

ROADWAY GEOTECHNICAL REPORT

Delhi Bypass
FAP Route 310
Section 42-2
Jersey County
D-98-034-00

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Attachments: Plan & Profile
Boring Logs
Soil Test Data Sheets
Subgrade Support Ratios
Slope Stability Analyses
Grid Reinforced Slope Analyses
Settlement Analyses

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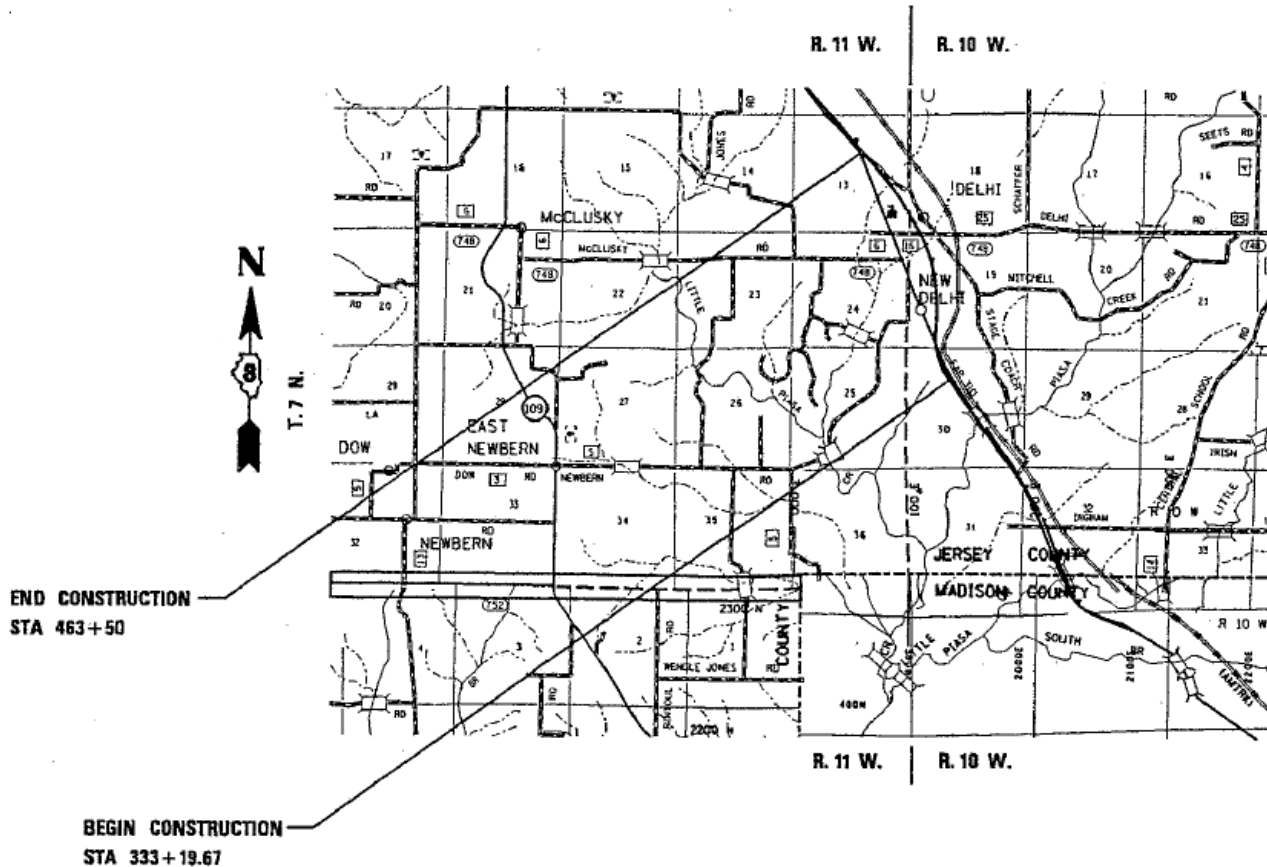
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SECTION 1 INTRODUCTION

This report summarizes the results of the geotechnical investigation for the FAP 310 alignment, Section 42-2 in Jersey County, Illinois. The project begins north of Piasa Creek and extends north, around the east side of New Delhi, connecting back to US 67, approximately $\frac{3}{4}$ mile north of Delhi Road. The length of the project is 13,030 feet (2.5 miles). The proposed improvement consists of providing a four-lane facility with 12 foot lanes, 4 to 6 foot inside shoulders, and 10 to 13 foot outside shoulders and involves drainage structures, side road realignment, and service/connector roads. See Figure 1 for the proposed alignment.

Figure 1: Proposed Alignment



Traffic analyses indicate that the Annual Average Daily Traffic (AADT) is as shown in Table 1.

Table 1: Summary of AADT Analyses

LOCATION	AADT	PASSENGER VEHICLES	SINGLE-AXLE TRUCK UNITS	MULTIPLE-AXLE TRUCK UNITS	HEAVY COMMERCIAL VEHICLES (HVC)
US 67	9000	8400	350	250	600
Delhi Rd (US 67 – Stagecoach)	950	921	29	0	29
Delhi Rd (Stagecoach – Limits)	1300	1271	29	0	29

The preliminary mainline pavement design consists of 10 ½ inches of full depth portland cement concrete atop 4" stabilized subbase pavement. The preliminary pavement design for the side roads is as follows:

- Stagecoach Road / US 67 Connector / Service Road: 3 inches bituminous concrete, with 8 inches Type B aggregate base course.
- Delhi Road: 3 inches bituminous concrete, with 11 inches of Type B aggregate base course.

SECTION 2 AREA GEOLOGY

The proposed alignment lies in the Springfield Plain physiographic province of Illinois and in the Till Plains Section of the Central Lowland physiographic province of the United States. The generalized soil profile consists of silty clay and silts overlying clay till. The till was laid down by advancing glacial ice and covered by the overlying loess.

Based on the review of the Jersey County Soil Survey, the soil associations along the proposed alignment consist of the following:

Bottomland and Terrace Soil Association: nearly level to gently sloping, poorly drained to well drained bottomland soils and nearly level to steep, imperfectly drained to well drained terrace soils.

Clinton-Keomah Association: nearly level to very steep, poorly drained to moderately well-drained, light-colored and moderately dark-colored upland soils developed from moderately thick loess.

The project consists of surficial materials from the Glasford Formation – the Radnor Till and Sterling Till Members. Bedrock is generally from the Carbondale Formation, formed during the Pennsylvanian period. The Carbondale Formation consists of four coal layers – Nos. 2, 5, 6, 7; limestone; sandstone; shale; and underclay.

Elevations along the existing ground surface vary between 514 and 623 feet. The alignment includes residential developments, wooded areas, and farmland.

According to the Illinois State Geological Survey (ISGS), the nearest mine to the proposed project is approximately 1,100 feet east of the northern project limits. This mine, identified in the ISGS Coal Mine directory as Mine 4430, is of unknown name, type, and operational information.

SECTION 3 GENERAL PROJECT INFORMATION

3.1 Slope Stability

Slope stability analyses were performed at locations with fill or cut height exceeding 15 feet with slopes of 2:1 or steeper. Soil strength parameters were obtained from the stability borings that indicated the most critical soil conditions. Undrained shear strength values were obtained from Rimac tests. The XSTABL program was used to calculate the factors of safety according to the simplified Bishop (1955) method of analysis. This method uses the equilibrium approach.

3.2 Settlement

Settlement analysis was conducted for locations having soil layers with strength values less than 1.00 tons per square foot (tsf), moisture contents greater than 25%, and fill heights over 15 feet.

3.3 Subgrade

The subgrade will consist of in-situ soils and fill supplied from nearby borrow pits and earth excavation. The soils from these areas typically consist of silty clay loam and silty clay and are considered frost susceptible. These types of soil have clay contents greater than 20%; plasticity indices (PI's) greater than 15; and are considered suitable for lime modification, unless otherwise noted. Soil conditions should be field verified with a dynamic cone penetrometer (DCP) to determine the appropriate treatment depth.

The Illinois Bearing Ratio (IBR) for the fill and in-situ soils, unless otherwise noted, should be assumed to be 3. These soils are considered poorly draining and susceptible to frost action. The subgrade support ratio (SSR) for the majority of these types of soil is poor. See Appendix D for the SSR charts.

3.4 Culverts

As per Section 2.3.2 of the Bridge Manual, TS&L's are required for cast-in-place multiple barrel box culverts. Borings were taken at the proposed culvert locations to determine the suitability of cast-in-place or pre-cast culverts.

3.5 Recommendations

The subgrade, unless otherwise noted, should be modified with lime by-product. The treatment depth should be determined based upon field DCP measurements and existing boring information. The lime by-product will reduce the soils plasticity and shrink-swell potential and provide a stable working platform during paving operations. For estimating purposes, 4% by dry weight, of lime by-product should be assumed.

Because of generally poor drainage and frost susceptible soils, longitudinal subsurface drains are recommended at the edges of pavement for the proposed roadway. The drains should be constructed in accordance with the subsurface drain standard 601001.

SECTION 4 FAP 310 MAINLINE
STATION 333+19.67 to STATION 470+00
Box Culvert Structure STA 349+37
Box Culvert Structure STA 437+56
Box Culvert Structure STA 460+86.3

US 67 Connector STA 30+00 = STA 354+89 (FAP 310)
Delhi Road STA 50+00 = STA 410+78.8 (FAP 310)

4.1 General Description

The proposed alignment from Station 333+19.67 to Station 470+00 begins in the north half of Section 30, Township 7 North, Range 11 West, 3rd Principal Meridian (P.M.) and extends to the northwester quarter of Section 13, Township 7, Range 10 West, 3rd P.M. The general ground surface is moderately to steeply sloping, with elevations

ranging from 514 to 623 feet. Cut sections vary between 0 and 37 feet and fill heights vary between 0 to 47 feet. See Appendix A for plan and profile. This location is predominantly cultivated fields and residential areas.

4.2 Soil Stratigraphy and Groundwater Conditions

Generalized subsurface profiles were developed along the proposed improvements. The subsurface profiles are based on hand augers and stability borings performed by the District. The soil investigation was conducted during January, October, and December 2002; and January, February, March, July, August, and October 2003. The rainfall was below average for January and August 2003; average for October and December 2002, and February, March, July, and October 2003; and above average for January 2002. The boring logs are provided in Appendix B; corresponding soil test data is found in Appendix C.

The soil profile consists of intermingling layers of very soft to hard clay, clay loam, loam, sand, sandy clay, sandy clay loam, sandy loam, silt, silty clay, silty clay loam, and silty loam. Unconfined compressive strengths (UCS) range from 0.2 to 10.0 tsf with an average value of 2.6 tsf. Moisture contents range from 6 to 68%, with an average value of 18%.

Water levels at the boring locations were allowed to stabilize and were recorded at the time of drilling. The elevation of the groundwater during the investigation ranged between 451 and 596 feet. At the majority of the locations dry holes were encountered, indicating groundwater was below the final depth of the boring.

4.3 Slope Stability

Table 2 provides the results of the slope stability analyses for this section of the alignment.

Table 2: Summary of Slope Stability Analyses

Station	Boring	Location	Cut / Fill Height	Slope	Factor of Safety		Req'd FOS (MB / S)
					Mod. Bishop	Seismic	
336+00	SB 28	336+00, 38 ft LT	Cut, 18 ft	10:2:1	5.830	4.219	1.7 / 1.0
338+00	SB 29	338+00, 9 ft RT	Cut, 33 ft	10:2:1	8.562	6.521	1.7 / 1.0
347+00	SB 33	347+00, 40 ft LT	Fill, 22 ft	1	2.728	2.212	1.5 / 1.0
349+00, Lt	SB 34	349+00, 10 ft RT	Fill, 44 ft	1	1.800	1.464	1.5 / 1.0
349+00, Rt	SB 34	349+00, 10 ft RT	Fill, 44 ft	2:1	1.683	1.404	1.5 / 1.0
436+00	SB 65	436+00, 41 ft RT	Fill, 30 ft	1	3.351	2.717	1.5 / 1.0
438+00, Lt	SB 66	438+00, 41 ft LT	Fill, 40 ft	1	2.057	1.666	1.5 / 1.0
438+00, Rt	SB 66	438+00, 41 ft LT	Fill, 40 ft	1	2.062	1.656	1.5 / 1.0
452+00, Lt	SB 71	452+00, 36 ft RT	Cut, 19 ft	2	5.157	4.079	1.7 / 1.0
452+00, Rt	SB 71	452+00, 36 ft RT	Cut, 16 ft	2	6.079	4.776	1.7 / 1.0
454+00, Lt	SB 72	454+00, 2 ft RT	Cut, 28 ft	2	3.521	2.793	1.7 / 1.0
454+00, Rt	SB 72	454+00, 2 ft RT	Cut, 21 ft	2	5.194	4.025	1.7 / 1.0
456+00, Lt	SB 73	456+00, 26 ft LT	Cut, 22 ft	2	8.686	5.697	1.7 / 1.0
456+00, Rt	SB 73	456+00, 26 ft LT	Cut, 22 ft	2	6.306	4.994	1.7 / 1.0
458+00	SB 74	458+00, 31 ft RT	Cut, 16 ft	2	7.825	5.792	1.7 / 1.0

¹ 2:1 slope with two 10:1 benches, each 10 feet long

² 2:1 slope with one 10:1 bench, 10 feet long

Slope reinforcement will be required at locations with 2:1 slopes and fill heights exceeding 15 feet. Grid Reinforced Slopes will be needed from Station 346+90 to Station 351+20 and from Station 435+90 to Station 440+10. The Grid Reinforced Slope profiles and cross sections can be found in Appendix F.

No further slope stability problems are anticipated for this location. The soil profiles for the stability analysis can be found in Appendix E.

4.4 Settlement

Settlement calculations were based on empirical formulas utilizing classification data. Table 3 provides the results of the settlement analyses for this section of the alignment.

Table 3: Summary of Settlement Analyses

Station	Boring	Location	Fill Height (ft)	Settlement (in)	t ₉₀ (days)
347+50	BXC-1-1	347+52, 159.3 ft LT	20.4	1.97	4.58
423+20	C-3-1	423+20, 122 ft RT	16.5	3.97	10.22
423+66	C-3-2	423+67, CL	16.0	1.17	13.65

The amount of settlement at Stations 347+50 and 423+66 is less than 2 inches, which is considered negligible and should have little to no impact on the roadway. Station 423+20 lies along the footprint of a pipe culvert (Station 423+30, Skew 5° Left Ahead), and the problematic in-situ soils lie above the flow line, and will be excavated for the culvert installation, thereby negating settlement potential at this location.

Detailed settlement analysis is provided in Appendix G.

The remaining locations along this section of the alignment do not require settlement analyses. The subsurface soils do not exhibit properties that indicate significant settlement. Less than 2 inches of settlement is anticipated.

4.5 Subgrade

The subgrade materials from Station 335+00 to 345+50 are not reactive with lime. Type B subbase material should be used in lieu of lime in these locations. Fly-ash or slag-modified Portland cement can be used in lieu of lime to obtain the desired soil strength.

4.6 Culverts

Table 4 presents a summary of the soil conditions and type recommendation for the box culverts along this section of alignment. The soil types, UCS values, and water contents are for the five feet of soil below the flow line.

Table 4: Culvert Summary

Station	Size	Soil Types	UCS (tsf)	Water Content (%)	CIP or Pre-Cast
349+37	6' x 6'	Sandy Loam, Silty Clay, Silty Loam	0.8 – 4.0 Avg = 3.0	16 – 25 Avg = 20	See Below
437+56	Double 8' x 8'	Sand, Sandy Loam, Silty Clay, Silty Clay Loam	1.3 – 6.8 Avg = 4.2	6 – 16 Avg = 13	Pre-Cast
460+86.3	12' x 6'	Clay, Clay Loam, Clayey Shale, Weathered Shale	0.6 – 4.5 Avg = 2.2	15 – 22 Avg = 18	Pre-Cast

The culverts at Stations 437+56 and 460+86.3 can be constructed as pre-cast structures. The culvert at Station 349+37 has soils at the flow line that have low strengths, and moderate water contents. This structure can be constructed as a pre-

cast structure if the 3.5 feet of unsuitable material below the flow line can be economically removed and replaced.

4.7 Recommendations

No settlement problems are anticipated along this section of alignment.

Grid Reinforced Slopes will be needed from Station 346+90 to Station 351+20 and from Station 435+90 to Station 440+10. An estimated quantity of 34,410 square yards of geogrid material will be necessary at these locations and a Grid Reinforced Slope Special Provision will be required.

The subgrade materials from Station 335+00 to 345+50 are not reactive with lime. Type B subbase material should be used in lieu of lime in these locations. Fly-ash or slag-modified Portland cement can be used in lieu of lime to obtain the desired soil strength.

The culverts at Stations 437+56 and 460+86.3 can be constructed as pre-cast structures. The culvert at Station 349+37 has soils at the flow line that have low strengths, and moderate water contents. This structure can be constructed as a pre-cast structure if the 3.5 feet of unsuitable material below the flow line can be economically removed and replaced.

SECTION 5 SERVICE ROAD STATION 100+00 to STATION 104+00

5.1 General Description

The proposed Service Road is located in the southwest quarter of Section 19, Township 7 North, Range 11 West, 3rd P.M. The general ground surface level ranges from 549 to 571 feet. Cut sections vary between 0 to 5 feet and fill heights vary between 0 to 14 feet. See Appendix A for plan and profile. The existing Service Road, known locally as Ivy Lane, will be reconstructed to allow access to existing US 67.

5.2 Soil Stratigraphy and Groundwater Conditions

Generalized subsurface profiles were developed along the proposed improvements. The subsurface profiles are based on stability borings performed by the District. The soil investigation was conducted during March 2003. The rainfall during this time was average. The boring logs are provided in Appendix B; corresponding soil test data is found in Appendix C.

The soil profile consists of intermingling layers of stiff to hard silty clay loam. UCS values ranged from 1.3 to 4.5 tsf, with average values of 2.2 tsf. Water contents ranged from 11 to 21%, with average values of 16%.

Dry holes were encountered at all boring locations, indicating groundwater was below the final depth of the boring.

**SECTION 6 US 67 CONNECTOR
STATION 24+30.47 to STATION 30+00**

6.1 General Description

The proposed US 67 Connector alignment is located in the southwest quarter of Section 19, Township 7 North, Range 11 West, 3rd P.M. The general ground surface level ranges from 547 to 572 feet. Cut sections vary between 0 to 9 feet and fill heights vary between 0 to 20 feet. See Appendix A for plan and profile. The US 67 will connect existing US 67 with proposed US 67, south of Delhi Road.

6.2 Soil Stratigraphy and Groundwater Conditions

Generalized subsurface profiles were developed along the proposed improvements. The subsurface profiles are based on hand augers and stability borings performed by the District. The soil investigation was conducted during March and August 2003. The rainfall during March 2003 was average and below average during August 2003. The boring logs are provided in Appendix B; corresponding soil test data is found in Appendix C.

The soil profile consists of intermingling layers of soft to hard clay loam, loamy sand, sandy loam, silty clay, silty clay loam, and silty loam. UCS values ranged from 0.5 to 7.5 tsf, with average values of 2.4 tsf. Water contents ranged from 12 to 27%, with average values of 18%.

Water levels at the boring locations were allowed to stabilize and were recorded at time intervals ranging from 0 to 24 hours. The elevation of the groundwater during investigation was 541.6 feet, and at some locations, dry holes were encountered, indicating groundwater was below the final depth of the boring.

6.3 Slope Stability

Table 5 provides the results of the slope stability analyses for this section of the alignment.

Table 5: Summary of Slope Stability Analyses

Station	Boring	Location	Fill Height (ft)	Slope	Factor of Safety		Req'd FOS (MB / S)
					Mod. Bishop	Seismic	
26+64	SB 79	26+65, 25 ft LT	20	3:1	3.272	2.499	1.5 / 1.0

No slope stability problems are anticipated for this location.

6.4 Settlement

Settlement calculations at Station 26+65 were based on empirical formulas utilizing classification data from boring SB 79 (Station 26+65, 25 ft Left). Settlement below the proposed embankment was based on a fill height of 17 ft and 90% consolidation. Settlement is estimated to be 3.41 inches in 7.3 days. If the top 2.5 feet of in-situ material can be economically removed and replaced, the settlement will be reduced to 0.38 inches, which would have negligible impact on the roadway. Settlement analyses can be found in Appendix F.

6.5 Recommendations

Settlement problems are anticipated at Station 26+65; however, the possibility of

settlement is diminished upon the removal of the upper 2.5 feet of soil and replacement with granular materials.

No slope stability problems are anticipated at this location.

**SECTION 7 DELHI ROAD
STATION 36+51.03 to STATION 66+20.86**

**Stagecoach Road North STA 70+00 = STA 55+00 (Delhi Road)
Stagecoach Road South STA 70+00 = STA 57+50 (Delhi Road)**

7.1 General Description

The proposed Delhi Road alignment begins in the southeast quarter of Section 13, Township 7 North, Range 11 West, 3rd P.M. and extends easterly to the southwest quarter of Section 18, Township 7 North, Range 10 West, 3rd P.M. The general ground surface level ranges from 576 to 598 ft. Cut sections vary from 0 to 10 feet and fill heights vary from 0 to 8 feet. See Appendix A for plan and profile. Existing Delhi Road will be reconstructed from existing US 67 east to just past Stagecoach Road.

7.2 Soil Stratigraphy and Groundwater Conditions

Generalized subsurface profiles were developed along the proposed improvements. The subsurface profiles are based on hand augers performed by the District. The soil investigation was conducted during February and March 2003. The rainfall during this time was average. The boring logs are provided in Appendix B; corresponding soil test data is found in Appendix C.

The soil profile consists of intermingling layers of clayey silt, loamy silt, sandy loam, silty clay, silty clay loam, and silty loam. UCS values ranged from 0.7 to 2.7 tsf, with average values of 1.8 tsf. Water contents ranged from 22 to 24%, with average values of 23%.

Along Delhi Road, dry holes were encountered; indicating groundwater was below the final depth of the boring.

REFERENCES

Das, B.M. (1999). *Principles of Foundation Engineering, 4th Ed.* PWS Publishing, Pacific Grove, Ca.

Day, R.W. (2000). *Geotechnical Engineers Portable Handbook.* McGraw-Hill, New York.

Directory of Coal Mines in Illinois – Jersey County. (2008). Coal Section, Illinois State Geological Society, Champaign, Ill.

Geotechnical Manual. (1999). Illinois Department of Transportation, Springfield, Ill.

Highway Standards. (2007). Bureau of Design and Environment, Illinois Department of Transportation, Springfield, Ill.

Holtz, R.D. and Kovacs, W.D. (1981). *An Introduction to Geotechnical Engineering.* Prentice-Hall, N.J.

“ISWS Climate Data – Results – Station 114489 (Jerseyville).” (2009). <<http://www.isws.illinois.edu/data/climatedb/data.asp>>. (August 13, 2009).

Soil Survey of Jersey County. (2007). Soil Conservation Service, United States Department of Agriculture, Washington D.C.

Appendix A

Plan and Profile

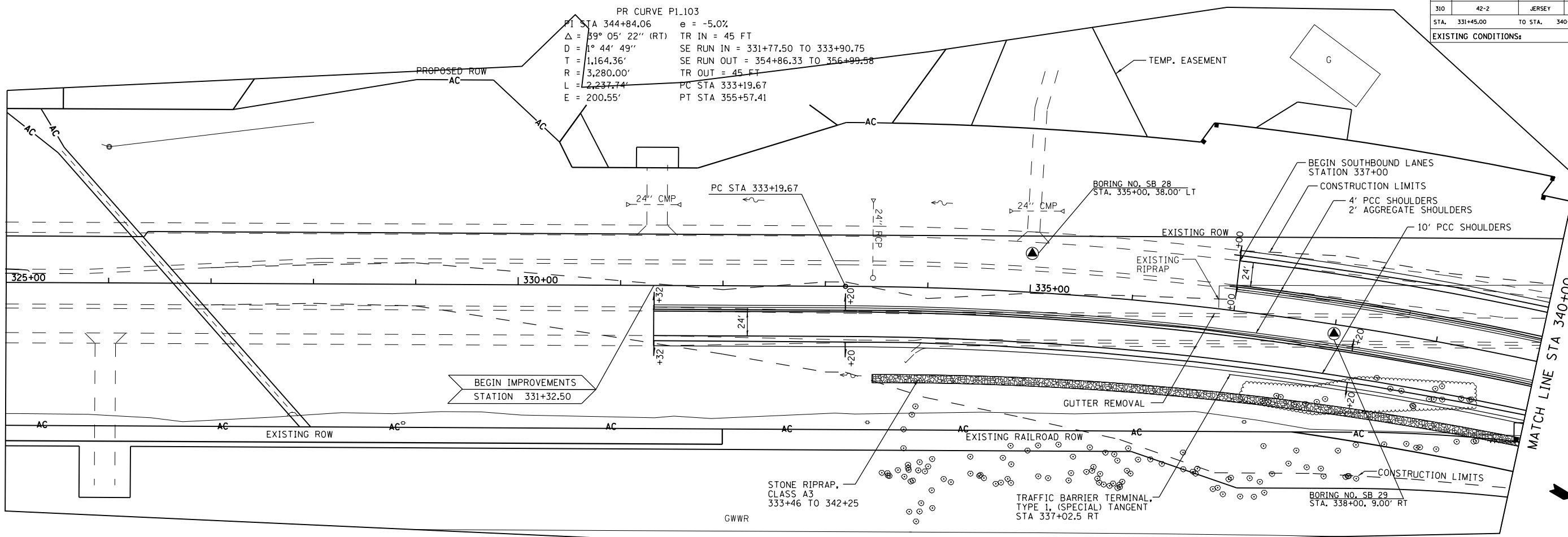
Plan and Profile

Station 333+19.67 to Station 463+50

FAP ROUTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO
310	42-2	JERSEY	393	36
STA. 331+45.00		TO STA. 340+00.00		

EXISTING CONDITIONS: CONTRACT NO. 76568

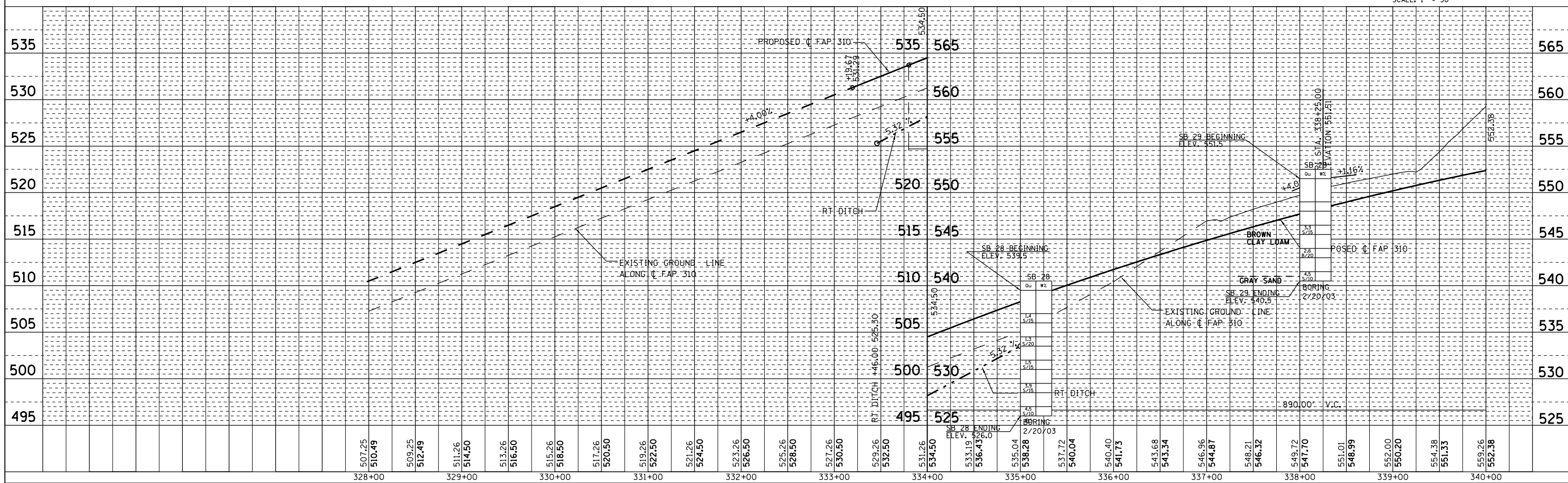
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 $\Delta = 89^\circ 05' 22''$ (RT) TR IN = 45 FT
 D = $1^\circ 44' 49''$ SE RUN IN = 331+77.50 TO 333+90.75
 T = 1,164.36' SE RUN OUT = 354+86.33 TO 356+99.58
 R = 3,280.00' TR OUT = 45 FT
 L = 2,237.74' PC STA 333+19.67
 E = 200.55' PT STA 355+57.41



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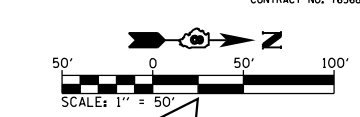
HOMER L. CHASTAIN & ASSOCIATES, LLP
 CONSULTING ENGINEERS
 DECATUR (317) 422-8544
 CHICAGO (773) 714-0050
 ROCKFORD (815) 489-0850
 184-001897

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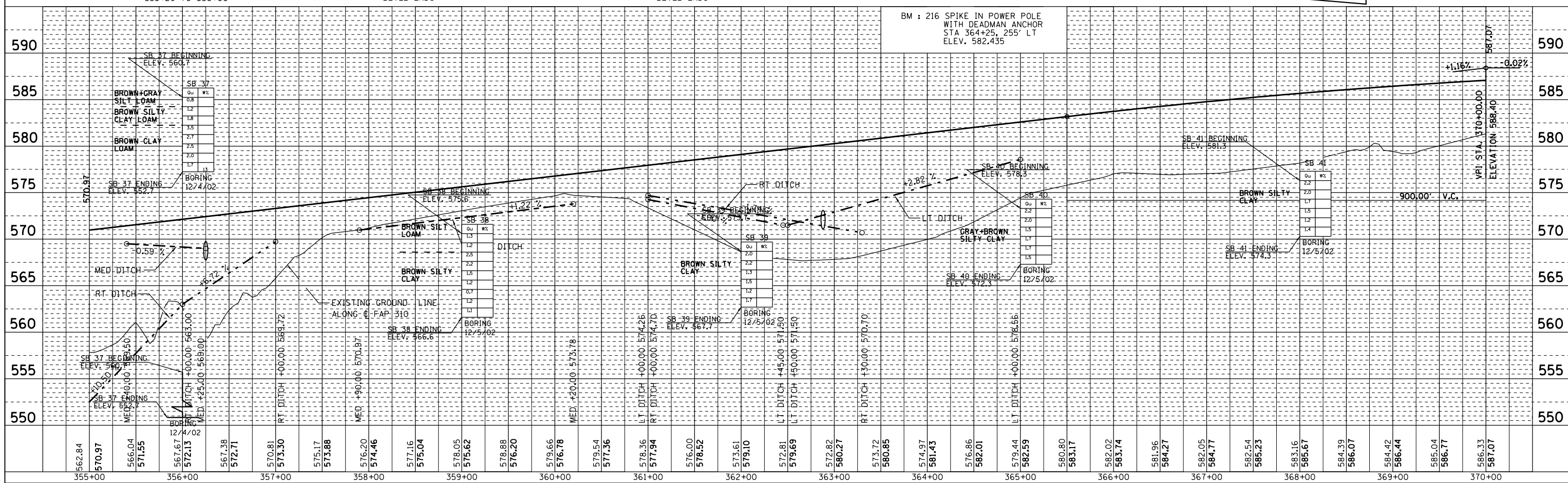
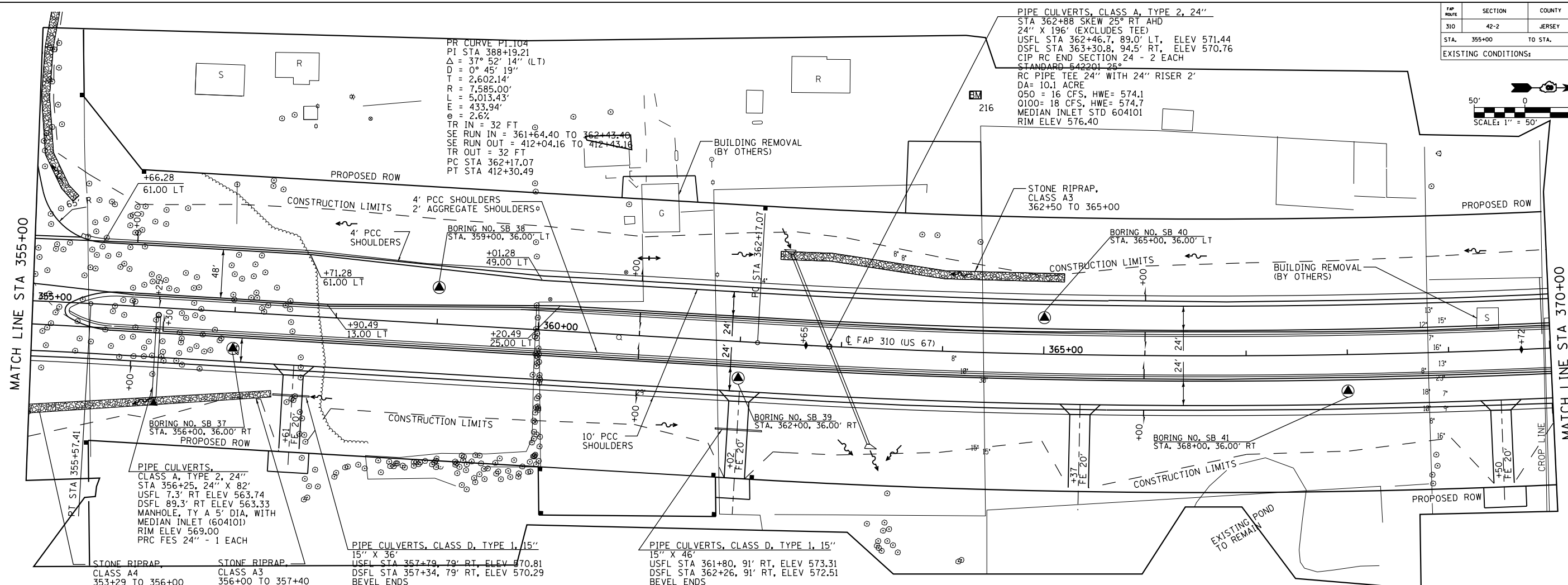


STA 328+00 TO STA 340+00

FAP ROUTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO
310	42-2	JERSEY	393	39
STA. 355+00		TO STA.	370+00	



CONTRACT NO. 76568



STA 355+00 TO STA 370+00

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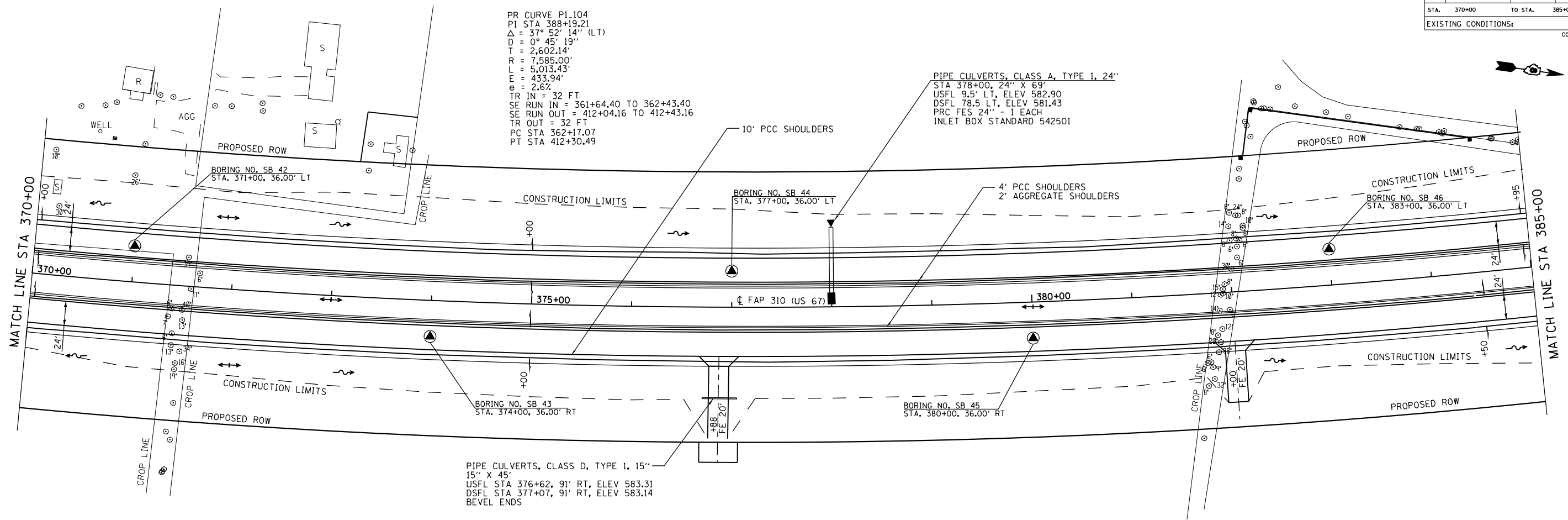
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EXISTING CONDITIONS:				
CONTRACT NO. 76568				

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 $L = 5,013.43'$
 $E = 433.94'$
 $e = 2.67'$
 TR IN = 32 FT
 SE RUN IN = 361+64.40 TO 362+43.40
 SE RUN OUT = 412+04.16 TO 412+43.16
 TR OUT = 32 FT
 PC STA 362+17.07
 PT STA 412+30.49

PIPE CULVERTS, CLASS A, TYPE 1, 24"
 STA 378+00, 24" X 69"
 USFL 9.5' LT, ELEV 582.90
 DSFL 78.5' LT, ELEV 581.43
 PRC FES 24" - 1 EACH
 INLET BOX STANDARD 542501

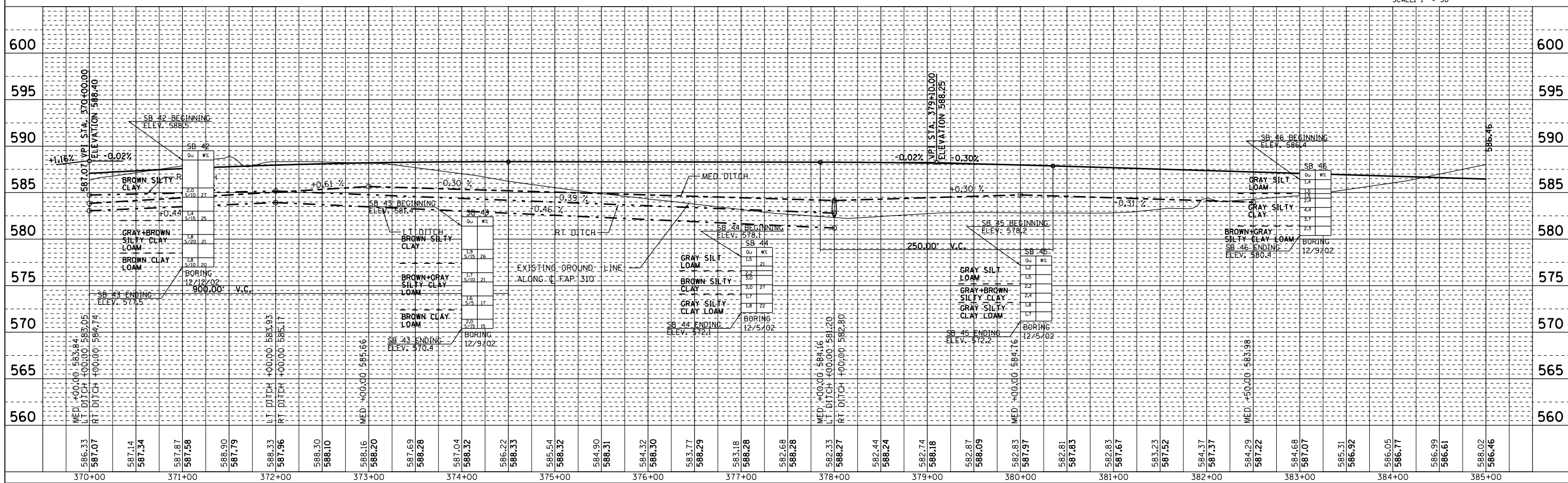
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 DSFL STA 377+07, 91' RT, ELEV 583.14
 BEVEL ENDS



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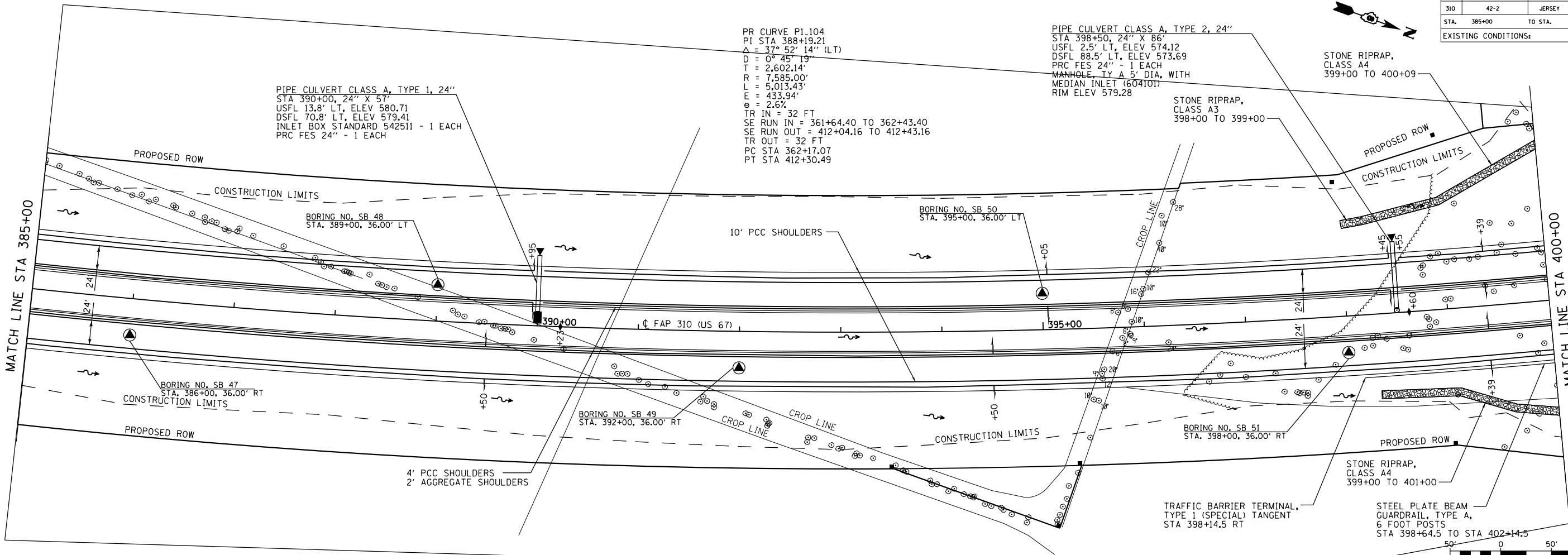
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STA 370+00 TO STA 385+00

FAP ROUTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO
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CONTRACT NO. 76568

