDING TO SECTION 107.22 OF THE STANDARD SPECIFICATIONS. THESE SURVEYS ARE REQUIRED IN ORDER FOR THE DEPARTMENT TO CONDUCT CULTURAL AND BIOLOGICAL RESOURCE SURVEYS FOR THE PROPOSED SITE.

THE REQUIRED ENVIRONMENTAL RESOURCE DOCUMENTATION SHALL INCLUDE THE FOLLOWING:

- BDE FORM 2289 (BORROW SITE REVIEW)
- BDE FORM 2290 (WASTE/USE AREA REVIEW)
- A LOCATION MAP SHOWING THE SIZE LIMITS AND LOCATION OF THE USE AREA
- COLOR PHOTOGRAPHS DEPICTING THE USE AREA
- BORROW AREA ENTRY AGREEMENT FORM - D4 PI0101

PRIOR TO ANY WASTE MATERIALS BEING REMOVED FROM THE CONSTRUCTION SITE THE REQUIRED ENVIRONMENTAL RESOURCE SURVEYS SHALL BE OBTAINED AND FILED BY THE CONTRACTOR. EXCESS WASTE PRODUCTS REMOVED FROM THE CONSTRUCTION SITE SHALL BE DISPOSED OF AS REQUIRED IN SECTION 202.03 OF THE STANDARD SPECIFICATIONS.

ANY PROTRUDING METAL BARS SHALL BE REMOVED PRIOR TO THE DISPOSAL OF BROKEN CONCRETE AT APPROVED DISPOSAL SITES.

PLEASE NOTE THAT A MINIMUM OF FOUR WEEKS SHALL BE ALLOWED FOR THE DISTRICT TO OBTAIN THE REQUIRED WASTE SITE ENVIRONMENTAL CLEARANCES AND SIX WEEKS FOR THE REQUIRED BORROW SITE ENVIRONMENTAL CLEARANCES.

GENERAL NOTES

- 1) ALL CONSTRUCTION SHALL BE DONE IN ACCORDANCE WITH THE STATE OF ILLINOIS "STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION, ADOPTED JANUARY 1, 2022", (HERE IN AFTER REFERRED TO AS THE STANDARD SPECIFICATIONS; THE LATEST EDITION OF THE "ILLINOIS MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES FOR STREETS AND HIGHWAYS"; THE DETAILS IN THE PLANS AND THE "SPECIAL PROVISIONS" INCLUDED IN THE DOCUMENTS.
- 2) ALL CLEARING, GRUBBING, FENCE REMOVAL, PAVEMENT REMOVAL, AND REMOVAL OF EXISTING DRAINAGE STRUCTURES SHALL BE INCLUDED IN THE COST OF EARTH EXCAVATION. ALL AGGREGATE AND BITUMINOUS PAVEMENT SHALL BE REMOVED AND PROPERLY DISPOSED OF BY THE CONTRACTOR IN A METHOD APPROVED BY THE ENGINEER. REMOVAL AND DISPOSAL OF PAVEMENT SHALL BE INCLUDED IN THE COST OF EARTH EXCAVATION AND NO ADDITIONAL COMPENSATION WILL BE ALLOWED.
- 3) WHERE SECTION OR SUBSECTION MONUMENTS ARE ENCOUNTERED, THE ENGINEER SHALL BE NOTIFIED BEFORE SUCH MONUMENTS ARE REMOVED. THE CONTRACTOR SHALL PROTECT AND CAREFULLY PRESERVE ALL PROPERTY MARKS AND MONUMENTS UNTIL THE OWNER, AN AUTHORIZED SURVEYOR OR AGENT HAS WITNESSED OR OTHERWISE REFERENCED THEIR LOCATION.
- 4) ANY REFERENCE TO STANDARDS THROUGHOUT THE PLANS OR SPECIAL PROVISIONS SHALL BE INTERPRETED TO BE THE LATEST STANDARD OF THE DEPARTMENT.
- 5) THE LOCATION ON THE PLANS OF EXISTING DRAINAGE STRUCTURES, TELEPHONE LINES, ELECTRIC LINES, WATER SERVICE LINES, GAS MAINS, AND OTHER UTILITY FACILITIES AS SHOWN ON THE PLANS ARE BASED ON FIELD INVESTIGATIONS AND THE BEST INFORMATION AVAILABLE, BUT THE LOCATIONS ARE NOT GUARANTEED. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO ASCERTAIN THEIR EXACT LOCATION FROM THE INDIVIDUAL UTILITY COMPANIES AND BY FIELD INSPECTION.

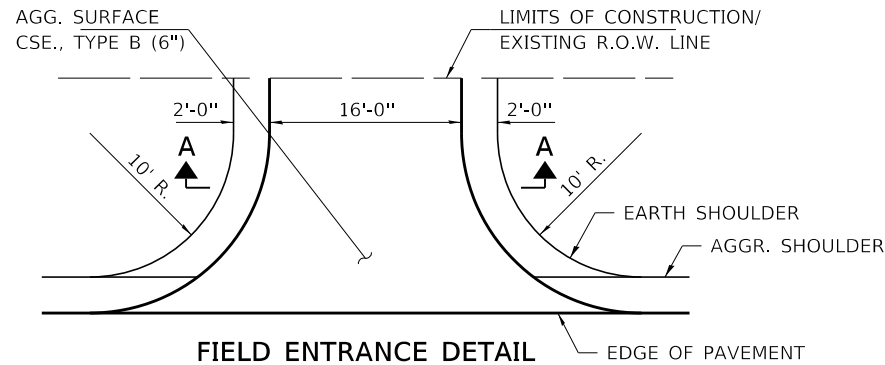
6) THE FOLLOWING RATES OF APPLICATION HAVE BEEN USED IN CALCULATING PLAN QUANTITIES

AGGREGATE SURFACE COURSE	2.05 TON/CU YD
AGGREGATE SHOULDERS	2.00 TON/CU YD
AGGREGATE DITCH	1.50 TON/CU YD
CONTROLLED LOW-STRENGTH MATERIAL	1.50 TON/CU YD
STONE RIPRAP	1.75 TON/CU YD

BITUMINOUS MATERIALS APPLICATION RATES

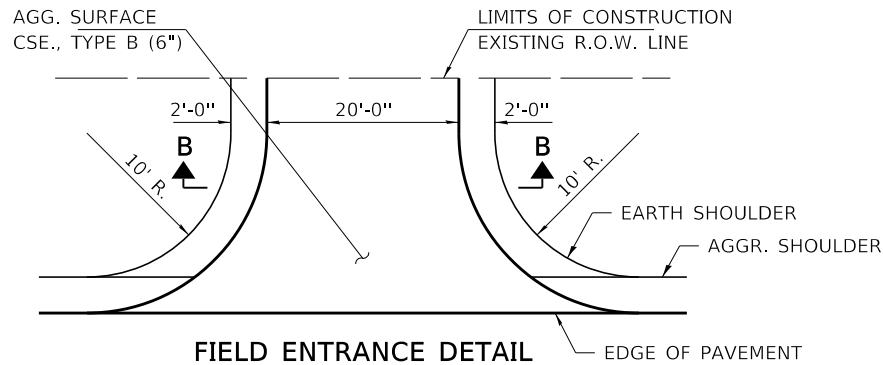
SURFACE TYPE	RESIDUAL RATE
AGGREGATE BASE (PRIME COAT)	0.250 LB/SQ FT
MILLED HMA OR PCC (TACK COAT)	0.050 LB/SQ FT
EXISTING PAVEMENT (TACK COAT)	0.050 LB/SQ FT
TACK COAT (BETWEEN LIFTS)	0.025 LB/SQ FT

- 7) THE FINAL SURFACE OF ALL EMBANKMENT AREAS SHALL BE SEEDED. THE TOP 4 INCHES OF THE SEEDED AREAS SHALL BE TOPSOIL SUBJECT TO THE APPROVAL OF THE ENGINEER. THE COST OF SHAPING THE SLOPES AND PROVIDING TOP SOIL WILL NOT BE PAID FOR SEPARATELY BUT SHALL BE INCLUDED IN THE COST OF EARTH EXCAVATION.
- 8) THE AREA TO BE SEEDED SHALL CONSIST OF ALL DISTURBED EARTH SURFACES WITHIN THE RIGHT OF WAY OR AS DIRECTED BY THE ENGINEER.
SEEDING, CLASS 2 (SPECIAL) = 0.6 ACRES
- 9) ALL WASTE MATERIAL FROM EXCAVATIONS SHALL BE DISPOSED OF BY THE CONTRACTOR. NO ADDITIONAL COMPENSATION WILL BE ALLOWED.
- 10) ALL ELEVATIONS SHOWN REFER TO U.S.G.S DATUM TO MEAN SEA LEVEL UNLESS OTHERWISE NOTED.
- 11) CONTINUOUS PAVING OPERATIONS ON THE MAIN ROADWAY SHALL BE MAINTAINED AT ALL TIMES DURING THE CONSTRUCTION OF THE HOT-MIX ASPHALT SURFACE. NO INTERRUPTIONS FOR SIDE ROADS, ENTRANCES, TURN LANES ECT. WILL BE ALLOWED.
- 12) THE CONTRACTOR SHALL CONSULT WITH THE ENGINEER IN REGARD TO THE EXACT LENGTH OF THE PIPE CULVERTS REQUIRED PRIOR TO ORDERING THIS ITEM.



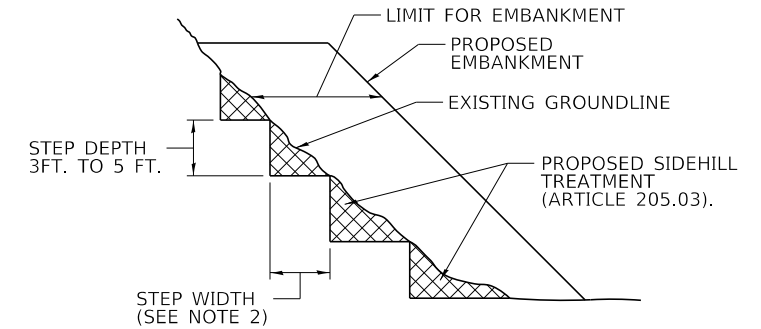
FIELD ENTRANCE DETAIL

STA. 24+56 LT.
STA. 27+51 RT.
STA. 31+27 RT.



FIELD ENTRANCE DETAIL

STA. 27+57 LT.
STA. 31+27 LT.



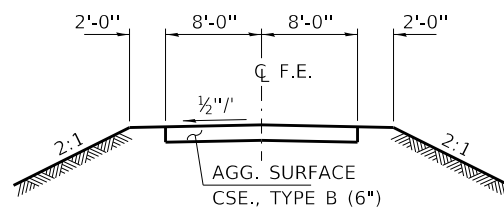
GENERAL NOTES

1. SLOPE STEPS WILL BE REQUIRED FOR ALL 12(300) MINIMUM THICKNESS "SLIVER FILLS" AND ON ALL FILLS WITH A HEIGHT OF 10 FEET OR GREATER.
2. THE STEP WIDTH SHALL BE TWICE THE STEP DEPTH BUT NOT LESS THAN 6 FEET.
3. REFER TO ARTICLE 205.03 FOR EMBANKMENT TO BE CONSTRUCTED ON HILLSIDE OR SLOPES, OR IF EXISTING EMBANKMENTS ARE TO BE WIDENED.

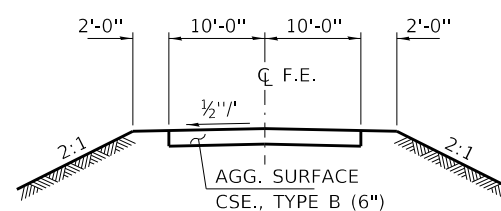
STANDARD EMBANKMENT (IN ACCORDANCE WITH 205 OF THE STANDARD SPECIFICATION).

SLOPE STEPS DETAIL

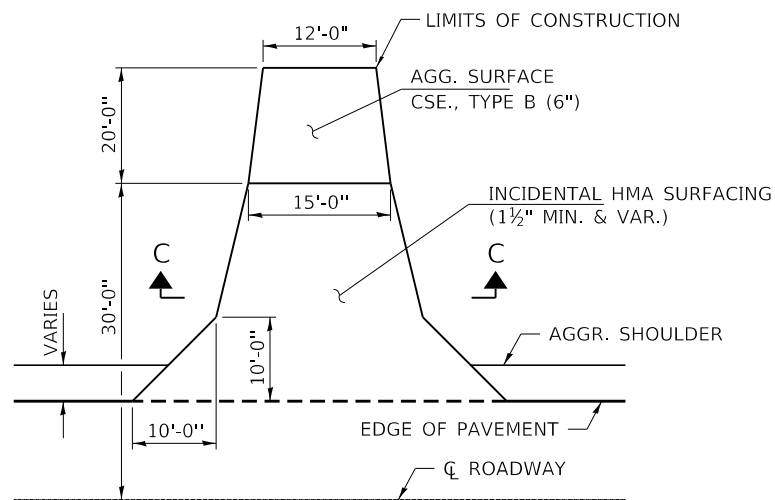
TYPICAL CROSS-SECTION EMBANKMENT CONSTRUCTION ON SIDHILL



SECTION A-A

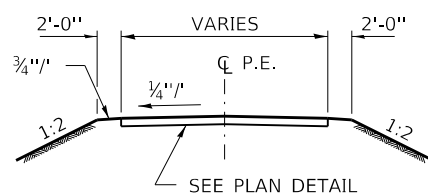


SECTION B-B

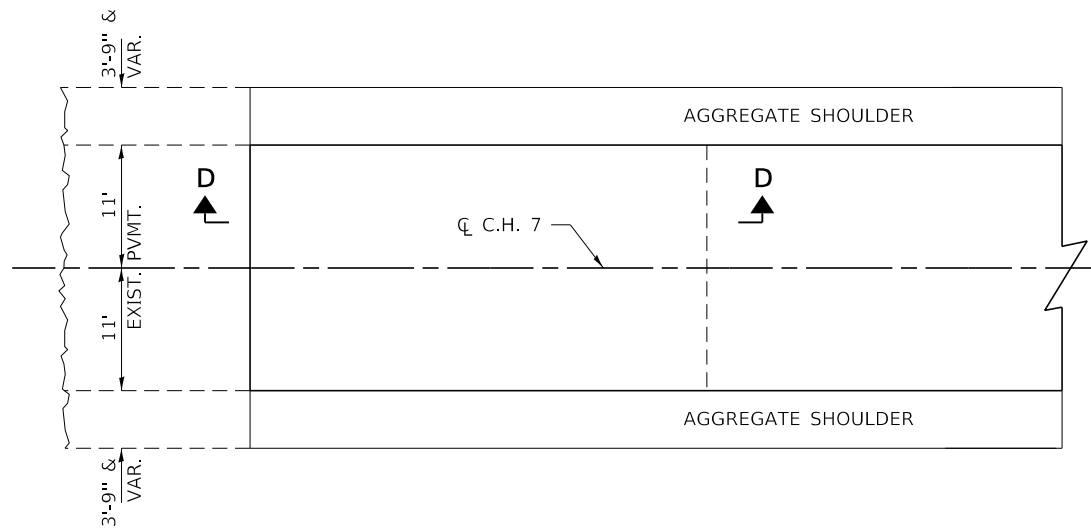


PRIVATE ENTRANCE DETAIL

STA. 32+40 LT.

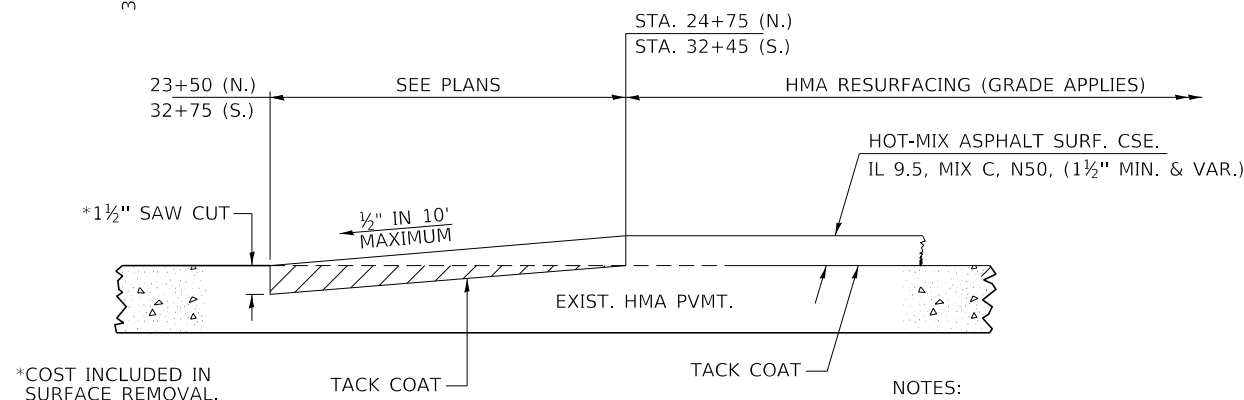


SECTION C-C

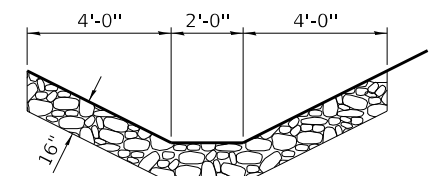


SECTION D-D

PAVEMENT REMOVAL DETAIL



NOTES:
HOT-MIX ASPHALT SURFACE REMOVAL LIMITS AND DEPTH DETERMINED BY PROVIDING 1 1/2" BELOW PROPOSED PROFILE GRADE.



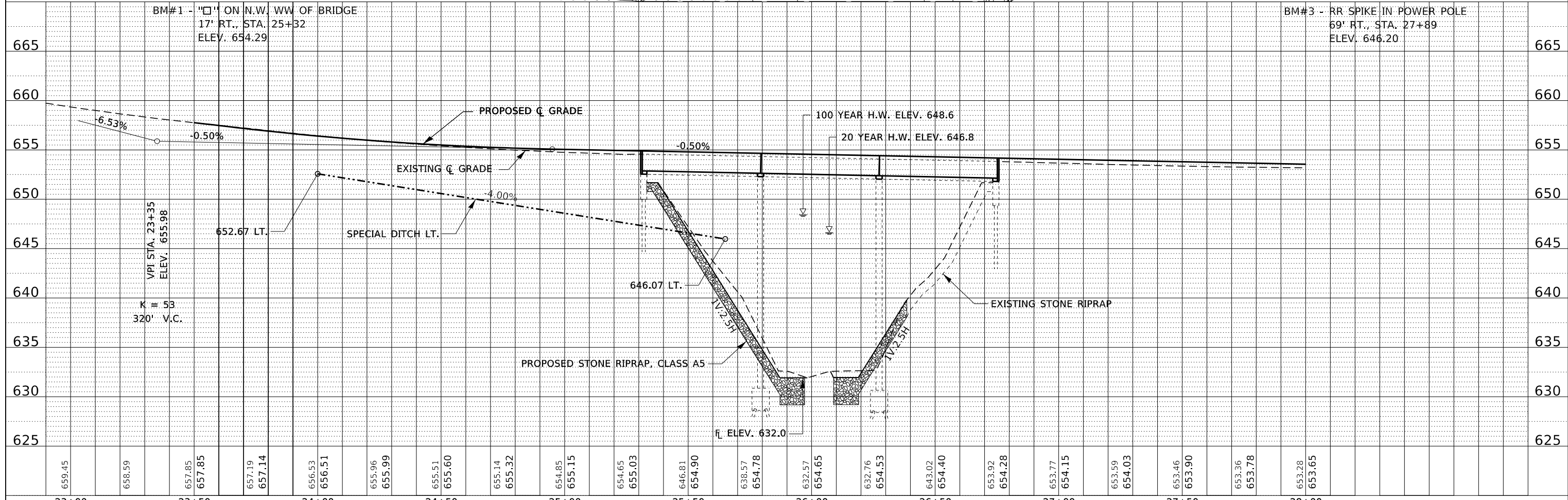
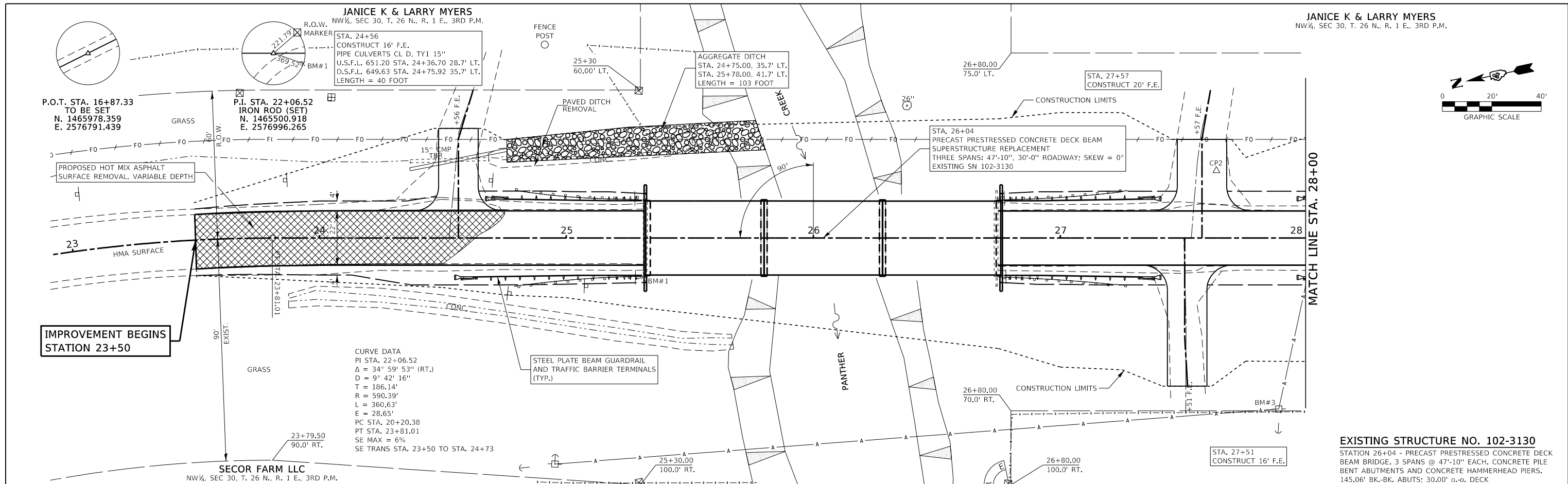
AGGREGATE DITCH (RR4)

STA. 24+75 TO STA. 25+60 LT

FILE NAME = 190013-shit-4ypsec@ons.dgn	USER NAME = smierzwa	DESIGNED - S.W.M.	REVISED -	STATE OF ILLINOIS WOODFORD COUNTY HIGHWAY DEPARTMENT	ENTRANCE DETAILS		F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
HAMPTON, LENZINI AND RENWICK, INC. 3088 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 ILLINOIS PROFESSIONAL DESIGN FIRM LS / PE / SE CORP. 184.000959	PLOT SCALE = \$SCALES	DRAWN - R.D.H.	REVISED -		SCALE: NONE	SHEET NO. 1 OF 1 SHEETS	STA. 23+50.00 TO STA. 32+75.00	363	18-00166-00-BR	WOODFORD	65	6
PLOT DATE = 2/28/2023	DATE = 01/12/2023	CHECKED - S.W.M.	REVISED -					CONTRACT NO. 89779				
		DATE = 01/12/2023	REVISED -					ILLINOIS FED. AID PROJECT ZH0Y(896)				

DATE	
BY	
PLAN	
REVIEWED	
PLOTTED	
ALIGNMENT CHECKED	
GRADES CHECKED	
NOTE BOOK NO.	
FILE NAME	

DATE	
BY	
PROFILE	
REVIEWED	
PLOTTED	
GRADES CHECKED	
STRUCTURE NOTATING CHECKED	
NOTE BOOK NO.	
FILE NAME	

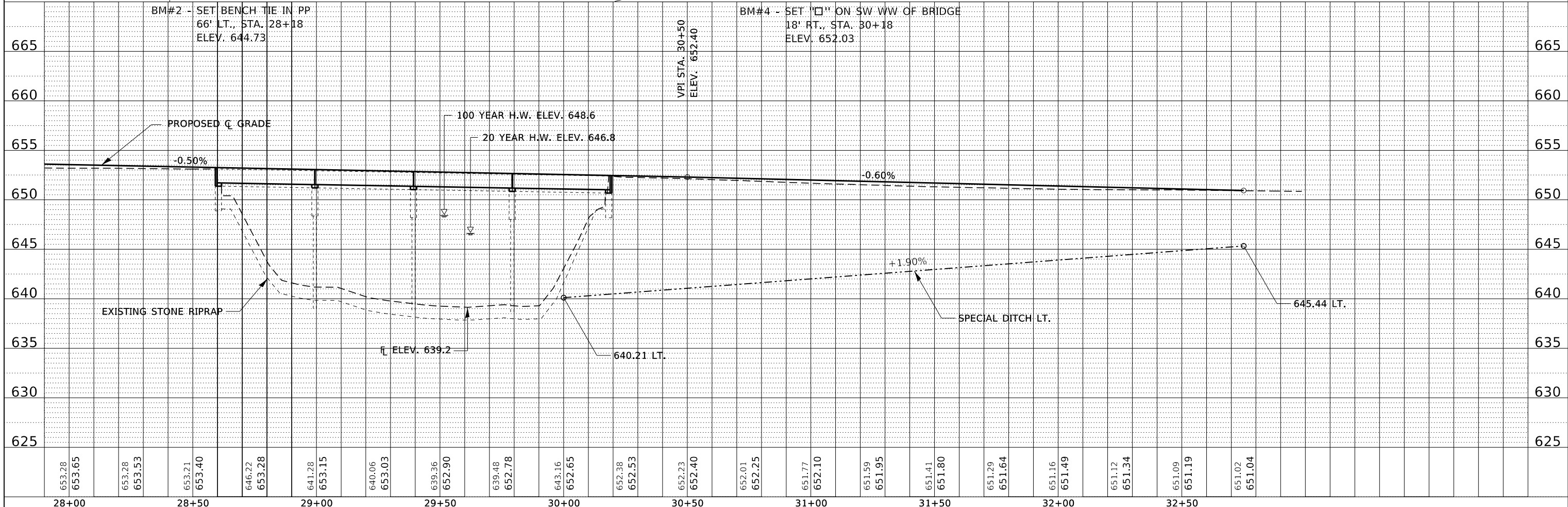
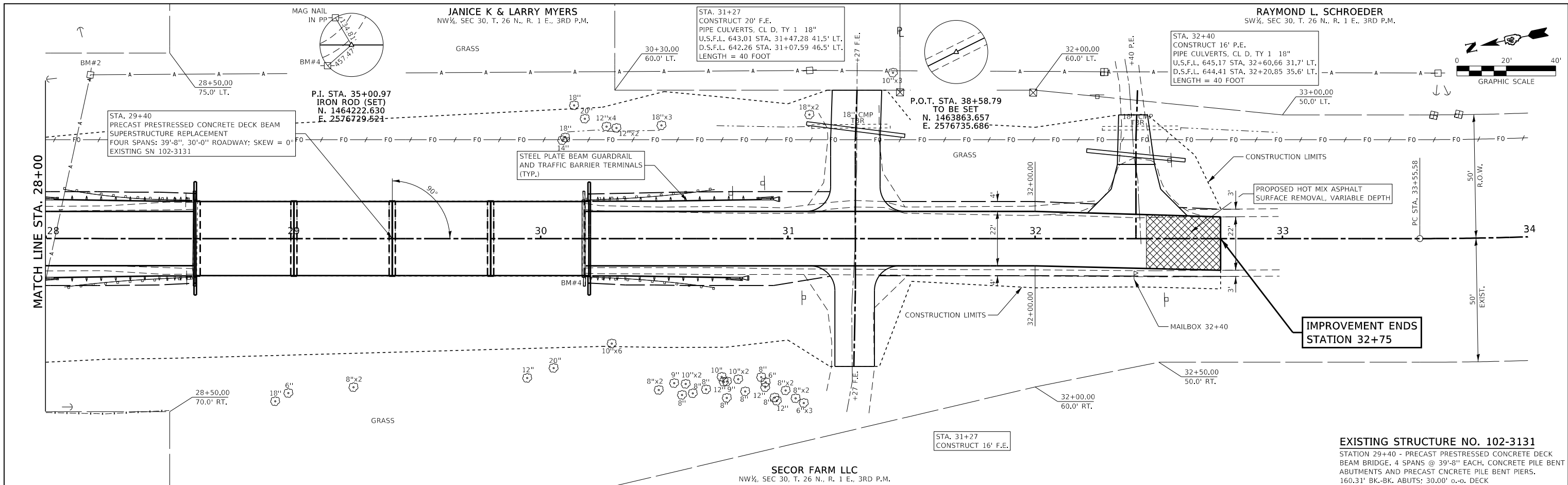


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HAMPTON, LENZINI AND RENWICK, INC.		DRAWN - T.W.K.	REVISED -			383	18-00166-00-BR	WOODFORD	65	7	
3085 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 ILLINOIS PROFESSIONAL DESIGN FIRM L.S./P.E./S.E. CORP. 184.000959		CHECKED - S.W.M.	REVISED -			CONTRACT NO. 89779					
		DATE - 01/12/2023	REVISED -			ILLINOIS FED. AID PROJECT ZH0Y(896)					

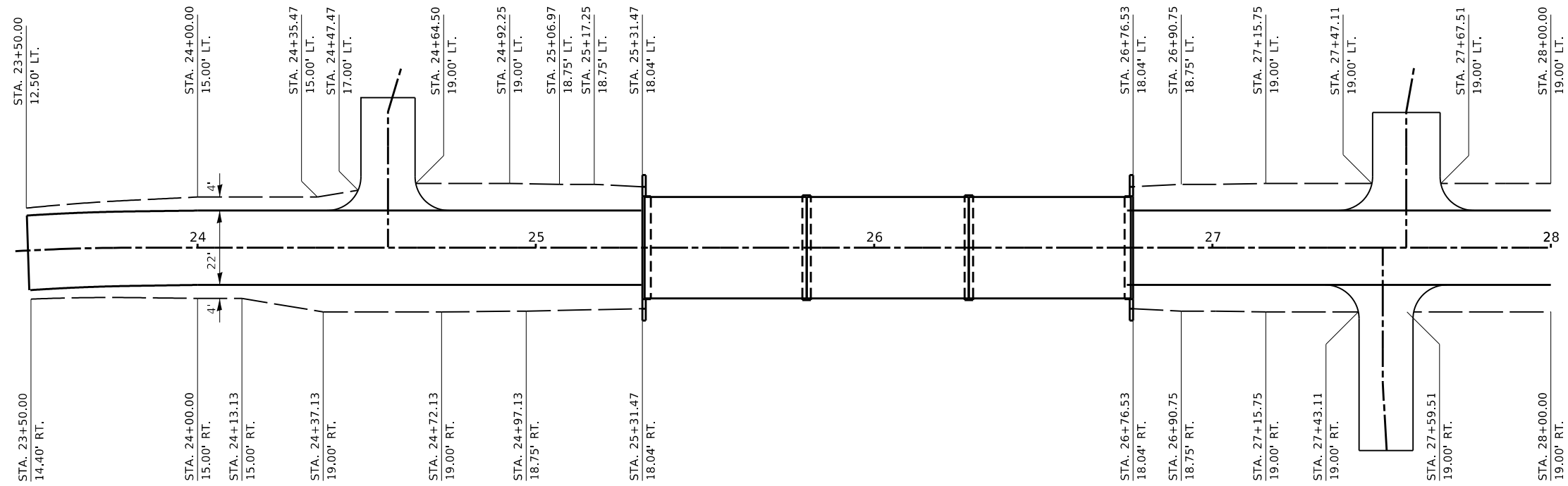
SCALE: 5V:20H SHEET NO. 1 OF 2 SHEETS STA. 23+00.00 TO STA. 28+00.00

DATE	
BY	
REVIEWED	
PLANNED	
NOTED	
NO.	

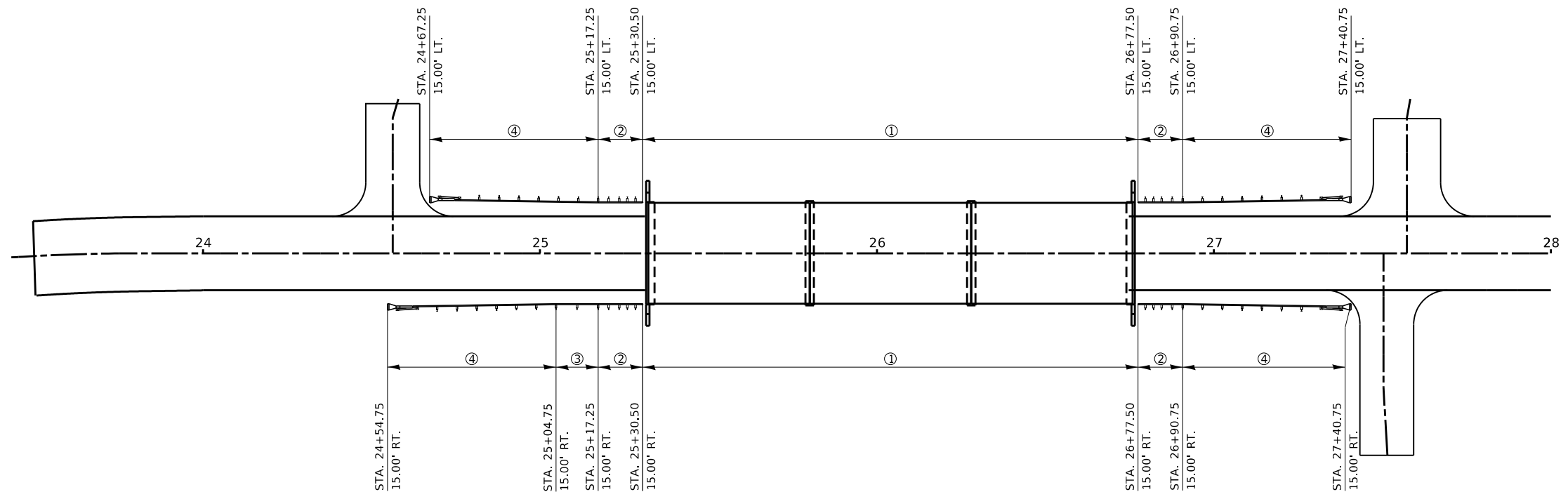
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BY	
REVIEWED	
PLANNED	
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NO.	



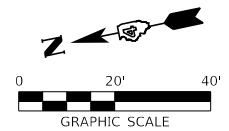
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HAMPPTON, LENZINI AND RENWICK, INC.		DRAWN - T.W.K.	REVISED -		363	18-00166-00-BR	WOODFORD	65	8
3088 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62710		CHECKED - S.W.M.	REVISED -		CONTRACT NO. 89779				
ILLINOIS PROFESSIONAL DESIGN FIRM L.S. / P.E. / S.E. CORP. 184.000959		DATE - 01/12/2023	REVISED -		ILLINOIS FED. AID PROJECT ZH0Y(896)				



SHOULDER LAYOUT



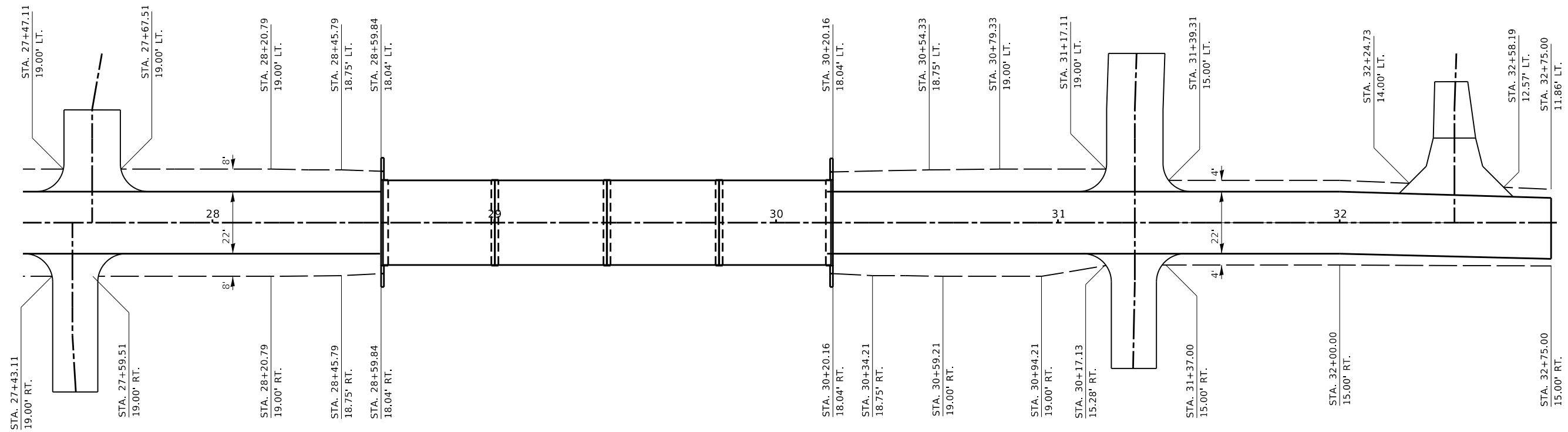
GUARDRAIL LAYOUT



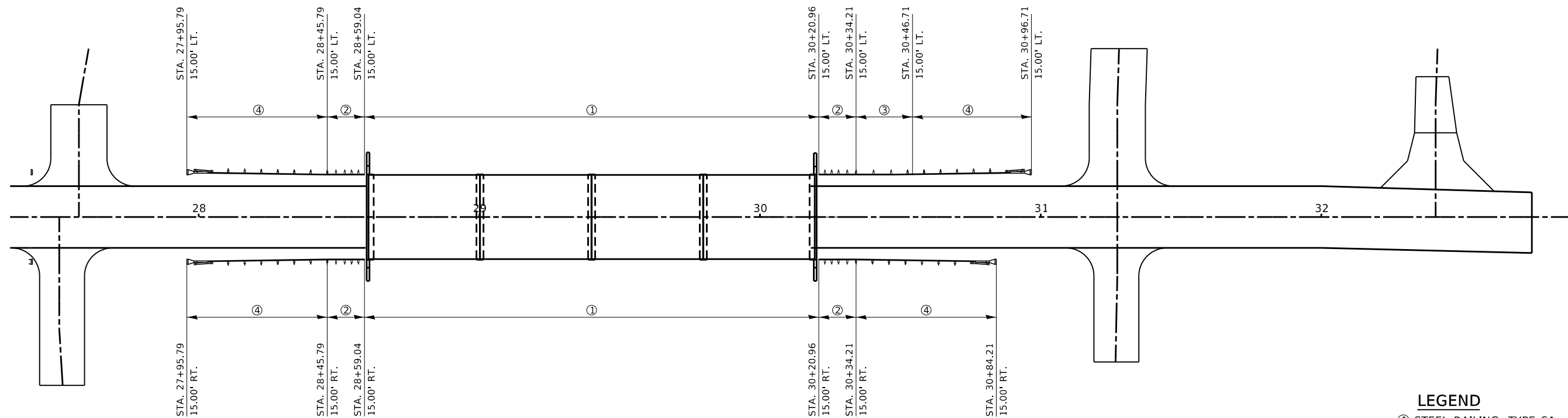
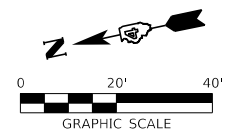
LEGEND

- ① STEEL RAILING, TYPE S1
- ② TBT TY 5A
- ③ SPBGR, TY A, 6 FOOT POSTS
- ④ TBT TY 1, SPECIAL TANGENT

FILE NAME = 190013-shi-shdgrd.rvt.dgn	USER NAME = smierzwa	DESIGNED - J.W.F.	REVISED -	STATE OF ILLINOIS WOODFORD COUNTY HIGHWAY DEPARTMENT	SHOULDER AND GUARDRAIL DETAIL STRUCTURE NO. 102-3130	F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
HAMPTON, LENZINI AND RENWICK, INC. 3035 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 ILLINOIS PROFESSIONAL DESIGN FIRM L.S. / P.E. / S.E. CORP. 184.000959	DRAWN - R.D.H.	REVISED -	363			18-00166-00-BR	WOODFORD	65	9	
PLOT SCALE = \$SCALE\$	CHECKED - S.W.M.	REVISED -	CONTRACT NO. 89779							
PLOT DATE = 2/28/2023	DATE - 01/12/2023	REVISED -	ILLINOIS FED. AID PROJECT 2H0Y(896)							
				SCALE: 1"=20'	SHEET NO. 1 OF 2 SHEETS	STA. 23+50.00	TO STA. 28+00.00			



SHOULDER LAYOUT



GUARDRAIL LAYOUT

LEGEND

- ① STEEL RAILING, TYPE S1
- ② TBT TY 5A
- ③ SPBGR, TY A, 6 FOOT POSTS
- ④ TBT TY 1, SPECIAL TANGENT

FILE NAME = 190013-shi-shdgrd.rdg
 HAMPTON, LENZINI AND RENWICK, INC.
 3035 STEVENSON DRIVE, SUITE 201
 SPRINGFIELD, ILLINOIS 62703
 ILLINOIS PROFESSIONAL DESIGN FIRM
 LS / PE / SE CORP. 184.000959

USER NAME = smierzwa
 PLOT SCALE = \$SCALE\$
 PLOT DATE = 2/28/2023

DESIGNED - J.W.F.
 DRAWN - R.D.H.
 CHECKED - S.W.M.
 DATE - 01/12/2023

REVISED -
 REVISED -
 REVISED -
 REVISED -

STATE OF ILLINOIS
 WOODFORD COUNTY HIGHWAY DEPARTMENT

SHOULDER AND GUARDRAIL DETAIL
 STRUCTURE NO. 102-3131

SCALE: 1"=20' SHEET NO. 2 OF 2 SHEETS STA. 28+00.00 TO STA. 32+75.00

F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
363	18-00166-00-BR	WOODFORD	65	10
CONTRACT NO. 89779				
ILLINOIS FED. AID PROJECT ZH0Y(866)				

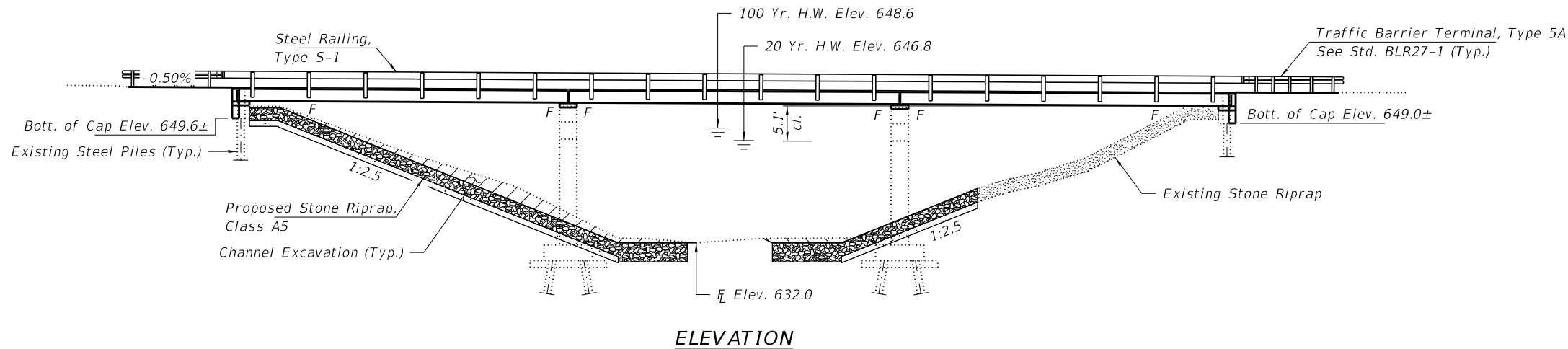
BENCHMARK: Chiseled "□" On N.W. WW of Bridge, 17' Rt., Sta. 25+32 Elev. 654.29

See sheet 2 of 17 for General Notes and Bill of Material.

EXISTING STRUCTURE NO. 102-3130: Three span PPC deck beam superstructure supported on concrete pile bent abutments with concrete caps and wingwalls and hammerhead piers with concrete pile supported footings. 145.06' bk.-bk. abuts., 30.0' o.-o. deck, and no skew.

Structure closed to traffic during construction.

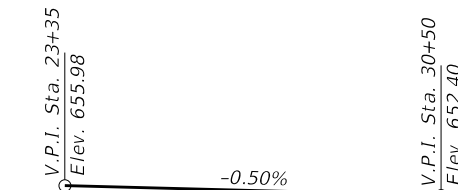
Salvage: Existing abutments and piers to be re-used in place. Salvage and reinstall existing Name Plate.



ELEVATION

INDEX OF STRUCTURE SHEETS

1. General Plan and Elevation
2. General Data
3. Riprap Layout
4. 21"x36" PPC Deck Beam - Spans 1, 2, and 3
5. 21"x36" PPC Deck Beam Details - Spans 1, 2, and 3
6. Superstructure Details
7. Steel Railing, Type S-1
8. North Abutment
9. South Abutment
10. Pier 1
11. Pier 2
12. Concrete Repair - Pier 1
13. Concrete Repair - Pier 2
- 14-17. Existing Plans

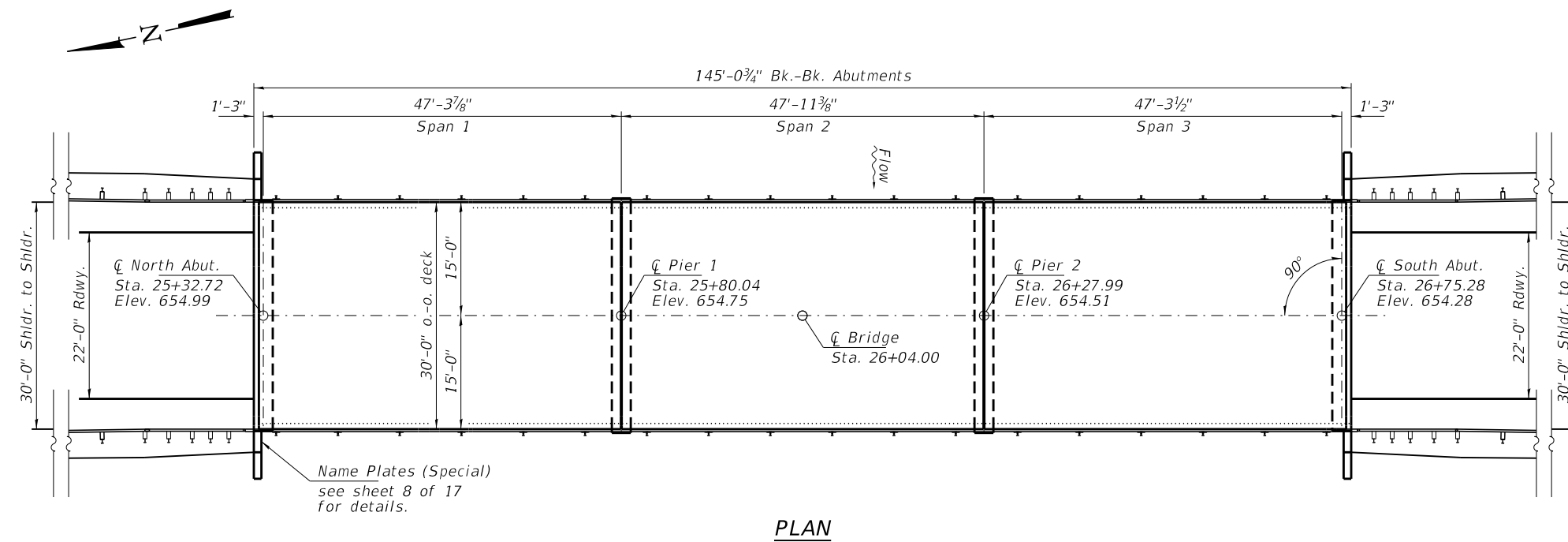


PROFILE GRADE
C.H. 7

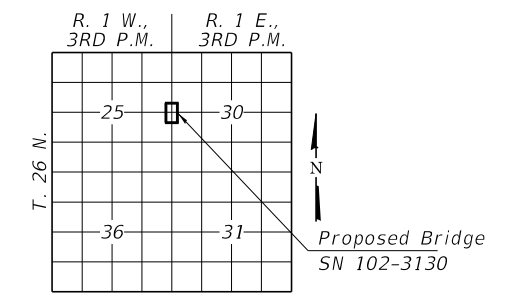
PANTHER CREEK
REBUILT 202_ BY
WOODFORD COUNTY
SEC. 18-00166-00-BR
F.A.S. 363 / C.H. 7
STR. NO. 102-3130
LOADING HL-93

NAME PLATE
See Std. 515001

Existing Name Plate shall be cleaned and relocated next to the proposed Name Plate. Cost included with Name Plates (Special).



PLAN



LOCATION SKETCH

DESIGN STRESSES

FIELD UNITS (NEW CONST.)

$f'_c = 3,500$ psi
 $f_y = 60,000$ psi (Reinf.)

