

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

**PROPOSED
HIGHWAY PLANS**
FAI ROUTE 72 (I 72)
SECTION (58,74-66)&(74-67&68)RS-1
MACON & PIATT COUNTY

C-95-056-99

RESURFACING

PIATT COUNTY LINE TO WHITE HEATH RD (FAS 1532) NE OF WHITE HEATH

STATION EQUATION:
1499+41.06 (BK) =
1162+63.35 (AH)

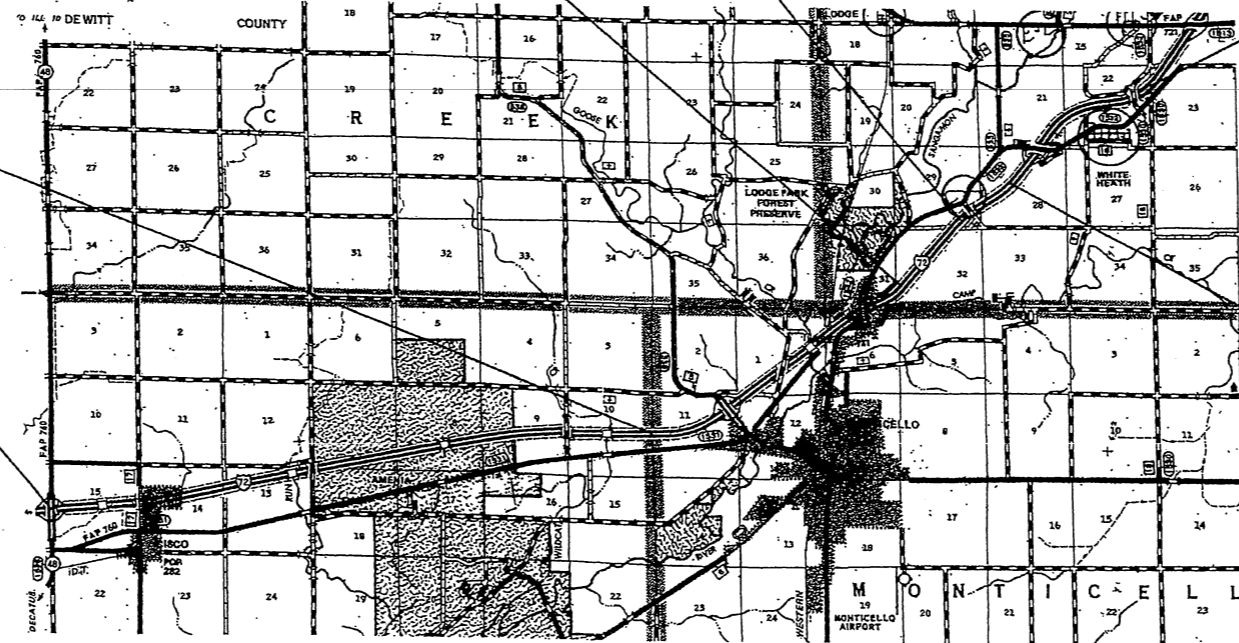
END URBAN LIMITS
STATION 1216+11.00

PROJECT ENDS
STATION 1376+84.00

STATION EQUATION:
1265+71.09 (BK) =
1265+30.54 (AH)

BEGIN URBAN LIMITS
STATION 1308+92.00

PROJECT BEGINS
STATION 910+00.00



F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
72	*	PIATT	251	1

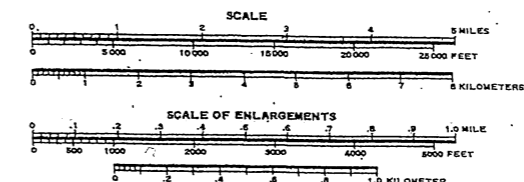
D-95-028-99



FOR UNDERGROUND UTILITY LOCATIONS CALL TOLL FREE J.U.L.I.E. TELEPHONE NO. 1-800-892-0123

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
SUBMITTED 2/8 20 10
Donald E. Gowen
DEPUTY DIRECTOR OF HIGHWAYS, REGION THREE ENGINEER
March 19, 20 10
Scott E. Stett, P.E.
Acting ENGINEER OF DESIGN AND ENVIRONMENT
March 19, 20 10
Christine M. Reed, P.E.
DIRECTOR OF HIGHWAYS/CHIEF ENGINEER

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TOTAL LENGTH OF SECTION & PROJECT = 80,407.26 FEET = 15.229 MILES
NET LENGTH OF SECTION & PROJECT = 79,298.56 FEET = 15.019 MILES

FOR INDEX OF SHEETS, SEE SHEET NO. 2
FOR SUMMARY OF QUANTITIES, SEE SHEET NO. 6-13

CURRENT ADT: 2005
10,400 - I-72 WEST OF MONTICELLO
13,800 - I-72 FROM MONTICELLO TO IL 47

B-SMART REPAIR (MAINLINE) SN 074-0063 SN 074-0064 SN 074-0067
B-SMART REPAIR (OVERHEADS) SN 074-0055 SN 074-0058 SN 074-0062

STRUCTURE OMISSIONS (MAINLINE) SN 074-0067 (EB) STA. 1421+86.52 TO STA. 1428+63.44
SN 074-0068 (WB) STA. 1422+80.50 TO STA. 1429+57.42
SN 074-0069 (EB) STA. 1438+95.20 TO STA. 1439+18.28
SN 074-0070 (WB) STA. 1438+22.94 TO STA. 1440+46.02
SN 074-0001 (EB) STA. 1300+54.10 TO STA. 1302+62.80
SN 074-0002 (WB) STA. 1300+45.65 TO STA. 1302+54.29

STRUCTURE OMISSIONS (OVERHEADS) SN 074-0060 SN 074-0065 SN 074-0066 SN 074-0068 SN 074-0071
CRASHWALL EXTENSIONS SN 074-0055 SN 074-0058 SN 074-0060 SN 074-0062 SN 074-0065 SN 074-0066 SN 074-0071

IMPACT ATTENUATOR PLACEMENTS SN 074-0055 SN 074-0062 SN 074-0065 SN 074-0066
IMPACT ATTENUATOR UPGRADES SN 074-0058 SN 074-0060 SN 074-0071 SN 074-0035 SN 074-0026 SN 074-0036

SEE GENERAL NOTES FOR ADDITIONAL STRUCTURE INFORMATION

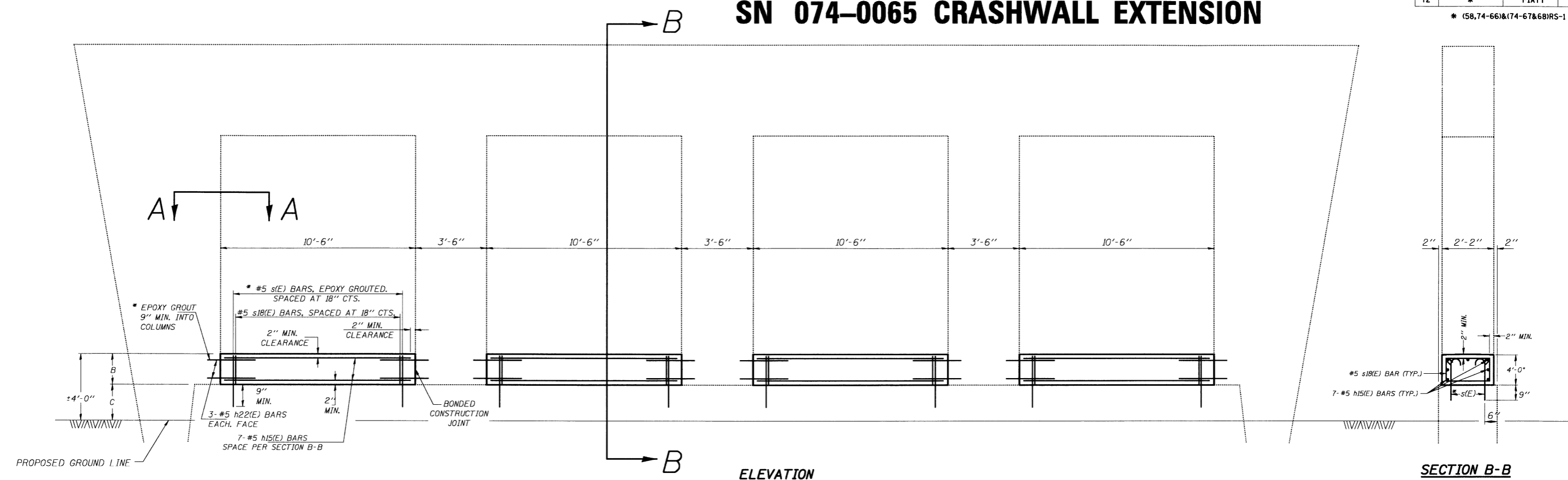
DESIGN DESIGNATION
INTERSTATE

CONTRACT NO. 70022 074-0065

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
72	*	PIATT	251	197

*(58,74-66)&(74-67&68)RS-1

SN 074-0065 CRASHWALL EXTENSION



ELEVATION

SECTION B-B

GENERAL NOTES

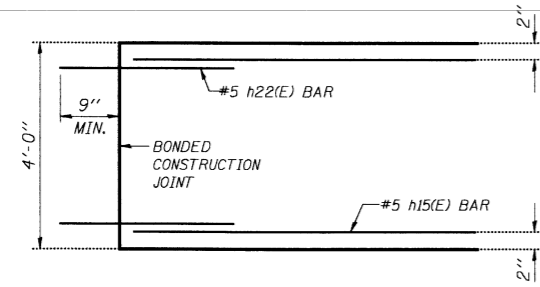
PLAN DIMENSIONS AND DETAILS RELATIVE TO EXISTING PLANS ARE SUBJECT TO ROUTINE VARIATIONS. THE CONTRACTOR SHALL FIELD VERIFY EXISTING DIMENSIONS AND DETAILS AFFECTING NEW CONSTRUCTION AND MAKE NECESSARY APPROVED ADJUSTMENTS PRIOR TO CONSTRUCTION OR ORDERING OF MATERIALS. SUCH VARIATIONS SHALL NOT BE CAUSE FOR ADDITIONAL COMPENSATION FOR A CHANGE IN SCOPE OF THE WORK. HOWEVER, THE CONTRACTOR WILL BE PAID FOR THE QUANTITY ACTUALLY FURNISHED BASED UPON THE UNIT PRICE BID FOR THE WORK.

THIS WORK WILL BE PAID FOR AT THE CONTRACT UNIT PRICE PER CUBIC YARD FOR CONCRETE STRUCTURES.

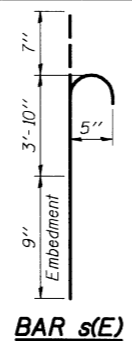
THE COST OF EPOXY GROUTING THREADED RODS SHALL BE INCLUDED WITH REINFORCEMENT BARS (EPOXY COATED).

REINFORCEMENT BARS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A706 OR 601L MODIFIED).

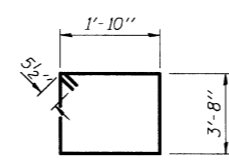
REINFORCEMENT BARS DESIGNATED (E) SHALL BE EPOXY COATED.



SECTION A-A



BAR s(E)



BAR s18(E)

* EPOXY GROUT BARS IN 9" MIN. HOLES ACCORDING TO ARTICLE 584 OF THE STANDARD SPECIFICATIONS.

CRASHWALL EXTENSION (TRAPEZOIDAL PIER)																									
STRUCTURE	STATION	DIMENSION		CONCRETE STRUCTURES (CU YD)	REINFORCEMENT BARS (EPOXY COATED)				REINFORCEMENT BARS (EPOXY COATED)																
		LOC.	B (FT)		C (FT)	BAR	QTY	SIZE	LENGTH	LBS	BAR	QTY	SIZE	LENGTH	SHAPE	LBS									
SN 074-0065	1359+51.04	MED	4'-0"	0'-0"	13.5	h15(E)	28	#5	10'-2"	296.2	h22(E)	48	#5	2'-9"	137.7	s(E)	56	#5	5'-2"	301.1	s18(E)	28	#5	11'-11"	347.1
CONCRETE STRUCTURES =					13.5	LBS = 296.2				LBS = 137.7				LBS = 301.1				LBS = 347.1							
										REINFORCEMENT BARS (EPOXY COATED) =				1082.1											

NOTE: MED INDICATES MEDIAN PIER.

SN 074-0065

ILLINOIS DEPARTMENT OF TRANSPORTATION
Crashwall Extensions
 F.A.I. ROUTE 72
 SECTION (58,74-66)&(74-67&68)RS-1
 PIATT COUNTY
 Sheet 5 of 7
 SCALE: NONE
 DATE: 12/15/09
 DRAWN BY:
 CHECKED BY: R.T.B.

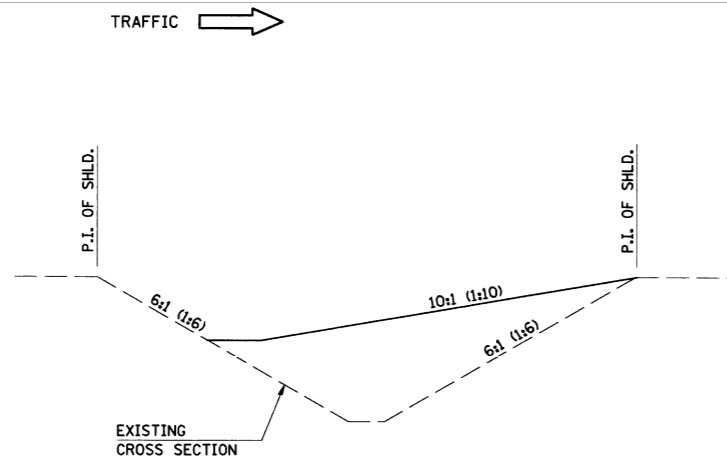
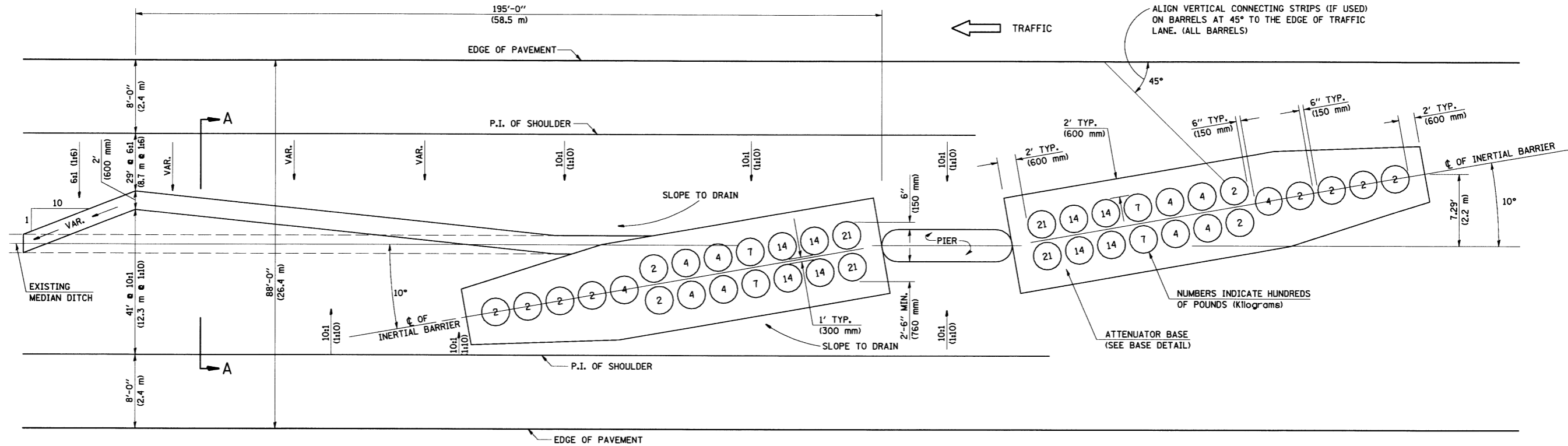
PLOT DATE = 2/6/2010
 PLOT NAME = I:\PROJECTS\PIATT\PERSONNEL\720289\dwg\sheet1.dwg
 PLOT SCALE = 1/8"=1'-0"
 USER NAME = r.t.b.