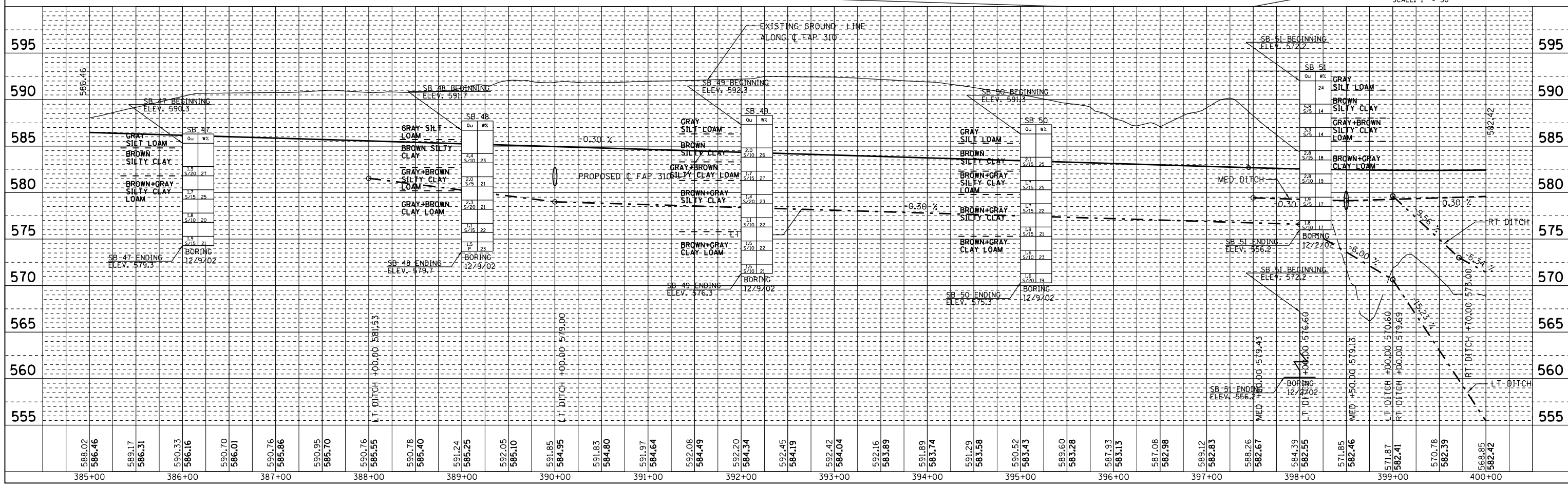


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	DATE	

STRUCTURE NOTATIONS GRID

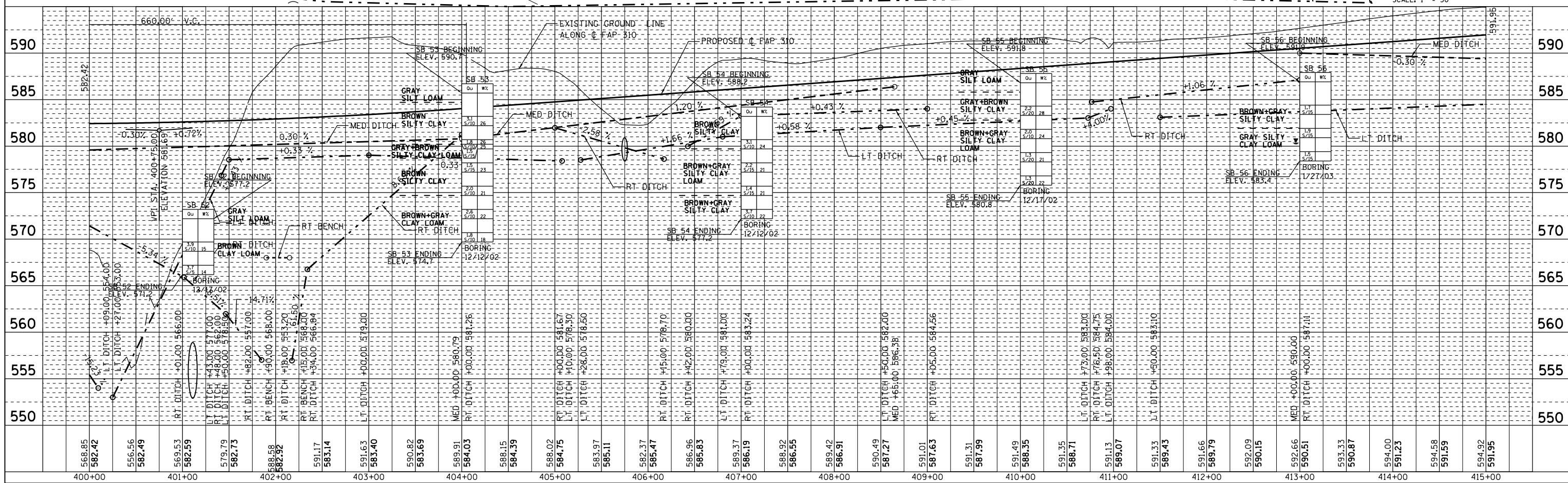
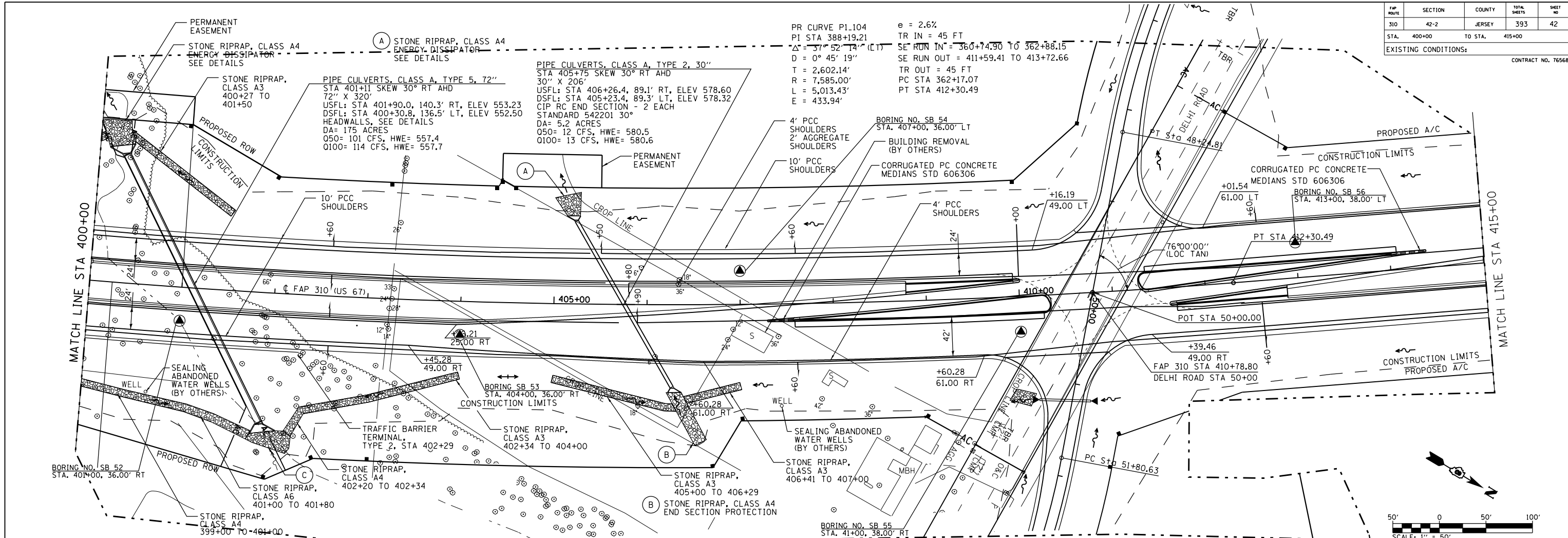


STA 385+00 TO STA 400+00

FAP ROUTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO
310	42-2	JERSEY	393	42
STA. 400+00		TO STA. 415+00		

EXISTING CONDITIONS:

CONTRACT NO. 76568



STA 400+00 TO STA 415+00

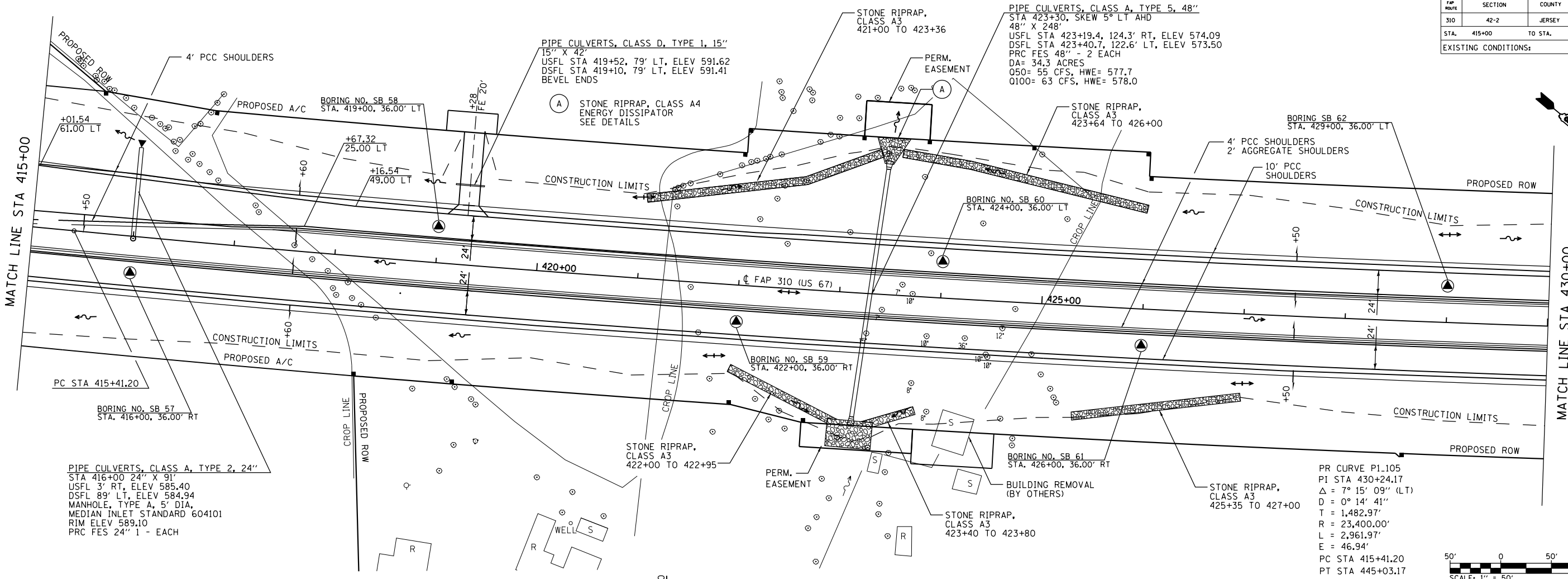
PLAN	DATE
BY	
DATE	
REVISION	
NO.	

HOMER L. CHASTAIN & ASSOCIATES, LLP
 CONSULTING ENGINEERS
 DECATUR CHICAGO ROCKFORD
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 184-001897

PROFILE	DATE
BY	
DATE	
REVISION	
NO.	

FAP ROUTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO
310	42-2	JERSEY	393	43
STA. 415+00		TO STA. 430+00		

EXISTING CONDITIONS: CONTRACT NO. 76568

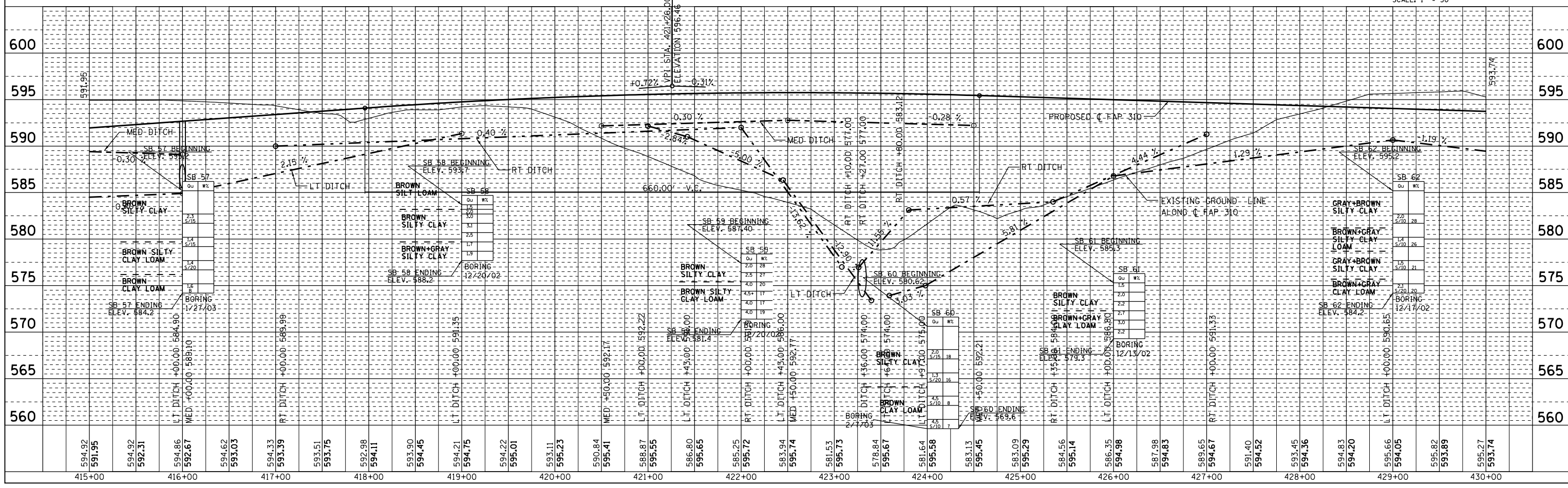


PLAN	DATE
REVISIONS	
NO.	
BY	
DATE	

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 DECATUR, IL 62521
 CHICAGO, IL 60604
 ROCKFORD, IL 61107
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 (815) 489-0050
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PROFILE	DATE
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DATE	

STRUCTURE NOTATIONS GRID



STA 415+00 TO STA 430+00

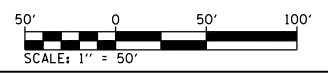
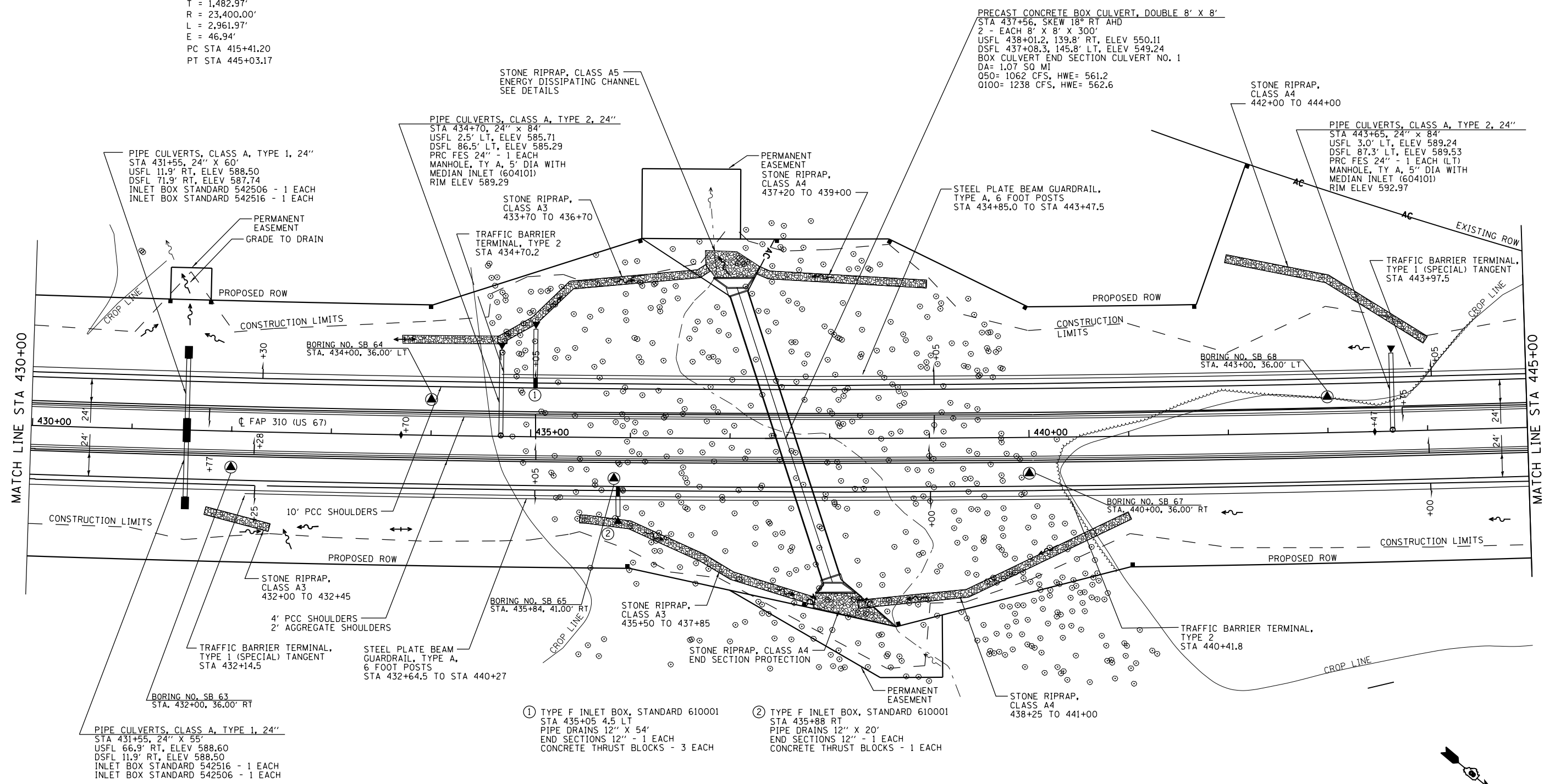
FAP ROUTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO
310	42-2	JERSEY	393	44
STA. 430+00 TO STA. 445+00			EXISTING CONDITIONS:	

CONTRACT NO. 76568

PR CURVE P1.105
 PI STA 430+24.17
 $\Delta = 7^{\circ} 15' 09''$ (LT)
 $D = 0^{\circ} 14' 41''$
 $T = 1,482.97'$
 $R = 23,400.00'$
 $L = 2,961.97'$
 $E = 46.94'$
 PC STA 415+41.20
 PT STA 445+03.17

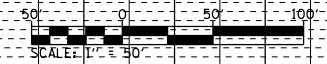
PLAN	SERIALIZED	DATE
NOTE BOOK	PLOTTED	
NO.	BY	
	DATE	
	NO.	
	NO.	
	NO.	
	NO.	

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 (773) 422-8544 (773) 714-0060 (815) 489-0850
 184-001897



ILLINOIS DEPARTMENT OF TRANSPORTATION
ROADWAY PLAN STA 430+00 TO 445+00
 FAP ROUTE 310 (US 67)
 SECTION 42-2
 JERSEY COUNTY
 DRAWN BY: EMR
 CHECKED BY: CAS

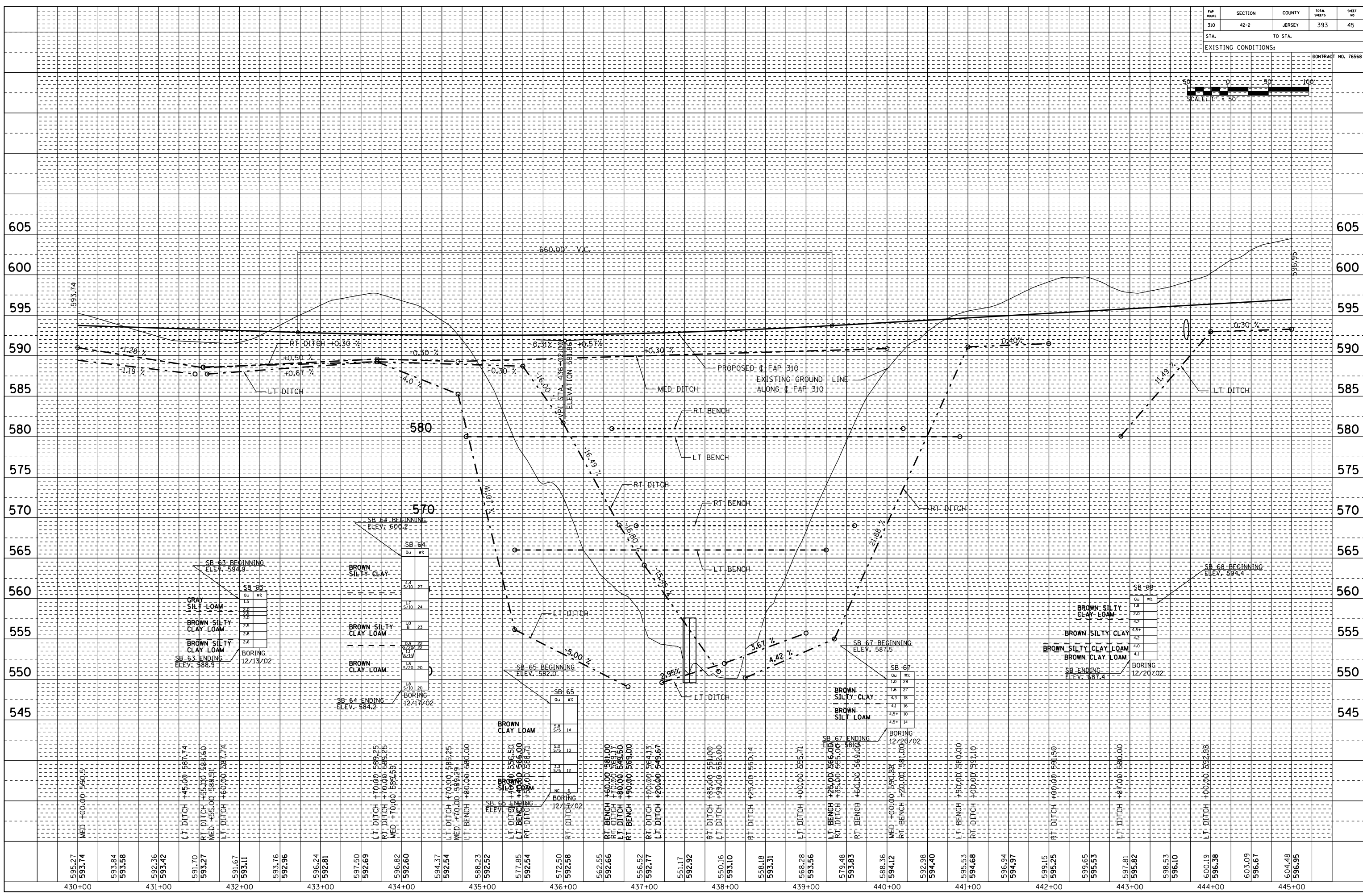
REVISIONS	
NAME	DATE



PLAN	DATE
DRAWN BY: _____ CHECKED BY: _____ DATE: _____	DATE: _____

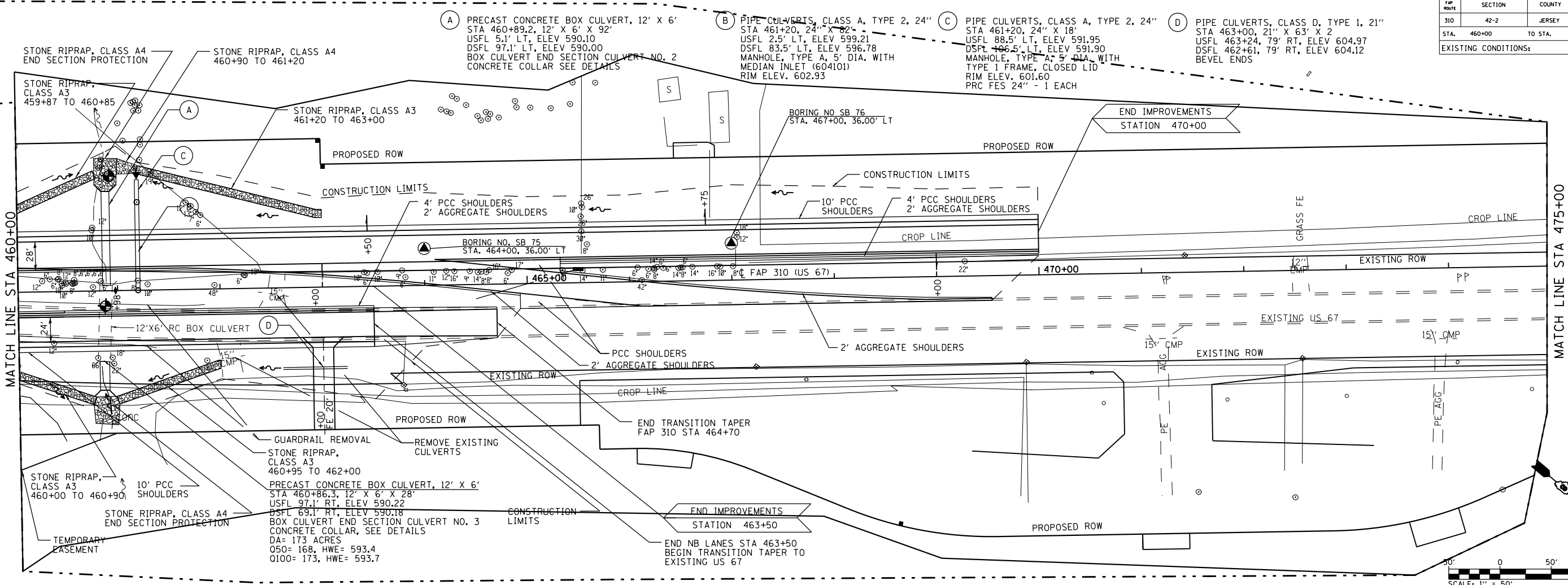
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 CONSULTING ENGINEERS
 DECATUR 617-422-8544
 CHICAGO 773-714-0050
 ROCKFORD 815-489-0850
 184-001897

PROFILE	DATE
DRAWN BY: _____ CHECKED BY: _____ DATE: _____	DATE: _____



FAP ROUTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO
310	42-2	JERSEY	393	47
STA. 460+00		TO STA. 475+00		

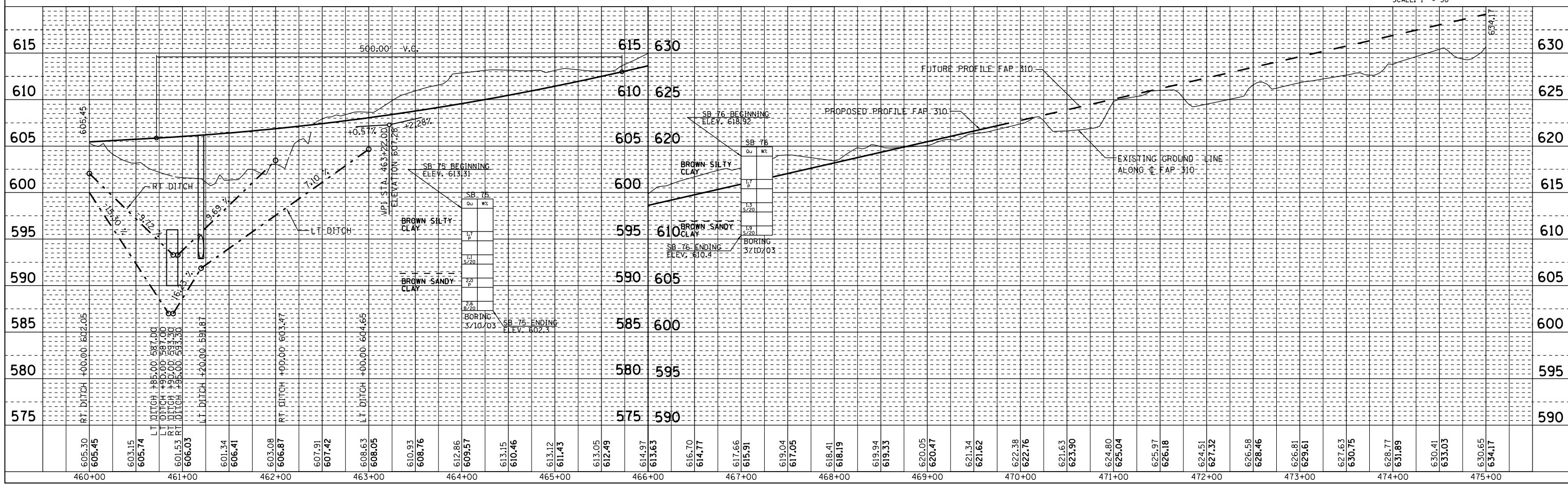
EXISTING CONDITIONS:
CONTRACT NO. 76568



PLAN	SURVEYED	DATE
	PLOTTED	
	NOTED	
	REVISIONS	
	BY	
	DATE	

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CHICAGO 773-714-0060
ROCKFORD 815-489-0860
184-001897

PROFILE	SURVEYED	DATE
	PLOTTED	
	NOTED	
	REVISIONS	
	BY	
	DATE	



STA 460+00 TO STA 475+00

Plan and Profile

Service Road

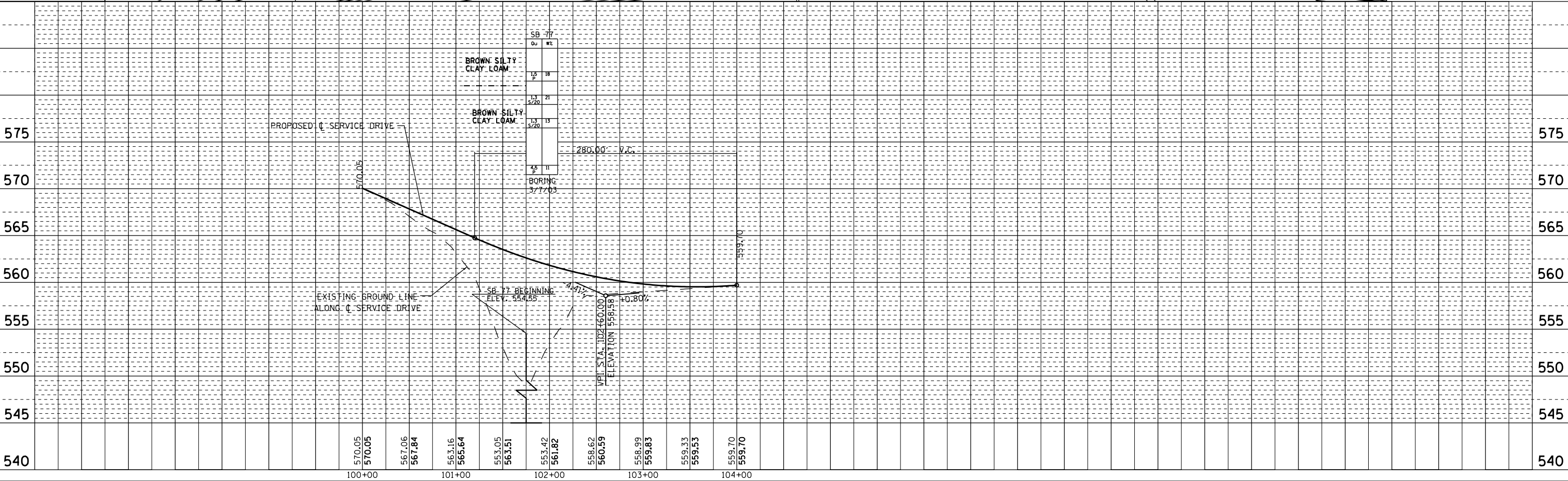
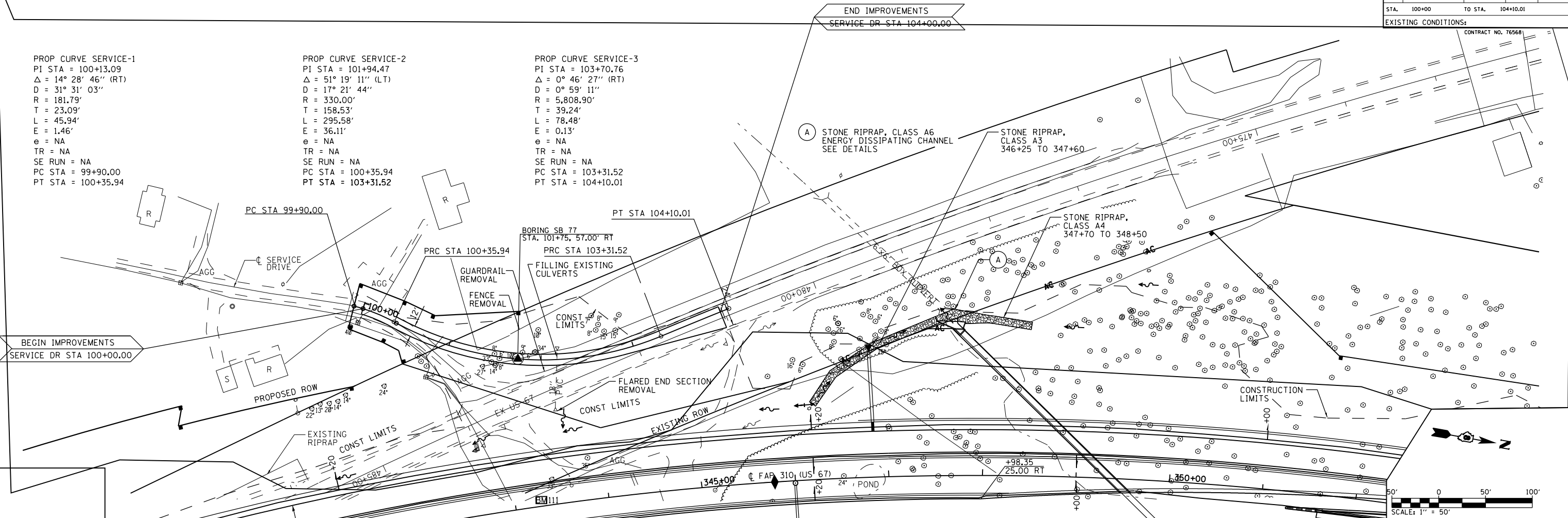
FAP ROUTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO
310	42-2	JERSEY	393	48
STA. 100+00		TO STA. 104+10.01		

EXISTING CONDITIONS: CONTRACT NO. 76568

PROP CURVE SERVICE-1
 PI STA = 100+13.09
 $\Delta = 14^\circ 28' 46''$ (RT)
 $D = 31^\circ 31' 03''$
 $R = 181.79'$
 $T = 23.09'$
 $L = 45.94'$
 $E = 1.46'$
 $e = NA$
 $TR = NA$
 $SE RUN = NA$
 $PC STA = 99+90.00$
 $PT STA = 100+35.94$

PROP CURVE SERVICE-2
 PI STA = 101+94.47
 $\Delta = 51^\circ 19' 11''$ (LT)
 $D = 17^\circ 21' 44''$
 $R = 330.00'$
 $T = 158.53'$
 $L = 295.58'$
 $E = 36.11'$
 $e = NA$
 $TR = NA$
 $SE RUN = NA$
 $PC STA = 100+35.94$
 $PT STA = 103+31.52$

PROP CURVE SERVICE-3
 PI STA = 103+70.76
 $\Delta = 0^\circ 46' 27''$ (RT)
 $D = 0^\circ 59' 11''$
 $R = 5,808.90'$
 $T = 39.24'$
 $L = 78.48'$
 $E = 0.13'$
 $e = NA$
 $TR = NA$
 $SE RUN = NA$
 $PC STA = 103+31.52$
 $PT STA = 104+10.01$



SERVICE DRIVE STA 100+00 TO STA 104+00

PLAN	DATE
SURVEYED	
PLOTTED	
NOTED	
NO. _____	

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PROFILE	DATE
SURVEYED	
PLOTTED	
NOTED	
NO. _____	

Plan and Profile

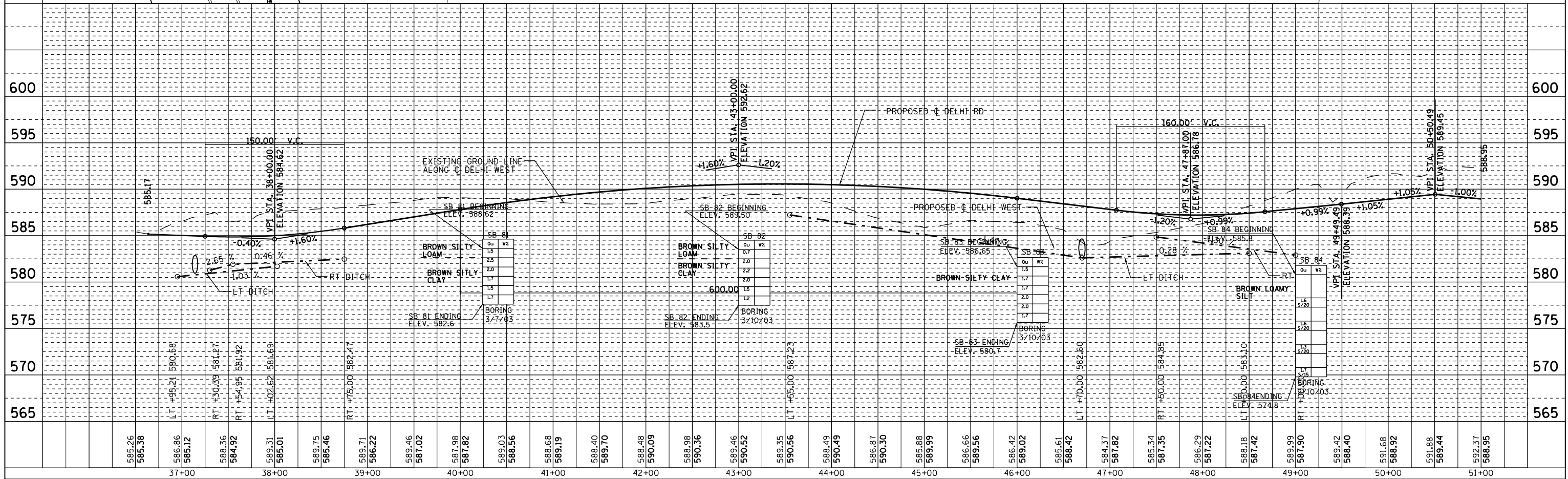
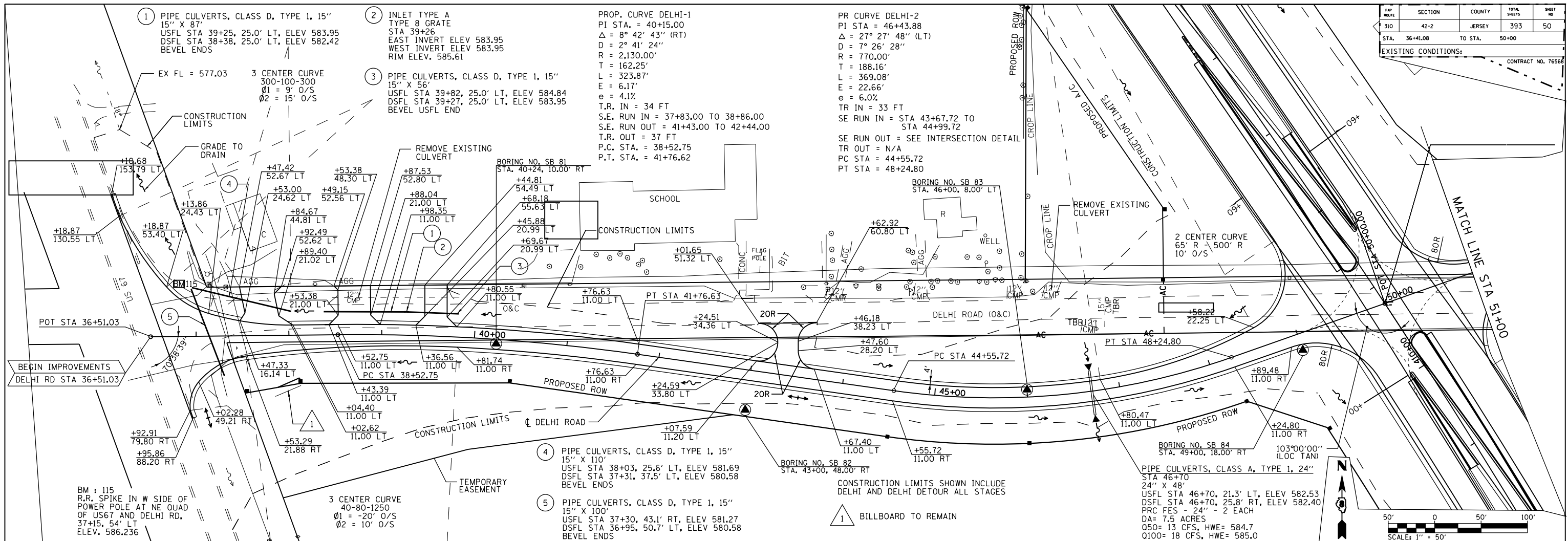
US 67 Connector

Plan and Profile

Delhi Road

FAP ROUTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO
310	42-2	JERSEY	393	50
STA. 36+41.08		TO STA. 50+00		

EXISTING CONDITIONS: CONTRACT NO. 76568



PLAN	DATE
BY: _____	DATE: _____
NO. _____	

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 DECATUR, CHICAGO, ROCKFORD

PROFILE	DATE
BY: _____	DATE: _____
NO. _____	

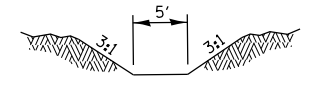
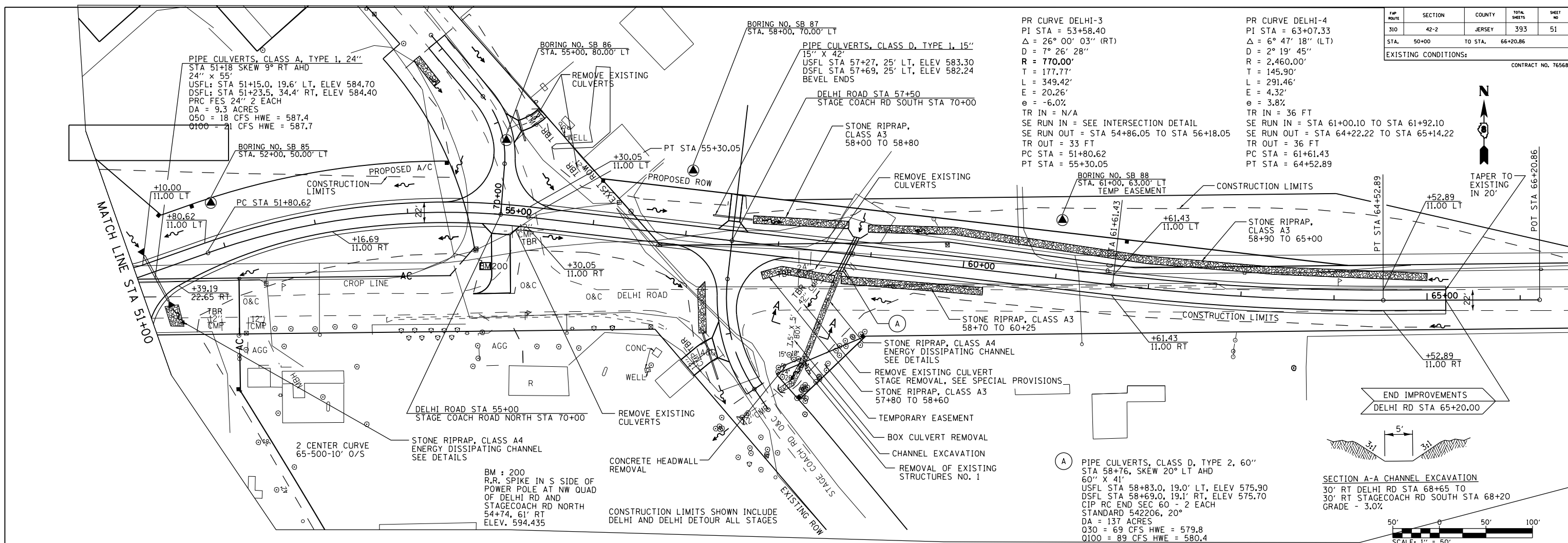
FIP ROUTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO
310	42-2	JERSEY	393	51
STA. 50+00		TO STA. 66+20.86		