PRECAST PRESTRESSED UNITS

$f'_c = 6,000$ psi
 $f'_{ci} = 5,000$ psi
 $f_{pu} = 270,000$ psi ($1/2"$ low lax. strands)
 $f_{pbt} = 201,960$ psi ($1/2"$ low lax. strands)
 $f_y = 60,000$ psi (Reinf.)

EXISTING CONSTRUCTION

$f_c = 1,400$ psi (Substructure)
 $f_s = 20,000$ psi (Reinf.)

DESIGN SPECIFICATIONS

2020 AASHTO LRFD Bridge Design Specifications, 9th Edition with all interims.

LOADING HL-93

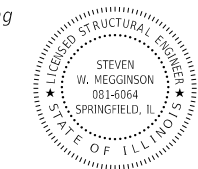
Allow 50#/sq. ft. for future wearing surface.

LOADING HS 20-44

Existing Conditions - Substructure

I certify that to the best of my knowledge, information and belief, this bridge design is structurally adequate for the design loading shown on the plans. The design is an economical one for the style of structure and complies with requirements of the current "AASHTO LRFD Specifications."

Steven W. Megginson 2/27/2023
ILLINOIS STRUCTURAL ENGINEER NO. 081-6064



Expires 11-30-2024

GENERAL PLAN & ELEVATION
BRIDGE OVER PANTHER CREEK
SECTION 18-00166-00-BR
F.A.S. 363 / C.H. 7
WOODFORD COUNTY
STATION 26+04.00
STRUCTURE NO. 102-3130

FILE NAME = 190013-shl-bridge - SN 102-3130.dgn	USER NAME = smlerzwa	DESIGNED - J.R.B	REVISED -	STATE OF ILLINOIS WOODFORD COUNTY HIGHWAY DEPARTMENT	GENERAL PLAN & ELEVATION STRUCTURE NO. 102-3130	F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
HAMPTON, LENZINI AND RENWICK, INC. 3035 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 ILLINOIS PROFESSIONAL DESIGN FIRM LS / PE / SE CORP. 184.000959	PLOT SCALE =	CHECKED - S.T.M.	REVISED -			363	18-00166-00-BR	WOODFORD	65	11
PLOT DATE = 2/28/2023		DRAWN - R.D.H.	REVISED -			CONTRACT NO. 89779				
		CHECKED - S.T.M./S.W.M.	REVISED -			ILLINOIS FED. AID PROJECT NO. ZH0Y(896)				

GENERAL NOTES

Reinforcement bars designated (E) shall be Epoxy Coated.
 Layout of the slope protection system may be varied to suit ground conditions in the field as directed by the Engineer.
 The Contractor shall salvage the existing stone riprap as directed by the Engineer and incorporate into the proposed layout.
 All proposed construction activities shall be in accordance with Nationwide Permit number 14 of the Department of the Army authorized under Section 404 of the Clean Water Act. The IEPA has issued Section 401 Water Quality Certification for this activity. See Special Provisions for conditions.
 Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.
 The PPC Deck Beams shall be applied with a Protective Coat as specified in the Special Provisions. This work shall be included in the cost with PPC Deck Beams.

DESIGN SCOUR ELEVATION TABLE

Event/Limit State	Design Scour Elevations (ft.)				Item
	N. Abut.	Pier 1	Pier 2	S. Abut.	
Q100	649.6	622.6	622.0	649.0	7
Design	649.6	635.0	634.4	649.0	

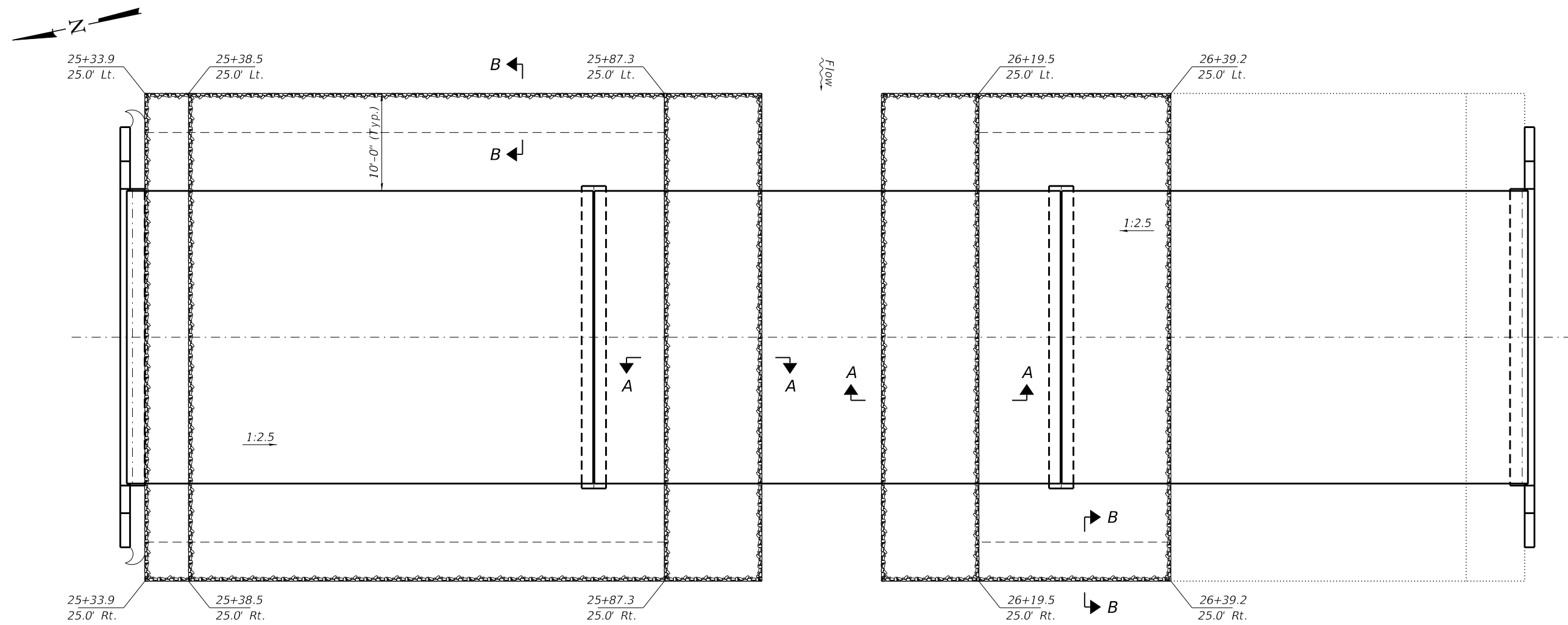
WATERWAY INFORMATION

Flood Event		Discharge (cfs)		Opening Sq. Ft.		Natural H.W.E.	Head - Ft.		Headwater El.	
		Exist.	Prop.	Exist.	Prop.		Exist.	Prop.	Exist.	Prop.
10	Main Channel			870	870					
	Relief Struct.	8450	8450	640	640	645.9	0.8	0.8	646.7	646.7
	Total			1510	1510					
20	Main Channel			950	950					
	Relief Struct.	10530	10530	740	740	646.8	0.9	0.9	647.7	647.7
	Total			1690	1690					
100	Main Channel			1130	1130					
	Relief Struct.	15400	15400	960	960	648.6	1.1	1.1	649.7	649.7
	Total			2090	2090					
200	Main Channel			1200	1200					
	Relief Struct.	17600	17600	1060	1060	649.3	1.0	1.0	650.3	650.3
	Total			2260	2260					
500/ Overtopping	Main Channel			1290	1290					
	Relief Struct.	20500	20500	1180	1180	650.2	1.0	1.0	651.2	651.2
	Total			2470	2470					

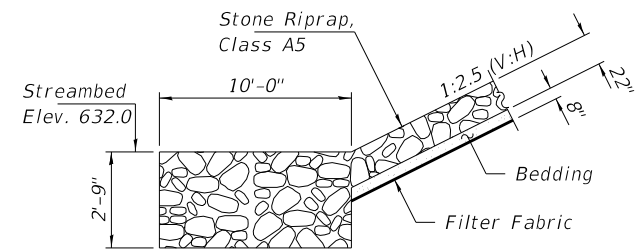
Drainage Area = 178.5 Sq. Mi. Existing Low Grade Elev. 651.0 @ Sta. 33+00
 Proposed Low Grade Elev. 651.0 @ Sta. 33+00
 10 Year Velocity through Existing Bridge = 5.6 fps 10 Year Velocity through Proposed Bridge = 5.6 fps

TOTAL BILL OF MATERIAL

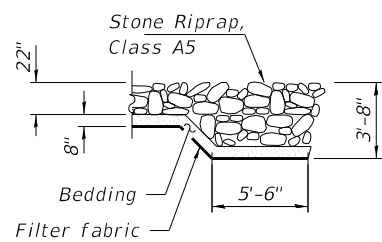
ITEM	UNIT	SUPER	SUB	TOTAL
Channel Excavation	Cu. Yd.			100
Stone Riprap, Class A5	Ton			685
Filter Fabric	Sq. Yd.			435
Hot-Mix Asphalt Surface Cse., IL-9.5, Mix "C", N50	Ton	49		49
Removal of Existing Superstructures	Each			1
Concrete Removal	Cu. Yd.		4.4	4.4
Concrete Structures	Cu. Yd.		13.1	13.1
Precast Prestressed Conc. Deck Beams (21" Depth)	Sq. Ft.	4,305		4,305
Reinforcement Bars, Epoxy Coated	Pound		1,320	1,320
Steel Railing, Type S1	Foot	294		294
Waterproofing Membrane System	Sq. Yd.	484		484
Portland Cement Mortar Fairing Course	Foot	325		325
Controlled Low-Strength Material	Cu. Yd.		24	24
Name Plates (Special)	Each		1	1
Asbestos Bearing Pad Removal	Each		66	66
Structural Repair of Concrete (Depth ≤ 5")	Sq. Ft.		120	120



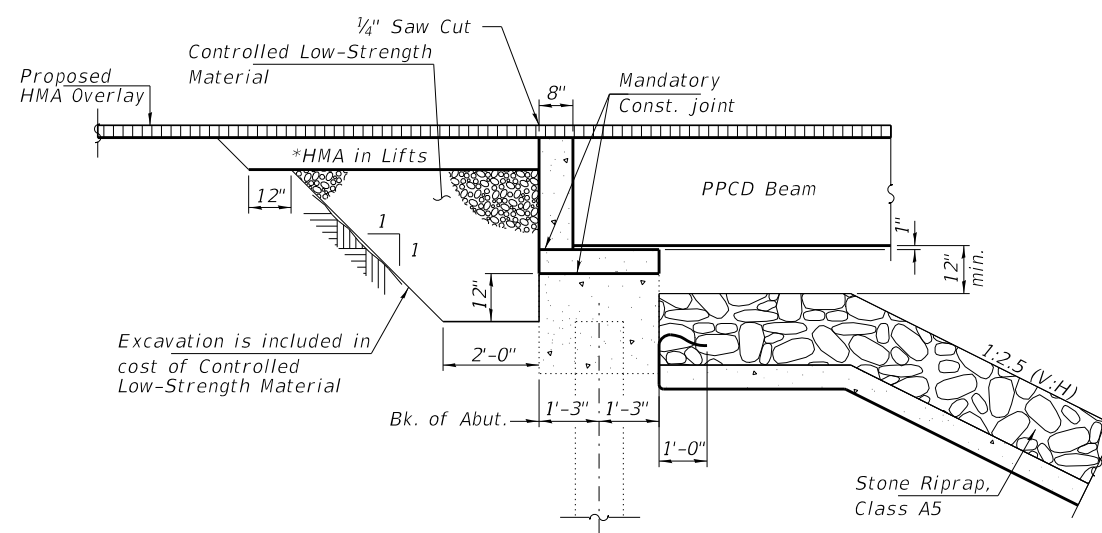
RIPRAP LAYOUT



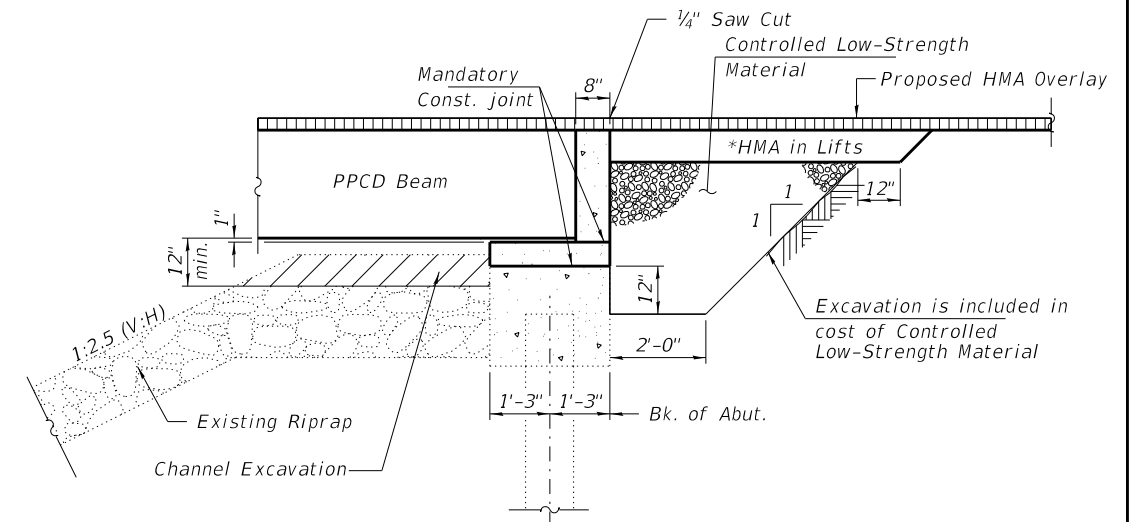
SECTION A-A



SECTION B-B



SECTION THRU NORTH ABUTMENT

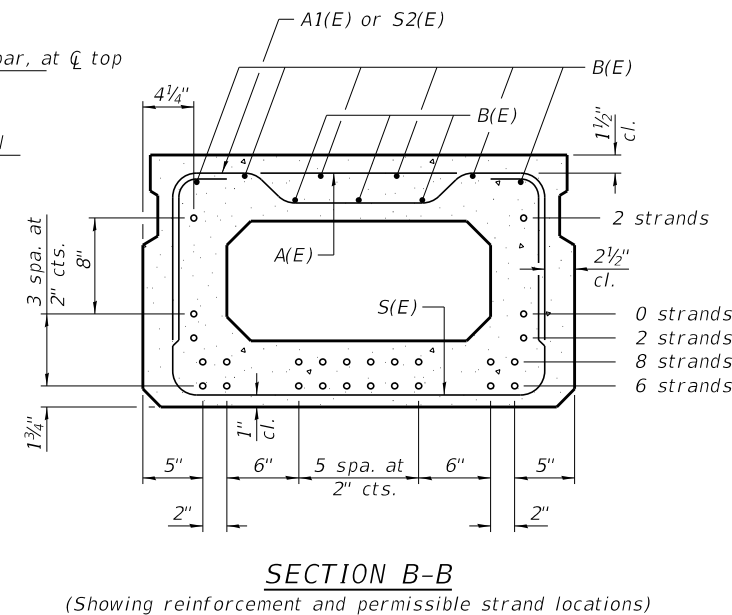
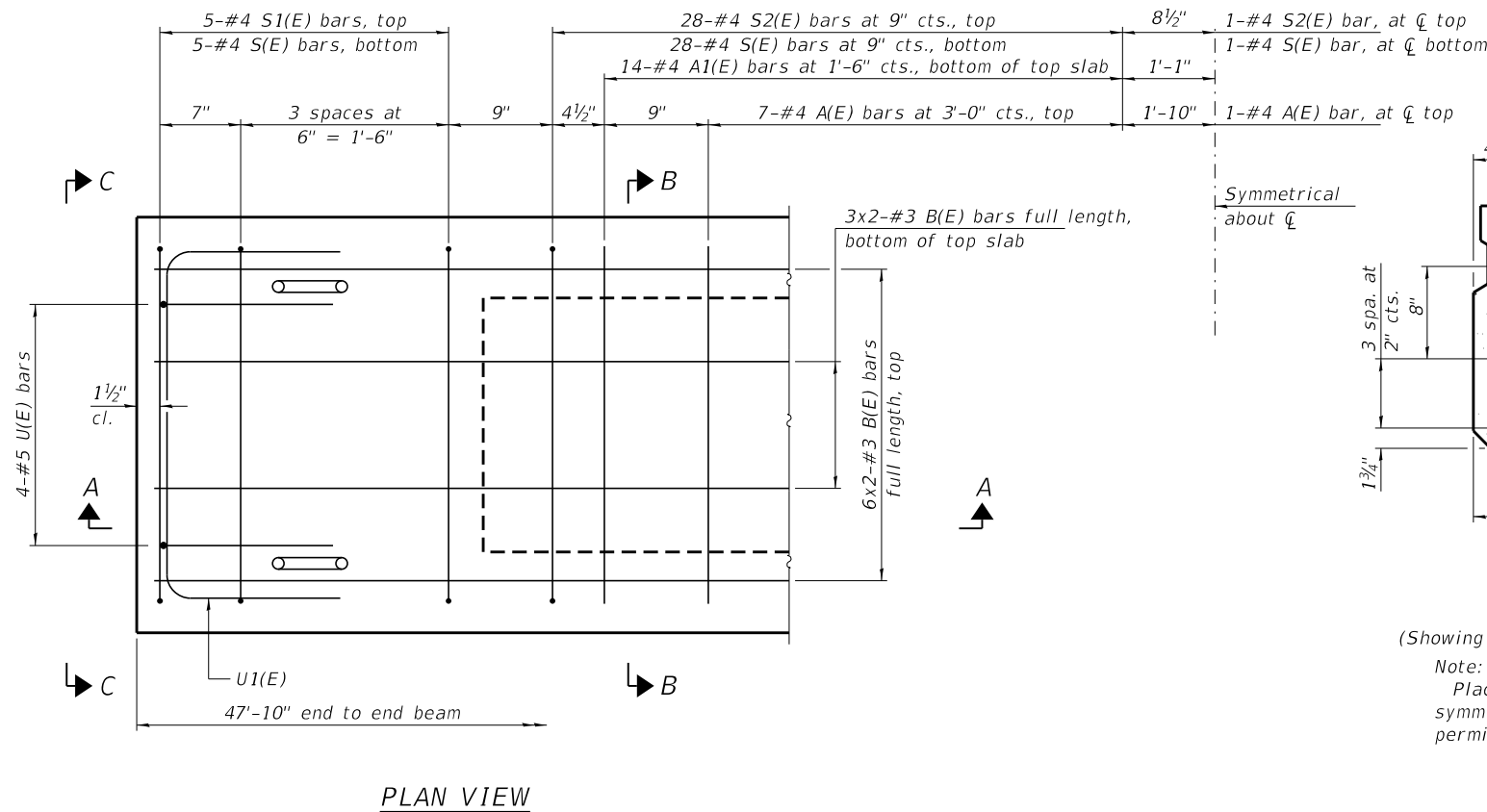
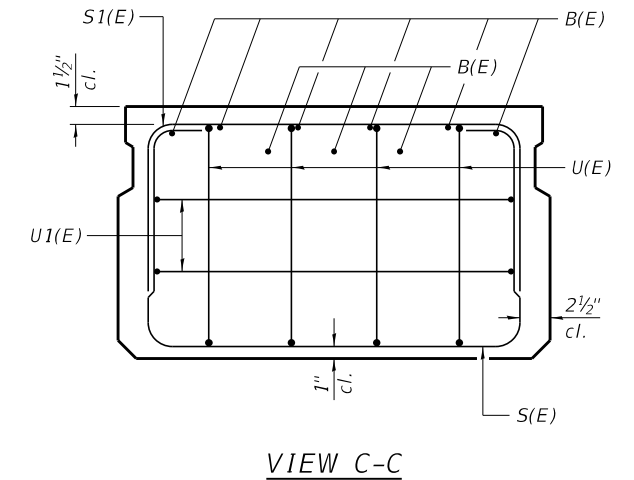
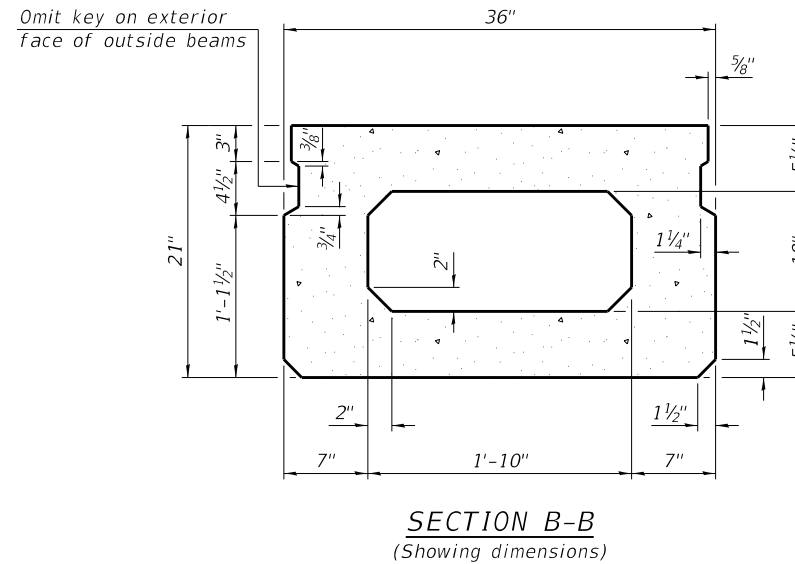
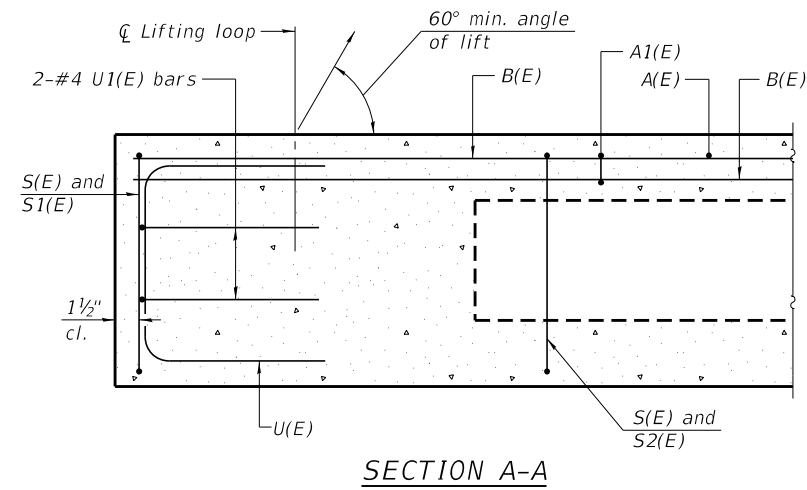


SECTION THRU SOUTH ABUTMENT

Contractor shall remove riprap, excavate and manually replace to provide 12" of clearance as shown. Cost included in Removal of Existing Superstructure.

*To be included with roadway plans (See Special Provisions)

FILE NAME = 190013-shi-bridge - SN 102-3130.dgn	USER NAME = smierzwa	DESIGNED - J.R.B	REVISED -	STATE OF ILLINOIS WOODFORD COUNTY HIGHWAY DEPARTMENT	RIPRAP LAYOUT STRUCTURE NO. 102-3130	F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
HAMPTON, LENZINI AND RENWICK, INC. 3035 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 ILLINOIS PROFESSIONAL DESIGN FIRM LS / PE / SE CORP. 184.000959	PLOT SCALE =	CHECKED - S.T.M.	REVISED -			363	18-00166-00-BR	WOODFORD	65	13	
PLOT DATE = 2/28/2023		DRAWN - R.D.H.	REVISED -			CONTRACT NO. 89779					
		CHECKED - S.T.M./S.W.M.	REVISED -			ILLINOIS FED. AID PROJECT NO. ZH0Y(896)					



BAR LIST
ONE BEAM ONLY
(For information only)

Bar	No.	Size	Length	Shape
A(E)	15	#4	2'-7"	—
A1(E)	28	#4	2'-10"	—
B(E)	18	#3	24'-7"	—
S(E)	67	#4	6'-5"	□
S1(E)	10	#4	4'-11"	□
S2(E)	57	#4	5'-2"	□
U(E)	8	#5	4'-0"	□
U1(E)	4	#4	5'-0"	□

Note:
Place the number of strands specified in each row symmetrically about the centerline of beam in the permissible strand locations shown.

MINIMUM BAR LAP
#3 bar = 1'-6"

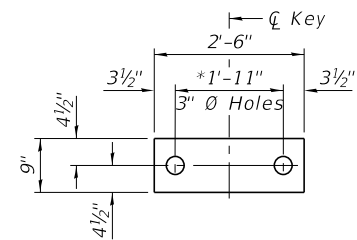
Note:
Spacing of S(E) and S2(E) bars may be adjusted up to 4" in the immediate area of the transverse tie diaphragms to miss the block outs for the transverse ties.
Bars indicated thus 6x2-#3 etc. indicates 6 lines of bars with 2 lengths per line

Note:
See sheet 4 and 5 of 17 for additional details and Bill of Material.

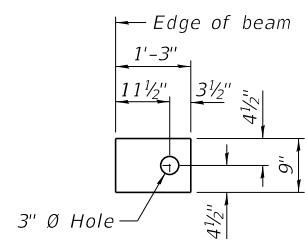
PD-2136-0 1-1-2020

FILE NAME = 190013-shi-bridge - SN 102-3130.dgn	USER NAME = smierzwa	DESIGNED - J.R.B	REVISIONS -	STATE OF ILLINOIS WOODFORD COUNTY HIGHWAY DEPARTMENT	21" x 36" PPC DECK BEAM - SPANS 1, 2 & 3 STRUCTURE NO. 102-3130	F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
HAMPTON, LENZINI AND RENWICK, INC. 3035 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 ILLINOIS PROFESSIONAL DESIGN FIRM L.S./P.E./S.E. CORP. 184.000959	PLOT SCALE =	CHECKED - S.T.M.	REVISIONS -			363	18-00166-00-BR	WOODFORD	65	14
PLOT DATE = 2/28/2023		DRAWN - R.D.H.	REVISIONS -			CONTRACT NO. 89779				
		CHECKED - S.T.M./S.W.M.	REVISIONS -			SHEET NO. 4 OF 17 SHEETS				

ILLINOIS FED. AID PROJECT NO. ZH0Y(896)



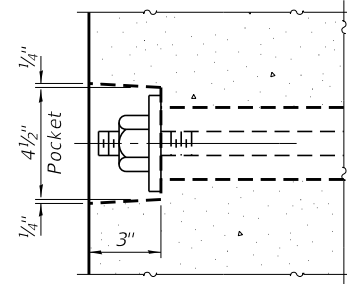
FABRIC BEARING PAD
(Interior - 54 req'd)



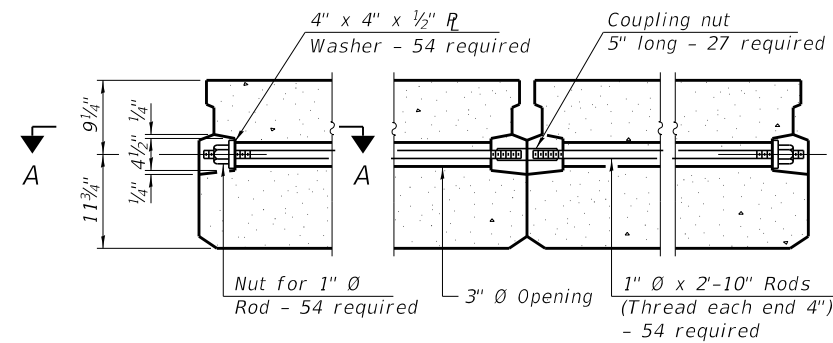
FABRIC BEARING PAD
(Exterior - 12 req'd)

FIXED

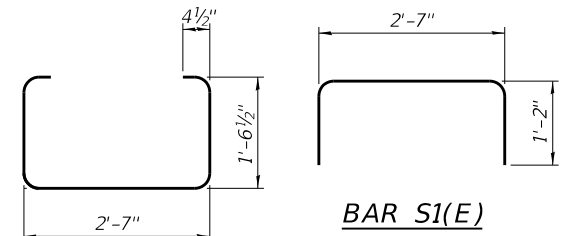
Notes:
All bearing pads shall be 1" thick.



SECTION A-A

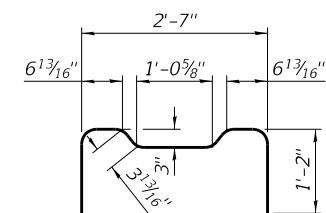


TYPICAL TRANSVERSE TIE ASSEMBLY



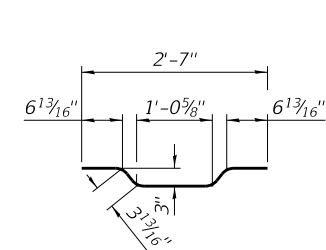
BAR S1(E)

BAR S(E)



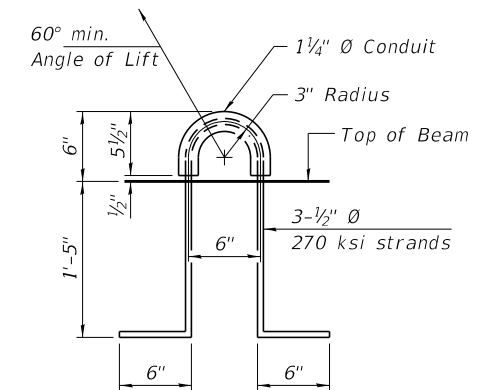
BAR U(E)

BAR S2(E)

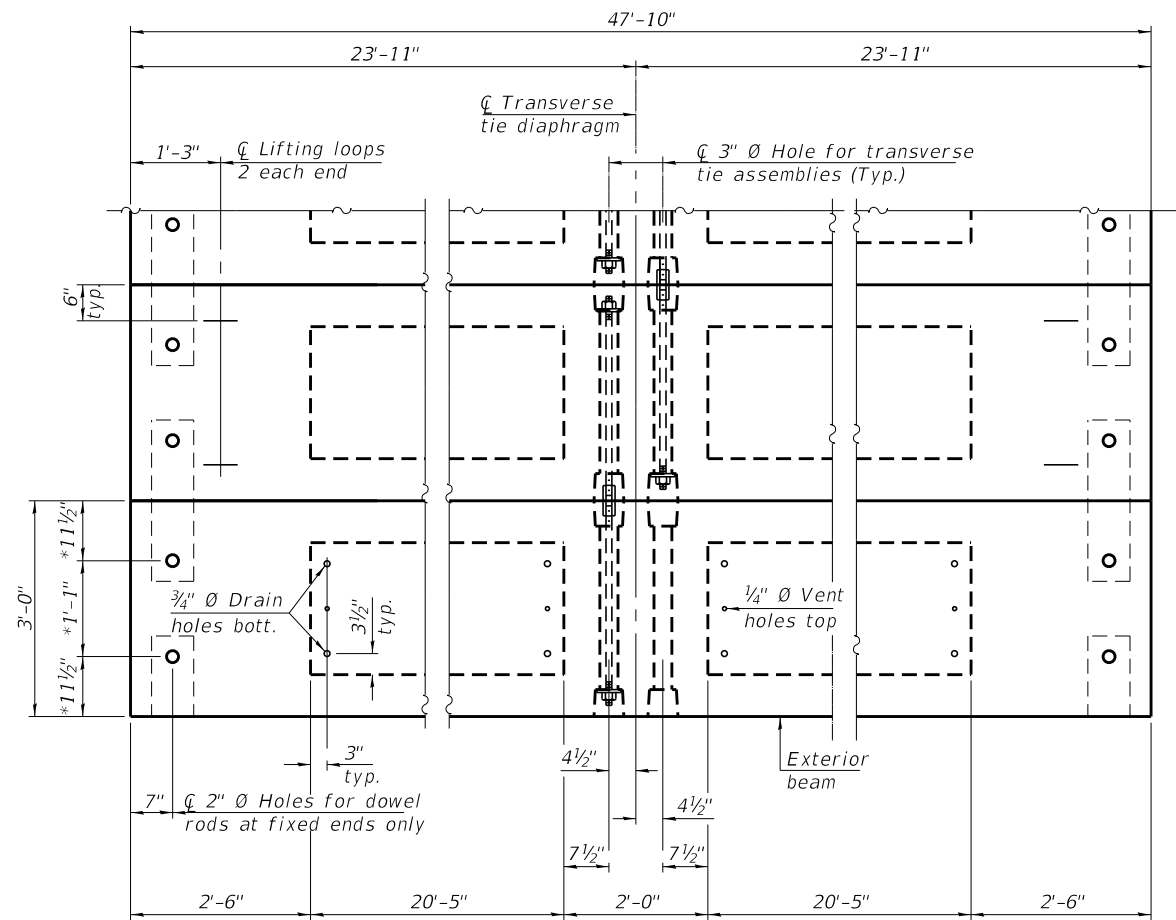


BAR U1(E)

BAR A1(E)



LIFTING LOOP DETAIL



PLAN VIEW

Note:
Connect beams in pairs with the transverse tie configuration shown.

*Dowel rod hole locations modified to miss existing dowels.

NOTES

Prestressing steel shall be uncoated high strength, low relaxation 7-wire strand, Grade 270. The nominal diameter shall be 1/2" and the nominal cross-sectional area shall be 0.153 sq. in. The 1" rods in the transverse tie assembly shall be tightened to a snug fit and the threads set. Pockets on exterior faces of bridge shall be filled with grout after transverse tie assembly is in place. Two 1/8" fabric adjusting shims of the dimensions of the exterior bearing pad shall be provided for each bearing pad location. A minimum 2 1/2" diameter lifting pin shall be used to engage the lifting loops during handling. Corrosion Inhibitor, per Article 1020.05(b)(10) and 1021.07 of the Standard Specifications, shall be used in the concrete for precast prestressed concrete deck beams. Compressive strength of prestressed concrete, f'c, shall be 6000 psi. Compressive strength of prestressed concrete at release, f'ci, shall be 5000 psi.

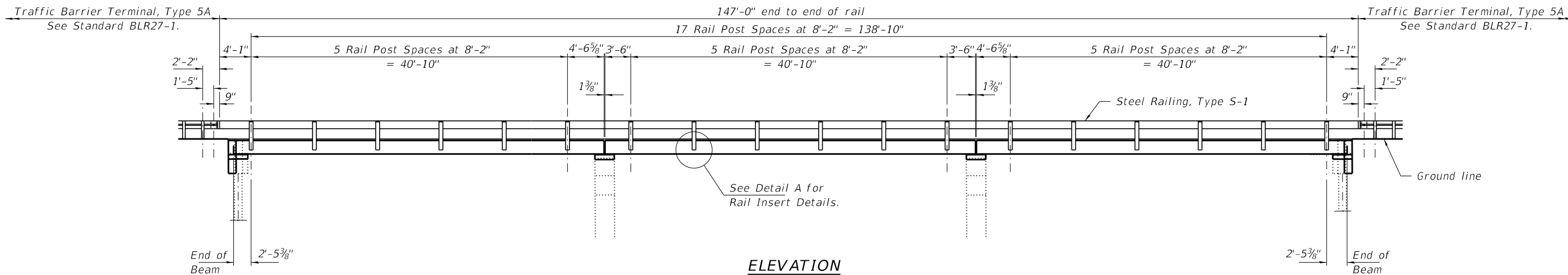
BILL OF MATERIAL

Precast Prestressed Conc. Deck Bms. (21" depth)	Sq. Ft.	4,305
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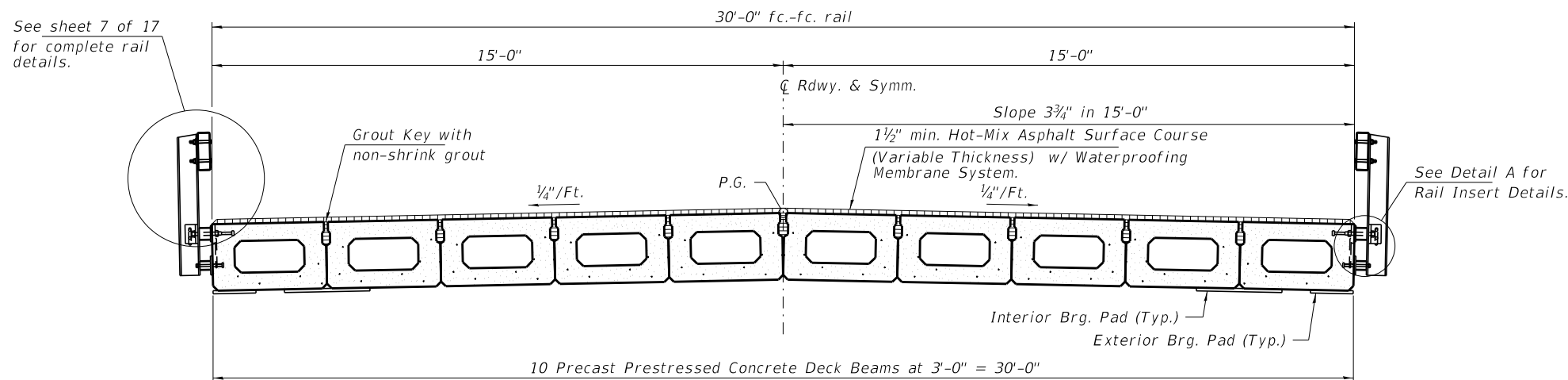
PDD-2136-0 1-1-2020

FILE NAME = 190013-shi-bridge - SN 102-3130.dgn	USER NAME = smierzwa	DESIGNED - J.R.B	REVISED -	STATE OF ILLINOIS WOODFORD COUNTY HIGHWAY DEPARTMENT	21" x 36" PPC DECK BEAM DETAILS - SPANS 1, 2 & 3 STRUCTURE NO. 102-3130	F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
HAMPTON, LENZINI AND RENWICK, INC. 3035 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62763 ILLINOIS PROFESSIONAL DESIGN FIRM L5 / PE / SE CORP. 184.000959	PLOT SCALE =	CHECKED - S.T.M.	REVISED -			363	18-00166-00-BR	WOODFORD	65	15	
	PLOT DATE = 2/28/2023	DRAWN - R.D.H.	REVISED -			CONTRACT NO. 89779					
		CHECKED - S.T.M./S.W.M.	REVISED -			SHEET NO. 5 OF 17 SHEETS					

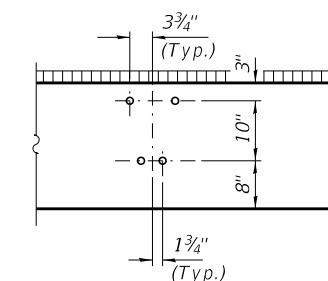
ILLINOIS FED. AID PROJECT NO. ZH0Y(896)



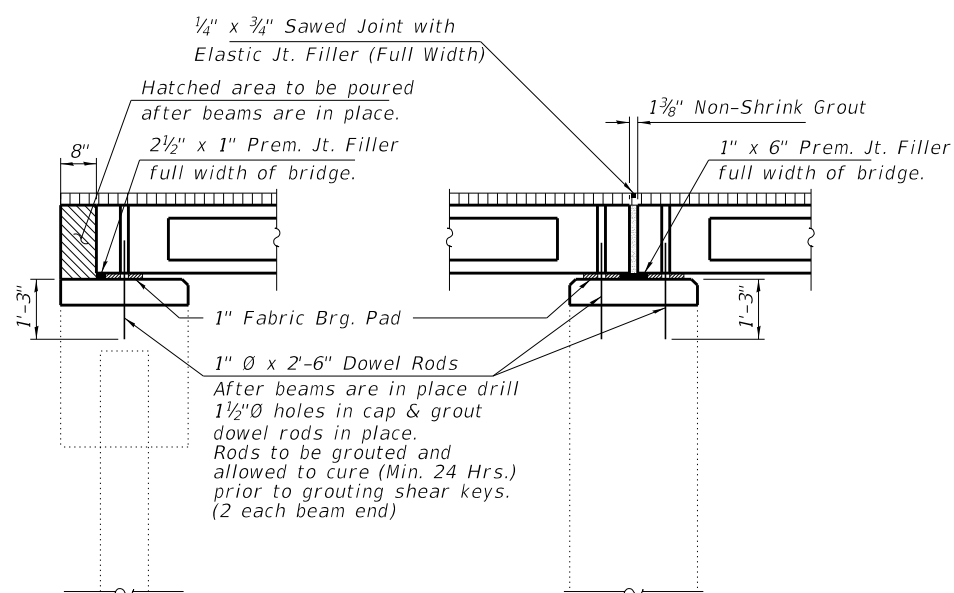
ELEVATION
Showing Rail Post Spaces
See sheet 7 of 17 for Railing Details.



CROSS SECTION
See sheets 4 & 5 of 17 for Superstructure Details.

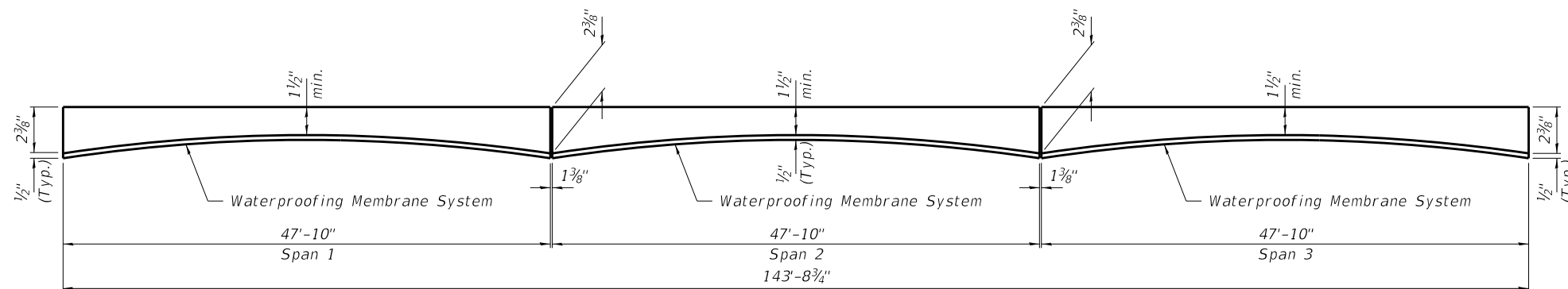


DETAIL A



SECTION AT ABUTMENTS

SECTION AT PIERS



ANTICIPATED HMA WEARING SURFACE PROFILE
(For information only - beam camber may vary in field.)

FILE NAME = 190013-shi-bridge - SN 102-3130.dgn	USER NAME = smierzwa	DESIGNED - J.R.B.	REVISED -	STATE OF ILLINOIS WOODFORD COUNTY HIGHWAY DEPARTMENT	SUPERSTRUCTURE DETAILS STRUCTURE NO. 102-3130	F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
HAMPTON, LENZINI AND RENWICK, INC. 3035 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 ILLINOIS PROFESSIONAL DESIGN FIRM LS / PE / SE CORP. 184.000959	PLOT SCALE =	CHECKED - S.T.M.	REVISED -			363	18-00166-00-BR	WOODFORD	65	16	
PLOT DATE = 2/28/2023		DRAWN - R.D.H.	REVISED -			CONTRACT NO. 89779					
		CHECKED - S.T.M./S.W.M.	REVISED -			SHEET NO. 6 OF 17 SHEETS					

Notes:

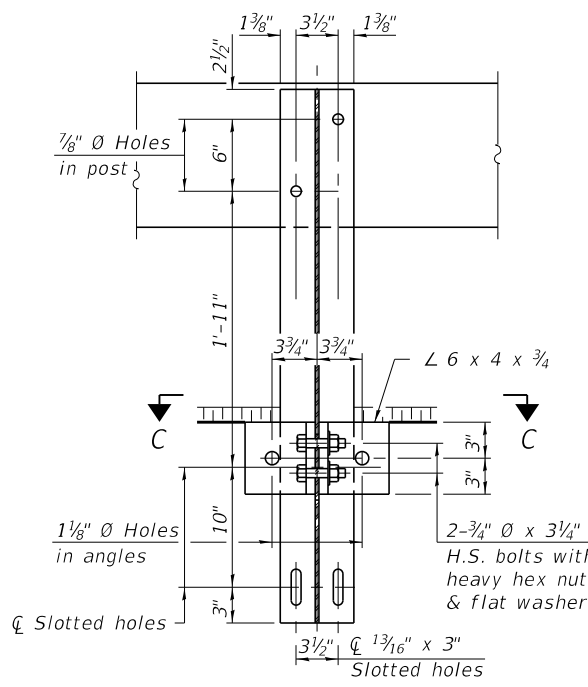
A sufficient number of shims of various thicknesses, sized to fit behind the top spacer assembly, 5" x 11 1/2", and bottom spacer assembly, 6" x 7", shall be provided to adjust posts for proper alignment. If the summation of shims is greater than 1/4" (top) or 1/2" (bottom), longer bolts are required. Cost included with Steel Railing, Type S-1.

All steel rail elements including shims shall be galvanized according to Article 509.05 of the Standard Specifications.

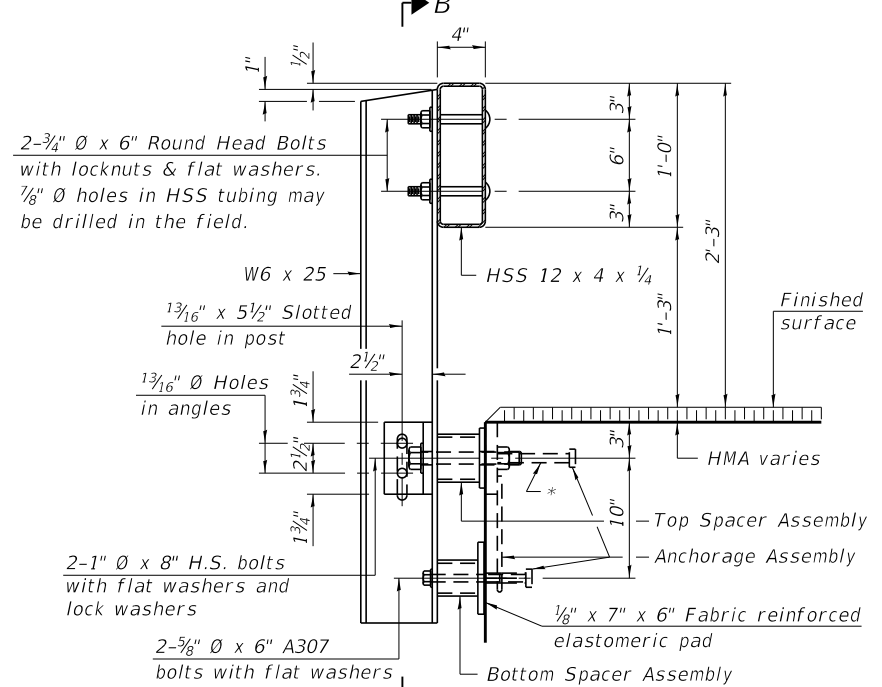
All HSS tubing serving as railing shall be CVN tested according to Article 1006.34(b) of the Standard Specifications.

Rail splice inserts may be built out of 2 - 3/8" bent plates in lieu of the 4 plate rail splice inserts shown, provided the outside dimensions are matched.

All round head bolts shall be ASTM A307 with locknuts according to ASTM A563 grade A.

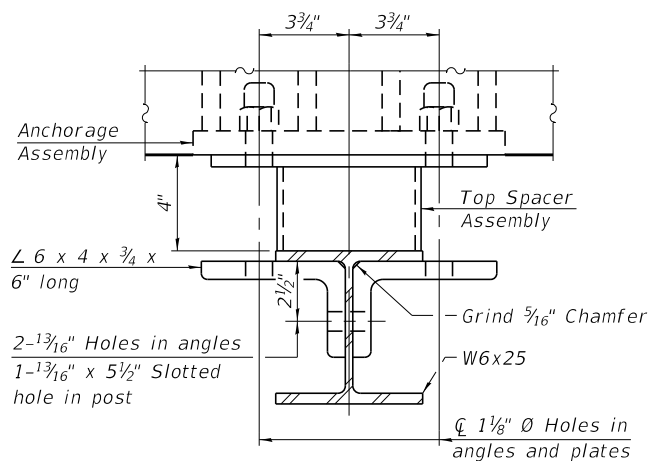


SECTION B-B

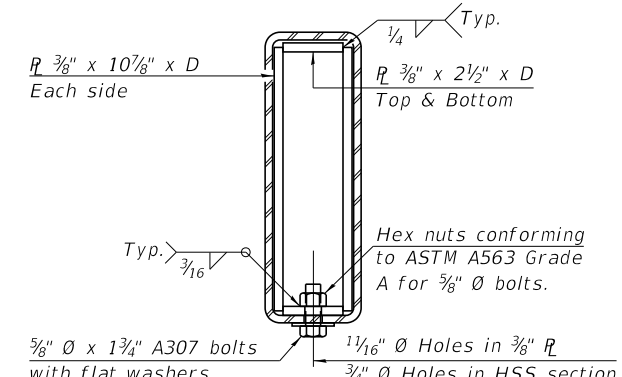


SECTION AT RAILING POST

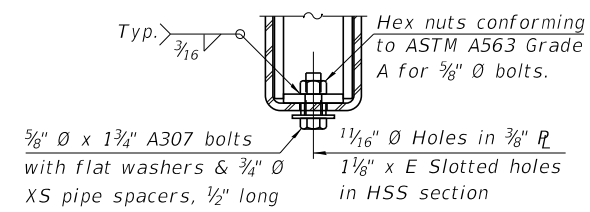
* The outermost longitudinal reinforcement bar shall be placed directly above the studs of the rail post anchorage assembly. The anchorage studs may be bent down 1/2" to accommodate the top reinforcement bar placement.