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
72	*	PIATT	251	207

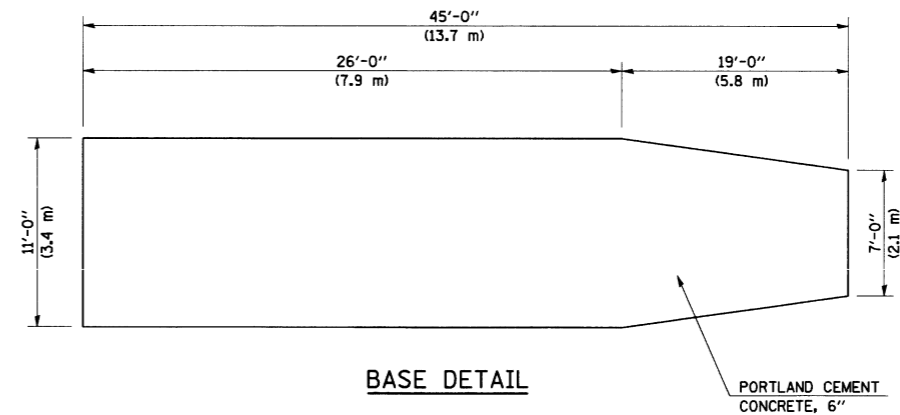
* (58,74-66)&(74-67&68)RS-1

70 MPH (110 km/h) DESIGN - 88' (26.4 m) MEDIAN

- STATION 932+59.22 (S. N. 074-0055)
- STATION 987+02.80 (S. N. 074-0058)
- STATION 1095+12.12 (S. N. 074-0060)
- STATION 1256+11.76 (S. N. 074-0062)
- STATION 1359+51.04 (S. N. 074-0065)
- STATION 1401+86.77 (S. N. 074-0066)



SECTION A-A
GRADING AND SHAPING DETAIL



BASE DETAIL

GENERAL NOTES

- ALL 10:1 (1:10) SLOPES SHOWN ON THIS DETAIL SHALL BE CONSTRUCTED 10:1 (1:10) OR FLATTER.
- THE SLOPES AS SHOWN ON THIS DETAIL SHALL APPLY TO BOTH ENDS OF THE BRIDGE PIERS.
- THE LENGTH X WIDTH OF MODULE LAYOUT IS 41.0' x 7.0' : 19 MODULES - 14,400 LBS. (12.5 m x 2.1 m : 19 MODULES - 6532 kg).
- IN AREAS OF 10:1 (1:10) SLOPES PRECEDING THE ATTENUATOR IN THE MEDIAN INSTALLATION, FOUR OR MORE WOOD POSTS SHALL BE PLACED AT 5' (1.5 m) INTERVALS IN THE MEDIAN \bar{C} . SEE SPECIAL PROVISIONS AND SCHEDULES.

Note: All dimensions are in INCHES (millimeters) unless otherwise shown.

DATE	REVISIONS	NAME

ILLINOIS DEPARTMENT OF TRANSPORTATION

IMPACT ATTENUATORS
(NON-REDIRECTIVE)
TEST LEVEL 3

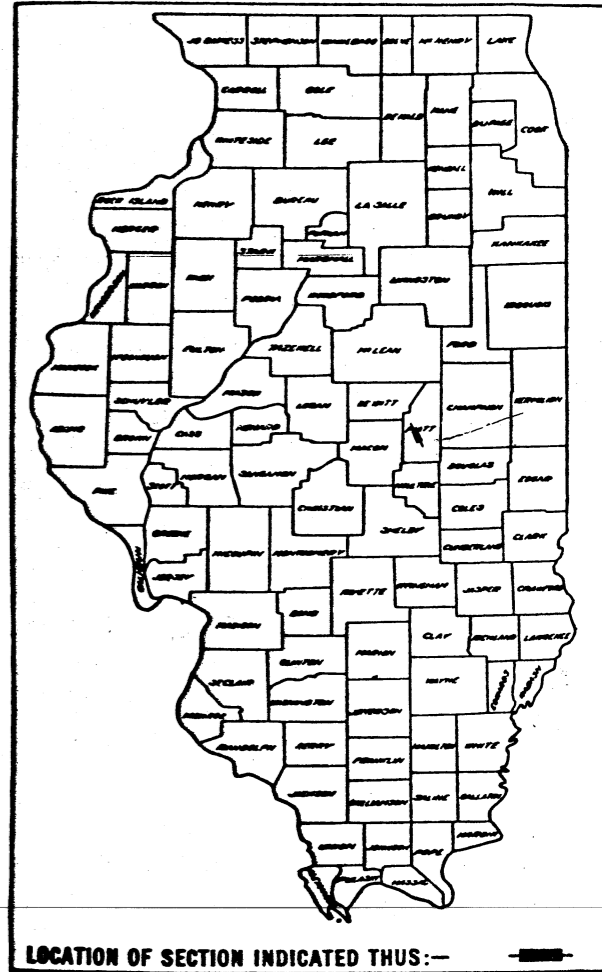
DISTRICT 5 DETAIL SPECIAL

PLOT DATE = 2/19/2018
 FILE NAME = c:\pwork\mtdot\PIERS\PIERS\PIERS.dgn
 PLOT SCALE = 1/8"=1'-0"
 USER NAME = p1111111

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

FEDERAL AID ROUTE NO.	SEC.	COUNTY	TOTAL SHEETS	SHEET NO.
FAI-72	74-68 HB-1	PIATT	36	1
S. P. W. REG. NO. 2		ILLINOIS	PROJECT I-72-2(52)60	

JOB NO. P-95-082-70



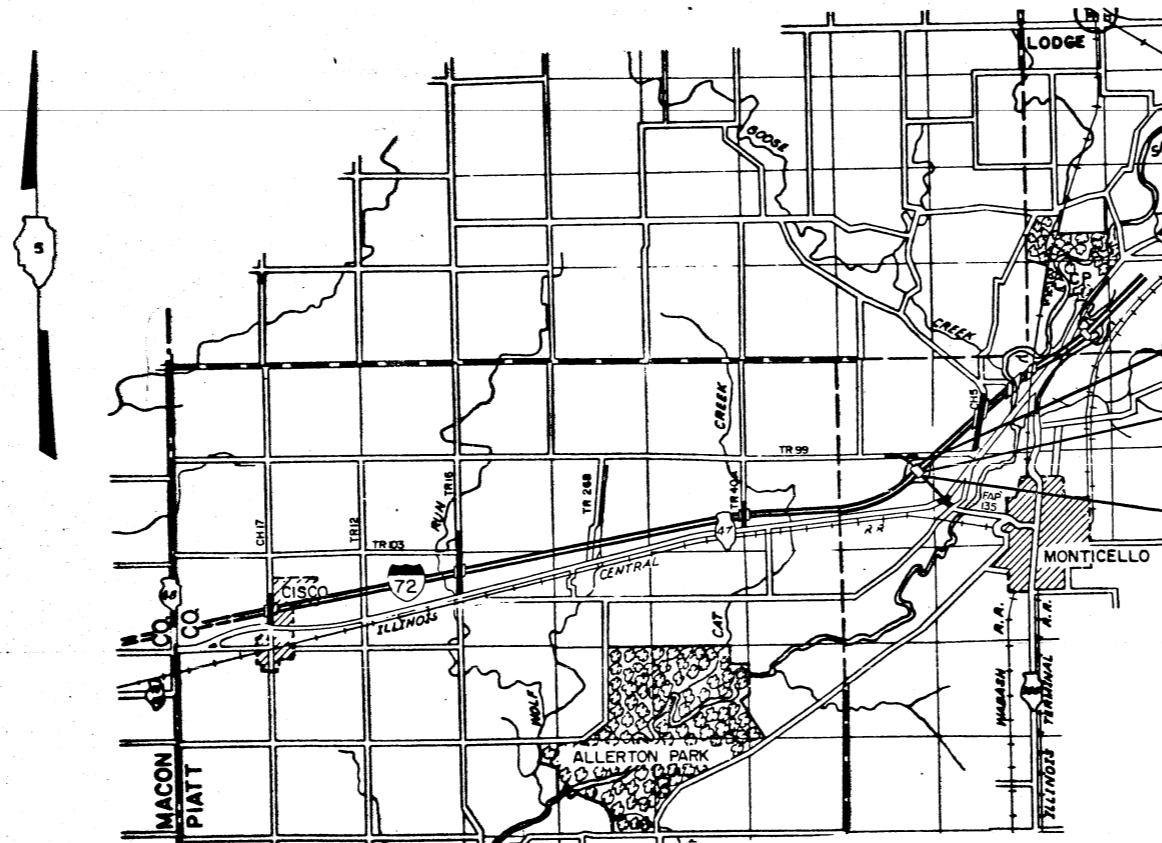
LOCATION OF SECTION INDICATED THUS:—

PLANS FOR PROPOSED
FEDERAL AID INTERSTATE HIGHWAY

SCALE IN FEET

MAIN LANES	PLAN & PROFILE	HOR.	0 100 200
MAIN LANES	PLAN & PROFILE	VERT.	0 10 20
CROSS ROADS	PLAN & PROFILE	HOR.	0 50 100
CROSS ROADS	PLAN & PROFILE	VERT.	0 5 10
CROSS-SECTIONS	CROSS-SECTIONS	HOR.	0 10 20
CROSS-SECTIONS	CROSS-SECTIONS	VERT.	0 5 10

F.A.I. ROUTE - 72 SECTION 74-68 HB-1
PIATT COUNTY
PROJECT I - 72-2(52)60
CONSTRUCTION JOB NO C-95-024-72



SECTION 74-68 HB-1 ENDS
STA. 51+24.67

STRUCTURE 74-68 HB-1
STA. 50+00.00 (L.F.A. 135)
STA. 1359+51.04 (L.F.A. 72)

SECTION 74-68 HB-1 BEGINS
STA. 48+75.33

STRUCTURE 74-68 HB-1, A TWO SPAN SEPARATION STRUCTURE CARRYING F.A.P. 135 OVER F.A.I. 72 AT STATION 1359+51.04. SPANS: 2 @ 125'-4" ROADWAY: 2 LANES @ 14' WIDTH, 18' MEDIAN

34
11 DEC 73
YIG

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SUBMITTED *Aug 13, 1973*
EXAMINED *Sept 26, 1973*
DESIGNED *Sept 26, 1973*
APPROVED *Sept 26, 1973*

J. C. Phillips DISTRICT ENGINEER
Ed. Williams ENGINEER OF PLANS AND CONTRACTS
W. Baumann ENGINEER OF DESIGN
Regis F. Quastman ACTING CHIEF TRANSPORTATION ENGINEER

DESIGN DESIGNATION:
612 (92) COLLECTOR 0.51 (S.R.P.C.C.-20)

LAYOUT

SCALE 0 50 100 200 FEET

TOTAL LENGTH OF SECTION	74-68 HB-1	=	250.34	FEET 0.047 MILES
NET LENGTH OF SECTION	74-68 HB-1	=	250.34	FEET 0.047 MILES
NET LENGTH OF PROJECT	I-72-2(52)60	=	0.00	FEET 0.000 MILES



Louis Paul Moeck

DEPARTMENT OF TRANSPORTATION
FEDERAL HIGHWAY ADMINISTRATION

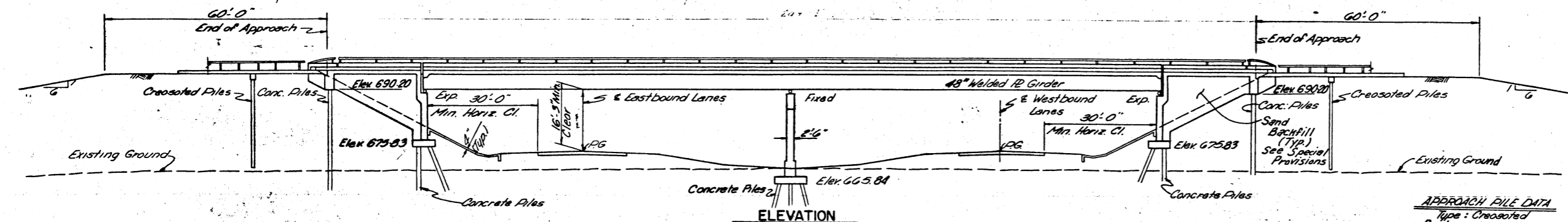
APPROVED _____ DATE _____

DIVISION ENGINEER

BRIGHTON ENGINEERING COMPANY
CONSULTING ENGINEERS
GLEN ELLYN, ILLINOIS

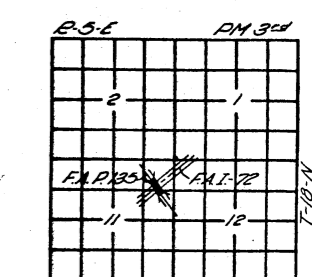
5-85

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
F.A.I.-72	74-68-HB1	PIATT	36	13
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT	

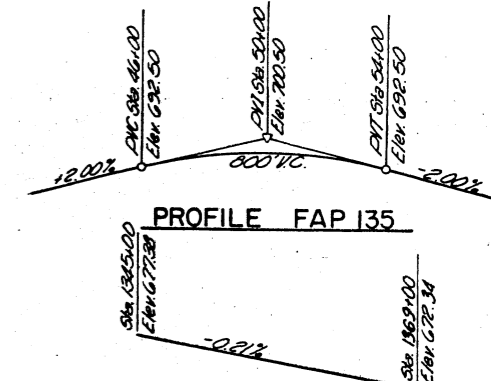


ELEVATION

APPROACH PILE DATA
Type: Creosoted
Req'd Length: 22' South Appr.
22' North Appr.
No Req'd: 8 @ Each Approach

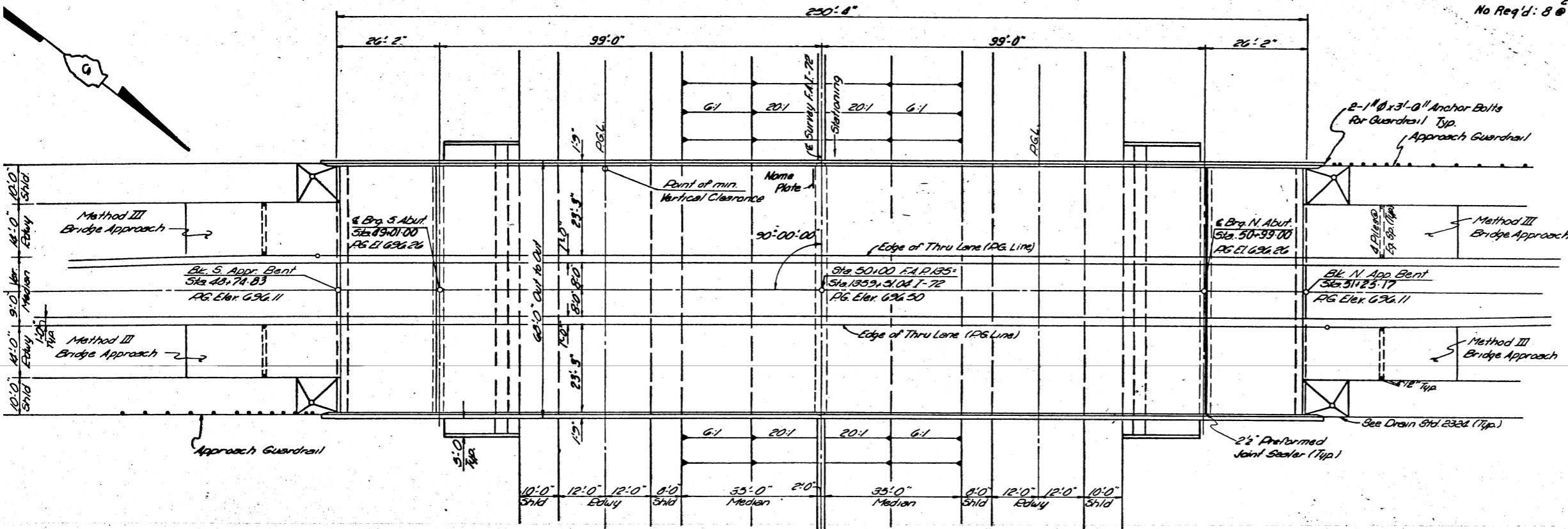


LOCATION SKETCH



PROFILE FAP 135

PROFILE FAI-72



PLAN

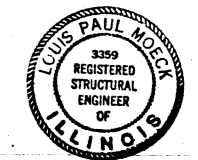
TOTAL BILL OF MATERIALS

Item	Unit	Super	Sub	Total
Aluminum Railing	Lin. Ft.	519		519
Sand Backfill	Cu. Yds.		727	727
Structure Excavation	Cu. Yds.		124	124
Class X Concrete	Cu. Yds.	579.2	426.0	1005.2
Furnishing & Erecting Structural Steel	Lump Sum	1		1
Furnishing Concrete Piles	Lin. Ft.		3663	3663
Furnishing Creosoted Piles (20' to 30' Ft.)	Lin. Ft.		352	352
Driving Timber Piles	Lin. Ft.		352	352
Driving Concrete Piles	Lin. Ft.		3663	3663
Test Piles (Concrete)	Each		3	3
Name Plates	Each	1		1
Preformed Joint Sealer (2 1/2")	Lin. Ft.	130		130
Slope Wall (4 Inch)	Sq. Yds.		357	357
Stud Shear Connectors	Each	2916		2916
Reinforcement Bars	Lb.	151,330	48,260	199,590