CONTRACT NO. 76568

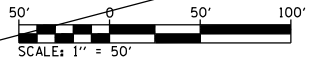
EXISTING CONDITIONS:

PR CURVE DELHI-3
 PI STA = 53+58.40
 $\Delta = 26^\circ 00' 03''$ (RT)
 $D = 7^\circ 26' 28''$
 $R = 770.00'$
 $T = 177.77'$
 $L = 349.42'$
 $E = 20.26'$
 $e = -6.0\%$
 $TR IN = N/A$
 $SE RUN IN = SEE INTERSECTION DETAIL$
 $SE RUN OUT = STA 54+86.05 TO STA 56+18.05$
 $TR OUT = 33 FT$
 $PC STA = 51+80.62$
 $PT STA = 55+30.05$

PR CURVE DELHI-4
 PI STA = 63+07.33
 $\Delta = 6^\circ 47' 18''$ (LT)
 $D = 2^\circ 19' 45''$
 $R = 2,460.00'$
 $T = 145.90'$
 $L = 291.46'$
 $E = 4.32'$
 $e = 3.8\%$
 $TR IN = 36 FT$
 $SE RUN IN = STA 61+00.10 TO STA 61+92.10$
 $SE RUN OUT = STA 64+22.22 TO STA 65+14.22$
 $TR OUT = 36 FT$
 $PC STA = 61+61.43$
 $PT STA = 64+52.89$



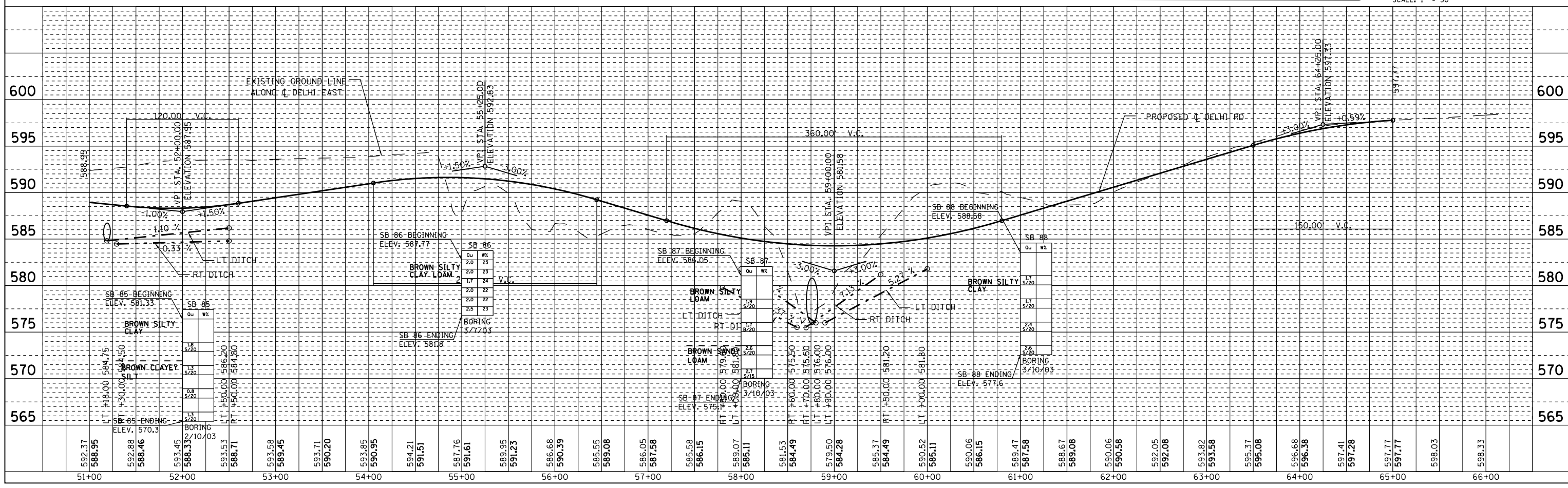
SECTION A-A CHANNEL EXCAVATION
 30' RT DELHI RD STA 68+65 TO
 30' RT STAGECOACH RD SOUTH STA 68+20
 GRADE = -3.0%



PLAN	DATE
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DELHI EAST STA 50+00 TO STA 66+20.86

Appendix B

Boring Logs

Boring Logs

Station 333+19.67 to Station 463+50



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Bypass Mainline LOGGED BY Larry Ford

SECTION 42-2 LOCATION NW 1/4, SEC. 30, TWP. 7N, RNG. 10W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO. _____
 Station _____

BORING NO. SB 33
 Station 347+00
 Offset 40.00ft LT
 Ground Surface Elev. 543.9 ft

D E P T H	B L O W S	U C S Qu	M O I S T T
(ft)	(/6")	(tsf)	(%)

Surface Water Elev. _____ ft
 Stream Bed Elev. _____ ft
 Groundwater Elev.:
 First Encounter DRY ft
 Upon Completion _____ ft
 After _____ Hrs. _____ ft

Brown Silt LOAM (Fill)	3		
	6	1.7	
	7	S/5	9
(Very Dry and Crumbly)	7		
	-5		
	8		
	9	NC	9
	6		
	9	2.7	
	9	S/5	9
	8		
	-10		
	14		
	13	NC	10
----- 532.4 -----			
Brown Clay LOAM	3		
	6	4.4	
	6	S/10	17
	4		
	-15		
	8	3.7	
	12	S/10	18
----- 527.9 -----			
End of Boring			
	-20		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Bypass Mainline LOGGED BY Larry Ford

SECTION 42-2 LOCATION NW 1/4, SEC. 30, TWP. 7N, RNG. 10W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO. Station	DEPTH (ft)	BLOW COUNT (/6")	UNIFORMITY COEFFICIENT (tsf)	MOISTURE CONTENT (%)	Surface Water Elev.	DEPTH (ft)	BLOW COUNT (/6")	UNIFORMITY COEFFICIENT (tsf)	MOISTURE CONTENT (%)
					ft				
BORING NO. <u>SB 34</u> Station <u>349+00</u> Offset <u>10.00ft RT</u> Ground Surface Elev. <u>527.9</u> ft					Stream Bed Elev. <u>517.0</u> ft				
					Groundwater Elev.:				
					First Encounter <u>DRY</u> ft				
					Upon Completion _____ ft				
					After _____ Hrs. _____ ft				
Gray SILT A-6(9) See Classification @ 2.0 feet					Gray and Brown Silty CLAY (continued)		8	4.2	23
							11	S/15	
					-----506.4-----				
		2			Brown and Gray Silty Clay LOAM		3		
		2	0.7	20			5	2.0	24
		2	S/5				7	S/10	
524.4									
Brown and Gray Clay LOAM									
	-5	2					4		
		3	1.3	12		-25	6	2.6	22
		4	S/15				8	S/15	
		2					4		
		2	1.0	23			6	2.4	24
		3	B				8	S/10	
		3					4		
	-10	6	4.0	16		-30	6	2.2	23
		7	S/10				8	S/10	33
516.4					497.4				
Gray and Brown Silty CLAY					Brown Silty CLAY with Organics			1.8	
		4						S/10	
		8	2.9	21	-----495.4-----				
		11	S/10		Gray Clay LOAM				
		5					6		
	-15	10	4.4	22		-35	10	3.8	19
		12	B				14	S/15	
					491.9				
					End of Boring				
		6							
		8	4.4	22					
		13	S/10						
	-20	5				-40			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Bypass Mainline LOGGED BY Mark Schreuder

SECTION 42-2 LOCATION SW 1/4, SEC. 19, TWP. 7N, RNG. 10W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO. Station	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev.	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)
					ft				
BORING NO. <u>SB 35</u> Station <u>351+00</u> Offset <u>56.00ft RT</u> Ground Surface Elev. <u>529.2</u> ft									
Brown Loamy SAND									
		4					5		
		3	0.3	10			9	4.9	15
		3	S/5				11	B	
	524.7								
Brown Clay LOAM									
		3					5		
		5	2.0	18			16	4.9	16
	523.2		S/20		503.7		30	S/20	18
		6			503.2				
Brown LOAM								1.7	
		7						S/5	
		9	2.9	14					
		15	S/20						
		7							
	-10								
	518.7								
		8	2.9	14					
		10	S/20	17					
Gray and Brown Clay LOAM			3.9						
		5	S/20						
		11	5.9	15					
		13	S/15						
		5							
	-15								
		9	5.2	16					
		10	S/15						
		7							
		16	5.5	14					
		22	S/20						
		5							
	-20								

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Bypass Mainline LOGGED BY Larry Ford

SECTION 42-2 LOCATION SW 1/4, SEC. 19, TWP. 7N, RNG. 10W, 3 PM

COUNTY Jersey DRILLING METHOD Hand Auger HAMMER TYPE _____

STRUCT. NO. Station	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev. _____ ft Stream Bed Elev. _____ ft Groundwater Elev.: First Encounter <u>DRY</u> ft Upon Completion _____ ft After _____ Hrs. _____ ft
Brown SILT A-4(8) See Classification @ 0.5 ft	566.9		0.6		
Brown Silty CLAY A-6(14) See Classification @ 3.5 ft			1.5	28	
			3.0		
	563.9		3.5		
Brown Silty Clay LOAM A-6(11) See Classification @ 5.5 ft			3.8	24	
	-5		3.5		
	561.9			16	
End of Hand Auger					
Pocket Penetrometer used for Qu					
	-10				
	-15				
	-20				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Bypass Mainline LOGGED BY Larry Ford

SECTION 42-2 LOCATION SW 1/4, SEC. 19, TWP. 7N, RNG. 10W, 3 PM

COUNTY Jersey DRILLING METHOD Hand Auger HAMMER TYPE _____

STRUCT. NO. _____
 Station _____

BORING NO. SB 39
 Station 362+00
 Offset 36.00ft RT
 Ground Surface Elev. 573.7 ft

D E P T H	B L O W S	U C S Qu	M O I S T
(ft)	(/6")	(tsf)	(%)

Surface Water Elev. _____ ft
 Stream Bed Elev. _____ ft
 Groundwater Elev.:
 First Encounter DRY ft
 Upon Completion _____ ft
 After _____ Hrs. _____ ft

Brown Silty CLAY		2.0	
		2.2	
		1.3	
		1.5	
		1.2	
		1.7	
567.7			
End of Hand Auger			
Pocket Penetrometer used for Qu			
	-10		
	-15		
	-20		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Bypass Mainline LOGGED BY Ford & Maue

SECTION 42-2 LOCATION SW 1/4, SEC. 19, TWP. 7N, RNG. 10W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO. Station	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev. _____ ft Stream Bed Elev. _____ ft
BORING NO. <u>SB 43</u> Station <u>374+00</u> Offset <u>36.00ft RT</u> Ground Surface Elev. <u>581.4</u> ft					Groundwater Elev.: First Encounter <u>DRY</u> ft Upon Completion _____ ft After _____ Hrs. _____ ft
Brown Silty CLAY	3 5 8	1.9 S/15	26		
----- 577.4					
Brown and Gray Silty Clay LOAM	5 7 10	1.7 S/10	21		
	5 6 7	1.6 S/5	17		
----- 572.4					
Brown Clay LOAM	5 7 8	2.0 S/15	15		
----- 570.4					
End of Boring	-15 -20				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Bypass Mainline LOGGED BY Larry Ford

SECTION 42-2 LOCATION SW 1/4, SEC. 19, TWP. 7N, RNG. 10W, 3 PM

COUNTY Jersey DRILLING METHOD Hand Auger HAMMER TYPE _____

STRUCT. NO. Station _____	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev. _____ ft Stream Bed Elev. _____ ft Groundwater Elev.: First Encounter _____ DRY ft Upon Completion _____ ft After _____ Hrs. _____ ft
Gray SILT A-4(8) See Classification @ 0.5 ft	576.6		1.5	21	
Brown Silty CLAY A-7-6(19) See Classification @ 2.5 ft	574.1		3.0	27	
Gray Silty Clay LOAM A-6(10) See Classification @ 4.5 ft	572.1		1.8	22	
End of Hand Auger					
Pocket Penetrometer used for Qu					
	-10				
	-15				
	-20				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Bypass Mainline LOGGED BY Ford / Maue

SECTION 42-2 LOCATION NW 1/4, SEC. 19, TWP. 7N, RNG. 10W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO. Station	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev. _____ ft Stream Bed Elev. _____ ft Groundwater Elev.: First Encounter _____ DRY ft Upon Completion _____ ft After _____ Hrs. _____ ft
Gray Silt LOAM	590.3				
Brown Silty CLAY		3			
		4	2.1		
		6	S/15	25	
	587.3				
Brown and Gray Silty Clay LOAM		4			
		6	1.7		
		7	S/15	25	
	584.8				
Brown and Gray Silty CLAY		4			
		4	1.7		
		5	S/15	22	
	-10	3			
		4	1.9		
	580.3	5	S/15	21	
Brown and Gray Clay LOAM		2			
		3	1.6		
		5	S/10	23	
	-15	2			
		4	1.6		
	575.3	6	S/20	19	
End of Boring					
	-20				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Delhi Bypass Mainline Culvert #2 LOGGED BY Mark Schreder

SECTION 42-2 LOCATION NW 1/4, SEC. 19, TWP. 7N, RNG. 10W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO.	D	B	U	M	Surface Water Elev.	D	B	U	M
Station	E	L	C	O	Stream Bed Elev.	E	L	C	O
BORING NO.	P	O	S	I	Groundwater Elev.:	P	O	S	I
Station	T	W	Qu	S	First Encounter	H	S	Qu	T
Offset	H	S	(tsf)	T	Upon Completion	(ft)	(/6")	(tsf)	(%)
Ground Surface Elev.	(ft)	(/6")	(tsf)	(%)	After _____ Hrs.	(ft)	(/6")	(tsf)	(%)
Brown Silty CLAY					551.5				
		4					21	5.5	9
		6	4.2	17			27	S/10	
		8	S/15						
		7							
	-5	10	3.3	12					
		15	S/15				11		
	569.0								
Brown Sandy LOAM		9							
		12	1.3	11					
		13	S/10						10
	-10	11							
		12	1.2	10					
		13	S/5						
	564.0								
Gray Sandy Clay LOAM		12							
		19	6.6	10					
		25	S/10						
	561.5								
Brown Loamy SAND	-15	30							
		49		15					
		32	NC						
	559.0								
Gray Sandy Clay LOAM		10							
		16	4.9	10					
		23	S/15						
	-20	11							

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Delhi Bypass Mainline Culvert #2 LOGGED BY Mark Schreader

SECTION 42-2 LOCATION NW 1/4, SEC. 19, TWP. 7N, RNG. 10W, 3 PM

COUNTY Jersey DRILLING METHOD Hand Auger HAMMER TYPE _____

STRUCT. NO. _____ Station _____	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev. _____ ft
BORING NO. <u>C-2-3</u> Station <u>401+80</u> Offset <u>127.50ft RT</u> Ground Surface Elev. <u>566.0</u> ft					Stream Bed Elev. _____ ft
Brown Silty Clay LOAM 565.0			1.7	10	Groundwater Elev.: First Encounter <u>DRY</u> ft Upon Completion _____ ft After _____ Hrs. _____ ft
Brown Silt LOAM 564.0			1.9	9	
Brown Sandy LOAM 562.5			2.1	9	
Brown SAND See Gradation @ 5 ft 560.0			NC	8	
	-5		NC	8	
Gray Glacial TILL (Very Sandy) 558.5			4.0	8	
			NC	8	
Gray Clay LOAM 556.0			4.2	12	
			NC	12	
			4.5+	16	
End of Hand Auger -15					
Pocket Penetrometer used for Qu -20					

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Bypass Mainline LOGGED BY Larry Ford

SECTION 42-2 LOCATION NW 1/4, SEC. 19, TWP. 7N, RNG. 10W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE _____

STRUCT. NO. Station	DEPTH (ft)	BLOW COUNT (/6")	UNIFORM SAMPLING Qu	MOISTURE CONTENT (%)	Surface Water Elev.
BORING NO. SB 53 Station 404+00 Offset 36.00ft RT Ground Surface Elev. 590.7 ft					ft
Gray Silt LOAM	589.7				Stream Bed Elev. _____ ft
Brown Silty CLAY	4				Groundwater Elev.: First Encounter <u>DRY</u> ft Upon Completion _____ ft After _____ Hrs. _____ ft
	5	3.1			
	8	S/10	26		
Gray and Brown Silty Clay LOAM	4				
	-5				
	585.2	5	1.8	26	
Brown Silty Clay LOAM A-6(14) See Classification @ 10.5 ft	7	S/10	25		
	583.7		1.5		
			S/15		
Brown and Gray Clay LOAM	3				
	5	1.5			
	6	S/10	23		
End of Boring	4				
	-10				
	579.7	5	2.0	21	
Brown and Gray Clay LOAM	6	S/10	21		
	4				
	5	2.6			
End of Boring	10	S/10	22		
	-15				
	574.7	4			
End of Boring	6	1.8			
	6	S/10	18		
	-20				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Bypass Mainline LOGGED BY Larry Ford

SECTION 42-2 LOCATION NW 1/4, SEC. 19, TWP. 7N, RNG. 10W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO. Station	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev. _____ ft Stream Bed Elev. _____ ft Groundwater Elev.: First Encounter _____ ft Upon Completion _____ ft After _____ Hrs. _____ ft
Brown Silty CLAY	4				
584.7	7	3.1			
	10	S/10	24		
Brown and Gray Silty Clay LOAM	4				
579.7	6	2.2			
	7	S/15	24		
	3				
	5	1.4			
	6	S/15	21		
Brown and Gray Silty CLAY	3				
577.2	5	3.7			
	10	S/10	22		
End of Boring	-15				
	-20				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Bypass Mainline LOGGED BY Brent Maue

SECTION 42-2 LOCATION SW 1/4, SEC. 18, TWP. 7N, RNG. 10W, 3 PM

COUNTY Jersey DRILLING METHOD Hand Auger HAMMER TYPE _____

STRUCT. NO. Station _____	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev. _____ ft
BORING NO. <u>SB 59</u> Station <u>422+00</u> Offset <u>36.00ft RT</u> Ground Surface Elev. <u>587.4</u> ft					Stream Bed Elev. _____ ft
Brown Silty CLAY A-7-6(18) See Classification @ 1.5 ft			2.0	28	Groundwater Elev.: First Encounter _____ ft
			2.5	27	Upon Completion _____ ft
	585.4		4.0	20	After _____ Hrs. _____ ft
Brown Silty Clay LOAM A-6(11) See Classification @ 5.5 ft			4.5+	17	
			4.0	17	
	-5		4.0	19	
	581.4				
End of Hand Auger					
Pocket Penetrometer used for Qu					
	-10				
	-15				
	-20				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Delhi Bypass Mainline Culvert #3 LOGGED BY Mark Schreader

SECTION 42-2 LOCATION SW 1/4, SEC. 18, TWP. 7N, RNG. 10W, 3 PM

COUNTY Jersey DRILLING METHOD Hand Auger HAMMER TYPE _____

STRUCT. NO. Station _____	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev. _____ ft
BORING NO. <u>C-3-3</u> Station <u>423+42</u> Offset <u>122.00ft LT</u> Ground Surface Elev. <u>575.6</u> ft					Stream Bed Elev. _____ ft
			1.5	30	Groundwater Elev.: First Encounter <u>DRY</u> ft Upon Completion _____ ft After _____ Hrs. _____ ft
Brown Silty LOAM			1.0	29	
573.6			1.25	23	
Brown Silty Clay LOAM			1.75	24	
			1.75	23	
	-5		1.5	22	
569.6					
End of Hand Auger					
Pocket Penetrometer used for Qu					
	-10				
	-15				
	-20				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Delhi Bypass Mainline Culvert #3 LOGGED BY Mark Schreder

SECTION 42-2 LOCATION SW 1/4, SEC. 18, TWP. 7N, RNG. 10W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO. Station	D E P T H (ft)	B L O W S (/6")	U C S Qu	M O I S T (%)	Surface Water Elev.	D E P T H (ft)	B L O W S (/6")	U C S Qu	M O I S T (%)
					ft				
BORING NO. <u>C-3-2</u> Station <u>423+67</u> Offset <u>0.00ft CL</u> Ground Surface Elev. <u>580.0</u> ft					Groundwater Elev.:				
					First Encounter <u>566.5</u> ft				
					Upon Completion _____ ft				
					After _____ Hrs. _____ ft				
Brown Silty CLAY					Brown Silty Clay LOAM (continued)		16	5.2	16
		1					24	S/20	
		2	1.2	25			9		
		1	S/15				16	5.9	14
							25	S/20	
575.5									
Brown Silt LOAM	-5	1					9		
		2	0.7	25			33	4.2	16
		1	S/20				28	S/20	
573.0									
Brown Sandy Clay LOAM		2					10		
		4	1.3	19			18	2.9	20
		4	S/20				25	S/15	
					551.5				
					End of Boring				
	-10	0					-30		
569.5		3		22					
Gray SAND with some Gravel See Gradation @ 11 ft		1	NC						
568.0									
Gray-Brown Medium to Fine SAND See Gradation @ 14 ft		7							
		15		14					
		25	NC						
565.0	-15	10					-35		
Brown Silty Clay LOAM		16	6.8	14					
		28	S/20						
		10							
		20	4.9	17					
		22	S/20						
		7							
	-20						-40		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Bypass Mainline LOGGED BY Mark Schreader

SECTION 42-2 LOCATION SW 1/4, SEC. 18, TWP. 7N, RNG. 10W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO. Station	B L O W S	U C S Qu	M O I S T	Surface Water Elev. _____ ft	
BORING NO. SB 60 Station 424+00 Offset 36.00ft LT Ground Surface Elev. 580.6 ft	(ft)	(#/6")	(tsf)	(%)	
Brown Clay LOAM A-6(13) See Classification @ 5.5 ft	3			Stream Bed Elev. _____ ft	
	4	2.0		Groundwater Elev.:	
	5	S/15	18	First Encounter _____ DRY ft	
	4			Upon Completion _____ ft	
	5	1.3		After _____ Hrs. _____ ft	
	7	S/20	16		
	----- 574.1 -----				
	Brown LOAM (Till) A-4(3) See Classification @ 10.5 ft	17			
		44	4.5		
		46	S/10	8	
17					
----- 569.6 -----					
End of Boring	23	4.5			
	27	S/10	7		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Bypass Mainline LOGGED BY Larry Ford

SECTION 42-2 LOCATION SE 1/4, SEC. 13, TWP. 7N, RNG. 11 W, 3 PM

COUNTY Jersey DRILLING METHOD Hand Auger HAMMER TYPE _____

STRUCT. NO. _____ Station _____
 BORING NO. SB 61 Station 426+00
 Offset 36.00ft RT
 Ground Surface Elev. 585.3 ft

Surface Water Elev. _____ ft
 Stream Bed Elev. _____ ft
 Groundwater Elev.:
 First Encounter DRY ft
 Upon Completion _____ ft
 After _____ Hrs. _____ ft

	D E P T H	B L O W S	U C S Qu	M O I S T
	(ft)	(/6")	(tsf)	(%)
			1.5	
Brown Silty CLAY			2.0	
			2.2	
582.3			2.7	
Brown and Gray Clay LOAM			3.0	
	-5		2.2	
579.3				
End of Hand Auger				
Pocket Penetrometer used for Qu				
	-10			
	-15			
	-20			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Bypass Mainline LOGGED BY Ford / Anderson

SECTION 42-2 LOCATION SE 1/4, SEC. 13, TWP. 7N, RNG. 11W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO. _____
 Station _____

BORING NO. SB 62
 Station 429+00
 Offset 36.00ft LT
 Ground Surface Elev. 595.2 ft

D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)
-----------------------------------	------------------------------------	--------------------------------	----------------------------------

Surface Water Elev. _____ ft
 Stream Bed Elev. _____ ft
 Groundwater Elev.:
 First Encounter DRY ft
 Upon Completion _____ ft
 After _____ Hrs. _____ ft

Gray and Brown Silty CLAY	3				
	5	2.0			
	6	S/10	28		
591.2					
Brown and Gray Silty Clay LOAM	-5				
	5	1.4			
	7	S/10	26		
588.7					
Gray and Brown Silty CLAY	3				
	4	1.5			
	5	S/10	21		
585.7					
Brown and Gray Clay LOAM	-10				
	5	2.1			
	6	S/20	20		
584.2					
End of Boring	-15				
	-20				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Bypass Mainline LOGGED BY Larry Ford

SECTION 42-2 LOCATION SE 1/4, SEC. 13, TWP. 7N, RNG. 11W, 3 PM

COUNTY Jersey DRILLING METHOD Hand Auger HAMMER TYPE _____

STRUCT. NO. _____ Station _____	D E P T H B L O W S U C S Q_u M O I S T (ft) (/6") (tsf) (%)	Surface Water Elev. _____ ft
BORING NO. <u>SB 63</u> Station <u>432+00</u> Offset <u>36.00ft RT</u>		Stream Bed Elev. _____ ft
Ground Surface Elev. <u>594.9</u> ft		Groundwater Elev.: _____ ft
		First Encounter <u>DRY</u> ft
		Upon Completion _____ ft
	After _____ Hrs. _____ ft	

Soil Description	Depth (ft)	Blow Count (/6")	UCS (tsf)	MOSIST (%)
Gray Silt LOAM	593.4	1.5	2.0	
			2.5	
Brown Silty CLAY		3.0	2.5	
			2.8	
	589.9	-5	2.6	
Brown and Gray Clay LOAM	588.9			
End of Hand Auger				
Pocket Penetrometer used for Q _u				
	-10			
	-15			
	-20			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Bypass Mainline LOGGED BY Ford / Anderson

SECTION 42-2 LOCATION SE 1/4, SEC. 13, TWP. 7N, RNG. 11W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO. _____ Station _____	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev. _____ ft
BORING NO. <u>SB 64</u> Station <u>434+00</u> Offset <u>36.00ft LT</u> Ground Surface Elev. <u>600.2</u> ft					Stream Bed Elev. _____ ft
Brown Silty CLAY 596.2		4			Groundwater Elev.: First Encounter <u>DRY</u> ft Upon Completion _____ ft After _____ Hrs. _____ ft
		6	4.4		
		8	S/10	22	
Brown Silty Clay LOAM 589.7	-5	6			
		6	1.7		
		5	S/10	24	
		2			
		3	1.0		
		4	B	23	
Brown Clay LOAM 584.2	-10	3			
		3	0.9	22	
		3	S/20	22	
		4	1.1		
			S/15		
		4	1.8		
End of Boring -20		6	S/20	20	
		4			
		6	1.8		
		8	S/10	20	

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Bypass Mainline LOGGED BY Ford / Anderson

SECTION 42-2 LOCATION SE 1/4, SEC. 13, TWP. 7N, RNG. 11W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO. _____
Station _____

BORING NO. SB 65
Station 435+84
Offset 41.00ft RT
Ground Surface Elev. 582.0 ft

D E P T H	B L O W S	U	M
		C S Qu	O I S T
(ft)	(/6")	(tsf)	(%)

Surface Water Elev. _____ ft
Stream Bed Elev. _____ ft
Groundwater Elev.:
First Encounter DRY ft
Upon Completion _____ ft
After _____ Hrs. _____ ft

Brown Clay LOAM	-			
	-			
	4			
	5	5.8		
	9	S/5	14	
	-			
	5			
	9	5.0		
	13	S/5	13	
	-			
	9			
	11	3.3		
	20	S/5	12	
	573.0			
Brown Silt LOAM	-10	5		
	-	7		
	9	NC	6	
	571.0			
End of Boring	-			
	-			
	-15			
	-			
	-			
	-			
	-			
	-			
	-			
	-20			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Delhi Bypass Mainline Box Culvert LOGGED BY Mark Schreader

SECTION 42-2 LOCATION SE 1/4, SEC. 13, TWP. 7N, RNG. 11W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO. Station _____	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev. _____ ft
BORING NO. <u>BXC-2-1</u> Station <u>436+95</u> Offset <u>145.00ft LT</u> Ground Surface Elev. <u>552.0</u> ft					Stream Bed Elev. _____ ft
Brown Sandy LOAM See Gradation @ 2 ft		3			Groundwater Elev.: First Encounter _____ ft Upon Completion _____ ft After _____ Hrs. _____ ft
	549.0	5	NC	6	
Brown Silty Clay LOAM		7	1.3	9	
			S/10		
		6			
	-5	12	4.2	15	
		12	S/10		
		4			
End of Boring		9	4.6	13	
		13	S/10		
	-10	5			
	541.0	9	3.1	16	
	13	S/20			
	-15				
	-20				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Delhi Bypass Mainline Box Culvert LOGGED BY Mark Schreader

SECTION 42-2 LOCATION SE 1/4, SEC. 13, TWP. 7N, RNG. 11W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO. _____
 Station _____

BORING NO. BXC-2-2
 Station 437+51
 Offset 96.30ft LT
 Ground Surface Elev. 555.4 ft

D E P T H (ft)	B L O W S (/6")	U C S (tsf)	M O I S T (%)
-----------------------------------	------------------------------------	--------------------------	----------------------------------

Surface Water Elev. _____ ft	D	B	U	M
Stream Bed Elev. _____ ft	E	L	C	O
Groundwater Elev.:	P	O	S	I
First Encounter _____ ft	T	W	S	S
Upon Completion _____ ft	H	S	Qu	T
After _____ Hrs. _____ ft	(ft)	(/6")	(tsf)	(%)

Soil Description	D E P T H (ft)	B L O W S (/6")	U C S (tsf)	M O I S T (%)	Soil Description	D E P T H (ft)	B L O W S (/6")	U C S (tsf)	M O I S T (%)
Brown Silty Clay LOAM	3				Gray Silty CLAY (continued)	13		3.9	17
	4		1.6	17		17		S/20	
Brown Sandy LOAM See Gradation @ 3.5 ft	5		S/20	12	End of Boring				
			1.5						
			S/10						
Brown Silty CLAY	-5	3				-25			
		5	3.3	15					
		7	S/5						
		5							
		11	6.8	14					
		15	S/15						
	-10	6				-30			
		13	6.7	11					
		15	S/20						
	543.9								
Brown Sandy LOAM See Gradation @ 15 ft		8							
		13		14					
		18	NC						
	-15	6				-35			
		9		17					
		12	NC						
		6							
	537.4	10	NC	16					
Gray Silty CLAY		12	3.4	17					
			S/20						
	-20	5				-40			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Delhi Bypass Mainline Box Culvert LOGGED BY Mark Schreader

SECTION 42-2 LOCATION SE 1/4, SEC. 13, TWP. 7N, RNG. 11W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO. Station	DEPTH (ft)	BLOW COUNT (/6")	UCS Qu (tsf)	MOISTURE S (%)	Surface Water Elev.	DEPTH (ft)	BLOW COUNT (/6")	UCS Qu (tsf)	MOISTURE S (%)
					ft				
BORING NO. <u>BXC-2-3</u> Station <u>438+08</u> Offset <u>5.00ft LT</u> Ground Surface Elev. <u>553.5</u> ft									
					Gray Silty CLAY (continued)		9	2.3	17
					532.5		13	S/20	
Brown Sandy LOAM with Gravel See Gradation @ 2.5 ft					End of Boring				
	5								
	7	1.5	11						
	7	S/20							
549.0									
	-5	5							
Brown Medium to Fine SAND See Gradation @ 6 ft									
		11	7						
		14	NC						
546.5									
Gray Silty CLAY		5							
		8	4.6	16					
		9	S/20						
	-10	5							
		9	4.2	16					
		3	S/15						
		9							
		12	4.2	15					
		14	S/20						
	-15	4							
Gravel		10	2.9	14					
		10	S/20						
		5							
		11	4.0	13					
		13	S/20						
	-20	5							

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Delhi Bypass Mainline Box Culvert LOGGED BY Mark Schreder

SECTION 42-2 LOCATION SE 1/4, SEC. 13, TWP. 7N, RNG. 11W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO. Station	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev.	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)
					ft				
BORING NO. <u>BXC-2-4</u> Station <u>438+20</u> Offset <u>82.00ft RT</u> Ground Surface Elev. <u>555.5</u> ft					Groundwater Elev.:				
					First Encounter <u>DRY</u> ft				
					Upon Completion _____ ft				
					After _____ Hrs. _____ ft				
Brown SAND with some Gravel See Gradation @ 2.5 ft	552.0	6 6 7	NC	7	Brown Silty Clay TILL (<i>continued</i>)	534.5	7 11	2.9 S/20	17
Brown Silty Clay TILL	-5	5 6 10	3.9 S/20	14	End of Boring				
		6							
		11 13	4.4 S/10	14					
	-10	4 7 12	4.5 S/15	15					
Gravel		6 8 9	3.6 S/20	18					
	-15	3 8 10	3.3 S/20	17					
		5 8 19	3.3 S/20	17					
	-20	4							

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Delhi Bypass Mainline Box Culvert LOGGED BY Mark Schreder

SECTION 42-2 LOCATION SE 1/4, SEC. 13, TWP. 7N, RNG. 11W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO. Station	B L O W S	U C S Qu	M O I S T T	Surface Water Elev. _____ ft	
BORING NO. <u>BXC-2-5</u> Station <u>438+80</u> Offset <u>123.00ft RT.</u> Ground Surface Elev. <u>554.5</u> ft	(ft)	(/6")	(tsf)	(%)	Stream Bed Elev. _____ ft
					Groundwater Elev.:
					First Encounter _____ DRY ft
					Upon Completion _____ ft
					After _____ Hrs. _____ ft
Brown Silty Clay TILL					
		5			
		8	4.8	13	
		8	S/15		
		6			
		9	3.7	11	
		14	S/5		
		4			
		10	5.9	15	
	11	S/10			
	3				
	8	3.4	16		
	11	S/20			
543.5					
End of Boring					

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
 BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Bypass Mainline LOGGED BY Brent Maue

SECTION 42-2 LOCATION SE 1/4, SEC. 13, TWP. 7N, RNG. 11W, 3 PM

COUNTY Jersey DRILLING METHOD Hand Auger HAMMER TYPE _____

STRUCT. NO. Station _____	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev. _____ ft
BORING NO. <u>SB 67</u> Station <u>440+00</u> Offset <u>36.00ft RT</u> Ground Surface Elev. <u>587.5</u> ft					Stream Bed Elev. _____ ft
Brown CLAY A-6(12) See Classification @ 1.5 ft	—	—	1.0	28	Groundwater Elev.: First Encounter <u>DRY</u> ft Upon Completion _____ ft After _____ Hrs. _____ ft
	—	—	1.6	27	
	—	—	4.3	18	
	584.5				
Brown Silty Clay LOAM A-6(12) See Classification @ 5.5 ft	—	—	4.1	16	
	—	—	4.5+	10	
	—	—	4.5+	14	
	581.5				
End of Hand Auger	—	—	—	—	
Pocket Penetrometer used for Qu	—	—	—	—	
	—	—	—	—	
	-10				
	—	—	—	—	
	-15				
	—	—	—	—	
	—	—	—	—	
	-20				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Bypass Mainline LOGGED BY Brent Maue

SECTION 42-2 LOCATION SE 1/4, SEC. 13, TWP. 7N, RNG. 11W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO. Station	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev. _____ ft Stream Bed Elev. _____ ft Groundwater Elev.: First Encounter <u>DRY</u> ft Upon Completion _____ ft After _____ Hrs. _____ ft
Brown Silty CLAY					
		2			
		4	2.2		
		6	S/15		
		-5			
Gray Silt LOAM		3			
		3	1.6		
		4	B		
					599.1
Gray Clay LOAM (Till)		4			
		5	2.6		
		6	S/15		
					596.6
Gray Clay LOAM (Till)		-10	3		
		4	2.0		
		5	B		
		3			
	3	2.3			
	6	S/10			
					-15
	3				
	3	1.9			
					590.1
Brown Sandy CLAY		4	S/15		
					589.6
End of Boring			0.9 E		
					-20

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Bypass Mainline LOGGED BY Brent Maue

SECTION 42-2 LOCATION SE 1/4, SEC. 13, TWP. 7N, RNG. 11W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO. _____
 Station _____
 BORING NO. SB 70
 Station 449+00
 Offset 36.00ft LT
 Ground Surface Elev. 609.2 ft

D E P T H	B L O W S	U C S Qu	M O I S T (%)
(ft)	(/6")	(tsf)	

Surface Water Elev. _____ ft
 Stream Bed Elev. _____ ft
 Groundwater Elev.:
 First Encounter DRY ft
 Upon Completion _____ ft
 After _____ Hrs. _____ ft

Brown Silty CLAY	3			
	4	2.0		
	5	S/10		
<hr/>				
Brown and Gray Silty Clay LOAM	3			
	4	2.1		
	4	S/20		
<hr/>				
Brown and Gray Clay LOAM (Till)	3			
	4	2.2		
	4	B		
<hr/>				
	4			
	5	3.1		
	7	S/15		
<hr/>				
	4			
	6	2.9		
	9	S/10		
<hr/>				
End of Boring				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Bypass Mainline LOGGED BY Mark Schreader

SECTION 42-2 LOCATION NW 1/4, SEC. 13, TWP. 7N, RNG. 11W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO. _____
 Station _____

BORING NO. SB 71
 Station 452+00
 Offset 36.00ft RT
 Ground Surface Elev. 614.1 ft

D E P T H	B L O W S	U C S Qu	M O I S T
(ft)	(/6")	(tsf)	(%)

Surface Water Elev. _____ ft
 Stream Bed Elev. _____ ft
 Groundwater Elev.:
 First Encounter 595.6 ft
 Upon Completion _____ ft
 After _____ Hrs. _____ ft

D E P T H	B L O W S	U C S Qu	M O I S T
(ft)	(/6")	(tsf)	(%)

Brown Silty CLAY	—			Brown Clay LOAM (Till) (continued)	45	4.5	
	—			593.1	50	S/5	
	2			End of Boring			
Brown Silty LOAM	4	2.2					
	6	S/20					
	—						
	-5	3			-25		
Brown Silty LOAM	3	1.5					
	3	S/20					
	—						
	607.1	3					
Brown Silty LOAM	3	1.1					
	4	S/20					
	—						
	-10	3			-30		
Brown Sandy LOAM	4	1.6					
	5	S/20					
	—						
	602.1	4					
Brown Sandy LOAM	5	1.9					
	7	S/15					
	—						
	-15	4			-35		
Brown Sandy LOAM	4	1.4					
	5	S/15					
	—						
	597.1	3					
Brown Loamy SAND	9	1.5					
	15	P					
Brown Clay LOAM (Till)	—						
	594.6	23			-40		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Bypass Mainline LOGGED BY Mark Schreuder

SECTION 42-2 LOCATION NW 1/4, SEC. 13, TWP. 7N, RNG. 11W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO. Station	DEPTH (ft)	BLOW COUNT (/6")	UCS (tsf)	MOISTURE (%)	Surface Water Elev. _____ ft	DEPTH (ft)	BLOW COUNT (/6")	UCS (tsf)	MOISTURE (%)
BORING NO. <u>SB 72</u> Station <u>454+00</u> Offset <u>2.00ft RT</u> Ground Surface Elev. <u>620.0</u> ft					Stream Bed Elev. _____ ft				
					Groundwater Elev.: First Encounter <u>DRY</u> ft Upon Completion _____ ft After _____ Hrs. _____ ft				
Brown Silty Clay LOAM A-7-6(15) See Classification @ 5.5 ft					Brown Sandy LOAM (Till) (continued) A-6(3) See Classification @ 28.0 ft		23	4.5	
		3					33	P	10
		5	1.8				19		
		5	S/20	27			31	4.5	
							47	P	10
		3					12		
		4	1.6				18	4.5	
		5	S/20	26			23	P	16
	613.0								
Brown Loamy SILT		4					9		
		4	1.8				11	2.2	
		6	S/20	21		591.5	15	S/20	14
		3			End of Boring				
		5	1.8				-30		
		5	S/20	20					
	608.0								
Brown Sandy Clay LOAM A-6(4) See Classification @ 18.0 ft		3							
		6	2.6						
		8	S/20	22					
		5					-35		
		6	0.7						
		6	S/10	19					
	603.0								
Brown Loamy SAND		1							
		2							
		3	NC	26					
	600.5								
Brown Sandy LOAM (Till)		10					-40		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Bypass Mainline LOGGED BY Mark Schreder

SECTION 42-2 LOCATION NW 1/4, SEC. 13, TWP. 7N, RNG. 11W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO. _____
 Station _____

BORING NO. SB 73
 Station 456+00
 Offset 26.00ft LT
 Ground Surface Elev. 616.0 ft

D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)
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Surface Water Elev. _____ ft
 Stream Bed Elev. _____ ft
 Groundwater Elev.:
 First Encounter _____ DRY ft
 Upon Completion _____ ft
 After _____ Hrs. _____ ft

D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)
-------------------------------	--------------------------------	----------------------------	------------------------------

Brown Silty Clay LOAM				Brown Clay LOAM (Till) (continued)			
				595.0	15	4.5	
				End of Boring			
	3						
	4	1.1					
	3	S/20					
	-5	3			-25		
		4	1.4				
		5	S/20				
				609.0			
Brown Clay LOAM (Till)	3						
	5	2.4					
	6	S/20					
	-10	4			-30		
		6	2.2				
		9	S/15				
		2					
		5	1.5				
		8	S/20				
	-15	10			-35		
		14	4.5				
		23	S/10				
		11					
		21	4.5				
		30	S/10				
	-20	8			-40		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



**Illinois Department
of Transportation**

Division of Highways
Illinois Department of Transportation

SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Bypass Mainline LOGGED BY Mark Schreader

SECTION 42-2 LOCATION NW 1/4, SEC. 13, TWP. 7N, RNG. 11W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO. _____
Station _____

BORING NO. SB 74
Station 458+00
Offset 31.00ft RT
Ground Surface Elev. 615.0 ft

D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)
-------------------------------	--------------------------------	----------------------------	------------------------------

Surface Water Elev. _____ ft
Stream Bed Elev. _____ ft
Groundwater Elev.:
First Encounter _____ ft
Upon Completion _____ ft
After _____ Hrs. _____ ft

Brown Clay LOAM			
	3		
	4	1.8	
	6	S/10	
	5		
	4	2.0	
	5	P	
	2		
	1	0.6	
	3	S/20	
	3		
	6	1.9	
	7	S/20	
	4		
	5	2.5	
	7	S/10	
	3		
	3	3.6	
	10	S/20	
	3		
	16	4.5	
	20	S/15	
End of Boring			

596.5

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



Illinois Department of Transportation

Division of Highways
Illinois Department of Transportation

SOIL BORING LOG

Date 1/15/02

ROUTE FAP 310 DESCRIPTION US 67 over Unnamed Drainage Ditch LOGGED BY M. Tappan

SECTION 42-2 LOCATION NW 1/4, SEC. 13, TWP. 7N, RNG. 11W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO. Station _____	DEPTH (ft)	BLOW COUNT (/6")	UCS Qu (tsf)	MOIST CONTENT (%)	Surface Water Elev.	DEPTH	BLOW	UCS	MOIST
					ft	(ft)	(/6")	(tsf)	(%)
BORING NO. <u>BX 10</u>					<u>590.8</u>				
Station <u>460+95</u>					<u>N/A</u>				
Offset <u>112.00ft LT</u>					Groundwater Elev.:				
Ground Surface Elev. <u>593.9</u> ft					First Encounter <u>DRY</u> ft				
					Upon Completion _____ ft				
					After <u>-</u> Hrs. <u>Plugged</u> ft				
Gray and Brown Moist Clay LOAM					Gray Dry Fissile Clayey SHALE				
					<i>(continued)</i>				
		1					9		
		2	0.6	15			53	10.0+	10
		3	B			568.9	47/3"	E	
588.4									
Gray and Yellowish Brown Moist Very Weathered Clayey SHALE					End of Boring				
		2							
		7	4.0	15					
586.4		3	P						
Brown and Dark Gray to Black Moist Weathered Clayey SHALE									
		3							
		10	4.5+	20					
583.9	-10	12	P			-30			
Brown and Dark Gray Moist Clayey SHALE (Iron Oxidized)									
		1							
		10	4.5+	16					
581.4		11	P						
Gray Dry Fissile Clayey SHALE									
		7							
		16	8.0	12					
	-15	25	E			-35			
		13							
		55	10.0+	10					
	-20	46/4"	E			-40			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



Illinois Department of Transportation

Division of Highways
Illinois Department of Transportation

SOIL BORING LOG

Date 1/15/02

ROUTE FAP 310 DESCRIPTION US 67 over Unnamed Drainage Ditch LOGGED BY M. Tappan

SECTION 42-2 LOCATION NW 1/4, SEC. 13, TWP. 7N, RNG. 11W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

Table with columns: STRUCT. NO., BORING NO., Station, Offset, Ground Surface Elev., DEPTH (ft), BLOW S ((/6")), UCS Qu (tsf), MOIST (%) and corresponding soil descriptions and elevations.