SECTION C-C



SECTION AT RAIL SPLICE

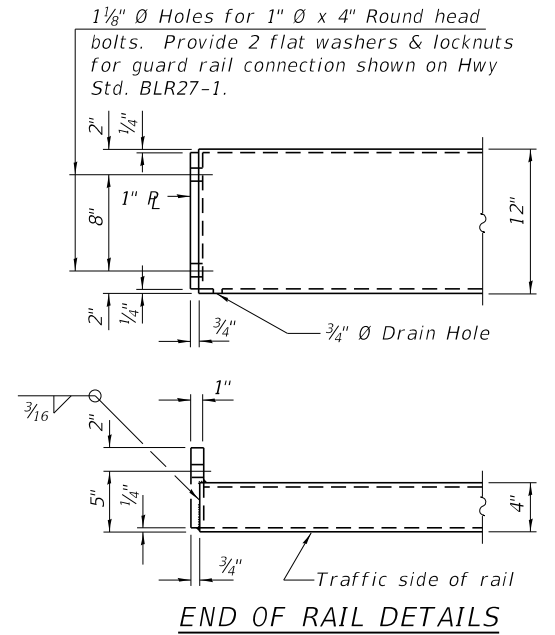


RAIL SPLICE CONNECTION AT EXPANSION JT.

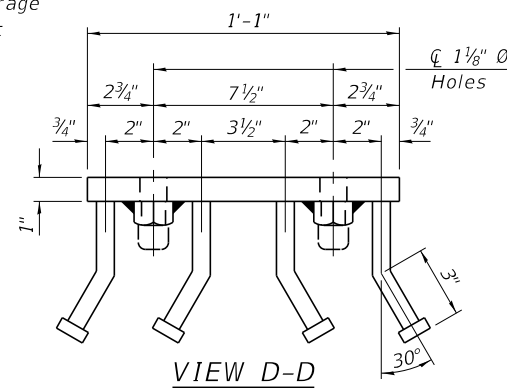
SPLICE DIMENSIONS

Location	T	A	B	C	D	E
All locs. not over exp. jts.	0	1/4"	4"	4"	1'-8"	-
Over Strip Seal Jt.	≤4"	2 1/2"	4 7/8"	4 7/8"	1'-10"	3 1/16"
Over Finger or Modular Jt.	≤9 1/2"	5 1/2"	7 3/8"	7 1/4"	2'-9 1/4"	5 1 3/16"
Over Finger or Modular Jt.	≤15"	8 1/4"	10 1/8"	10"	3'-8 1/4"	8 9/16"

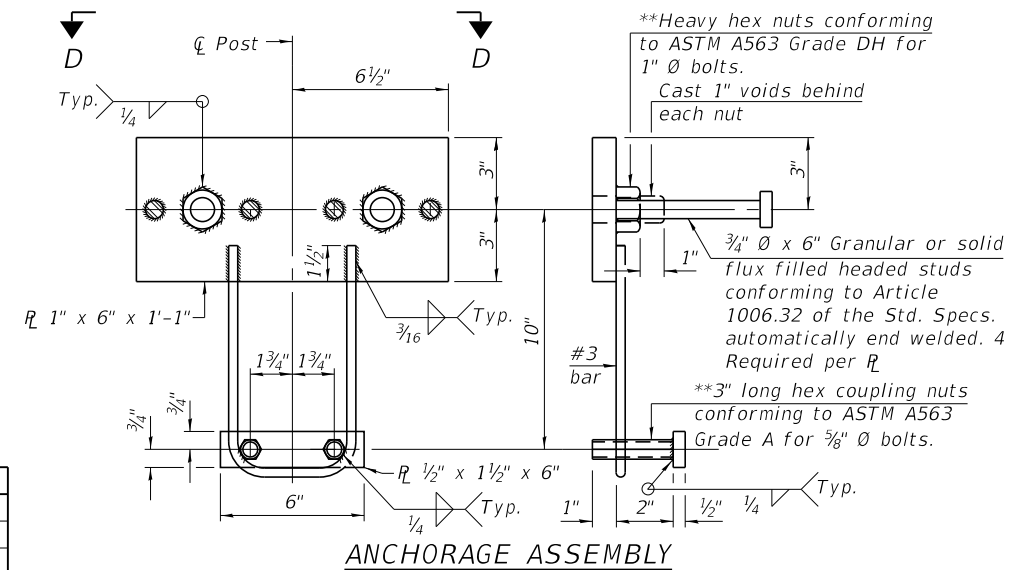
T = ; total movement along centerline of roadway at expansion joint.



END OF RAIL DETAILS

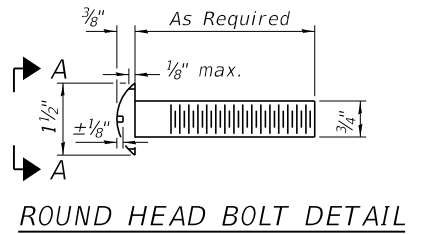


VIEW D-D

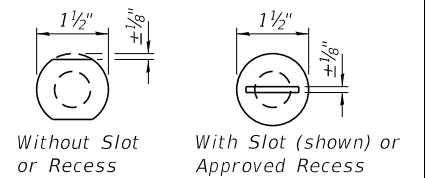


ANCHORAGE ASSEMBLY

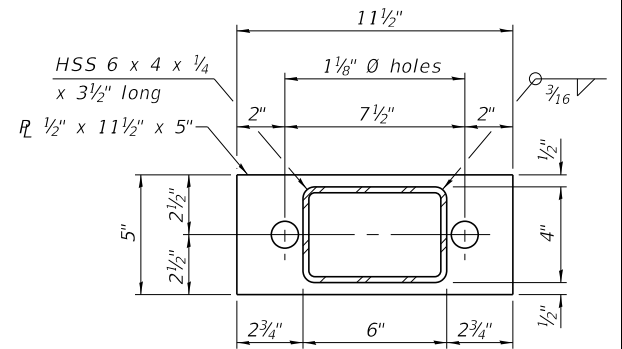
** Threaded areas shall be plugged or blocked off during casting of concrete.



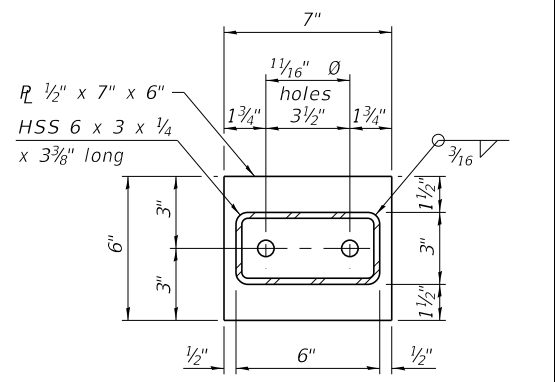
ROUND HEAD BOLT DETAIL



VIEW A-A



TOP SPACER ASSEMBLY



BOTTOM SPACER ASSEMBLY

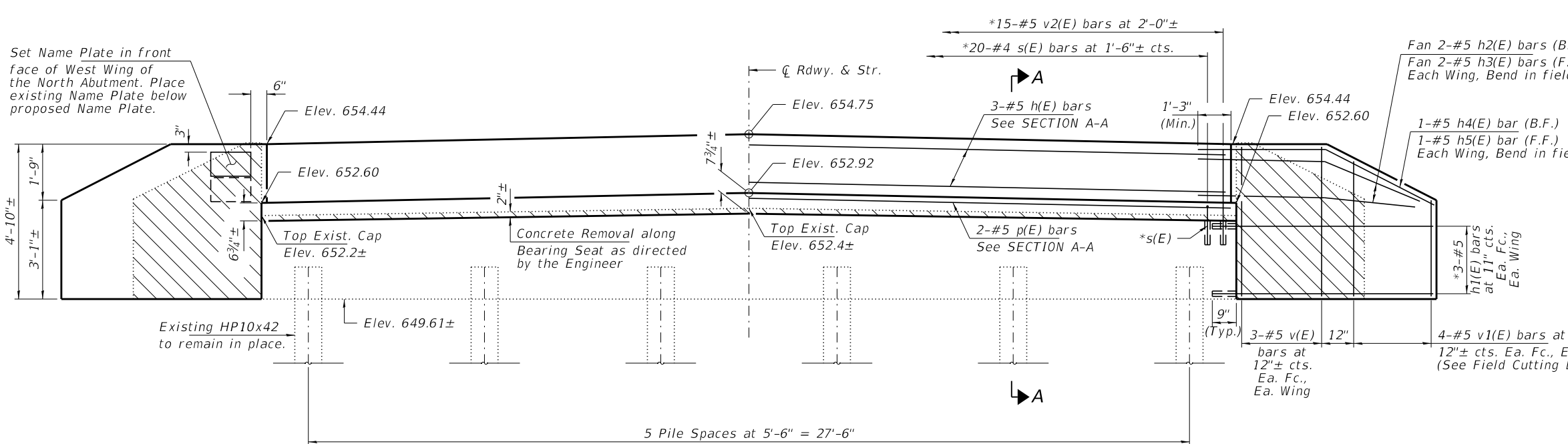
BILL OF MATERIAL

Item	Unit	Quantity
Steel Railing, Type S-1	Foot	294

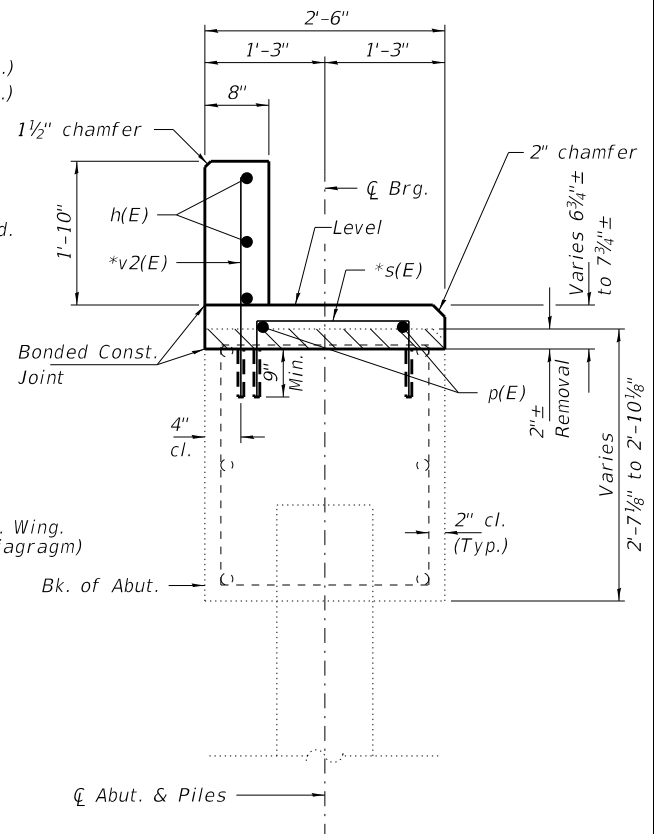
RAILING CRITERIA

NCHRP 350 Test Level	2
Railing Weight (plf)	50
Max Post Spacing	10'-9"
HMA thickness range (in)	1 1/4" - 3 1/8"

R-23A 10-12-2021



ELEVATION
(Looking North)

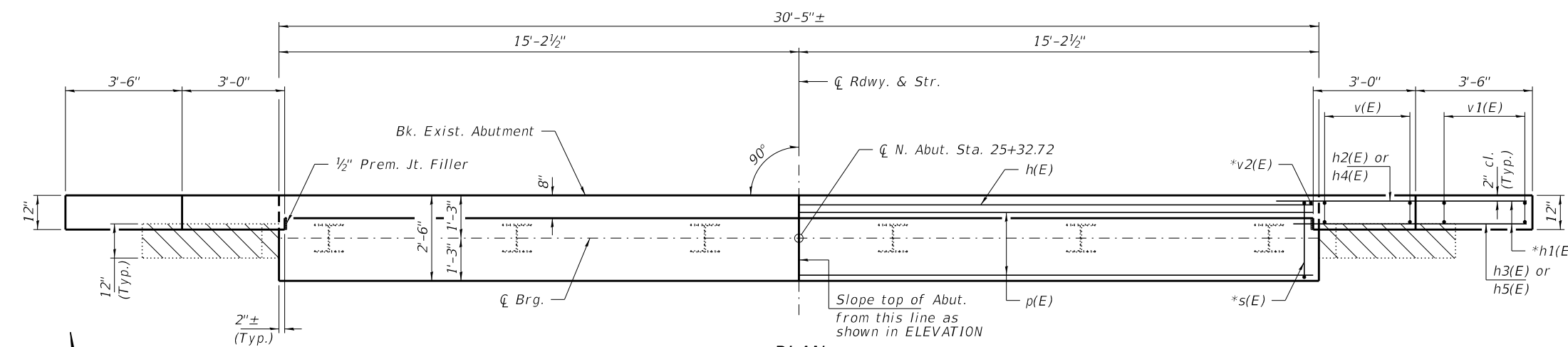


SECTION A-A

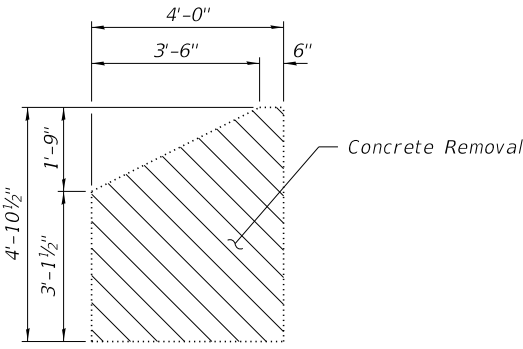
Backwall and wingwalls are to be poured after beams are in place.

*h1(E), s(E), and v2(E) bars shall be drilled and set according to Art. 509.06 of the Standard Specifications. Bars shall have a 9" minimum embedment depth, be set in 1"Ø holes and filled with an approved epoxy grout.

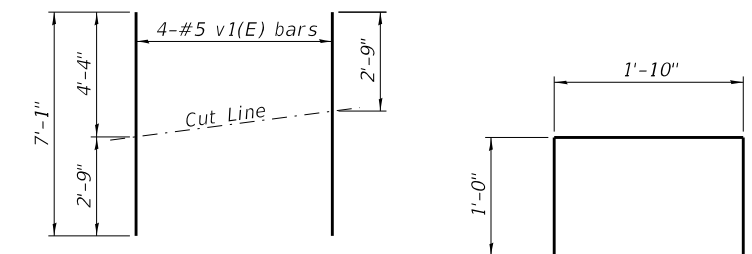
Hatched area indicates Concrete Removal



PLAN

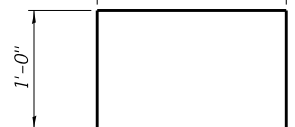


**EXISTING WINGWALL
CONCRETE REMOVAL**



FIELD CUTTING DIAGRAM

Order v1(E) bars full length. Cut as shown and use remainder of bars in opposite face.

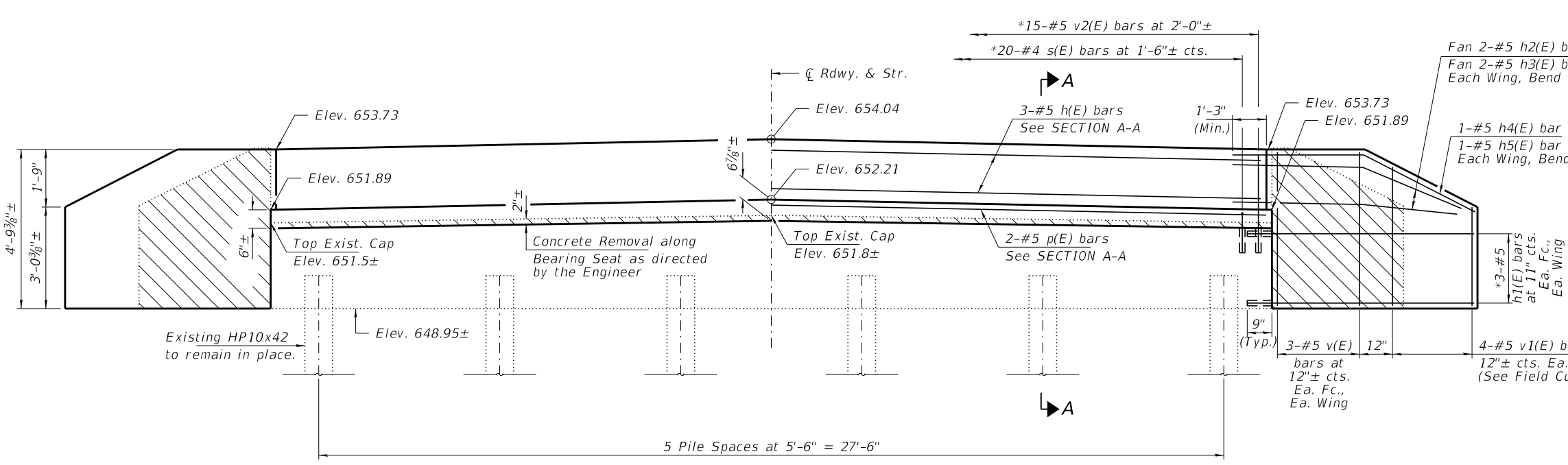


BAR S(E)

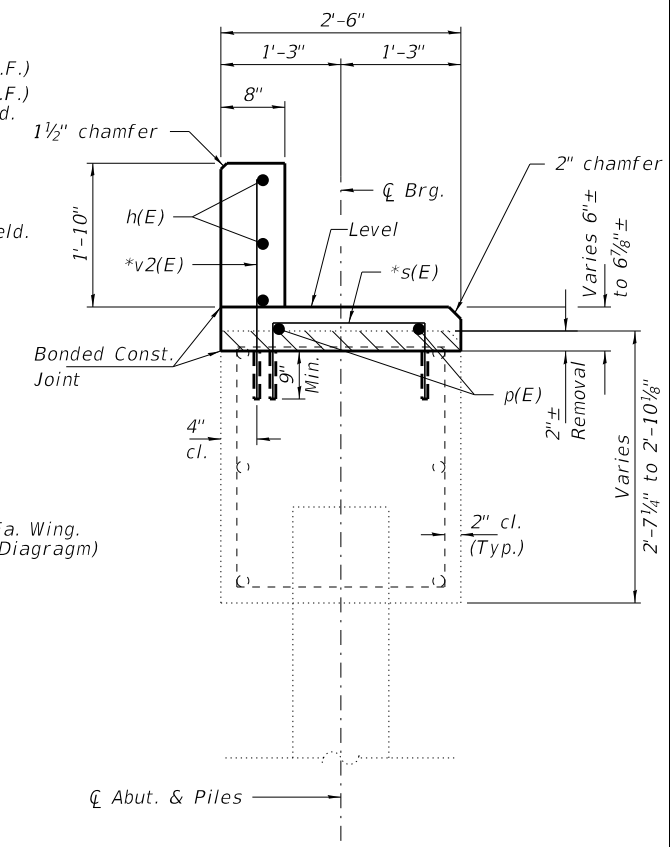
Notes:
The contractor shall repair, at their own expense, any damage to the substructure caused by removal operations.
Cost of drilling and grouting reinforcement bars included in cost for Concrete Structures.
Existing wingwall reinforcement which extends into the abutment cap shall be burned off flush with surface, ground smooth, and sealed with epoxy. Cost included with Concrete Removal.
The existing top of abutment cap is sloped. The Contractor shall level top of cap to the satisfaction of the Engineer during removal.
Backwall and wingwalls shall be poured after the beams are in place.
See Existing Plans for existing reinforcement placement.

BILL OF MATERIAL - N. ABUT.

BAR	NO.	SIZE	LENGTH	SHAPE
h(E)	3	#5	30'-1"	—
h1(E)	12	#5	7'-0"	—
h2(E)	4	#5	7'-4"	—
h3(E)	4	#5	5'-10"	—
h4(E)	2	#5	8'-0"	—
h5(E)	2	#5	6'-6"	—
p(E)	2	#5	30'-1"	—
s(E)	20	#4	3'-10"	□
v(E)	12	#5	4'-5"	—
v1(E)	8	#5	7'-1"	—
v2(E)	15	#5	3'-0"	—
Concrete Structures			Cu. Yd.	5.1
Concrete Removal			Cu. Yd.	1.7
Reinf. Bars, Epoxy Coated			Pound	540
Name Plates (Special)			Each	1
Asbestos Bearing Pad Removal			Each	11



ELEVATION
(Looking South)

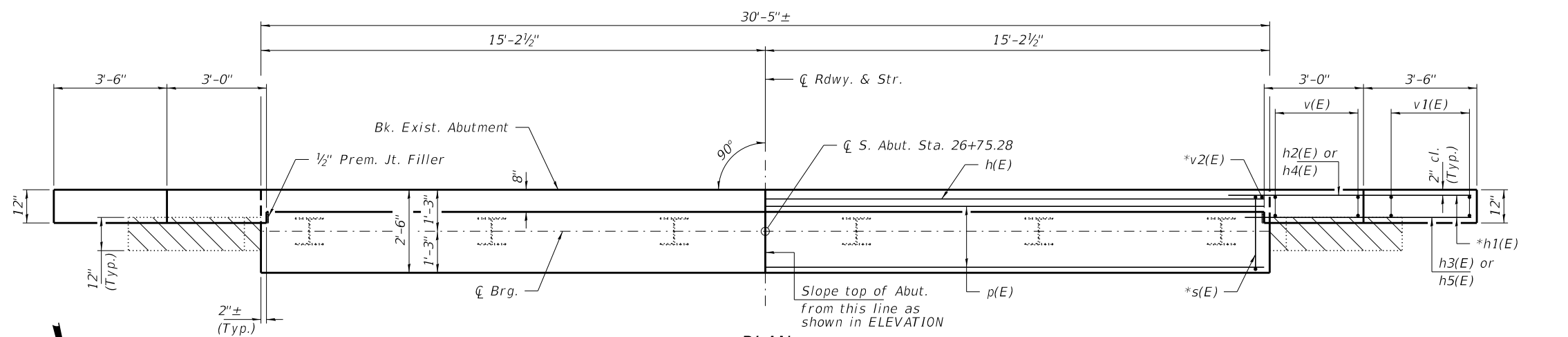


SECTION A-A

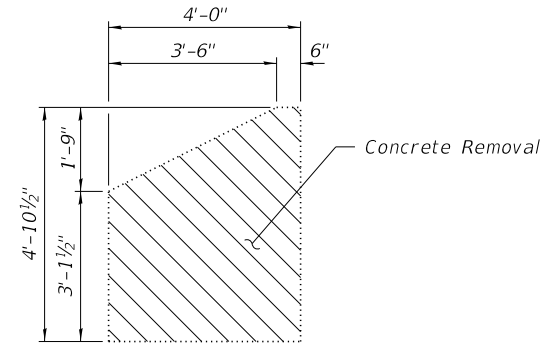
Backwall and wingwalls are to be poured after beams are in place.

*h1(E), s(E), and v2(E) bars shall be drilled and set according to Art. 509.06 of the Standard Specifications. Bars shall have a 9" minimum embedment depth, be set in 1"Ø holes and filled with an approved epoxy grout.

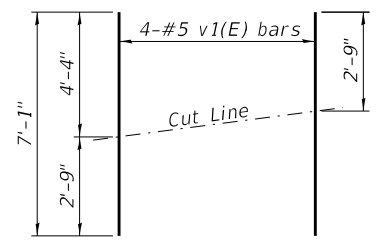
Hatched area indicates Concrete Removal



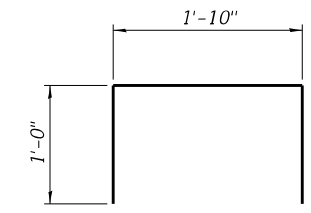
PLAN



EXISTING WINGWALL CONCRETE REMOVAL



FIELD CUTTING DIAGRAM
Order v1(E) bars full length. Cut as shown and use remainder of bars in opposite face.

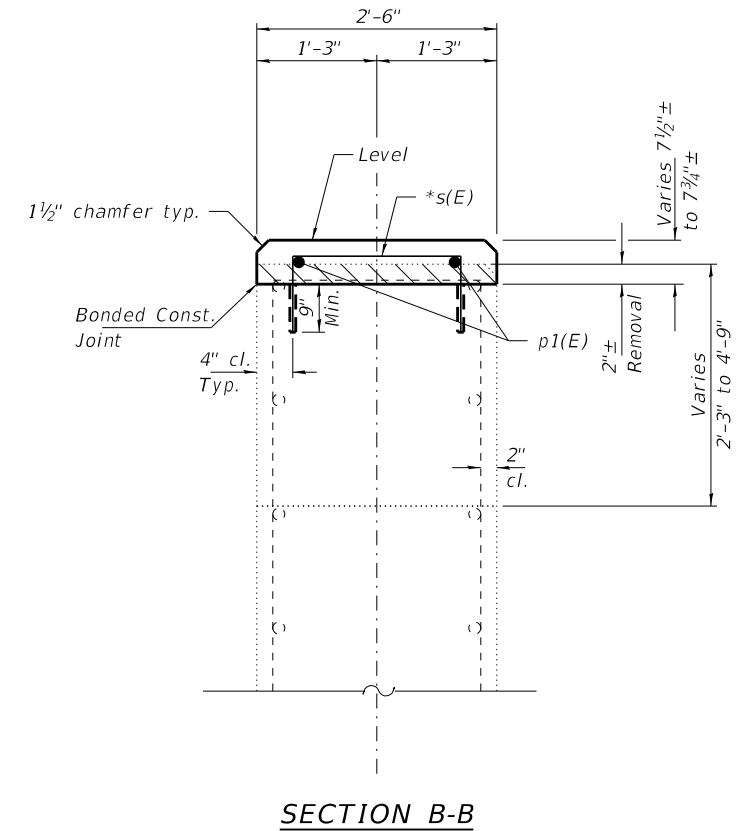
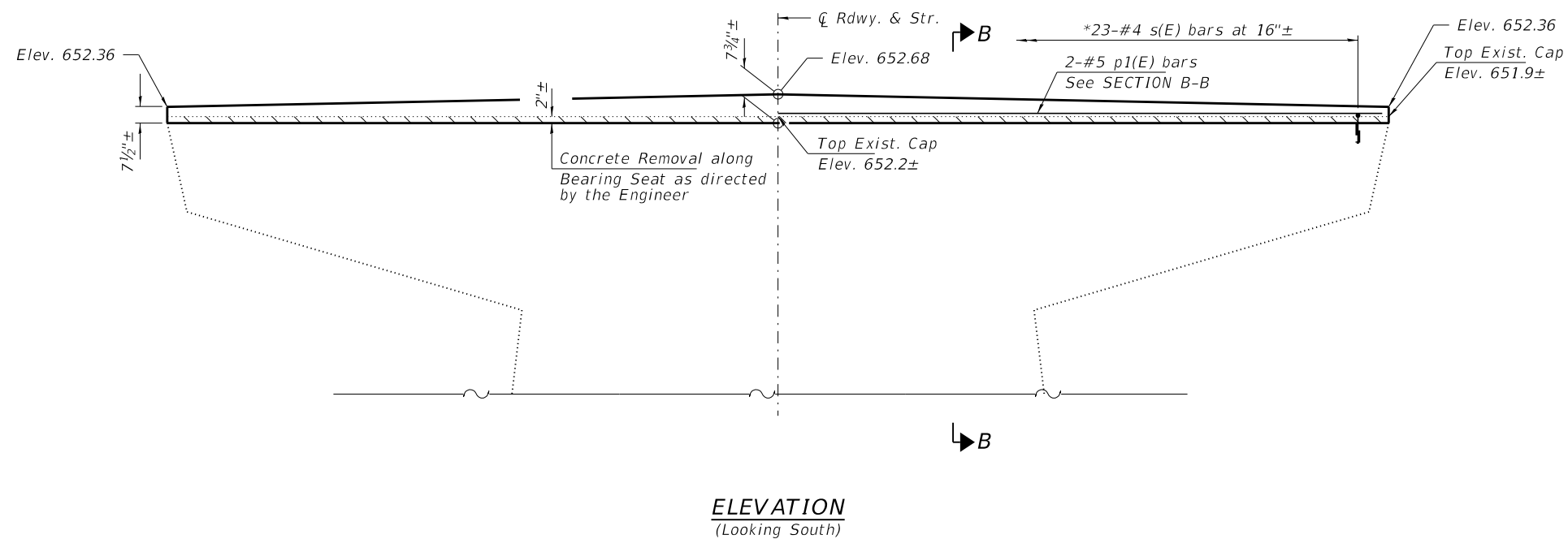


BAR S(E)

Notes:
The contractor shall repair, at their own expense, any damage to the substructure caused by removal operations.
Cost of drilling and grouting reinforcement bars included in cost for Concrete Structures.
Existing wingwall reinforcement which extends into the abutment cap shall be burned off flush with surface, ground smooth, and sealed with epoxy. Cost included with Concrete Removal.
The existing top of abutment cap is sloped. The Contractor shall level top of cap to the satisfaction of the Engineer during removal.
Backwall and wingwalls shall be poured after the beams are in place.
See Existing Plans for existing reinforcement placement.

BILL OF MATERIAL - S. ABUT.

BAR	NO.	SIZE	LENGTH	SHAPE	
h(E)	3	#5	30'-1"	—	
h1(E)	12	#5	7'-0"	—	
h2(E)	4	#5	7'-4"	—	
h3(E)	4	#5	5'-10"	—	
h4(E)	2	#5	8'-0"	—	
h5(E)	2	#5	6'-6"	—	
p(E)	2	#5	30'-1"	—	
s(E)	20	#4	3'-10"	□	
v(E)	12	#5	4'-5"	—	
v1(E)	8	#5	7'-1"	—	
v2(E)	15	#5	3'-0"	—	
Concrete Structures				Cu. Yd.	4.9
Concrete Removal				Cu. Yd.	1.7
Reinf. Bars, Epoxy Coated				Pound	540
Asbestos Bearing Pad Removal				Each	11

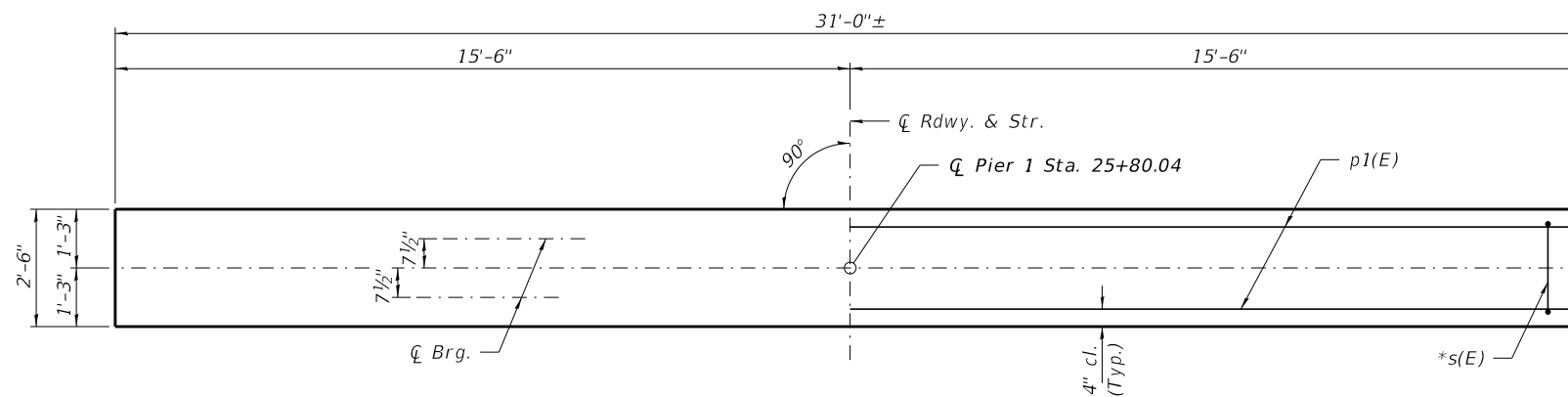


ELEVATION
(Looking South)

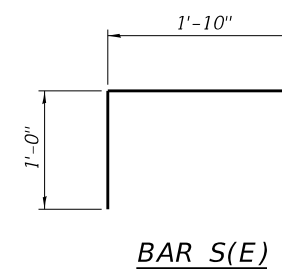
SECTION B-B

*s(E) bars shall be drilled and set according to Art. 509.06 of the Standard Specifications. Bars shall have a 9" minimum embedment depth, be set in 1"Ø holes and filled with an approved epoxy grout.

Hatched area indicates Concrete Removal



PLAN



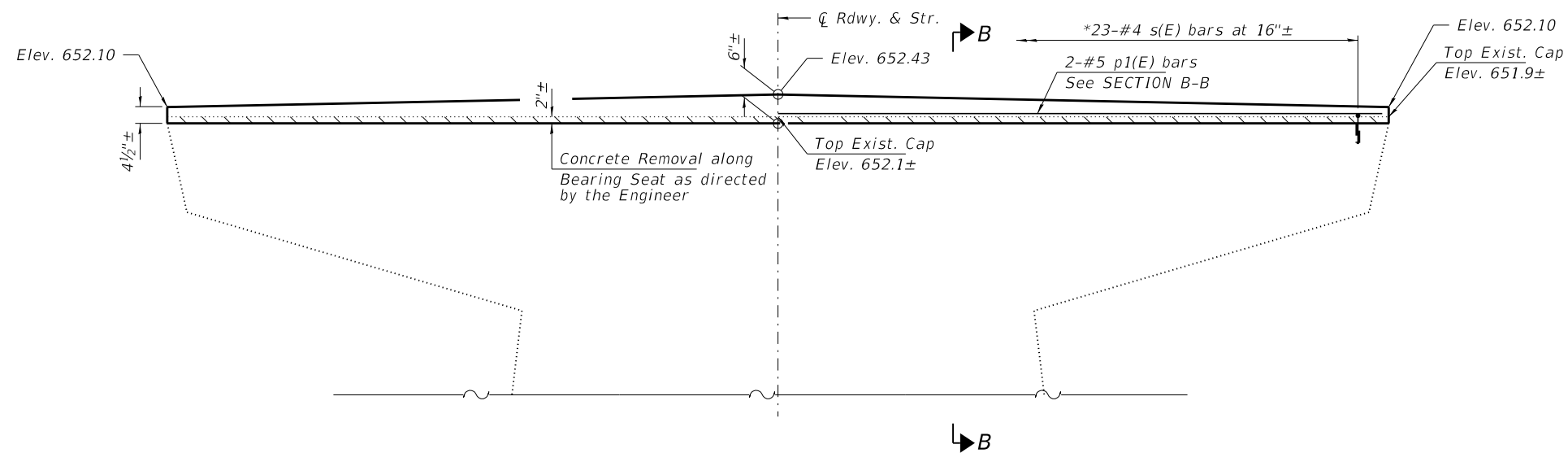
BAR S(E)

Notes:
The contractor shall repair, at their own expense, any damage to the substructure caused by removal operations.
Cost of drilling and grouting reinforcement bars included in cost for Concrete Structures.
The existing top of pier cap is sloped. The Contractor shall level top of cap to the satisfaction of the Engineer during removal.
See Existing Plans for existing reinforcement placement.

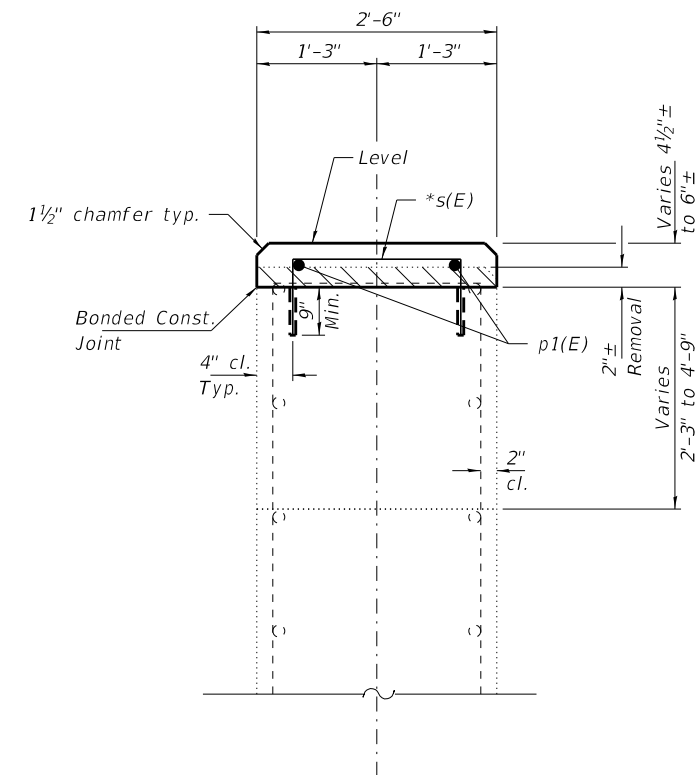
BILL OF MATERIAL - PIER 1

BAR	NO.	SIZE	LENGTH	SHAPE
p1(E)	2	#5	30'-8"	—
s(E)	23	#4	3'-10"	□
Concrete Structures			Cu. Yd.	1.8
Concrete Removal			Cu. Yd.	0.5
Reinf. Bars, Epoxy Coated			Pound	120
Structural Repair of Concrete (Depth ≤ 5')			Sq. Ft.	30
Asbestos Bearing Pad Removal			Each	22

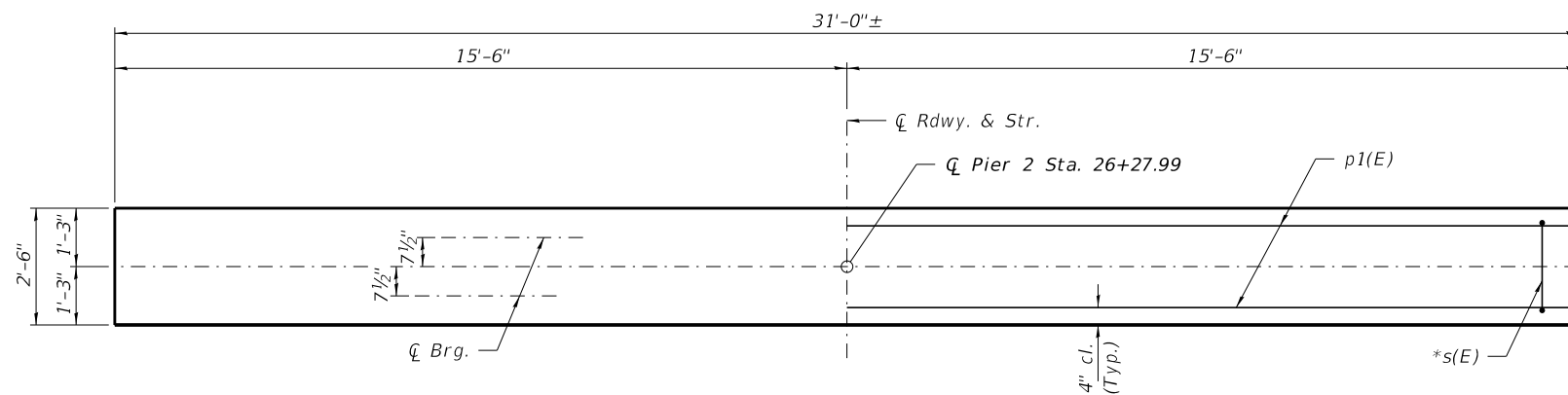
See sheet 12 of 17 for Structural Repair of Concrete (Depth ≤ 5')



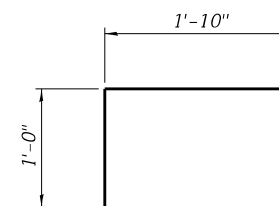
ELEVATION
(Looking South)



SECTION B-B



PLAN



BAR S(E)

*s(E) bars shall be drilled and set according to Art. 509.06 of the Standard Specifications. Bars shall have a 9" minimum embedment depth, be set in 1"Ø holes and filled with an approved epoxy grout.

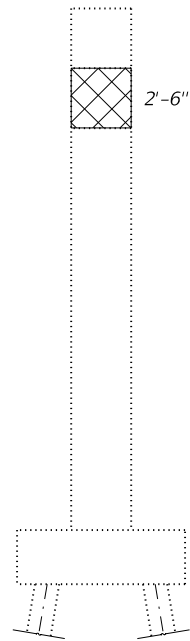
Hatched area indicates Concrete Removal

Notes:
 The contractor shall repair, at their own expense, any damage to the substructure caused by removal operations.
 Cost of drilling and grouting reinforcement bars included in cost for Concrete Structures.
 The existing top of pier cap is sloped. The Contractor shall level top of cap to the satisfaction of the Engineer during removal.
 See Existing Plans for existing reinforcement placement.

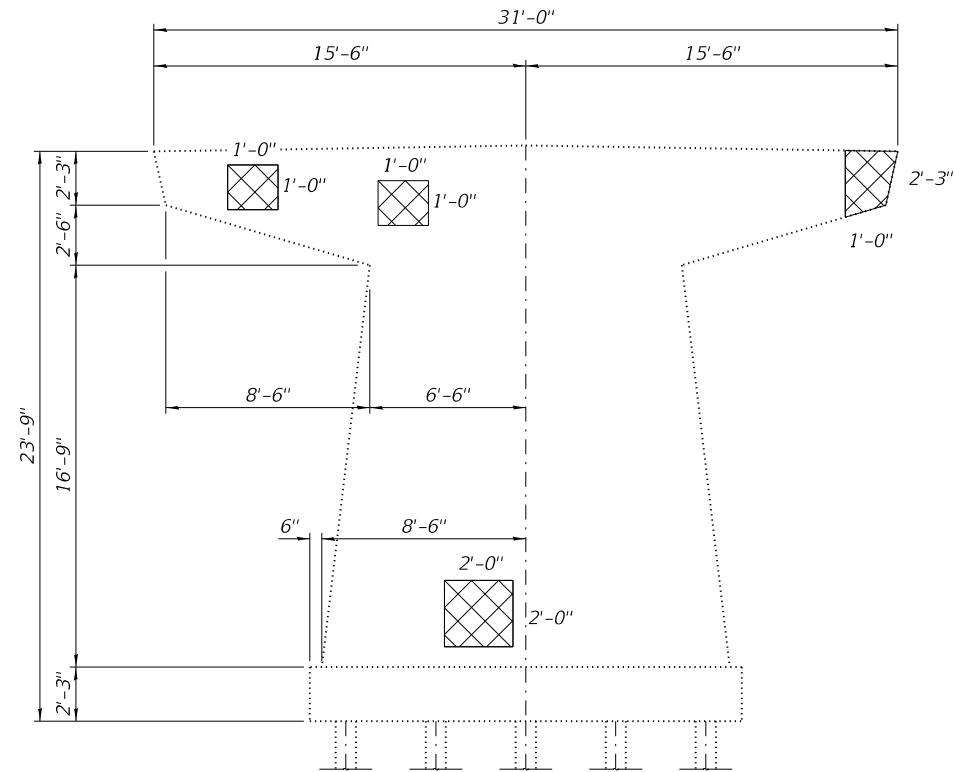
BILL OF MATERIAL - PIER 2

BAR	NO.	SIZE	LENGTH	SHAPE	
p1(E)	2	#5	30'-8"	—	
s(E)	23	#4	3'-10"	□	
Concrete Structures				Cu. Yd.	1.3
Concrete Removal				Cu. Yd.	0.5
Reinf. Bars, Epoxy Coated				Pound	120
Structural Repair of Concrete (Depth ≤ 5')				Sq. Ft.	90
Asbestos Bearing Pad Removal				Each	22

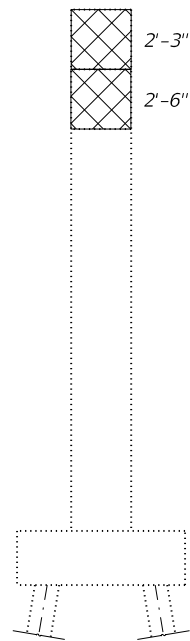
See sheet 13 of 17 for Structural Repair of Concrete (Depth ≤ 5')



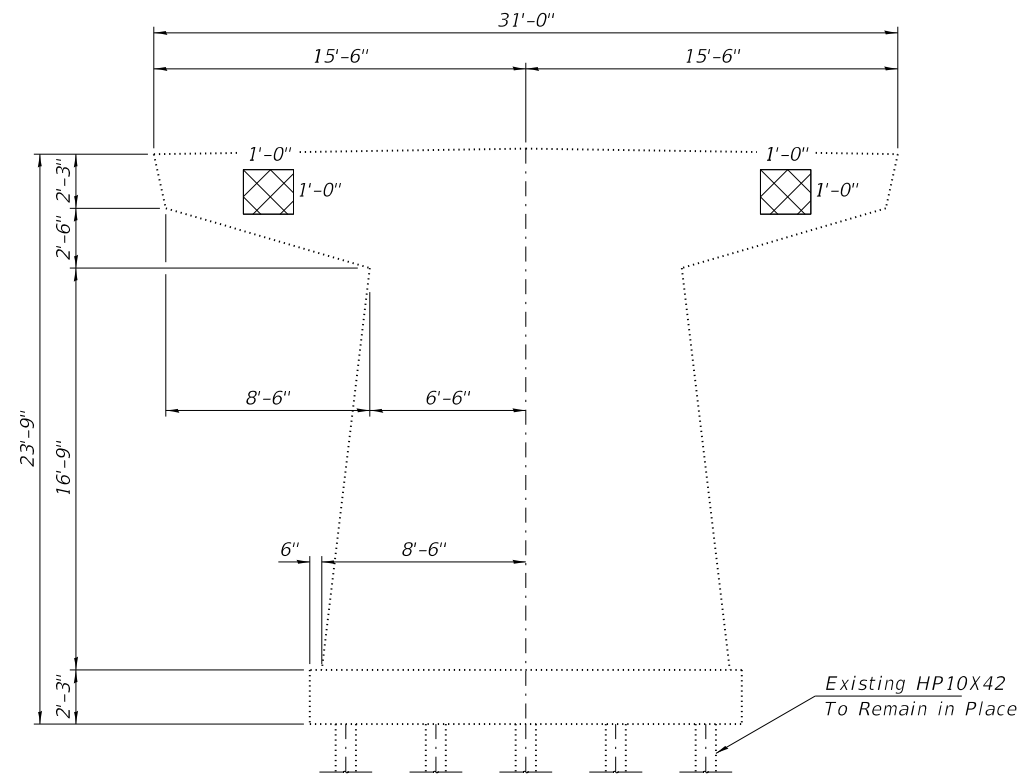
EAST END VIEW
(Looking West)



NORTH FACE ELEVATION
(Looking South)



WEST END VIEW
(Looking East)

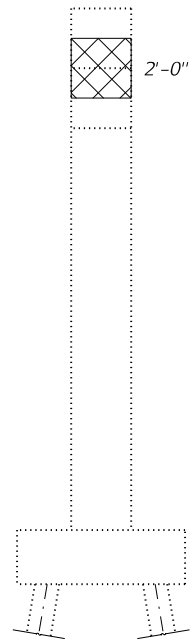


SOUTH FACE ELEVATION
(Looking North)

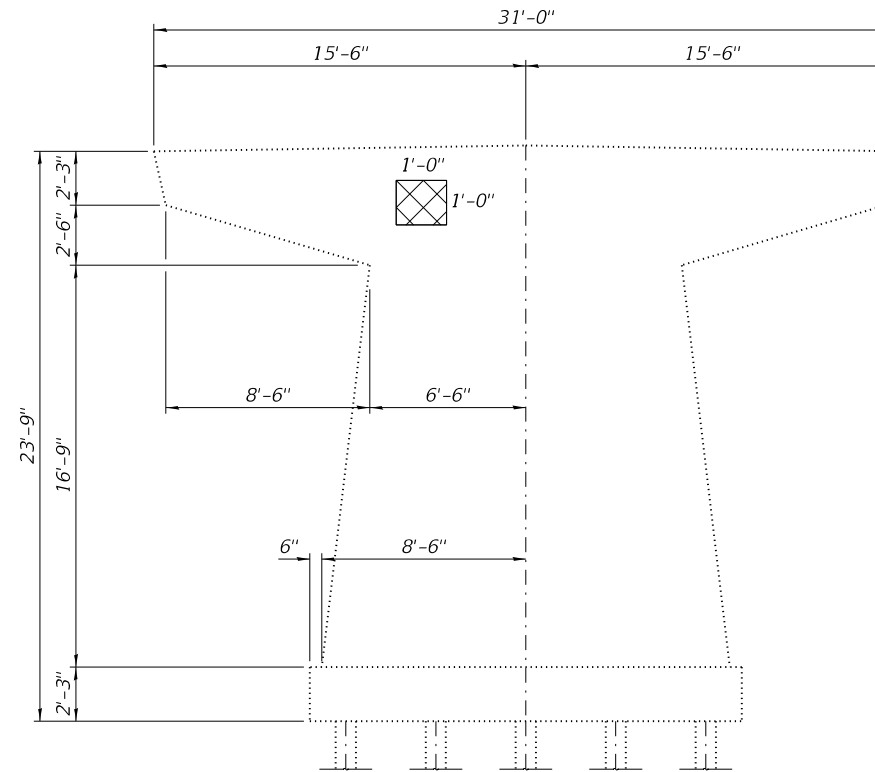


Structural Repair of
Concrete ($\leq 5''$)