250-4
52-4
198

B.M. 43 - Top of ER spike in North side of 10' dia Elm. S' South of Sta. 1359+00. Elev. 678.73

APPROVED
FOR STRUCTURAL ADEQUACY ONLY

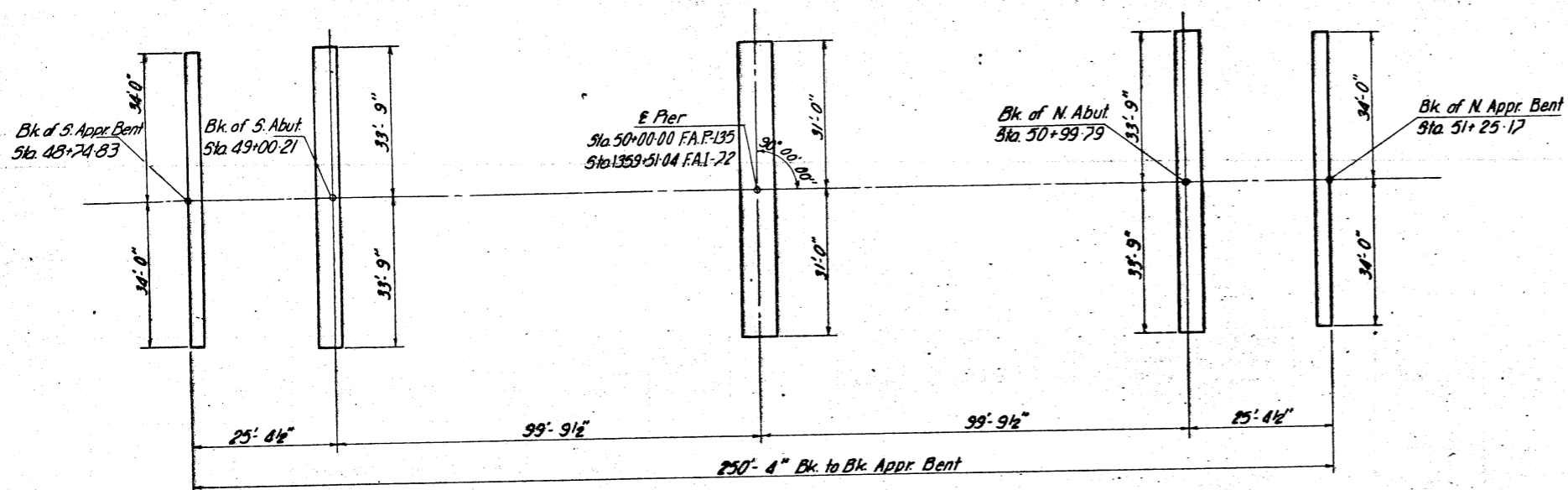


DESIGNED	K.S.Y.
CHECKED	S.T.
DRAWN	D.D. 11-10-71
CHECKED	P.M. & D.M.

GENERAL PLAN & ELEVATION
PROJ. I-72-2(52)60
F.A.I. 72 SEC. 74-68-HB 1
PIATT COUNTY
STA. 1359+51.04 (F.A.I.-72)

Class X Conc. from 994.0 Cu. Yds. to 1005.2 Cu. Yds. Furn. & Driving Conc. Piles from 3078 Lin. Ft. to 3663 Lin. Ft. Test Piles (Conc.) from 1 to 3. Reinf. Bars from 198,370# to 199,590#
Metal Protective Coat - 572 Sq. Yds. Bit. Conc. Surf. Course. Class I - 151 Tons. Coal Tar Interlayer Protective Coat 1356 Sq. Yds. 9-10-73 L.W.

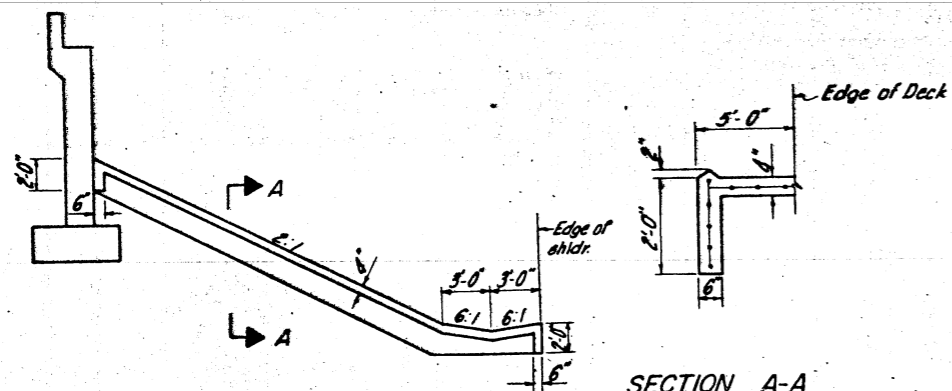
ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
S.B.L. 74-68	HB-1	PLATT	36	14
F.A.I. 72				
FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT		



FOOTING LAYOUT

GENERAL NOTES

- All Reinforcement Bars Shall Be Lapped 24 Diameters Unless Otherwise Shown.
- Fasteners Shall Be High Strength Bolts. Bolts 3/4"; Open Holes 1 1/16", Unless Otherwise Noted.
- Calculated Weight Of Structural Steel = 409,222 lbs.
- The Basic Lead Silico Chromate Paint System Shall Be Used For Shop And Field Painting Of Structural Steel.
- Field Welding Of Construction Accessories Will Not Be Permitted To The Bottom Flange Of Beams Or Girders Nor To The Top Flange For A Distance Equal To One-Fourth The Span Length Each Way From The Pier Supports. Field Welding In Other Areas Will Be Permitted Only When Approved By The Engineer.
- Anchor Bolts Shall Be Set Before Bolting Diaphragms (Bolting Cross Frames) Over Supports.
- Slope Wall Shall Be Reinforced With Welded Wire Fabric 6"x6" Mesh, Weighing 50# Per 100 Sq. Ft.
- The Contractor Shall Drive 3 Concrete Test Piles In A Permanent Location, (1) Each Abutment As Directed By The Engineer Before Ordering The Remainder Of Piles.
- The Embankment Configuration Shown Shall Be The Minimum Embankment That Must Be Constructed Prior To Construction Of The Abutments.
- The Concrete Rail Section Above The Mandatory Construction Joint At The Top Of The Slab Shall Be Constructed Of Class X Concrete, Except The Aggregates Shall Conform To The Requirements Of Handrail Concrete.
- Bearing Seat Surfaces Shall Be Constructed Or Adjusted To The Designated Elevations Within A Tolerance Of ± 1/8 Inch. Adjustment Shall Be Made Either By Grinding The Surface Or By Shimming The Bearing. Two 1/2" Adjusting Shims, Of The Dimensions Of The Bottom Bearing Plate, Shall Be Provided For Each Bearing In Addition To All Other Plates Or Shims.
- Protective Coat Shall Not Be Applied To Surfaces To Which Coat Or Interlayer Protective Coat Is Applied.
- Concrete Piles at Abutments shall be Driven in Holes Precored through the Embankment in Accordance with Art. 513.09(c) of the Std. Specs.
- The main load carrying member components subject to the Supplemental Requirements for Notch Toughness are the flanges, webs, and splice plates of the steel girders or wide flange beams.

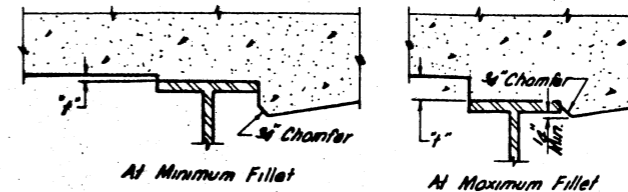


SLOPE WALL DETAIL

STATION 1359+51.04
BUILT 197 BY
STATE OF ILLINOIS
F.A.I-72 SEC. 74-68-HB-1
F.A. PROJ. 1-72-2(52)
LOADING HS20 @ ALT.
BRIDGE NAME PLATE
See Sect. 2119

DESIGNED S.T.
CHECKED M.S.S.
DRAWN M.M.P. 6-27-72
CHECKED K.S.Y.

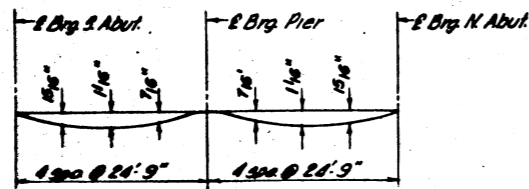
SUBSTRUCTURE LAYOUT
F.A.I-72 SEC 74-68-HB-1
PLATT COUNTY
STA. 1359+51.04 (F.A.I-72)



FILLET HEIGHTS

To determine "f": After all Structural Steel or Precast Prestressed has been erected, elevations of the top flanges of the beams shall be taken at intervals shown below. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflection" shown below, minus slab thickness, equals the fillet heights "f" above top flange of beams.

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
F.A.I. #	74-68 HB-1	PIATT	36	15
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT	



DEAD LOAD DEFLECTION DIAGRAM

(Includes weight of concrete only)
Note: The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections as shown below.

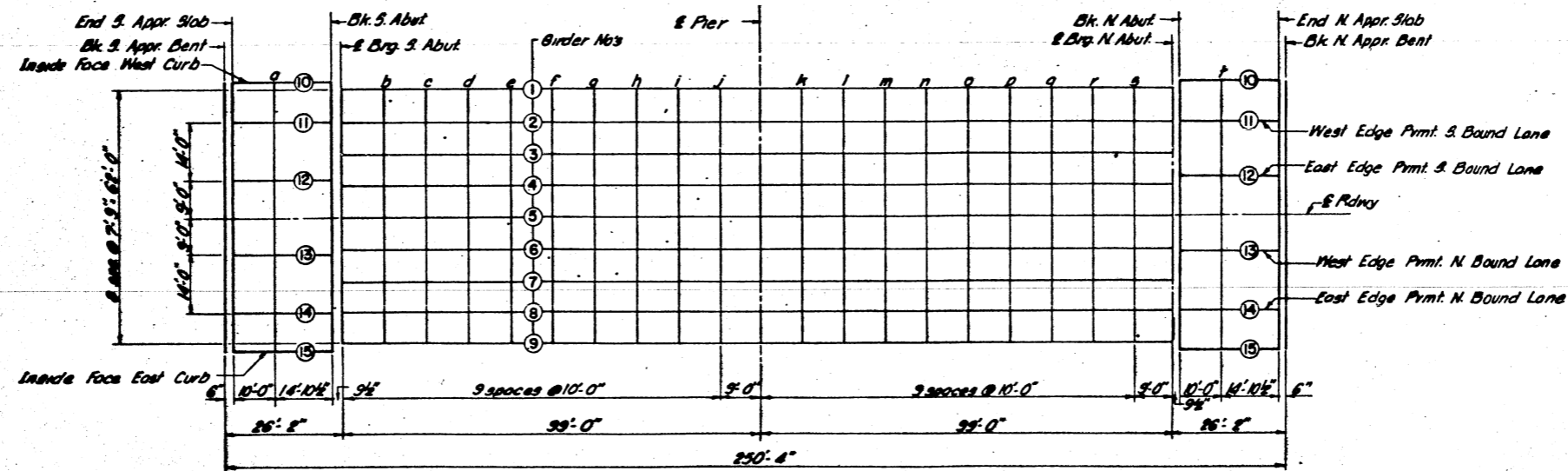
SOUTH APPROACH SLAB

Location	Station	Offset	Theoretical Grade Elevation	Theoretical Grade Elev. Adjusted For D.L. Deflection
BACK OF APPROACH BENT				
10	4874.830	-32.250	695.574	695.574
11	4874.830	-23.000	695.765	695.765
12	4874.830	-9.000	695.983	695.983
13	4874.830	9.000	695.983	695.983
14	4874.830	23.000	695.765	695.765
15	4874.830	32.250	695.574	695.574
BACK OF APPROACH SLAB				
10	4875.330	-32.250	695.578	695.578
11	4875.330	-23.000	695.768	695.768
12	4875.330	-9.000	695.987	695.987
13	4875.330	9.000	695.987	695.987
14	4875.330	23.000	695.768	695.768
15	4875.330	32.250	695.578	695.578
BACK OF ABUTMENT				
10	4885.330	-32.250	695.637	695.637
11	4885.330	-23.000	695.828	695.828
12	4885.330	-9.000	696.046	696.046
13	4885.330	9.000	696.046	696.046
14	4885.330	23.000	695.828	695.828
15	4885.330	32.250	695.637	695.637

Location	Girder	Station	Offset	Theoretical Grade Elevation	Theoretical Grade Elev. Adjusted For D.L. Deflection
E Brg. S. Abut.	1	4901.000	-31.000	695.747	695.747
	2	4901.000	-23.250	695.906	695.906
	3	4901.000	-15.500	696.029	696.029
	4	4901.000	-7.750	696.150	696.150
	5	4901.000	-0.000	696.271	696.271
	6	4901.000	7.750	696.150	696.150
	7	4901.000	15.500	696.029	696.029
	8	4901.000	23.250	695.906	695.906
	9	4901.000	31.000	695.747	695.747
b	1	4911.000	-31.000	695.794	695.832
	2	4911.000	-23.250	695.953	695.991
	3	4911.000	-15.500	696.076	696.113
	4	4911.000	-7.750	696.197	696.234
	5	4911.000	-0.000	696.318	696.343
	6	4911.000	7.750	696.197	696.234
	7	4911.000	15.500	696.076	696.113
	8	4911.000	23.250	695.953	695.991
	9	4911.000	31.000	695.794	695.832
c	1	4921.000	-31.000	695.836	695.905
	2	4921.000	-23.250	695.995	696.064
	3	4921.000	-15.500	696.118	696.187
	4	4921.000	-7.750	696.239	696.308
	5	4921.000	-0.000	696.360	696.385
	6	4921.000	7.750	696.239	696.308
	7	4921.000	15.500	696.118	696.187
	8	4921.000	23.250	695.995	696.064
	9	4921.000	31.000	695.836	695.905

Location	Girder	Station	Offset	Theoretical Grade Elevation	Theoretical Grade Elev. Adjusted For D.L. Deflection
d	1	4931.000	-31.000	695.873	695.962
	2	4931.000	-23.250	696.032	696.121
	3	4931.000	-15.500	696.155	696.244
	4	4931.000	-7.750	696.276	696.365
	5	4931.000	-0.000	696.397	697.022
	6	4931.000	7.750	696.276	696.365
	7	4931.000	15.500	696.155	696.244
	8	4931.000	23.250	696.032	696.121
	9	4931.000	31.000	695.873	695.962
e	1	4941.000	-31.000	695.905	696.001
	2	4941.000	-23.250	696.064	696.160
	3	4941.000	-15.500	696.187	696.282
	4	4941.000	-7.750	696.308	696.403
	5	4941.000	-0.000	696.429	697.054
	6	4941.000	7.750	696.308	696.403
	7	4941.000	15.500	696.187	696.282
	8	4941.000	23.250	696.064	696.160
	9	4941.000	31.000	695.905	696.001
f	1	4951.000	-31.000	695.932	696.021
	2	4951.000	-23.250	696.091	696.181
	3	4951.000	-15.500	696.214	696.303
	4	4951.000	-7.750	696.335	696.424
	5	4951.000	-0.000	696.456	697.081
	6	4951.000	7.750	696.335	696.424
	7	4951.000	15.500	696.214	696.303
	8	4951.000	23.250	696.091	696.181
	9	4951.000	31.000	695.932	696.021

Location	Girder	Station	Offset	Theoretical Grade Elevation	Theoretical Grade Elev. Adjusted For D.L. Deflection
g	1	4961.000	-31.000	695.954	696.025
	2	4961.000	-23.250	696.113	696.185
	3	4961.000	-15.500	696.236	696.307
	4	4961.000	-7.750	696.357	696.428
	5	4961.000	-0.000	696.478	697.103
	6	4961.000	7.750	696.357	696.428
	7	4961.000	15.500	696.236	696.307
	8	4961.000	23.250	696.113	696.185
	9	4961.000	31.000	695.954	696.025
h	1	4971.000	-31.000	695.971	696.018
	2	4971.000	-23.250	696.130	696.178
	3	4971.000	-15.500	696.253	696.300
	4	4971.000	-7.750	696.374	696.421
	5	4971.000	-0.000	696.495	697.120
	6	4971.000	7.750	696.374	696.421
	7	4971.000	15.500	696.253	696.300
	8	4971.000	23.250	696.130	696.178
	9	4971.000	31.000	695.971	696.018
i	1	4981.000	-31.000	695.983	696.006
	2	4981.000	-23.250	696.142	696.166
	3	4981.000	-15.500	696.265	696.288
	4	4981.000	-7.750	696.386	696.409
	5	4981.000	-0.000	696.507	697.132
	6	4981.000	7.750	696.386	696.409
	7	4981.000	15.500	696.265	696.288
	8	4981.000	23.250	696.142	696.166
	9	4981.000	31.000	695.983	696.006



PLAN

Note:
Elevations Given For Girders 4, 5, 6 Are For A Theoretical Top of Roadway Deck Slab - If Top of Median Slab is Desired Add .625 ft. To The Elevation Given Above.