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Bypass Mainline LOGGED BY Mark Schreuder

SECTION 42-2 LOCATION NW 1/4, SEC. 13, TWP. 7N, RNG. 11W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO. _____
 Station _____
 BORING NO. SB 76
 Station 467+00
 Offset 36.00ft LT
 Ground Surface Elev. 618.92 ft

D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)

Surface Water Elev. _____ ft
 Stream Bed Elev. _____ ft
 Groundwater Elev.:
 First Encounter DRY ft
 Upon Completion _____ ft
 After _____ Hrs. _____ ft

Brown Silty CLAY	3				
	3	1.7			
	4	P			
	4				
	-5	4			
	4	1.3			
	5	S/20			
611.9	4				
Brown Sandy CLAY	5	1.9			
610.4	8	S/20			
End of Boring					
	-10				
	-15				
	-20				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
 BBS, from 137 (Rev. 8-99)

Boring Logs

Service Road



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on Service Road LOGGED BY Mark Schreder

SECTION 42-2 LOCATION NW 1/4, SEC. 30, TWP. 7N, RNG. 10W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO. _____ Station _____	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev. _____ ft
BORING NO. <u>SB 77</u> Station <u>101+75</u> Offset <u>57.00ft RT</u> Ground Surface Elev. <u>554.55</u> ft					Stream Bed Elev. _____ ft
					Groundwater Elev.:
					First Encounter <u>DRY</u> ft
					Upon Completion _____ ft
					After _____ Hrs. _____ ft

Brown Silty Clay LOAM A-6(11) See Classification @ 2.5 ft	-				
	2				
	3	1.5	18		
	3	P			
550.1					
	-5	2			
Brown Silty Clay LOAM (Till) A-6(10) See Classification @ 10.0 ft	3	1.3	21		
	5	S/20			
	6				
	7	1.3	13		
	10	S/20			
	15				
-10	22		-		
	31	NS			
	10				
541.1	39	4.5	11		
	31	P			
End of Boring	-15				
	-20				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

Boring Logs
US 67 Connector



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on US 67 Connector LOGGED BY Mark Schreader

SECTION 42-2 LOCATION SW 1/4, SEC. 19, TWP. 7N, RNG. 10W, 3 PM

COUNTY Jersey DRILLING METHOD Hand Auger HAMMER TYPE _____

STRUCT. NO. _____ Station _____	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev. _____ ft
					Stream Bed Elev. _____ ft
BORING NO. <u>SB 78</u> Station <u>24+50</u> Offset <u>0.00ft CL</u> Ground Surface Elev. <u>565.8</u> ft					Groundwater Elev.: First Encounter _____ DRY ft Upon Completion _____ ft After _____ Hrs. _____ ft
Brown Silty LOAM	—		0.6		
	—		0.6		
	—		1.0		
	—		1.2		
561.8					
Brown Sandy LOAM	—		1.5		
	-5		1.5		
559.8					
End of Hand Auger	—				
Pocket Penetrometer used for Qu	—				
	—				
	—				
	—				
	-10				
	—				
	—				
	—				
	—				
	-15				
	—				
	—				
	—				
	—				
	—				
	-20				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on US 67 Connector LOGGED BY Mark Schreuder

SECTION 42-2 LOCATION SW 1/4, SEC. 19, TWP. 7N, RNG. 10W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO. _____
Station _____

BORING NO. SB 79
Station 26+65
Offset 25.00ft LT
Ground Surface Elev. 552.1 ft

D E P T H
 B L O W S
 U C S
 M O I S T
 (ft) (/#") (tsf) (%)

Surface Water Elev. _____ ft
 Stream Bed Elev. _____ ft
 Groundwater Elev.:
 First Encounter 541.6 ft ▼
 Upon Completion _____ ft
 After _____ Hrs. _____ ft

Gray Silty Clay LOAM	0				
	1	NC	27		
	1				
547.6					
	-5	2			
Gray Clay LOAM	3	1.1	21		
	4	S/20			
	1				
544.1	2	0.5	19		
	3	S/10	21		
Gray Loamy SAND		NC			
	-10	18			
541.6 ▼	16	NC	17		
	27	4.0	12		
Gray Clay LOAM		P			
	6				
	18	7.5	12		
	25	S/20			
	-15	13			
536.1	20	4.4	12		
	21	S/10			
End of Boring					
	-20				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on US 67 Connector LOGGED BY Mark Schreader

SECTION 42-2 LOCATION NW 1/4, SEC. 19, TWP. 7N, RNG. 10W, 3 PM

COUNTY Jersey DRILLING METHOD Hand Auger HAMMER TYPE _____

STRUCT. NO. _____
 Station _____
 BORING NO. SB 80
 Station 28+50
 Offset 0.00ft CL
 Ground Surface Elev. 563.2 ft

D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)
-------------------------------	--------------------------------	--------------------------------	----------------------------------

Surface Water Elev. _____ ft
 Stream Bed Elev. _____ ft
 Groundwater Elev.:
 First Encounter DRY ft
 Upon Completion _____ ft
 After _____ Hrs. _____ ft

Brown Silty LOAM	560.2		2.5	
			2.0	
			3.0	
Brown Silty Clay LOAM			3.5	
			2.5	
	557.2		3.0	
End of Hand Auger				
Pocket Penetrometer used for Qu				
	-10			
	-15			
	-20			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

Boring Logs

Delhi Road



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Road LOGGED BY Mark Schreader

SECTION 42-2 LOCATION NW 1/4, SEC. 19, TWP. 7N, RNG. 10W, 3 PM

COUNTY Jersey DRILLING METHOD Hand Auger HAMMER TYPE _____

STRUCT. NO. _____
Station _____

BORING NO. SB 81
Station 40+24
Offset 10.00ft RT
Ground Surface Elev. 588.6 ft

D
E
P
T
H

B
L
O
W
S

U
C
S

Qu

M
O
I
S
T

(ft) (/6") (tsf) (%)

Surface Water Elev. _____ ft
Stream Bed Elev. _____ ft
Groundwater Elev.:
First Encounter DRY ft
Upon Completion _____ ft
After _____ Hrs. _____ ft

Brown Silty LOAM	587.6		1.5	
Brown Silty CLAY			2.5	
			2.0	
			1.7	
			1.5	
		-5	1.7	
	582.6			
End of Hand Auger				
Pocket Penetrometer used for Qu				
		-10		
		-15		
		-20		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Road LOGGED BY Mark Schreader

SECTION 42-2 LOCATION NW 1/4, SEC. 19, TWP. 7N, RNG. 10W, 3 PM

COUNTY Jersey DRILLING METHOD Hand Auger HAMMER TYPE _____

STRUCT. NO. Station	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev.
					ft
BORING NO. <u>SB 82</u> Station <u>43+00</u> Offset <u>48.00ft RT</u> Ground Surface Elev. <u>589.5</u> ft					Stream Bed Elev. _____ ft
			0.7		Groundwater Elev.: _____ ft
Brown Silt LOAM					First Encounter <u>DRY</u> ft
588.5			2.0		Upon Completion _____ ft
Brown Silty CLAY			2.2		After _____ Hrs. _____ ft
			2.0		
			1.5		
	-5		1.2		
583.5					
End of Hand Auger					
Pocket Penetrometer used for Qu					
	-10				
	-15				
	-20				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



Illinois Department of Transportation

Division of Highways
Illinois Department of Transportation

SOIL BORING LOG

Date 2/10/03

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Road LOGGED BY Mark Schreader

SECTION 42-2 LOCATION NW 1/4, SEC. 19, TWP. 7N, RNG. 10W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO. _____
Station _____

BORING NO. SB 84
Station 49+00
Offset 18.00ft RT
Ground Surface Elev. 585.8 ft

D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)
-----------------------------------	------------------------------------	--------------------------------	----------------------------------

Surface Water Elev. _____ ft
Stream Bed Elev. _____ ft

Groundwater Elev.:
First Encounter DRY ft
Upon Completion _____ ft
After _____ Hrs. _____ ft

Brown Loamy SILT	3			
	4	1.6		
	5	S/20		
	3			
	4	1.6		
	5	S/20		
	3			
	3	1.3		
	5	S/20		
	4			
	4	1.7		
	6	S/15		
End of Boring				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, from 137 (Rev. 8-99)



Illinois Department of Transportation

Division of Highways
Illinois Department of Transportation

SOIL BORING LOG

Date 3/7/03

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Road LOGGED BY Mark Schreader

SECTION 42-2 LOCATION SW 1/4, SEC. 18, TWP. 7N, RNG. 10W, 3 PM

COUNTY Jersey DRILLING METHOD Hand Auger HAMMER TYPE _____

STRUCT. NO. Station _____	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev. _____ ft
BORING NO. <u>SB 86</u> Station <u>55+00</u> Offset <u>80.00ft LT</u> Ground Surface Elev. <u>587.8</u> ft					Stream Bed Elev. _____ ft
Brown Silty Clay LOAM A-6(11) See Classification @ 2.5 ft			2.0	23	Groundwater Elev.: First Encounter <u>DRY</u> ft Upon Completion _____ ft After _____ Hrs. _____ ft
			2.0	23	
			1.7	24	
			2.0	22	
			2.0	22	
		-5		2.5	
581.8					
End of Hand Auger					
Pocket Penetrometer used for Qu					
	-10				
	-15				
	-20				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



Illinois Department of Transportation

Division of Highways
Illinois Department of Transportation

SOIL BORING LOG

Date 3/10/03

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Road LOGGED BY Mark Schreder

SECTION 42-2 LOCATION SW 1/4, SEC. 18, TWP. 7N, RNG. 10W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO. Station	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev.
					ft
BORING NO. SB 87 Station 58+00 Offset 70.00ft LT Ground Surface Elev. 586.1 ft					Stream Bed Elev. _____ ft
					Groundwater Elev.:
					First Encounter _____ ft
					Upon Completion _____ ft
					After _____ Hrs. _____ ft
Brown Silty LOAM					
	4				
	5	1.9			
	6	S/20			
	-5	3			
	5	1.7			
	5	B/20			
	579.1				
Brown Sandy LOAM		3			
	6	2.6			
	8	S/20			
	-10	4			
	5	2.7			
	8	S/15			
	575.1				
End of Boring					
	-15				
	-20				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

ROUTE FAP 310 DESCRIPTION Soil Survey on Delhi Road LOGGED BY Mark Schreader

SECTION 42-2 LOCATION SW 1/4, SEC. 18, TWP. 7N, RNG. 10W, 3 PM

COUNTY Jersey DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO. Station	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev.	
					ft	
					Stream Bed Elev.	ft
BORING NO. <u>SB 88</u> Station <u>61+00</u> Offset <u>63.00ft LT</u> Ground Surface Elev. <u>588.6</u> ft					Groundwater Elev.:	
					First Encounter	<u>DRY</u> ft
					Upon Completion	ft
					After ____ Hrs.	ft
Brown Silty CLAY		3				
		5	1.7			
		5	S/20			
		5				
		6	1.7			
		7	S/20			
		4				
577.6		6	2.4			
		7	S/20			
		4				
-10		5	2.6			
		6	S/20			
End of Boring						
		-15				
	-20					

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

Appendix C

Soil Test Data Sheets

Soil Test Data Sheets

Station 333+19.67 to Station 463+50

STATE OF ILLINOIS
Department of Transportation

SOIL TEST DATA

STATE JOB NUMBER D-98-034-00 ROUTE FAP 310 PROJECT US 67 (Delhi Bypass)
SECTION 42-2 COUNTY Jersey

BORING NO.		SB 31	SB 31	SB 31	SB 34	SB 36
LAB. NO.						
STATION		342+79	342+79	342+79	349+00	353+00
LOCATION		36 ft LT	36 ft LT	36 ft LT	10 ft RT	36 ft LT
DEPTH		0 - 5 ft	5 - 9 ft	9 - 21 ft	0 - 3.5 ft	0 - 1 ft
HRB CLASSIFICATION & GROUP INDEX		A-7-6(17)	A-6(10)	A-6(11)	A-6(9)	A-4(8)
GRAIN SIZE CLASSIFICATION		Silt	Silt	Silty Loam	Silt	Silt
GRADUATION-PASSING 1" SIEVE	%	100.00	100.00	100.00	100.00	100.00
"	3/4" %	100.00	100.00	100.00	100.00	100.00
"	1/2" %	100.00	100.00	99.17	100.00	100.00
"	NO. 4 %	100.00	100.00	94.58	99.88	100.00
"	NO. 10 %	100.00	99.93	90.32	98.78	100.00
"	NO. 40 %	100.00	99.93	89.92	98.31	99.92
"	NO. 100 %	99.98	99.25	88.65	96.25	99.09
"	NO. 200 %	99.95	98.98	88.13	95.55	98.46
SAND	%	0.1	1.0	6.4	4.3	1.5
SILT	%	86.5	89.1	77.3	82.8	82.1
CLAY	%	13.5	9.9	10.9	12.7	16.3
LIQUID LIMIT	%	45	29	29	27	27
PLASTICITY INDEX	%	30	15	18	13	6
BEARING RATIO	%					
STANDARD DRY DENSITY AASHTO T99	%					
OPTIMUM MOISTURE	%					
REMARKS:				5.4% Gravel in Sample	0.1% Gravel in Sample	

STATE OF ILLINOIS
Department of Transportation

SOIL TEST DATA

STATE JOB NUMBER D-98-034-00 ROUTE FAP 310 PROJECT US 67 (Delhi Bypass)
SECTION 42-2 COUNTY Jersey

BORING NO.		SB 36	SB 36	SB 37	SB 44	SB 44
LAB. NO.						
STATION		353+00	353+00	356+00	377+00	377+00
LOCATION		36 ft LT	36 ft LT	36 ft RT	36 ft LT	36 ft LT
DEPTH		1 - 4 ft	4 - 6 ft	3 - 8 ft	0 - 1.5 ft	1.5 - 4 ft
HRB CLASSIFICATION & GROUP INDEX		A-6(14)	A-6(11)	A-6(9)	A-4(8)	A-7-6(19)
GRAIN SIZE CLASSIFICATION		Silty Clay Loam	Silty Clay Loam	Clay Loam	Silt	Silty Clay
GRADUATION-PASSING 1" SIEVE	%	100.00	100.00	100.00	100.00	100.00
"	3/4" %	100.00	100.00	100.00	100.00	100.00
"	1/2" %	100.00	100.00	100.00	100.00	100.00
"	NO. 4 %	100.00	100.00	99.06	100.00	100.00
"	NO. 10 %	100.00	100.00	93.07	99.94	100.00
"	NO. 40 %	99.90	99.97	84.91	99.50	99.94
"	NO. 100 %	99.67	99.65	63.83	98.63	98.59
"	NO. 200 %	99.47	99.43	55.59	98.20	97.68
SAND	%	0.5	0.6	43.2	1.8	2.3
SILT	%	69.7	77.6	34.4	85.7	57.7
CLAY	%	29.8	21.8	21.5	12.5	40.0
LIQUID LIMIT	%	40	31	35	24	54
PLASTICITY INDEX	%	24	17	23	4	37
BEARING RATIO	%					
STANDARD DRY DENSITY AASHTO T99	%					
OPTIMUM MOISTURE	%					
REMARKS:				0.9% Gravel in Sample		

STATE OF ILLINOIS
Department of Transportation

SOIL TEST DATA

STATE JOB NUMBER D-98-034-00 ROUTE FAP 310 PROJECT US 67 (Delhi Bypass)
SECTION 42-2 COUNTY Jersey

BORING NO.		SB 44	SB 51	SB 51	SB 51	SB 53
LAB. NO.						
STATION		377+00	398+00	398+00	398+00	404+00
LOCATION		36 ft LT	36 ft RT	36 ft RT	36 ft RT	36 ft RT
DEPTH		4 - 6 ft	0 - 1 ft	1 - 4 ft	4 - 6.5 ft	7 - 11 ft
HRB CLASSIFICATION & GROUP INDEX		A-6(10)	A-4(8)	A-7-6(18)	A-6(11)	A-6(14)
GRAIN SIZE CLASSIFICATION		Silty Clay Loam	Silt	Silty Clay	Silty Clay Loam	Silty Clay Loam
GRADUATION-PASSING 1" SIEVE	%	100.00	100.00	100.00	100.00	100.00
"	3/4"	%	100.00	100.00	100.00	100.00
"	1/2"	%	100.00	100.00	100.00	100.00
"	NO. 4	%	100.00	100.00	100.00	100.00
"	NO. 10	%	100.00	100.00	100.00	99.76
"	NO. 40	%	99.98	99.30	99.97	98.80
"	NO. 100	%	99.95	99.16	99.76	94.74
"	NO. 200	%	99.90	98.66	99.54	92.86
SAND	%	0.1	1.3	0.5	0.9	7.1
SILT	%	79.2	81.3	68.3	78.4	68.9
CLAY	%	20.7	17.3	31.3	20.6	24.0
LIQUID LIMIT	%	33	34	50	34	38
PLASTICITY INDEX	%	16	10	35	17	24
BEARING RATIO	%					
STANDARD DRY DENSITY AASHTO T99	%					
OPTIMUM MOISTURE	%					

REMARKS:

STATE OF ILLINOIS
Department of Transportation

SOIL TEST DATA

STATE JOB NUMBER D-98-034-00 ROUTE FAP 310 PROJECT US 67 (Delhi Bypass)
SECTION 42-2 COUNTY Jersey

BORING NO.		SB 59	SB 59	SB 60	SB 60	SB 67
LAB. NO.						
STATION		422+00	422+00	424+00	424+00	440+00
LOCATION		36 ft RT	36 ft RT	36 ft LT	36 ft LT	36 ft RT
DEPTH		0 - 2 ft	2 - 6 ft	0 - 6.5 ft	6.5 - 11 ft	0 - 3 ft
HRB CLASSIFICATION & GROUP INDEX		A-7-6(18)	A-6(11)	A-6(13)	A-4(3)	A-6(12)
GRAIN SIZE CLASSIFICATION		Silty Clay	Silty Clay Loam	Clay Loam	Loam	Clay
GRADUATION-PASSING 1" SIEVE	%	100.00	100.00	100.00	100.00	100.00
"	3/4" %	100.00	100.00	100.00	100.00	100.00
"	1/2" %	100.00	100.00	100.00	100.00	100.00
"	NO. 4 %	100.00	100.00	98.85	96.68	100.00
"	NO. 10 %	100.00	99.68	96.38	91.16	100.00
"	NO. 40 %	99.61	99.66	88.16	73.24	96.42
"	NO. 100 %	96.40	99.63	72.45	56.85	84.55
"	NO. 200 %	95.01	99.57	65.53	48.95	79.58
SAND	%	5.0	0.4	33.3	47.7	20.4
SILT	%	61.2	76.2	36.7	35.6	48.8
CLAY	%	33.8	23.4	28.8	13.3	30.8
LIQUID LIMIT	%	48	32	38	19	36
PLASTICITY INDEX	%	33	17	26	10	20
BEARING RATIO	%					
STANDARD DRY DENSITY AASHTO T99	%					
OPTIMUM MOISTURE	%					
REMARKS:				1.1% Gravel in Sample	3.3% Gravel in Sample	

STATE OF ILLINOIS
Department of Transportation

SOIL TEST DATA

STATE JOB NUMBER D-98-034-00 ROUTE FAP 310 PROJECT US 67 (Delhi Bypass)
SECTION 42-2 COUNTY Jersey

BORING NO.	SB 67	SB 72	SB 72	SB 72	
LAB. NO.					
STATION	440+00	454+00	454+00	454+00	
LOCATION	36 ft RT	2 ft RT	2 ft RT	2 ft RT	
DEPTH	3 - 6 ft	0 - 7 ft	12 - 17 ft	19.5 - 28.5 ft	
HRB CLASSIFICATION & GROUP INDEX	A-6(12)	A-7-6(15)	A-6(4)	A-6(3)	
GRAIN SIZE CLASSIFICATION	Silty Clay Loam	Silty Clay Loam	Sandy Clay Loam	Sandy Loam	
GRADUATION-PASSING 1" SIEVE	% 100.00	% 100.00	% 100.00	% 100.00	
" 3/4"	% 100.00	% 100.00	% 100.00	% 100.00	
" 1/2"	% 100.00	% 100.00	99.26	99.51	
" NO. 4	% 99.66	% 100.00	92.84	97.52	
" NO. 10	% 99.10	% 100.00	86.19	92.21	
" NO. 40	% 98.77	99.87	74.73	78.39	
" NO. 100	% 98.18	98.87	52.24	59.02	
" NO. 200	% 97.75	98.39	42.94	50.05	
SAND	% 1.9	1.6	49.9	47.5	
SILT	% 74.3	70.6	22.0	35.4	
CLAY	% 23.4	27.8	21.0	14.6	
LIQUID LIMIT	% 32	41	30	31	
PLASTICITY INDEX	% 19	26	20	18	
BEARING RATIO	%				
STANDARD DRY DENSITY AASHTO T99	%				
OPTIMUM MOISTURE	%				
REMARKS:	0.3% Gravel in Sample		7.2% Gravel in Sample	2.5% Gravel in Sample	

Soil Test Data Sheets

Service Road

STATE OF ILLINOIS
Department of Transportation

SOIL TEST DATA

STATE JOB NUMBER D-98-034-00 ROUTE FAP 310 PROJECT Service Road
SECTION 42-2 COUNTY Jersey

BORING NO.		SB 77	SB 77		
LAB. NO.					
STATION		101+75	101+75		
LOCATION		57 ft RT	57 ft RT		
DEPTH		0 - 4.5 ft	4.5 - 13.5 ft		
HRB CLASSIFICATION & GROUP INDEX		A-6(11)	A-6(10)		
GRAIN SIZE CLASSIFICATION		Silty Clay Loam	Silty Clay Loam		
GRADUATION-PASSING 1" SIEVE	%	100.00	100.00		
"	3/4"	%	100.00	100.00	
"	1/2"	%	100.00	100.00	
"	NO. 4	%	97.85	98.70	
"	NO. 10	%	94.23	94.96	
"	NO. 40	%	93.42	94.15	
"	NO. 100	%	91.55	92.46	
"	NO. 200	%	90.75	91.72	
SAND	%	7.1	7.0		
SILT	%	66.0	67.7		
CLAY	%	24.7	24.1		
LIQUID LIMIT	%	31	28		
PLASTICITY INDEX	%	18	16		
BEARING RATIO	%				
STANDARD DRY DENSITY AASHTO T99	%				
OPTIMUM MOISTURE	%				
REMARKS:		2.1% Gravel in Sample	1.3% Gravel in Sample		

Soil Test Data Sheets

Delhi Road

STATE OF ILLINOIS
Department of Transportation

SOIL TEST DATA

STATE JOB NUMBER D-98-034-00 ROUTE FAP 310 PROJECT Delhi Road
SECTION 42-2 COUNTY Jersey

BORING NO.		SB 86			
LAB. NO.					
STATION		55+00			
LOCATION		80 ft LT			
DEPTH		0 - 6 ft			
HRB CLASSIFICATION & GROUP INDEX		A-6(11)			
GRAIN SIZE CLASSIFICATION		Silty Clay Loam			
GRADUATION-PASSING 1" SIEVE	%	100.00			
"	3/4"	%	100.00		
"	1/2"	%	100.00		
"	NO. 4	%	100.00		
"	NO. 10	%	100.00		
"	NO. 40	%	99.92		
"	NO. 100	%	99.51		
"	NO. 200	%	99.31		
SAND	%	0.7			
SILT	%	76.7			
CLAY	%	22.7			
LIQUID LIMIT	%	34			
PLASTICITY INDEX	%	18			
BEARING RATIO	%				
STANDARD DRY DENSITY AASHTO T99	%				
OPTIMUM MOISTURE	%				

REMARKS:

Appendix D

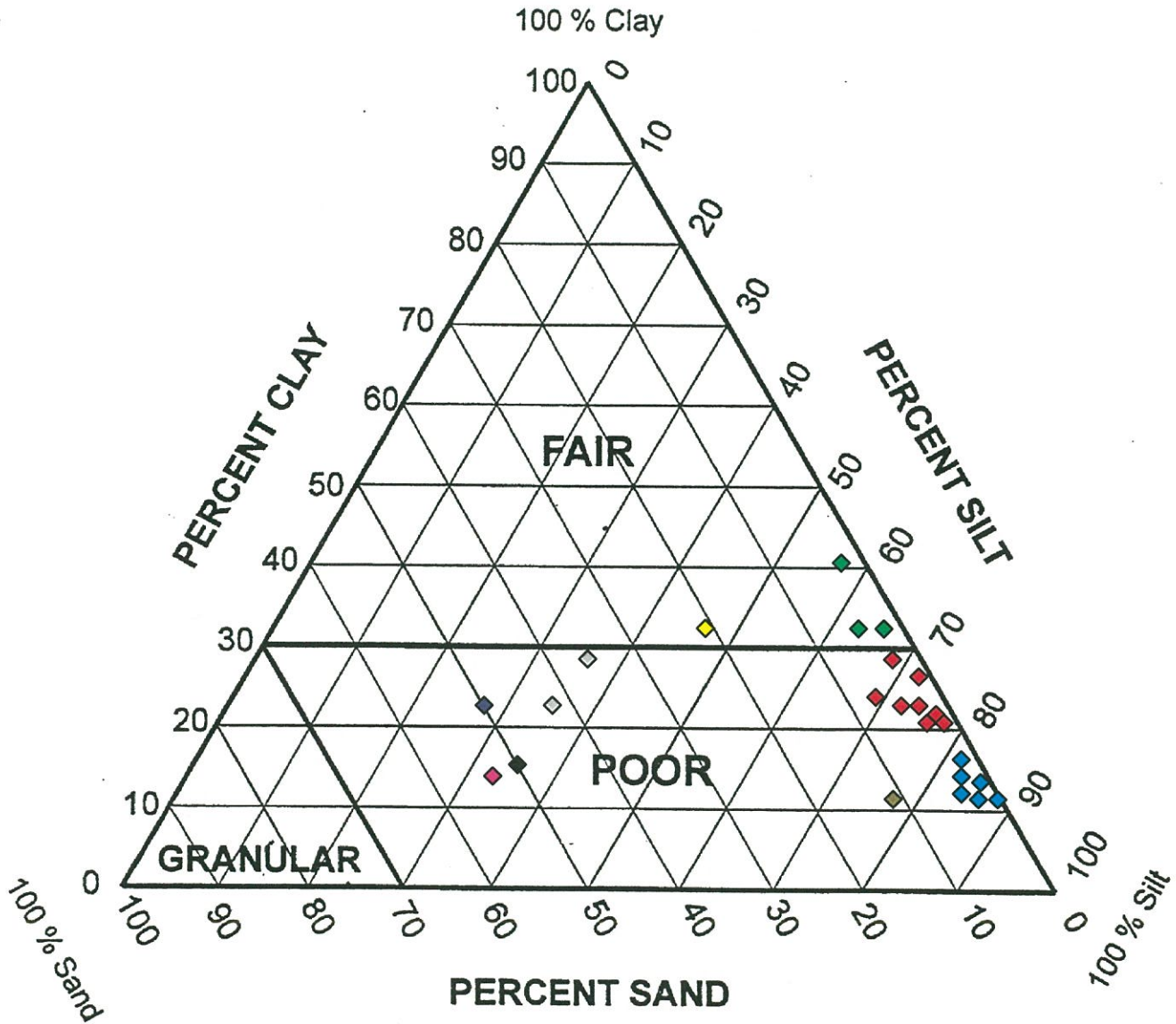
Subgrade Support Rating

Subgrade Support Rating

Station 333+19.67 to Station 463+50

SUBGRADE SUPPORT RATING

US 67 from Station 331+32.5 to 463+50



The subgrade support rating for the majority of these types of soil is **POOR**.

Soil classifications were conducted at approximately 1110 foot intervals along US 67.

Particle Size Limits
 Sand = 2.000 – 0.075 mm
 Silt = 0.075 – 0.002 mm
 Clay = Less than 0.002 mm

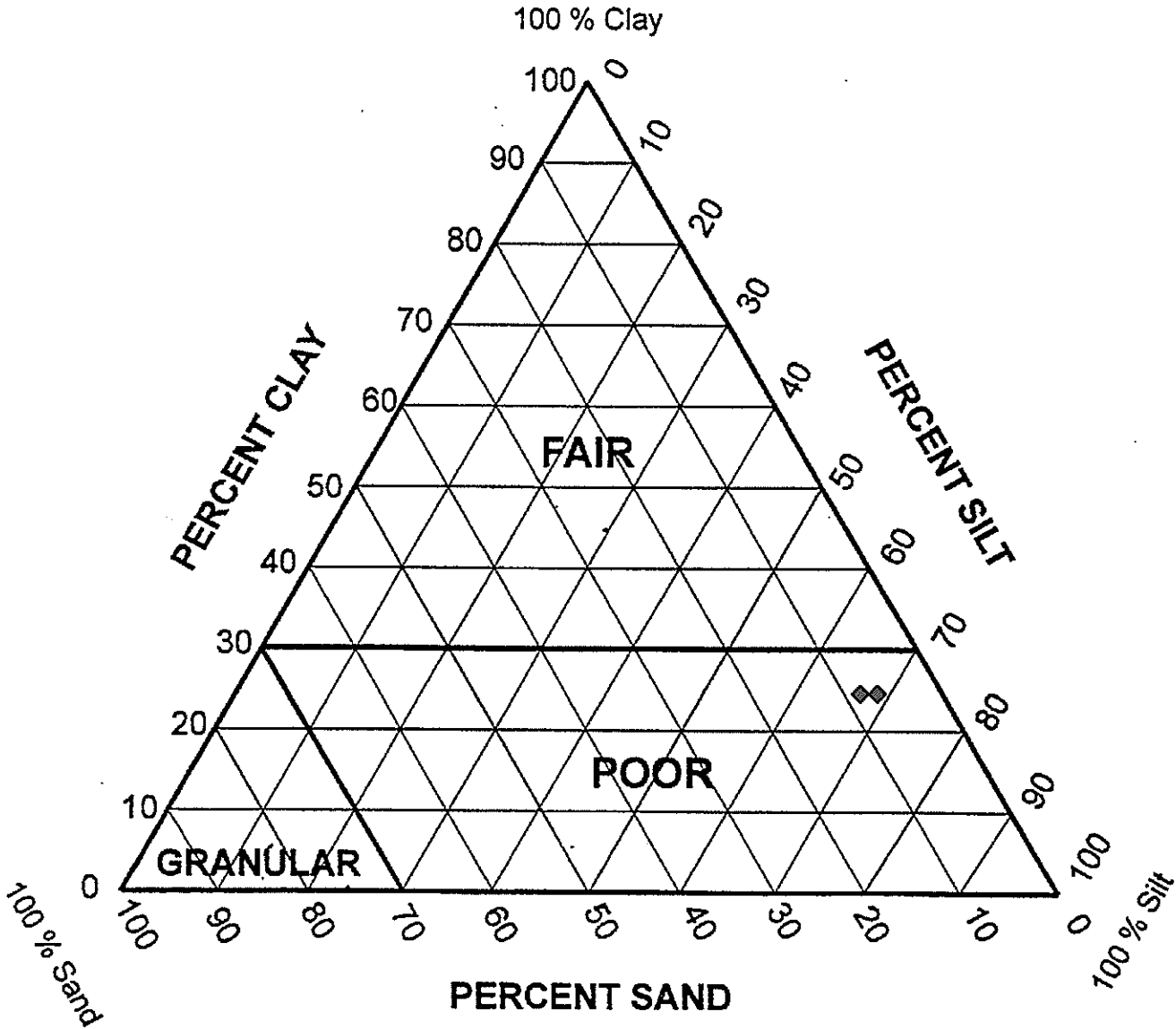
KEY: ◆ CLAY, ◆ CLAY LOAM, ◆ LOAM, ◆ SANDY CLAY LOAM, ◆ SANDY LOAM, ◆ SILT, ◆ SILTY CLAY, ◆ SILTY CLAY LOAM, ◆ SILTY LOAM

Subgrade Support Rating

Service Road

SUBGRADE SUPPORT RATING

Service Road



The subgrade support rating for these types of soil is **POOR**.

Soil classifications were conducted at Station 101+75, 57 feet Right on Service Road.

Particle Size Limits
 Sand = 2.000 – 0.075 mm
 Silt = 0.075 – 0.002 mm
 Clay = Less than 0.002 mm

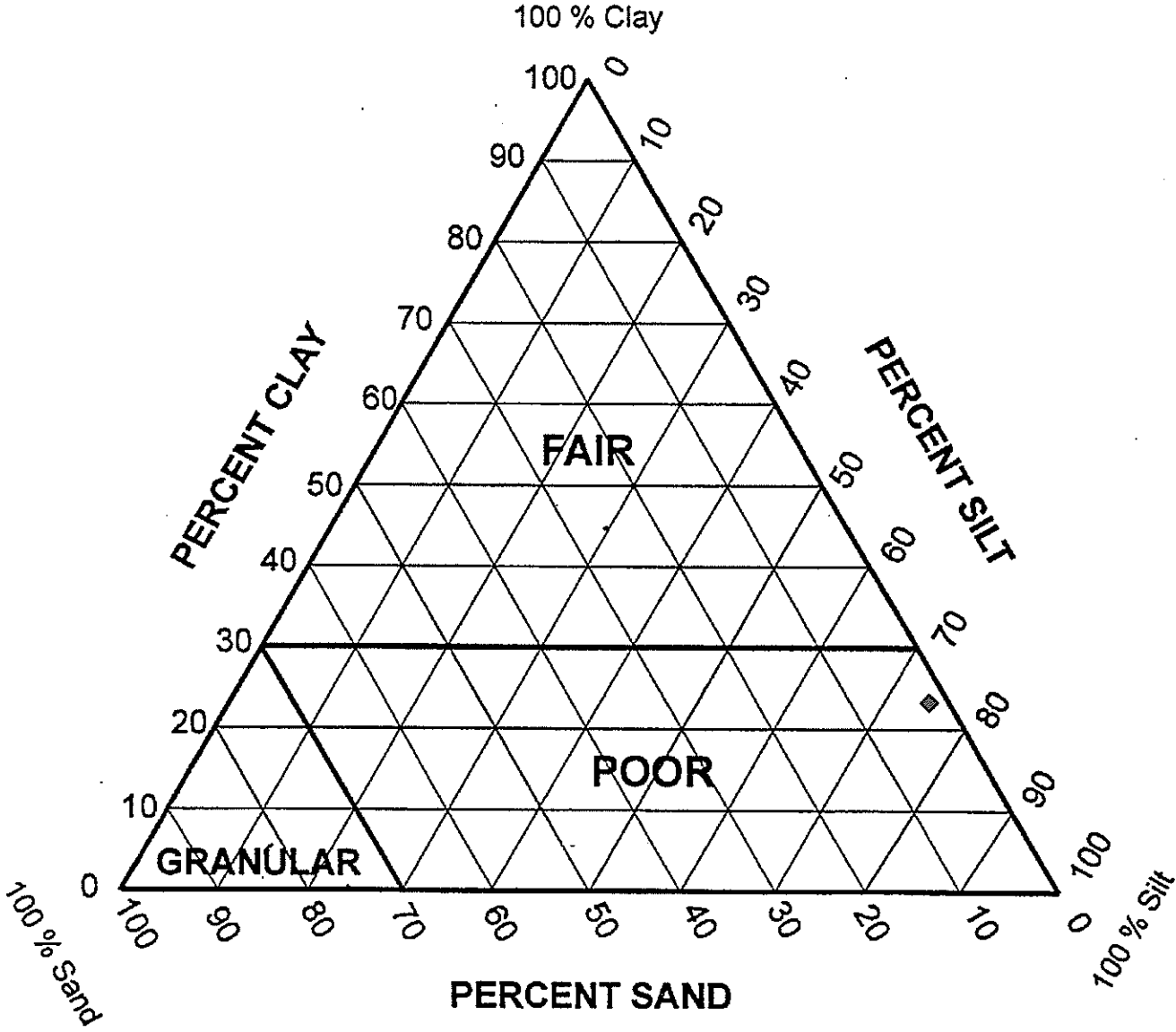
KEY: ◆ SILTY CLAY LOAM

Subgrade Support Rating

Delhi Road

SUBGRADE SUPPORT RATING

Delhi Road



The subgrade support rating for the majority of these types of soil is **POOR**.

Soil classifications were conducted at Station 55+00, 80 feet Left on Delhi Road.