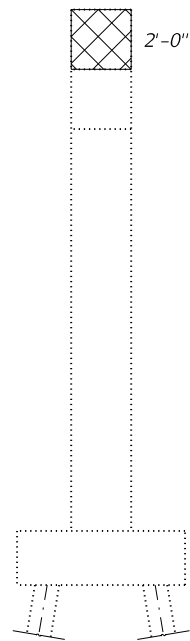
FILE NAME = 190013-shi-bridge - SN 102-3130.dgn	USER NAME = smierzwa	DESIGNED - J.R.B	REVISED -	STATE OF ILLINOIS WOODFORD COUNTY HIGHWAY DEPARTMENT	CONCRETE REPAIR - PIER 1 STRUCTURE NO. 102-3130	F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
HAMPTON, LENZINI AND RENWICK, INC. 3035 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 ILLINOIS PROFESSIONAL DESIGN FIRM LS / PE / SE CORP. 184.000959		CHECKED - S.T.M.	REVISED -			363	18-00166-00-BR	WOODFORD	65	22	
	PLOT SCALE =	DRAWN - R.D.H.	REVISED -			CONTRACT NO. 89779					
	PLOT DATE = 2/28/2023	CHECKED - S.T.M./S.W.M.	REVISED -			SHEET NO. 12 OF 17 SHEETS					
				ILLINOIS FED. AID PROJECT NO. ZH0Y(896)							



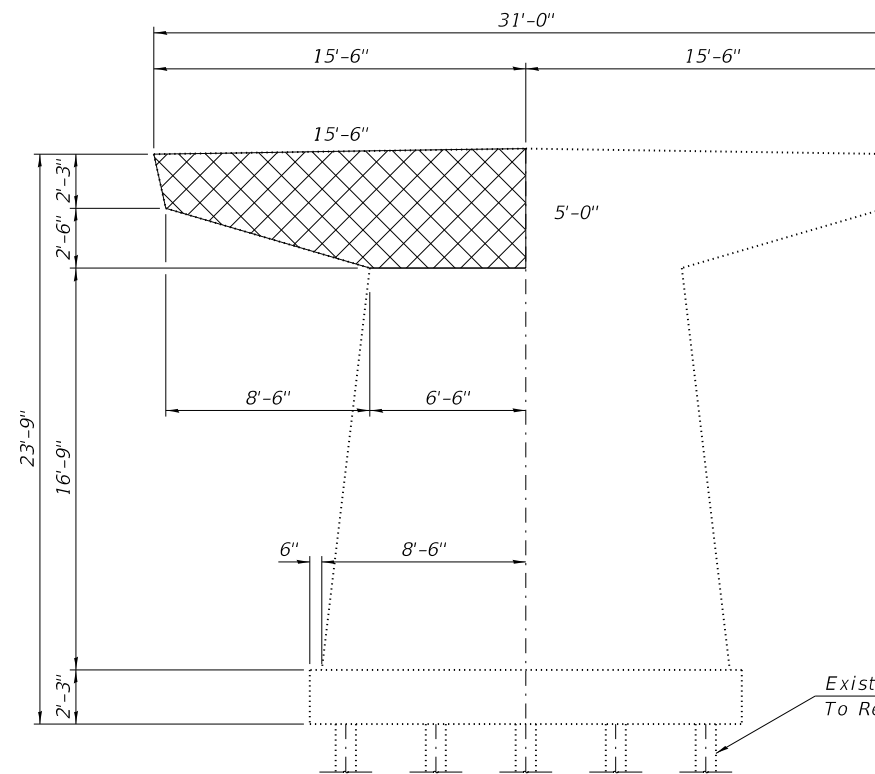
EAST END VIEW
(Looking West)



NORTH FACE ELEVATION
(Looking South)



WEST END VIEW
(Looking East)

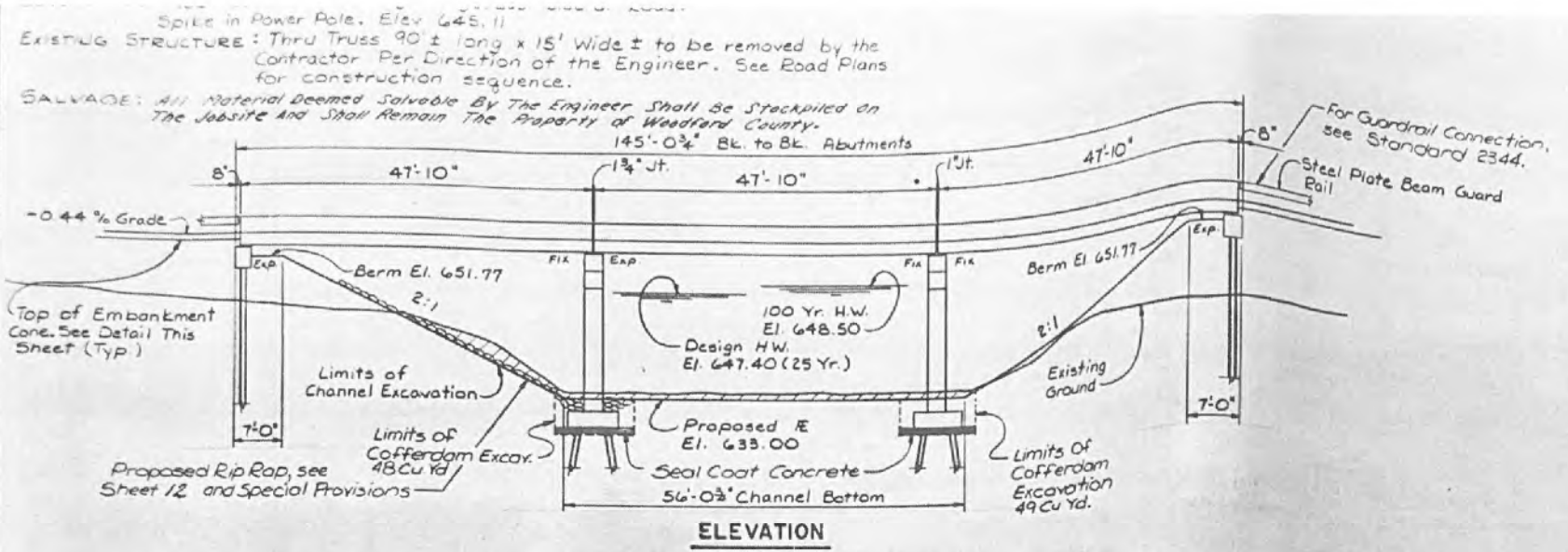


SOUTH FACE ELEVATION
(Looking North)



Structural Repair of
Concrete ($\leq 5"$)

FILE NAME = 190013-shi-bridge - SN 102-3130.dgn	USER NAME = smierzwa	DESIGNED - J.R.B	REVISED -	STATE OF ILLINOIS WOODFORD COUNTY HIGHWAY DEPARTMENT	CONCRETE REPAIR - PIER 2 STRUCTURE NO. 102-3130	F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
HAMPTON, LENZINI AND RENWICK, INC. 3035 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 ILLINOIS PROFESSIONAL DESIGN FIRM LS / PE / SE CORP. 184.000959		CHECKED - S.T.M.	REVISED -			363	18-00166-00-BR	WOODFORD	65	23
PLOT SCALE =		DRAWN - R.D.H.	REVISED -			CONTRACT NO. 89779				
PLOT DATE = 2/28/2023		CHECKED - S.T.M./S.W.M.	REVISED -			SHEET NO. 13 OF 17 SHEETS				
				ILLINOIS FED. AID PROJECT NO. ZH0Y(896)						



WATERWAY DATA

Drainage Area	186	Sq. Mi. **
Existing Opening	1036	Sq. Ft.
Required Opening (25 Yr.)	1130	Sq. Ft.
Proposed Opening	1130	Sq. Ft.
Design Frequency Discharge (25 Yr.)	5310	C.F.S.
Created Head For Design Flood	0.4	Ft. **
100 Year Discharge	11,130	C.F.S. **
Created Head For 100 Year Flood	0.5	Ft. **

** Main Structure and Overflow Structure-Combined

GENERAL NOTES

Standard Specifications for Road and Bridge Construction, adopted by the Illinois Department of Transportation July 1, 1976 shall apply.

All structural steel shall be shop painted with two coats of basic lead silico chromate paint.

Protective Coat shall not be applied to surfaces to which Waterproofing Membrane System is applied.

For Soil Borings See Special Provisions.

All Transverse Tie Assemblies (nuts, bolts, and washers) shall be Hot Dipped Galvanized in accordance with AASHTO Designation M 232.

The Contractor shall drive two test piles in a permanent location as directed by the Engineer before ordering the remainder of piles. The test piles are to be located, one at Pier #1 and one at South Abutment.

It is the Contractor's responsibility to comply with all applicable Local, State and Federal Laws, including Occupational Safety and Health Act (OSHA) and Regulations adopted pursuant thereto. The Contractor shall be responsible for construction means, methods, techniques, sequences and procedures; and for safety precautions and programs, including the safety and adequacy of shoring, formwork, sheeting, safety railing, scaffolding and equipment. Ralph Hahn and Associates Consulting & Design Engineers Inc. assumes no responsibility or liability in these matters whatsoever.

The embankment configuration shown shall be the minimum embankment that must be constructed prior to construction of the abutments.

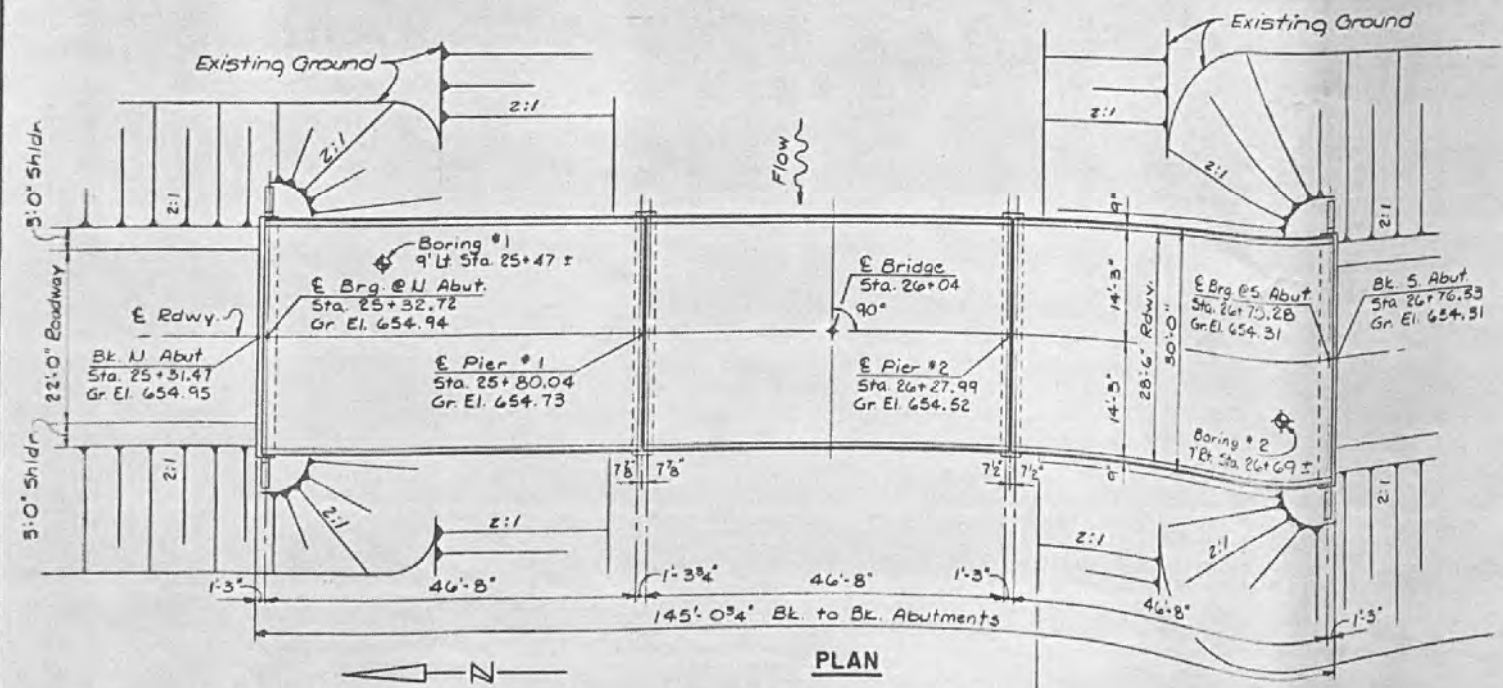
For Proposed Channel Configuration see Sheet No. 12

Expansion guards which are not cast in the precast unit shall be fabricated and erected in accordance with Article 503.07(c) of the Standard Specifications and are included in quantity of structural steel.

The top surface of the beams shall be finished in accordance with Article 505.06 of the Standard Specifications except that the surface shall not be roughened by brooming. The finished surface shall be free of depressions or high spots with sharp corners.

The concrete rail section above the mandatory construction joint at the top of the slab shall be constructed of Class X concrete, except the aggregates shall conform to the requirements of Normal Concrete.

Provisions shall be made to allow cofferdam to flood when water exceeds elevation 635.00.



DESIGN STRESSES

PRECAST PRESTRESSED CONCRETE

f'_c = 5,000 p.s.i.

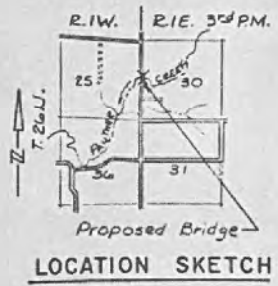
DESIGN STRESSES

f_c = 1400 p.s.i. (Sub)

f_s = 20,000 p.s.i. (Reinf.)

n = 8.5

APPROVED
 FOR STRUCTURAL ADEQUACY ONLY
 Frank L. Johnson
 Structural Engineer



TOTAL BILL OF MATERIALS

Item	Unit	Quantity
* Channel Excavation	Cu. Yd.	551
Bituminous Conc Surface Course - Class II-MIX. D	Ton	43
Removal of Existing Structures	Each	1
Cofferdam Excavation	Cu. Yd.	97
Cofferdams	Each	2
Protective Coat	Sq. Yd.	91
Class X Concrete	Cu. Yd.	1263
Seal Coat Concrete	Cu. Yd.	179
PP Concrete Deck Beams 21" Depth	Sq. Ft.	2305
Waterproofing Membrane System	Sq. Yd.	404
Furnishing Erecting Structural Steel	Pound	4020
Reinforcement Bars	Pound	15800
Furnished Steel Pipes HP10x42	Lin. Ft.	239
Driving Steel Piles	Lin. Ft.	539
Test Pile Steel HP10x42	Each	2
Name Plates	Each	1
* Dumped Rip Rap	Ton	102
* Preformed Joint Sealer 2"	Lin. Ft.	51
* Portland Cement Mortar Furring Course	Lin. Ft.	1291

* See Special Provisions

STATION 26+04
 BRIDGE OVER PANTHER CREEK
 BUILT 1978
 FAS RT. 363 SEC. 77-00059-00-BR
 FA PROJ. BR-5-363 (104)
 LOADING HS 20
 NBIP 102-3130

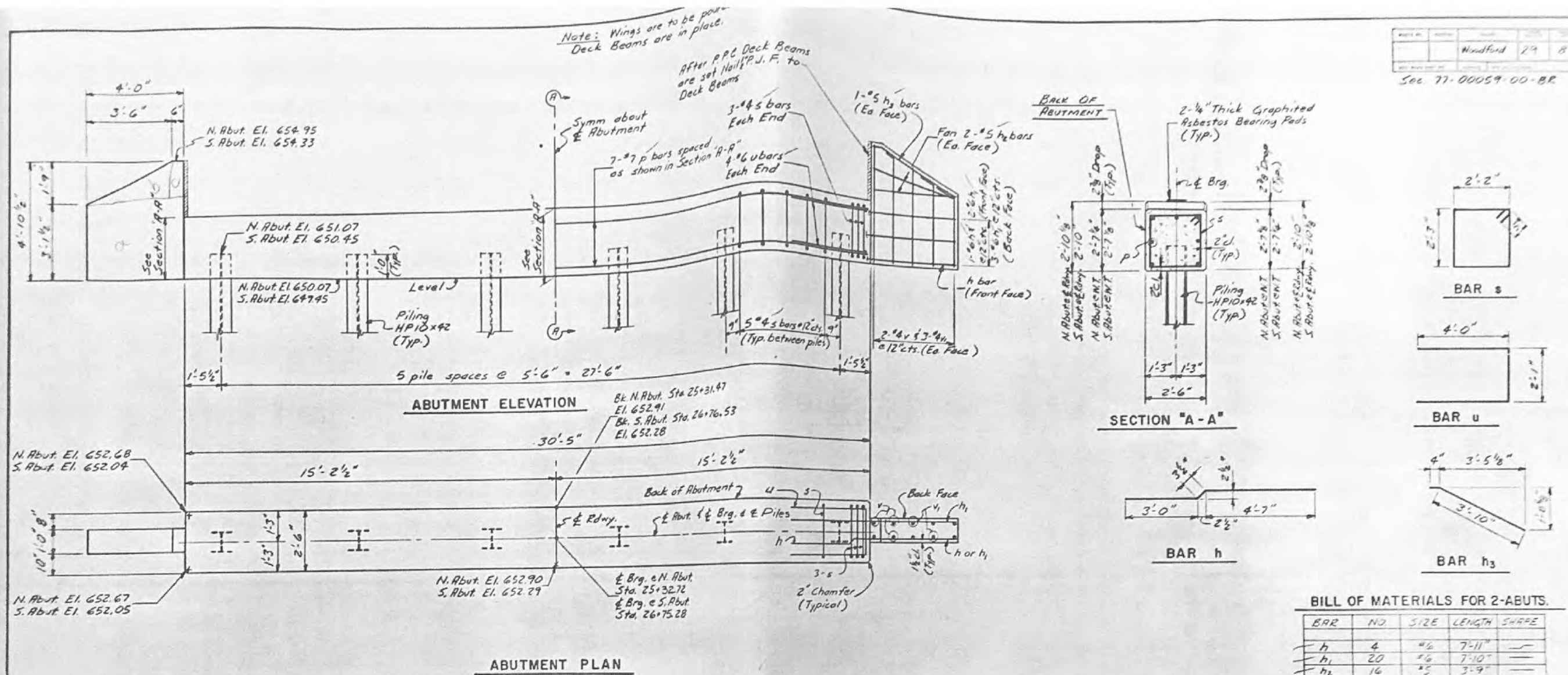
NAME PLATE
 (See Standard 2113)



Frank L. Johnson
 ILLINOIS STRUCTURAL NO. 3100

LOADING HS20 44
 RALPH HAHN & ASSOCIATES
 CONSULTING & DESIGN ENGINEERS

**GENERAL PLAN & ELEVATION
 BRIDGE OVER PANTHER CREEK
 PALESTINE ROAD DISTRICT
 SECTION 77-00059-00-BR**



BILL OF MATERIALS FOR 2-ABUTS.

BAR	NO	SIZE	LENGTH	SHAPE
h	4	#6	7'-11"	—
h ₁	20	#6	7'-10"	—
h ₂	16	#5	3'-9"	—
h ₃	8	#5	4'-2"	—
p	14	#7	30'-2"	—
s	62	#4	9'-8"	□
u	16	#6	10'-1"	□
v	16	#4	4'-8"	—
w	24	#4	3'-10"	—

Class X Concrete Cyt'd 177
 Reinforcement Bars Pound 2000
 For Steel Pile HP10x42 Lin. Ft. 330
 Test Pile Steel HP10x42 Each 1

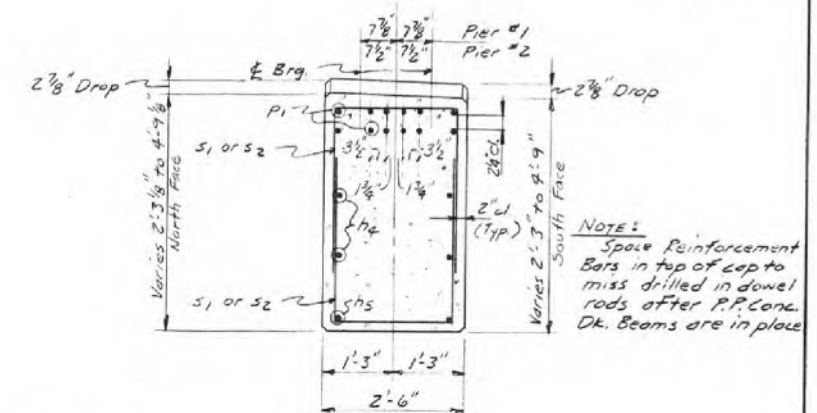
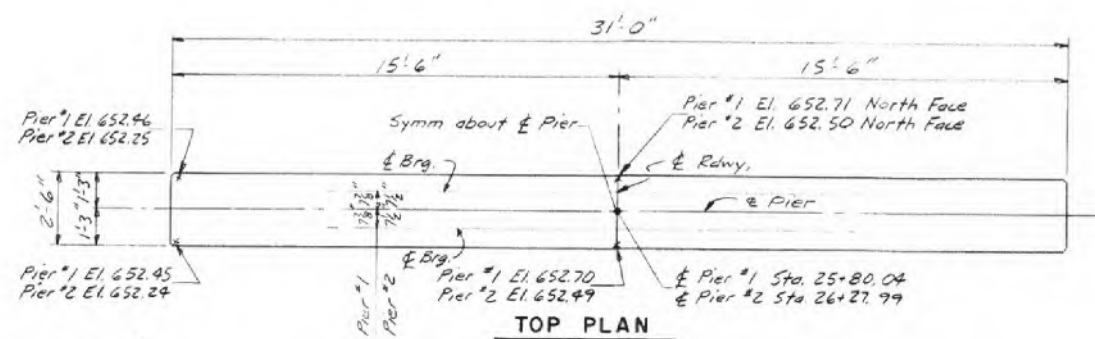
PILE DATA

	North Abut	South Abut
Type	HP10x42	HP10x42
Capacity	30 Tons	30 Tons
Est Length	30'	30'
No. Req'd.	6	5 + 1 Test Pile

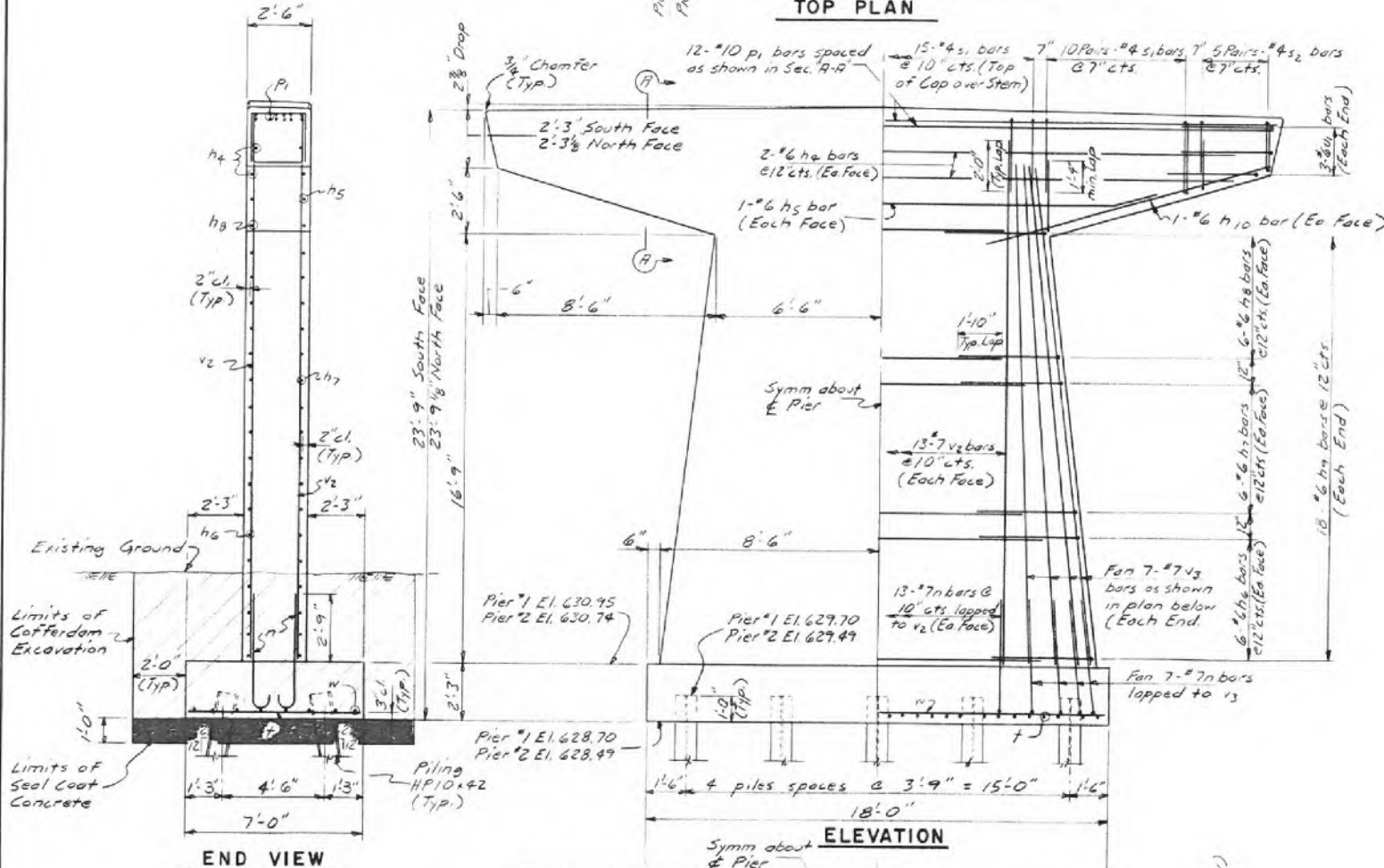
ABUTMENTS
 BRIDGE OVER PANTHER CREEK
 PALESTINE ROAD DISTRICT
 SECTION 77-00059-00-BR

PROJECT NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	Woodford	29	9	

Sec 77-00059-00-82



NOTE:
Space Reinforcement Bars in top of cap to miss drilled in dowel rods after P.P. Conc. Dk. Beams are in place

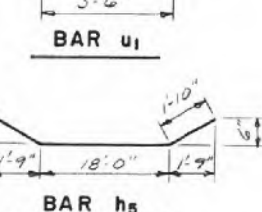
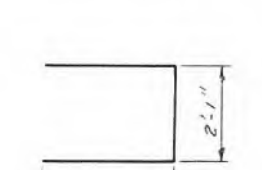
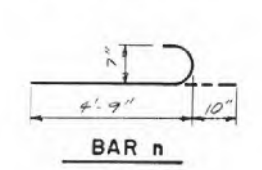
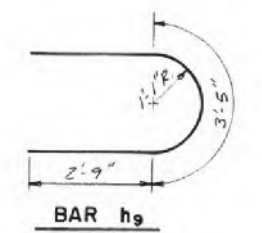
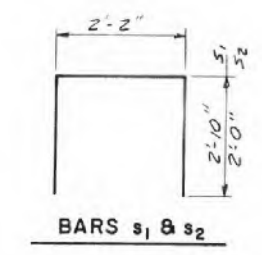


END VIEW

PILE DATA

Type	Pier #1	Pier #2
Capacity	45 Tons	45 Tons
Est. Length	11'	10'
No. Req'd.	9+1 Test Pile	10

(Pile Tip El. 619.0)



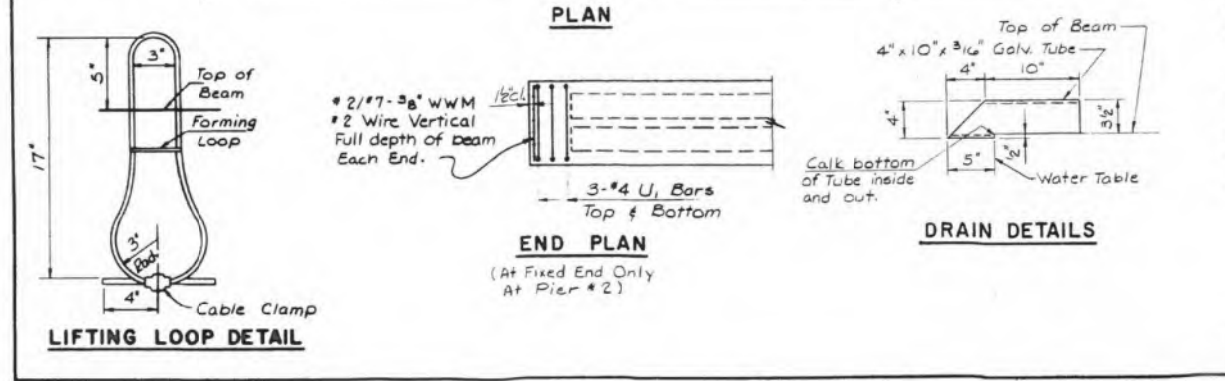
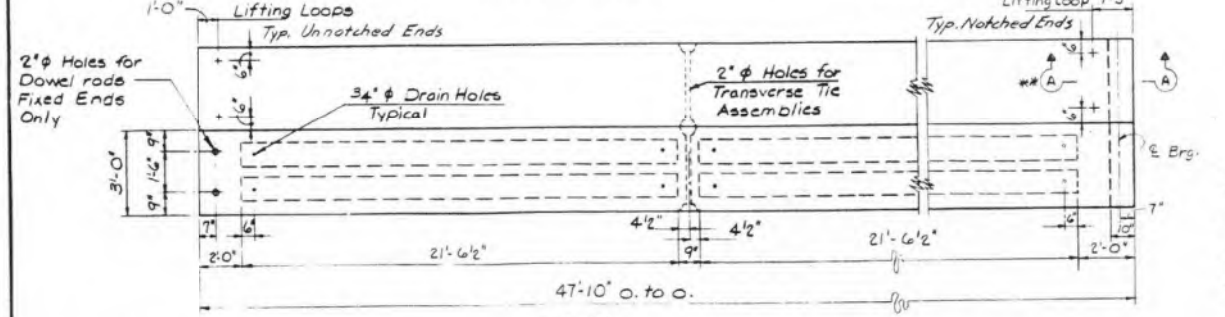
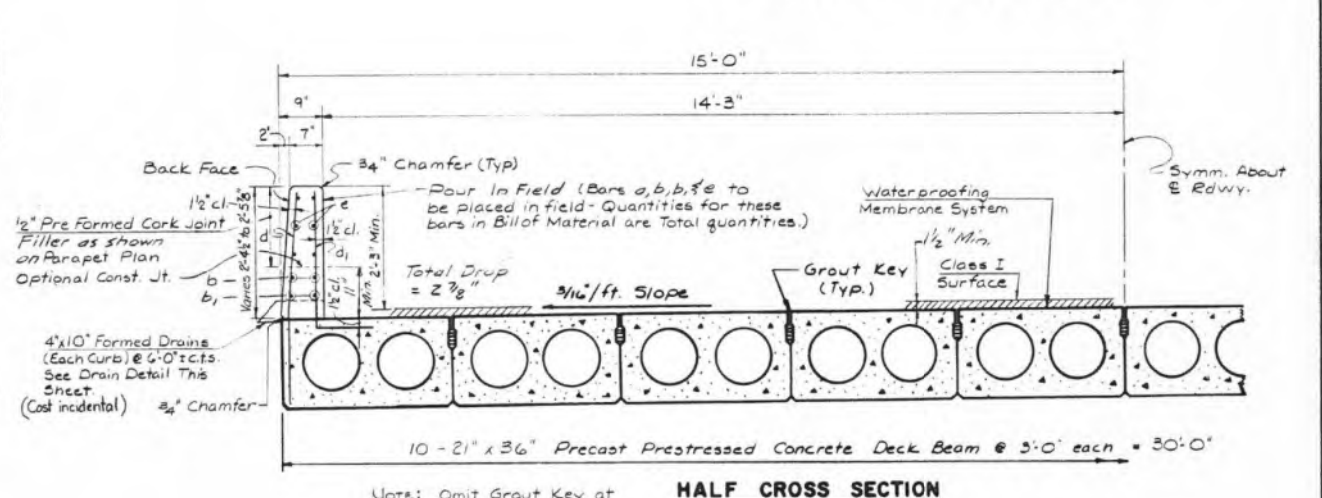
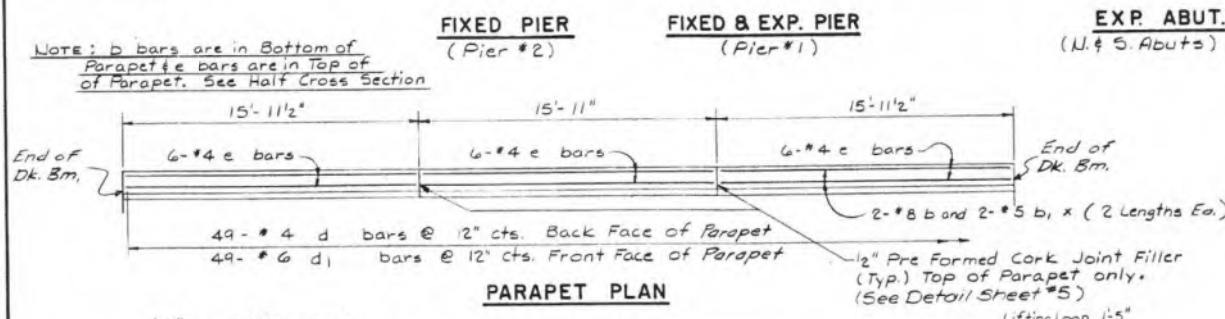
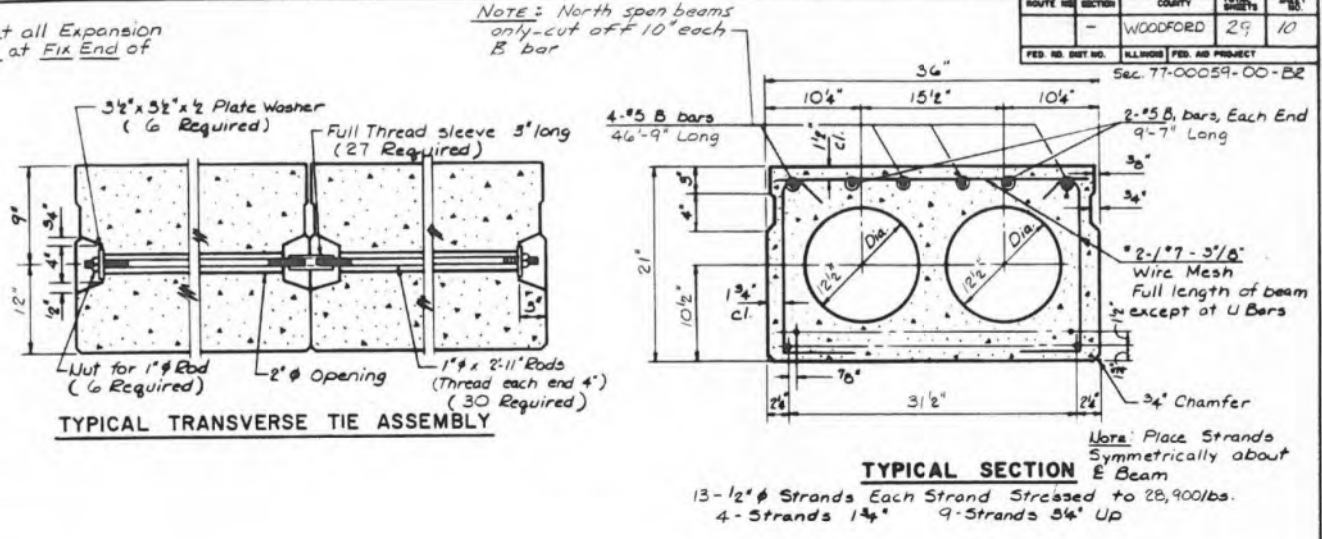
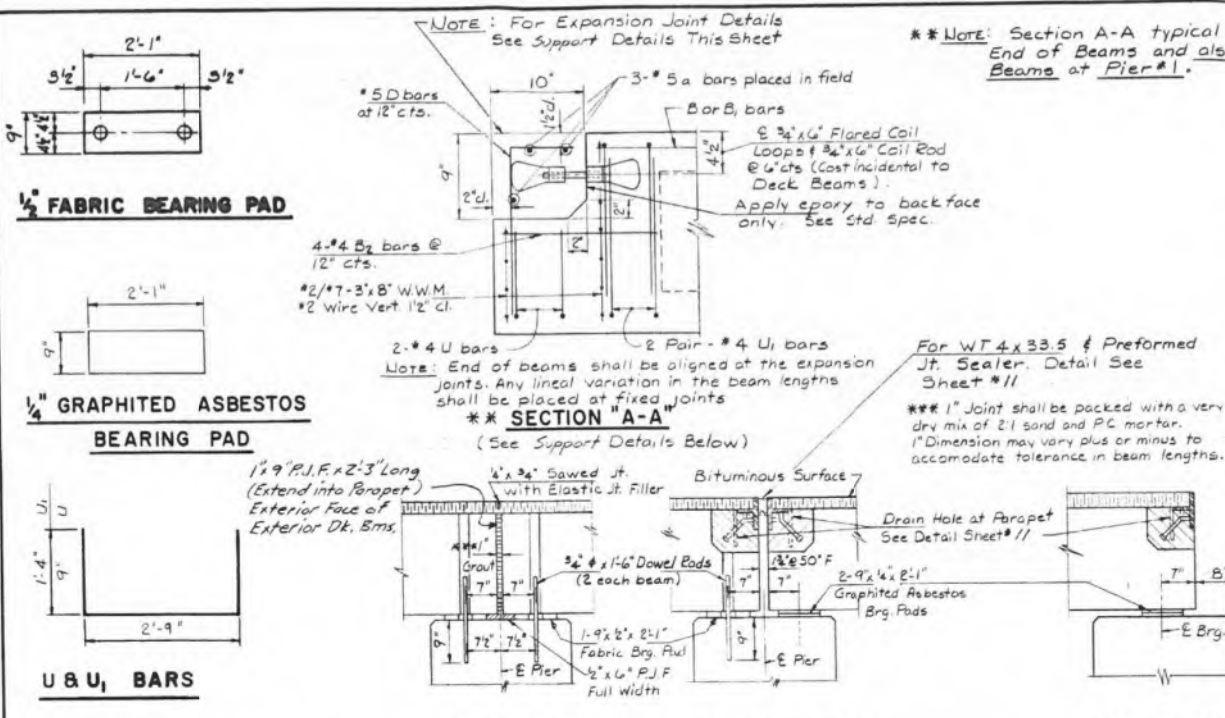
BILL OF MATERIAL FOR 2-PIERS

BAR	NO.	SIZE	LENGTH	SHAPE
h4	8	#6	27'-6"	—
h5	4	#6	21'-8"	—
h6	24	#6	12'-8"	—
h7	24	#6	11'-2"	—
h8	24	#6	9'-10"	—
h9	72	#6	8'-11"	—
h10	8	#6	11'-6"	—
n	80	#7	57"	—
p1	24	#10	30'-6"	—
s1	110	#4	7'-10"	U
s2	90	#4	6'-2"	U
t	56	#6	6'-8"	—
u1	12	#6	9'-1"	—
v2	52	#7	20'-6"	—
v3	28	#7	19'-6"	—
w	16	#5	17'-8"	—
Class A Concrete		Cu. Yd.	89.6	
Reinforcement Bars		Pound	11900	
Steel Piles (HP10x42)		Lin Ft.	209	
Test Pile Steel (HP10x42)		Each	1	
Seal Coat Concrete		Cu. Yd.	17.9	

PIERS
BRIDGE OVER PANTHER CREEK
PALESTINE ROAD DISTRICT
SECTION 77-00059-00-BR
WOODFORD COUNTY
STATION 26+04
102-3130

BR-77-102

ROUTE NO. SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	WOODFORD	27	10
FED. RD. DIST. NO.	ILLINOIS FED. AID PROJECT	Sec. 77-00059-00-BR	



GENERAL NOTES

Prestressing steel shall be non-galvanized high strength, stress-relieved 7-wire strand, Grade 270. The nominal diameter shall 7/8" and the nominal cross-sectional area shall be 0.153 square inch.

Lifting loops shall be 5/8" diameter, 6x25 class wire rope with fiber core and shall have a minimum ultimate tensile strength of 33,000 lbs.

The 1" rods in the transverse tie assembly shall be tightened to a snug fit and the threads set. Pockets that receive transverse tie bar on outside shall be filled with grout after transverse tie assembly is in place.

Longitudinal shear keys shall be packed with a very dry mix of 2:1 sand and PC mortar.

After beams have been erected, holes for dowel anchors shall be drilled into the substructure and the anchor dowels shall be grouted in place.

Cost of reinforcement and accessories cast into the beam, of bearing pads, & grouting longitudinal shear keys is included in unit price bid for "Prestressed Concrete Deck Beams".

After fabrication the transverse tie assemblies (Tie rods, Nuts, Washers and Sleeves) shall be Hot Dipped Galvanized in accordance with AASHTO M 232 Re: Deck Beams.

Steel for Expansion Guards shall be structural steel AASHTO designation M 183.

BAR D
 1'-6" x 1'-0"

BAR d₁
 2'-4" x 1'-0"

BAR d
 2'-0" x 1'-0"

*****NOTE:** Quantity includes only a, b, b₁ & e bars.

BILL OF MATERIAL

BAR	NO.	SIZE	LENGTH	SHAPE
B	4	#5	46'-9"	
B ₁	4	#5	9'-7"	
B ₂	4	#4	1'-9"	
D	4	#5	2'-1"	
U	2	#4	4'-3"	
U ₁	10	#4	5'-5"	
a	12	#5	29'-9"	
b	24	#8	25'-0"	
b ₁	24	#5	24'-6"	
d	49	#4	3'-9"	
d ₁	49	#6	3'-4"	
e	108	#2	15'-9"	
Class X Concrete Cu. Yd.				20.0
Reinforcement Bars Pounds				3,720
P.P. Conc. Dk. Brm. Sq. Ft.				4,305

***Reinforcing List Is For One Unit Only In Precast.
 Note: d₁ & d₂ shall be placed in the Precast Plant In Exterior Beams Only.

SUPERSTRUCTURE
 BRIDGE OVER PANTHER CREEK
 PALESTINE ROAD DISTRICT
 SECTION 77-00059-00-BR
 WOODFORD COUNTY
 STATION 26+04
 102-3130 BR-77-102 II

BENCHMARK: Chiseled "□" On S.W. WW of Bridge, 18' Rt., Sta. 30+18 Elev. 652.03

See sheet 2 of 14 for General Notes and Bill of Material.

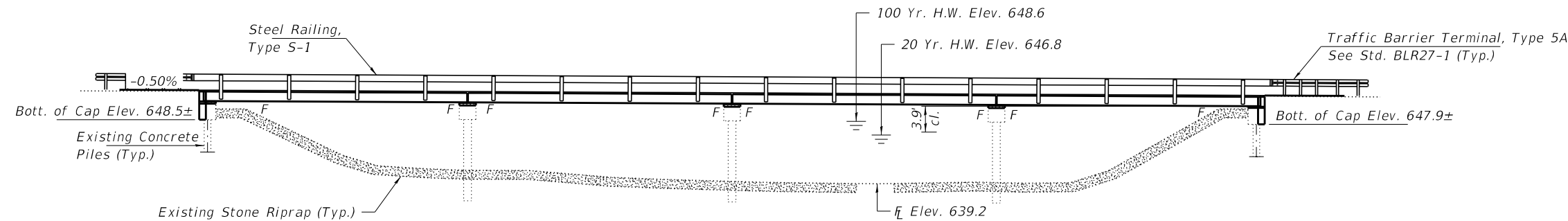
EXISTING STRUCTURE NO. 102-3131: Four span PPC deck beam superstructure supported on concrete pile bent abutments with concrete caps and wingwalls and exposed pile bent piers with concrete caps. 160.3' bk.-bk. abuts., 30.0' o.-o. deck, and no skew.

Structure closed to traffic during construction.

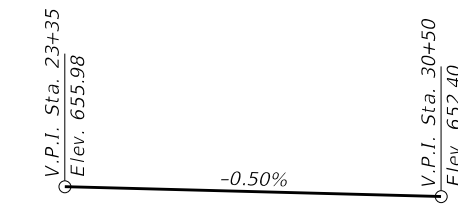
Salvage: Existing abutments and piers to be re-used in place.
Salvage and reinstall existing Name Plate.

INDEX OF STRUCTURE SHEETS

1. General Plan & Elevation
2. General Data
3. 17"x36" PPC Deck Beam - Spans 1, 2, 3 & 4
4. 17"x36" PPC Deck Beam Details - Spans 1, 2, 3 & 4
5. Superstructure Details
6. Steel Railing, Type S-1
7. North Abutment
8. South Abutment
9. Pier 1
10. Pier 2
11. Pier 3
- 12-14. Existing Plans



ELEVATION

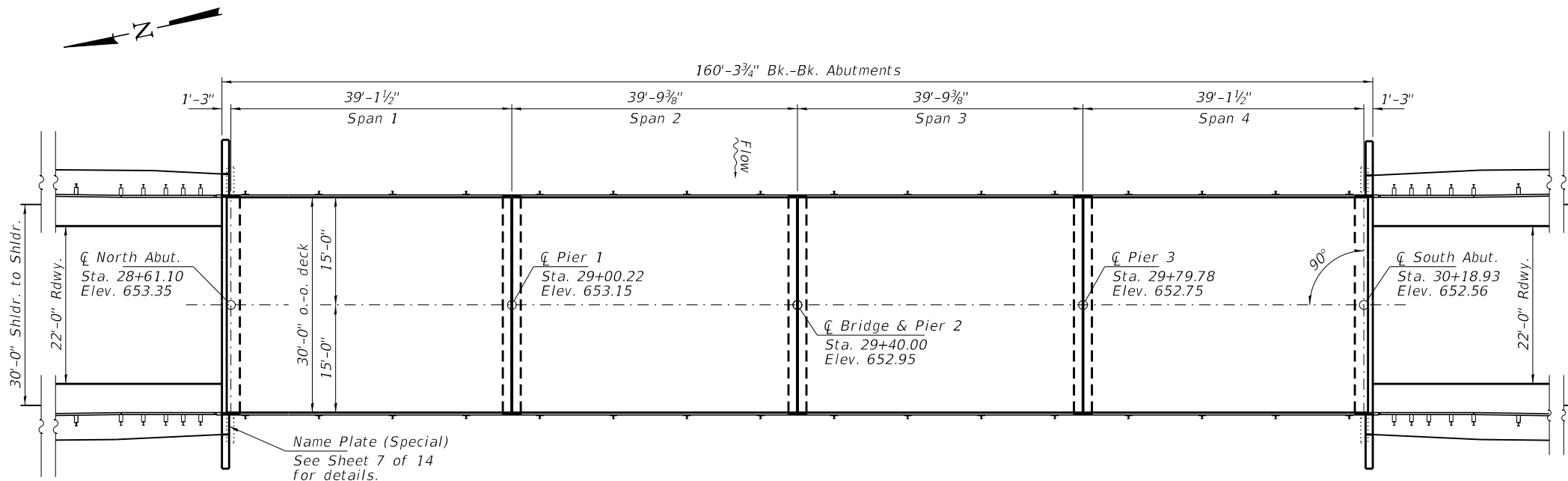


PROFILE GRADE
C.H. 7

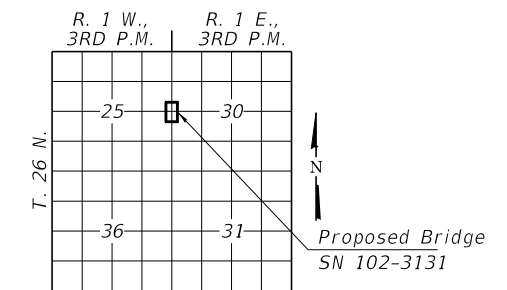
PANTHER CREEK
REBUILT 202_ BY
WOODFORD COUNTY
SEC. 18-00166-00-BR
F.A.S. 363 / C.H. 7
STR. NO. 102-3131
LOADING HL-93

NAME PLATE
See Std. 515001

Existing Name Plate shall be cleaned and relocated next to the Proposed Name Plate. Cost included with Name Plates (Special).