DESIGNED K.S.Y.
CHECKED C.S.
DRAWN R.R./B.C.
CHECKED K.S.Y.

TOP OF SLAB ELEVATIONS
FAI-72 SEC. 74-68-HB-1
PIATT COUNTY
STA. 1359+51.04 (FAI-72)

Location	Girder	Station	Offset	Theoretical Grade Elevation	Theoretical Grade Elev. Adjusted For D.L. Deflection
J	1	4991.000	-31.000	695.990	695.996
	2	4991.000	-23.250	696.14	696.156
	3	4991.000	-15.500	696.27	696.278
	4	4991.000	-7.750	696.393	696.399
	5	4991.000	-0.000	696.514	697.139
	6	4991.000	7.750	696.393	696.399
	7	4991.000	15.500	696.272	696.278
	8	4991.000	23.250	696.149	696.156
	9	4991.000	31.000	695.990	695.996
E Pier	1	5000.000	-31.000	695.992	695.992
	2	5000.000	-23.250	696.151	696.151
	3	5000.000	-15.500	696.274	696.274
	4	5000.000	-7.750	696.395	696.395
	5	5000.000	-0.000	696.516	697.141
	6	5000.000	7.750	696.395	696.395
	7	5000.000	15.500	696.274	696.274
	8	5000.000	23.250	696.151	696.151
	9	5000.000	31.000	695.992	695.992
K	1	5010.000	-31.000	695.989	695.996
	2	5010.000	-23.250	696.149	696.156
	3	5010.000	-15.500	696.271	696.278
	4	5010.000	-7.750	696.392	696.399
	5	5010.000	-0.000	696.513	697.131
	6	5010.000	7.750	696.392	696.399
	7	5010.000	15.500	696.271	696.278
	8	5010.000	23.250	696.149	696.156
	9	5010.000	31.000	695.989	695.996
I	1	5020.000	-31.000	695.982	696.007
	2	5020.000	-23.250	696.141	696.167
	3	5020.000	-15.500	696.264	696.289
	4	5020.000	-7.750	696.385	696.410
	5	5020.000	-0.000	696.506	697.131
	6	5020.000	7.750	696.385	696.410
	7	5020.000	15.500	696.264	696.289
	8	5020.000	23.250	696.141	696.167
	9	5020.000	31.000	695.982	696.007

Location	Girder	Station	Offset	Theoretical Grade Elevation	Theoretical Grade Elev. Adjusted For D.L. Deflection
m	1	5030.000	-31.000	695.969	696.019
	2	5030.000	-23.250	696.129	696.179
	3	5030.000	-15.500	696.251	696.301
	4	5030.000	-7.750	696.372	696.422
	5	5030.000	-0.000	696.493	697.118
	6	5030.000	7.750	696.372	696.422
	7	5030.000	15.500	696.251	696.301
	8	5030.000	23.250	696.129	696.179
	9	5030.000	31.000	695.969	696.019
n	1	5040.000	-31.000	695.952	696.026
	2	5040.000	-23.250	696.111	696.185
	3	5040.000	-15.500	696.234	696.307
	4	5040.000	-7.750	696.355	696.428
	5	5040.000	-0.000	696.476	697.101
	6	5040.000	7.750	696.355	696.428
	7	5040.000	15.500	696.234	696.307
	8	5040.000	23.250	696.111	696.185
	9	5040.000	31.000	695.952	696.026
o	1	5050.000	-31.000	695.929	696.020
	2	5050.000	-23.250	696.089	696.179
	3	5050.000	-15.500	696.211	696.301
	4	5050.000	-7.750	696.332	696.422
	5	5050.000	-0.000	696.453	697.078
	6	5050.000	7.750	696.332	696.422
	7	5050.000	15.500	696.211	696.301
	8	5050.000	23.250	696.089	696.179
	9	5050.000	31.000	695.929	696.020
p	1	5060.000	-31.000	695.902	695.997
	2	5060.000	-23.250	696.061	696.157
	3	5060.000	-15.500	696.184	696.279
	4	5060.000	-7.750	696.305	696.400
	5	5060.000	-0.000	696.426	697.051
	6	5060.000	7.750	696.305	696.400
	7	5060.000	15.500	696.184	696.279
	8	5060.000	23.250	696.061	696.157
	9	5060.000	31.000	695.902	695.997

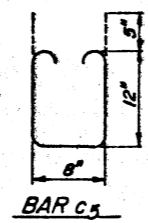
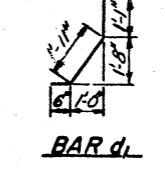
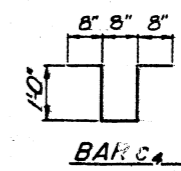
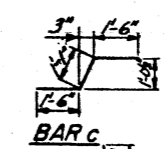
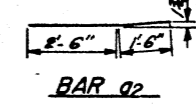
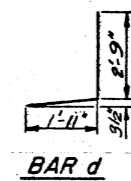
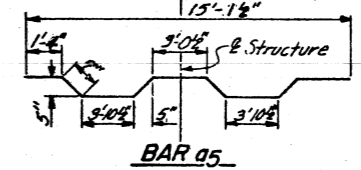
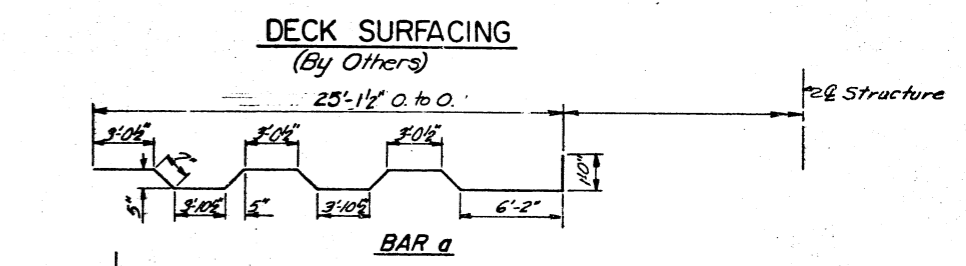
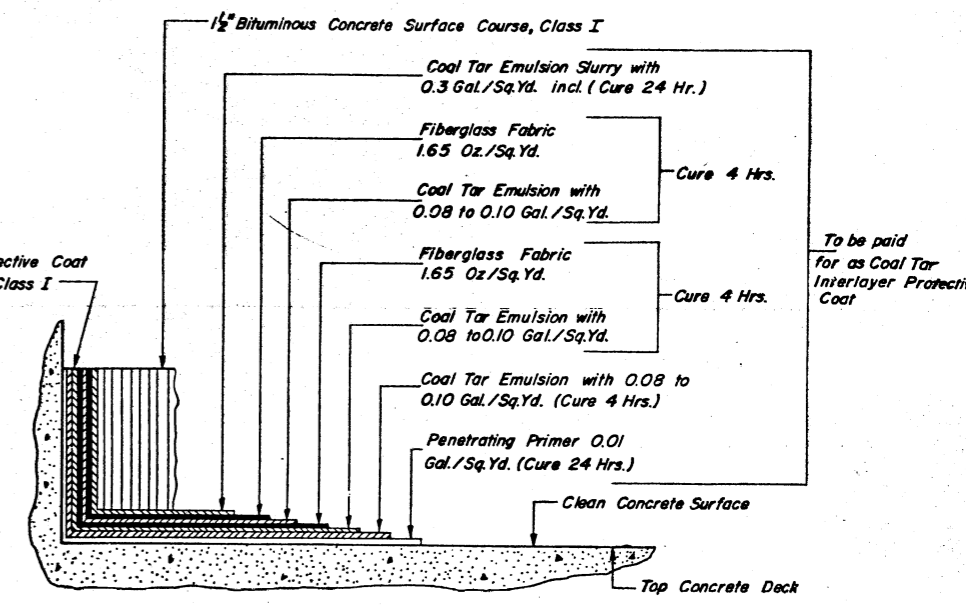
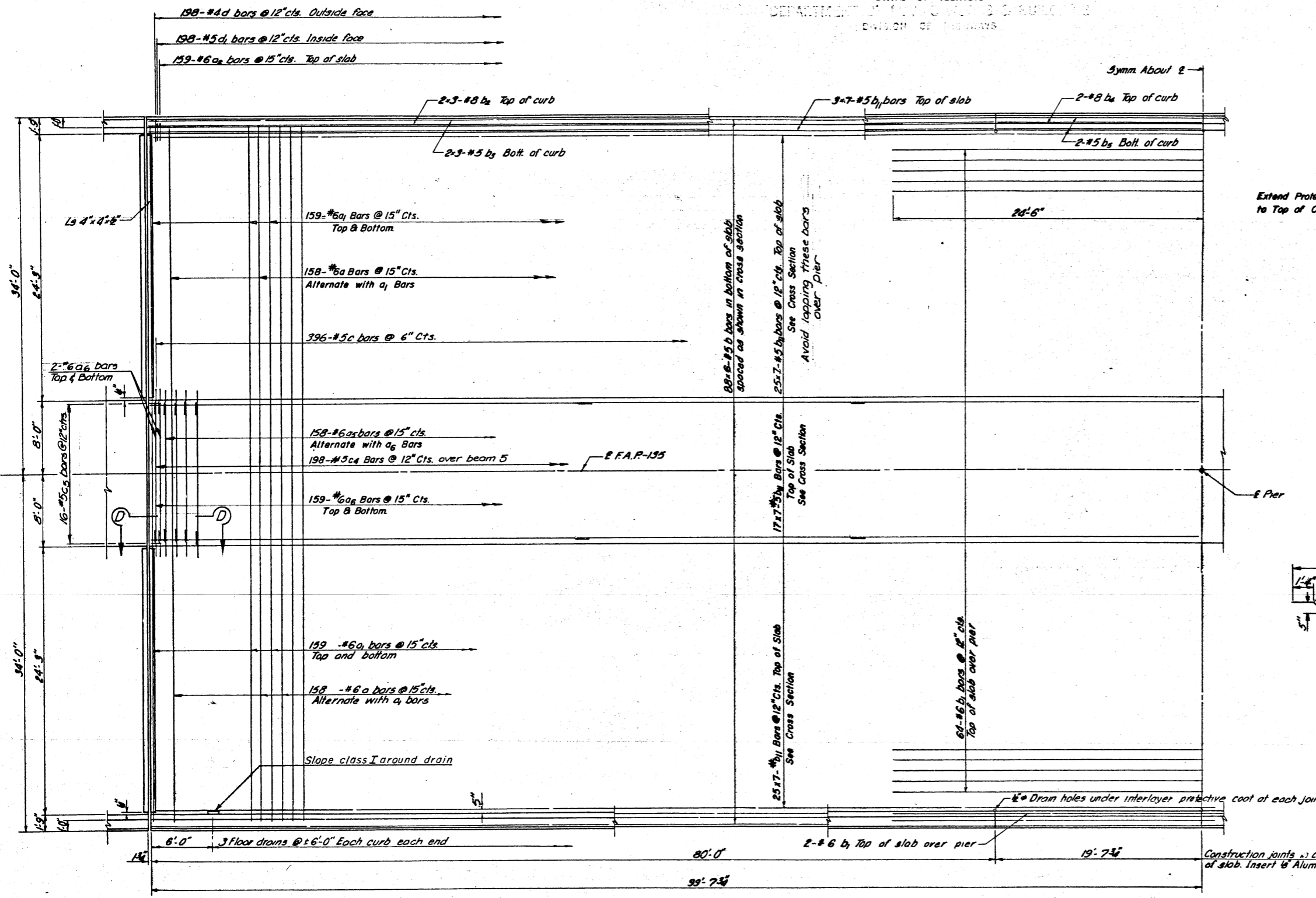
Location	Girder	Station	Offset	Theoretical Grade Elevation	Theoretical Grade Elev. Adjusted For D.L. Deflection
q	1	5070.000	-31.000	695.869	695.957
	2	5070.000	-23.250	696.029	696.116
	3	5070.000	-15.500	696.151	696.238
	4	5070.000	-7.750	696.272	696.359
	5	5070.000	-0.000	696.393	697.018
	6	5070.000	7.750	696.272	696.359
	7	5070.000	15.500	696.151	696.238
	8	5070.000	23.250	696.029	696.116
	9	5070.000	31.000	695.869	695.957
r	1	5080.000	-31.000	695.832	695.898
	2	5080.000	-23.250	695.991	696.057
	3	5080.000	-15.500	696.114	696.180
	4	5080.000	-7.750	696.235	696.301
	5	5080.000	-0.000	696.356	696.981
	6	5080.000	7.750	696.235	696.301
	7	5080.000	15.500	696.114	696.180
	8	5080.000	23.250	695.991	696.057
	9	5080.000	31.000	695.832	695.898
s	1	5090.000	-31.000	695.789	695.823
	2	5090.000	-23.250	695.949	695.983
	3	5090.000	-15.500	696.071	696.105
	4	5090.000	-7.750	696.192	696.226
	5	5090.000	-0.000	696.313	696.938
	6	5090.000	7.750	696.192	696.226
	7	5090.000	15.500	696.071	696.105
	8	5090.000	23.250	695.949	695.983
	9	5090.000	31.000	695.789	695.823
E Brg. N. Abut.	1	5099.000	-31.000	695.747	695.747
	2	5099.000	-23.250	695.906	695.906
	3	5099.000	-15.500	696.029	696.029
	4	5099.000	-7.750	696.150	696.150
	5	5099.000	-0.000	696.271	696.896
	6	5099.000	7.750	696.150	696.150
	7	5099.000	15.500	696.029	696.029
	8	5099.000	23.250	695.906	695.906
	9	5099.000	31.000	695.747	695.747

NORTH APPROACH SLAB

Location	Station	Offset	Theoretical Grade Elevation	Theoretical Grade Elev. Adjusted For D.L. Deflection
BACK OF ABUTMENT				
10	5099.790	-32.250	695.717	695.717
11	5099.790	-23.000	695.908	695.908
12	5099.790	-9.000	696.126	696.126
13	5099.790	9.000	696.126	696.126
14	5099.790	23.000	695.908	695.908
15	5099.790	32.250	695.717	695.717
t				
10	5109.790	-32.250	695.665	695.665
11	5109.790	-23.000	695.855	695.855
12	5109.790	-9.000	696.074	696.074
13	5109.790	9.000	696.074	696.074
14	5109.790	23.000	695.855	695.855
15	5109.790	32.250	695.665	695.665
BACK OF APPROACH SLAB				
10	5124.670	-32.250	695.578	695.578
11	5124.670	-23.000	695.768	695.768
12	5124.670	-9.000	695.987	695.987
13	5124.670	9.000	695.987	695.987
14	5124.670	23.000	695.768	695.768
15	5124.670	32.250	695.578	695.578
BACK OF APPROACH BENT				
10	5125.170	-32.250	695.574	695.574
11	5125.170	-23.000	695.765	695.765
12	5125.170	-9.000	695.983	695.983
13	5125.170	9.000	695.983	695.983
14	5125.170	23.000	695.765	695.765
15	5125.170	32.250	695.574	695.574

DESIGNED	K.S.Y.
CHECKED	M.S.S.
DRAWN	B.G.
CHECKED	K.S.Y.