Particle Size Limits
 Sand = 2.000 – 0.075 mm
 Silt = 0.075 – 0.002 mm
 Clay = Less than 0.002 mm

KEY: ◆ SILTY CLAY LOAM



Appendix E

Slope Stability Analyses

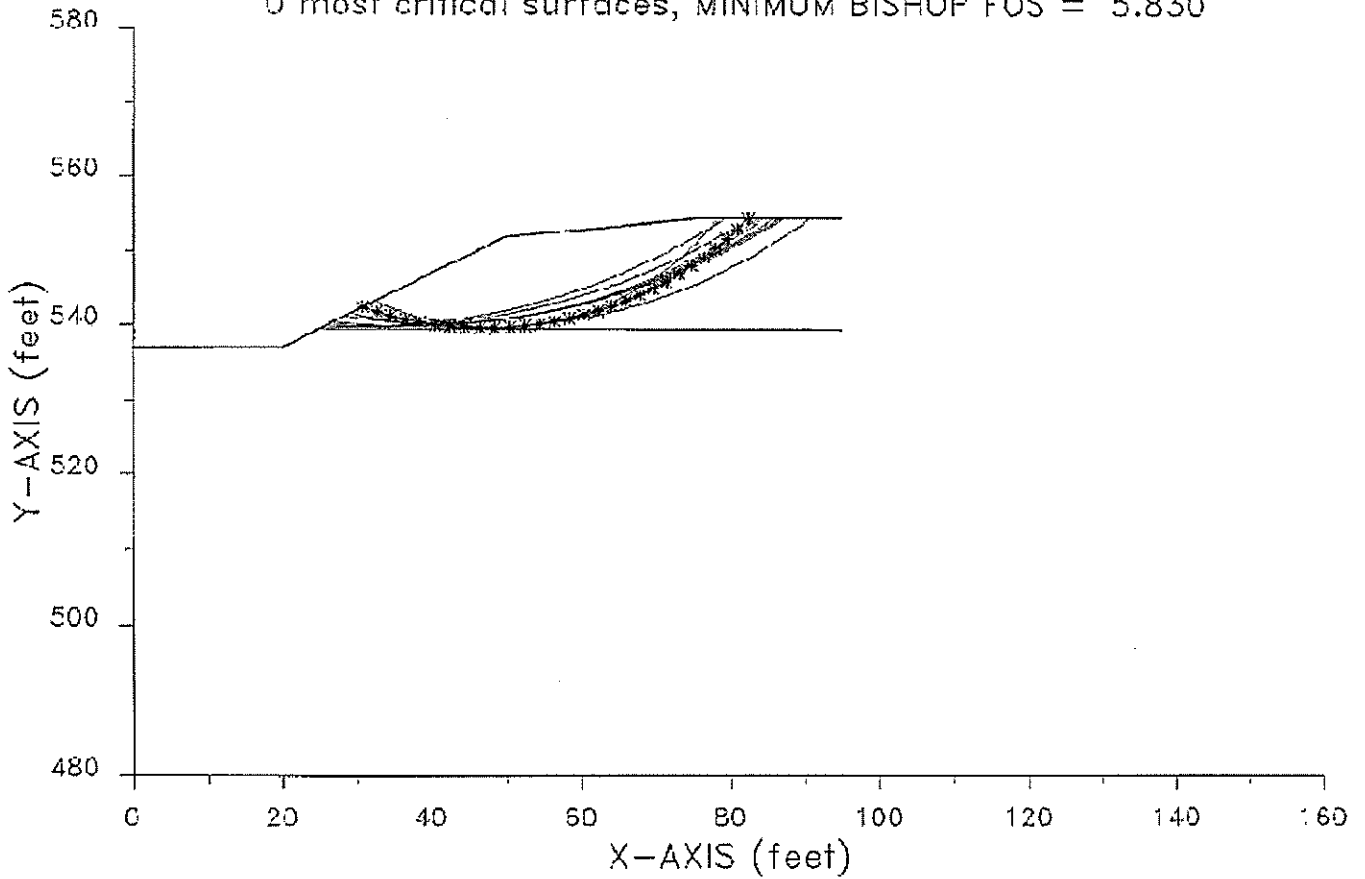


Slope Stability Analyses

Station 333+19.67 to Station 463+50

Station 336+00

10 most critical surfaces, MINIMUM BISHOP FOS = 5.830



PROFIL
Station 336+00

FILE: 33600

11-12-**

11:20

ft

6	5				
.0	537.0	20.0	537.0	2	
20.0	537.0	24.9	539.5	2	
24.9	539.5	50.0	552.0	1	
50.0	552.0	76.0	554.5	1	
76.0	554.5	95.0	554.5	1	
24.9	539.5	95.0	539.5	2	

SOIL

2

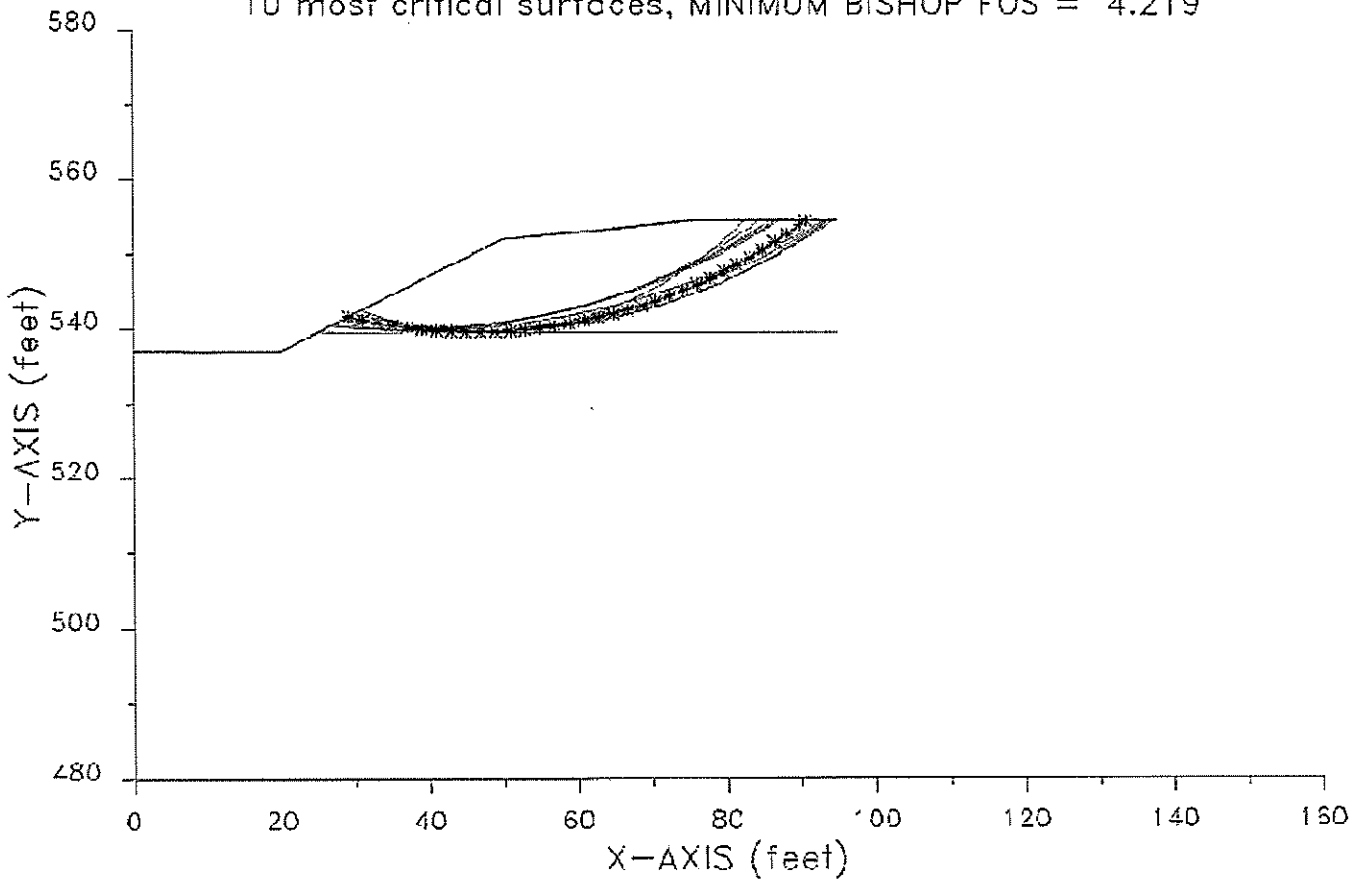
120.0	125.0	1000.0	.00	.000	.0	0
120.0	125.0	2520.0	.00	.000	.0	0

CIRCL2

25	50			
15.0	35.0	77.0	95.0	
526.0	.0	.0	.0	

Station 336+00, Seismic

10 most critical surfaces, MINIMUM BISHOP FOS = 4.219



PROFIL
Station 336+00, Seismic

FILE: 33600E 11-12-** 11:21 ft

6	5				
.0	537.0	20.0	537.0	2	
20.0	537.0	24.9	539.5	2	
24.9	539.5	50.0	552.0	1	
50.0	552.0	76.0	554.5	1	
76.0	554.5	95.0	554.5	1	
24.9	539.5	95.0	539.5	2	

SOIL

2							
120.0	125.0	1000.0	.00	.000	.0	0	
120.0	125.0	2520.0	.00	.000	.0	0	

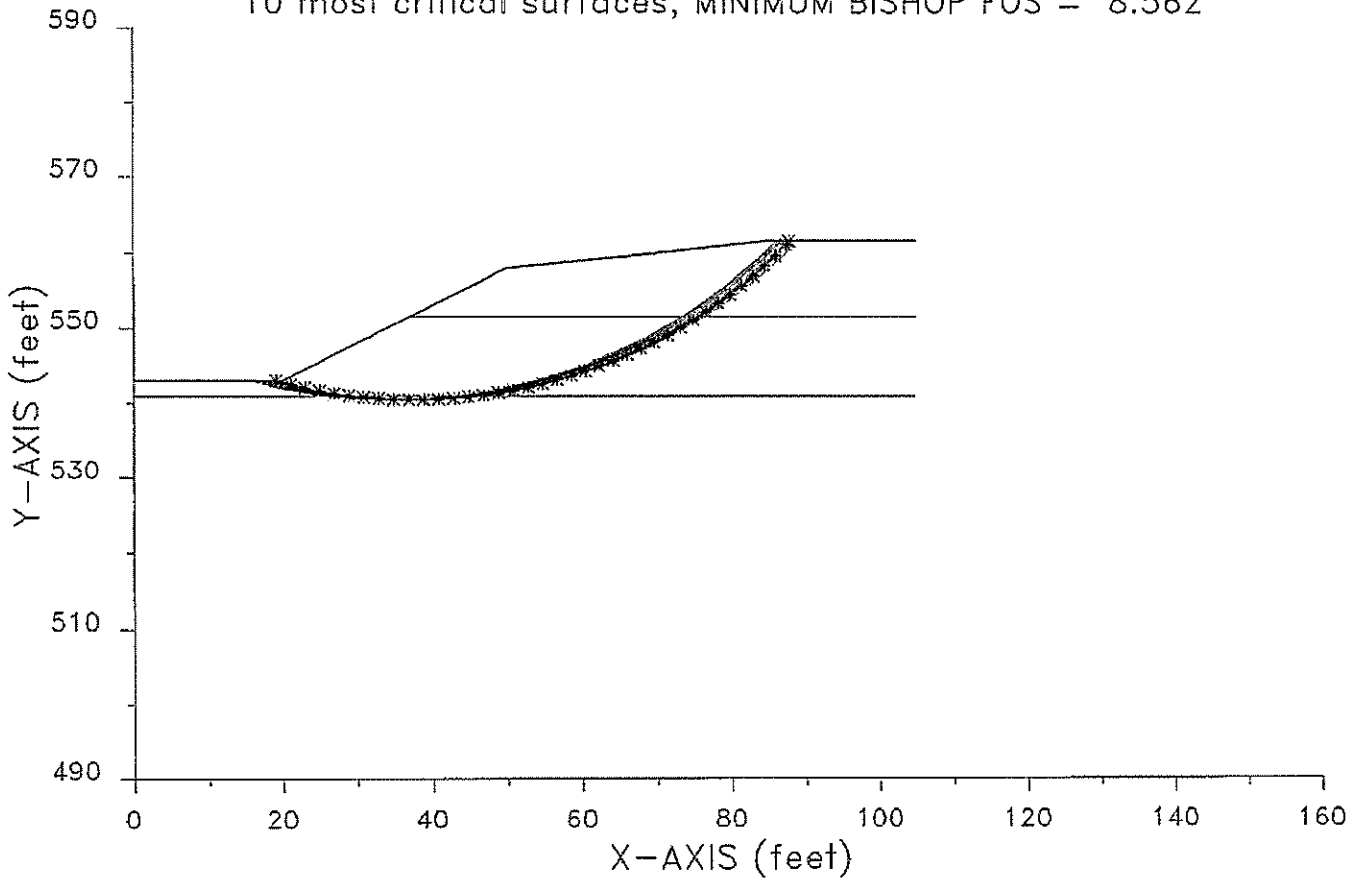
EQUAKE
.085 .000

CIRCL2

25	50			
15.0	35.0	77.0	95.0	
526.0	.0	.0	.0	

Station 338+00

10 most critical surfaces, MINIMUM BISHOP FOS = 8.562



PROFIL
Station 338+00

FILE: 33800

11-12-**

11:14

ft

7	5				
.0	543.0	20.0	543.0	2	
20.0	543.0	37.0	551.5	2	
37.0	551.5	50.0	558.0	1	
50.0	558.0	85.0	561.5	1	
85.0	561.5	105.0	561.5	1	
37.0	551.5	105.0	551.5	2	
.0	541.0	105.0	541.0	3	

SOIL

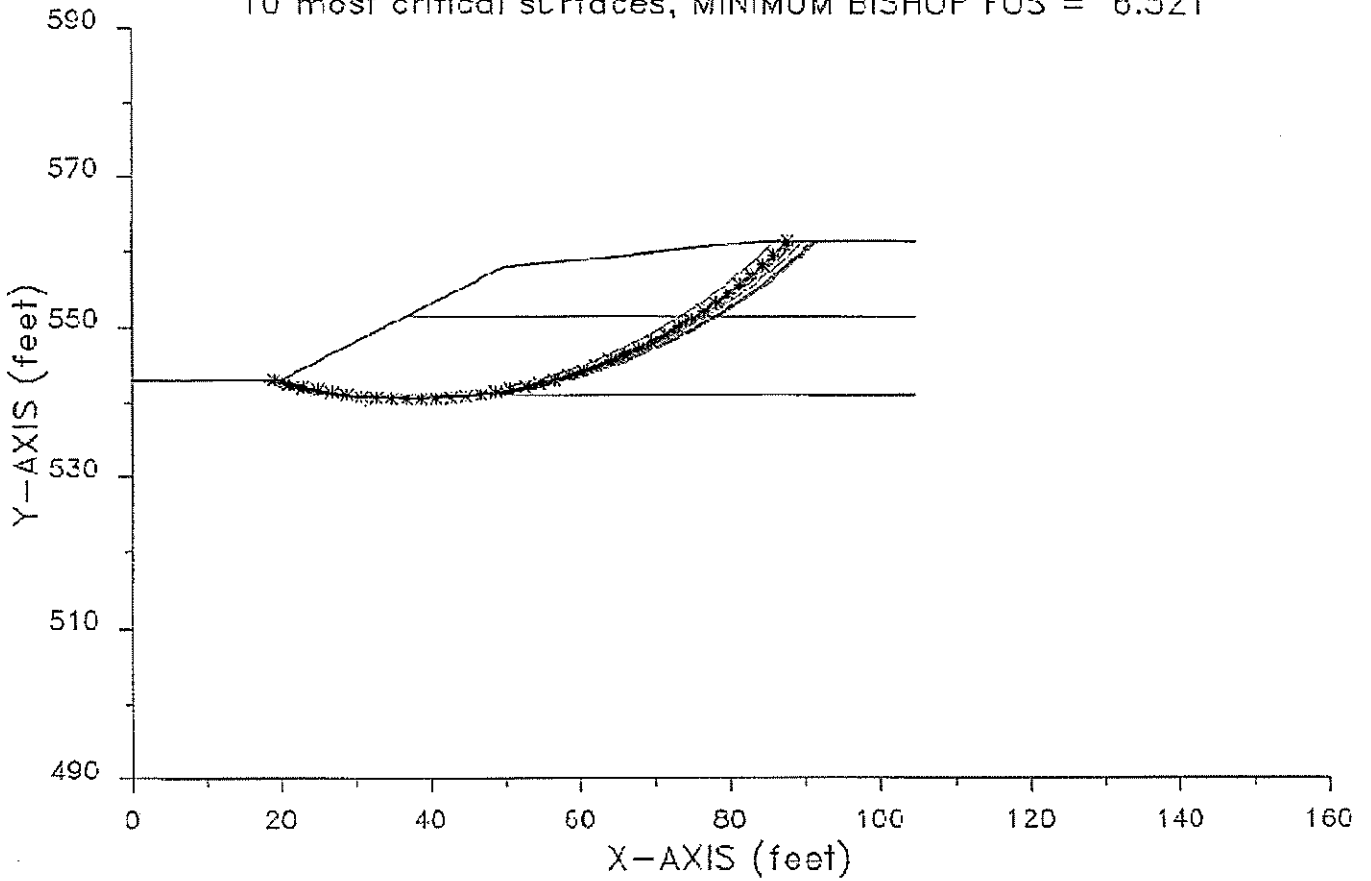
3						
120.0	125.0	1000.0	.00	.000	.0	0
120.0	125.0	2950.0	.00	.000	.0	0
120.0	125.0	.0	50.00	.000	.0	0

CIRCL2

25	50			
15.0	35.0	86.0	105.0	
540.5	.0	.0	.0	

Station 338+00, Seismic

10 most critical surfaces, MINIMUM BISHOP FOS = 6.521



PROFIL
Station 338+00, seismic

FILE: 33800E 11-12-** 11:14 ft

7	5				
.0	543.0	20.0	543.0	2	
20.0	543.0	37.0	551.5	2	
37.0	551.5	50.0	558.0	1	
50.0	558.0	85.0	561.5	1	
85.0	561.5	105.0	561.5	1	
37.0	551.5	105.0	551.5	2	
.0	541.0	105.0	541.0	3	

SOIL

3						
120.0	125.0	1000.0	.00	.000	.0	0
120.0	125.0	2950.0	.00	.000	.0	0
120.0	125.0	.0	50.00	.000	.0	0

EQUAKE

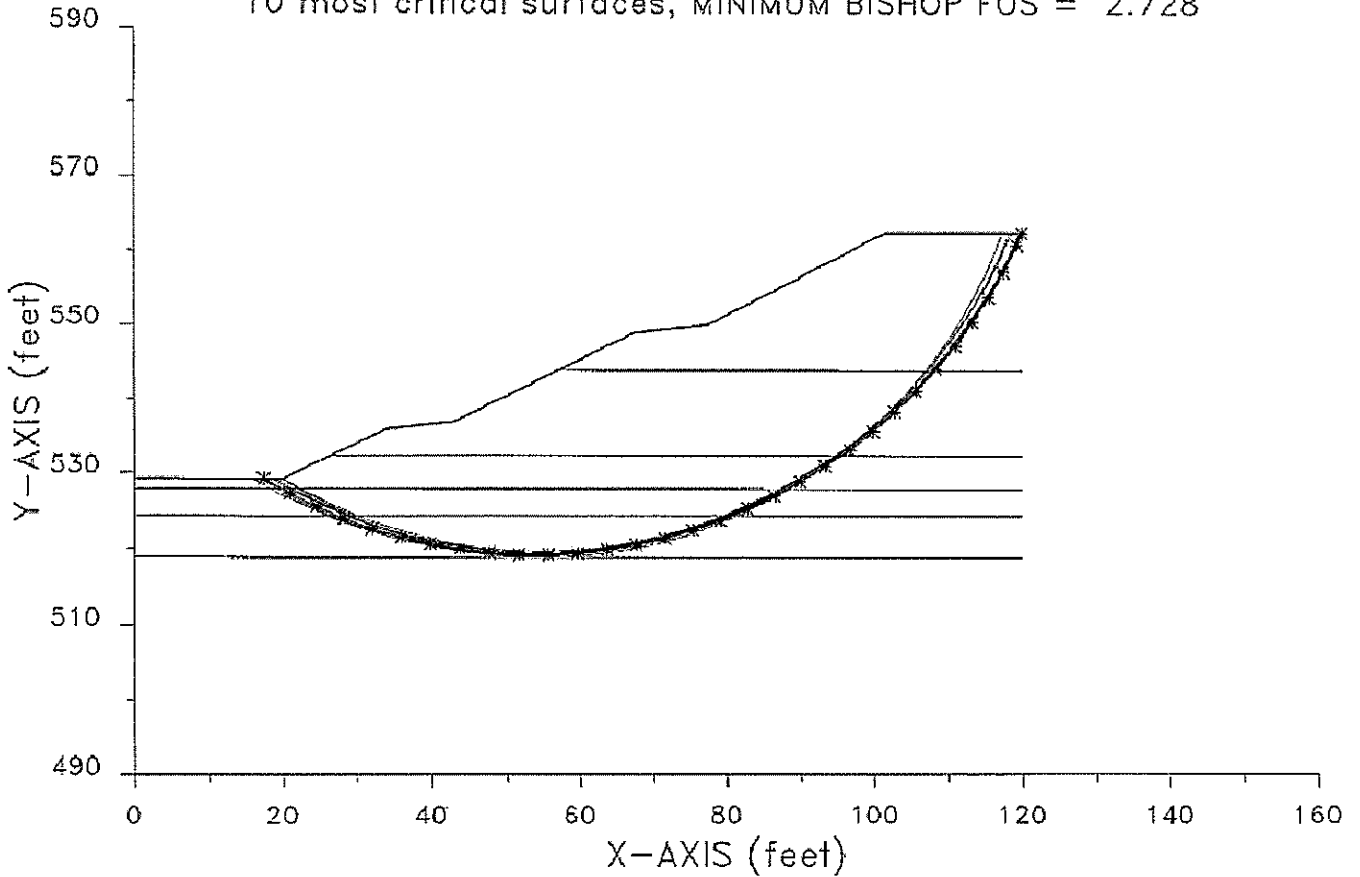
.085 .000

CIRCL2

25	50			
15.0	35.0	86.0	105.0	
540.5	.0	.0	.0	

Station 347+00

10 most critical surfaces, MINIMUM BISHOP FOS = 2.728



PROFIL
Station 347+00

FILE: 34700

11 12 88

11:23

ft

Station	Y (ft)	X (ft)	Y (ft)	Order
14	9			
.0	529.2	20.0	529.2	3
20.0	529.2	26.6	532.4	2
26.6	532.4	34.0	536.0	2
34.0	536.0	43.0	537.0	2
43.0	537.0	57.1	543.9	2
57.1	543.9	67.5	549.0	1
67.5	549.0	77.5	550.0	1
77.5	550.0	101.5	562.5	1
101.5	562.5	120.0	562.5	1
57.1	543.9	120.0	543.9	2
26.6	532.4	120.0	532.4	3
.0	527.9	120.0	527.9	4
.0	524.4	120.0	524.4	5
.0	518.9	120.0	518.9	6

SOIL

6

X (ft)	Y (ft)	Thickness (ft)	Unit Weight (pcf)	cohesion (pcf)	friction (pcf)	phi (deg)	c (pcf)
120.0	125.0	1000.0	.00	.000	.0	0	
120.0	125.0	2075.0	.00	.000	.0	0	
120.0	125.0	4050.0	.00	.000	.0	0	
120.0	125.0	700.0	.00	.000	.0	0	
120.0	125.0	1150.0	.00	.000	.0	0	
120.0	125.0	4000.0	.00	.000	.0	0	

CIRCL2

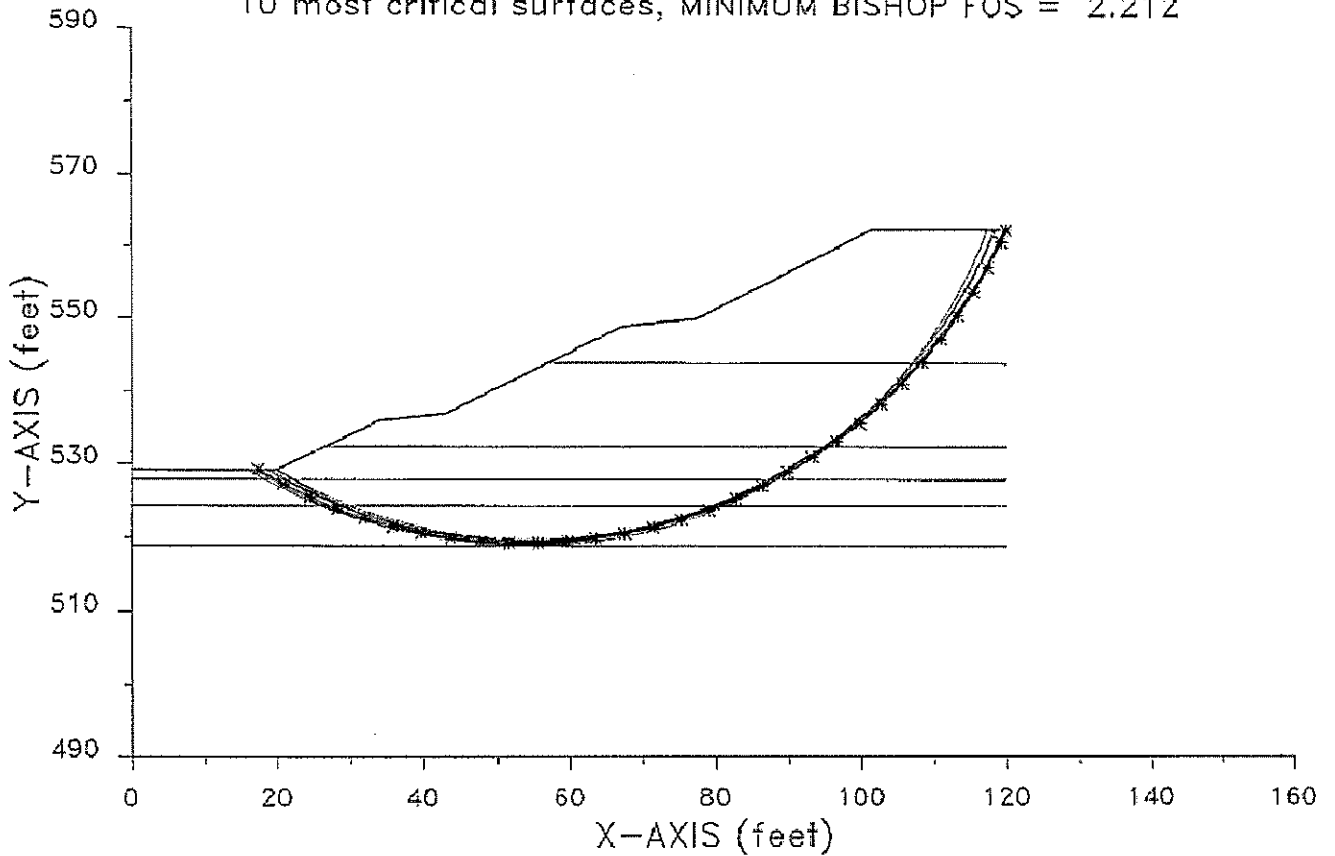
25

50

15.0	27.0	102.0	120.0
516.4	.0	0	0

Station 347+00, Seismic

10 most critical surfaces, MINIMUM BISHOP FOS = 2.212



PROFIL

FILE: 34700E

11-12-**

11:23

ft

Station 347+00, Seismic

Station	Y (ft)	X (ft)	Y (ft)	Order
14	9			
.0	529.2	20.0	529.2	3
20.0	529.2	26.6	532.4	3
26.6	532.4	34.0	536.0	2
34.0	536.0	43.0	537.0	2
43.0	537.0	57.1	543.9	2
57.1	543.9	67.5	549.0	1
67.5	549.0	77.5	550.0	1
77.5	550.0	101.5	562.5	1
101.5	562.5	120.0	562.5	1
57.1	543.9	120.0	543.9	2
26.6	532.4	120.0	532.4	3
.0	527.9	120.0	527.9	4
.0	524.4	120.0	524.4	5
.0	518.9	120.0	518.9	6

SOIL

Station	Y (ft)	U (ft)	C (lb/ft ³)	Phi (deg)	Cohesion (lb/ft ²)	Gamma (lb/ft ³)
6						
120.0	125.0	1000.0	.00	.000	.0	0
120.0	125.0	2075.0	.00	.000	.0	0
120.0	125.0	4050.0	.00	.000	.0	0
120.0	125.0	700.0	.00	.000	.0	0
120.0	125.0	1150.0	.00	.000	.0	0
120.0	125.0	4000.0	.00	.000	.0	0

EQUAKE

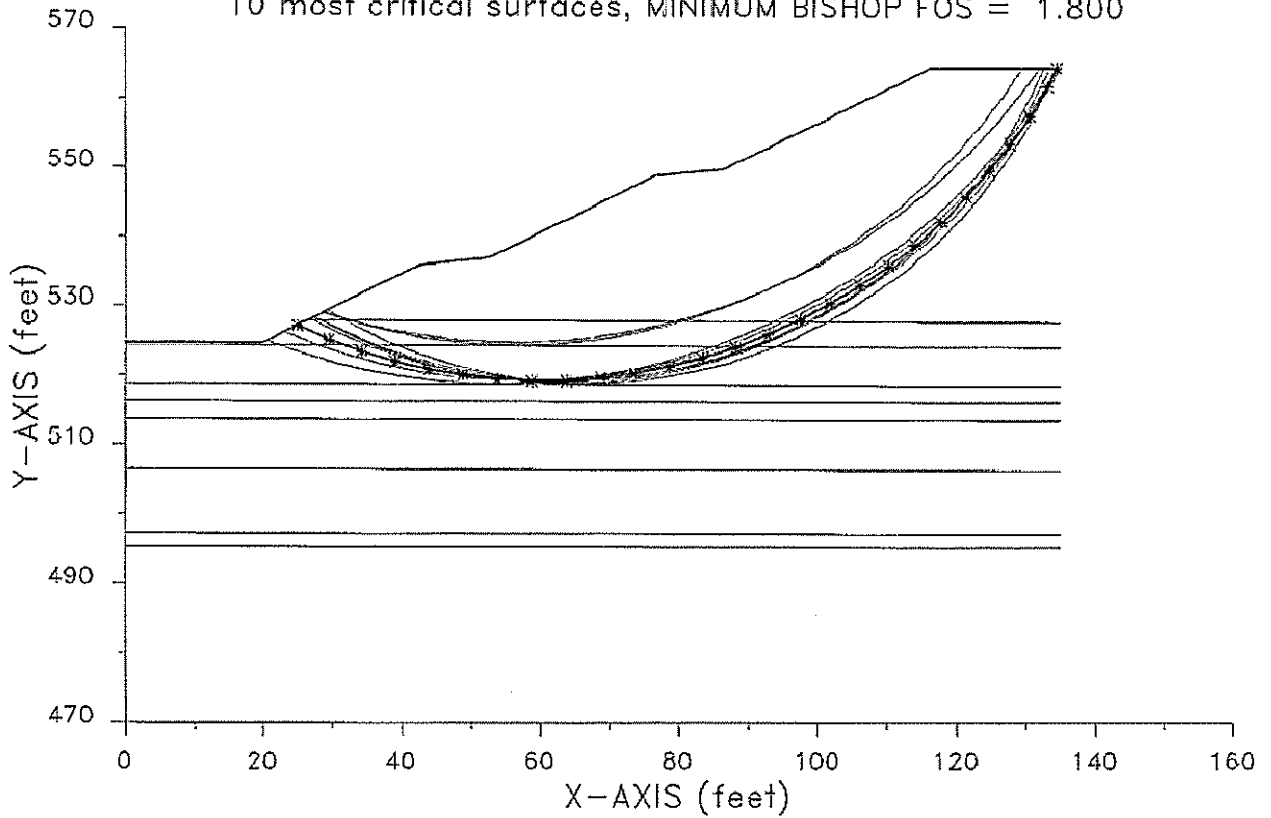
.085 .000

CIRCL2

Station	Y (ft)	U (ft)	C (lb/ft ³)	Phi (deg)
25	50			
15.0	27.0	102.0	120.0	
516.4	.0	.0	.0	

Station 349+00, Left

10 most critical surfaces, MINIMUM BISHOP FOS = 1.800



PROFIL
Station 349+00, Left
16 8

FILE: 34900L 11-12-*** 11:15 ft

.0	524.7	20.0	524.7	2
20.0	524.7	26.5	527.9	2
26.5	527.9	42.7	536.0	1
42.7	536.0	52.7	537.0	1
52.7	537.0	76.7	549.0	1
76.7	549.0	86.7	550.0	1
86.7	550.0	116.3	564.8	1
116.3	564.8	135.0	564.8	1
26.5	527.9	135.0	527.9	2
.0	524.4	135.0	524.4	3
.0	518.9	135.0	518.9	4
.0	516.4	135.0	516.4	5
.0	513.9	135.0	513.9	6
.0	506.4	135.0	506.4	7
.0	497.4	135.0	497.4	8
.0	495.4	135.0	495.4	9

SOIL

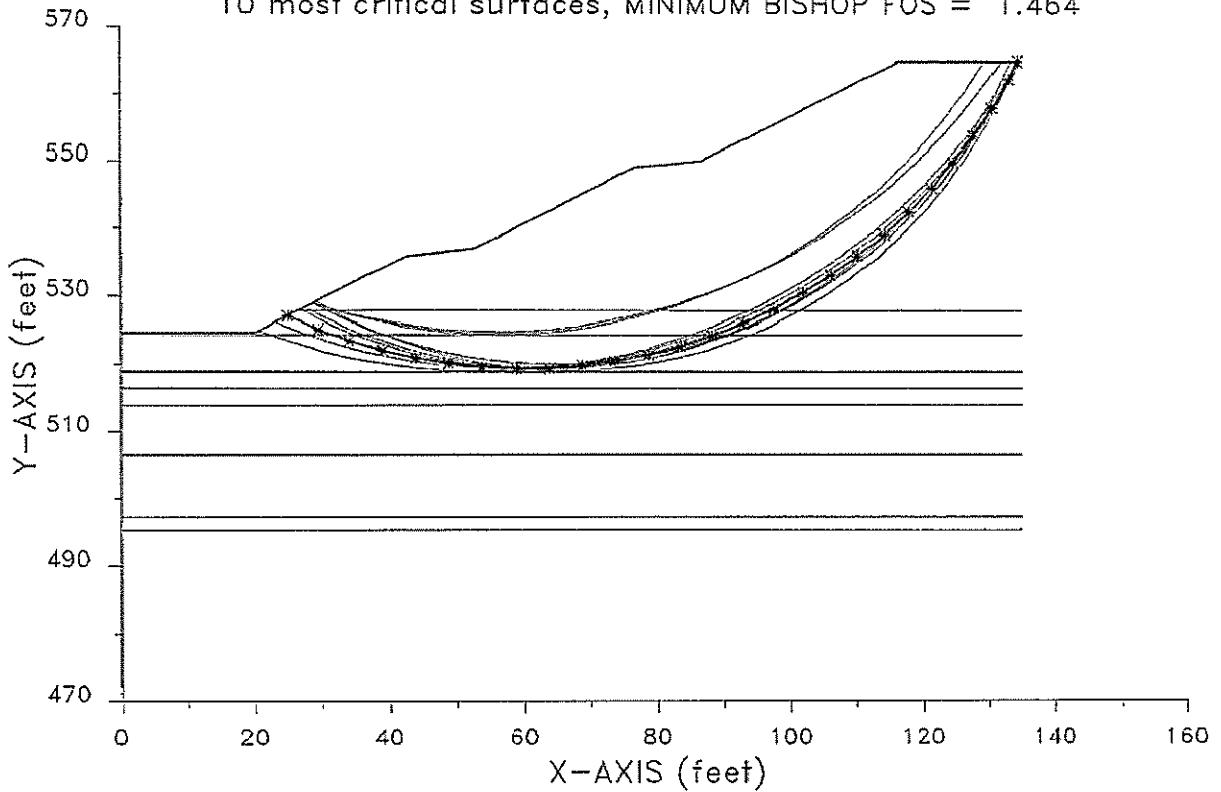
9						
120.0	125.0	1000.0	.00	.000	.0	0
120.0	125.0	700.0	.00	.000	.0	0
120.0	125.0	1150.0	.00	.000	.0	0
120.0	125.0	4000.0	.00	.000	.0	0
120.0	125.0	2900.0	.00	.000	.0	0
120.0	125.0	4333.3	.00	.000	.0	0
120.0	125.0	2300.0	.00	.000	.0	0
120.0	125.0	1800.0	.00	.000	.0	0
120.0	125.0	3800.0	.00	.000	.0	0

CIRCL2

25	50				
15.0	30.0	117.0	135.0		
491.9	.0	.0	.0		

Station 349+00, Left, Seismic

10 most critical surfaces, MINIMUM BISHOP FOS = 1.464



PROFIL
Station 349+00, Left, Seismic

FILE: 34900LE 11-12-** 11:16 ft

15	8				
.0	524.7	20.0	524.7	2	
20.0	524.7	26.5	527.9	2	
26.5	527.9	42.7	536.0	1	
42.7	536.0	52.7	537.0	1	
52.7	537.0	76.7	549.0	1	
76.7	549.0	86.7	550.0	1	
86.7	550.0	116.3	564.8	1	
116.3	564.8	135.0	564.8	1	
26.5	527.9	135.0	527.9	2	
.0	524.4	135.0	524.4	3	
.0	518.9	135.0	518.9	4	
.0	516.4	135.0	516.4	5	
.0	513.9	135.0	513.9	6	
.0	506.4	135.0	506.4	7	
.0	497.4	135.0	497.4	8	
.0	495.4	135.0	495.4	9	

SOIL

9						
120.0	125.0	1000.0	.00	.000	.0	0
120.0	125.0	700.0	.00	.000	.0	0
120.0	125.0	1150.0	.00	.000	.0	0
120.0	125.0	4000.0	.00	.000	.0	0
120.0	125.0	2900.0	.00	.000	.0	0
120.0	125.0	4333.3	.00	.000	.0	0
120.0	125.0	2300.0	.00	.000	.0	0
120.0	125.0	1800.0	.00	.000	.0	0
120.0	125.0	3800.0	.00	.000	.0	0

EQUAKE

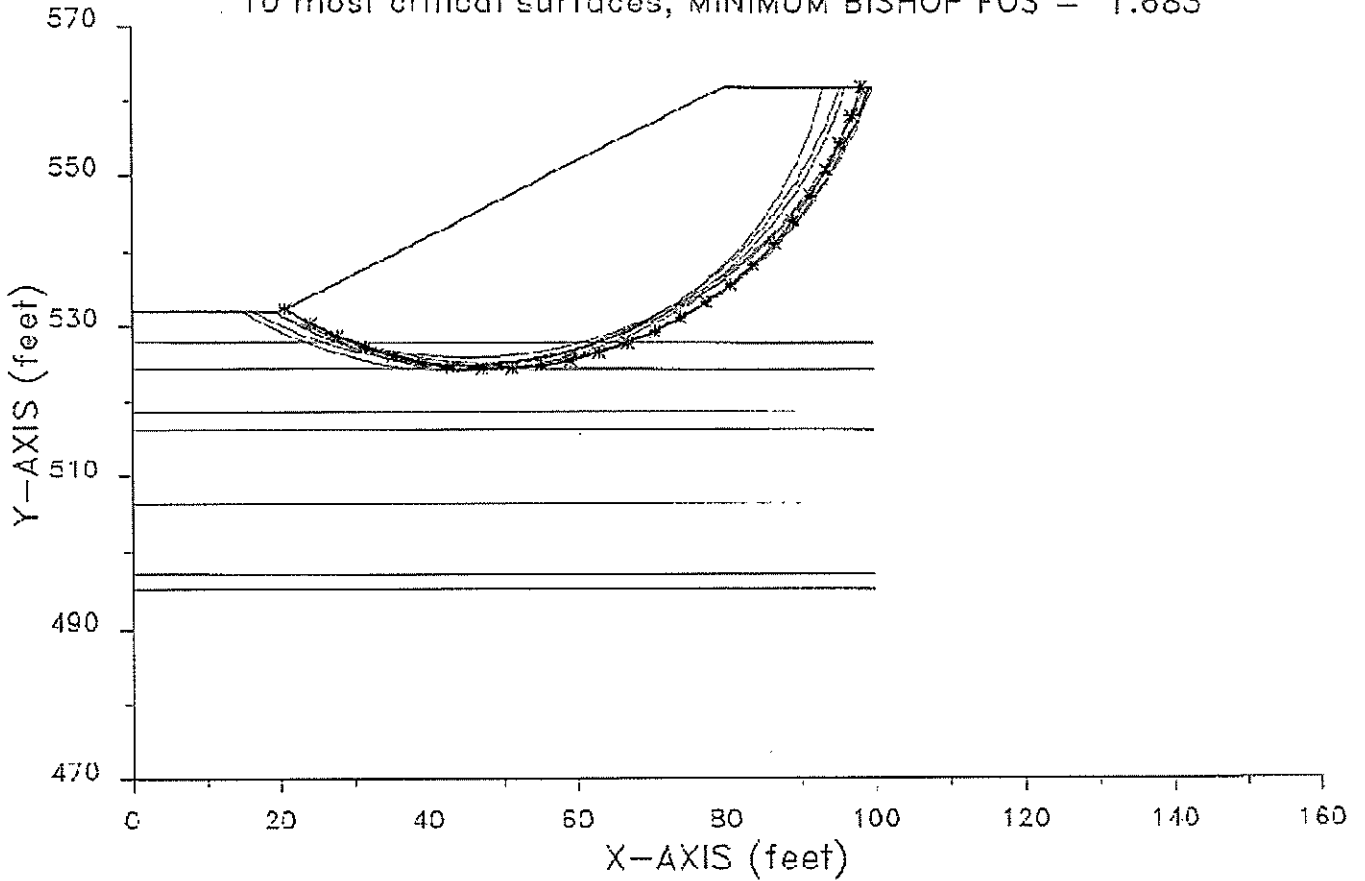
.085 .000

CIRCL2

25	50			
15.0	30.0	117.0	135.0	
491.9	.0	.0	.0	

Station 349+00, Right

10 most critical surfaces, MINIMUM BISHOP FOS = 1.683



PROFIL
Station 349+00, Right

FILE: 34900R 11-12-** 11:24 ft

10	3				
.0	532.0	20.0	532.0	1	
20.0	532.0	80.0	562.0	1	
80.0	562.0	100.0	562.0	1	
.0	527.9	100.0	527.9	2	
.0	524.4	100.0	524.4	3	
.0	518.9	100.0	518.9	4	
.0	516.4	100.0	516.4	5	
.0	506.4	100.0	506.4	6	
.0	497.4	100.0	497.4	7	
.0	495.4	100.0	495.4	8	

SOIL

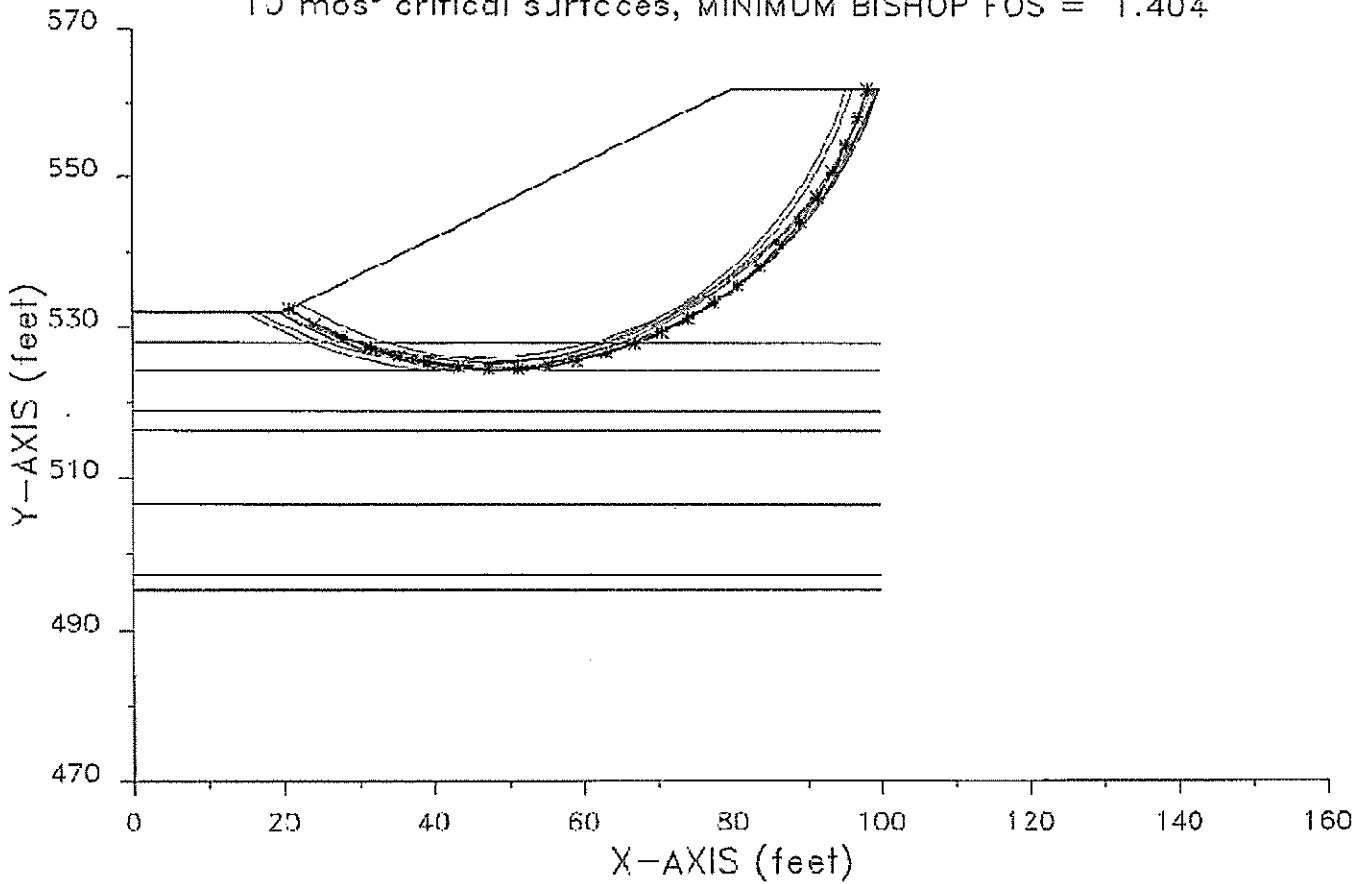
8							
120.0	125.0	1000.0	.00	.000	.0	0	
120.0	125.0	700.0	.00	.000	.0	0	
120.0	125.0	1150.0	.00	.000	.0	0	
120.0	125.0	4000.0	.00	.000	.0	0	
120.0	125.0	3975.0	.00	.000	.0	0	
120.0	125.0	2300.0	.00	.000	.0	0	
120.0	125.0	1800.0	.00	.000	.0	0	
120.0	125.0	3800.0	.00	.000	.0	0	

CIRCL2

25	50			
15.0	50.0	81.0	100.0	
491.9	.0	.0	.0	

Station 349+00, Right, Seismic

10 most critical surfaces, MINIMUM BISHOP FOS = 1.404



PROFIL
Station 349+00, Right, Seismic

FILE: 34900RE 11-12-** 11:25 ft

10	3				
.0	532.0	20.0	532.0	1	
20.0	532.0	80.0	562.0	1	
80.0	562.0	100.0	562.0	1	
.0	527.9	100.0	527.9	2	
.0	524.4	100.0	524.4	3	
.0	518.9	100.0	518.9	4	
.0	516.4	100.0	516.4	5	
.0	506.4	100.0	506.4	6	
.0	497.4	100.0	497.4	7	
.0	495.4	100.0	495.4	8	