PLAN



LOCATION SKETCH

DESIGN STRESSES

FIELD UNITS (NEW CONST.)

$f'_c = 3,500$ psi
 $f_y = 60,000$ psi (Reinf.)

PRECAST PRESTRESSED UNITS

$f'_c = 6,000$ psi
 $f'_{ci} = 5,000$ psi
 $f_{pu} = 270,000$ psi ($1/2$ " \emptyset low lax. strands)
 $f_{pbt} = 201,960$ psi ($1/2$ " \emptyset low lax. strands)
 $f_y = 60,000$ psi (Reinf.)

EXISTING CONSTRUCTION

$f_c = 1,400$ psi (Substructure)
 $f_s = 20,000$ psi (Reinf.)

DESIGN SPECIFICATIONS

2020 AASHTO LRFD Bridge Design Specifications, 9th Edition with all interims.

LOADING HL-93

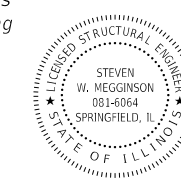
Allow 50#/sq. ft. for future wearing surface.

LOADING HS 20-44

Existing Conditions - Substructure

I certify that to the best of my knowledge, information and belief, this bridge design is structurally adequate for the design loading shown on the plans. The design is an economical one for the style of structure and complies with requirements of the current "AASHTO LRFD Specifications."

Steven W. Megginson 2/27/2023
ILLINOIS STRUCTURAL ENGINEER NO. 081-6064



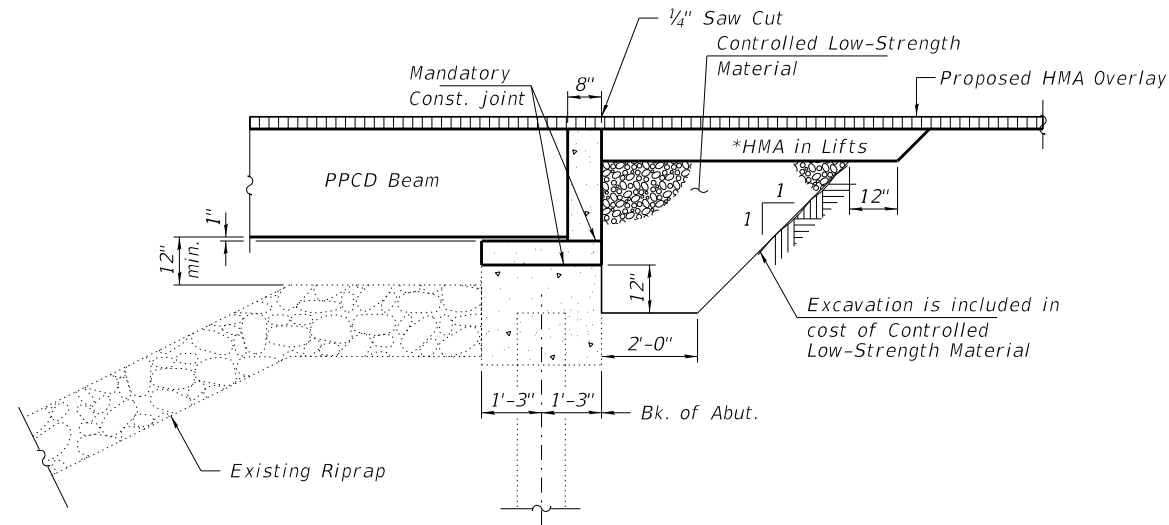
Expires 11-30-2024

**GENERAL PLAN & ELEVATION
OVERFLOW STRUCTURE
AT PANTHER CREEK
SECTION 18-00166-00-BR
F.A.S. 363 / C.H. 7
WOODFORD COUNTY
STATION 29+40.00
STRUCTURE NO. 102-3131**

FILE NAME = 190013-shi-bridge - SN 102-3131.dgn	USER NAME = smierzwa	DESIGNED - J.R.B.	REVISED -	STATE OF ILLINOIS WOODFORD COUNTY HIGHWAY DEPARTMENT	GENERAL PLAN & ELEVATION STRUCTURE NO. 102-3131	F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
HAMPTON, LENZINI AND RENWICK, INC. 3085 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 ILLINOIS PROFESSIONAL DESIGN FIRM LS / PE / SE CORP. 184.009959	PLOT SCALE =	CHECKED - S.T.M.	REVISED -			363	18-00166-00-BR	WOODFORD	65	28	
PLOT DATE = 2/28/2023		DRAWN - R.D.H.	REVISED -			CONTRACT NO. 89779					
		CHECKED - S.T.M./S.W.M.	REVISED -			ILLINOIS FED. AID PROJECT NO. ZH0Y(896)					

GENERAL NOTES

Reinforcement bars designated (E) shall be Epoxy Coated.
 All proposed construction activities shall be in accordance with Nationwide Permit number 14 of the Department of the Army authorized under Section 404 of the Clean Water Act. The IEPA has issued Section 401 Water Quality Certification for this activity. See Special Provisions for conditions.
 Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.
 The PPC Deck Beams shall be applied with a Protective Coat as specified in the Special Provisions. This work shall be included in the cost with PPC Deck Beams.



SECTION THRU ABUTMENT

*To be included with roadway plans (See Special Provisions)

DESIGN SCOUR ELEVATION TABLE

Event/Limit	Design Scour Elevations (ft.)					Item
	N. Abut.	Pier 1	Pier 2	Pier 3	S. Abut.	
Q100	649.0	625.9	624.4	626.8	648.0	113
Design	649.0	640.7	639.2	638.7	648.0	7

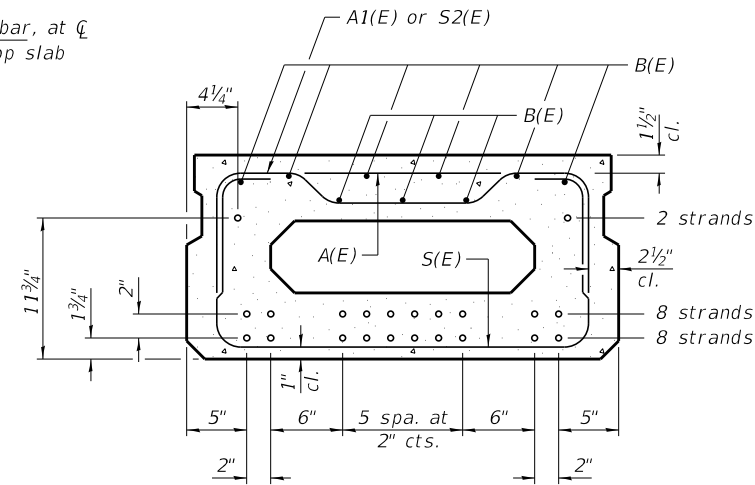
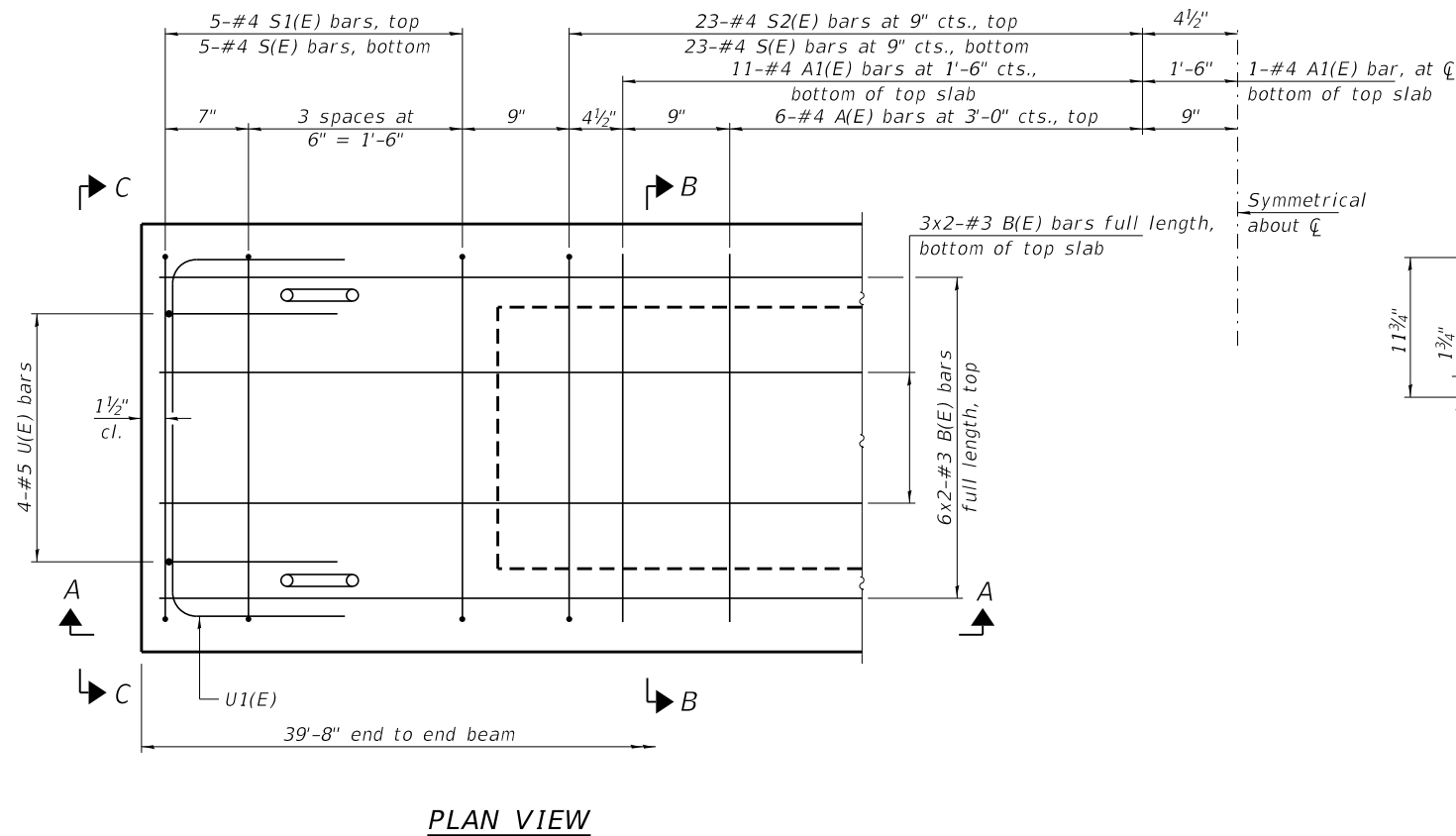
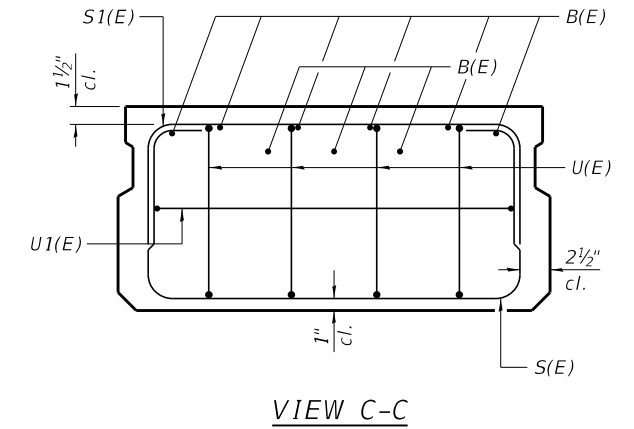
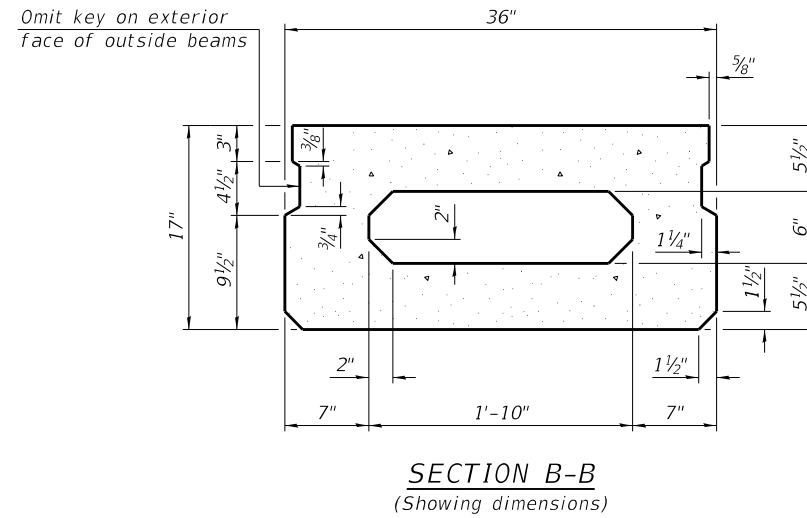
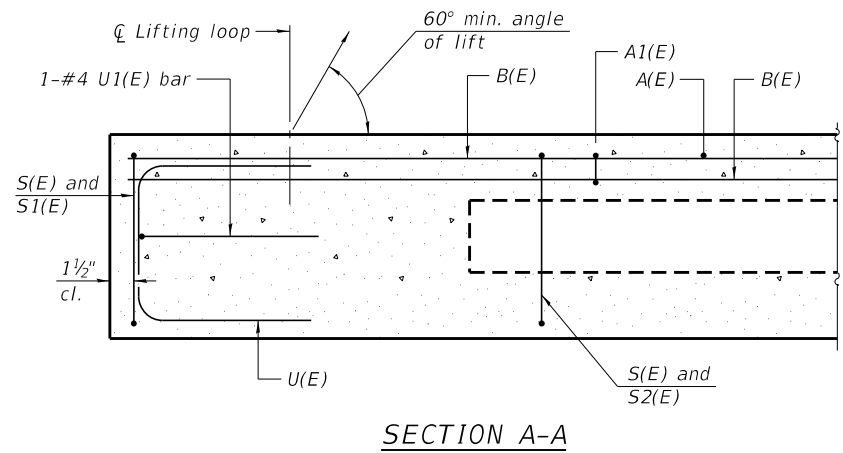
WATERWAY INFORMATION

Flood Event		Discharge (cfs)		Opening Sq. Ft.		Natural H.W.E.	Head - Ft.		Headwater El.	
		Exist.	Prop.	Exist.	Prop.		Exist.	Prop.	Exist.	Prop.
10	Main Channel			870	870					
	Relief Struct.	8450	8450	640	640	645.9	0.8	0.8	646.7	646.7
	Total			1510	1510					
20	Main Channel			950	950					
	Relief Struct.	10530	10530	740	740	646.8	0.9	0.9	647.7	647.7
	Total			1690	1690					
100	Main Channel			1130	1130					
	Relief Struct.	15400	15400	960	960	648.6	1.1	1.1	649.7	649.7
	Total			2090	2090					
200	Main Channel			1200	1200					
	Relief Struct.	17600	17600	1060	1060	649.3	1.0	1.0	650.3	650.3
	Total			2260	2260					
500/ Overtopping	Main Channel			1290	1290					
	Relief Struct.	20500	20500	1180	1180	650.2	1.0	1.0	651.2	651.2
	Total			2470	2470					

Drainage Area = 178.5 Sq. Mi. Existing Low Grade Elev. 651.0 @ Sta. 33+00 Proposed Low Grade Elev. 651.0 @ Sta. 33+00
 10 Year Velocity through Existing Bridge = 5.6 fps 10 Year Velocity through Proposed Bridge = 5.6 fps

TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Hot-Mix Asphalt Surface Cse., 1L-9.5, Mix "C", N50	Ton	55		55
Removal of Existing Superstructures	Each			1
Concrete Removal	Cu. Yd.		4.7	4.7
Concrete Structures	Cu. Yd.		10.8	10.8
Precast Prestressed Conc. Deck Beams (17" Depth)	Sq. Ft.	4,760		4,760
Reinforcement Bars, Epoxy Coated	Pound		1,400	1,400
Steel Railing, Type S1	Foot		324	324
Waterproofing Membrane System	Sq. Yd.	535		535
Portland Cement Mortar Fairing Course	Foot	360		360
Controlled Low-Strength Material	Cu. Yd.		20	20
Name Plates (Special)	Each		1	1
Asbestos Bearing Pad Removal	Each		88	88
Structural Repair of Concrete (Depth ≤ 5")	Sq. Ft.		5	5



SECTION B-B
(Showing reinforcement and permissible strand locations)
Note:
Place the number of strands specified in each row symmetrically about the centerline of beam in the permissible strand locations shown.

BAR LIST
ONE BEAM ONLY
(For information only)

Bar	No.	Size	Length	Shape
A(E)	12	#4	2'-7"	—
A1(E)	23	#4	2'-10"	—
B(E)	18	#3	20'-6"	—
S(E)	56	#4	5'-9"	□
S1(E)	10	#4	4'-3"	□
S2(E)	46	#4	4'-6"	□
U(E)	8	#5	3'-8"	□
U1(E)	2	#4	5'-0"	□

Note:
See sheets 4 & 5 of 14 for additional details and Bill of Material.

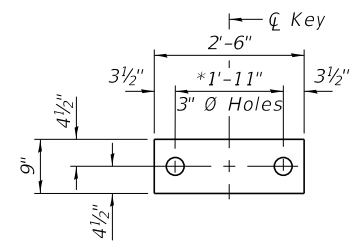
Note:
Spacing of S(E) and S2(E) bars may be adjusted up to 4" in the immediate area of the transverse tie diaphragms to miss the block outs for the transverse ties.
Bars indicated thus 6x2-#3 etc. indicates 6 lines of bars with 2 length per line.

MINIMUM BAR LAP
#3 bar = 1'-6"

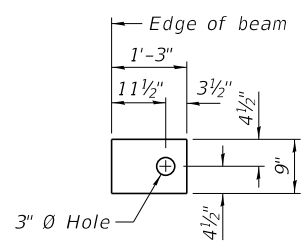
PD-1736-0

1-1-2020

FILE NAME = 190013-shl-bridge - SN 102-3131.dgn	USER NAME = smierzwa	DESIGNED - J.R.B.	REVISED -	STATE OF ILLINOIS WOODFORD COUNTY HIGHWAY DEPARTMENT	17" x 36" PPC DECK BEAM - SPANS 1, 2, 3 & 4 STRUCTURE NO. 102-3131	F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
HAMPTON, LENZINI AND RENWICK, INC. 3035 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 ILLINOIS PROFESSIONAL DESIGN FIRM L.S./P.E./S.E. CORP. 184.000959	PLOT SCALE =	CHECKED - S.T.M.	REVISED -			363	18-00166-00-BR	WOODFORD	65	30
PLOT DATE = 2/28/2023	CHECKED - S.T.M./S.W.M.	DRAWN - R.D.H.	REVISED -			CONTRACT NO. 89779				
SHEET NO. 3 OF 14 SHEETS						ILLINOIS FED. AID PROJECT NO. ZH0Y(896)				



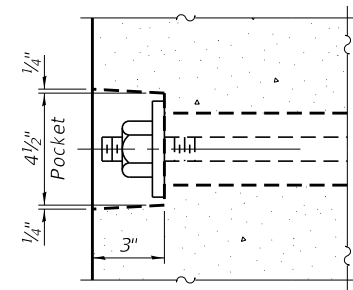
FABRIC BEARING PAD
(Interior - 72 req'd)



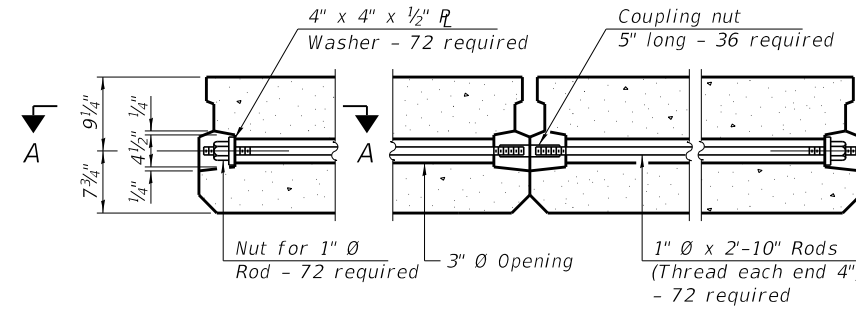
FABRIC BEARING PAD
(Exterior - 16 req'd)

FIXED

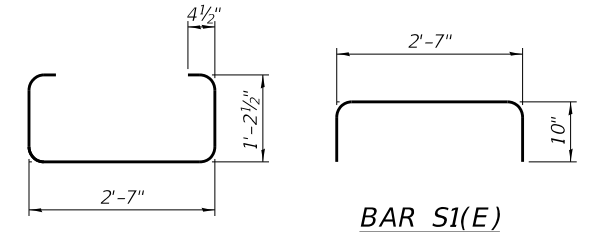
Notes:
All bearing pads shall be 1" thick.



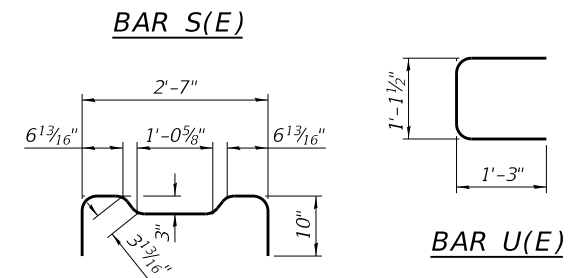
SECTION A-A



TYPICAL TRANSVERSE TIE ASSEMBLY

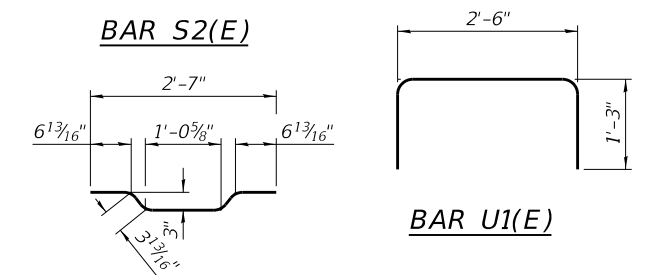


BAR S1(E)



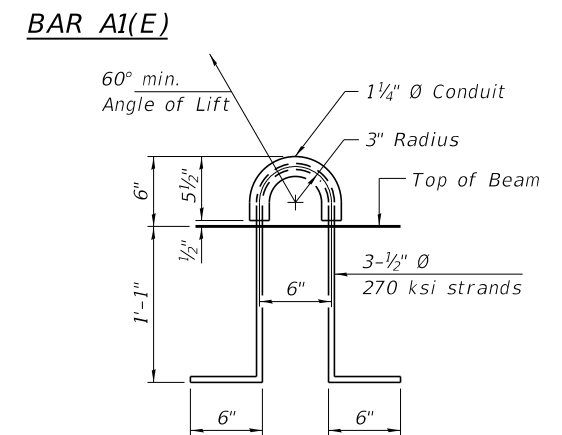
BAR S(E)

BAR U(E)

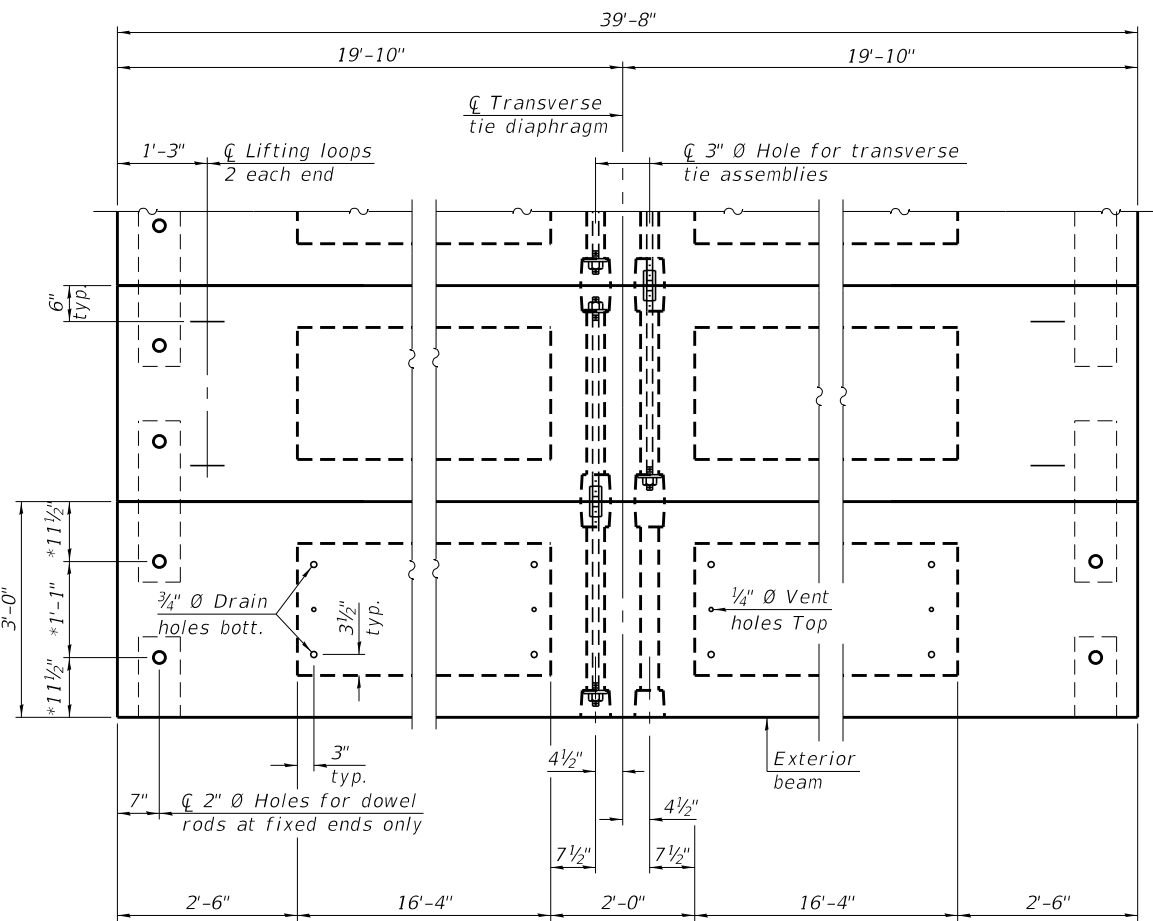


BAR S2(E)

BAR U1(E)



LIFTING LOOP DETAIL



PLAN VIEW

NOTES

- Prestressing steel shall be uncoated high strength, low relaxation 7-wire strand, Grade 270. The nominal diameter shall be 1/2" and the nominal cross-sectional area shall be 0.153 sq. in.
- The 1" rods in the transverse tie assembly shall be tightened to a snug fit and the threads set. Pockets on exterior faces of bridge shall be filled with grout after transverse tie assembly is in place.
- Two 1/8" fabric adjusting shims of the dimensions of the exterior bearing pad shall be provided for each bearing pad location.
- A minimum 2 1/2" diameter lifting pin shall be used to engage the lifting loops during handling.
- Corrosion Inhibitor, per Article 1020.05(b)(10) and 1021.07 of the Standard Specifications, shall be used in the concrete for precast prestressed concrete deck beams.
- Compressive strength of prestressed concrete, f'c, shall be 6000 psi.
- Compressive strength of prestressed concrete at release, f'ci, shall be 5000 psi.

BILL OF MATERIAL

Precast Prestressed Conc. Deck Bms. (17" depth)	Sq. Ft.	4,760
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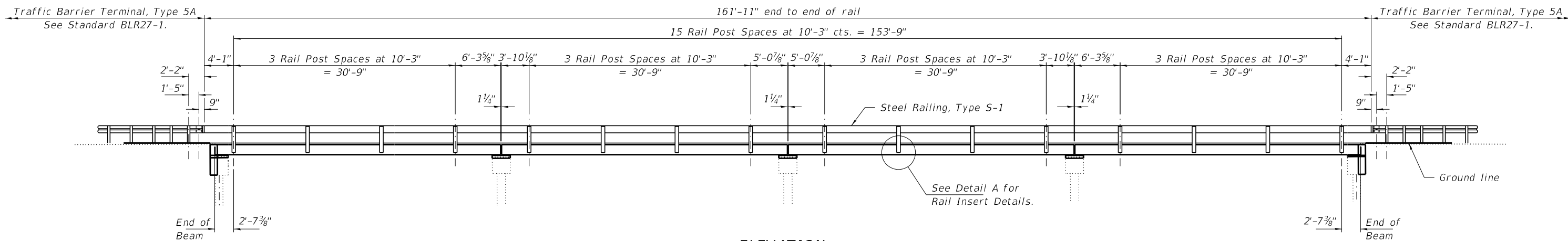
Note:
Connect beams in pairs with the transverse tie configuration shown.

*Dowel rod hole locations modified to miss existing dowels.

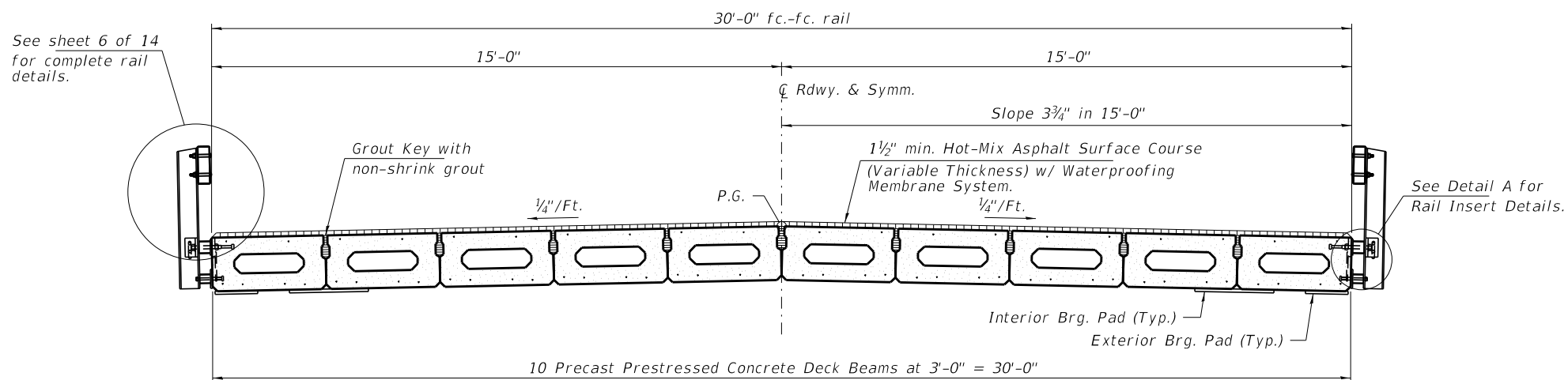
PDD-1736-0 1-1-2020

FILE NAME = 190013-shl-bridge - SN 102-3131.dgn	USER NAME = smierzwa	DESIGNED - J.R.B.	REVISED -	STATE OF ILLINOIS WOODFORD COUNTY HIGHWAY DEPARTMENT	17" x 36" PPC DECK BEAM DETAILS - SPANS 1, 2, 3 & 4 STRUCTURE NO. 102-3131	F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
HAMPTON, LENZINI AND RENWICK, INC. 3035 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 ILLINOIS PROFESSIONAL DESIGN FIRM L.S./P.E./S.E. CORP. 184.000959	PLOT SCALE =	CHECKED - S.T.M.	REVISED -			363	18-00166-00-BR	WOODFORD	65	31	
	PLOT DATE = 2/28/2023	DRAWN - R.D.H.	REVISED -			CONTRACT NO. 89779					
		CHECKED - S.T.M./S.W.M.	REVISED -			SHEET NO. 4 OF 14 SHEETS					

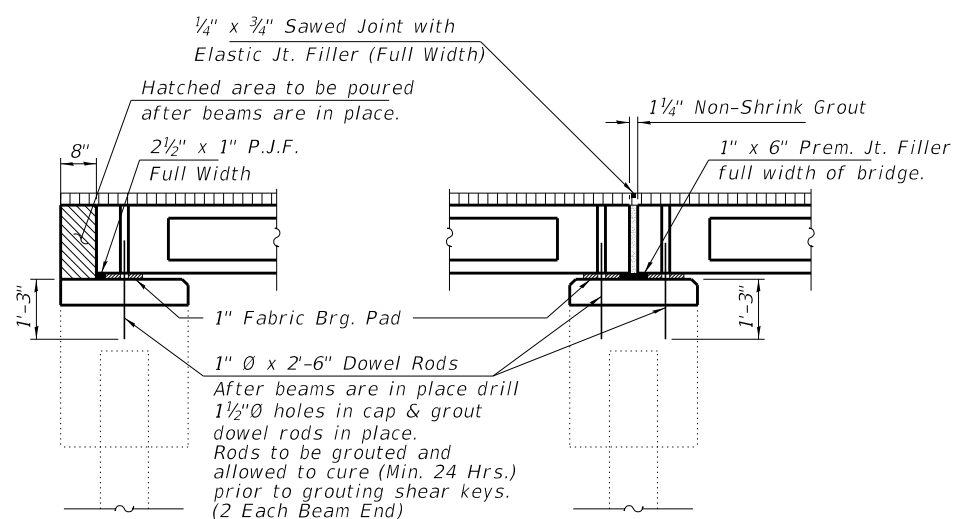
ILLINOIS FED. AID PROJECT NO. ZH0Y(896)



ELEVATION
Showing Rail Post Spaces
See sheet 6 of 14 for Railing Details.

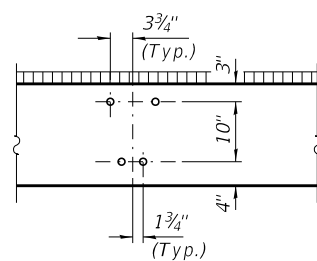


CROSS SECTION
See sheets 3 & 4 of 14 for Superstructure Details.

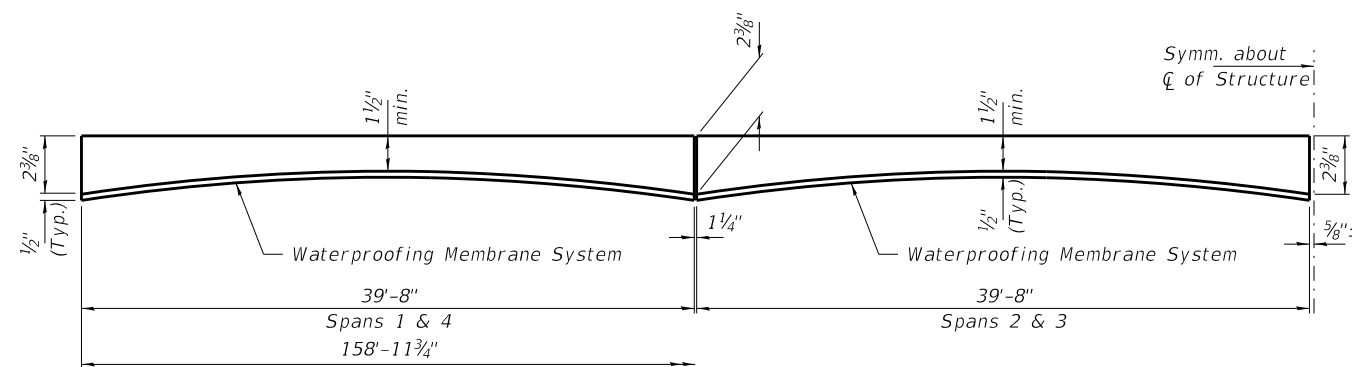


SECTION AT ABUTMENTS

SECTION AT PIERS



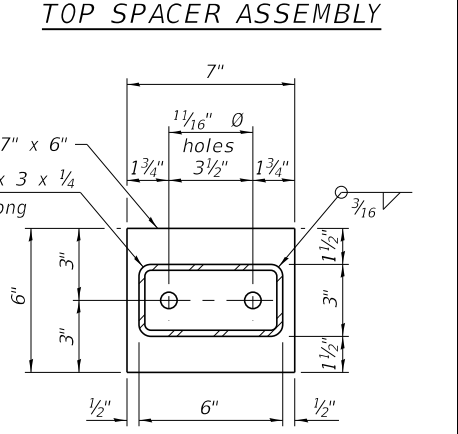
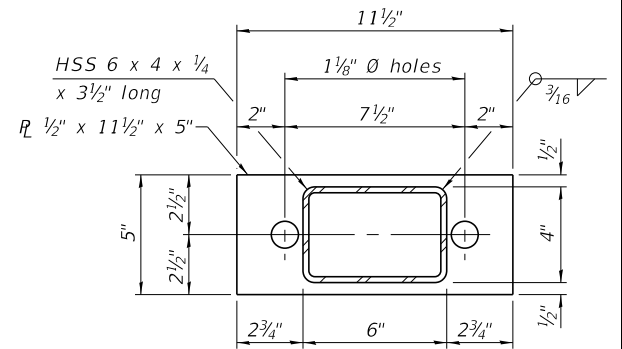
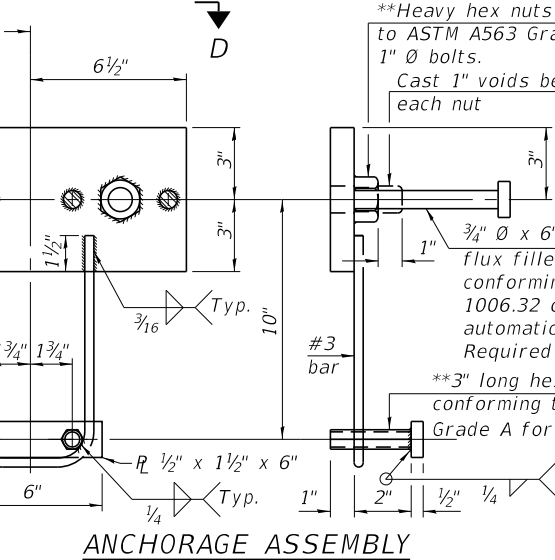
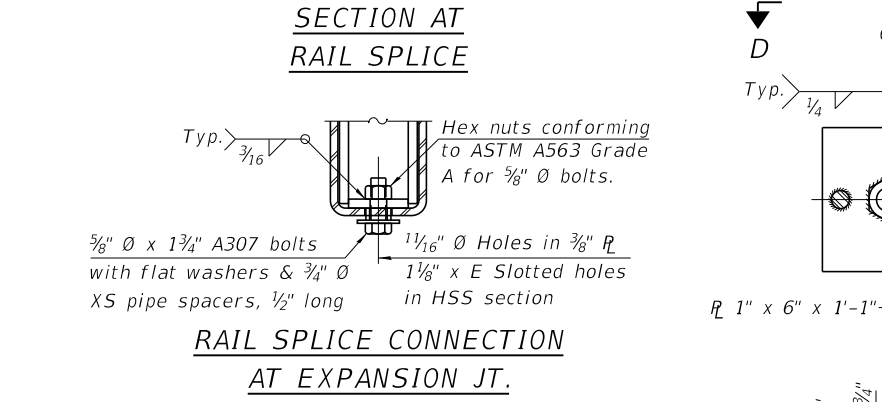
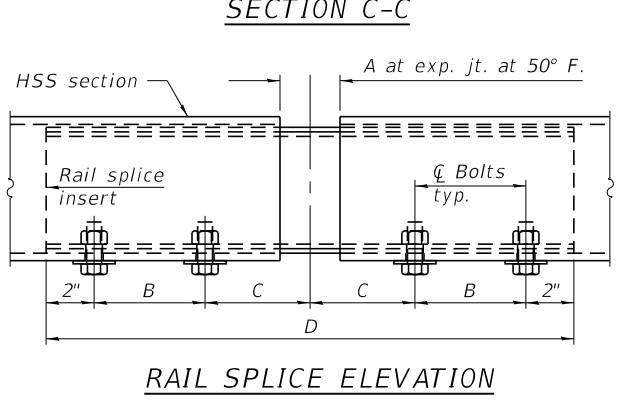
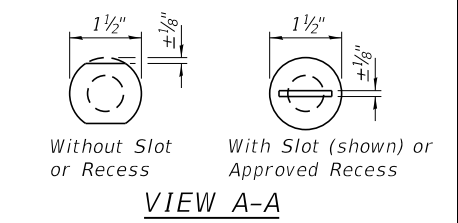
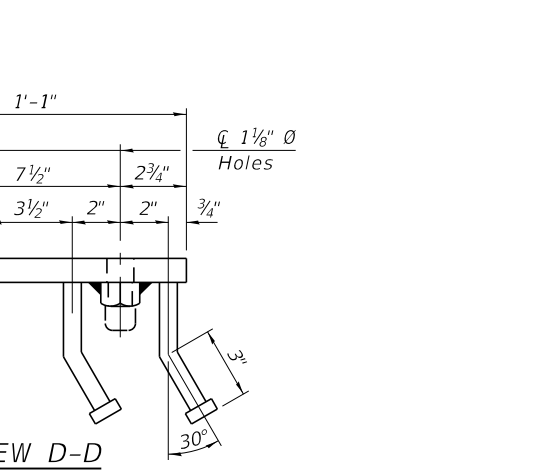
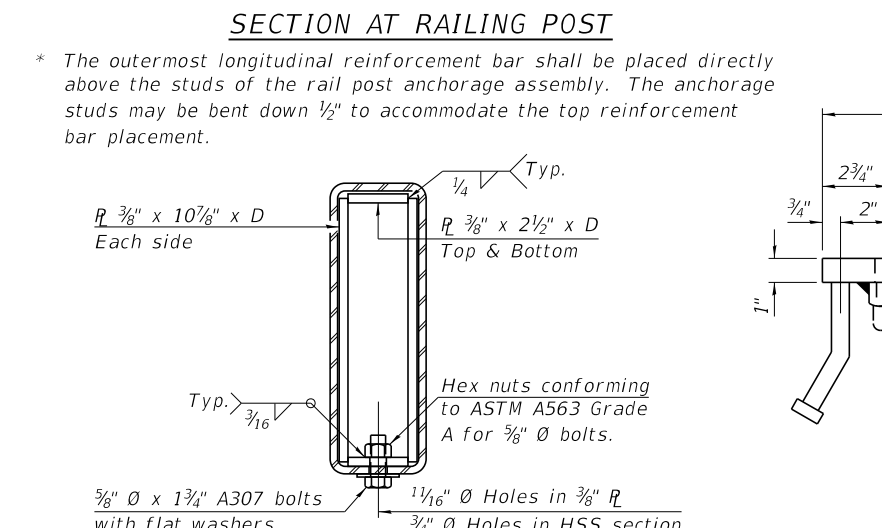
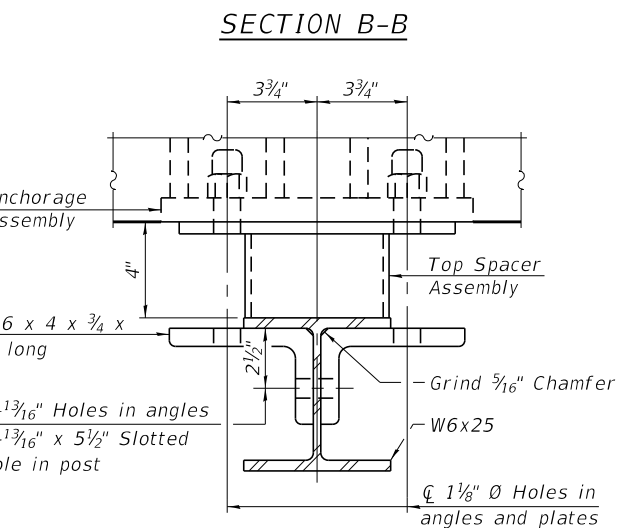
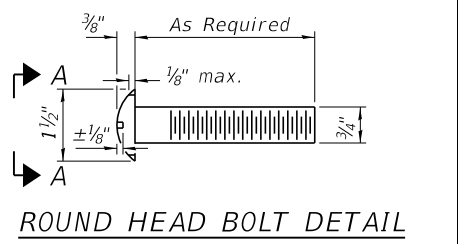
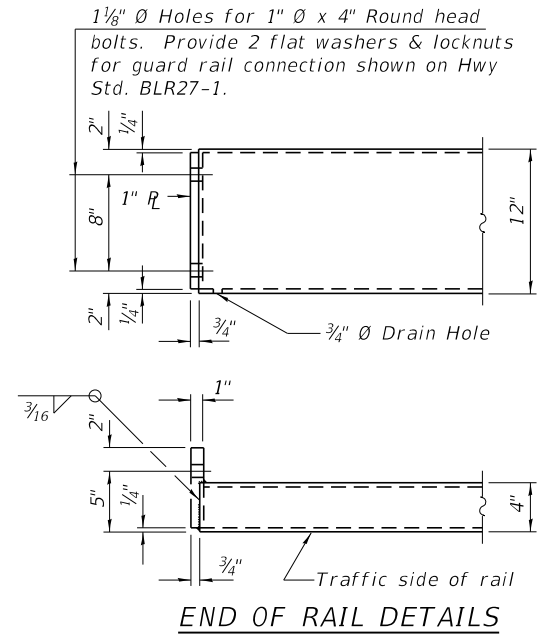
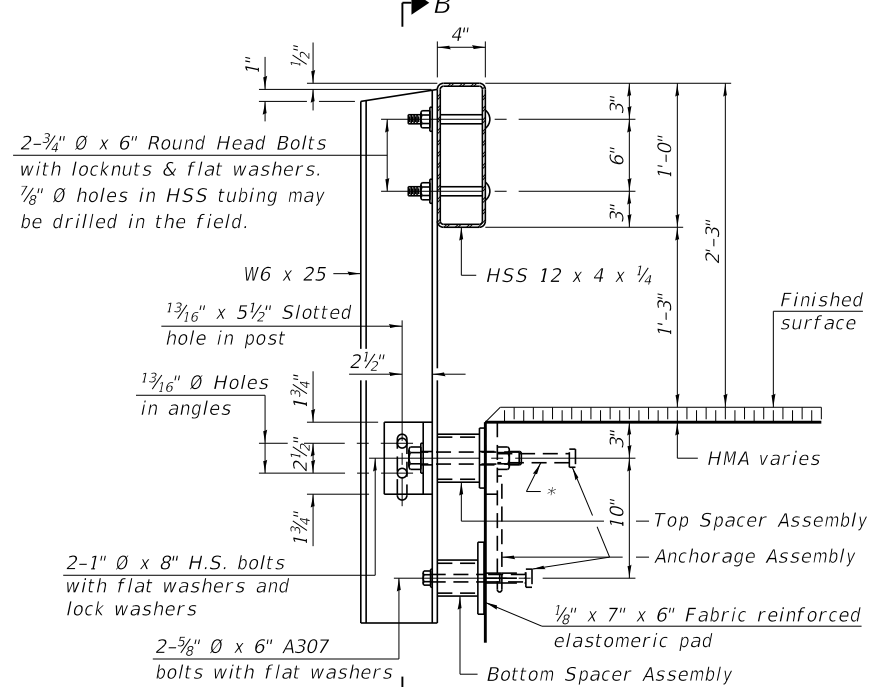
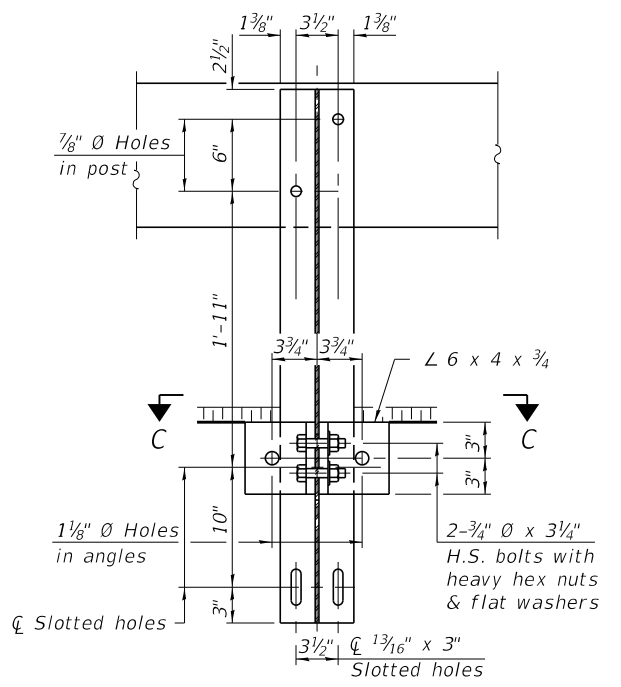
DETAIL A



ANTICIPATED HMA WEARING SURFACE PROFILE
(For information only - beam camber may vary in field.)