TOP OF SLAB ELEVATIONS
F.A.I.-72 SEC. 74-68+HB-1
PIATT COUNTY
STA. 1359+51.04 (F.A.I.-72)

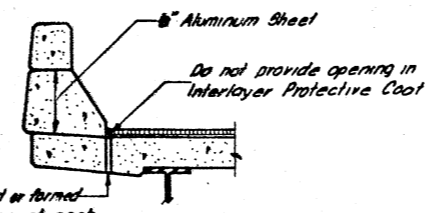


BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a	316	#6	26'-11 1/2"	
a ₁	636	#6	25'-3"	
a ₂	318	#6	4'-0"	
a5	158	#6	15'-9 1/2"	
a ₆	326	#6	15'-2"	
b	528	#5	34'-4"	
b ₁	68	#6	49'-0"	
b ₂	24	#8	28'-0"	
b ₃	24	#5	27'-6"	
b ₄	8	#8	19'-3"	
b ₅	8	#5	19'-3"	
b ₁₁	511	#5	29'-8"	
c	792	#5	4'-1"	
c4	198	#5	4'-0"	
c5	32	#5	3'-6"	
d	396	#4	4'-8"	
d ₁	396	#5	5'-6"	
Reinforcement Bars		Lbs.	93,780	
Class x Concrete		Cu Yds.	385.8	

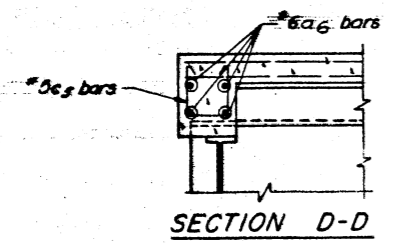
Parapet reinforcement and class x concrete are billed on sheet #8

HALF PLAN

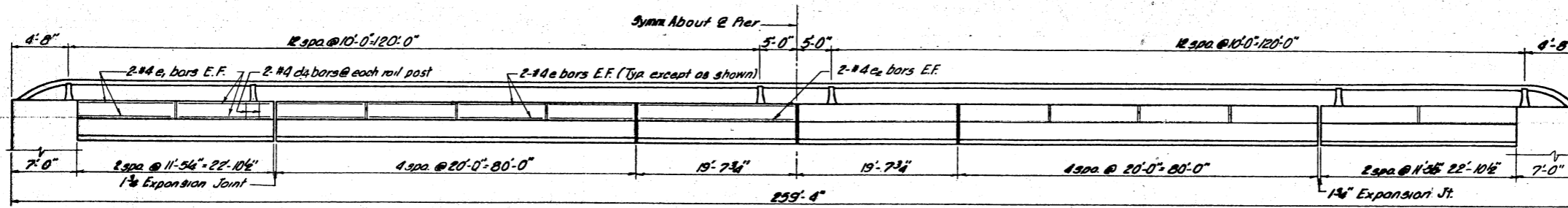


SECTION AT CURB JOINTS

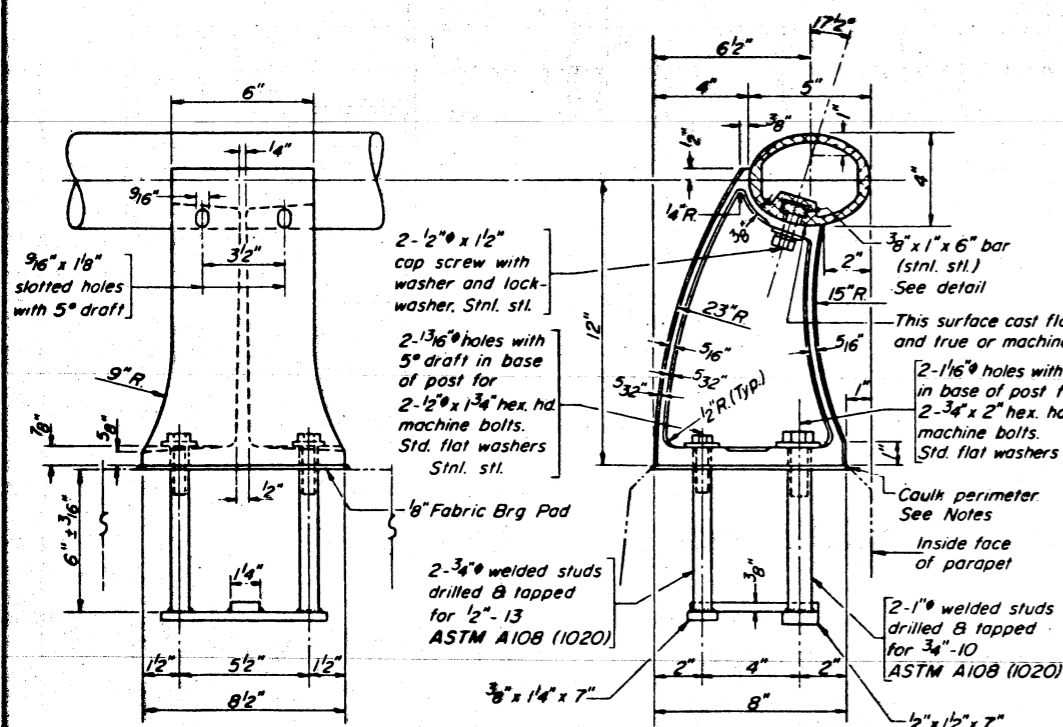
DESIGNED **J.T.**
 CHECKED **M.S.S.**
 DRAWN **M.M.P. Aug 2nd '72**
 CHECKED **K.S.Y.**



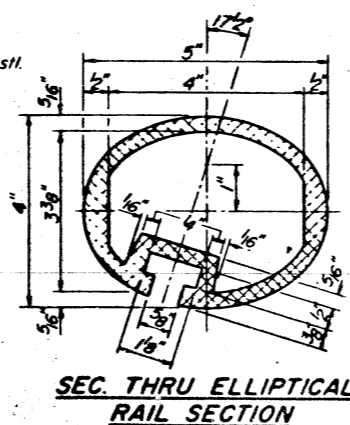
SLAB DETAILS
 F.A.I.-72 SEC. 74-68-HB-1
 PIATT COUNTY
 STA. 1359+51.04 (F.A.I.-72)



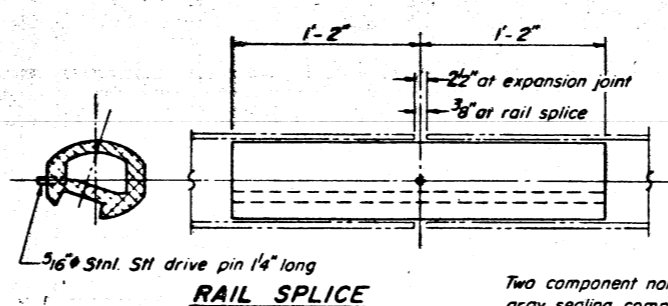
ELEVATION



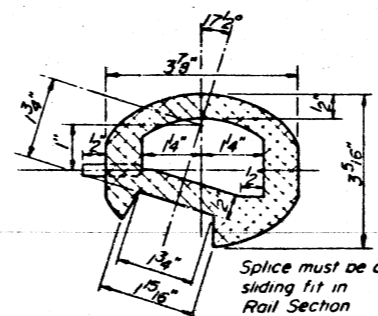
RAIL POST DETAILS



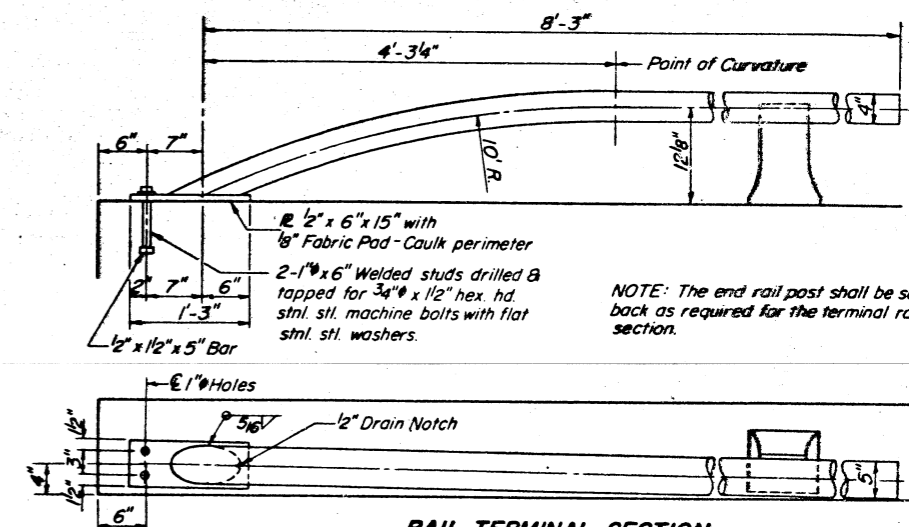
SEC. THRU ELLIPTICAL RAIL SECTION



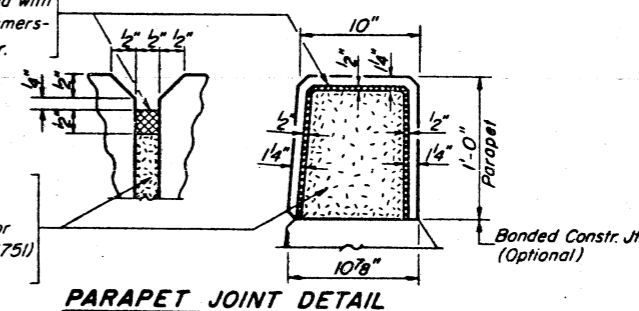
RAIL SPLICE



SEC. THRU SPLICE



RAIL TERMINAL SECTION



PARAPET JOINT DETAIL

Two component non-staining gray sealing compound with polysulfide liquid polymers - gun grade with primer.

1/2" Preformed Cork Asphalt Joint Filler. (meets qualifications for ASTM: Designation D 1751) Cast incidental.

NOTES:

All Aluminum Alloy Extruded Rail shall be supplied in modular lengths of 30 feet, except at the end of bridge or over open joints in bridge deck where the rail shall be attached to a minimum of 2 posts. If the rail is on a horizontal curve of 2300 foot radius or less, the modular lengths may be reduced but shall be attached to a minimum of 2 posts.

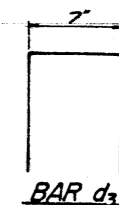
All joints in rail shall be spliced per detail. Provide 1-8" and 2-16" Aluminum Shims for 25% of the Posts. Rail element shall be parallel to Grade - high spots shall be ground and low spots shimmed.

Seal perimeter of base of post to parapet with two component non-staining gray sealing compound with polysulfide liquid polymers, gun grade with primer. Fabric Bearing Pad shall have same dimensions as base of post.

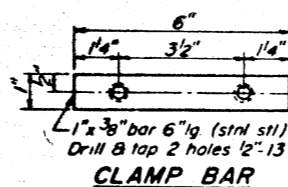
Aluminum alloy rail shall conform to ASTM B 221 alloy 6061-T6 or 6351-T5 with min. yield 35 ksi, min. tensile 38 ksi, and elongation of 10% in 2 inches.

TWO PARAPETS & RAILS
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
e	64	#4	19'-5"	—
e ₁	32	#4	11'-1"	—
e ₂	16	#4	19'-5"	—
d ₄	104	#4	2'-1"	□
Reinforcement Bars		Lbs.	1430	
Class X Concrete		Cu Yds.	15.8	
Aluminum Railing		Lin. Ft.	519	

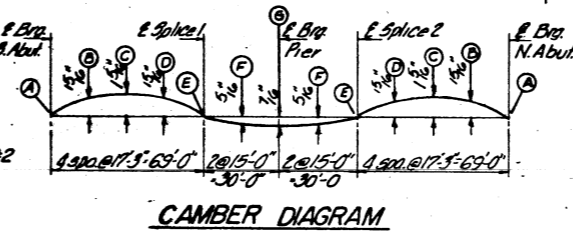
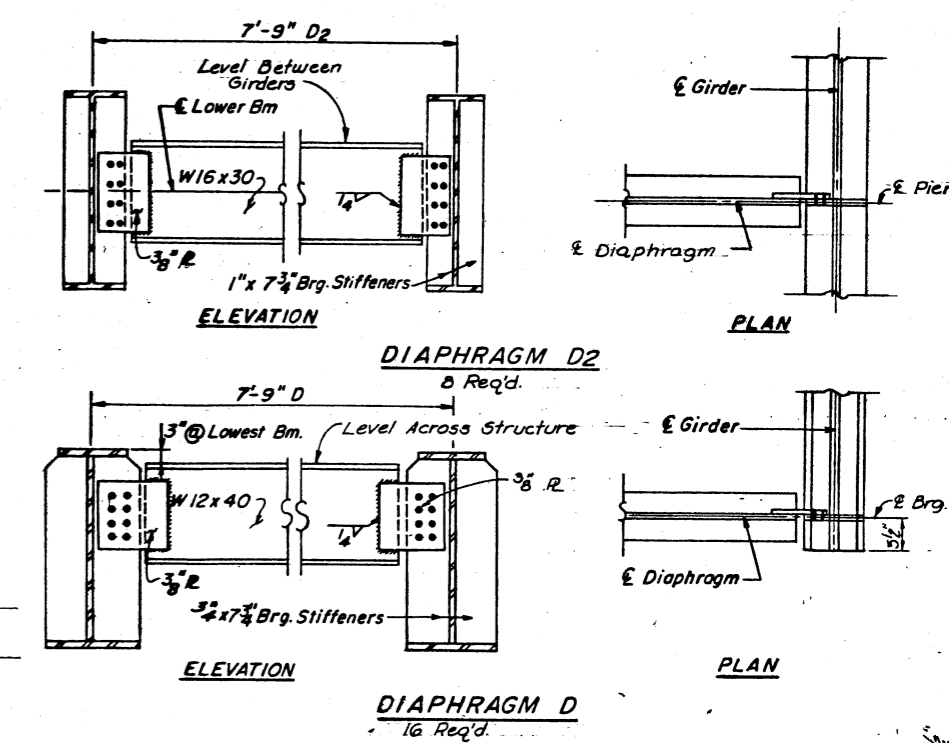
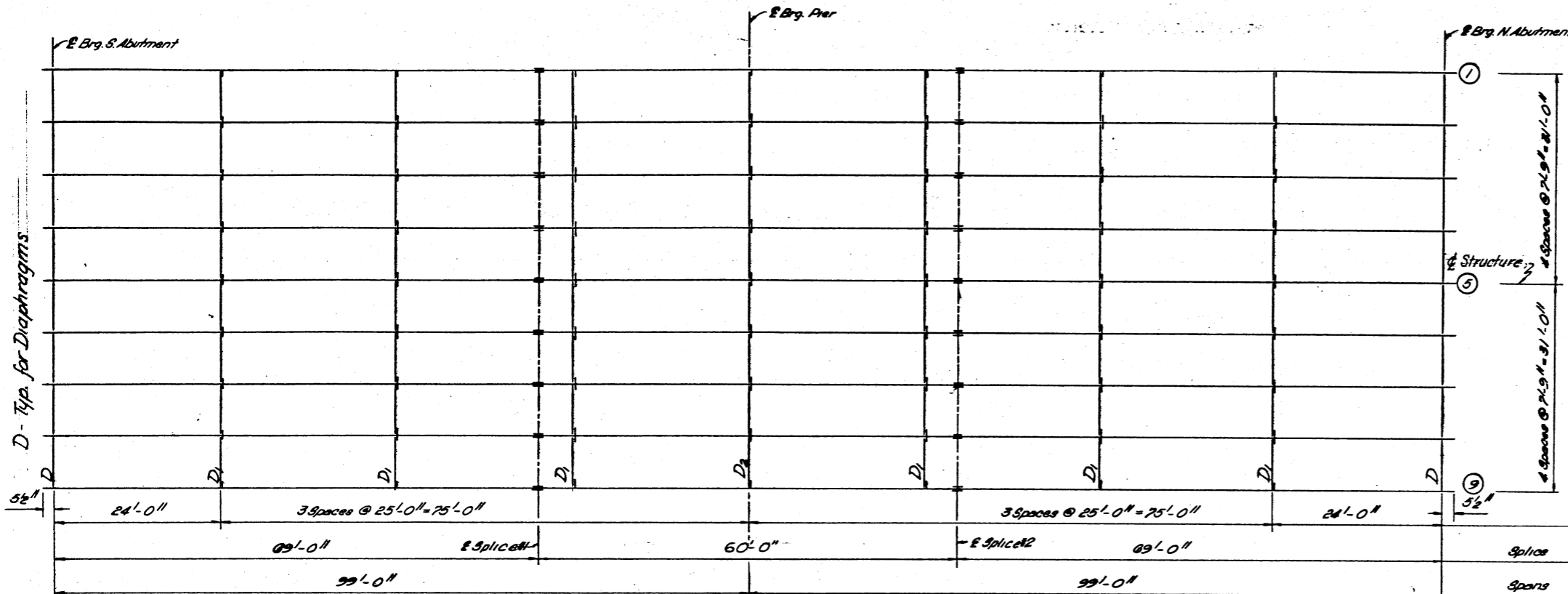