SOIL

3							
120.0	125.0	1000.0	.00	.000	.C	0	
120.0	125.0	700.0	.00	.000	.C	0	
120.0	125.0	1150.0	.00	.000	.C	0	
120.0	125.0	4000.0	.00	.000	.C	0	
120.0	125.0	3975.0	.00	.000	.C	0	
120.0	125.0	2300.0	.00	.000	.C	0	
120.0	125.0	1800.0	.00	.000	.C	0	
120.0	125.0	3800.0	.00	.000	.C	0	

EQUAKE

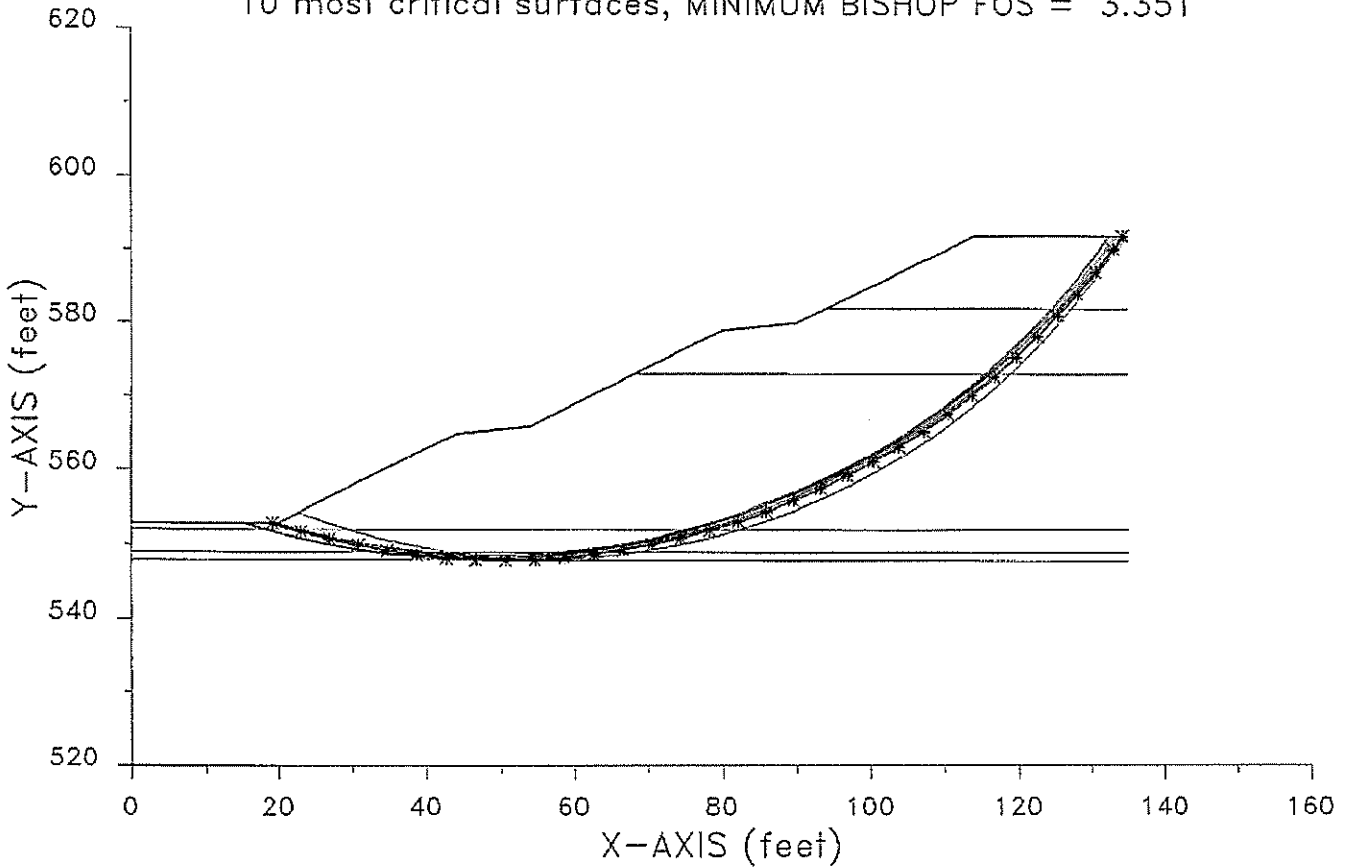
.085 .000

CIRCL2

25	50				
15.0	50.0	R1.0	100.0		
491.9	.0	.0	.0		

Station 436+00

10 most critical surfaces, MINIMUM BISHOP FOS = 3.351



PROFIL
Station 436+00
15 10

FILE: 43600 11-12-** 11:18 ft

.0	552.9	20.0	552.9	3
20.0	552.9	44.2	565.0	3
44.2	565.0	54.2	566.0	3
54.2	566.0	64.2	571.0	3
64.2	571.0	68.2	573.0	3
68.2	573.0	80.2	579.0	2
80.2	579.0	90.2	580.0	2
90.2	580.0	94.2	582.0	2
94.2	582.0	114.2	592.0	1
114.2	592.0	135.0	592.0	1
94.2	582.0	135.0	582.0	2
68.2	573.0	135.0	573.0	3
.0	552.0	135.0	552.0	4
.0	549.0	135.0	549.0	5
.0	548.0	135.0	548.0	6

SOIL

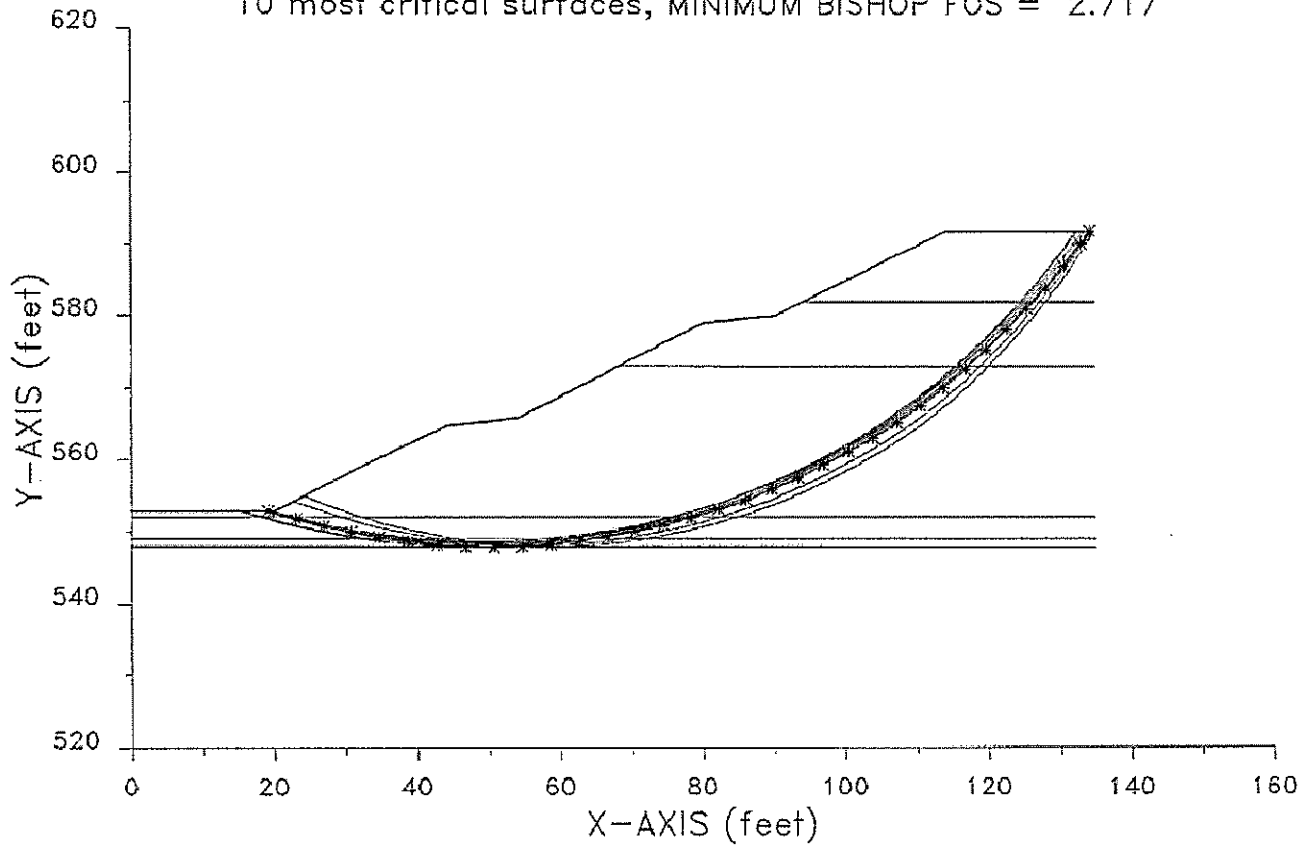
6						
120.0	125.0	1000.0	.00	.000	.0	0
120.0	125.0	4700.0	.00	.000	.0	0
120.0	125.0	2133.3	.00	.000	.0	0
120.0	125.0	.0	31.00	.000	.0	0
120.0	125.0	1300.0	.00	.000	.0	0
120.0	125.0	3967.7	.00	.000	.0	0

CIRCL2

25	50			
15.0	32.0	115.0	135.0	
541.0	.0	.0	.0	

Station 436+00, Seismic

10 most critical surfaces, MINIMUM BISHOP FOS = 2.717



PROFIL
Station 436+00, Seismic

FILE: 43600E 11-12-** 11:19 ft

15	10				
.0	552.9	20.0	552.9	3	
20.0	552.9	44.2	565.0	3	
44.2	565.0	54.2	566.0	3	
54.2	566.0	64.2	571.0	3	
64.2	571.0	68.2	573.0	3	
68.2	573.0	80.2	579.0	2	
80.2	579.0	90.2	580.0	2	
90.2	580.0	94.2	582.0	2	
94.2	582.0	114.2	592.0	1	
114.2	592.0	135.0	592.0	1	
94.2	582.0	135.0	582.0	2	
68.2	573.0	135.0	573.0	3	
.0	552.0	135.0	552.0	4	
.0	549.0	135.0	549.0	5	
.0	548.0	135.0	548.0	6	

SOIL

6						
120.0	125.0	1000.0	.00	.000	.0	0
120.0	125.0	4700.0	.00	.000	.0	0
120.0	125.0	2133.3	.00	.000	.0	0
120.0	125.0	.0	31.00	.000	.0	0
120.0	125.0	1300.0	.00	.000	.0	0
120.0	125.0	3967.7	.00	.000	.0	0

EQUAKE

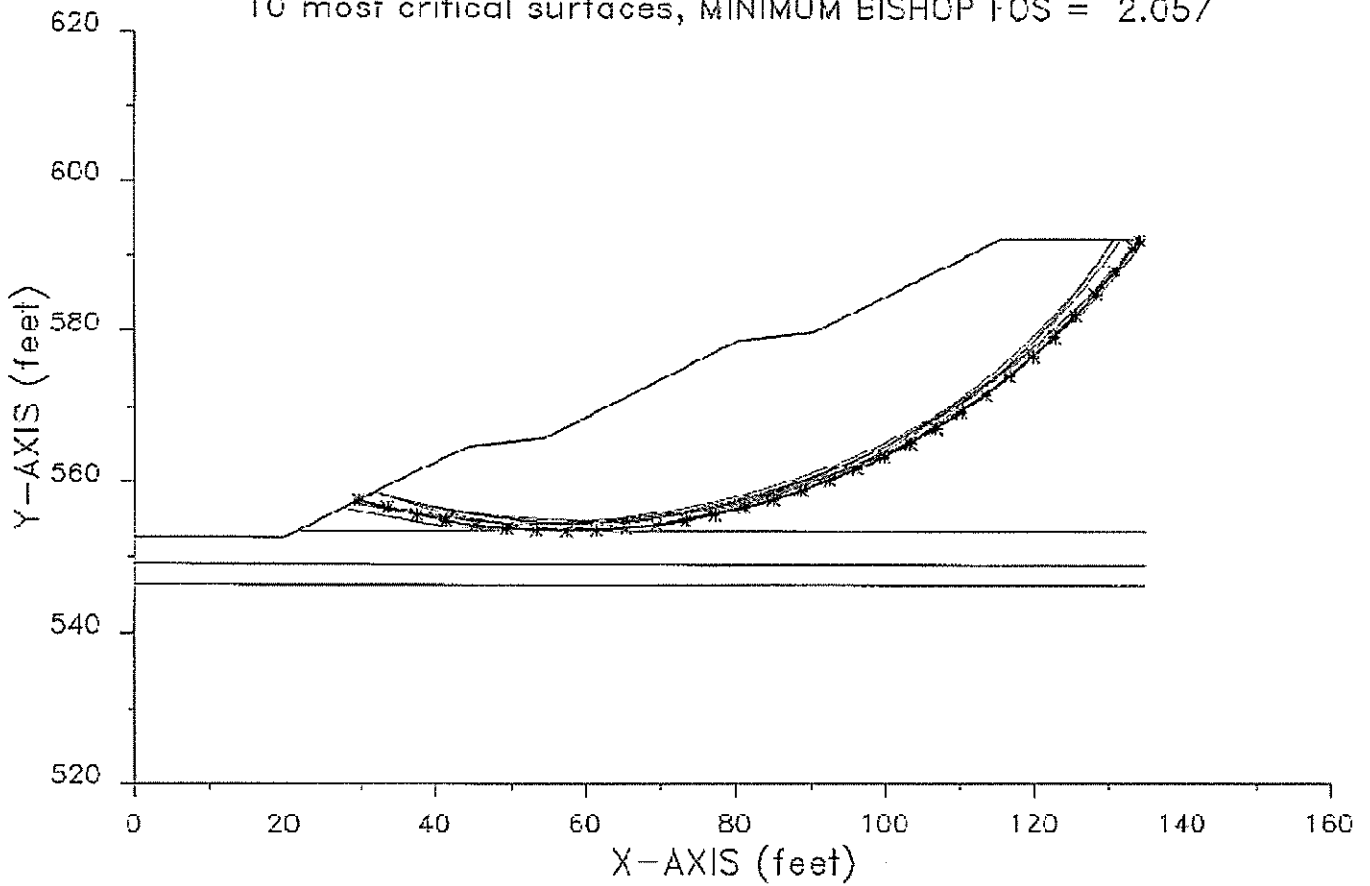
.085 .000

CIRCL2

25	50			
15.0	32.0	115.0	135.0	
541.0	.0	.0	.0	

Station 438+00, Left

10 most critical surfaces, MINIMUM BISHOP FOS = 2.057



PROFIL

FILE: 43800L

11-12-**

11:25

ft.

Station 438+00, Left

11 8

.0	552.6	20.0	552.6	2
20.0	552.6	21.8	553.5	2
21.8	553.5	44.5	565.0	1
44.5	565.0	54.5	566.0	1
54.5	566.0	80.5	579.0	1
80.5	579.0	90.5	580.0	1
90.5	580.0	115.5	592.5	1
115.5	592.5	135.0	592.5	1
21.8	553.5	135.0	553.5	2
.0	549.0	135.0	549.0	3
.0	546.5	135.0	546.5	4

SOIL

4

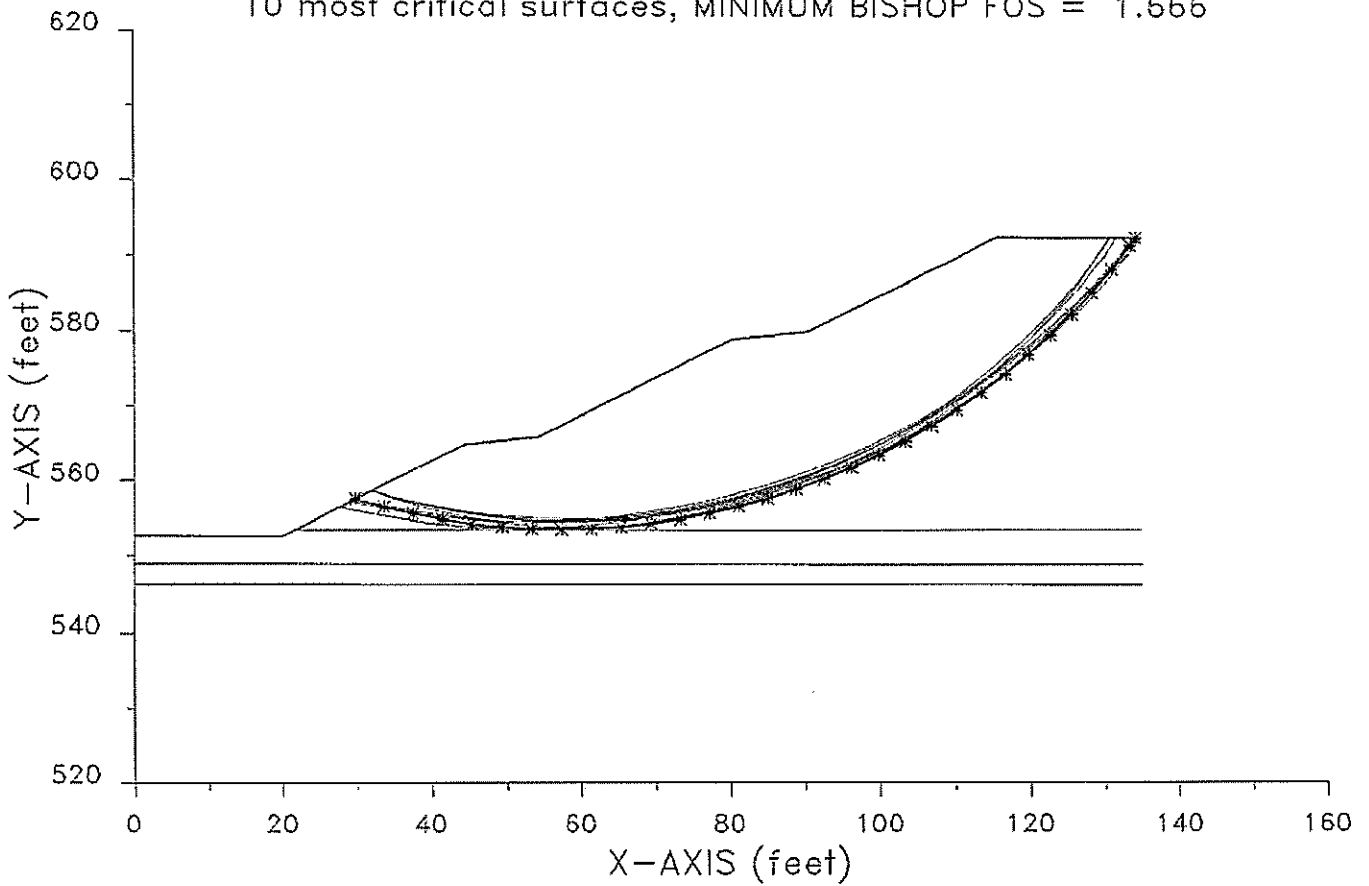
120.0	125.0	1000.0	.00	.000	.0	0
120.0	125.0	1500.0	.00	.000	.0	0
120.0	125.0	.0	37.50	.000	.0	0
120.0	125.0	3700.0	.00	.000	.0	0

CIRCL2

25 50

15.0	32.0	116.0	135.0
532.5	.0	.0	.0

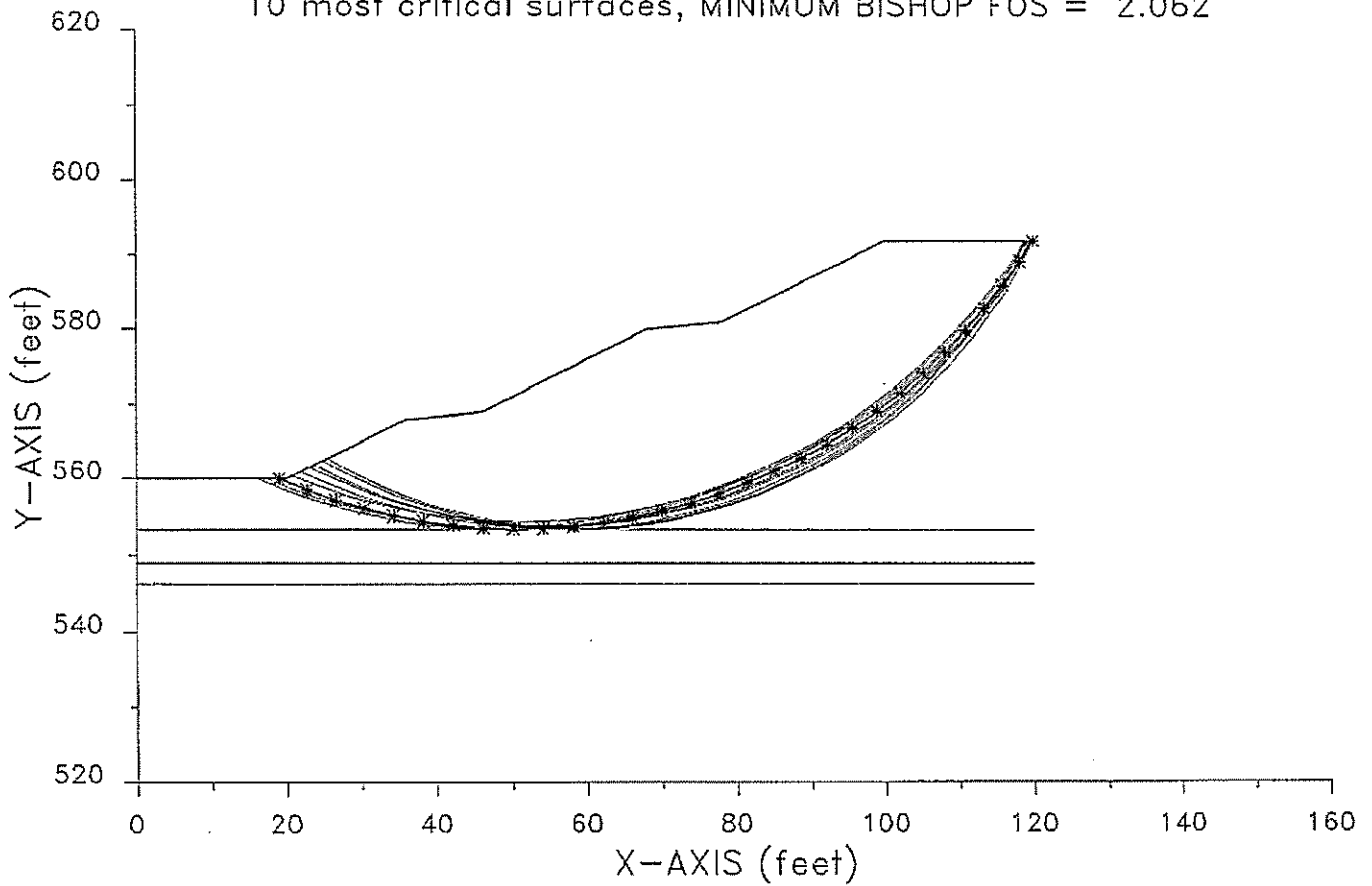
Station 438+00, Left, Seismic
 10 most critical surfaces, MINIMUM BISHOP FOS = 1.666



PROFIL		FILE: 43800LE 11-12-** 11:26 ft				
Station 438+00, Left, Seismic						
11	8					
.0	552.6	20.0	552.6	2		
20.0	552.6	21.8	553.5	2		
21.8	553.5	44.5	565.0	1		
44.5	565.0	54.5	566.0	1		
54.5	566.0	80.5	579.0	1		
80.5	579.0	90.5	580.0	1		
90.5	580.0	115.5	592.5	1		
115.5	592.5	135.0	592.5	1		
21.8	553.5	135.0	553.5	2		
.0	549.0	135.0	549.0	3		
.0	546.5	135.0	546.5	4		
SOIL						
4						
120.0	125.0	1000.0	.00	.000	.0	0
120.0	125.0	1500.0	.00	.000	.0	0
120.0	125.0	.0	37.50	.000	.0	0
120.0	125.0	3700.0	.00	.000	.0	0
EQUAKE						
.085	.000					
CIRCL2						
25	50					
15.0	32.0	116.0	135.0			
532.5	.0	.0	.0			

Station 438+00, Right

10 most critical surfaces, MINIMUM BISHOP FOS = 2.062



PROFIL
Station 438+00, Right

FILE: 43800R 11-12-*** 11:11 ft

10	?				
.0	560.0	20.0	560.0	1	
20.0	560.0	36.0	560.0	1	
35.0	563.0	46.0	569.0	1	
45.0	569.0	60.0	580.0	1	
63.0	580.0	78.0	581.0	1	
73.0	581.0	100.0	592.0	1	
100.0	592.0	120.0	592.0	1	
.0	553.5	120.0	553.5	2	
.0	549.0	120.0	549.0	3	
.0	546.5	120.0	546.5	4	

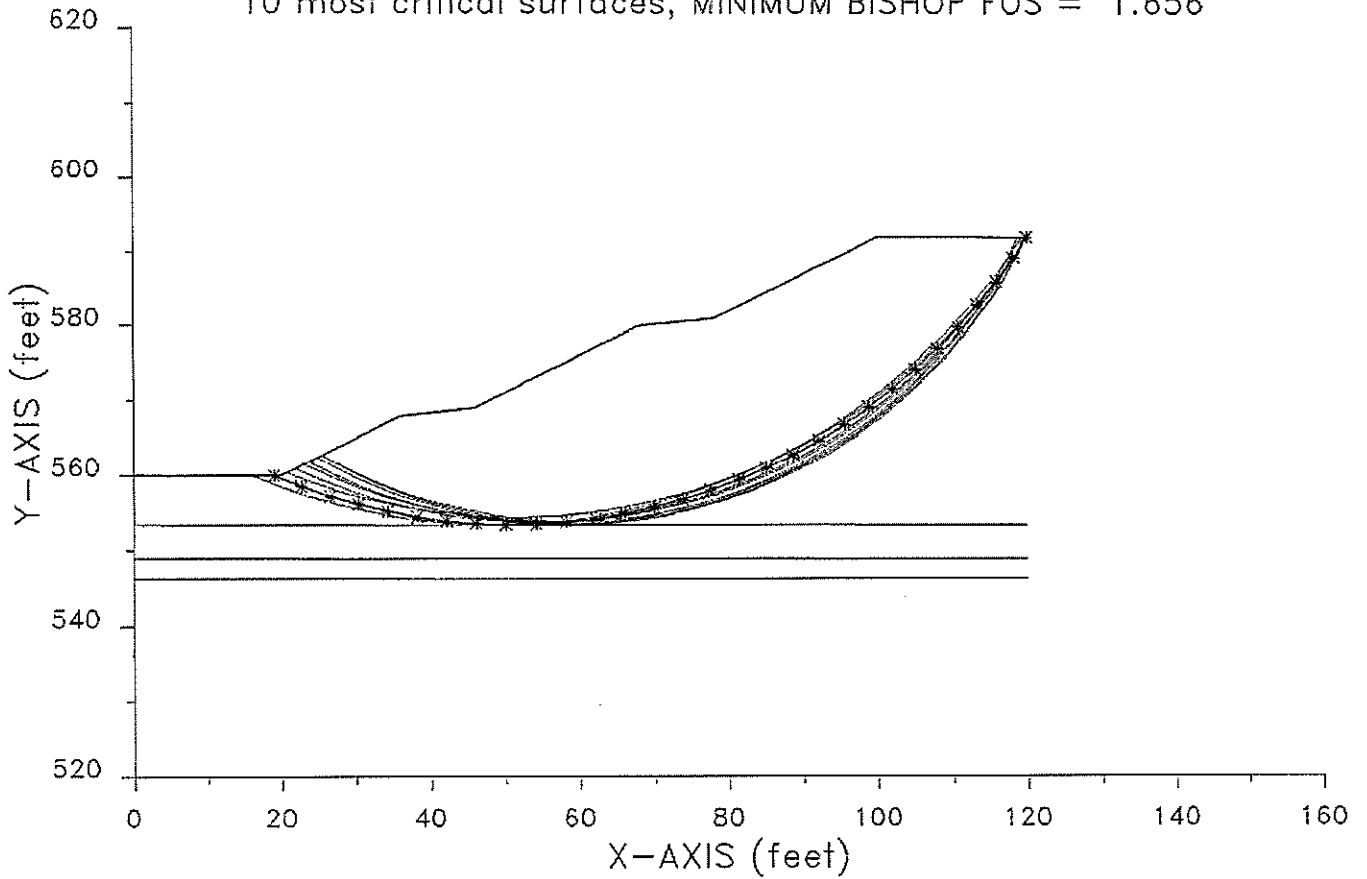
SOIL

4						
120.0	125.0	1000.0	.00	.000	.0	0
120.0	125.0	1500.0	.00	.000	.0	0
120.0	125.0	.0	37.50	.000	.0	0
120.0	125.0	3700.0	.00	.000	.0	0

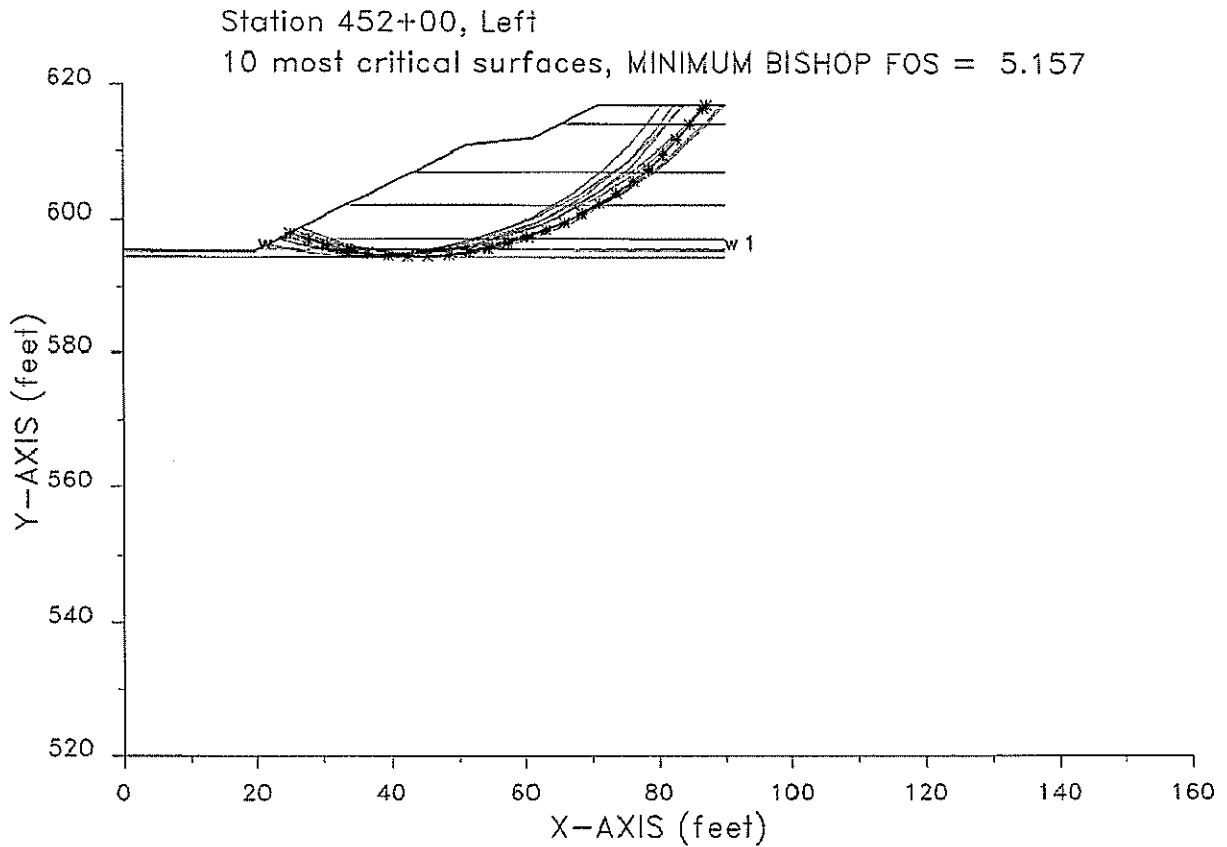
CIRCL2

25	50			
15.0	29.0	101.0	120.0	
532.5	.0	.0	.0	

Station 438+00, Right, Seismic
 10 most critical surfaces, MINIMUM BISHOP FOS = 1.656



PROFIL	FILE: 43800RE	11-12-**	11:11	ft
Station 438+00, Right, Seismic				
10	7			
.0	560.0	20.0	560.0	1
20.0	560.0	36.0	568.0	1
36.0	568.0	46.0	569.0	1
46.0	569.0	68.0	580.0	1
68.0	580.0	78.0	581.0	1
78.0	581.0	100.0	592.0	1
100.0	592.0	120.0	592.0	1
.0	553.5	120.0	553.5	2
.0	549.0	120.0	549.0	3
.0	546.5	120.0	546.5	4
SOIL				
4				
120.0	125.0	1000.0	.00	.000
120.0	125.0	1500.0	.00	.000
120.0	125.0	.0	37.50	.000
120.0	125.0	3700.0	.00	.000
EQUAKE				
.085	.000			
CIRCL2				
25	50			
15.0	29.0	101.0	120.0	
532.5	.0	.0	.0	



PROFIL
Station 452+00, Left
16 10

FILE: 45200L 11-12-** 11:12 ft

.0	595.4	20.0	595.4	6
20.0	595.4	20.4	595.6	6
20.4	595.6	23.4	597.1	6
23.4	597.1	33.4	602.1	4
33.4	602.1	43.4	607.1	3
43.4	607.1	51.2	611.0	2
51.2	611.0	61.2	612.0	2
61.2	612.0	65.4	614.1	2
65.4	614.1	71.2	617.0	1
71.2	617.0	90.0	617.0	1
65.4	614.1	90.0	614.1	2
43.4	607.1	90.0	607.1	3
33.4	602.1	90.0	602.1	4
23.4	597.1	90.0	597.1	5
20.4	595.6	90.0	595.6	6
.0	594.6	90.0	594.6	7

SOIL

7						
120.0	125.0	1030.0	.00	.000	.0	0
120.0	125.0	1850.0	.00	.000	.0	0
120.0	125.0	1350.0	.00	.000	.0	0
120.0	125.0	1630.0	.00	.000	.0	0
120.0	125.0	1530.0	.00	.000	.0	0
120.0	125.0	1530.0	.00	.000	.0	1
120.0	125.0	4530.0	.00	.000	.0	1

WATER

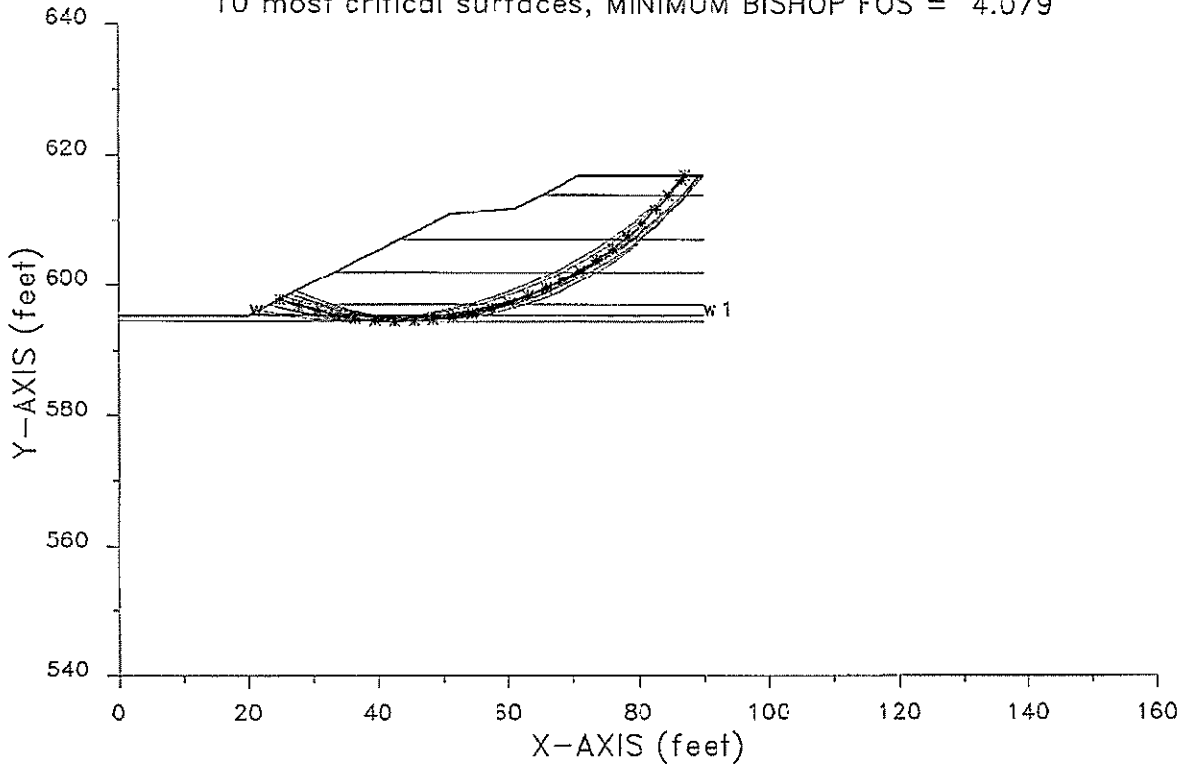
1	62.40					
2						
	20.4	595.6				
	90.0	595.6				

CIRCL2

25	50					
15.0	35.0	72.0	90.0			
593.1	.0	.0	.0			

Station 452+00, Left, Seismic

10 most critical surfaces, MINIMUM BISHOP FOS = 4.079



PROFIL Station 452+00, Left, Seismic FILE: 45200LE 11-12-** 11:13 ft

Station	Y (feet)	X (feet)	Y (feet)	Order
15	595.4	20.0	595.4	6
20.0	595.4	20.4	595.6	6
20.4	595.6	23.4	597.1	5
23.4	597.1	33.4	602.1	4
33.4	602.1	43.4	607.1	3
43.4	607.1	51.2	611.0	2
51.2	611.0	61.2	612.0	2
51.2	612.0	65.4	614.1	2
55.4	614.1	71.2	617.0	1
71.2	617.0	90.0	617.0	1
55.4	614.1	90.0	614.1	2
43.4	607.1	90.0	607.1	3
33.4	602.1	90.0	602.1	4
23.4	597.1	90.0	597.1	5
20.4	595.6	90.0	595.6	6
15	594.6	90.0	594.6	7

SOIL

Station	Y (feet)	Y (feet)	Y (feet)	Y (feet)	Y (feet)	Y (feet)	Y (feet)
7	120.0	125.0	1000.0	.00	.000	.0	0
7	120.0	125.0	1850.0	.00	.000	.0	0
7	120.0	125.0	1350.0	.00	.000	.0	0
7	120.0	125.0	1650.0	.00	.000	.0	0
7	120.0	125.0	1500.0	.00	.000	.0	0
7	120.0	125.0	1500.0	.00	.000	.0	1
7	120.0	125.0	2500.0	.00	.000	.0	1

WATER

Station	Y (feet)	Y (feet)
1	62	40
2	20.4	595.6
2	90.0	595.6

EQUAKE

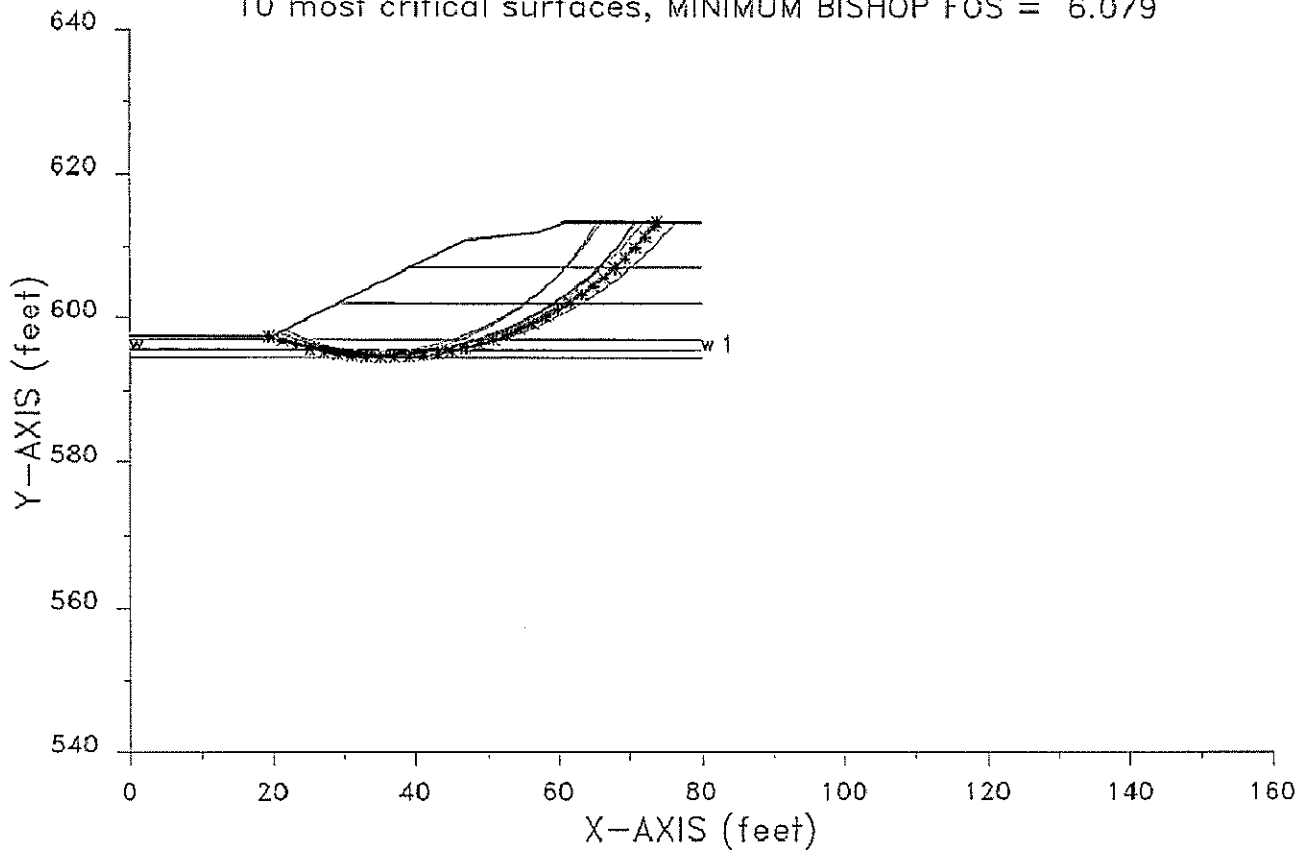
Station	Y (feet)
.085	.000

CIRCL2

Station	Y (feet)	Y (feet)	Y (feet)	Y (feet)	
25	50	15.0	35.0	72.0	90.0
25	50	593.1	.0	.0	.0