FILE NAME = 190013-shi-bridge - SN 102-3131.dgn	USER NAME = smierzwa	DESIGNED - J.R.B.	REVISED -	STATE OF ILLINOIS WOODFORD COUNTY HIGHWAY DEPARTMENT	SUPERSTRUCTURE DETAILS STRUCTURE NO. 102-3131	F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
HAMPTON, LENZINI AND RENWICK, INC. 3035 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 ILLINOIS PROFESSIONAL DESIGN FIRM LS / PE / SE CORP. 184.000959		CHECKED - S.T.M.	REVISED -			363	18-00166-00-BR	WOODFORD	65	32	
PLOT SCALE =		DRAWN - R.D.H.	REVISED -			CONTRACT NO. 89779					
PLOT DATE = 2/28/2023		CHECKED - S.T.M./S.W.M.	REVISED -			SHEET NO. 5 OF 14 SHEETS					

Notes:
 A sufficient number of shims of various thicknesses, sized to fit behind the top spacer assembly, 5" x 11 1/2", and bottom spacer assembly, 6" x 7", shall be provided to adjust posts for proper alignment. If the summation of shims is greater than 1/4" (top) or 1/2" (bottom), longer bolts are required. Cost included with Steel Railing, Type S-1.
 All steel rail elements including shims shall be galvanized according to Article 509.05 of the Standard Specifications.
 All HSS tubing serving as railing shall be CVN tested according to Article 1006.34(b) of the Standard Specifications.
 Rail splice inserts may be built out of 2 - 3/8" bent plates in lieu of the 4 plate rail splice inserts shown, provided the outside dimensions are matched.
 All round head bolts shall be ASTM A307 with locknuts according to ASTM A563 grade A.



BILL OF MATERIAL

Item	Unit	Quantity
Steel Railing, Type S-1	Foot	324

RAILING CRITERIA

Criteria	Value
NCHRP 350 Test Level	2
Railing Weight (plf)	50
Max Post Spacing	10'-9"
HMA thickness range (in)	1 1/4" - 3 1/8"

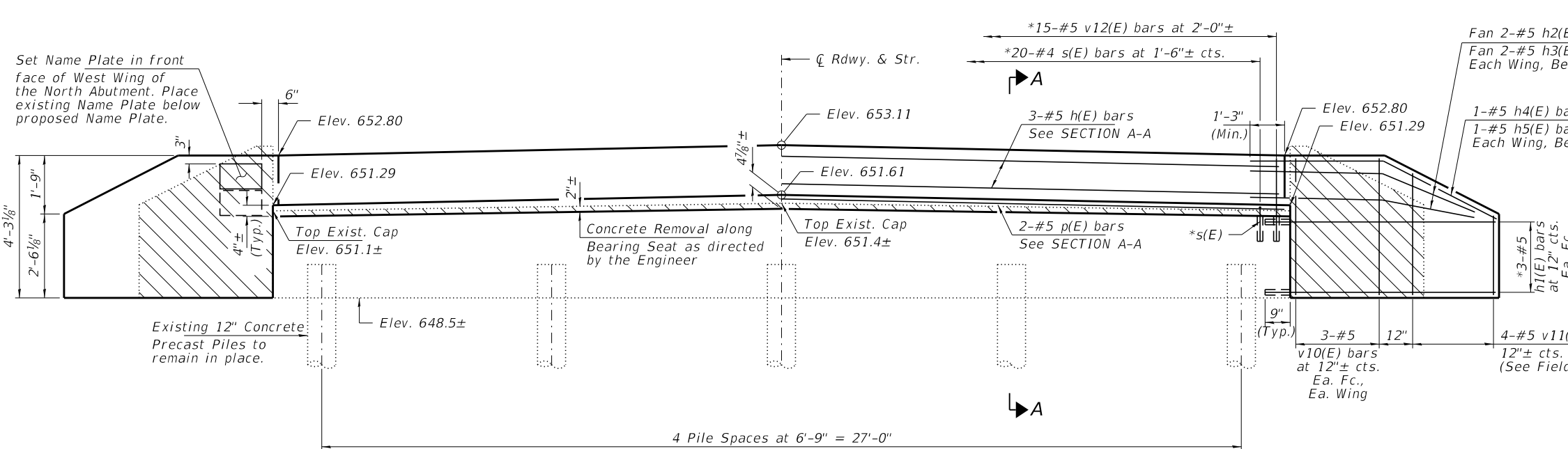
R-23A 10-12-2021

SPLICE DIMENSIONS

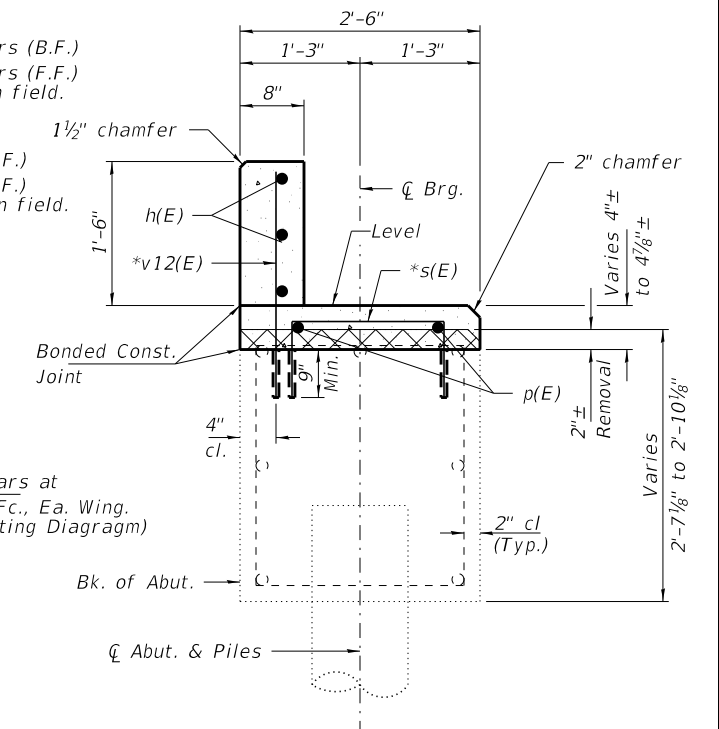
Location	T	A	B	C	D	E
All locs. not over exp. jts.	0	1/4"	4"	4"	1'-8"	-
Over Strip Seal Jt.	≤4"	2 1/2"	4 3/8"	4 3/8"	1'-10"	3 1/16"
Over Finger or Modular Jt.	≤9 1/2"	5 1/2"	7 3/8"	7 1/4"	2'-9 1/4"	5 1 3/16"
Over Finger or Modular Jt.	≤15"	8 1/4"	10 1/8"	10"	3'-8 1/4"	8 9/16"

T = ; total movement along centerline of roadway at expansion joint.

** Threaded areas shall be plugged or blocked off during casting of concrete.



ELEVATION
(Looking North)

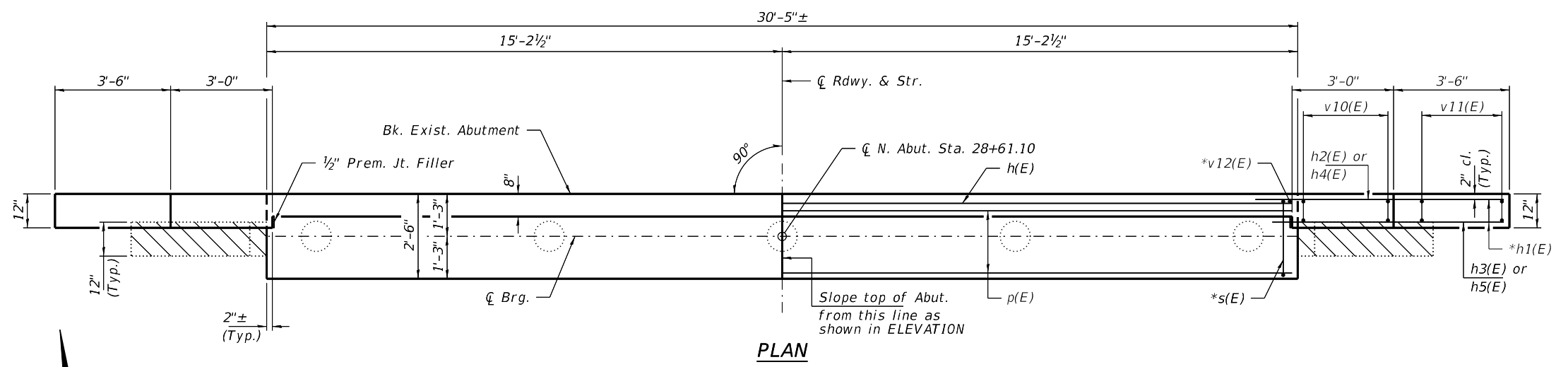


SECTION A-A

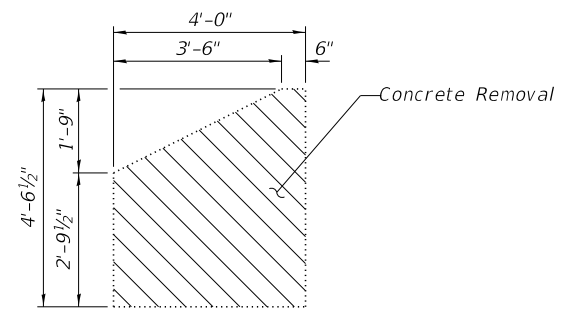
Backwall and wingwalls are to be poured after beams are in place.

*h1(E), s(E), and v12(E) bars shall be drilled and set according to Art. 509.06 of the Standard Specifications. Bars shall have a 9" minimum embedment depth, be set in 1"Ø holes and filled with an approved epoxy grout.

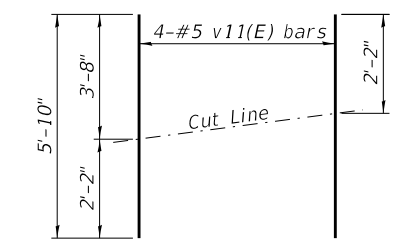
Hatched area indicates Concrete Removal



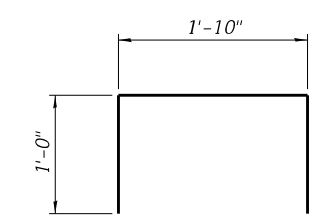
PLAN



EXISTING WINGWALL CONCRETE REMOVAL



FIELD CUTTING DIAGRAM
Order v11(E) bars full length. Cut as shown and use remainder of bars in opposite face.

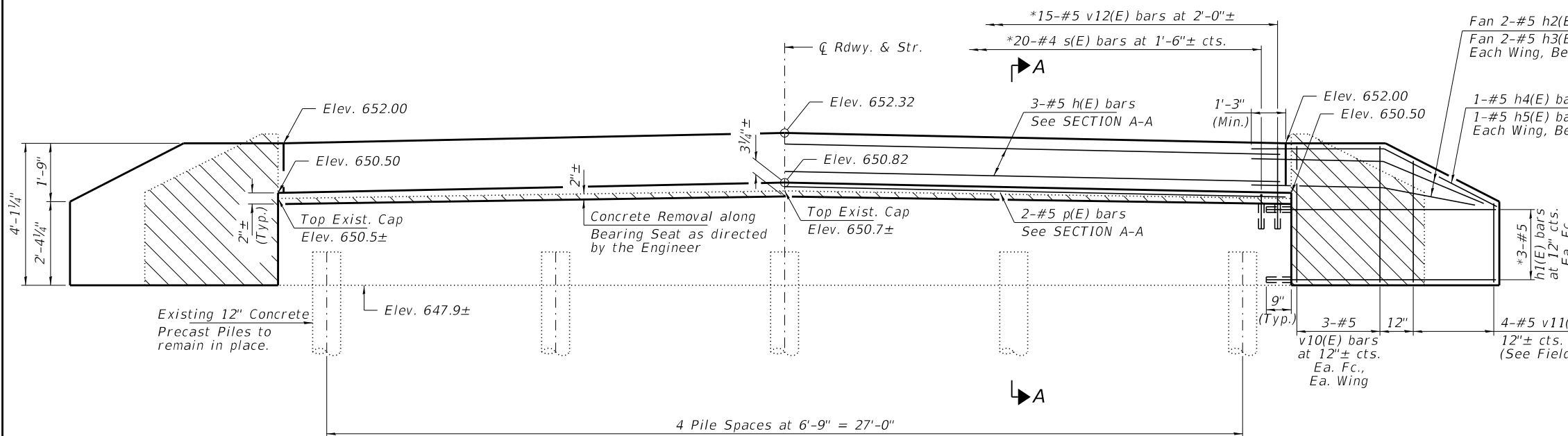


BAR s(E)

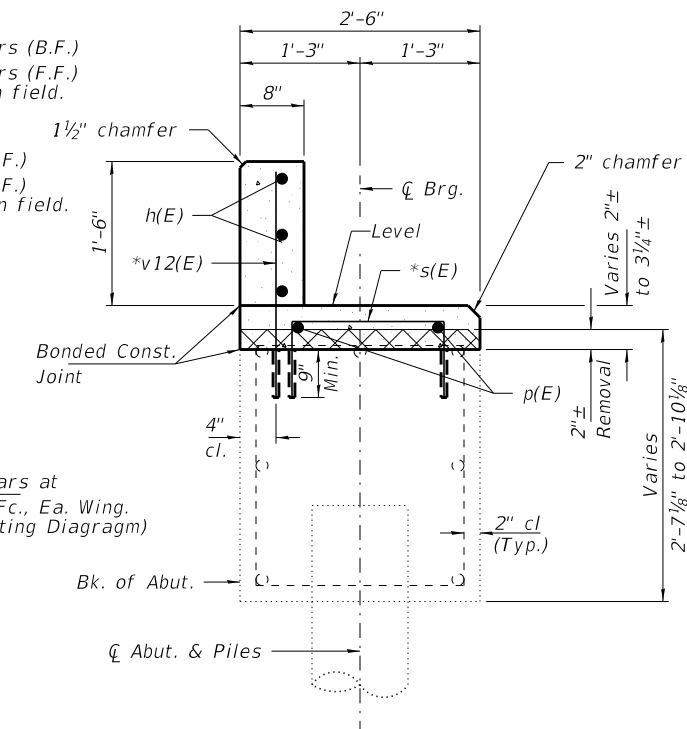
Notes:
The contractor shall repair, at their own expense, any damage to the substructure caused by removal operations.
Cost of drilling and grouting reinforcement bars included in cost for Concrete Structures.
Existing wingwall reinforcement which extends into the abutment cap shall be burned off flush with surface, ground smooth, and sealed with epoxy. Cost included with Concrete Removal.
The existing top of abutment cap is sloped. The Contractor shall level top of cap to the satisfaction of the Engineer during removal.
Backwall and wingwalls shall be poured after the beams are in place.
See Existing Plans for existing reinforcement placement.

BILL OF MATERIAL - N. ABUT.

BAR	NO.	SIZE	LENGTH	SHAPE
h(E)	3	#5	30'-1"	—
h1(E)	12	#5	7'-0"	—
h2(E)	4	#5	7'-4"	—
h3(E)	4	#5	5'-10"	—
h4(E)	2	#5	8'-0"	—
h5(E)	2	#5	6'-6"	—
p(E)	2	#5	30'-1"	—
s(E)	20	#4	3'-10"	□
v10(E)	12	#5	3'-10"	—
v11(E)	8	#5	5'-10"	—
v12(E)	15	#5	2'-5"	—
Concrete Structures			Cu. Yd.	4.0
Concrete Removal			Cu. Yd.	1.6
Reinf. Bars, Epoxy Coated			Pound	520
Name Plates (Special)			Each	1
Asbestos Bearing Pad Removal			Each	11



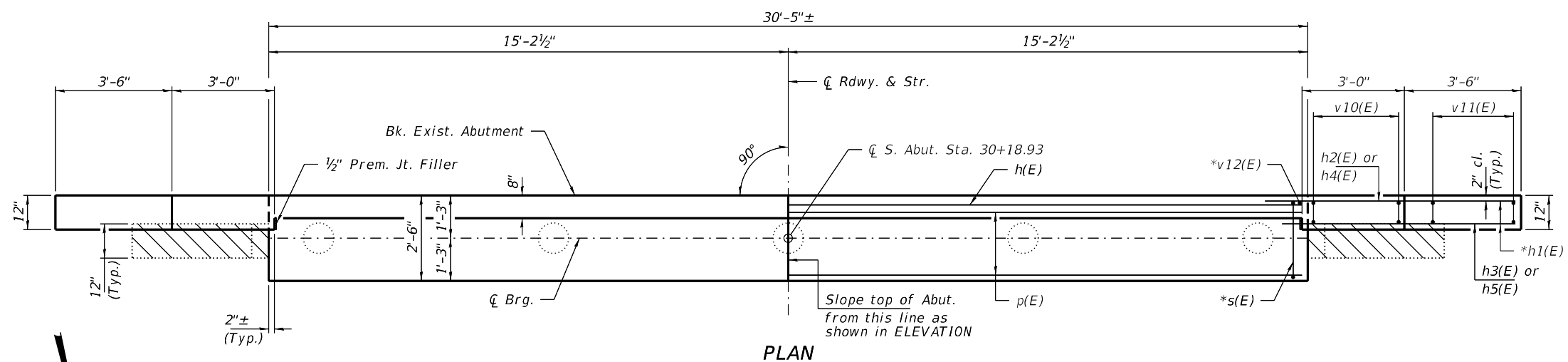
ELEVATION
(Looking South)



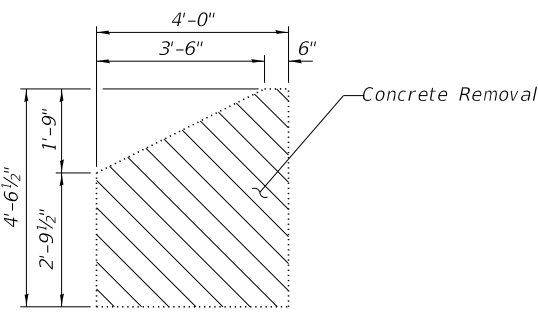
SECTION A-A
Backwall and wingwalls are to be poured after beams are in place.

*h1(E), s(E), and v12(E) bars shall be drilled and set according to Art. 509.06 of the Standard Specifications. Bars shall have a 9" minimum embedment depth, be set in 1"Ø holes and filled with an approved epoxy grout.

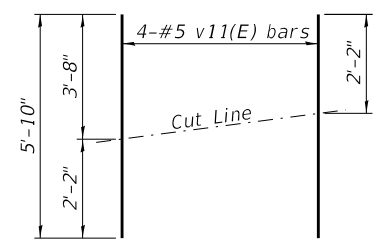
Hatched area indicates Concrete Removal



PLAN

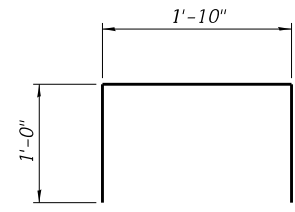


EXISTING WINGWALL CONCRETE REMOVAL



FIELD CUTTING DIAGRAM

Order v11(E) bars full length. Cut as shown and use remainder of bars in opposite face.

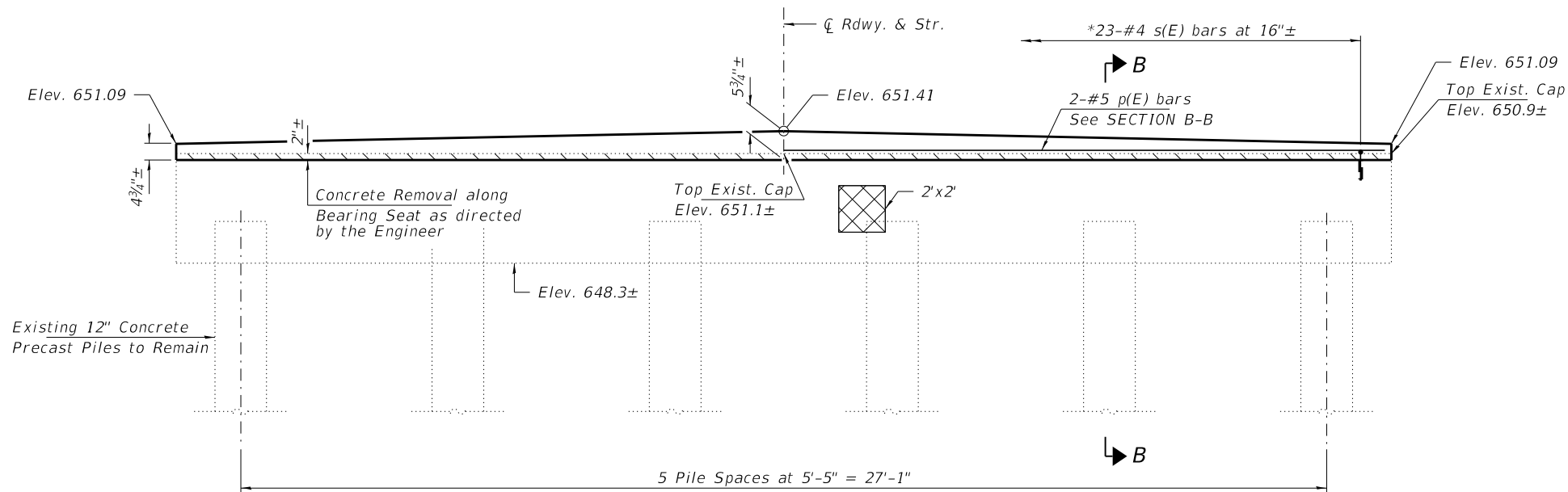


BAR s(E)

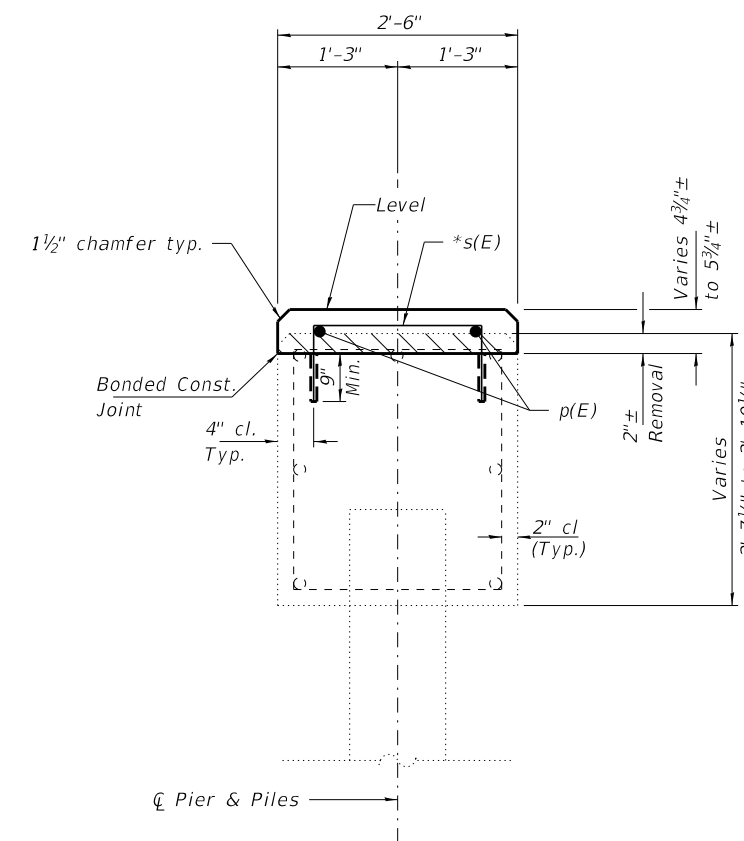
Notes:
The contractor shall repair, at their own expense, any damage to the substructure caused by removal operations.
Cost of drilling and grouting reinforcement bars included in cost for Concrete Structures.
Existing wingwall reinforcement which extends into the abutment cap shall be burned off flush with surface, ground smooth, and sealed with epoxy. Cost included with Concrete Removal.
The existing top of abutment cap is sloped. The Contractor shall level top of cap to the satisfaction of the Engineer during removal.
Backwall and wingwalls shall be poured after the beams are in place.
See Existing Plans for existing reinforcement placement.

BILL OF MATERIAL - S. ABUT.

BAR	NO.	SIZE	LENGTH	SHAPE
h(E)	3	#5	30'-1"	—
h1(E)	12	#5	7'-0"	—
h2(E)	4	#5	7'-4"	—
h3(E)	4	#5	5'-10"	—
h4(E)	2	#5	8'-0"	—
h5(E)	2	#5	6'-6"	—
p(E)	2	#5	30'-1"	—
s(E)	20	#4	3'-10"	□
v10(E)	12	#5	3'-10"	—
v11(E)	8	#5	5'-10"	—
v12(E)	15	#5	2'-5"	—
Concrete Structures			Cu. Yd.	3.4
Concrete Removal			Cu. Yd.	1.6
Reinf. Bars, Epoxy Coated			Pound	520
Asbestos Bearing Pad Removal			Each	11



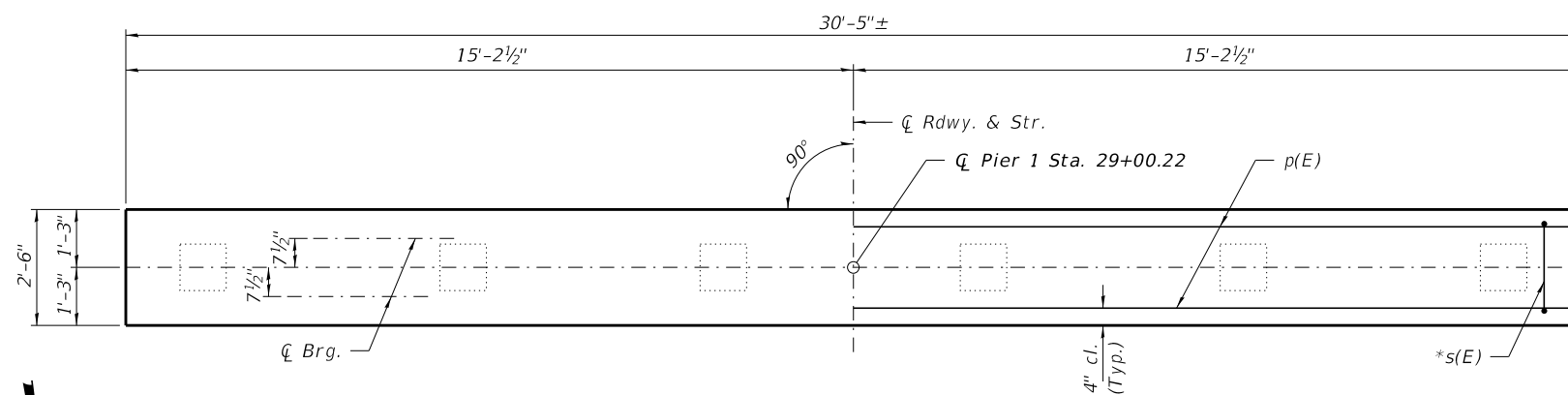
ELEVATION
(North Face Shown, Looking South)



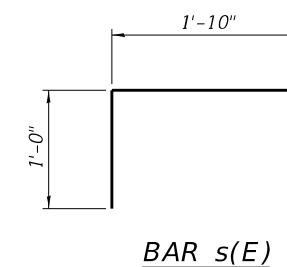
SECTION B-B

*s(E) bars shall be drilled and set according to Art. 509.06 of the Standard Specifications. Bars shall have a 9" minimum embedment depth, be set in 1"Ø holes and filled with an approved epoxy grout.

- Hatched area indicates Concrete Removal
- Hatched area indicates Structural Repair of Concrete (Depth ≤ 5")



PLAN

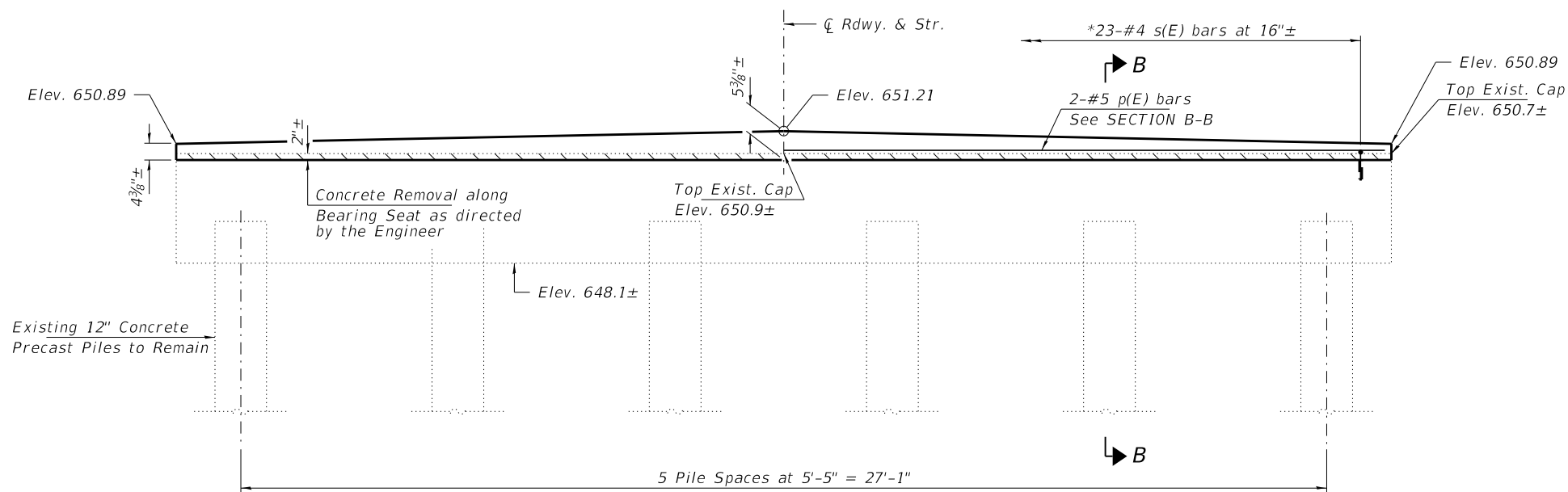


BAR s(E)

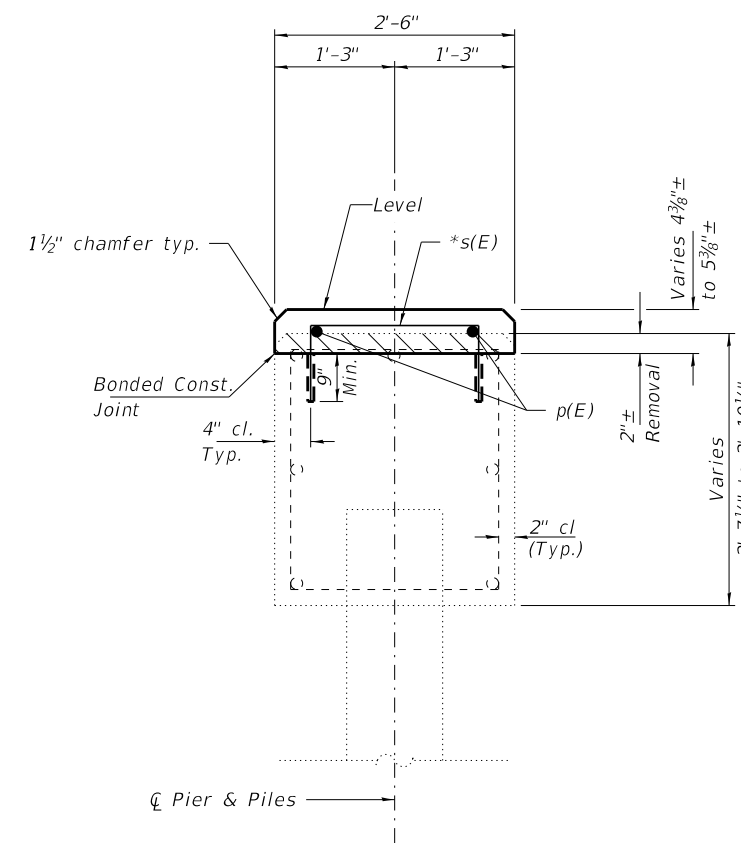
BILL OF MATERIAL - PIER 1

BAR	NO.	SIZE	LENGTH	SHAPE
p(E)	2	#5	30'-1"	—
s(E)	23	#4	3'-10"	□
Concrete Structures			Cu. Yd.	1.2
Concrete Removal			Cu. Yd.	0.5
Reinf. Bars, Epoxy Coated			Pound	120
Structural Repair of Concrete (Depth ≤ 5")			Sq. Ft.	4
Asbestos Bearing Pad Removal			Each	22

Notes:
 The contractor shall repair, at their own expense, any damage to the substructure caused by removal operations.
 Cost of drilling and grouting reinforcement bars included in cost for Concrete Structures.
 The existing top of pier cap is sloped. The Contractor shall level top of cap to the satisfaction of the Engineer during removal.
 See Existing Plans for existing reinforcement placement.



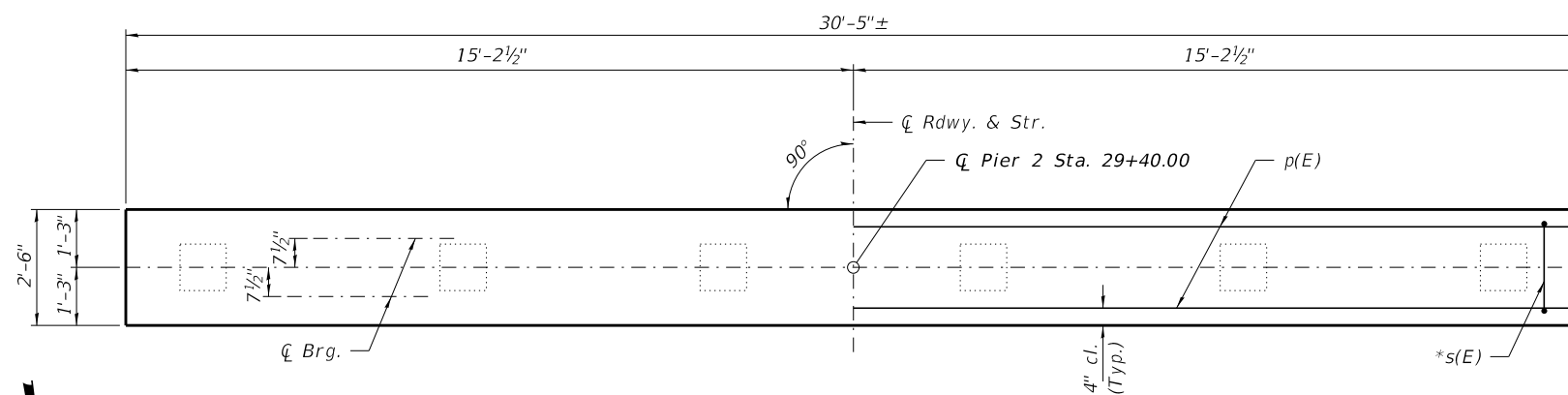
ELEVATION
(North Face Shown, Looking South)



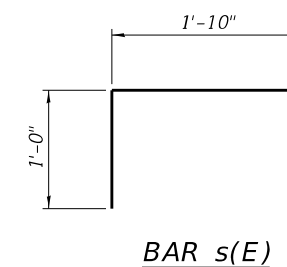
SECTION B-B

*s(E) bars shall be drilled and set according to Art. 509.06 of the Standard Specifications. Bars shall have a 9" minimum embedment depth, be set in 1"Ø holes and filled with an approved epoxy grout.

Hatched area indicates Concrete Removal



PLAN

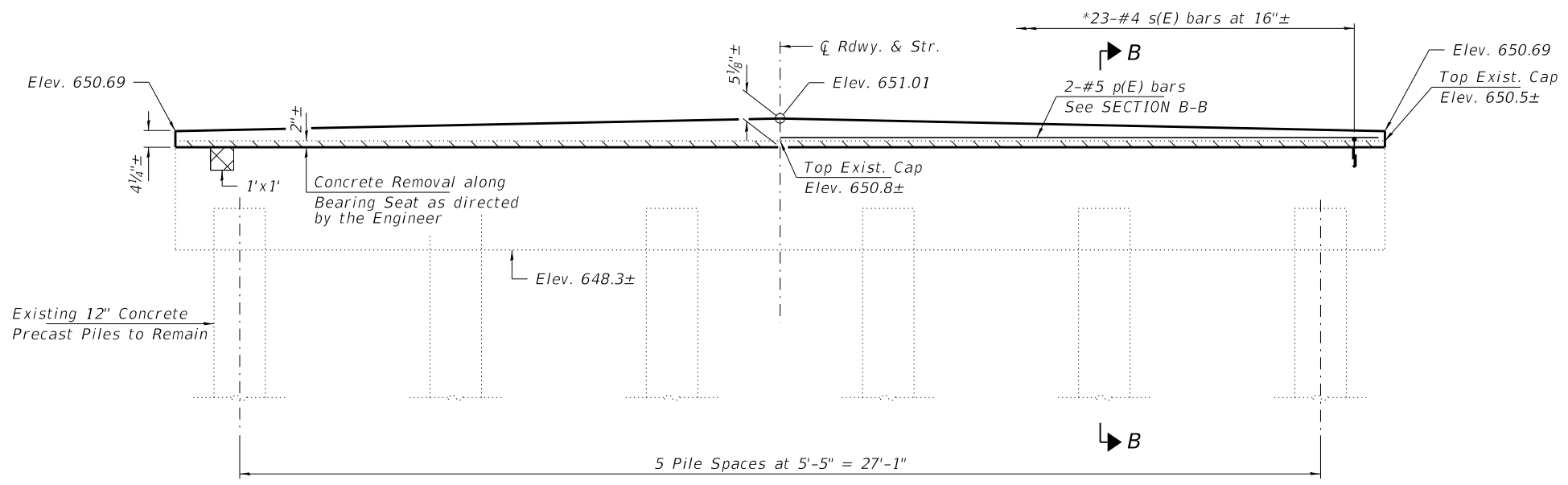


BAR s(E)

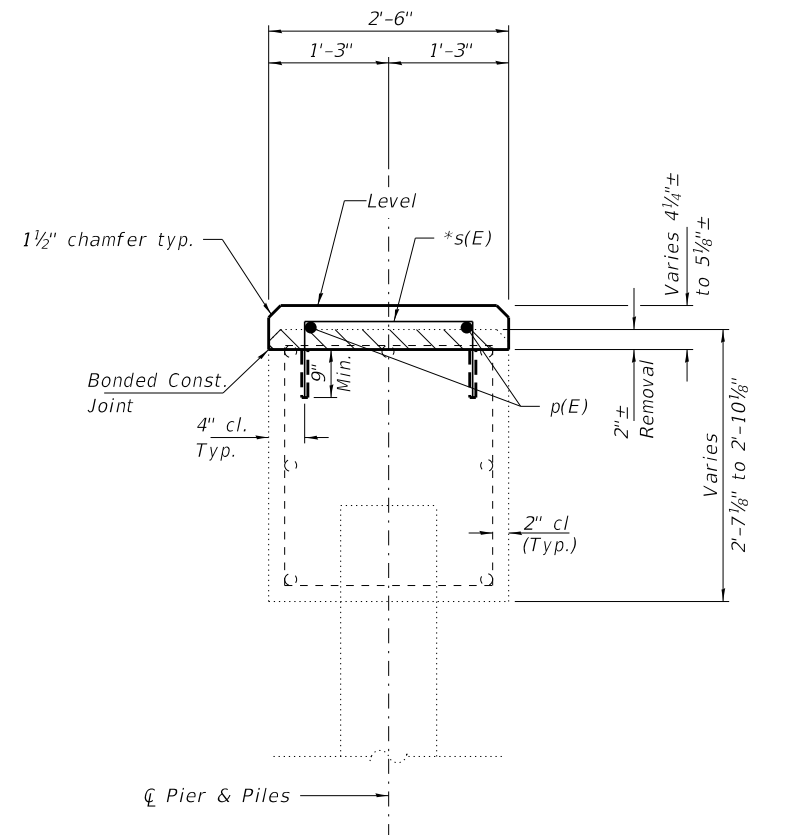
BILL OF MATERIAL - PIER 2

BAR	NO.	SIZE	LENGTH	SHAPE
p(E)	2	#5	30'-1"	—
s(E)	23	#4	3'-10"	□
Concrete Structures			Cu. Yd.	1.1
Concrete Removal			Cu. Yd.	0.5
Reinf. Bars, Epoxy Coated			Pound	120
Asbestos Bearing Pad Removal			Each	22

Notes:
 The contractor shall repair, at their own expense, any damage to the substructure caused by removal operations.
 Cost of drilling and grouting reinforcement bars included in cost for Concrete Structures.
 The existing top of pier cap is sloped. The Contractor shall level top of cap to the satisfaction of the Engineer during removal.
 See Existing Plans for existing reinforcement placement.



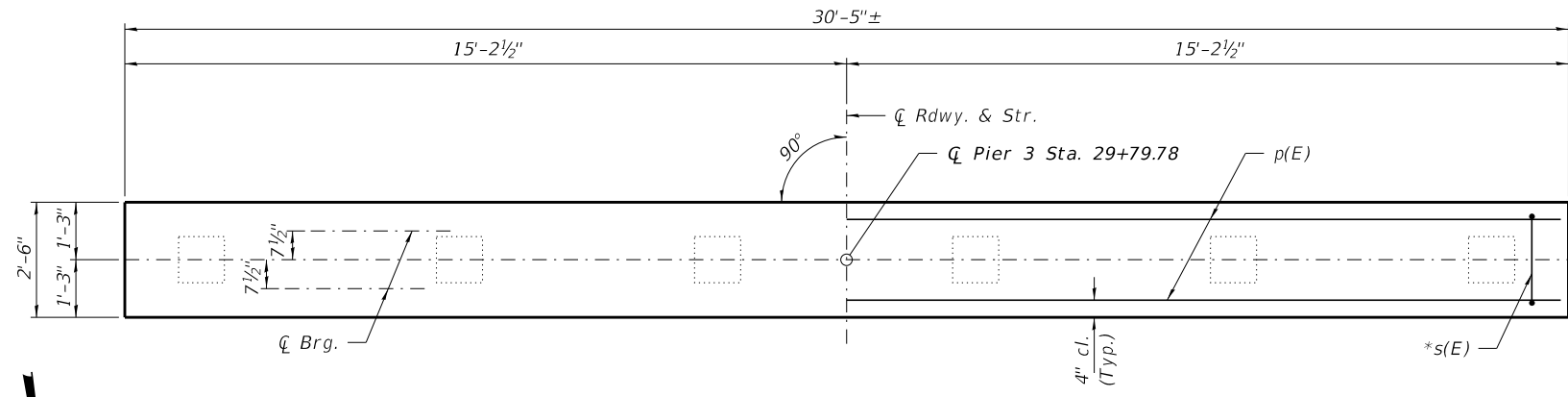
ELEVATION
(North Face Shown, Looking South)



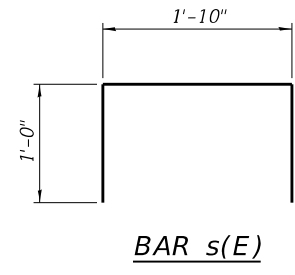
SECTION B-B

*s(E) bars shall be drilled and set according to Art. 509.06 of the Standard Specifications. Bars shall have a 9" minimum embedment depth, be set in 1"Ø holes and filled with an approved epoxy grout.

- Hatched area indicates Concrete Removal
- Hatched area indicates Structural Repair of Concrete (Depth ≤ 5")



PLAN



BAR s(E)

Notes:
 The contractor shall repair, at their own expense, any damage to the substructure caused by removal operations.
 Cost of drilling and grouting reinforcement bars included in cost for Concrete Structures.
 The existing top of pier cap is sloped. The Contractor shall level top of cap to the satisfaction of the Engineer during removal.
 See Existing Plans for existing reinforcement placement.

BILL OF MATERIAL - PIER 3

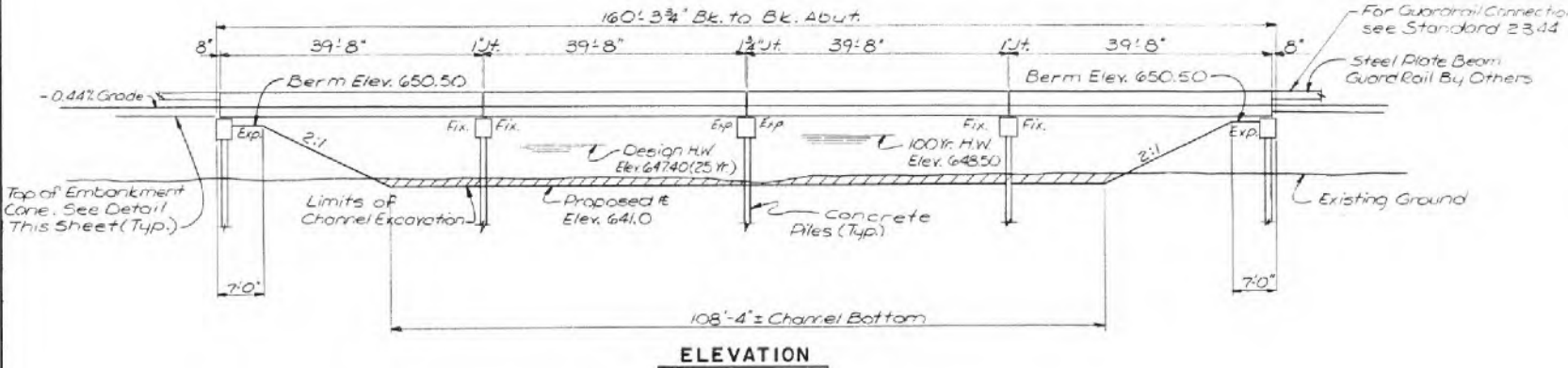
BAR	NO.	SIZE	LENGTH	SHAPE
p(E)	2	#5	30'-1"	—
s(E)	23	#4	3'-10"	□
Concrete Structures			Cu. Yd.	1.1
Concrete Removal			Cu. Yd.	0.5
Reinf. Bars, Epoxy Coated			Pound	120
Structural Repair of Concrete (Depth ≤ 5")			Sq. Ft.	1
Asbestos Bearing Pad Removal			Each	22

Benchmark: 100' South of Existing Bridge, East Side of Road.
Spike in Power Pole, Elev. 645.11
Existing Structure: None

POST NO.	SECTION	COUNTY	SHEET NO.
		WOODFORD	29 15
Sec. 77-00059-00-BR			

GENERAL NOTES

Standard Specifications for Road and Bridge Construction, adopted by the Illinois Department of Transportation, July 1, 1976 shall apply.
All structural steel shall be shop painted with two coats of basic lead silico chromate paint, including exposed piling.
Protective Coat shall not be applied to surfaces to which Water-Proofing Membrane System is applied.
For Soil Borings see Special Provisions.
All Transverse Tie Assemblies (nuts, bolts, and washers) shall be Hot Dipped Galvanized in accordance with AASHTO Designation M252.
The Contractor shall drive one test pile in a permanent location as directed by the Engineer before ordering the remainder of piles. The test pile is to be located at Pier #2.
It is the Contractor's responsibility to comply with all applicable Local, State and Federal Laws, including Occupational Safety and Health Act (OSHA) and Regulations adopted pursuant thereto. The Contractor shall be responsible for construction means, methods, techniques, sequences and procedures; and for safety precautions and programs, including the safety and adequacy of shoring, formwork, sheeting, safety railing, scaffolding and equipment.
Ralph Hahn and Associates Consulting & Design Engineers Inc. assumes no responsibility or liability in these matters whatsoever.
The embankment configuration shown shall be the minimum embankment that must be constructed prior to construction of the abutments.
For Proposed Channel Configuration see Sheet #12

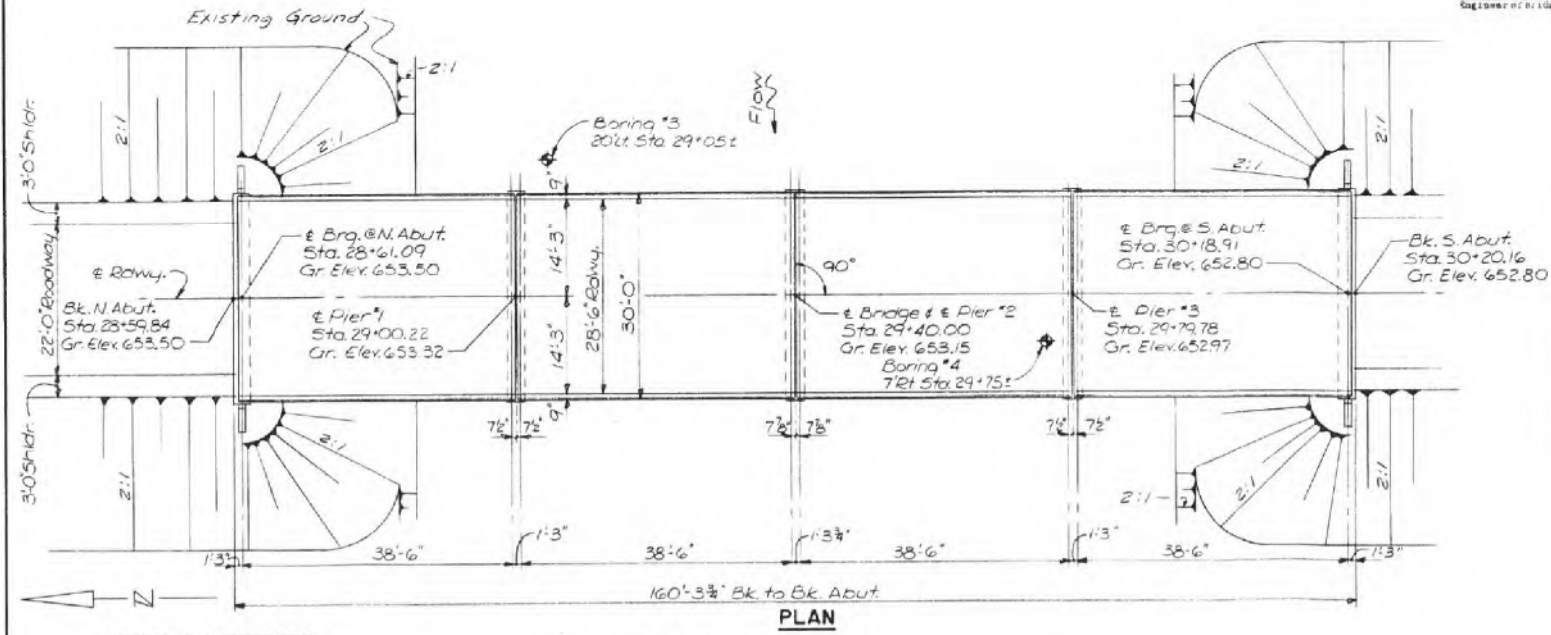


WATERWAY DATA

Drainage Area	186 Sq. Mi. **
Existing Opening (Over Rd. Flow)	2586 Sq. Ft.
Required Opening (25 Yr.)	740 Sq. Ft.
Proposed Opening	740 Sq. Ft.
Design Frequency Discharge (25 Yr.)	3490 C.F.S.
Created Head For Design Flood	0.4 Ft. **
100 Yr. Discharge	11,130 C.F.S. **
Created Head For 100 Yr. Flood	0.5 Ft. **

** Main Structure and Overflow Structure

Expansion guards which are not cast in the precast unit shall be fabricated and erected in accordance with Article 503.07(c) of the Standard Specifications and are included in quantity of structural steel.
The top surface of the beams shall be finished in accordance with Article 505.06 of the Standard Specifications except that the surface shall not be roughened by brooming. The finished surface shall be free of depressions or high spots with sharp corners.
The concrete rail section above the mandatory construction joint at the top of the slab shall be constructed of Class X Concrete, except the aggregates shall conform to the requirements of Handrail Concrete.