BAR d₃

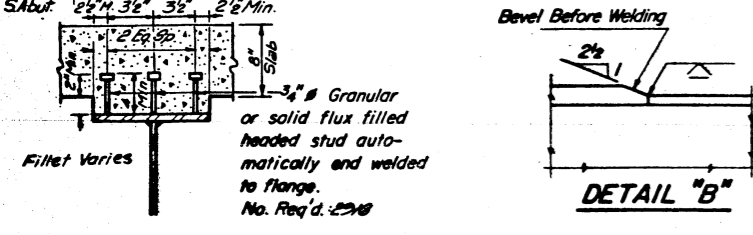
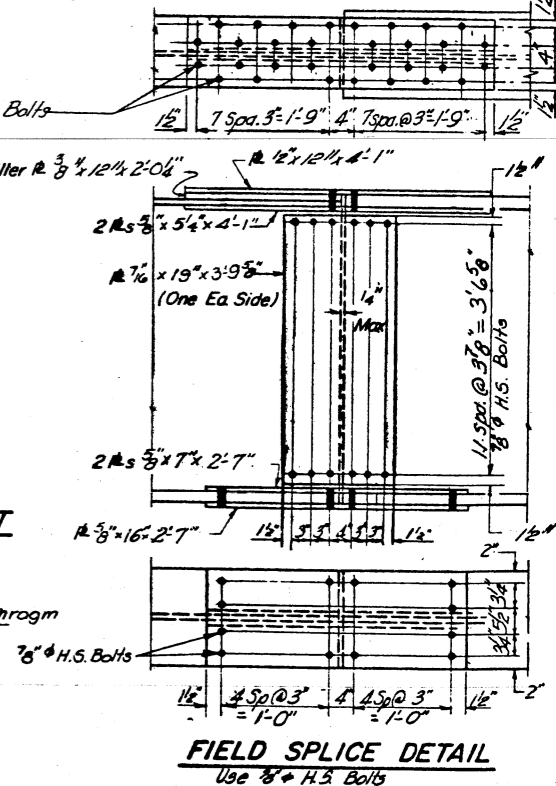
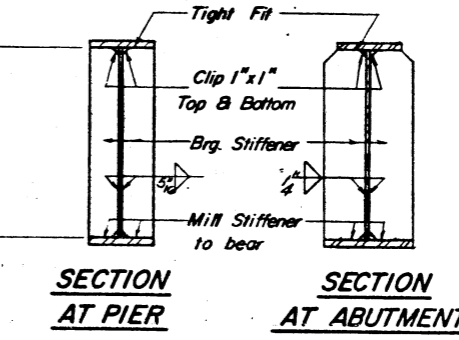
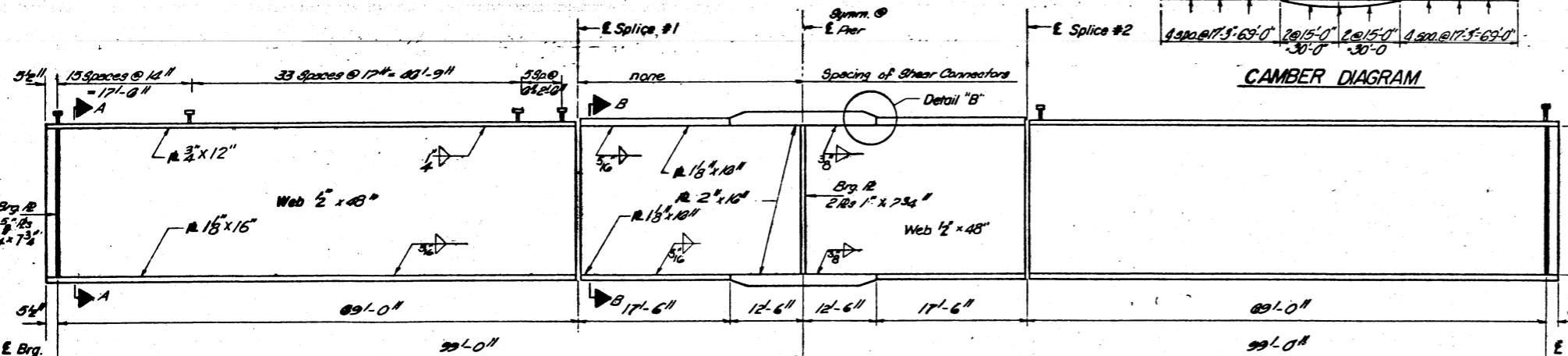


CLAMP BAR

DESIGNED BY	
CHECKED BY	
DRAWN BY	M.M.P. 8-4-72
CHECKED BY	K.S.V.



Dimension	A	B	C	D	E	F	G
Total Camber	0	15/16	15/16	15/16	0	5/16	7/16
Camber Due to Vertical Curve	0	1/4	5/16	1/4	0	3/16	1/8
Camber Due to Deflection	0	1/16	1/16	1/16	0	1/2	11/16



Interior Girder	
	Pier
I _g (in ⁴)	44629
I _c (in ⁴)	55462
S _x (in ³)	1717
S _y (in ³)	1352
I _p (in ⁴)	992
M _g (k)	573
M _c (k)	7.2
S _g (in ³)	.36
M _g (k)	250
M _c (k)	882
M _{max} (k)	196
TOTAL (k)	1328
R _{LL} (k)	11.8
R _{TOTAL} (k)	19.0
VR (k)	67.2

Girder	S. Abut.	Splice #1	Pier	Splice #2	N. Abut.
149	024.98	025.16	025.12	025.16	024.98
248	025.14	025.32	025.28	025.32	025.14
347	025.26	025.44	025.40	025.44	025.26
446	025.38	025.56	025.52	025.56	025.38
5	025.50	025.68	025.64	025.68	025.50

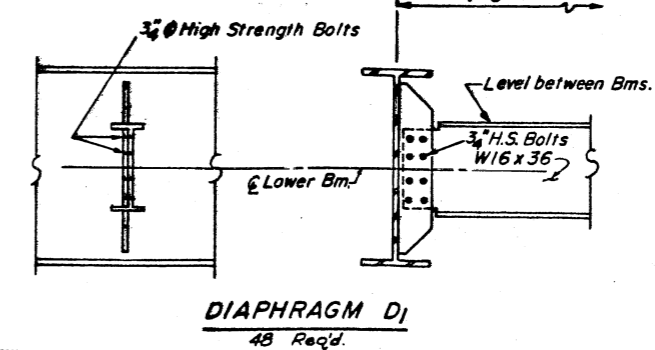
* For Fabrication Only

Interior Girder		
	Abutment	Pier
R _g (K)	47.2	173.8
R _{LL} (K)	45.0	79.1
I _{max} (K)	9.9	16.3
R _{TOTAL} (K)	102.1	263.2

Note: I_g and S_g are the moment of inertia and section modulus of the steel section.

I_c and S_c are the moment of inertia and section modulus of the composite section used in computing I_p.

* V_r is the maximum &+I shear range in span used to determine shear stud spacing max.



DESIGNED: ST

CHECKED: MSS

DRAWN: DD 8-4-72

CHECKED: K.S.Y.

G-1 3-29-71

FRAMING DETAILS

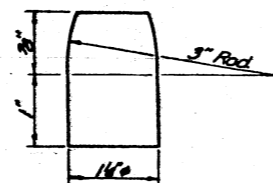
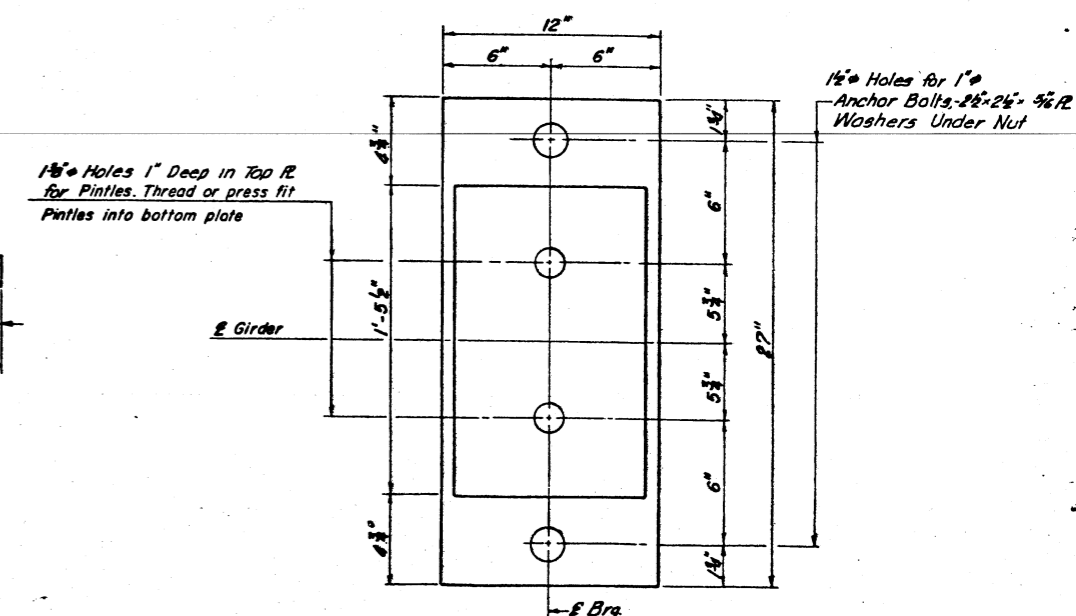
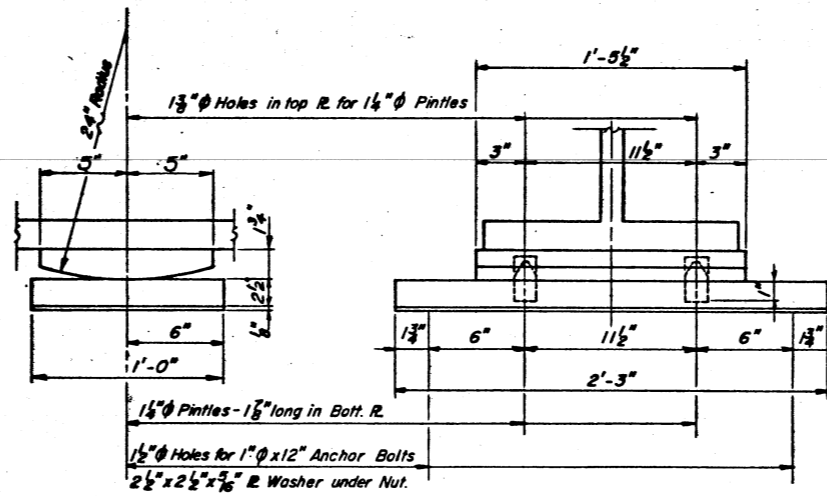
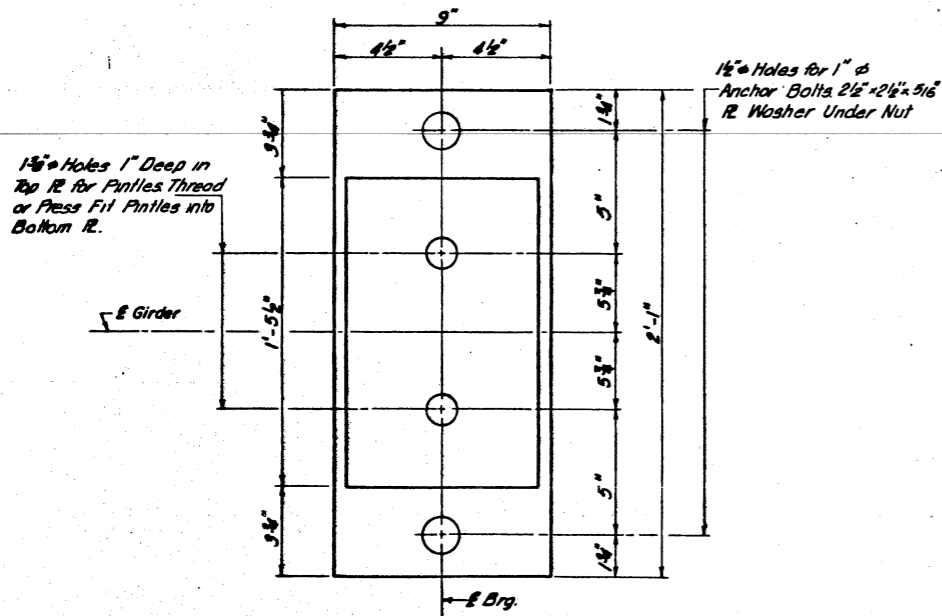
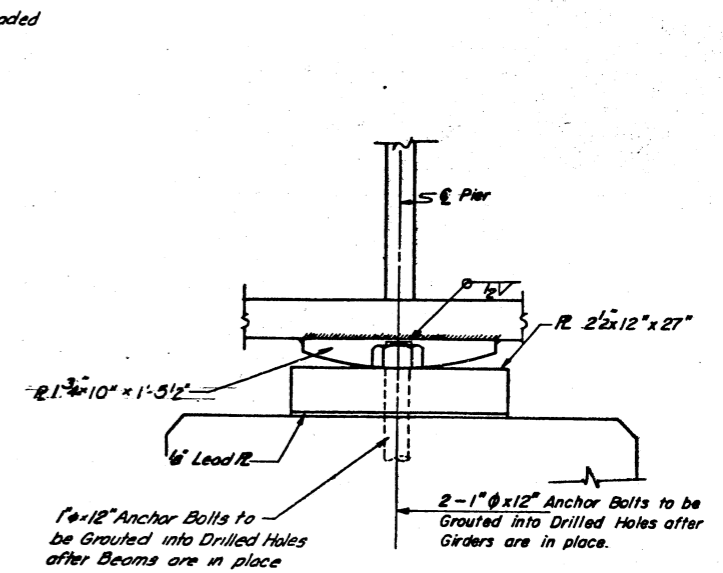
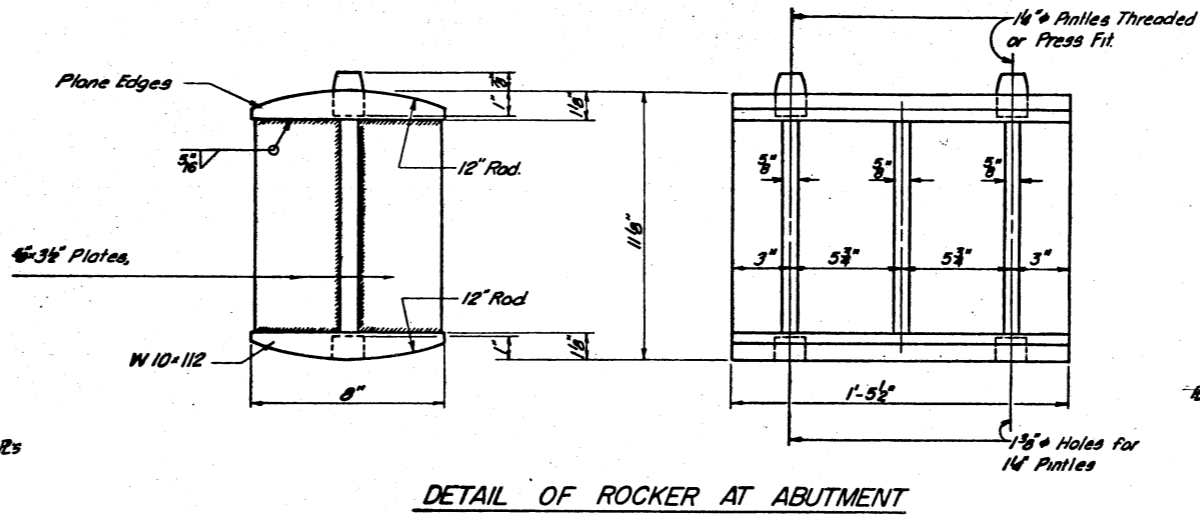
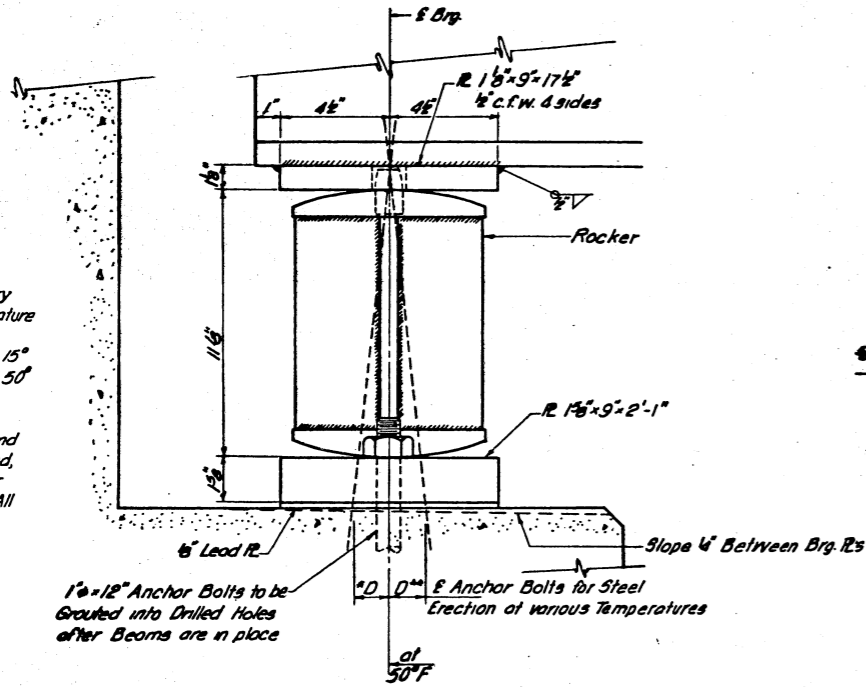
F.A.I.-72 SEC. 74-68HB-1