Station 452+00, Right

10 most critical surfaces, MINIMUM BISHOP FOS = 6.079



PROFIL
Station 452+00, Right
12 7

FILE: 45200R 11-12-*** 11:27 ft

.0	597.5	20.0	597.5	3
20.0	597.5	29.2	602.1	3
29.2	602.1	39.2	607.1	2
39.2	607.1	47.0	611.0	1
47.0	611.0	57.0	612.0	1
57.0	612.0	61.0	613.5	1
61.0	613.5	80.0	613.5	1
39.2	607.1	80.0	607.1	2
29.2	602.1	80.0	602.1	3
.0	597.1	80.0	597.1	4
.0	595.6	80.0	595.6	5
.0	594.6	80.0	594.6	6

SOIL

6						
120.0	125.0	1850.0	.00	.000	.0	0
120.0	125.0	1350.0	.00	.000	.0	0
120.0	125.0	1650.0	.00	.000	.0	0
120.0	125.0	1500.0	.00	.000	.0	0
120.0	125.0	1500.0	.00	.000	.0	1
120.0	125.0	4500.0	.00	.000	.0	1

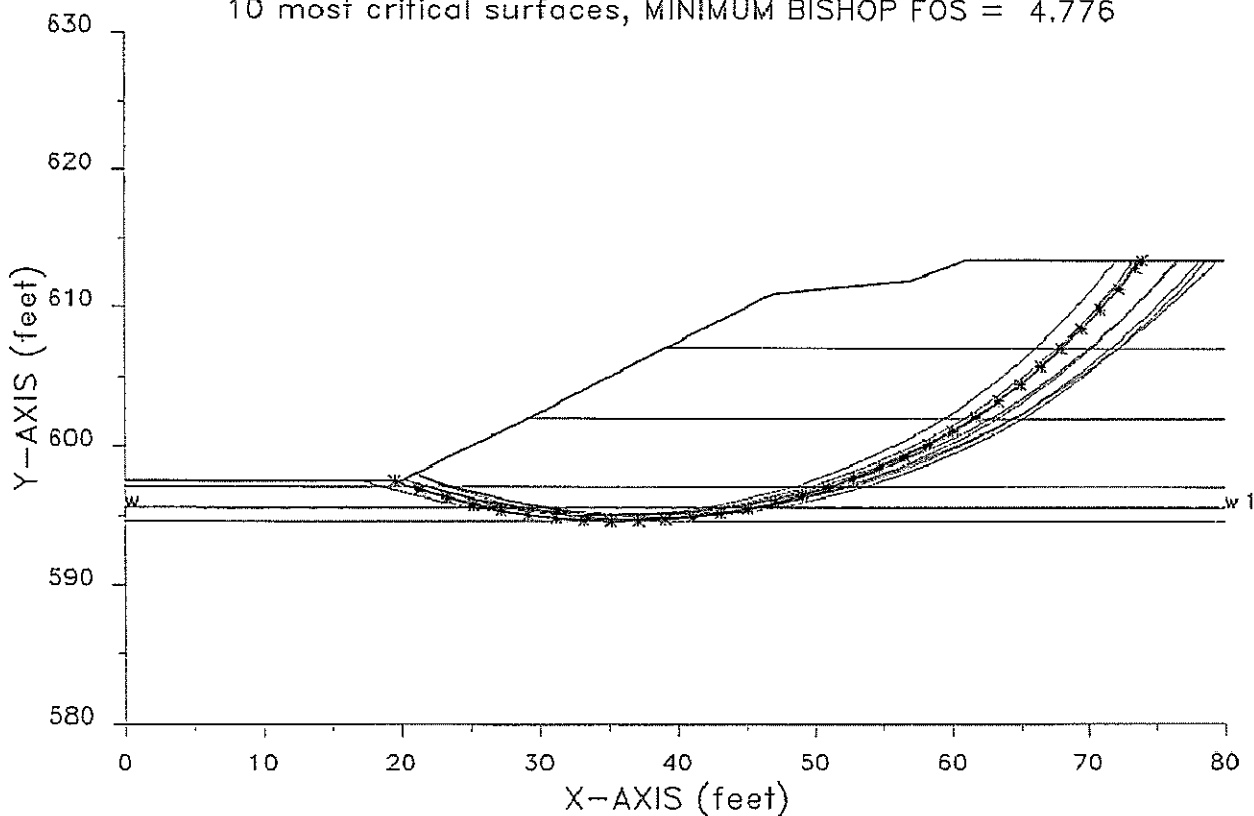
WATER

1	62.40		
2		.0	595.6
		80.0	595.6

CIRCL2

25	50		
15.0	33.0	62.0	80.0
593.1	.0	.0	.0

Station 452+00, Right, Seismic
 10 most critical surfaces, MINIMUM BISHOP FOS = 4.776



PROFIL
 Station 452+00, Right, Seismic

FILE: 45200RE 11-12-** 11:28 ft

12	7				
.0	597.5	20.0	597.5	3	
20.0	597.5	29.2	602.1	3	
29.2	602.1	39.2	607.1	2	
39.2	607.1	47.0	611.0	1	
47.0	611.0	57.0	612.0	1	
57.0	612.0	61.0	613.5	1	
61.0	613.5	80.0	613.5	1	
39.2	607.1	80.0	607.1	2	
29.2	602.1	80.0	602.1	3	
.0	597.1	80.0	597.1	4	
.0	595.6	80.0	595.6	5	
.0	594.6	80.0	594.6	6	

SOIL

6						
120.0	125.0	1850.0	.00	.000	.0	0
120.0	125.0	1350.0	.00	.000	.0	0
120.0	125.0	1650.0	.00	.000	.0	0
120.0	125.0	1500.0	.00	.000	.0	0
120.0	125.0	1500.0	.00	.000	.0	1
120.0	125.0	4500.0	.00	.000	.0	1

WATER

1	2		
1	62.40		
		.0	595.6
		80.0	595.6

EQUAKE

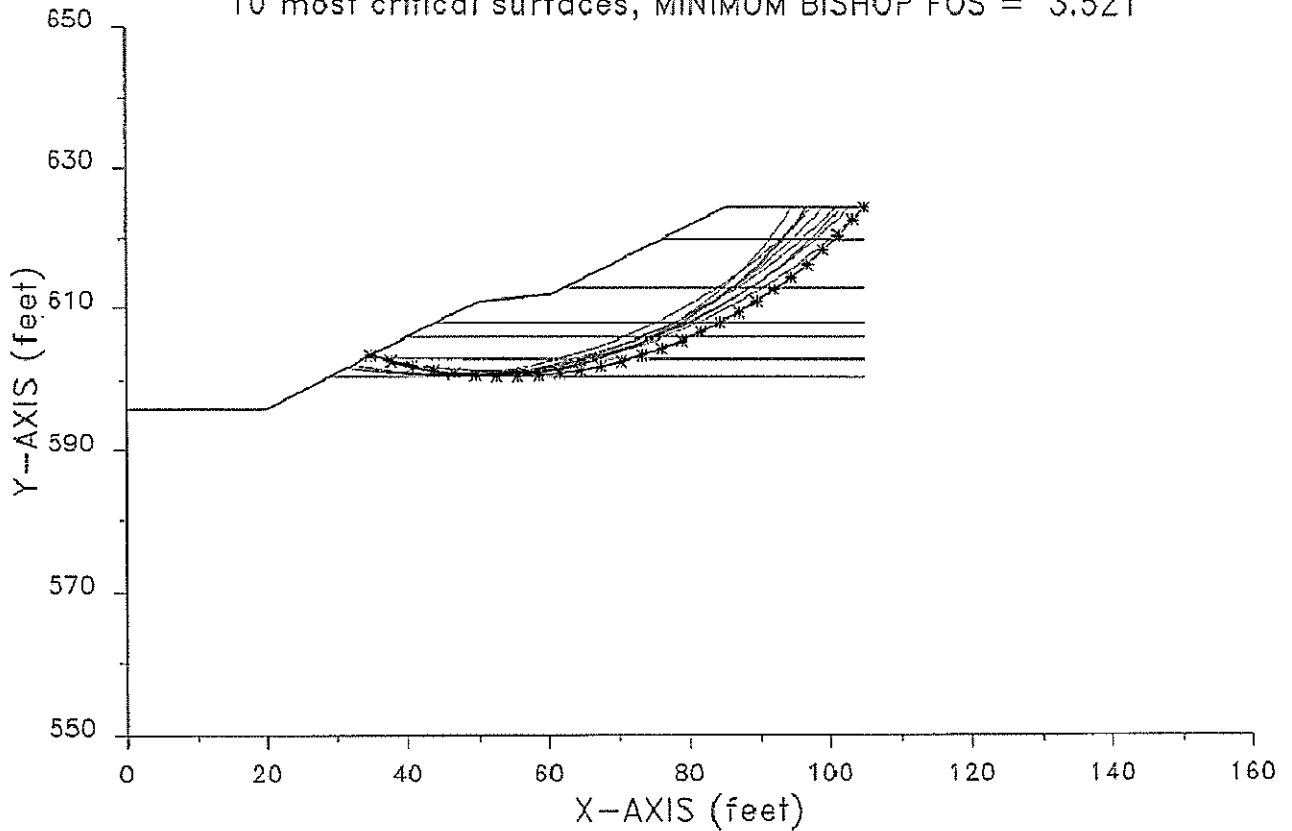
.085	.000
------	------

CIRCL2

25	50				
15.0	33.0	62.0	80.0		
593.1	.0	.0	.0		

Station 454+00, Left

10 most critical surfaces, MINIMUM BISHOP FOS = 3.521



PROFIL
Station 454+00, Left
17 11

FILE: 45400L 11-12-*** 13:24 ft

.0	596.0	20.0	596.0	7
20.0	596.0	29.0	600.5	7
29.0	600.5	34.0	603.0	6
34.0	603.0	40.0	606.0	5
40.0	606.0	44.0	608.0	4
44.0	608.0	50.4	611.0	3
50.4	611.0	60.4	612.0	3
60.4	612.0	62.4	613.0	3
62.4	613.0	76.4	620.0	2
76.4	620.0	85.4	624.5	1
85.4	624.5	105.0	624.5	1
76.4	620.0	105.0	620.0	2
62.4	613.0	105.0	613.0	3
44.0	608.0	105.0	608.0	4
40.0	606.0	105.0	606.0	5
34.0	603.0	105.0	603.0	6
29.0	600.5	105.0	600.5	7

SOIL

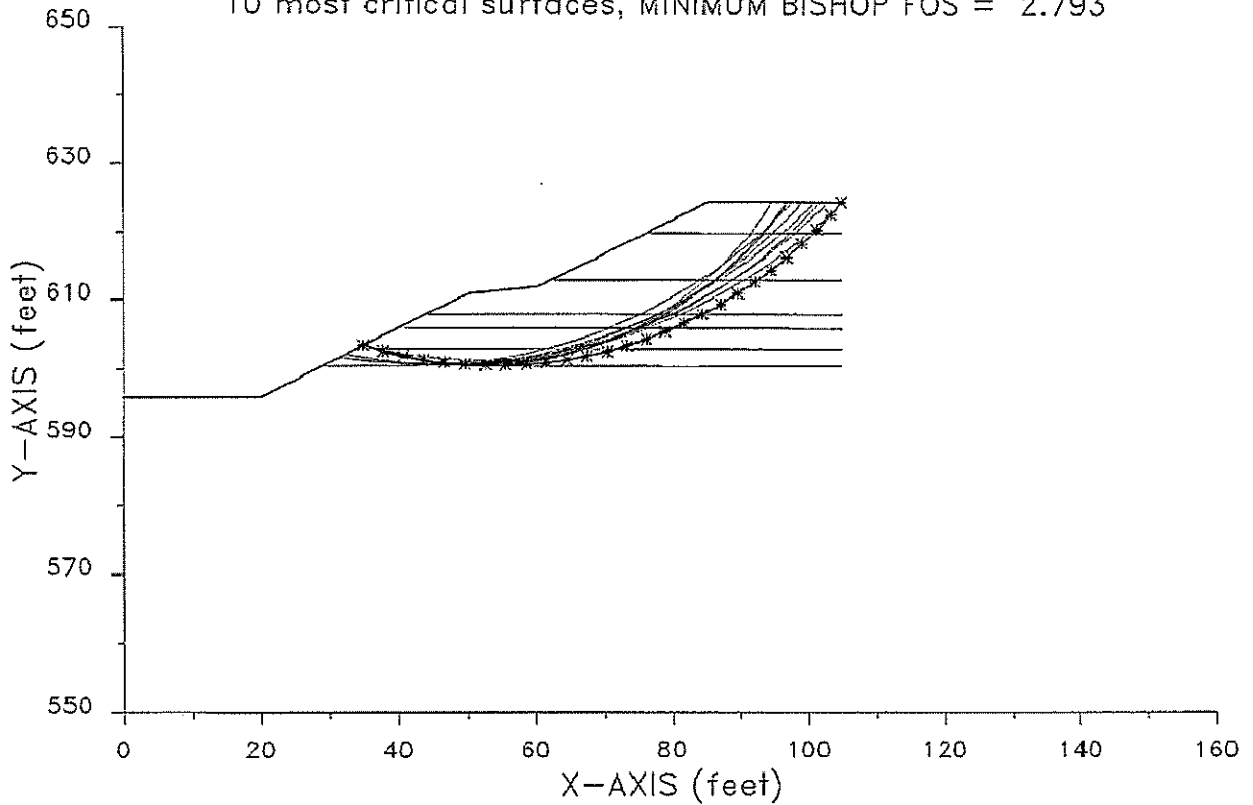
7						
120.0	125.0	1000.0	.00	.000	.0	0
120.0	125.0	1700.0	.00	.000	.0	0
120.0	125.0	1800.0	.00	.000	.0	0
120.0	125.0	2600.0	.00	.000	.0	0
120.0	125.0	700.0	.00	.000	.0	0
120.0	125.0	.0	29.00	.000	.0	0
120.0	125.0	3925.0	.00	.000	.0	0

CIRCL2

25	50				
15.0	35.0	86.0	105.0		
591.5	.0	.0	.0		

Station 454+00, Left, Seismic

10 most critical surfaces, MINIMUM BISHOP FOS = 2.793



PROFIL
Station 454+00, Left, Seismic

FILE: 45400LE 11-12-*** 13:25 ft

17	11				
.0	596.0	20.0	596.0	7	
20.0	596.0	29.0	600.5	7	
29.0	600.5	34.0	603.0	6	
34.0	603.0	40.0	606.0	5	
40.0	606.0	44.0	608.0	4	
44.0	608.0	50.4	611.0	3	
50.4	611.0	60.4	612.0	3	
60.4	612.0	62.4	613.0	3	
62.4	613.0	76.4	620.0	2	
76.4	620.0	85.4	624.5	1	
85.4	624.5	105.0	624.5	1	
76.4	620.0	105.0	620.0	2	
62.4	613.0	105.0	613.0	3	
44.0	608.0	105.0	608.0	4	
40.0	606.0	105.0	606.0	5	
34.0	603.0	105.0	603.0	6	
29.0	600.5	105.0	600.5	7	

SOIL

7						
120.0	125.0	1000.0	.00	.000	.0	0
120.0	125.0	1700.0	.00	.000	.0	0
120.0	125.0	1800.0	.00	.000	.0	0
120.0	125.0	2600.0	.00	.000	.0	0
120.0	125.0	700.0	.00	.000	.0	0
120.0	125.0	.0	29.00	.000	.0	0
120.0	125.0	3925.0	.00	.000	.0	0

EQUAKE

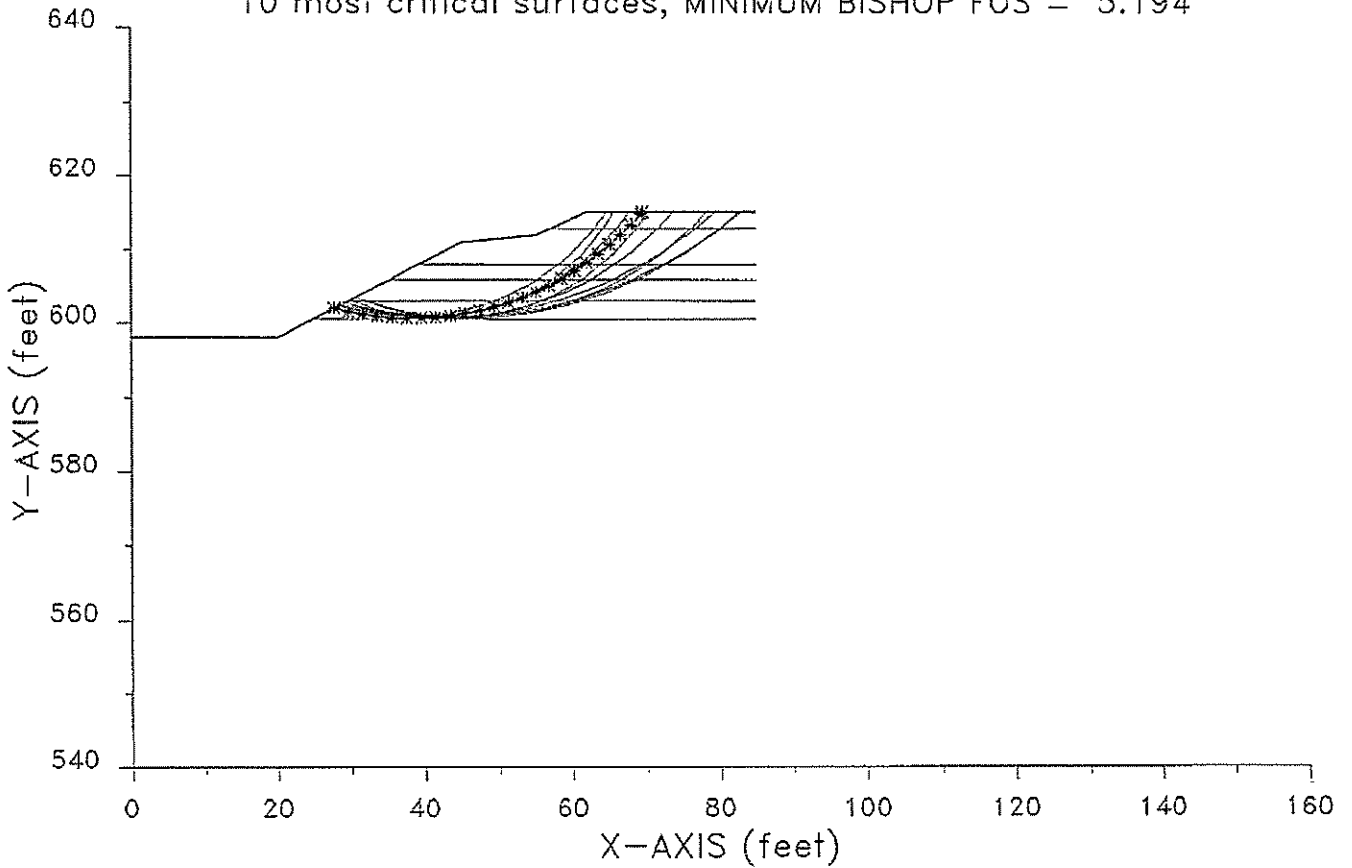
.085 .000

CIRCL2

25	50			
15.0	35.0	86.0	105.0	
391.5	.0	.0	.0	

Station 454+00, Right

10 most critical surfaces, MINIMUM BISHOP FOS = 5.194



PROFIL
Station 454+00, Right

FILE: 45400R 11-12-*** 11:32 ft

Station	Y (feet)	X (feet)	Y (feet)	Y (feet)
15	10			
.0	598.0	20.0	598.0	6
20.0	598.0	24.8	600.5	6
24.8	600.5	29.6	603.0	5
29.6	603.0	35.4	606.0	4
35.4	606.0	39.2	608.0	3
39.2	608.0	45.0	611.0	2
45.0	611.0	55.0	612.0	2
55.0	612.0	57.2	613.0	2
57.2	613.0	62.0	615.2	1
62.0	615.2	85.0	615.2	1
57.2	613.0	85.0	613.0	2
39.2	608.0	85.0	608.0	3
35.4	606.0	85.0	606.0	4
29.6	603.0	85.0	603.0	5
24.8	600.5	85.0	600.5	6

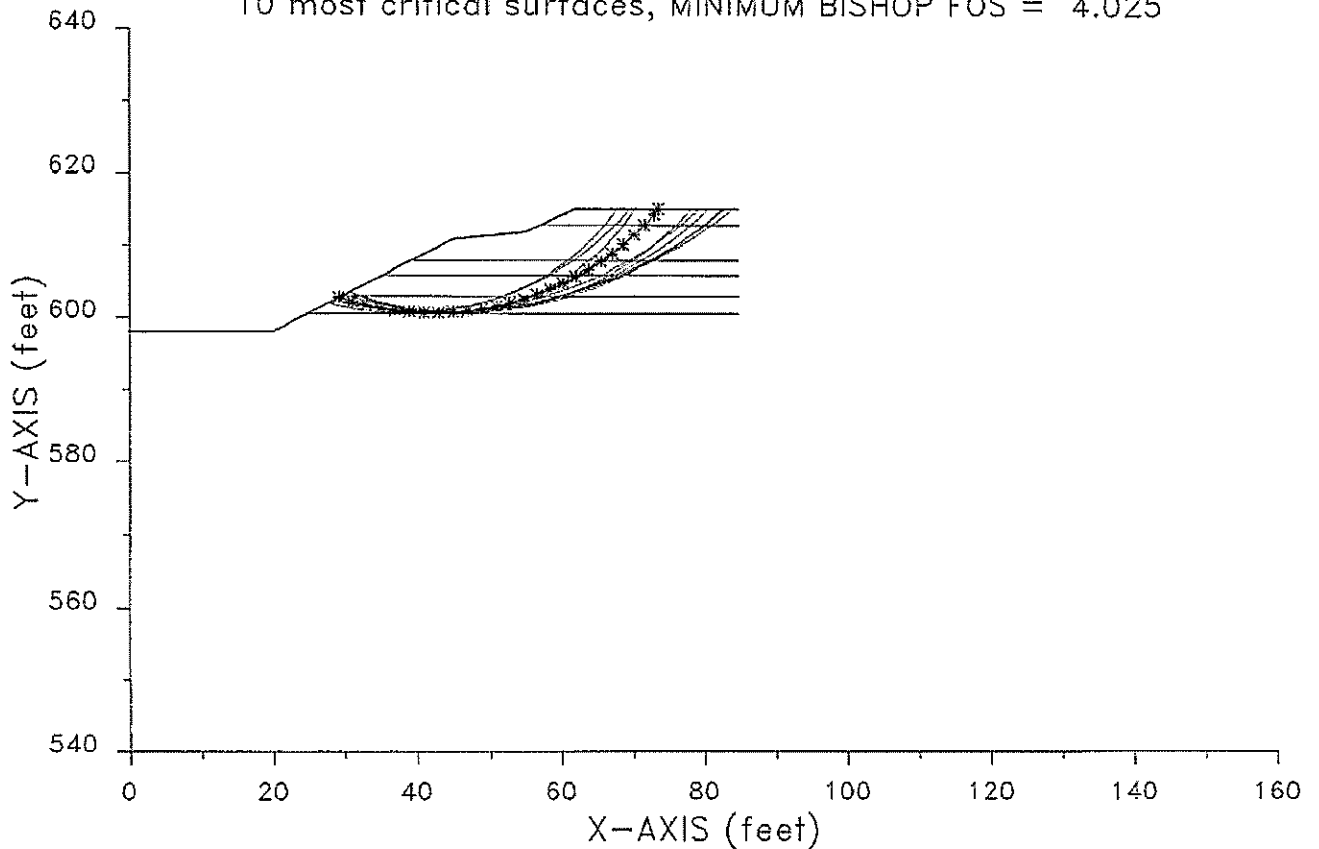
SOIL

Station	Y (feet)	Y (feet)	Y (feet)	Y (feet)	Y (feet)	Y (feet)
6						
120.0	125.0	1700.0	.00	.000	.0	0
120.0	125.0	1800.0	.00	.000	.0	0
120.0	125.0	2600.0	.00	.000	.0	0
120.0	125.0	700.0	.00	.000	.0	0
120.0	125.0	.0	29.00	.000	.0	0
120.0	125.0	3925.0	.00	.000	.0	0

CIRCL2

Station	Y (feet)	Y (feet)	Y (feet)	Y (feet)
25	50			
15.0	33.0	63.0	85.0	
591.5	.0	.0	.0	

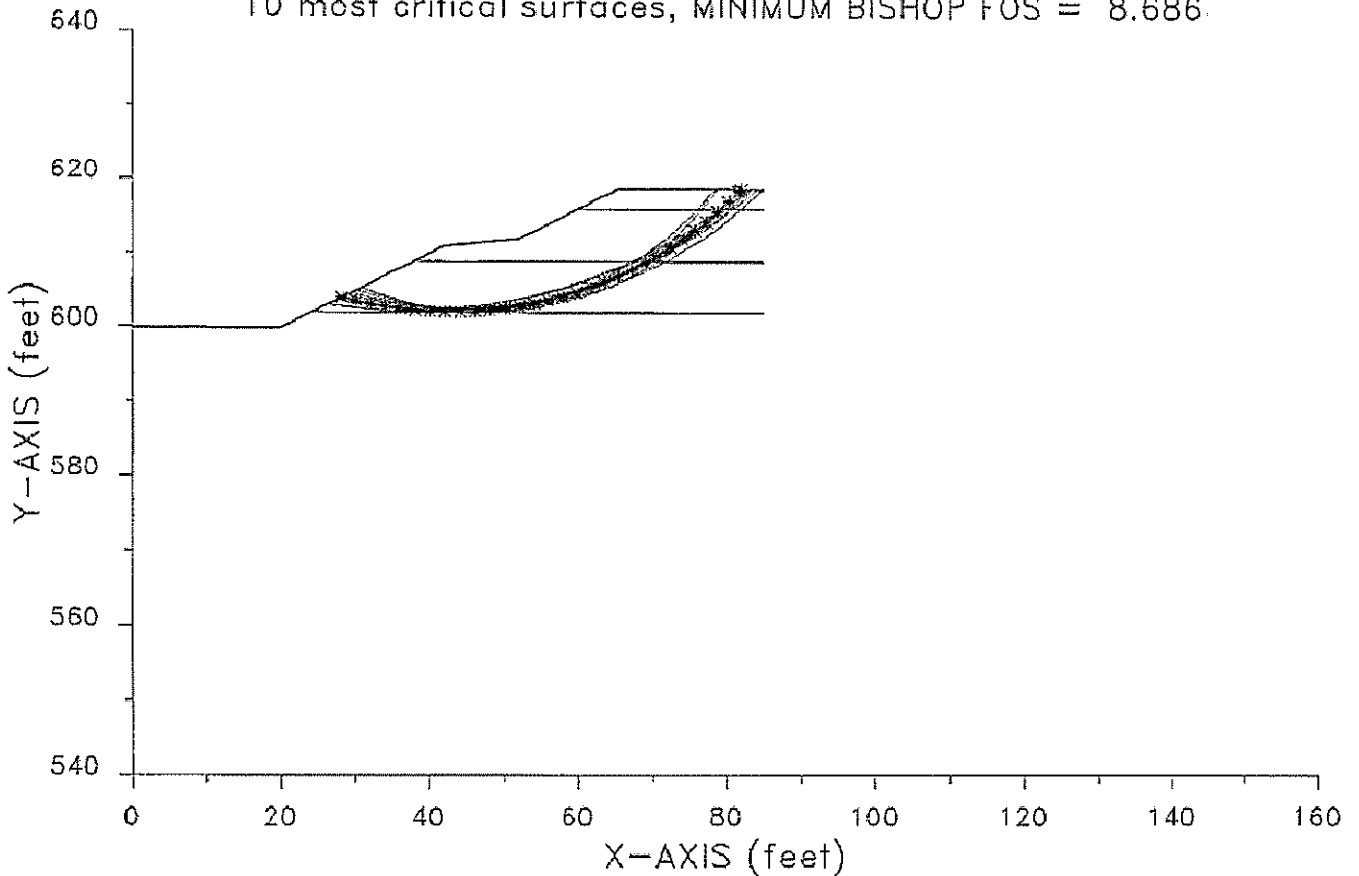
Station 454+00, Right, Seismic
 10 most critical surfaces, MINIMUM BISHOP FOS = 4.025



PROFIL		FILE: 45400RE 11-12-** 11:32 ft				
Station 454+00, Right, Seismic						
15	10					
.0	598.0	20.0	598.0	6		
20.0	598.0	24.8	600.5	6		
24.8	600.5	29.6	603.0	5		
29.6	603.0	35.4	606.0	4		
35.4	606.0	39.2	608.0	3		
39.2	608.0	45.0	611.0	2		
45.0	611.0	55.0	612.0	2		
55.0	612.0	57.2	613.0	2		
57.2	613.0	62.0	615.2	1		
62.0	615.2	85.0	615.2	1		
57.2	613.0	85.0	613.0	2		
39.2	608.0	85.0	608.0	3		
35.4	606.0	85.0	606.0	4		
29.6	603.0	85.0	603.0	5		
24.8	600.5	85.0	600.5	6		
SOIL						
6						
120.0	125.0	1700.0	.00	.000	.0	0
120.0	125.0	1800.0	.00	.000	.0	0
120.0	125.0	2600.0	.00	.000	.0	0
120.0	125.0	700.0	.00	.000	.0	0
120.0	125.0	.0	29.00	.000	.0	0
120.0	125.0	3925.0	.00	.000	.0	0
EQUAKE						
.085	.000					
CIRCL2						
25	50					
15.0	33.0	63.0	85.0			
591.5	.0	.0	.0			

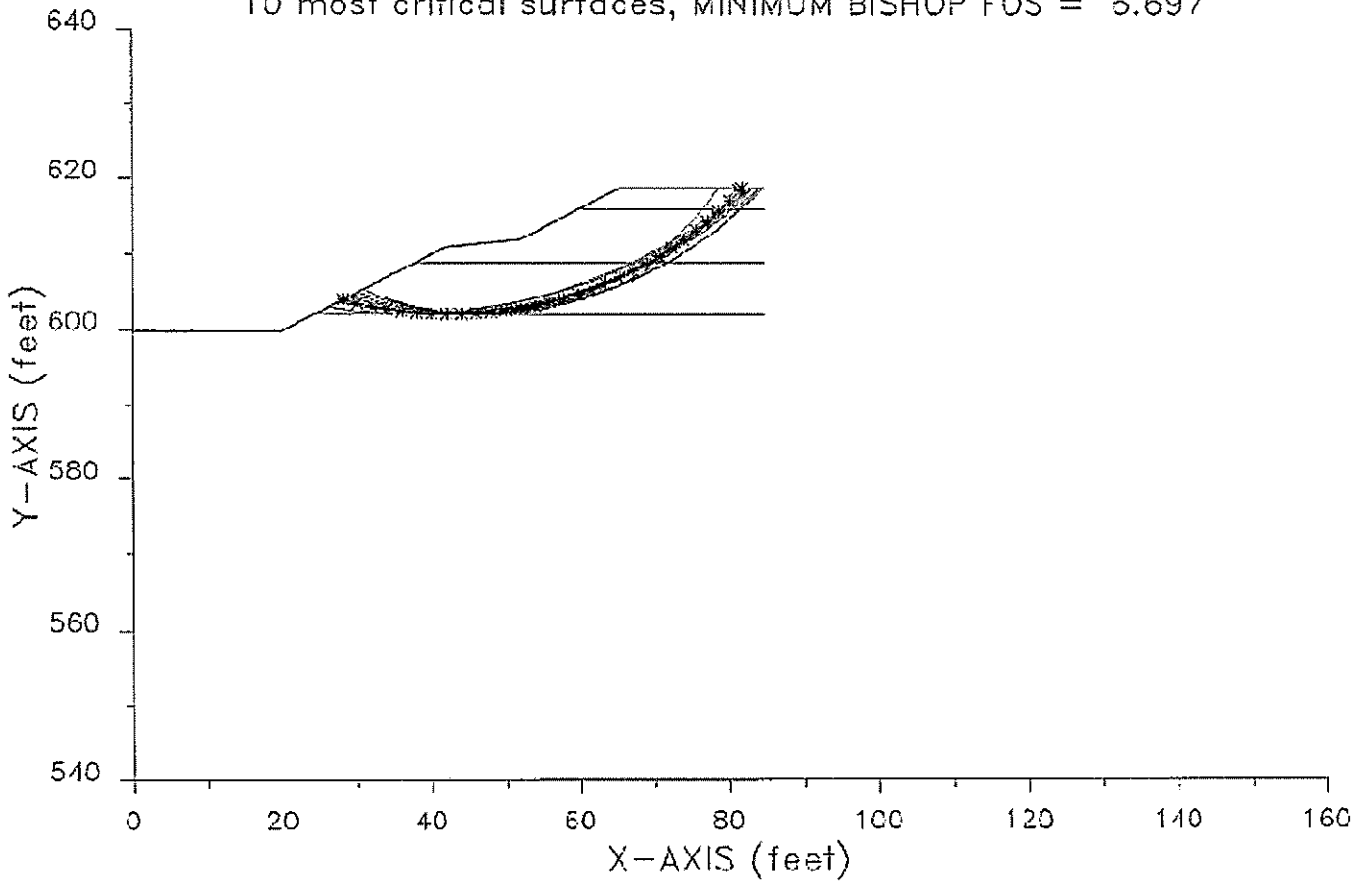
Station 456+00, Left

10 most critical surfaces, MINIMUM BISHOP FOS = 8.686



PROFIL				FILE: 45600L	11-12-***	13:27	ft
Station 456+00, Left							
11	8						
.0	599.8	20.0	599.8	4			
20.0	599.8	24.3	602.0	4			
24.3	602.0	38.1	609.0	3			
38.1	609.0	42.0	611.0	2			
42.0	611.0	52.0	612.0	2			
52.0	612.0	60.0	616.0	2			
60.0	616.0	65.4	618.7	1			
65.4	618.7	85.0	618.7	1			
60.0	616.0	85.0	616.0	2			
38.1	609.0	85.0	609.0	3			
24.3	602.0	85.0	602.0	4			
SOIL							
4							
120.0	125.0	1000.0	.00	.000	.0	0	
120.0	125.0	1250.0	.00	.000	.0	0	
120.0	125.0	2033.0	.00	.000	.0	0	
120.0	125.0	4500.0	.00	.000	.0	0	
CIRCL2							
25	50						
19.0	31.0	66.0	85.0				
595.0	.0	.0	.0				

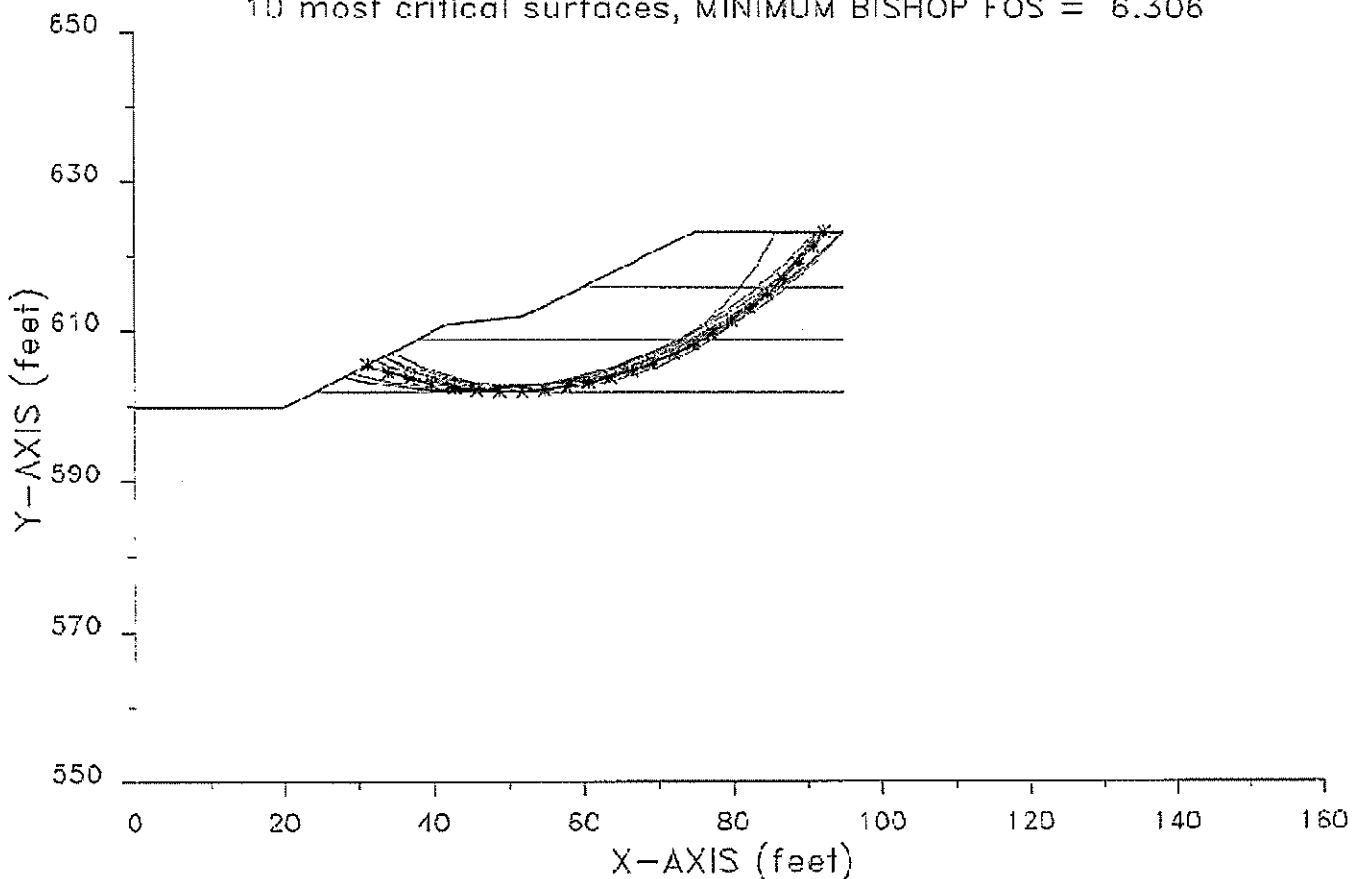
Station 456+00, Left, Seismic
 10 most critical surfaces, MINIMUM BISHOP FOS = 5.697



PROFIL		FILE: 45600LE 11-12-** 13:28 ft			
Station 456+00, Left, Seismic					
11 8					
.0	599.8	20.0	599.8	4	
20.0	599.8	24.3	602.0	4	
24.3	602.0	38.1	609.0	3	
38.1	609.0	42.0	611.0	2	
42.0	611.0	52.0	612.0	2	
52.0	612.0	60.0	616.0	2	
60.0	616.0	65.4	618.7	1	
65.4	618.7	85.0	618.7	1	
60.0	616.0	85.0	616.0	2	
38.1	609.0	85.0	609.0	3	
24.3	602.0	85.0	602.0	4	
SOIL					
4					
120.0	125.0	1000.0	.00	.000	.0 0
120.0	125.0	1250.0	.00	.000	.0 0
120.0	125.0	2033.0	.00	.000	.0 0
120.0	125.0	4500.0	.00	.000	.0 0
EQUAKE					
.085	.000				
CIRCL2					
25 50					
15.0	31.0	66.0	85.0		
595.0	.0	.0	.0		

Station 456+00, Right

10 most critical surfaces, MINIMUM BISHOP FOS = 6.306



PROFIL
Station 456+00, Right

FILE: 45600R 11-12-** 11:07 ft

11		8			
.0	599.8	20.0	599.8	4	
20.0	599.8	24.3	602.0	4	
24.3	602.0	38.1	609.0	3	
38.1	609.0	42.0	611.0	2	
42.0	611.0	52.0	612.0	2	
52.0	612.0	60.0	616.0	2	
60.0	616.0	75.0	623.5	1	
75.0	623.5	95.0	623.5	1	
60.0	616.0	95.0	616.0	2	
38.1	609.0	95.0	609.0	3	
24.3	602.0	95.0	602.0	4	

SOIL

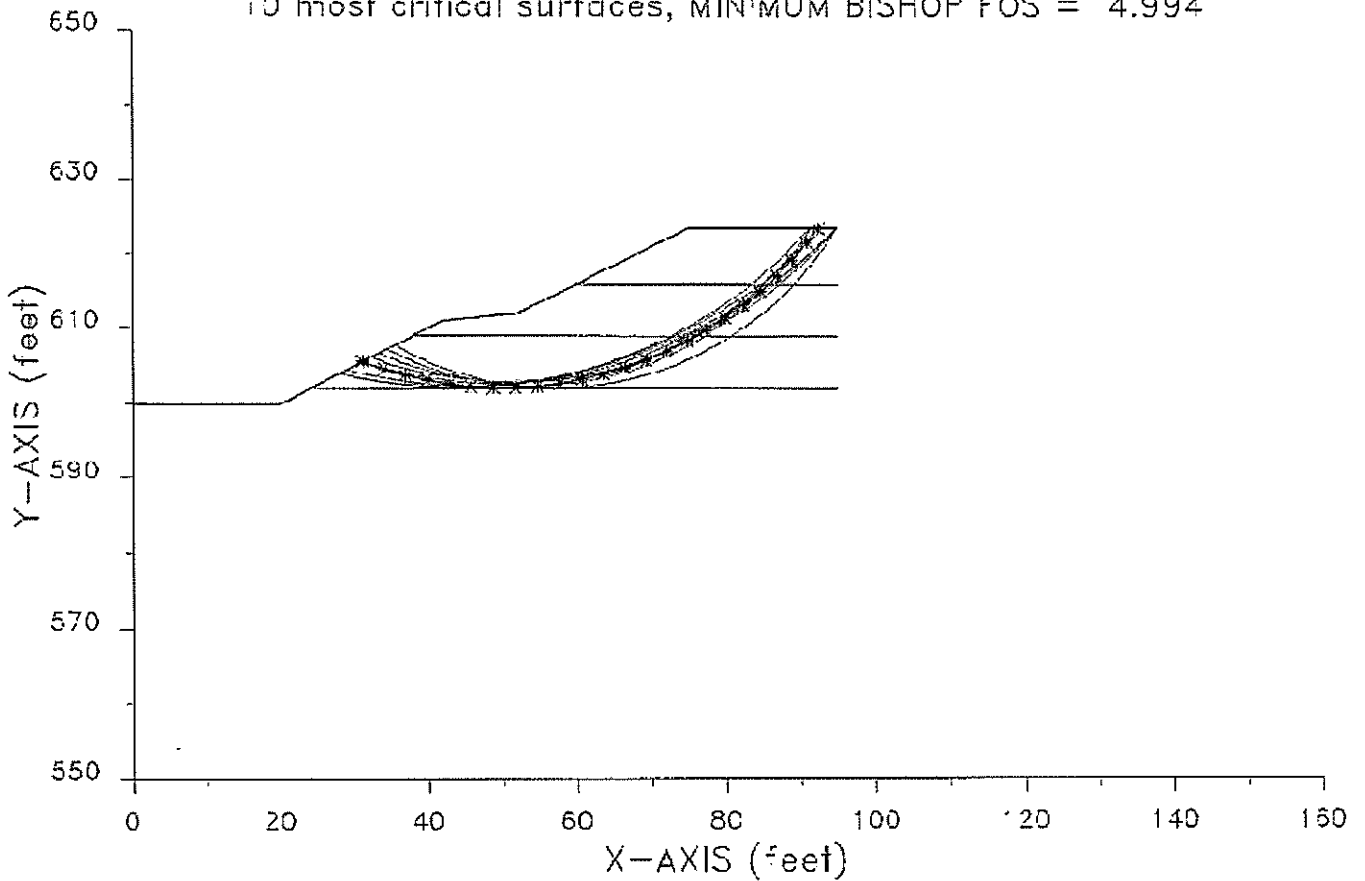
4							
120.0	125.0	1000.0	.00	.000	.0	0	
120.0	125.0	1250.0	.00	.000	.0	0	
120.0	125.0	2033.0	.00	.000	.0	0	
120.0	125.0	4500.0	.00	.000	.0	0	