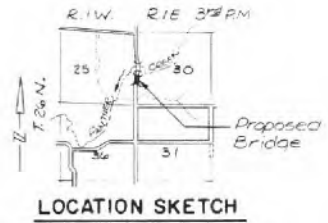


DESIGN STRESSES

PRECAST PRESTRESSED CONCRETE	LOADING HS20-44
$f_c = 5,000$ p.s.i.	$f_c = 1400$ p.s.i. (Sub.)
$f_{ci} = 4,000$ p.s.i.	$f_s = 20,000$ p.s.i. (Reinf.)
$f_s = 270,000$ p.s.i. (1/2" Strands)	$n = 8.5$
$f_{si} = 189,000$ p.s.i.	

Design Specifications: AASHTO 1973 and all Applicable Interim Specifications - 74, 75, 76 & 77.

FOR CITY OF
Carl E. Thurman, P.E.
ENGINEER IN CHARGE



STATION 29+40
OVERFLOW STRUCTURE AT PANTHER CREEK
BUILT 1978
FAS. RT. 363 SEC. 77-00059-00-BR.
FA. PROJ. BR. 5-363(104)
LOADING HS 20
N&P 102-3131

NAME PLATE
(See Standard 2113)

TOTAL BILL OF MATERIALS

Item	Unit	Quantity
Channel Excavation	Cu. Yd.	3032
Bituminous Concrete Surface Course, Class I	Ton	42
Protective Coat	Sq. Yd.	100
Class X Concrete	Cu. Yd.	61.5
RR Concrete Deck Beams 17"	Sq. Ft.	4760
Waterproofing Membrane System	Sq. Yd.	503
Furnishing and Erecting Structural Steel	Pound	3720
Reinforcement Bars	Pound	8340
Furnishing Concrete Piles	Lin. Ft.	864
Driving Concrete Piles	Lin. Ft.	864
Test Pile - Concrete	Each	1
Name Plates	Each	1
Seeding - Class II	Acre	1.4
Preformed Joint Sealer 2 1/2"	Lin. Ft.	31
Erosion Control Fabric	Sq. Yd.	5278
Erosion Control Mat	Sq. Yd.	1357
Portland Cement Mortar Fairing Course	Lin. Ft.	1428

* See Special Provisions



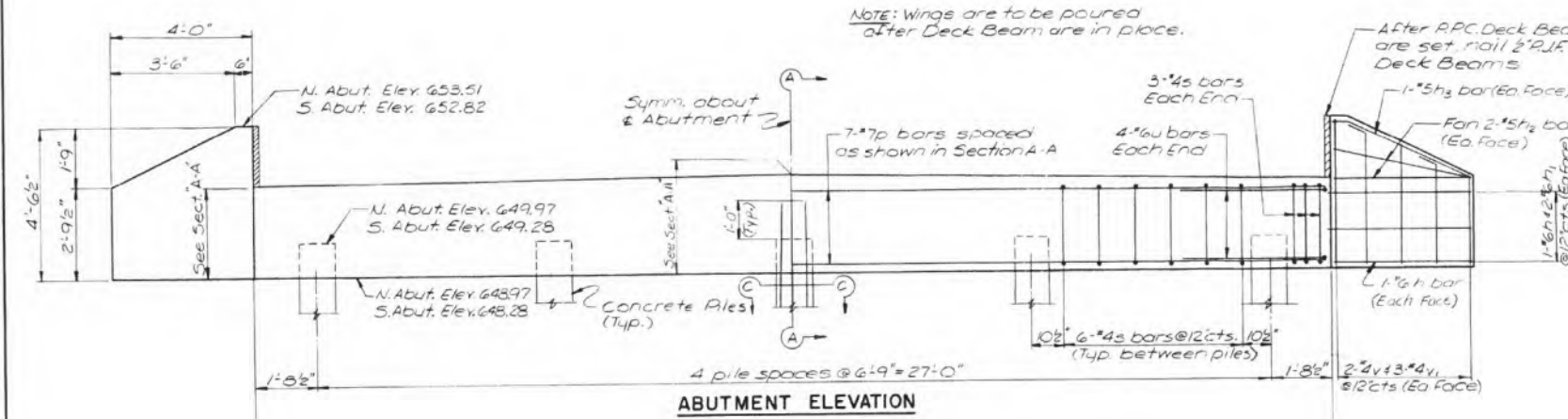
Frank L. Stiley
ILLINOIS STRUCTURAL NO. 3100

LOADING HS20 44
RALPH HAHN & ASSOCIATES
CONSULTING & DESIGN ENGINEERS INC.
1320 South State Street
Springfield, Illinois 62704

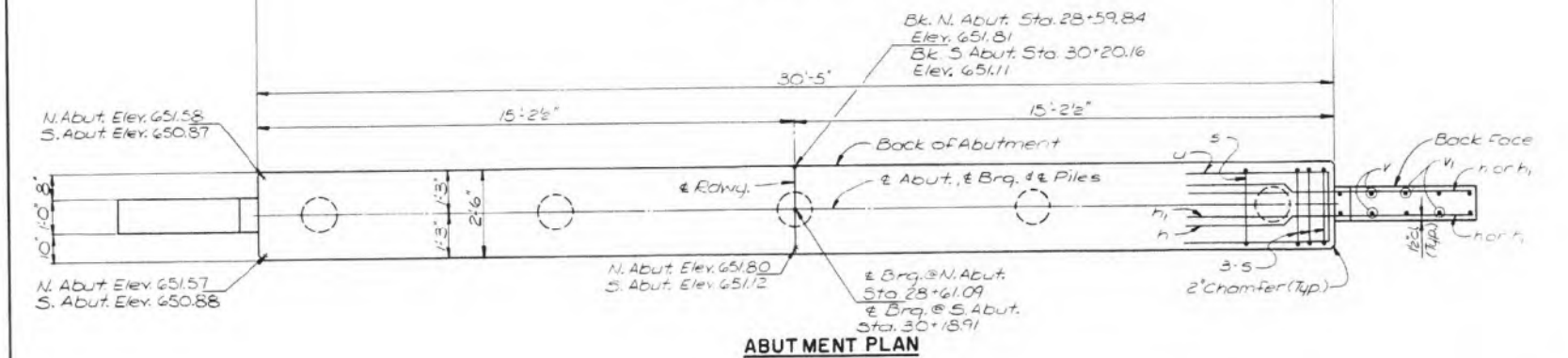
**GENERAL PLAN & ELEVATION
OVERFLOW STRUCTURE AT PANTHER CREEK
PALESTINE ROAD DISTRICT
SECTION 77-00059-00-BR
WOODFORD COUNTY
STATION 29+40
102-3131**

SUBMITTED APRIL 12 1978
Robert A. Channing
WOODFORD COUNTY
SUPERINTENDENT OF HIGHWAYS

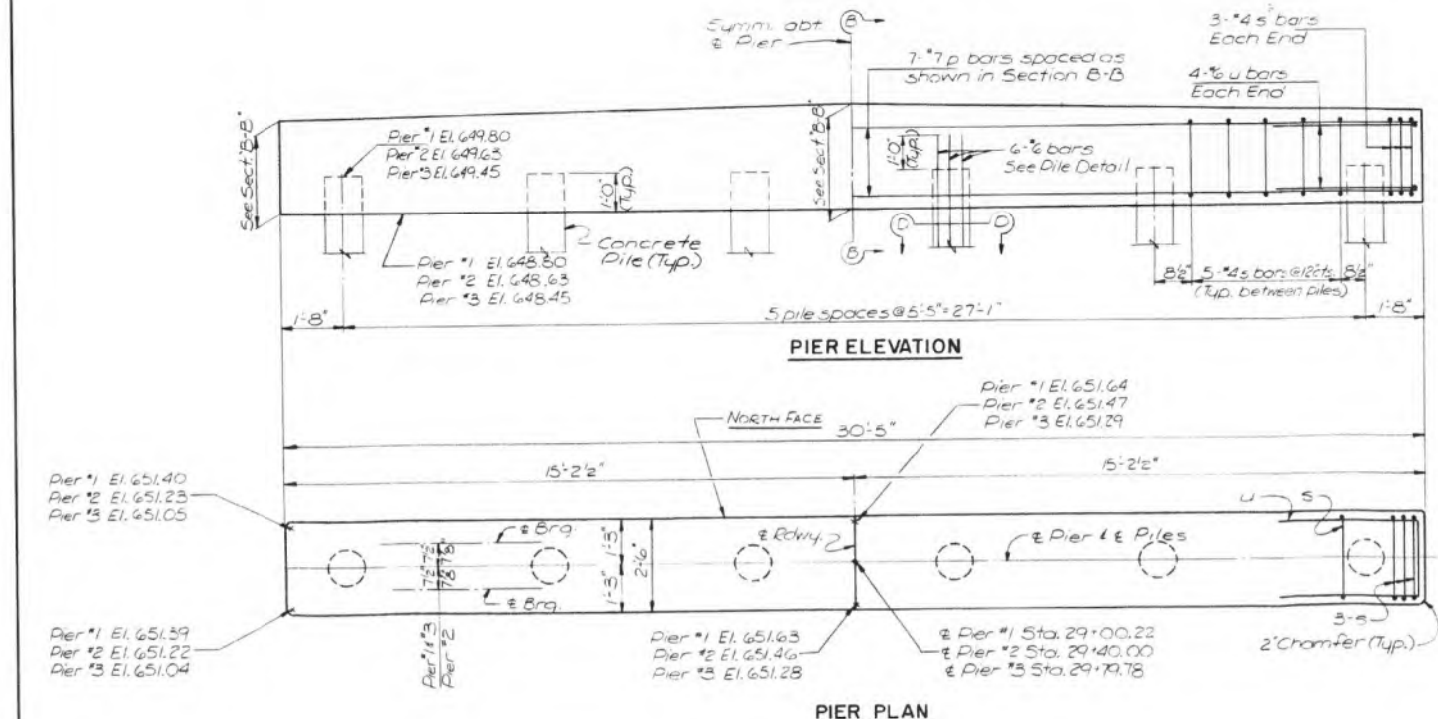
APPROVED _____ 1978
DISTRICT ENGINEER



ABUTMENT ELEVATION

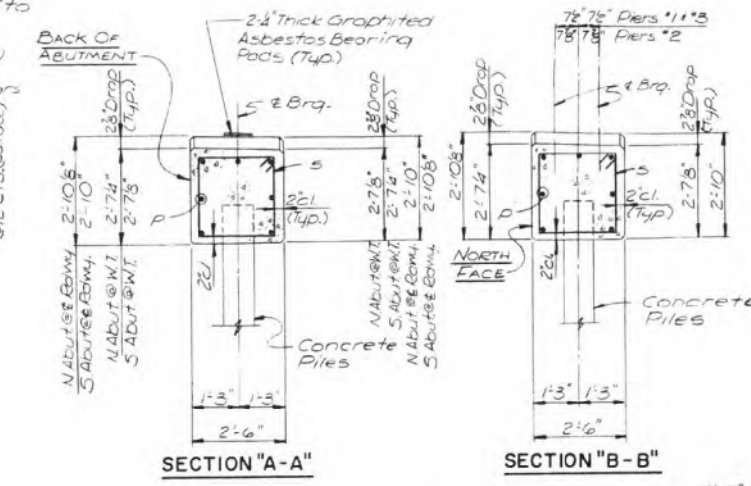


ABUTMENT PLAN



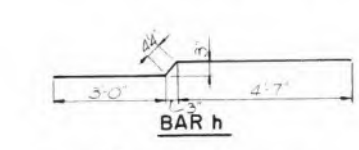
PIER ELEVATION

PIER PLAN

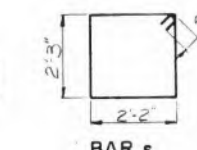


SECTION "A-A"

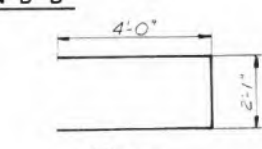
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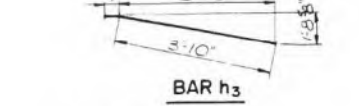
BAR h



BAR s



BAR u



BAR h3

PILE DATA

	* N. & S. Abut.	* Piers #1 & 3	* Pier #2
Type	Concrete	Concrete	Concrete
Capacity	30 Tons	35 Tons	35 Tons
Est. Length	32'	32'	32'
No. Req'd.	5 Ea. Abut.	6 Ea. Pier	5 + 1 Test Pile

* NOTE: Do Not Overdrive Piles. Piles will Stop Abruptly @ El. 619.00 ±
For Pile Details - See Sheet #19

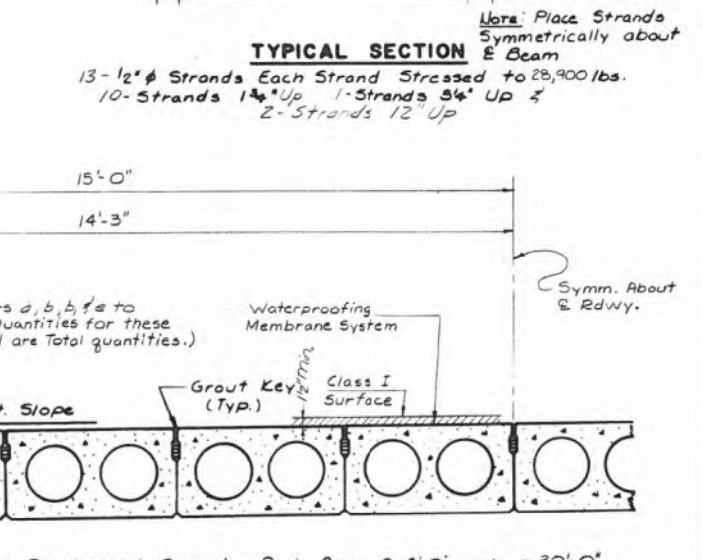
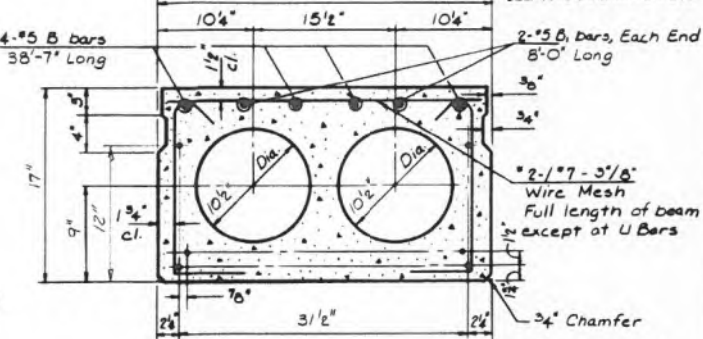
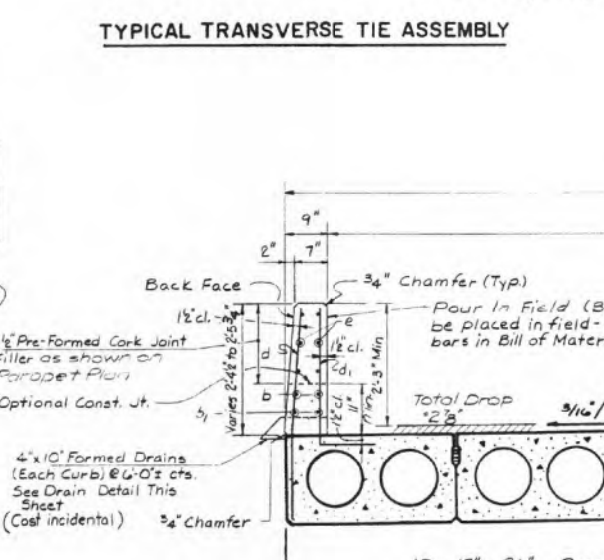
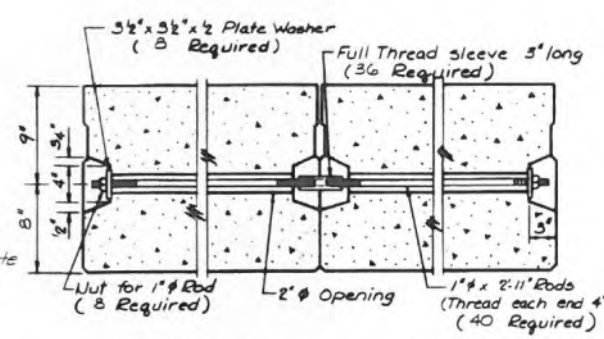
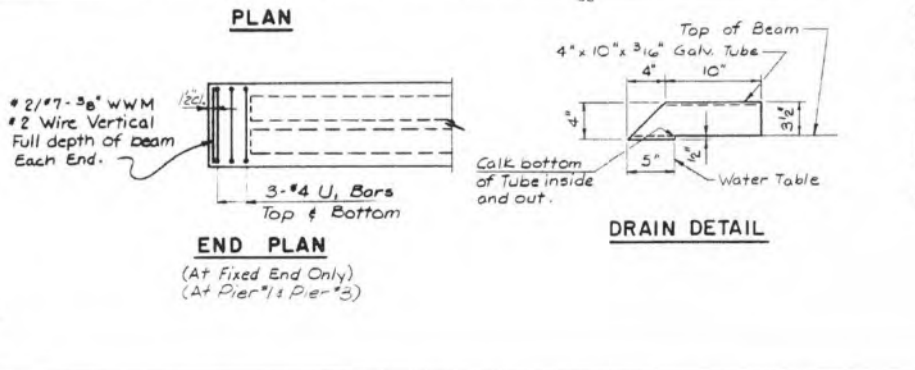
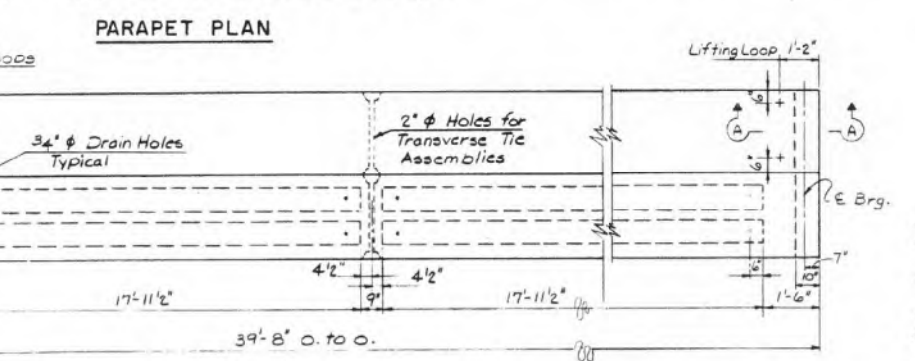
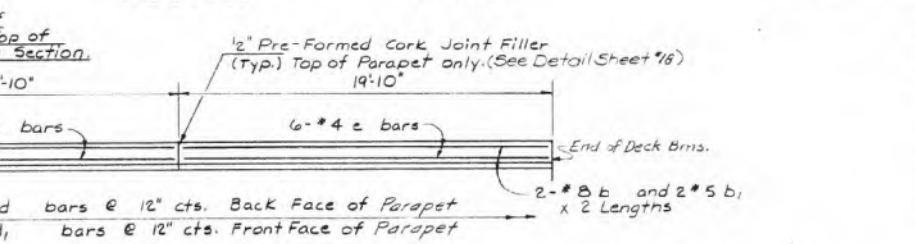
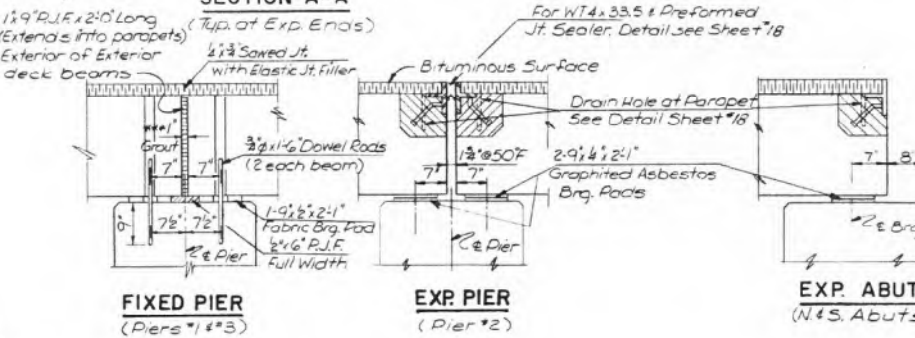
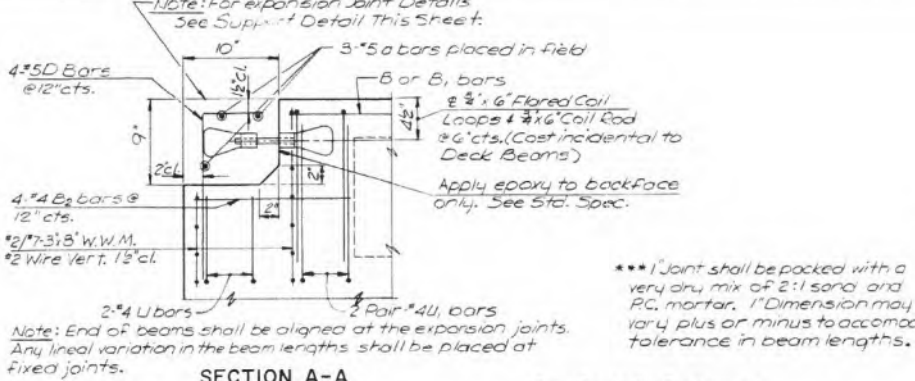
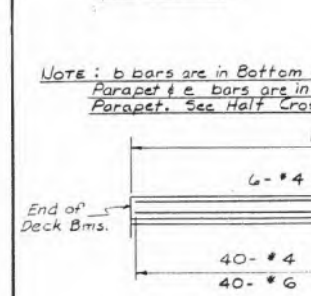
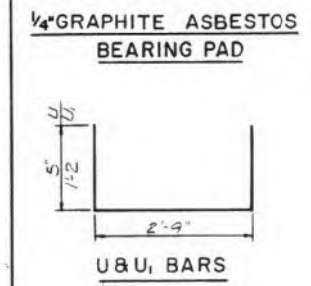
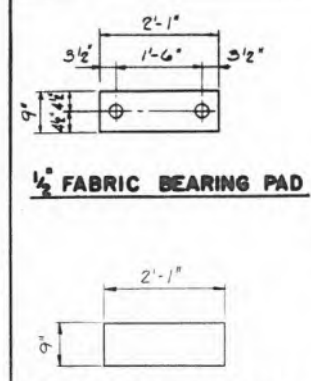
BILL OF MATERIALS FOR 2-ABUTS. & 3-PIERS

BAR	NO.	SIZE	LENGTH	SHAPE
F	8	#6	8'-0"	—
F1	16	#6	7'-10"	—
F2	16	#5	3'-9"	—
F3	8	#5	4'-2"	—
D	35	#7	30'-2"	—
S	153	#4	9'-8"	□
U	40	#6	10'-1"	—
V	16	#4	4'-4"	—
V1	24	#4	3'-6"	—
Class I Concrete				Cu Yd 397
Reinforcement Bars				Pounds 4250
FUP Concrete Pile				Lin. Ft. 864
Test Pile Concrete				Each 1

**ABUTMENTS & PIERS
OVERFLOW STRUCTURE AT PANTHER CREEK
PALESTINE ROAD DISTRICT
SECTION 77-00059-00-BR
WOODFORD COUNTY
STATION 29+40
102-3131**

BR-77-102

ROUTE NO. SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	WOODFORD	27	17
FED. AID DIST. NO.	ALLIANCE FED. AID PROJECT		
	Sec. 77-00059-00-BR		



BILL OF MATERIAL

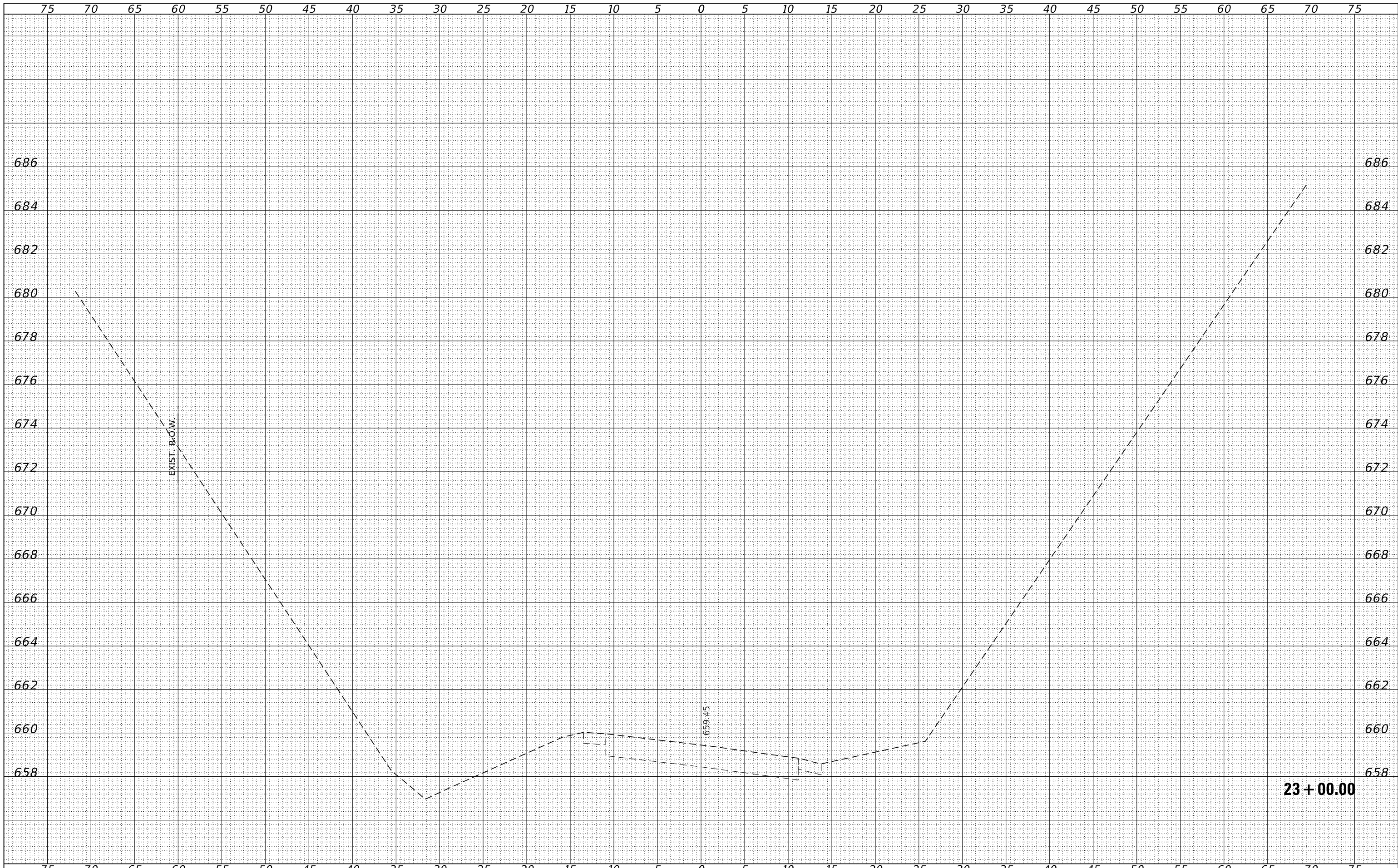
BAR	NO.	SIZE	LENGTH	SHAPE
B	4	#5	38'-7"	—
B ₁	4	#5	8'-0"	—
B ₂	4	#4	15'-3"	—
D	4	#5	17'-9"	—
U	2	#4	37'-9"	—
U ₁	10	#4	51'-9"	—
a	12	#5	29'-9"	—
b	32	#5	21'-0"	—
b ₁	32	#5	20'-6"	—
d	40	#4	37'-7"	—
d ₁	40	#6	37'-4"	—
e	96	#4	19'-7"	—
Class I Concrete				Cu. Yd. 21.8
Reinforcement Bars				Pound 4,110
P.P. Conc. Dk. Bm. 17" Sq. Ft.				476.0

*** Reinforcing List is for One Unit Only - In Precast.
NOTE: d & d₁ shall be placed in the Precast Plant in Exterior Beams Only.

SUPERSTRUCTURE OVERFLOW STRUCTURE AT PANTHER CREEK PALESTINE ROAD DISTRICT SECTION 77-00059-00-BR WOODFORD COUNTY STATION 29+40 102-3131

DATE	
BY	
FINISHED SURVEY	
NOTED SURVEY	
PLOTTED SURVEY	
TEMPLATE SURVEY	
AREAS SURVEY	
CHECKED SURVEY	

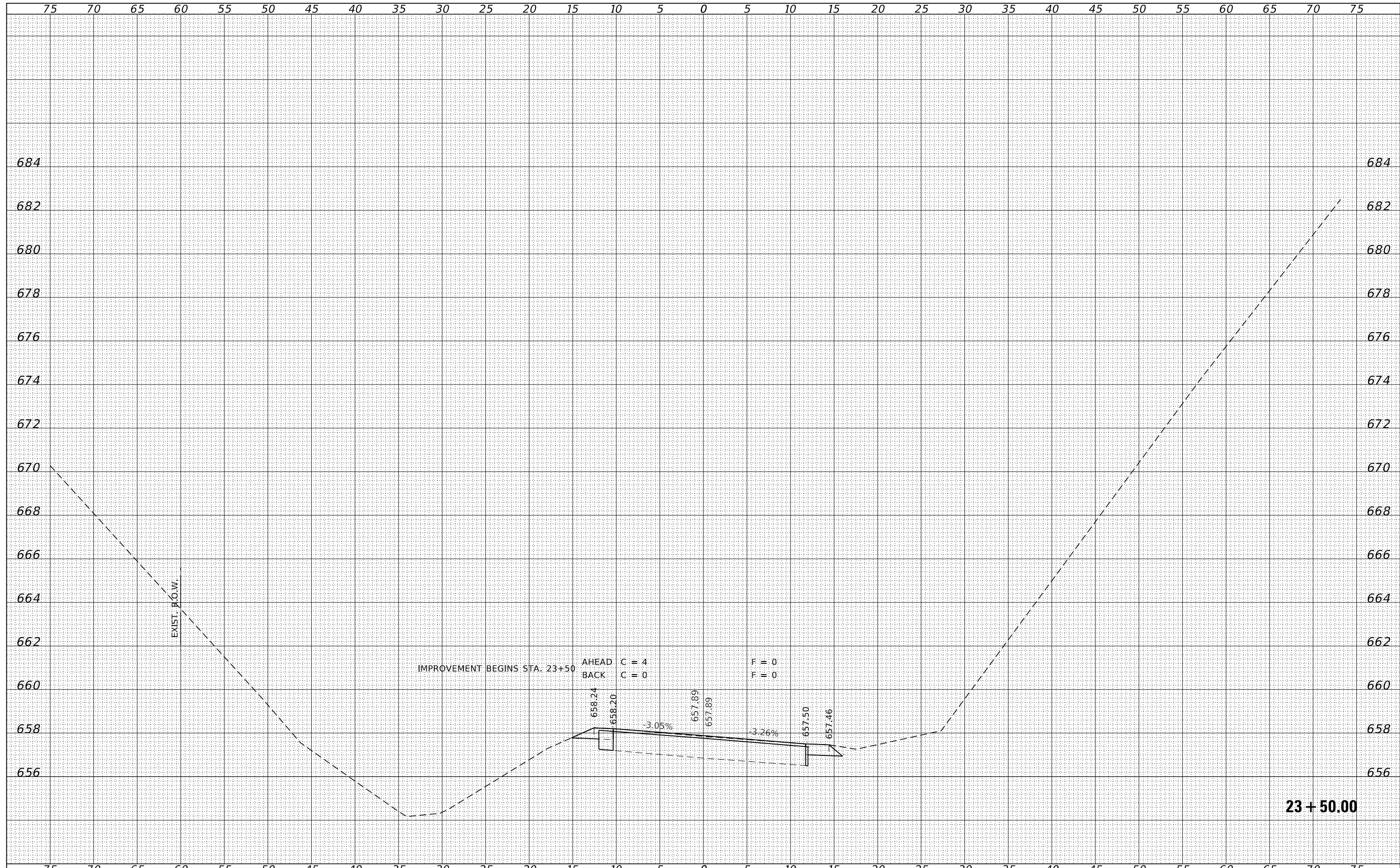
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HAMPTON, LENZINI AND RENWICK, INC. 3885 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 ILLINOIS PROFESSIONAL DESIGN FIRM L.S. / P.E. / S.E. CORP. 184.000958	PLOT SCALE = \$\$SCALE\$	DRAWN - R.D.H.	REVISIONS -		383	18-00166-00-BR	WOODFORD	65	42			
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DATE - 01/12/2023	REVISIONS -	ILLINOIS	FED. AID PROJECT ZH0Y(896)									

DATE	
BY	
SURVEYED	
PLOTTED	
TEMPLATE	
AREAS	
CHECKED	
FINAL SURVEY	
NOTE BOOK	
NO.	

DATE	
BY	
SURVEYED	
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ORIGINAL SURVEY	
NOTE BOOK	
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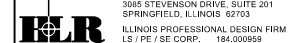
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STATE OF ILLINOIS
 WOODFORD COUNTY HIGHWAY DEPARTMENT

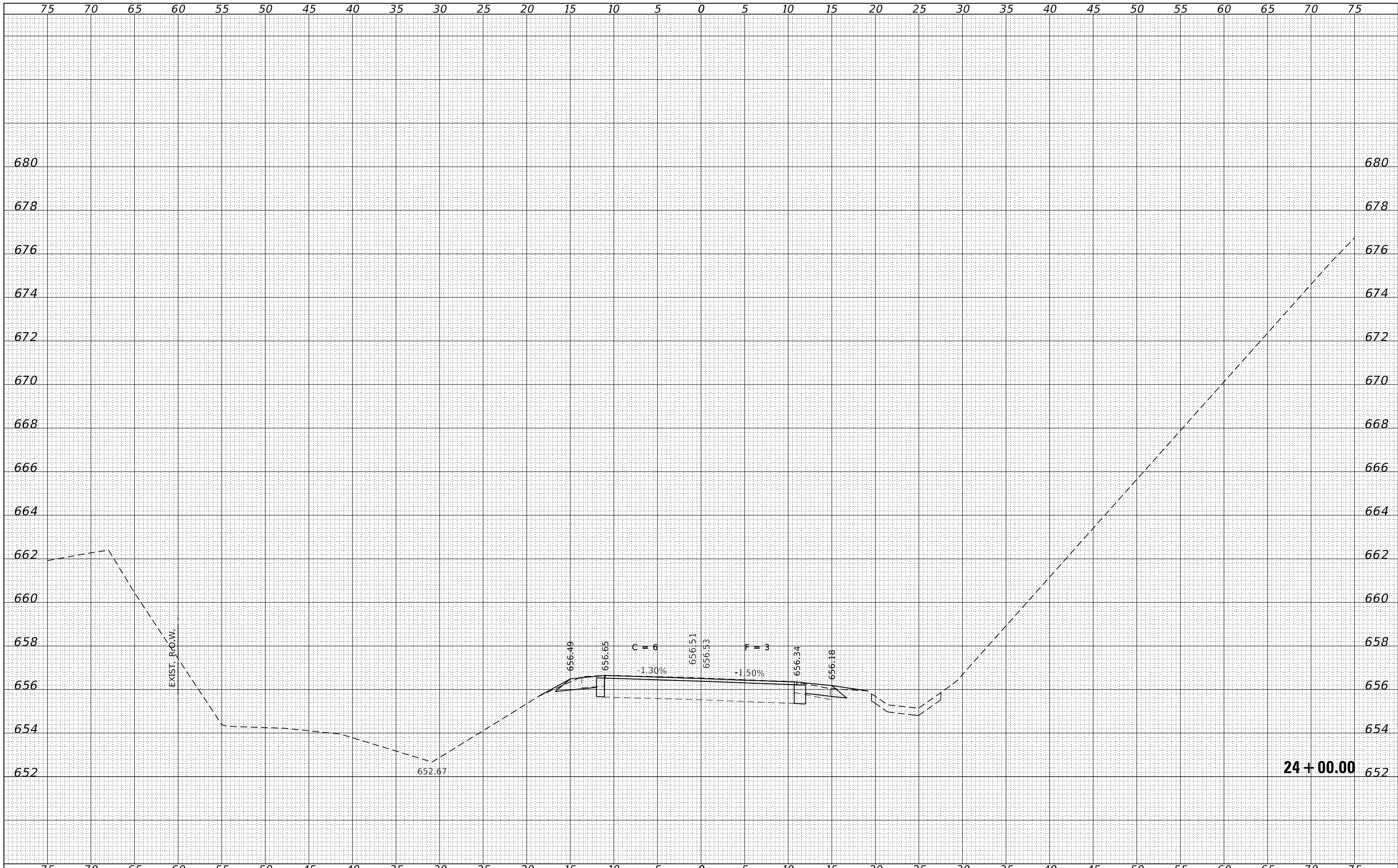
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 STA. 23+50.00 TO STA. 23+50.00

F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
383	18-00166-00-BR	WOODFORD	65	43
CONTRACT NO. 89779				
ILLINOIS FED. AID PROJECT ZH0Y(896)				



DATE	
BY	
FINAL SURVEY	SURVEYED
NOTE BOOK	PLOTTED
NO.	TEMPLATE
	AREAS
	CHECKED

DATE	
BY	
ORIGINAL SURVEY	SURVEYED
NOTE BOOK	PLOTTED
NO.	TEMPLATE
	AREAS
	CHECKED



24 + 00.00

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 PLOT DATE = 2/28/2023

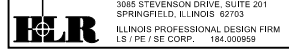
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 CHECKED - S.W.M.
 DATE - 01/12/2023

REVISIONS
 REVISION NO. | DATE | DESCRIPTION
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STATE OF ILLINOIS
WOODFORD COUNTY HIGHWAY DEPARTMENT

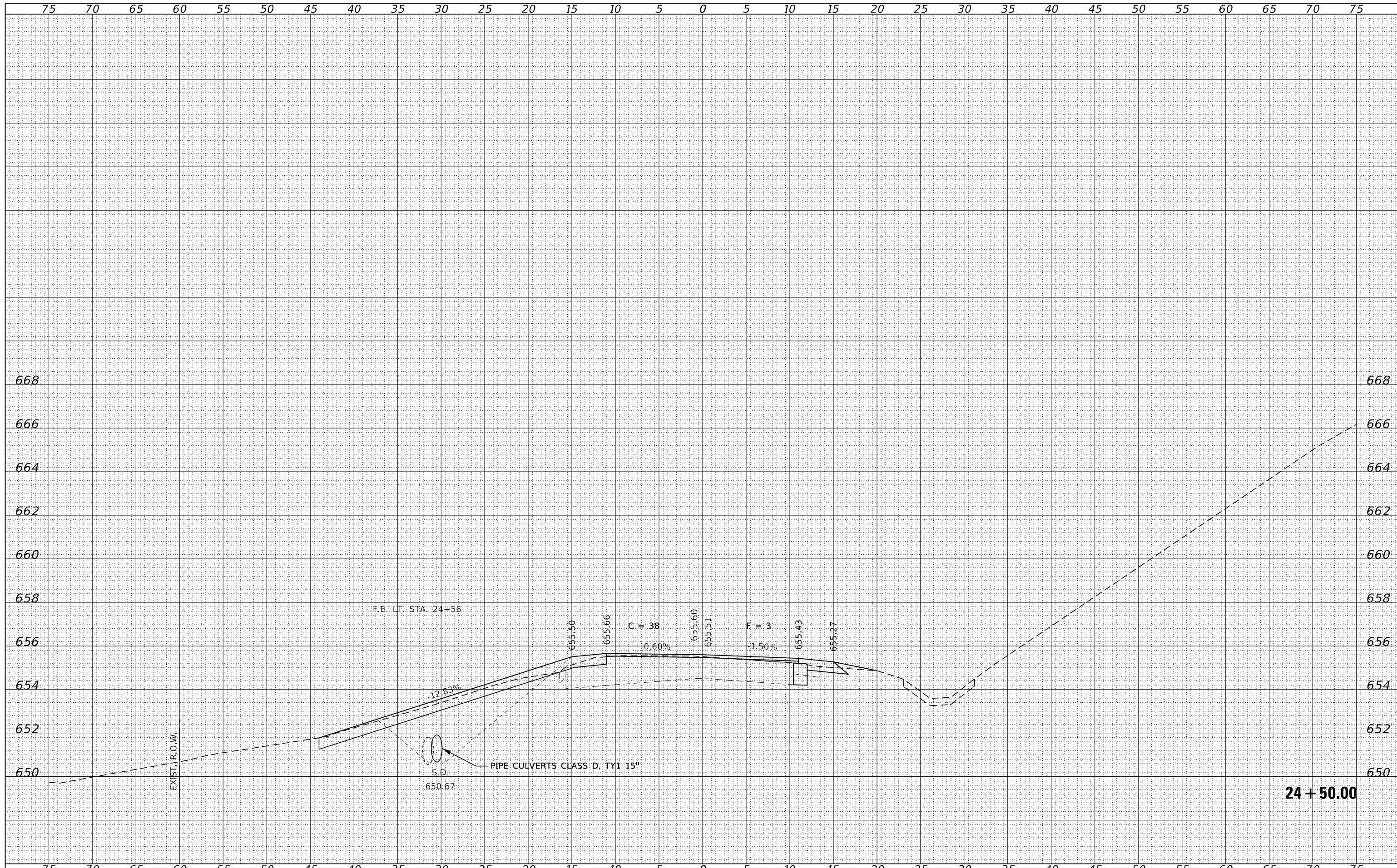
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 STA. 24+00.00 TO STA. 24+00.00

F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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CONTRACT NO. 89779				
ILLINOIS FED. AID PROJECT ZH0Y(896)				



BY	DATE

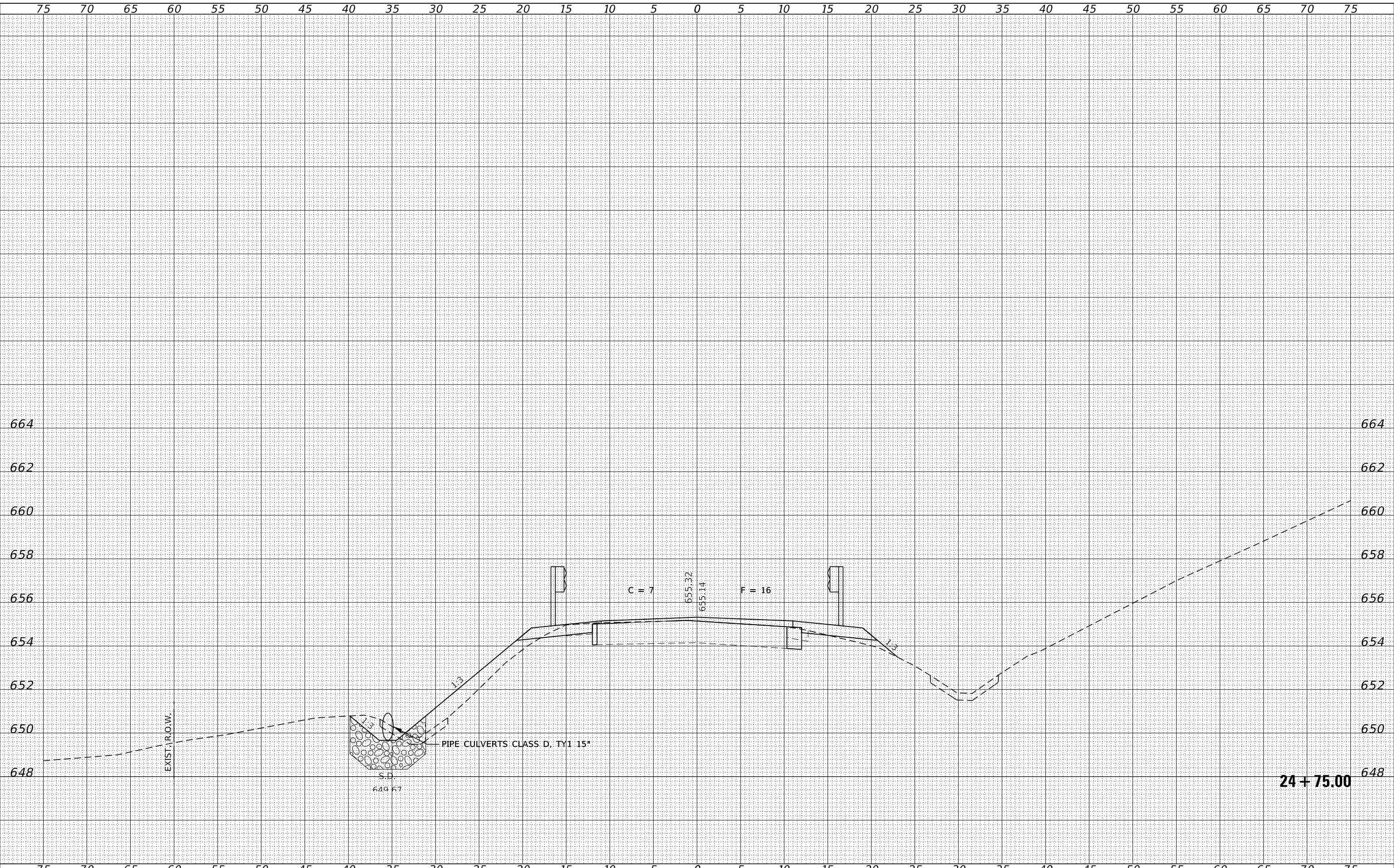
ORIGINAL SURVEY	DATE



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HAMPTON, LENZINI AND RENWICK, INC.		DRAWN - R.D.H.	REVISED -		383	18-00166-00-BR	WOODFORD	65	45
3888 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 ILLINOIS PROFESSIONAL DESIGN FIRM L/S / P/E / SE CORP. 184-009858	PLOT SCALE = \$Scales	CHECKED - S.W.M.	REVISED -		CONTRACT NO. 89779				
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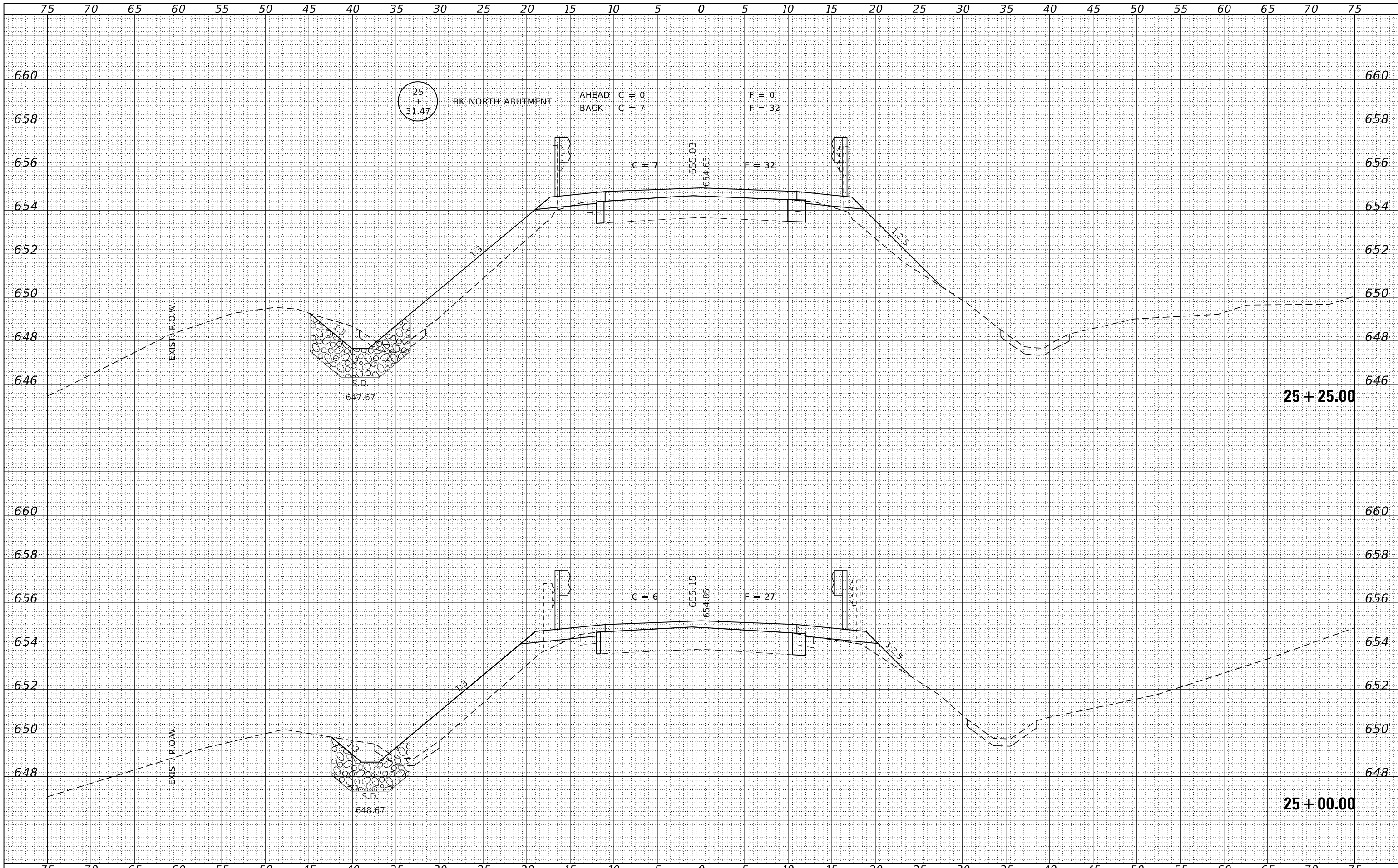
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BY	
ORIGINAL SURVEY	
NOTE BOOK NO.	
SURVEYED	
PLOTTED	
TEMPLATE	
AREAS CHECKED	