PIATT COUNTY

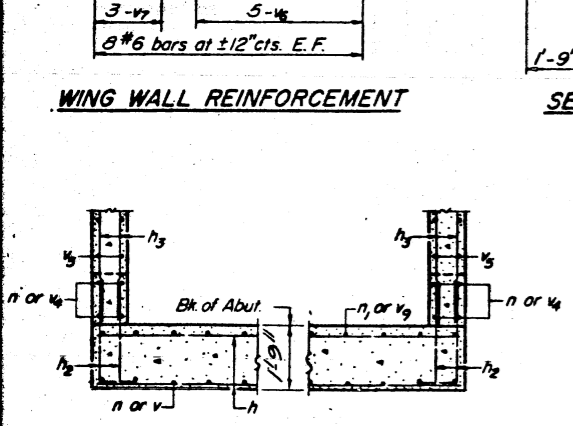
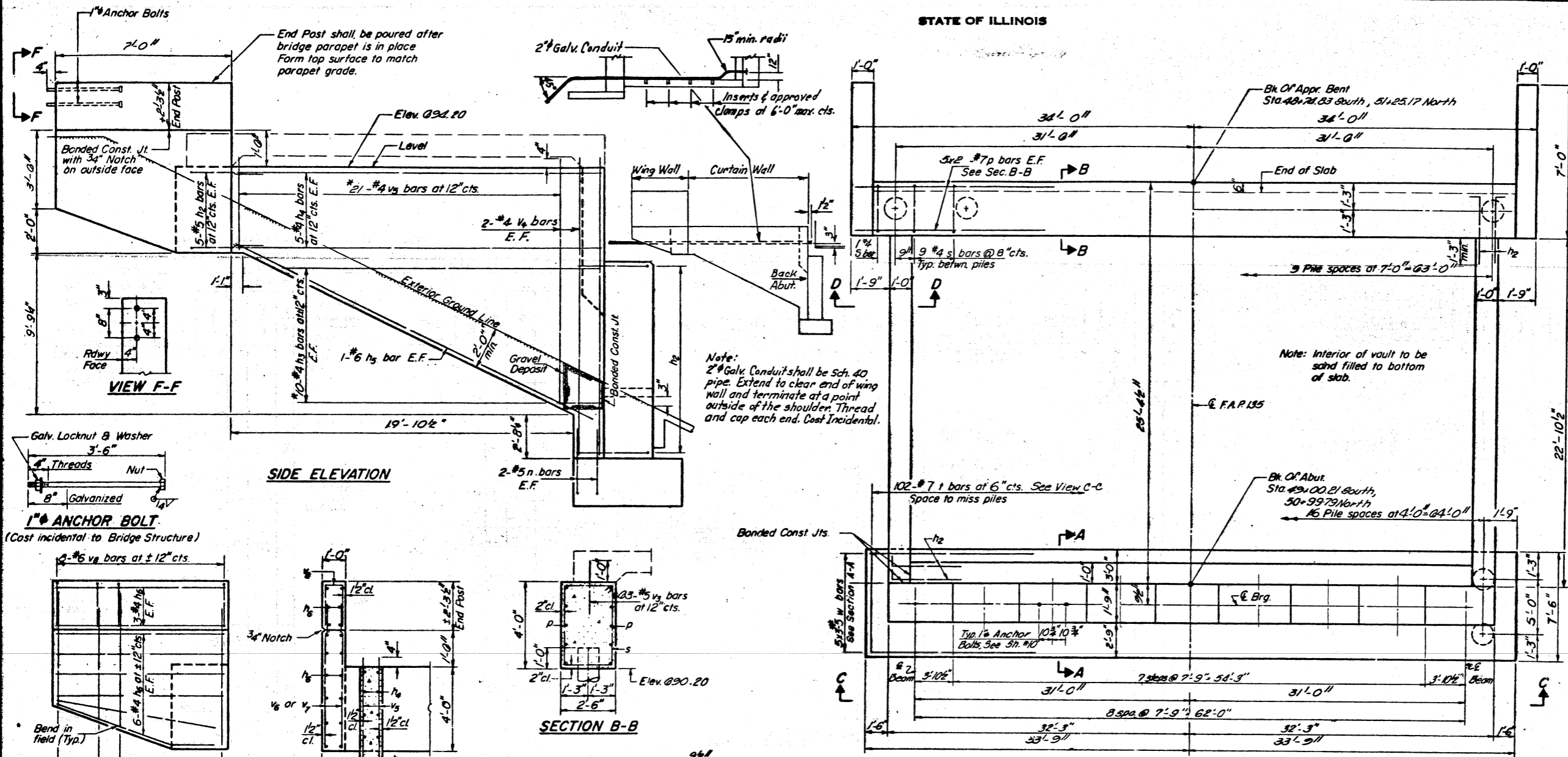
STA. 1359+51.04 (F.A.I.-72)

6"/100ft. of Expansion for Every 15° Below the Normal Temperature of 50°
 6"/100ft. of Expansion for Every 15° Above the Normal Temperature of 50°

After beams have been erected and dimensions D' and D'' determined, holes shall be drilled and anchor bolts shall be grouted in place. All fixed anchor bolts may be built into the masonry



DESIGNED S.T.
CHECKED M.S.S.
DRAWN M.M.P. 8-9-72
CHECKED K.S.Y.

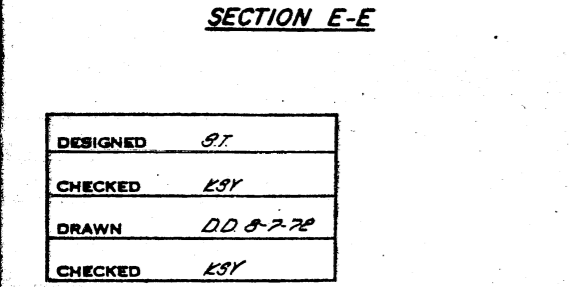


ABUT.-PILE DATA

Type	Concrete Piles
Capacity	35 Tons
Est. Length	34' South Abut.
No. Req'd.	33+1 Test Pile
Est. Length	25' North Abut.
No. Req'd.	33+1 Test Pile

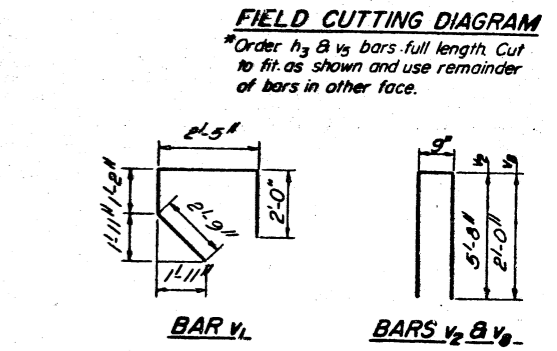
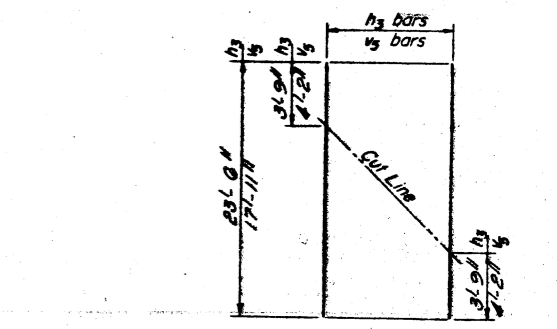
APPR. BENT.-PILE DATA

Type	Concrete Piles
Capacity	35 Tons
Est. Length	49' South
No. Req'd.	10
Est. Length	38' North
No. Req'd.	10



DESIGNED	G.T.
CHECKED	K.S.Y.
DRAWN	D.D. 8-2-72
CHECKED	K.S.Y.

Rev. Reinf. Bars from 13,760# to 14,370# Cl. X Conc. from 144.5 Cu. Yds. to 150.1 Cu. Yds. Test Piles (Conc.) from 1 to 2.



ONE ABUTMENT BILL OF MATERIAL

Bar	No	Size	Length	Shape
h	81	#5	22'-2 1/2"	—
h	36	#5	21'-7 1/2"	—
h2	92	#5	4'-0"	—
h3	20	#4	23'-6"	—
h4	10	#4	22'-6"	—
h5	4	#6	26'-4"	—
h6	30	#4	6'-5 1/2"	—
n	73	#5	3'-2 1/2"	—
n1	78	#8	4'-11"	—
p	20	#7	32'-9"	—
s	83	#4	12'-5 1/2"	—
1	102	#7	7'-2"	—
v	65	#5	11'-6"	—
v1	83	#5	8'-4 1/2"	—
v2	83	#5	12'-1 1/2"	—
v3	28	#5	2'-3 1/2"	—
v4	8	#4	12'-5 1/2"	—
v5	42	#4	17'-1 1/2"	—
v6	20	#6	8'-0"	—
v7	12	#6	5'-4 1/2"	—
v8	18	#6	4'-9 1/2"	—
v9	78	#7	11'-6"	—
w	15	#5	23'-3 1/2"	—

Reinforcement Bars * Lbs. 14,370
 Class X Concrete * Cu. Yds. 150.1
 Test Piles (Concrete) Each 2
 Concrete Piles (2 Abut.) Lin. Ft. 2751
 Sand Backfill Cu. Yds. 323.5

* Multiply by 2 for both Abutments

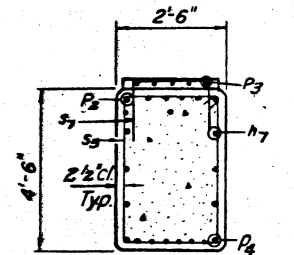
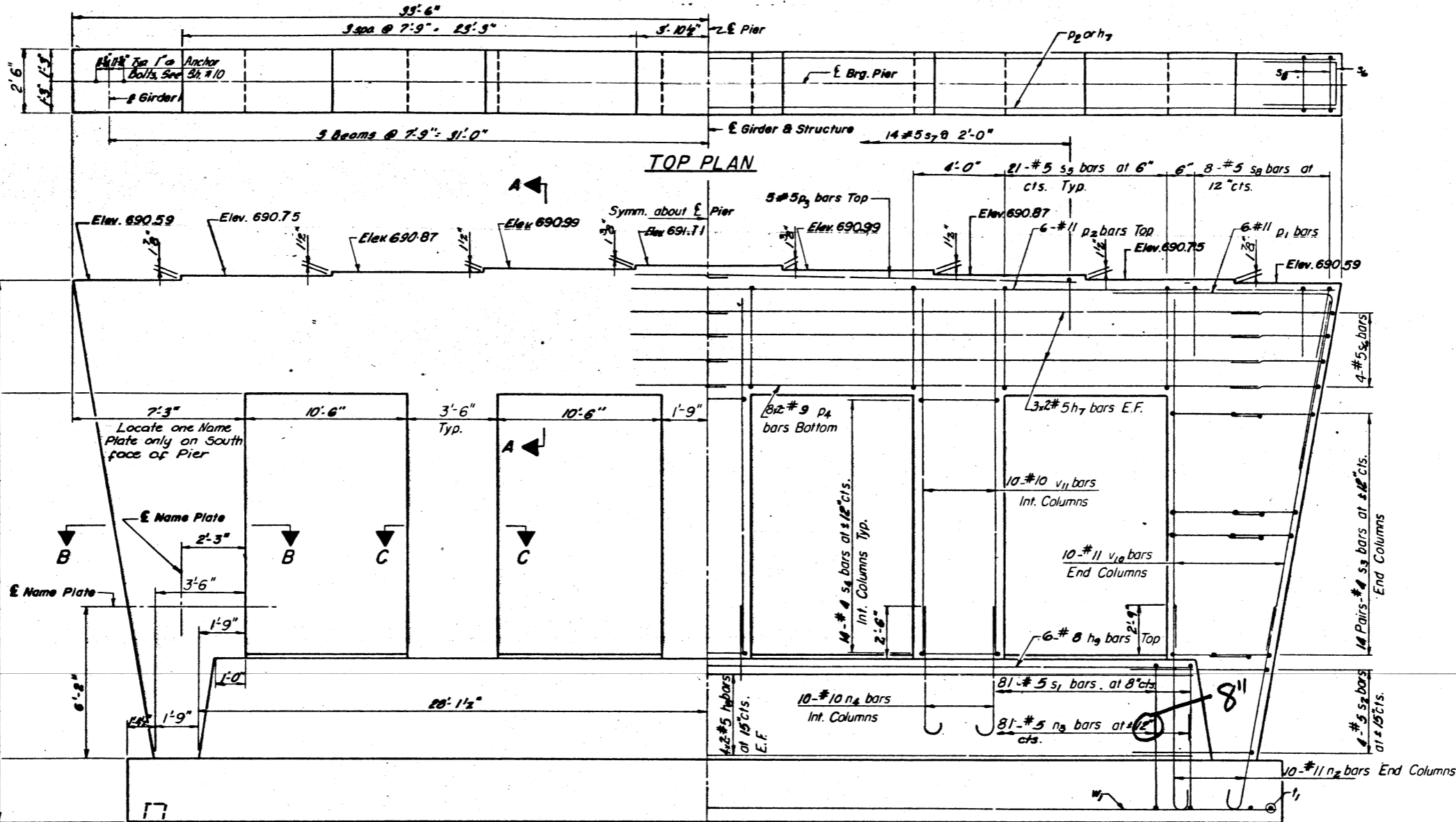
ABUTMENT DETAILS
 FAI-72 SEC. 74-68 HB-1
 PIATT COUNTY
 STA. 1359 + 5104 (FAI-72)

NOTES:

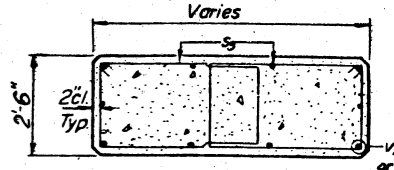
Space reinforcement in cap to miss anchor bolts.
 All edges shall have standard $\frac{3}{4}$ " chamfers except as noted.
 Four steps monolithically with cap.