DATE	
BY	
SURVEYED	
PLOTTED	
TEMPLATE	
NOTE BOOK	
AREAS CHECKED	
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DATE	
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SURVEYED	
PLOTTED	
TEMPLATE	
NOTE BOOK	
AREAS CHECKED	
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DESIGNED - J.W.F.
 DRAWN - R.D.H.
 CHECKED - S.W.M.
 DATE - 01/12/2023

REVISED -
 REVISED -
 REVISED -
 REVISED -

STATE OF ILLINOIS
 WOODFORD COUNTY HIGHWAY DEPARTMENT

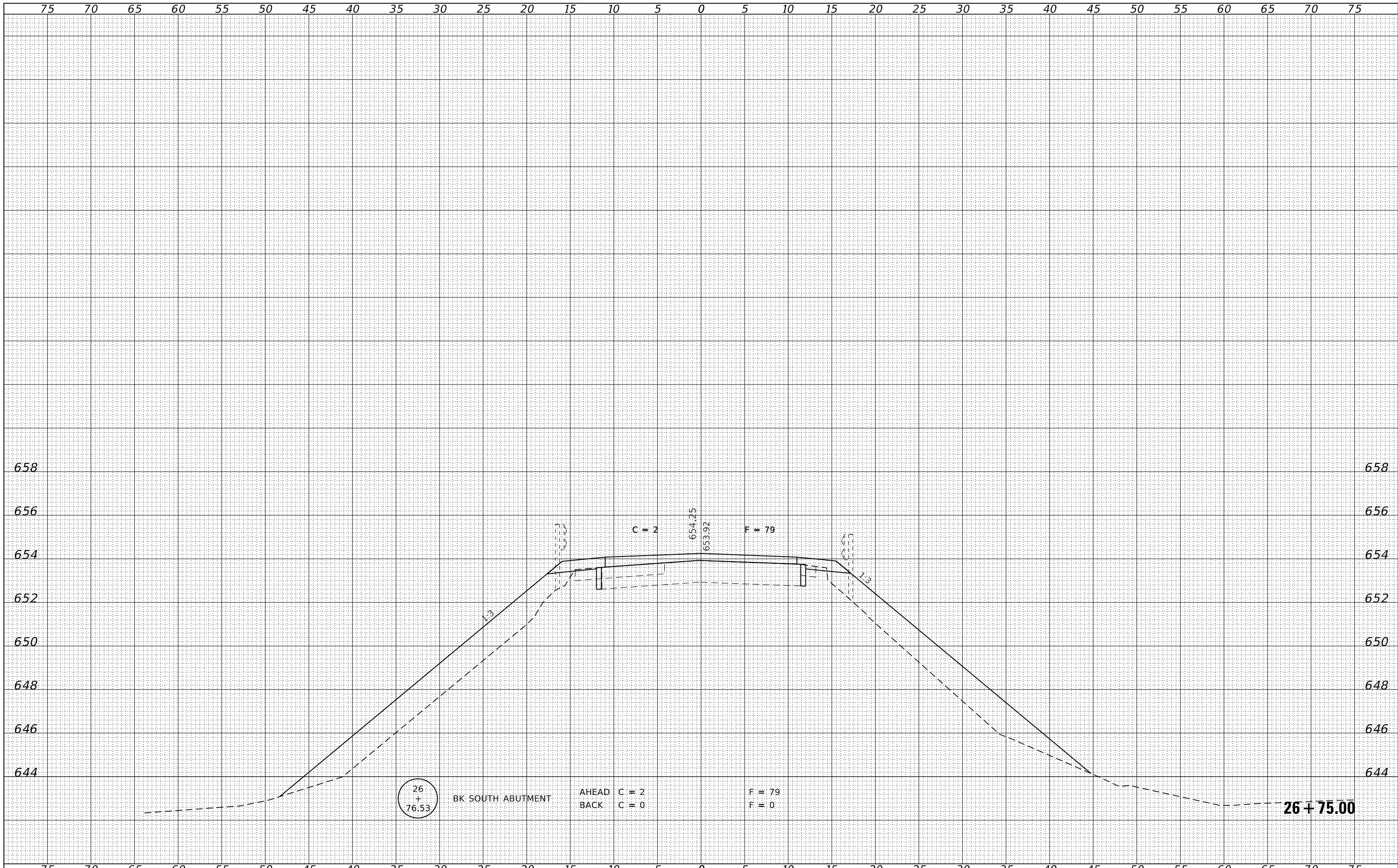
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 STA. 25+00.00 TO STA. 25+25.00

STATION CROSS SECTIONS

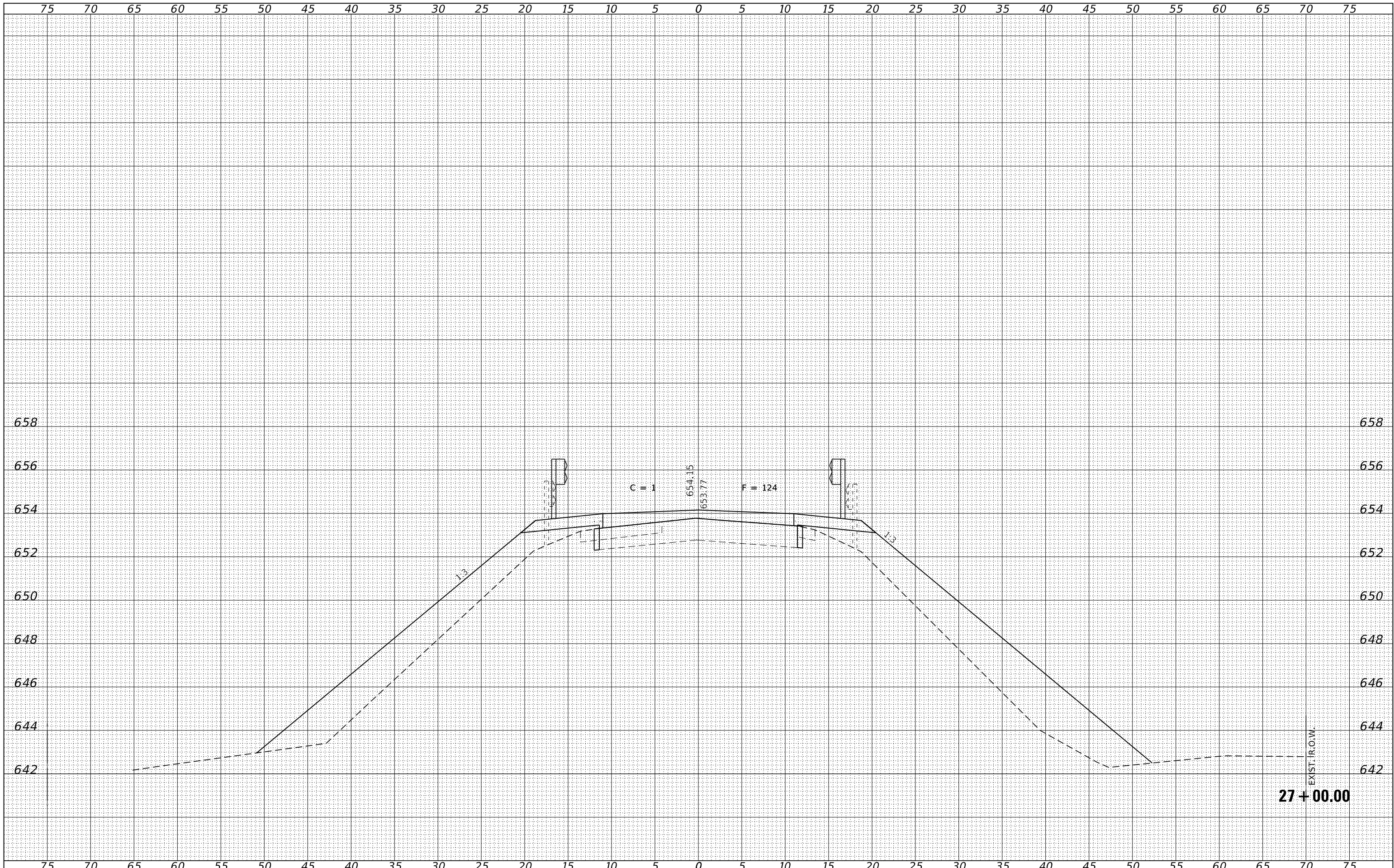
F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
383	18-00166-00-BR	WOODFORD	65	47
CONTRACT NO. 89779				
ILLINOIS FED. AID PROJECT ZH0Y(896)				

BY	DATE
FINAL SURVEY	SURVEYED
NOTE BOOK	PLOTTED
NO.	TEMPLATE
	AREAS
	CHECKED

BY	DATE
ORIGINAL SURVEY	SURVEYED
NOTE BOOK	PLOTTED
NO.	TEMPLATE
	AREAS
	CHECKED



FILE NAME = 190013-shl-xssheets.dgn	USER NAME = smlerzwa	DESIGNED - J.W.F.	REVISED -	STATE OF ILLINOIS WOODFORD COUNTY HIGHWAY DEPARTMENT	STATION CROSS SECTIONS		F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
HAMPTON, LENZINI AND RENWICK, INC. 3888 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 ILLINOIS PROFESSIONAL DESIGN FIRM LS / PE / SE CORP. 184.009958	PLOT SCALE = \$SCALE\$	DRAWN - R.D.H.	REVISED -		383	18-00166-00-BR	WOODFORD	65	48		
PLOT DATE = 2/28/2023	DATE - 01/12/2023	CHECKED - S.W.M.	REVISED -		CONTRACT NO. 89779		ILLINOIS FED. AID PROJECT 2H0Y(896)				
SCALE: 5H:2V	SHEET NO. 7 OF 24 SHEETS	DATE - 01/12/2023	REVISED -		STA. 26+75.00 TO STA. 26+75.00						



BY	DATE
FINAL SURVEY	SURVEYED
NOTE BOOK	PLOTTED
NO.	TEMPLATE
	AREAS
	CHECKED

BY	DATE
ORIGINAL SURVEY	SURVEYED
NOTE BOOK	PLOTTED
NO.	TEMPLATE
	AREAS
	CHECKED

FILE NAME = 190013-sh1-xssheets.dgn
 DESIGNED - J.W.F.
 DRAWN - R.D.H.
 CHECKED - S.W.M.
 DATE - 01/12/2023

USER NAME = smlerzwa
 PLOT SCALE = \$SCALE\$
 PLOT DATE = 2/28/2023

DESIGNED - J.W.F.
 DRAWN - R.D.H.
 CHECKED - S.W.M.
 DATE - 01/12/2023

REVISIONS
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**STATE OF ILLINOIS
 WOODFORD COUNTY HIGHWAY DEPARTMENT**

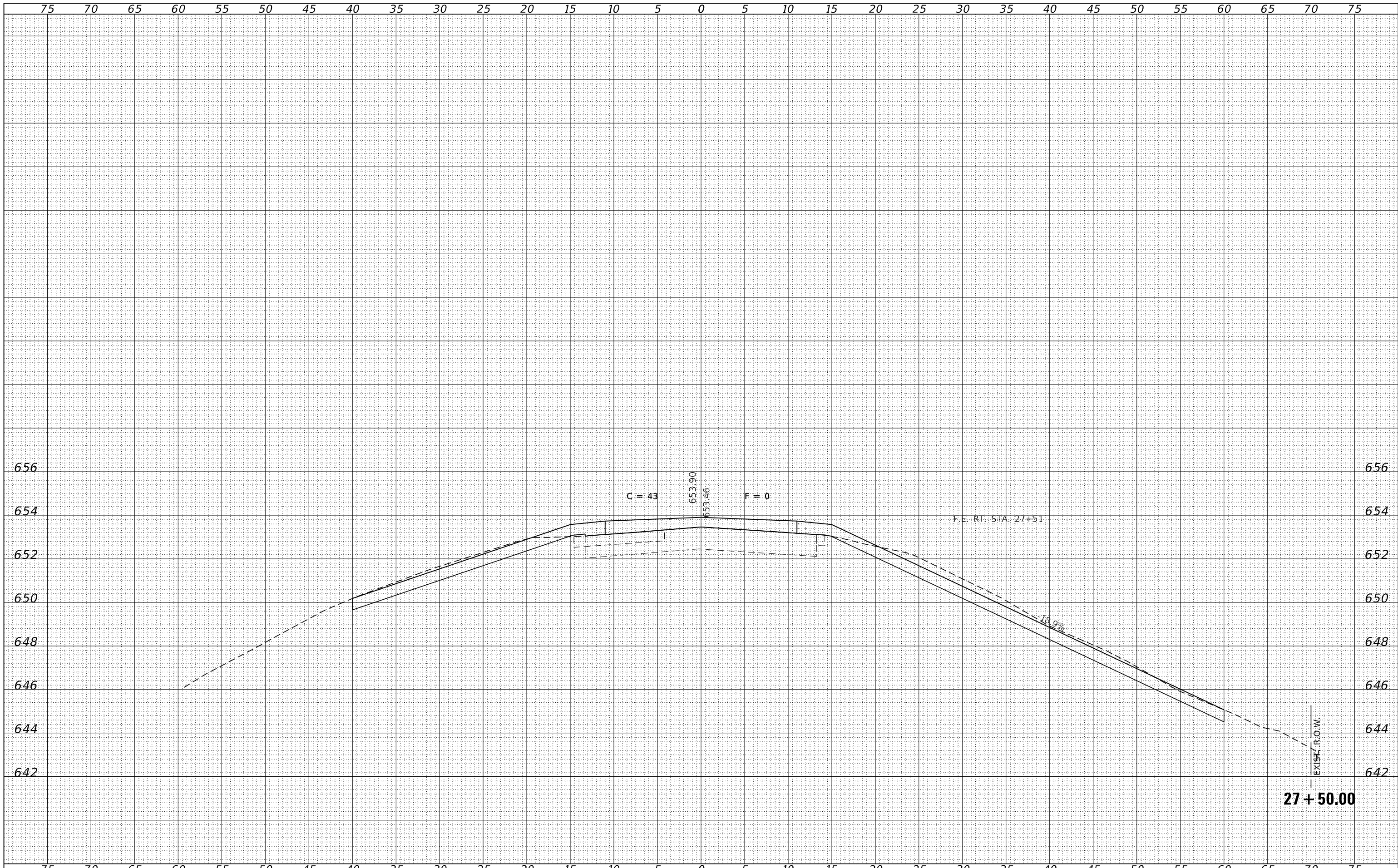
STATION CROSS SECTIONS

SCALE: 5H:2V SHEET NO. 8 OF 24 SHEETS STA. 27+00.00 TO STA. 27+00.00

F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
383	18-00166-00-BR	WOODFORD	65	49
CONTRACT NO. 89779				
ILLINOIS FED. AID PROJECT ZHOY(896)				

BY	DATE
FINAL SURVEY NO.	SURVEYED PLOTTED AREAS CHECKED
	NOTE BOOK TEMPLATE AREAS CHECKED

BY	DATE
ORIGINAL SURVEY NO.	SURVEYED PLOTTED AREAS CHECKED
	NOTE BOOK TEMPLATE AREAS CHECKED

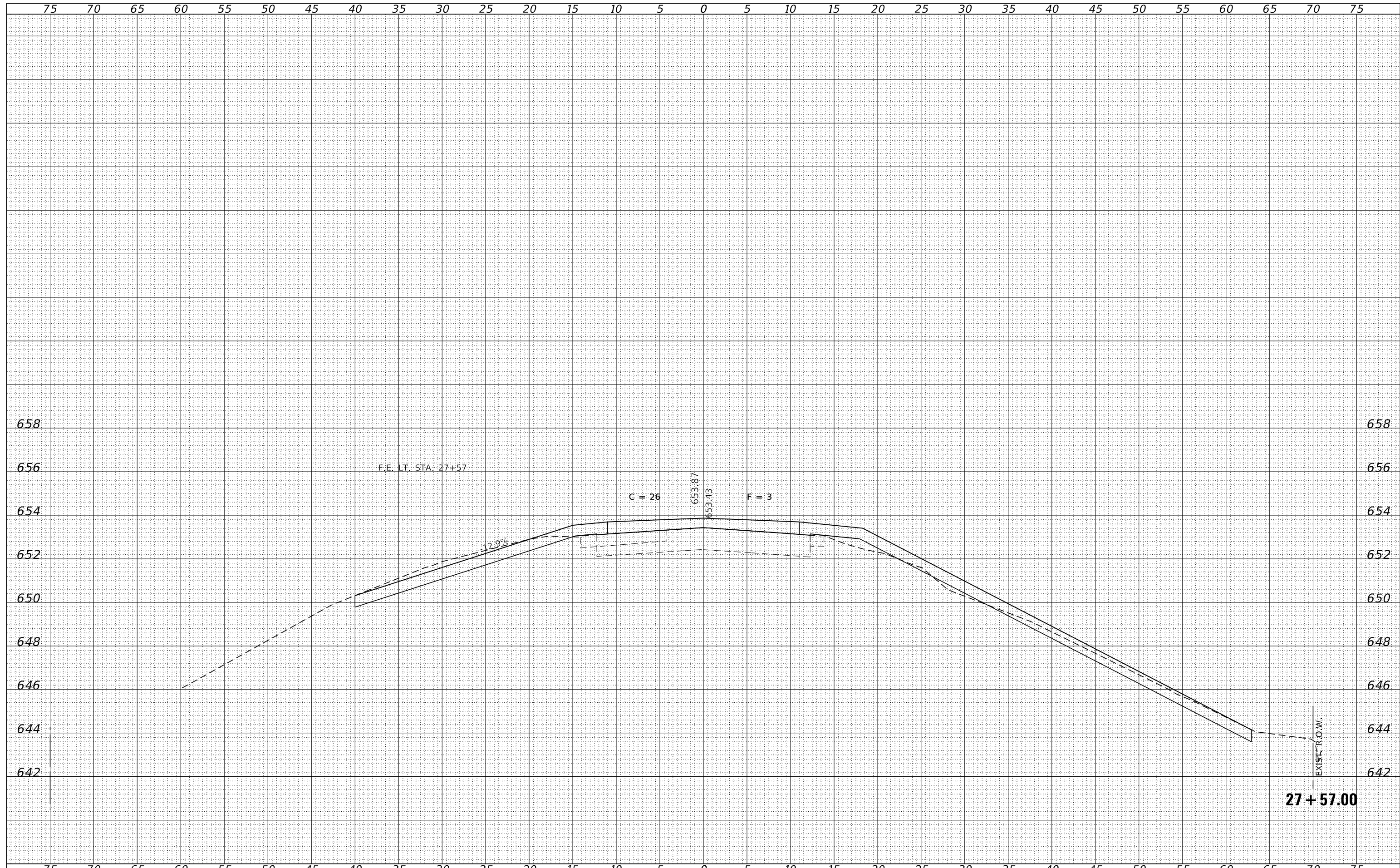


FILE NAME = 190013-shl-xssheets.dgn	USER NAME = smlerzwa	DESIGNED - J.W.F.	REVISD -	<p align="center">STATE OF ILLINOIS WOODFORD COUNTY HIGHWAY DEPARTMENT</p> <p align="center">STATION CROSS SECTIONS</p>	F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
HAMPTON, LENZINI AND RENWICK, INC. 3888 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 ILLINOIS PROFESSIONAL DESIGN FIRM L/S / PE / SE CORP. 184.000958	PLOT SCALE = \$SCALES	DRAWN - R.D.H.	REVISD -		383	18-00166-00-BR	WOODFORD	65	51
PLOT DATE = 2/28/2023	CHECKED - S.W.M.	REVISD -	SCALE: 5H:2V		SHEET NO. 10 OF 24 SHEETS		STA. 27+50.00 TO STA. 27+50.00		CONTRACT NO. 89779
	DATE - 01/12/2023	REVISD -			ILLINOIS		FED. AID PROJECT ZH0Y(896)		

27 + 50.00

BY	DATE
FINAL SURVEY	SURVEYED
NOTE BOOK	PLOTTED
NO.	TEMPLATE
	AREAS
	CHECKED

BY	DATE
ORIGINAL SURVEY	SURVEYED
NOTE BOOK	PLOTTED
NO.	TEMPLATE
	AREAS
	CHECKED



FILE NAME = 190013-sh1-xssheets.dgn
 USER NAME = smlerzwa
HAMPTON, LENZINI AND RENWICK, INC.
 3888 STEVENSON DRIVE, SUITE 201
 SPRINGFIELD, ILLINOIS 62703
 ILLINOIS PROFESSIONAL DESIGN FIRM
 LS / PE / SE CORP. 184.009658

DESIGNED - J.W.F.
 DRAWN - R.D.H.
 CHECKED - S.W.M.
 DATE - 01/12/2023

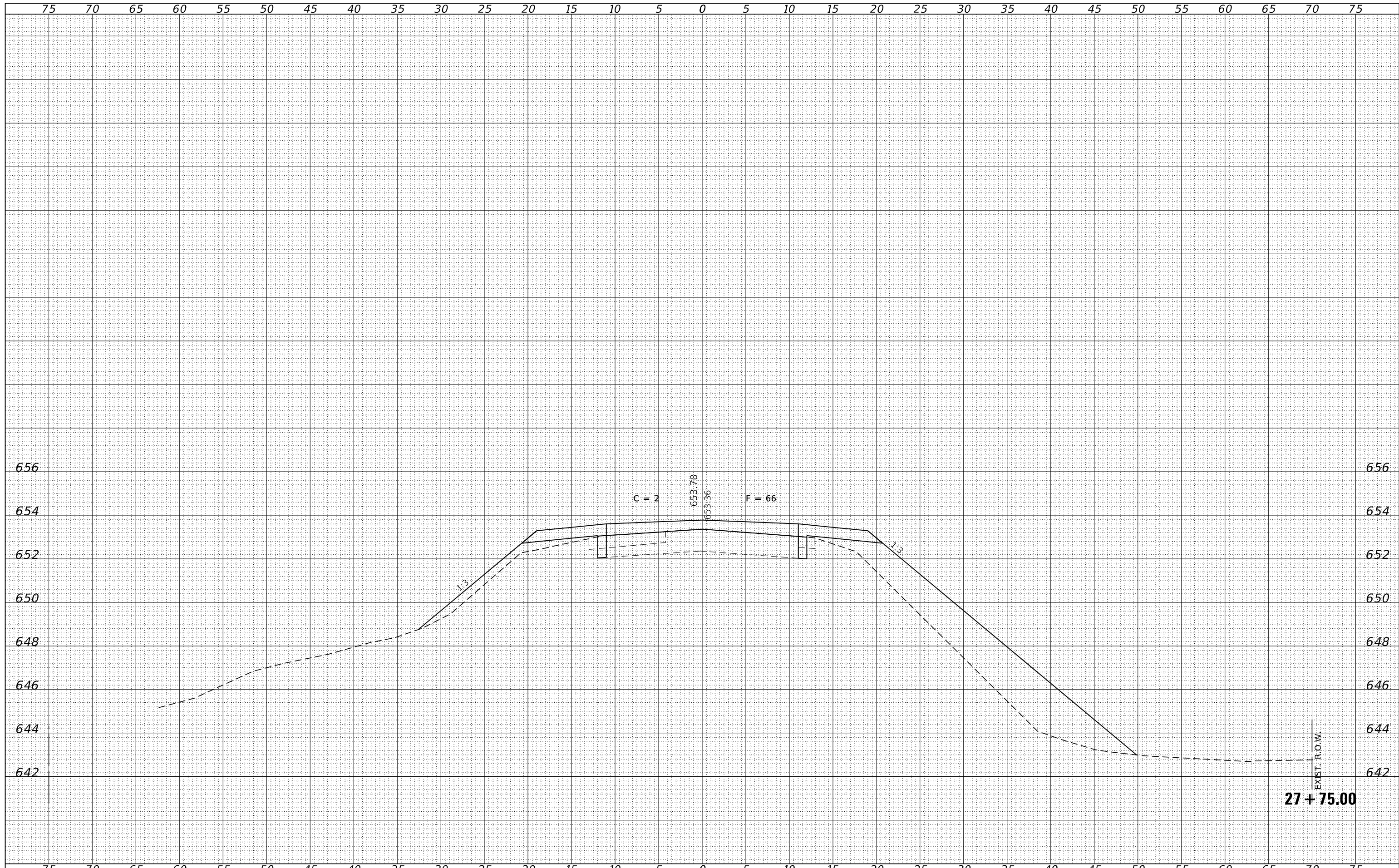
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STATE OF ILLINOIS
WOODFORD COUNTY HIGHWAY DEPARTMENT

STATION CROSS SECTIONS

SCALE: 5H:2V SHEET NO. 11 OF 24 SHEETS STA. 27+57.00 TO STA. 27+57.00

F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
383	18-00166-00-BR	WOODFORD	65	52
CONTRACT NO. 89779				
ILLINOIS FED. AID PROJECT ZH0Y(896)				



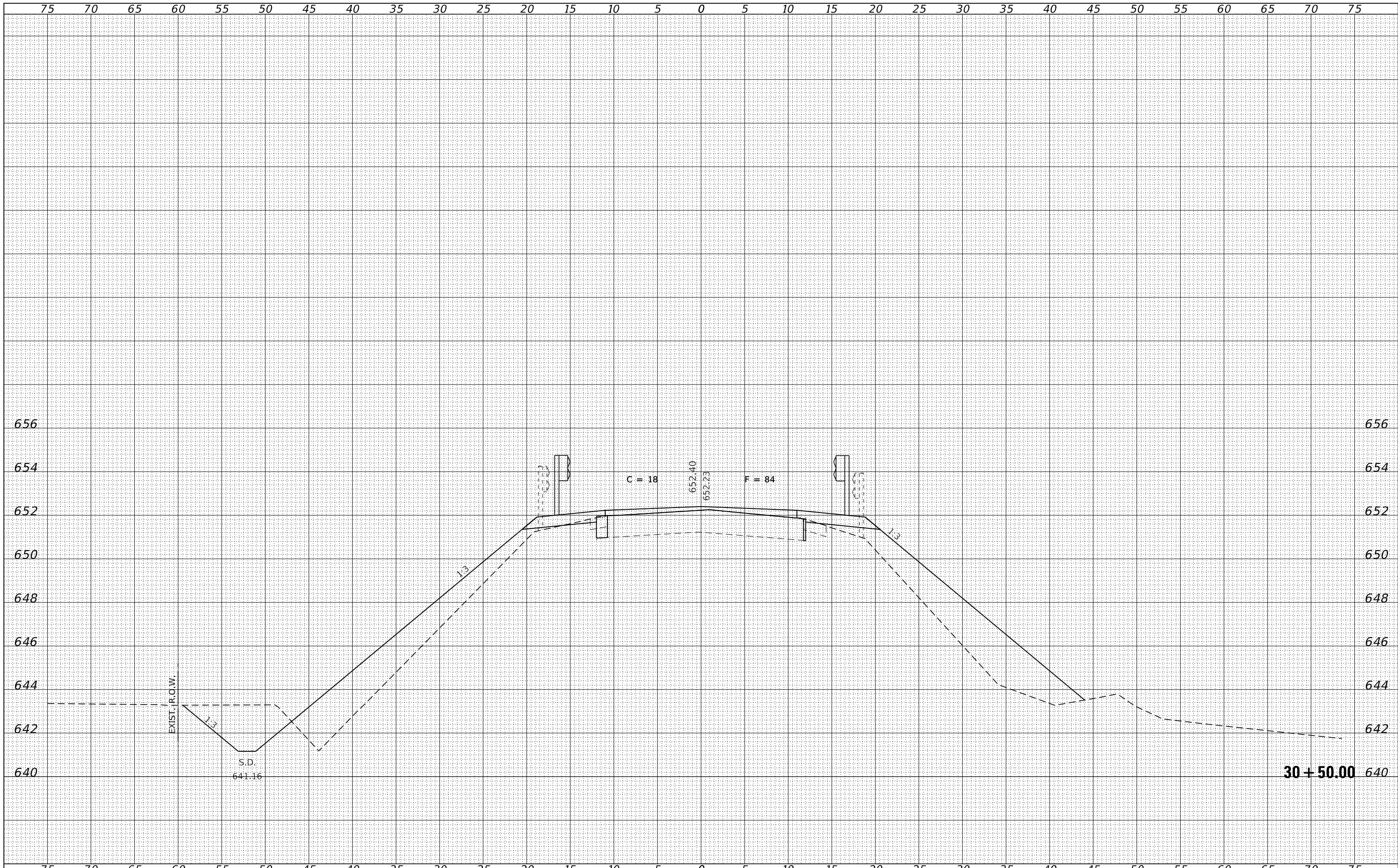
BY	DATE
FINAL SURVEY	SURVEYED
NOTE BOOK	PLOTTED
NO.	TEMPLATE
	AREAS CHECKED
	AREAS CHECKED

BY	DATE
ORIGINAL SURVEY	SURVEYED
NOTE BOOK	PLOTTED
NO.	TEMPLATE
	AREAS CHECKED
	AREAS CHECKED

FILE NAME = 190013-shl-xssheets.dgn	USER NAME = smlerzwa	DESIGNED - J.W.F.	REVISED -	<p align="center">STATE OF ILLINOIS WOODFORD COUNTY HIGHWAY DEPARTMENT</p>	STATION CROSS SECTIONS		F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
HAMPTON, LENZINI AND RENWICK, INC. 3888 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703 ILLINOIS PROFESSIONAL DESIGN FIRM LS / PE / SE CORP. 184-009658	PLOT SCALE = \$SCALES	DRAWN - R.D.H.	REVISED -		383	18-00166-00-BR	WOODFORD	65	53	CONTRACT NO. 89779		
PLOT DATE = 2/28/2023	DATE - 01/12/2023	CHECKED - S.W.M.	REVISED -		SCALE: 5H:2V	SHEET NO. 12 OF 24 SHEETS	STA. 27+75.00 TO STA. 27+75.00	ILLINOIS FED. AID PROJECT ZHOY(896)				
		REVISIONS										

BY	DATE
FINAL SURVEY	SURVEYED
NOTE BOOK	PLOTTED
NO.	TEMPLATE
	AREAS CHECKED
	AREAS CHECKED

BY	DATE
ORIGINAL SURVEY	SURVEYED
NOTE BOOK	PLOTTED
NO.	TEMPLATE
	AREAS CHECKED
	AREAS CHECKED



FILE NAME = 190013-shl-vssheets.dgn
 3888 STEVENSON DRIVE, SUITE 201
HAMPTON, LENZINI AND RENWICK, INC.
 SPRINGFIELD, ILLINOIS 62703
 ILLINOIS PROFESSIONAL DESIGN FIRM
 LS / PE / SE CORP. 184.009558

USER NAME = smlerzwa
 PLOT SCALE = \$\$SCALE\$
 PLOT DATE = 2/28/2023

DESIGNED - J.W.F.
 DRAWN - R.D.H.
 CHECKED - S.W.M.
 DATE - 01/12/2023

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**STATE OF ILLINOIS
 WOODFORD COUNTY HIGHWAY DEPARTMENT**

STATION CROSS SECTIONS

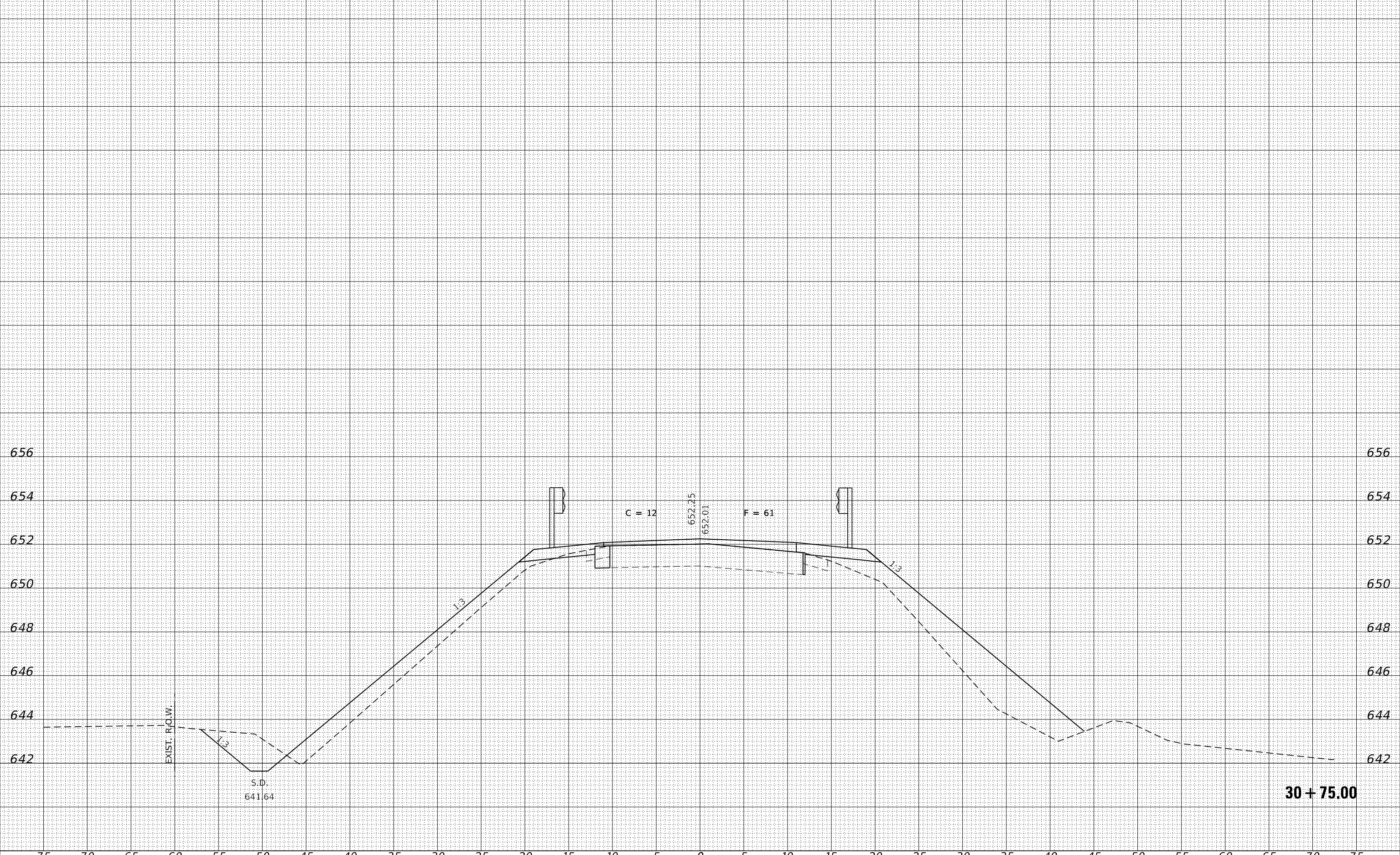
SCALE: 5H:2V SHEET NO. 17 OF 24 SHEETS STA. 30+50.00 TO STA. 30+50.00

F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
383	18-00166-00-BR	WOODFORD	65	58
CONTRACT NO. 89779				
ILLINOIS FED. AID PROJECT ZH0Y(896)				

75 70 65 60 55 50 45 40 35 30 25 20 15 10 5 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75

DATE	
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FINAL SURVEY	SURVEYED
NOTE BOOK	PLOTTED
NO.	TEMPLATE
	AREAS
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DATE	
BY	DATE
ORIGINAL SURVEY	SURVEYED
NOTE BOOK	PLOTTED
NO.	TEMPLATE
	AREAS
	CHECKED



FILE NAME = 190013-sh1-ssheets.dgn
 3885 STEVENSON DRIVE, SUITE 201
HAMPTON, LENZINI AND RENWICK, INC.
 SPRINGFIELD, ILLINOIS 62703
 ILLINOIS PROFESSIONAL DESIGN FIRM
 LS / PE / SE CORP. 184.009558

USER NAME = smlerzwa
 DESIGNED - J.W.F.
 DRAWN - R.D.H.
 CHECKED - S.W.M.
 DATE - 01/12/2023
 PLOT SCALE = \$\$SCALE\$
 PLOT DATE = 2/28/2023

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**STATE OF ILLINOIS
 WOODFORD COUNTY HIGHWAY DEPARTMENT**

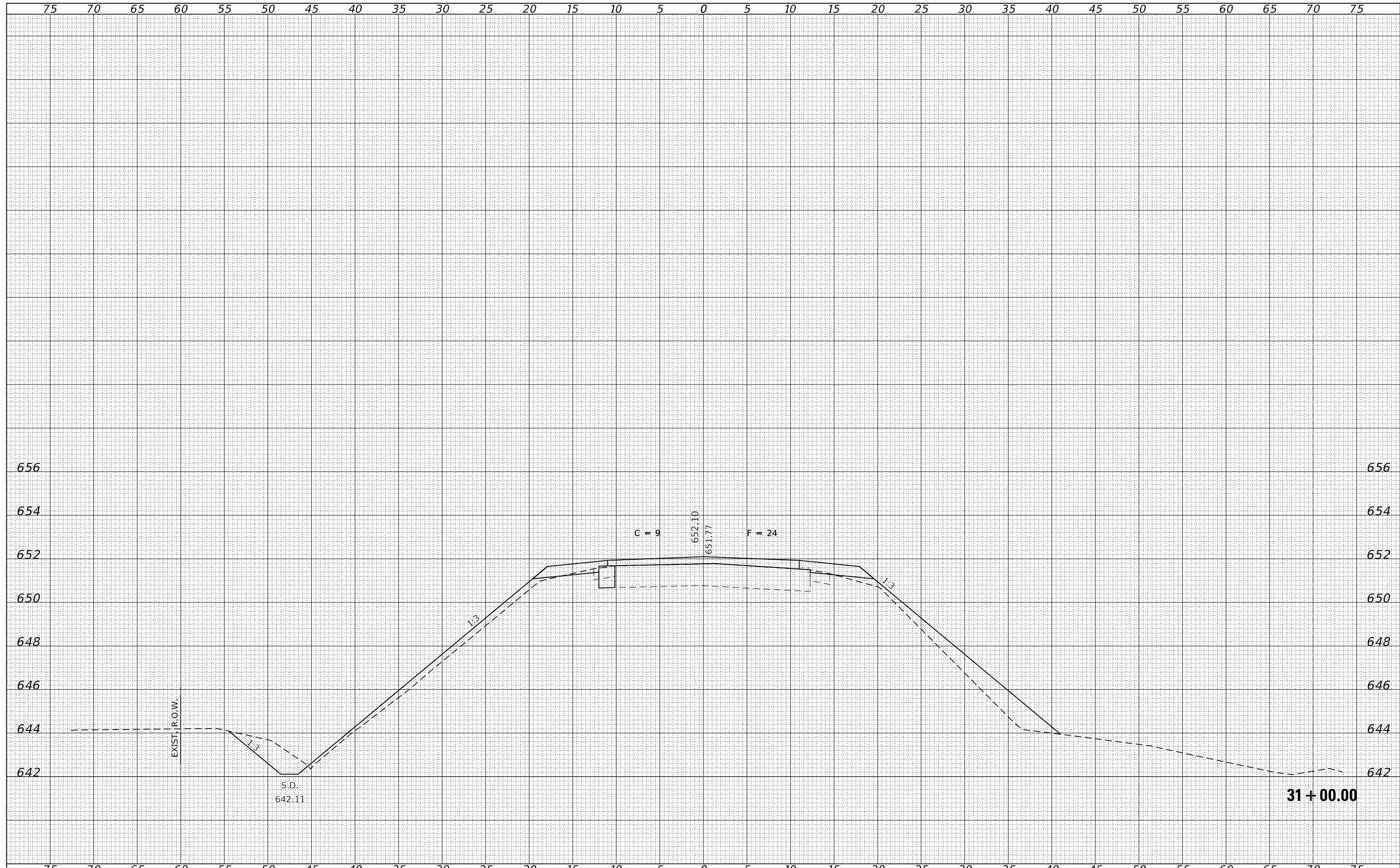
STATION CROSS SECTIONS

SCALE: 5H:2V SHEET NO. 18 OF 24 SHEETS STA. 30+75.00 TO STA. 30+75.00

F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
383	18-00166-00-BR	WOODFORD	65	59
CONTRACT NO. 89779				
ILLINOIS FED. AID PROJECT ZH0Y(896)				

DATE	
BY	
SURVEYED	
PLOTTED	
TEMPLATE	
AREAS	
CHECKED	
FINAL SURVEY	
NOTE BOOK	
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DATE	
BY	
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PLOTTED	
TEMPLATE	
AREAS	
CHECKED	
ORIGINAL SURVEY	
NOTE BOOK	
NO.	



FILE NAME = 190013-sh1-xssheets.dgn
 USER NAME = smlerzwa
 DESIGNED - J.W.F.
 DRAWN - R.D.H.
 CHECKED - S.W.M.
 DATE - 01/12/2023
 PLOT SCALE = \$SCALE\$
 PLOT DATE = 2/28/2023

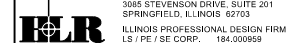
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STATE OF ILLINOIS
 WOODFORD COUNTY HIGHWAY DEPARTMENT

STATION CROSS SECTIONS

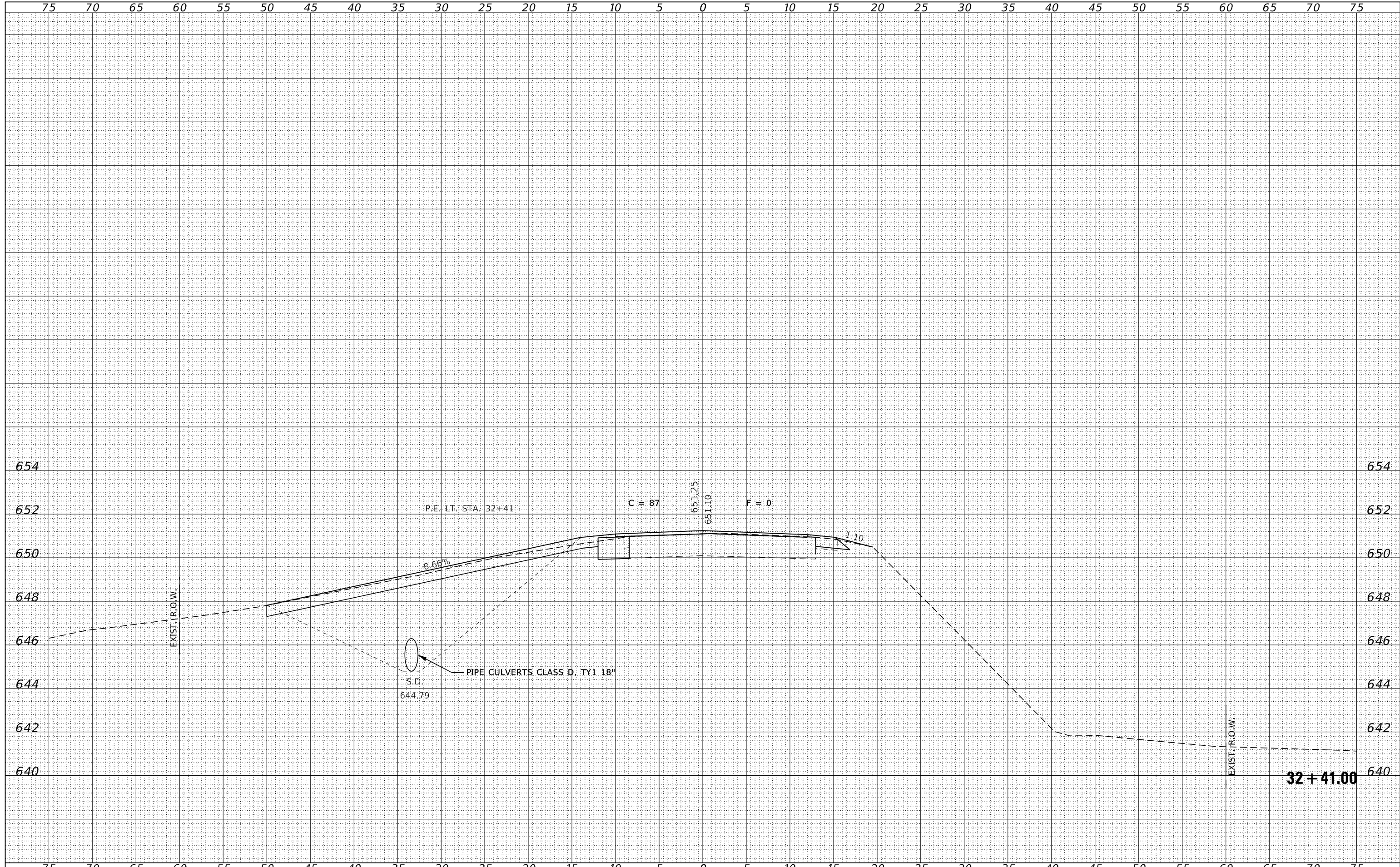
SCALE: 5H:2V SHEET NO. 19 OF 24 SHEETS STA. 31+00.00 TO STA. 31+00.00

F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
383	18-00166-00-BR	WOODFORD	65	60
CONTRACT NO. 89779				
ILLINOIS FED. AID PROJECT ZHOY(896)				



BY	DATE

ORIGINAL SURVEY	SURVEYED
NOTE BOOK	PLOTTED
NO.	TEMPLATE
	AREAS
	CHECKED



FILE NAME = 190013-shl-xssheets.dgn
 USER NAME = smlerzwa
 DESIGNED - J.W.F.
 DRAWN - R.D.H.
 CHECKED - S.W.M.
 DATE - 01/12/2023
 PLOT SCALE = \$SCALE\$
 PLOT DATE = 2/28/2023

REVISIONS:
 REVISION NO. | DATE | DESCRIPTION
 1 | |
 2 | |
 3 | |
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**STATE OF ILLINOIS
 WOODFORD COUNTY HIGHWAY DEPARTMENT**

STATION CROSS SECTIONS

SCALE: 5H:2V SHEET NO. 22 OF 24 SHEETS STA. 32+41.00 TO STA. 32+41.00

F.A.S.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
383	18-00166-00-BR	WOODFORD	65	63
CONTRACT NO. 89779				
ILLINOIS FED. AID PROJECT ZH0Y(896)				