STATE OF ILLINOIS

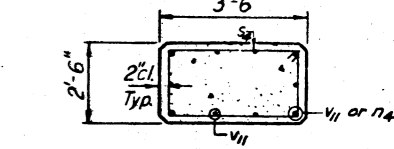
ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	SHEET NO. 12
AA172	74-68 HB-1	PIATT	36	24	SHEETS 14
FED. ROAD DIST. NO. 7		ILLINOIS FED. AID PROJECT			



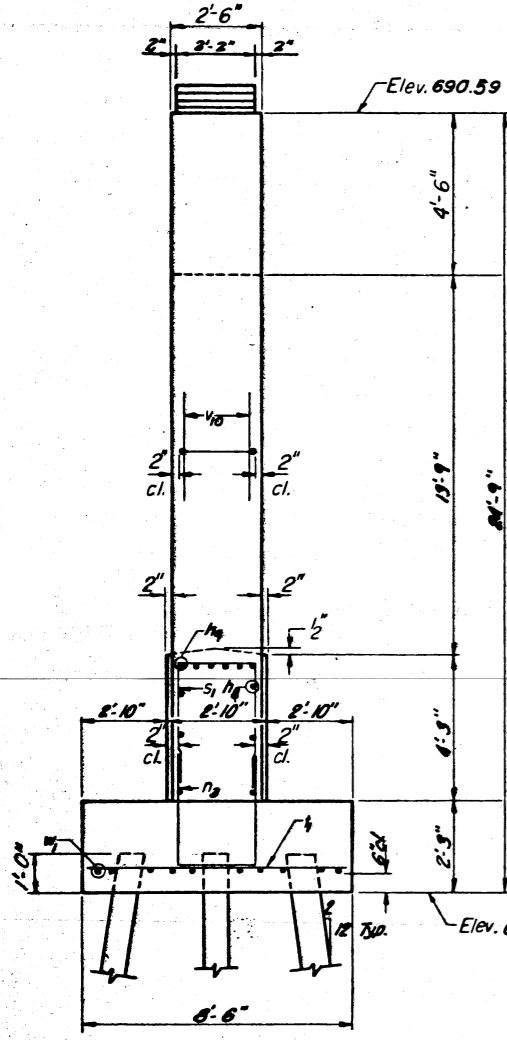
SECTION A-A



SECTION B-B

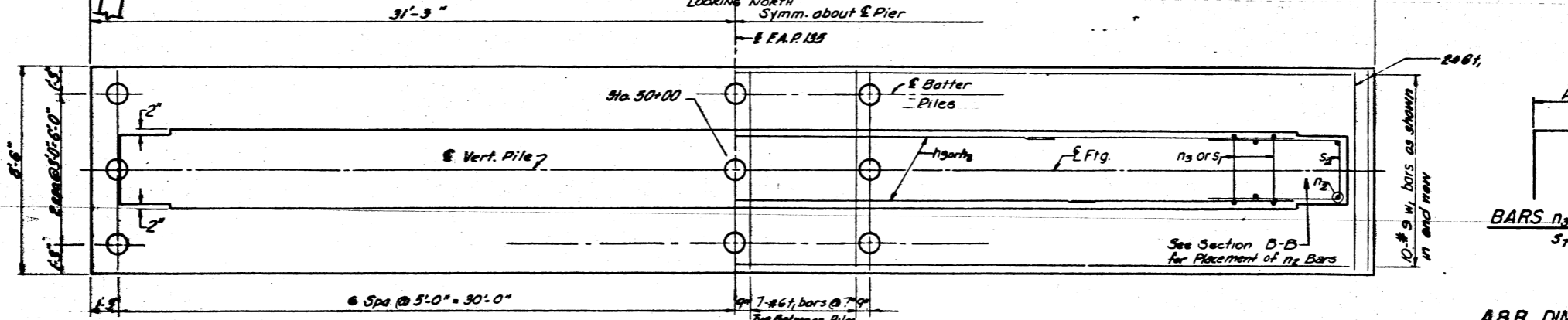


SECTION C-C



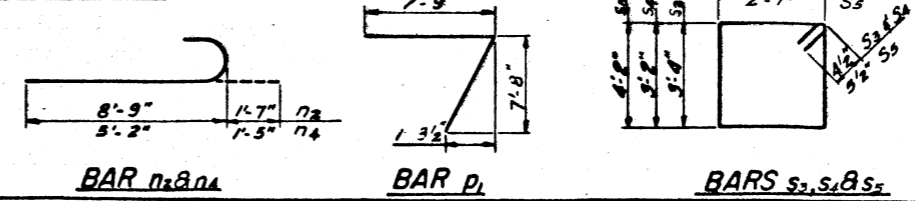
END VIEW

DIMENSIONS ELEVATION REINFORCEMENT



PILE LAYOUT & DIMENSIONS

FOOTING PLAN



A&B DIMENSIONS

Bar	A	B
n7	2'-6"	4'-3"
s1	2'-6"	3'-0"
s2	2'-2"	4'-6"
s6	2'-2"	5'-1"
s7	1'-11"	1'-6"
s9	2'-1"	3'-4"

BILL OF MATERIAL

Bar	No.	Size	Length	Shape
n7	12	#5	30'-3"	—
n8	16	#5	28'-0"	—
n9	6	#8	54'-0"	—
n2	20	#11	10'-4"	—
n3	81	#5	11'-0"	U
n4	30	#10	6'-7"	—
p1	12	#11	15'-6"	7
p2	6	#11	64'-0"	—
p3	10	#5	15'-9"	—
p4	16	#9	30'-0"	—
s1	81	#5	8'-6"	□
s2	8	#5	11'-2"	□
s3	56	#4	11'-9"	□
s4	42	#4	11'-5"	□
s5	84	#5	12'-5"	□
s6	8	#5	12'-4"	□
s7	14	#5	4'-11"	□
s8	16	#5	8'-9"	□
v1	88	#6	8'-2"	—
v10	20	#11	16'-9"	—
v11	30	#10	16'-9"	—
w	10	#9	62'-0"	—
Class X Concrete				Cu. Yds. 125.9
Reinforcement Bars				Lbs. 19,520
Test Pile Concrete				Each 1
Concrete Piles				Lin. Ft. 912

PILE DATA
 Type Concrete Piles
 Capacity 35 Tons
 Est. Length 28'-0"
 No. Required 38 + 1 Test Pile

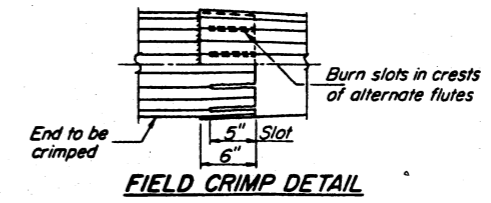
DESIGNED	J.T.
CHECKED	K.B.Y.
DRAWN	M.M.P. 8-8-72
CHECKED	K.B.Y.

P-14 11-18-69

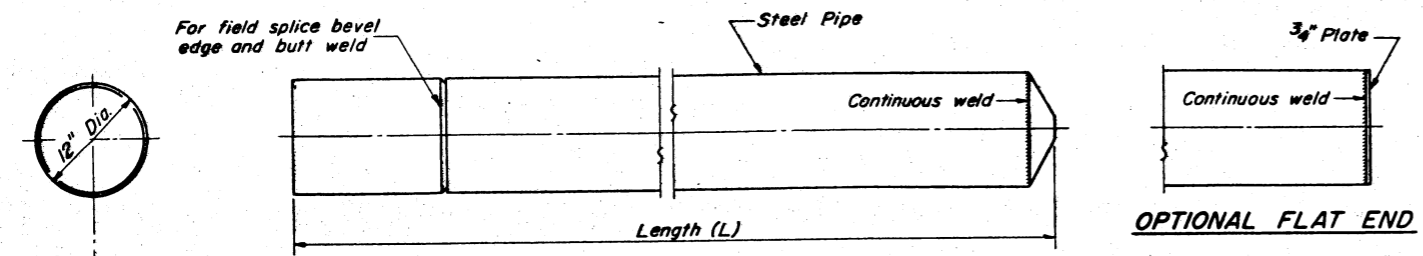
PIER DETAILS
 FA.I-72 SEC. 74-68-HB-1
 PIATT COUNTY
 STA. 1359+51.04 (FA.I-72)

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

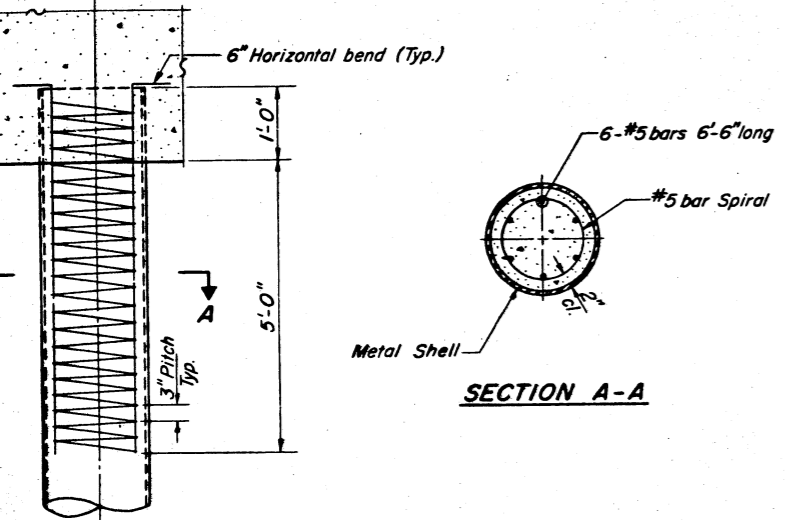
STATE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	SHEET NO. 14 14 SHEETS
R. & L.	74-68 HB-1	Piatt	36	26	
R. & L. 72					
FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT			



Note: 6" Crimp shall either be supplied on the cylindrical section or made in the field as detailed.



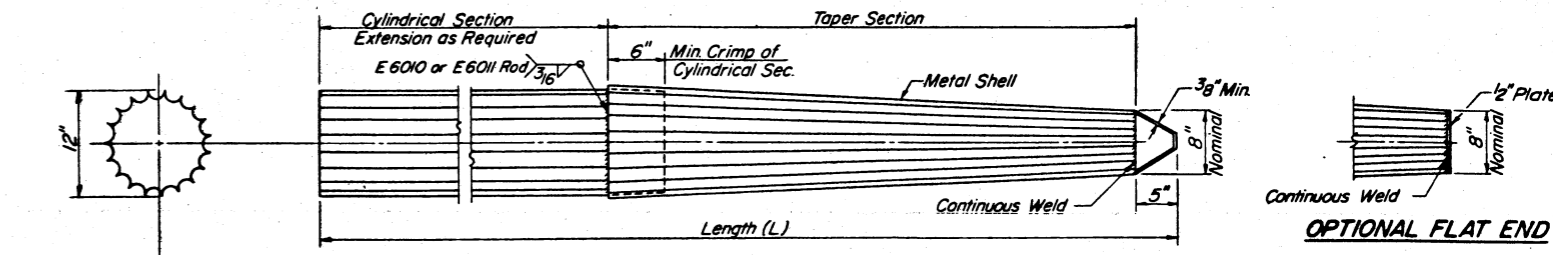
OPTIONAL FLAT END



The cost of Reinforcement is incidental to the cost of furnishing piles. The thickness of the shell shall be .1793 inches with a tolerance of 5%. Reinforcement in top pile shall be omitted under pier footings when placed in natural ground.

Note: Driving and bearing ends of pipe shall be cut square.

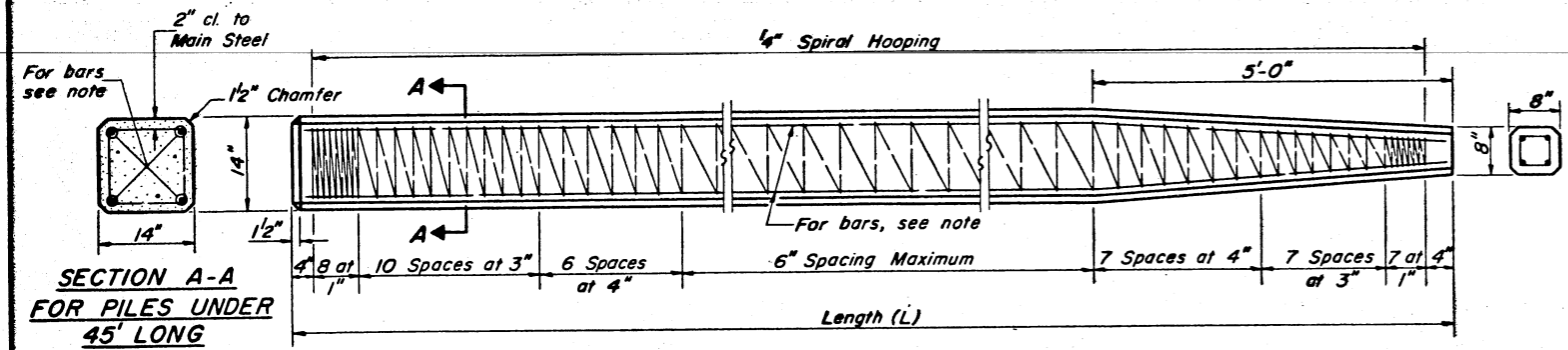
DETAIL OF CYLINDRICAL STEEL SHELL FOR CAST IN PLACE CONCRETE PILES



ALLOWABLE TAPER SECTIONS
10' Length - Taper 1" in 2'-6"
17' Length - Taper 1" in 4'-0"
25' Length - Taper 1" in 7'-0"
30' Length - Taper 1" in 7'-0"

The cost of Reinforcement is incidental to the cost of furnishing piles. The thickness of the shell shall be .1793 inches with a tolerance of 5%. Reinforcement in top of pile shall be omitted under pier footings when placed in natural ground.

DETAIL OF TAPERED METAL SHELL FOR CAST IN PLACE CONCRETE PILES

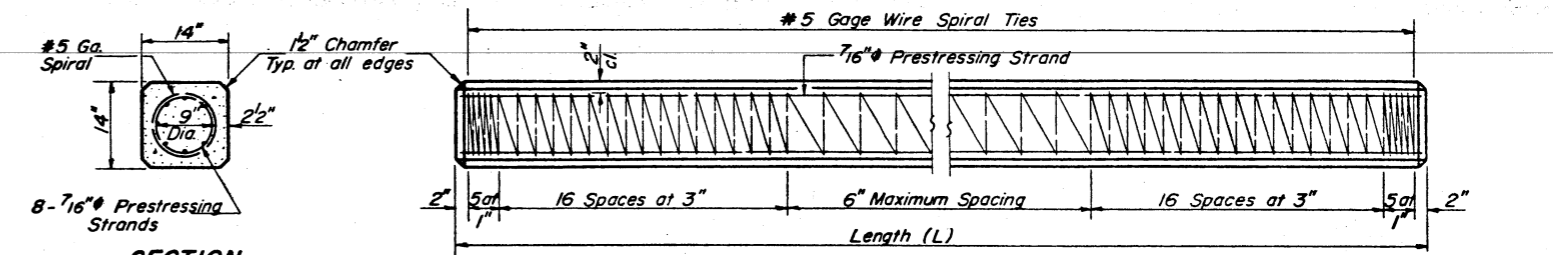


Note: For 14" Piles 45' long or more use 8-#8 bars 4 for the full length and 4 to the point of bevel. For 14" Piles under 45' long use 4-#9 bars full length.

Handling: For Pile lengths up to 45', use two slings placed at a distance of 0.21 L from each end. For Piles longer than 45', use three slings placed at a distance of 0.12 L from each end and at mid-point of pile.

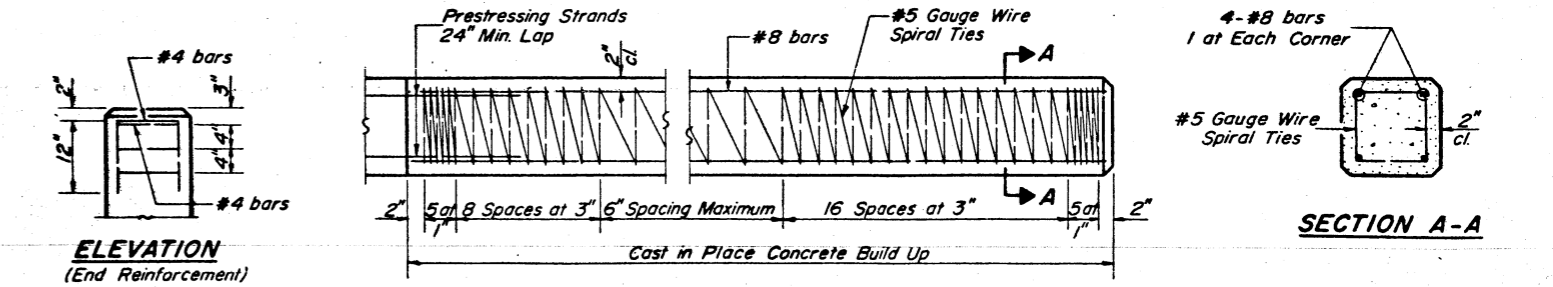
SECTION A-A FOR PILES 45' OR MORE

DETAIL OF PRECAST CONCRETE PILES



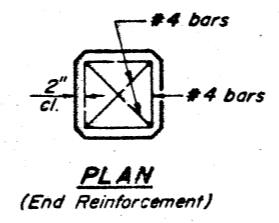
SECTION THRU PILE

PILE PLAN



ELEVATION (End Reinforcement)

PILE BUILD UP



PLAN (End Reinforcement)

DESIGN STRESSES
f_c' = 5,000 psi.
f_c' = 4,000 psi.
f_s' = 268,000 psi. (31,000 lbs.)
f_s' = 188,000 psi. (21,700 lbs.)

Note: Prestressing steel shall be non-galvanized extra high strength stress-relieved 7 wire strand. The nominal diameter shall be 7/16" and the minimum nominal cross-sectional area shall be 0.1155 square inch.

Handling: For pile lengths up to 65', use two slings placed at a distance of 0.21 L from each end. For piles longer than 65', use three slings placed at a distance of 0.12 L from each end at midpoint of pile.

DETAIL OF PRECAST PRESTRESSED CONCRETE PILES

PILE DETAILS
F.A.I. 72 SEC. 74-68-HB-1
PIATT COUNTY
STA 1359+51.04 (F.A.I.-72)