CIRCL2

25		50			
15.0	41.0	76.0	95.0		
595.0	.0	.0	.0		

Station 456+00, Right, Seismic

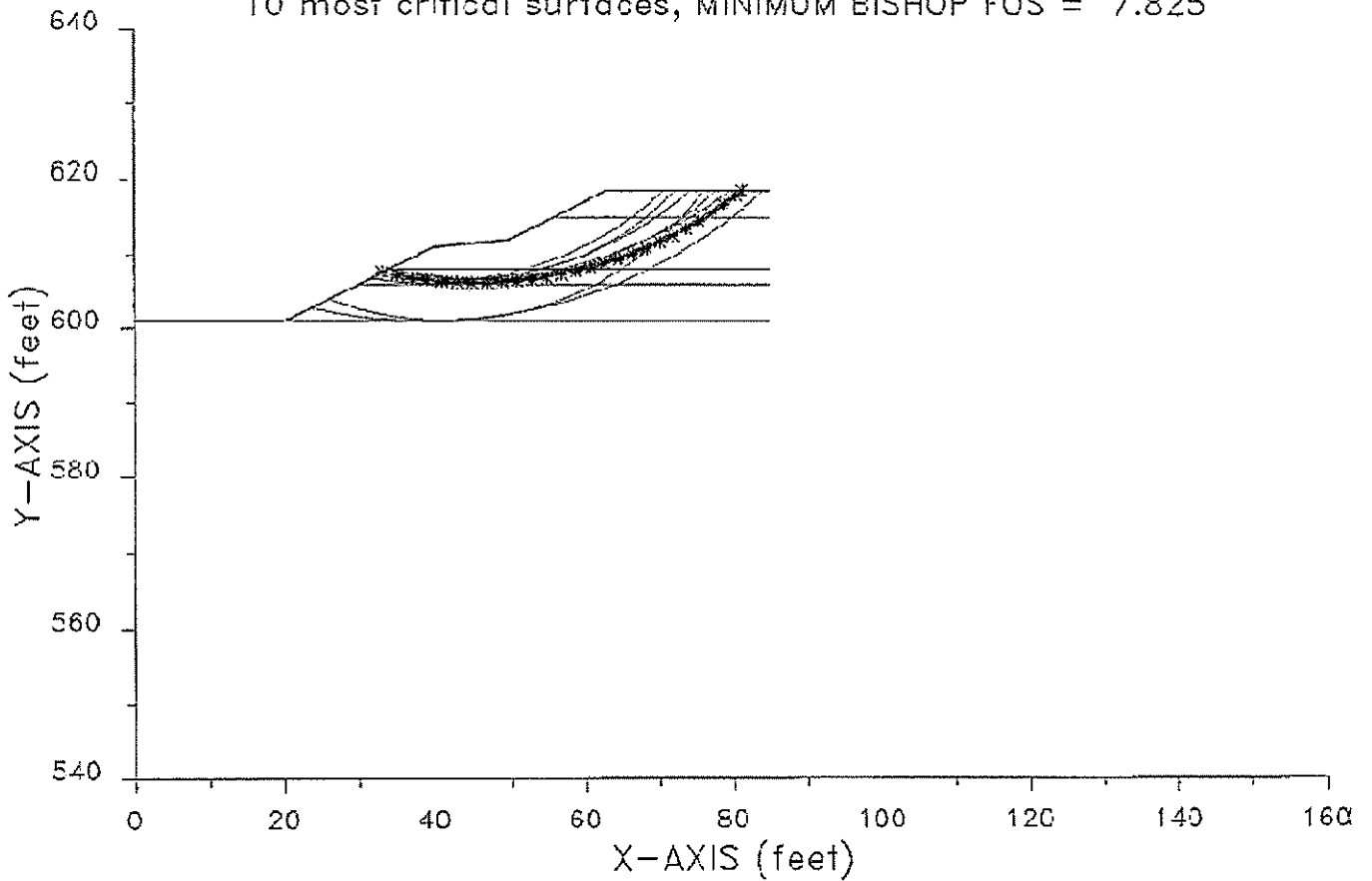
10 most critical surfaces, MINIMUM BISHOP FOS = 4.994



PROFIL		FILE: 45600RE		11-12-++		11:08		ft	
Station 456+00, Right, Seismic									
11	8								
.0	599.8	20.0	599.8	4					
20.0	599.8	24.3	602.0	4					
24.3	602.0	38.1	609.0	3					
38.1	609.0	42.0	611.0	2					
42.0	611.0	52.0	612.0	2					
52.0	612.0	60.0	616.0	2					
60.0	616.0	75.0	623.5	1					
75.0	623.5	95.0	623.5	1					
60.0	616.0	95.0	616.0	2					
38.1	609.0	95.0	609.0	3					
24.3	602.0	95.0	602.0	4					
SOIL									
4									
120.0	125.0	1000.0	.00	.000	.0	0			
120.0	125.0	1250.0	.00	.000	.0	0			
120.0	125.0	2033.0	.00	.000	.0	0			
120.0	125.0	4500.0	.00	.000	.0	0			
EQUAKE									
.085	.000								
CIRCL2									
25	50								
15.0	41.0	76.0	95.0						
595.0	.0	.0	.0						

Station 458+00

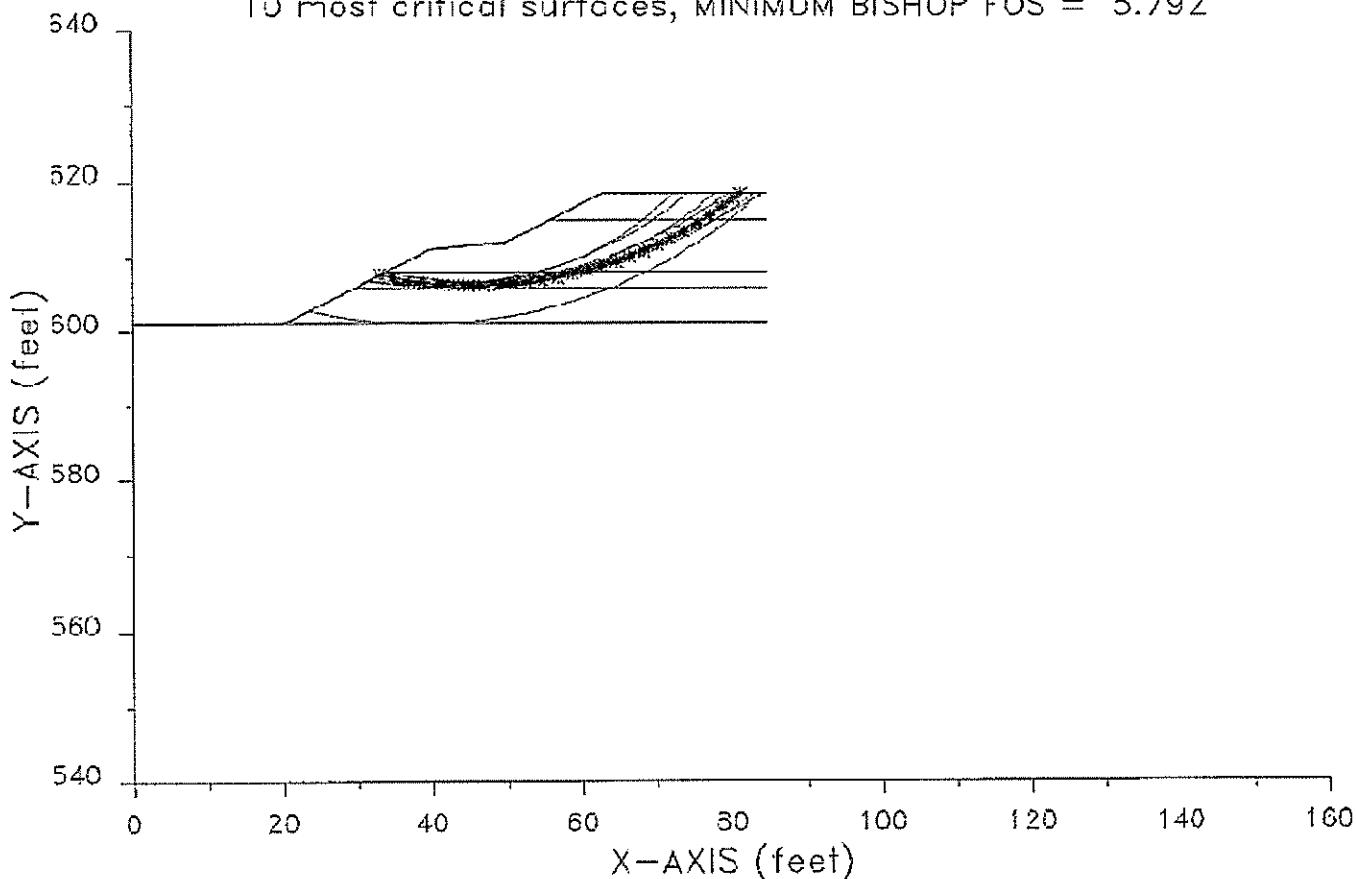
10 most critical surfaces, MINIMUM BISHOP FOS = 7.825



PROFIL	FILE: 45800	11-12-88	11:09	ft		
Station 458+00						
13 9						
.0	600.9	20.0	600.9	5		
20.0	600.9	20.2	601.0	5		
20.2	601.0	30.1	605.0	4		
30.1	605.0	34.0	608.0	3		
34.0	608.0	40.0	611.0	2		
40.0	611.0	50.0	612.0	2		
50.0	612.0	55.0	615.0	2		
55.0	615.0	63.0	618.5	1		
63.0	618.5	85.0	618.5	1		
56.0	615.0	85.0	615.0	2		
34.0	608.0	85.0	608.0	3		
30.1	605.0	85.0	605.0	4		
20.2	601.0	85.0	601.0	5		
SOIL						
5						
120.0	125.0	1000.0	.00	.000	.0	0
120.0	125.0	1900.0	.00	.000	.0	0
120.0	125.0	600.0	.00	.000	.0	0
120.0	125.0	2200.0	.00	.000	.0	0
120.0	125.0	4050.0	.00	.000	.0	0
CIRCLE2						
25 50						
15.0	34.0	64.0	85.0			
596.5	.0	.0	.0			

Station 458+00, Seismic

10 most critical surfaces, MINIMUM BISHOP FOS = 5.792



PROFIL

Station 458+00, Seismic

FILE: 45800E

11-12-**

11:09

ft

13	9			
.0	600.9	20.0	600.9	5
20.0	600.9	20.2	601.0	5
20.2	601.0	30.1	606.0	4
30.1	605.0	34.0	608.0	3
34.0	608.0	40.0	611.0	2
40.0	611.0	50.0	612.0	2
50.0	612.0	56.0	615.0	2
56.0	615.0	63.0	618.5	1
63.0	618.5	85.0	618.5	1
56.0	615.0	85.0	615.0	2
34.0	608.0	85.0	608.0	3
30.1	606.0	85.0	606.0	4
20.2	601.0	85.0	601.0	5

SOIL

5						
120.0	125.0	1000.0	.00	.000	.0	0
120.0	125.0	1900.0	.00	.000	.0	0
120.0	125.0	600.0	.00	.000	.0	0
120.0	125.0	2200.0	.00	.000	.0	0
120.0	125.0	4050.0	.00	.000	.0	0

EQUAKE

.085 .000

CIRCL2

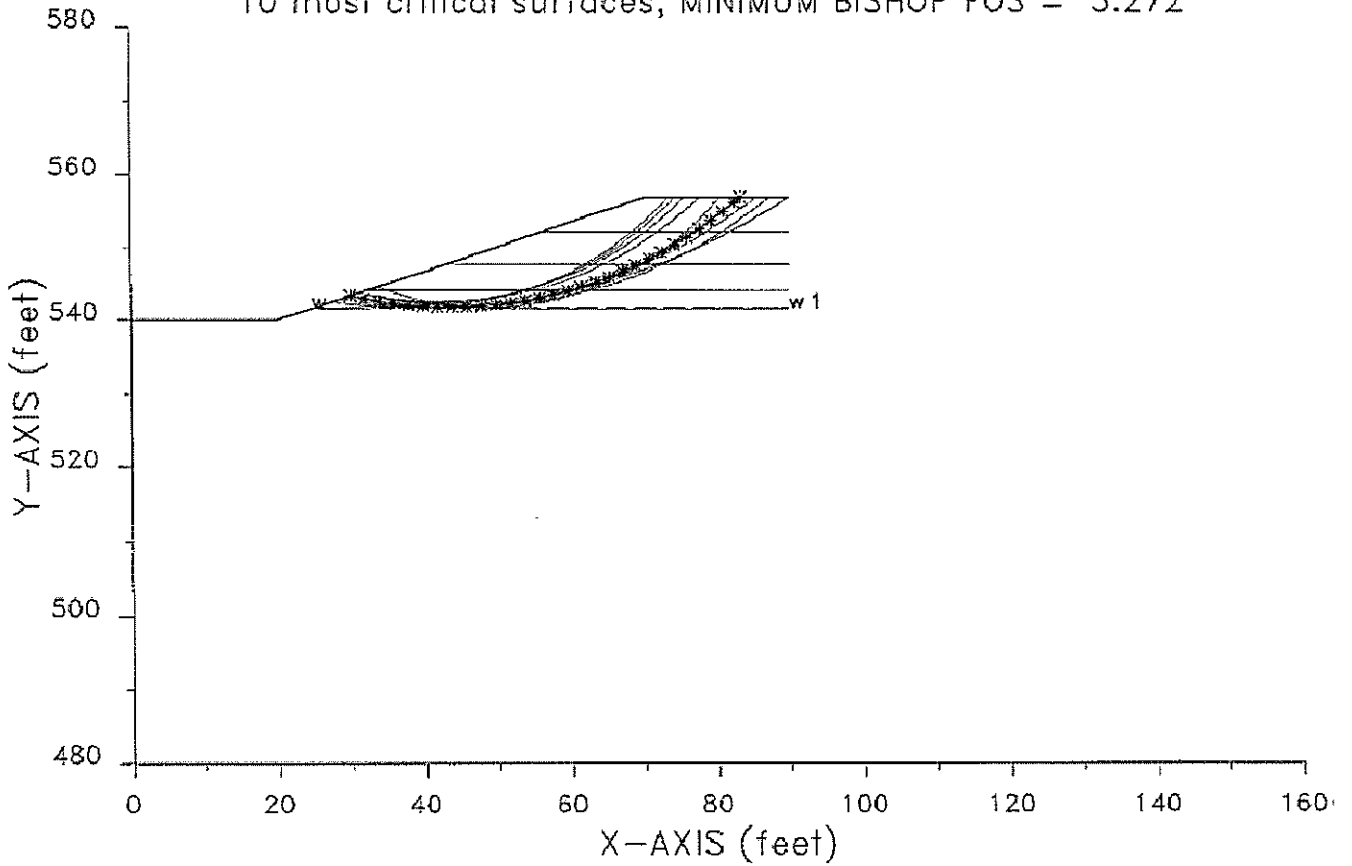
25	50			
15.0	34.0	64.0	85.0	
596.5	.0	.0	.0	

Slope Stability Analyses

US 67 Connector

Connector, Station 26+64

10 most critical surfaces, MINIMUM BISHOP FOS = 3.272



PROFIL
Connector, Station 26+64

FILE: 2664

11-13-**

13:23

ft

11	7				
.0	540.0	20.0	540.0	5	
20.0	540.0	24.8	541.6	5	
24.8	541.6	32.3	544.1	4	
32.3	544.1	42.8	547.6	3	
42.8	547.6	56.3	552.1	2	
56.3	552.1	70.3	556.8	1	
70.3	556.8	90.0	556.8	1	
56.3	552.1	90.0	552.1	2	
42.8	547.6	90.0	547.6	3	
32.3	544.1	90.0	544.1	4	
24.8	541.6	90.0	541.6	5	

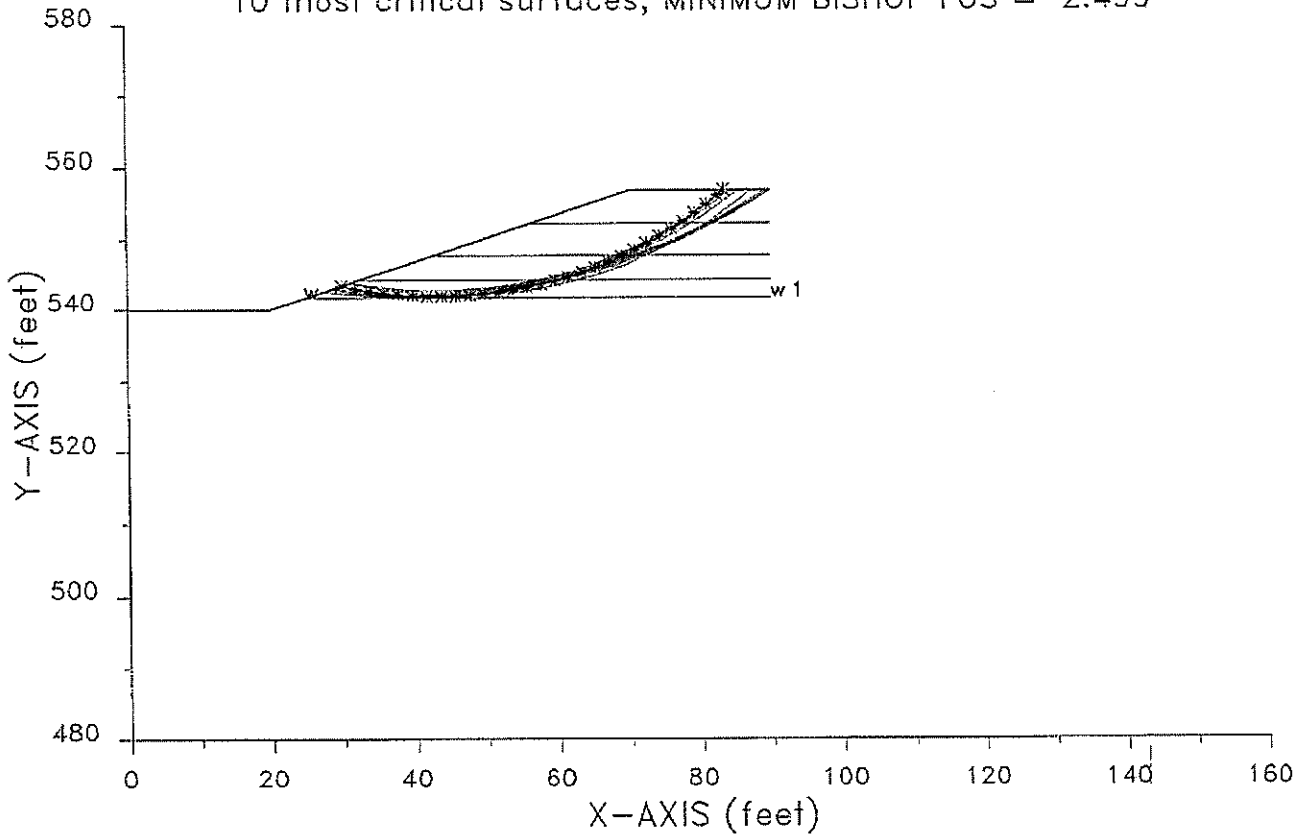
SOIL							
5							
120.0	125.0	1000.0	.00	.000	.0	0	
120.0	125.0	250.0	.00	.000	.0	0	
120.0	125.0	800.0	.00	.000	.0	0	
120.0	125.0	.0	34.00	.000	.0	0	
120.0	125.0	5300.0	.00	.000	.0	1	

WATER	
1	62.40
2	

34.8	541.6
90.0	541.6

CIRCL2				
25	50			
15.0	45.0	71.0	90.0	
536.1	.0	.0	.0	

Connector, Station 26+64, Seismic
 10 most critical surfaces, MINIMUM BISHOP FOS = 2.499



PROFIL
 Connector, Station 26+64, Seismic
 11 7

FILE: 2664E 11-13-** 13:24 ft

.0	540.0	20.0	540.0	5
20.0	540.0	24.8	541.6	5
24.8	541.6	32.3	544.1	4
32.3	544.1	42.8	547.6	3
42.8	547.6	56.3	552.1	2
56.3	552.1	70.3	556.8	1
70.3	556.8	90.0	556.8	1
56.3	552.1	90.0	552.1	2
42.8	547.6	90.0	547.6	3
32.3	544.1	90.0	544.1	4
24.8	541.6	90.0	541.6	5

SOIL

5						
120.0	125.0	1000.0	.00	.000	.0	0
120.0	125.0	250.0	.00	.000	.0	0
120.0	125.0	800.0	.00	.000	.0	0
120.0	125.0	.0	34.00	.000	.0	0
120.0	125.0	5300.0	.00	.000	.0	1

WATER

1	62.40		
2			
	24.8	541.6	
	90.0	541.6	

EQUAKE

.085 .000

CIRCL2

25	50			
15.0	45.0	71.0	90.0	
536.1	.0	.0	.0	

Appendix F
Grid Reinforced Slope

Grid Reinforced Embankment Profile
Sec. 42-2

Sta. 351+00 RT.



1.0 Ft.
VERTICAL
SPACING

9.0 Ft.

2:1

1.0 Ft.
COVER

EXISTING
GROUND LINE

0 10 20 30 40 50 60 70 80 90 100 110 120 130 140

505
500
505
500
505
500

F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
310	42-2	JERSEY	393	178
STA.		TO STA.		
FED. ROAD DIST. NO.	ILLINOIS	FED. AID PROJECT		

BY: _____ DATE: _____

SURVEYED: _____ PLOTTED: _____

NOTE BOOK: _____ AREAS CHECKED: _____

NO.: _____

POWER I. CHASTAIN & ASSOCIATES LLP

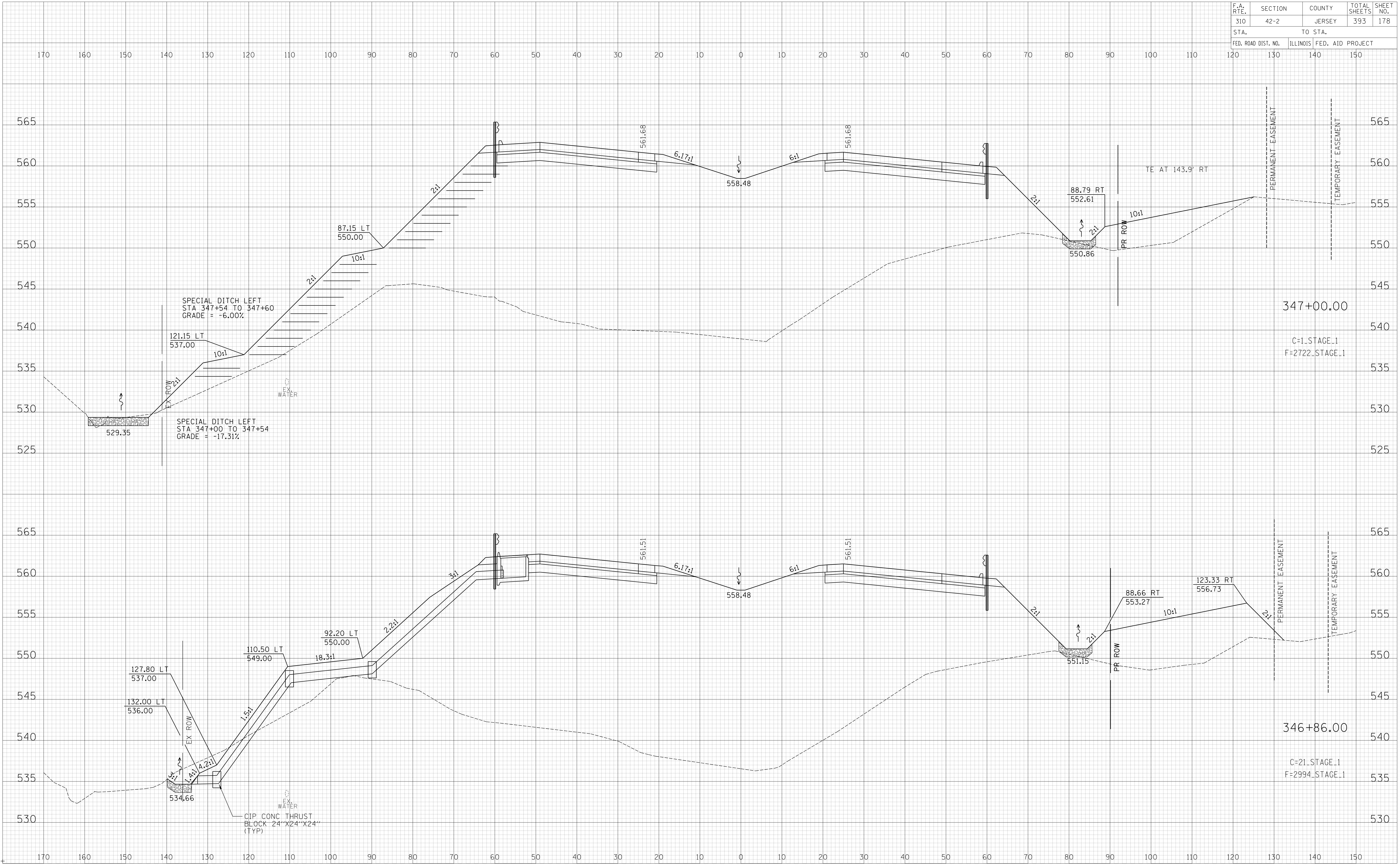
300 N. LAUREL STREET, SUITE 200
CHICAGO, ILLINOIS 60610
TEL: 312.467.8800
FAX: 312.467.8801
WWW.CHASTAIN-PA.COM

BY: _____ DATE: _____

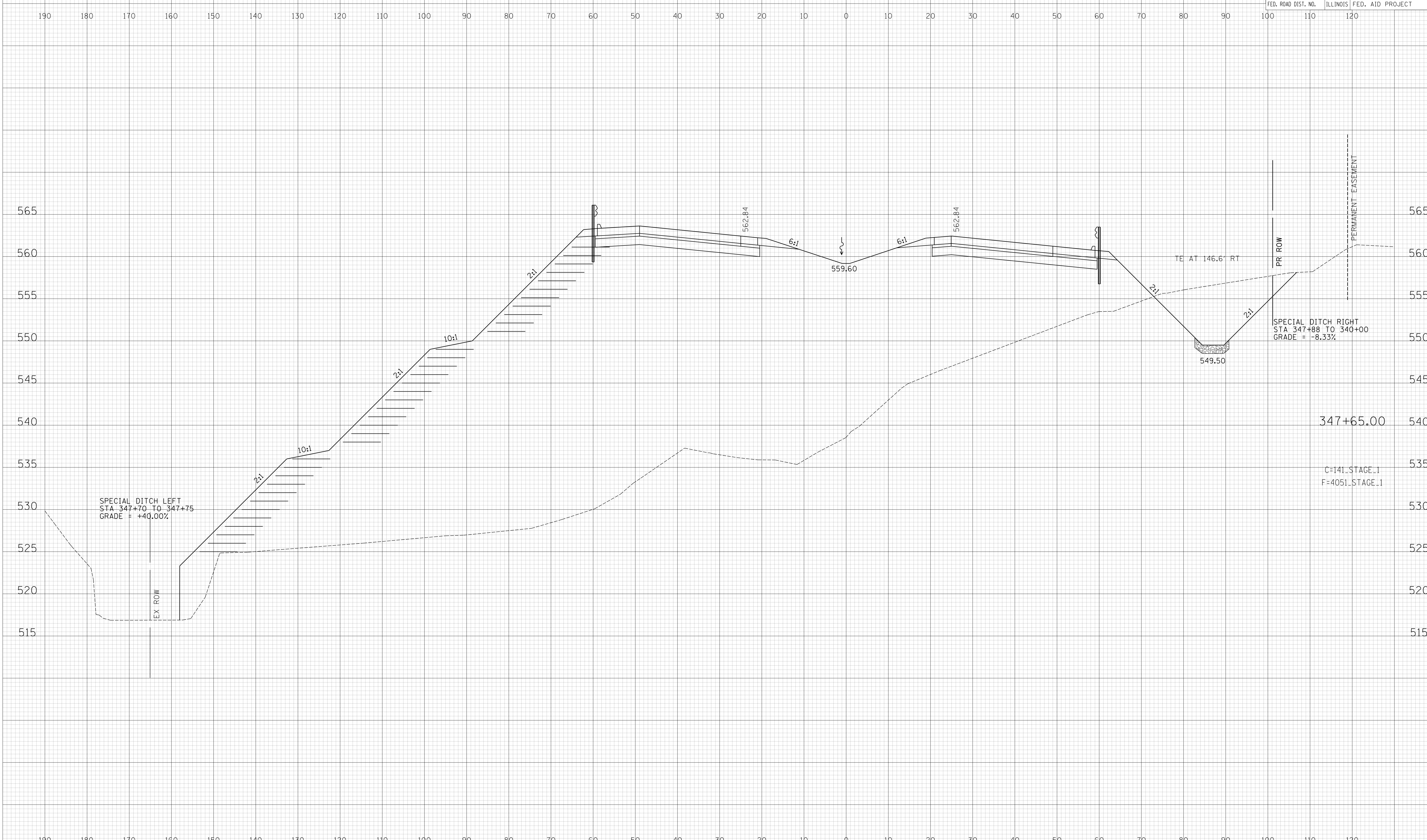
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NOTE BOOK: _____ AREAS CHECKED: _____

NO.: _____



F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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STA.	TO STA.			
FED. ROAD DIST. NO.	ILLINOIS	FED. AID PROJECT		

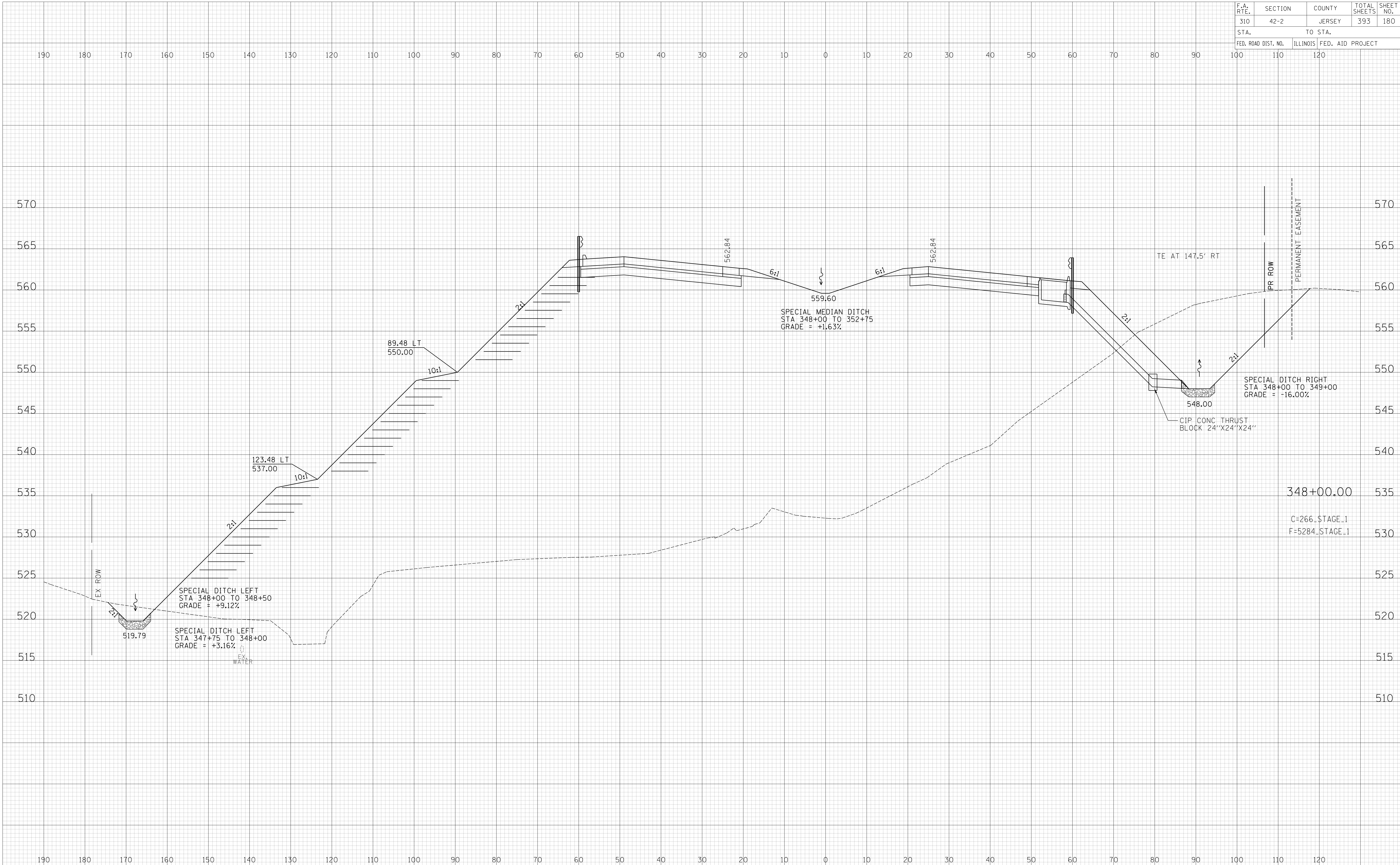


BY: _____ DATE: _____
 SURVEYED: _____
 PLOTTED: _____
 NOTE BOOK NO. _____
 AREAS CHECKED: _____

POWER I. CHASTAIN & ASSOCIATES LLP
 1000 N. LAKE ST. SUITE 200
 CHICAGO, IL 60610
 (773) 714-0050
 (815) 480-0050
 Fax: (773) 714-0057

BY: _____ DATE: _____
 SURVEYED: _____
 PLOTTED: _____
 NOTE BOOK NO. _____
 AREAS CHECKED: _____

F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
310	42-2	JERSEY	393	180
STA.	TO STA.			
FED. ROAD DIST. NO.	ILLINOIS	FED. AID PROJECT		

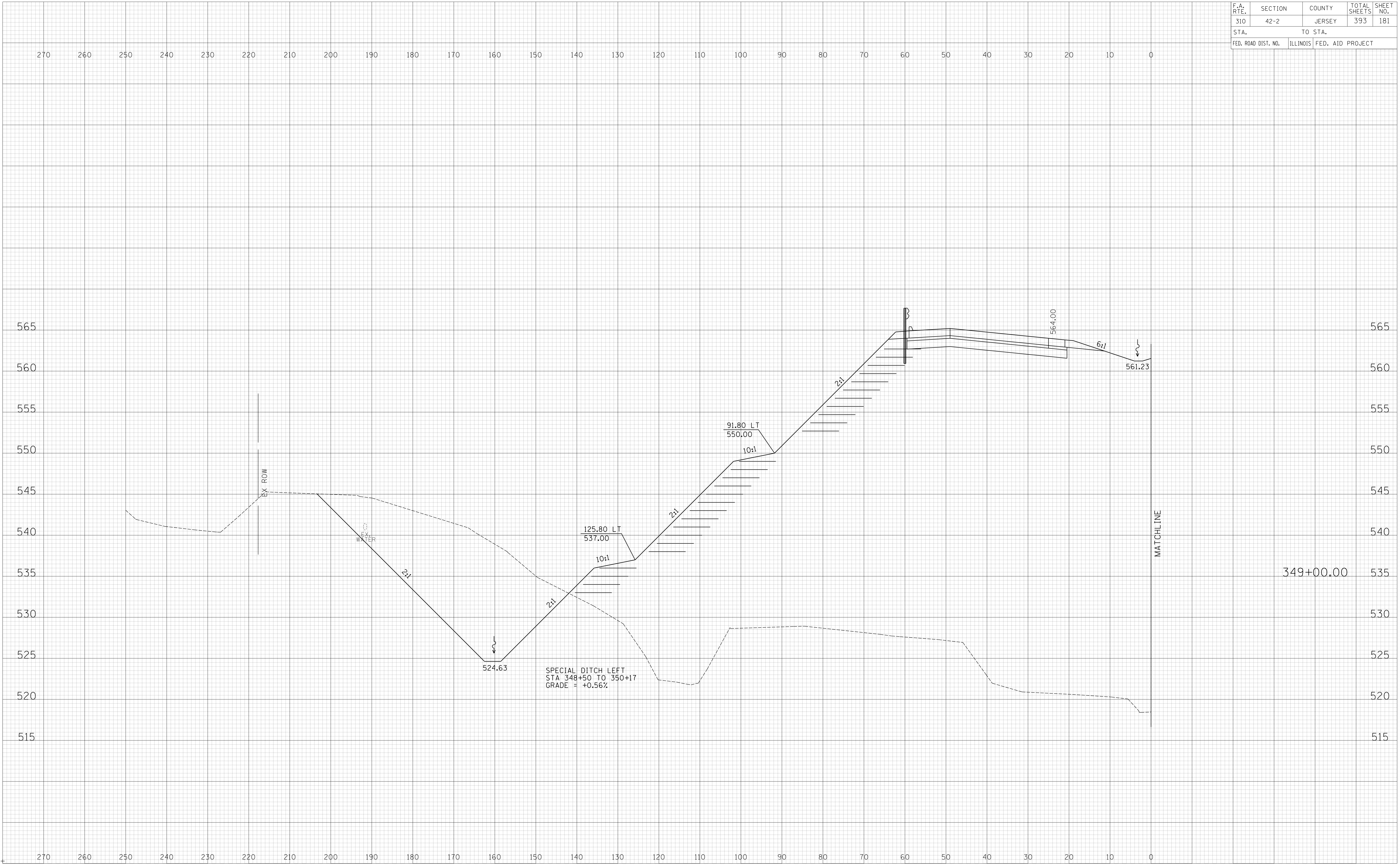


DATE	BY
FINAL SURVEY	SURVEYED
NOTED BOOK	PLOTTED
NO.	AREAS CHECKED

POWER I. CHASTAIN & ASSOCIATES LLP
 100 N. LAUREL ST. SUITE 200
 CHICAGO, IL 60610
 (773) 714-0050
 (815) 486-0050
 Fax: (815) 486-0057

DATE	BY
ORIGINAL SURVEY	SURVEYED
NOTED BOOK	PLOTTED
NO.	AREAS CHECKED

F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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STA.	TO STA.			
FED. ROAD DIST. NO.	ILLINOIS	FED. AID PROJECT		

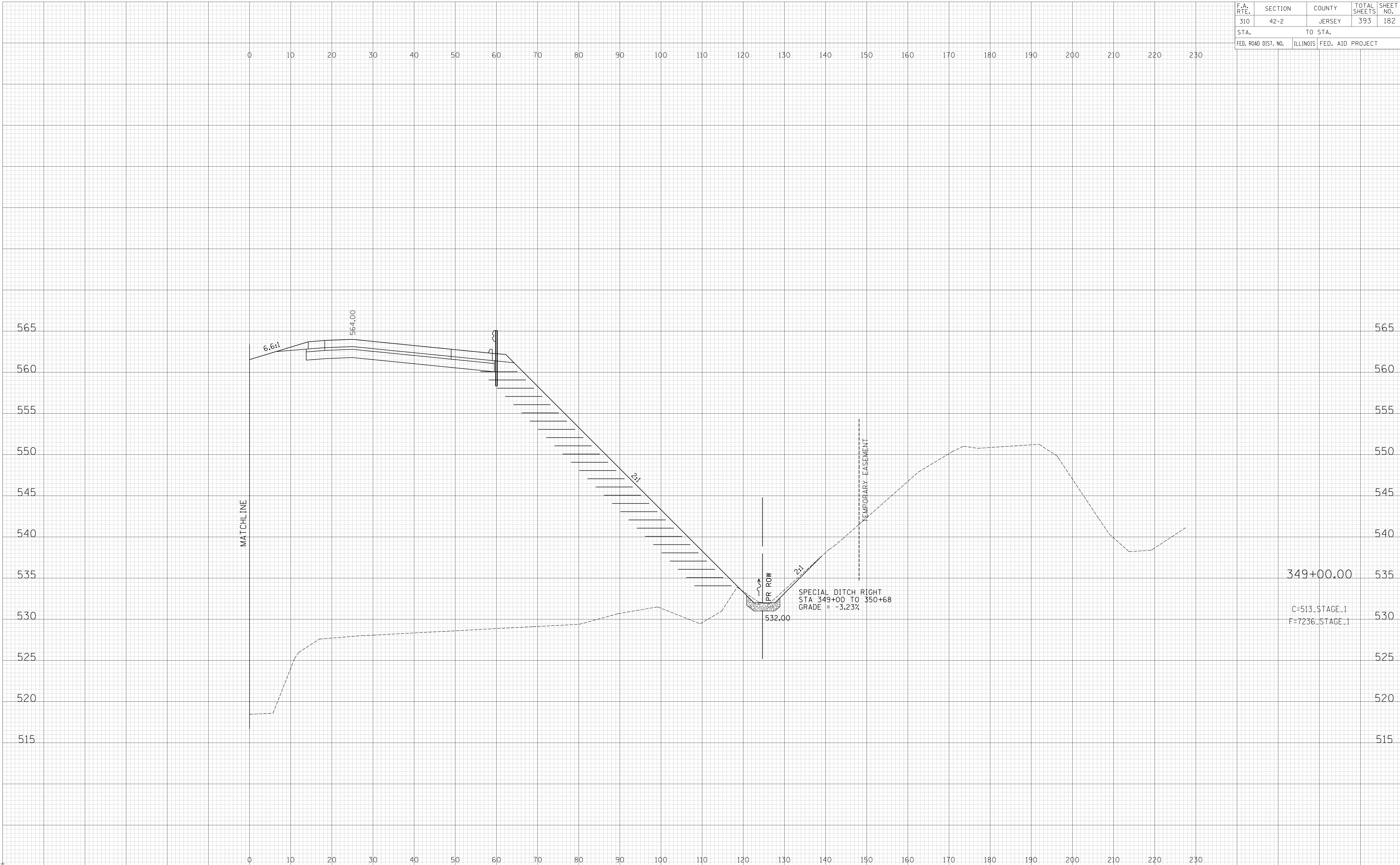


DATE	BY
NO.	AREAS CHECKED
NO.	AREAS CHECKED
NO.	AREAS CHECKED

HOOPER & CHASTAIN
 & ASSOCIATES, LLP
 SURVEYING & ENGINEERING
 CHICAGO, ILLINOIS 60604
 (773) 714-0860
 (618) 889-0000
 ROCKFORD, ILLINOIS 61107

DATE	BY
NO.	AREAS CHECKED
NO.	AREAS CHECKED
NO.	AREAS CHECKED

F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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STA.		TO STA.		
FED. ROAD DIST. NO.		ILLINOIS	FED. AID PROJECT	



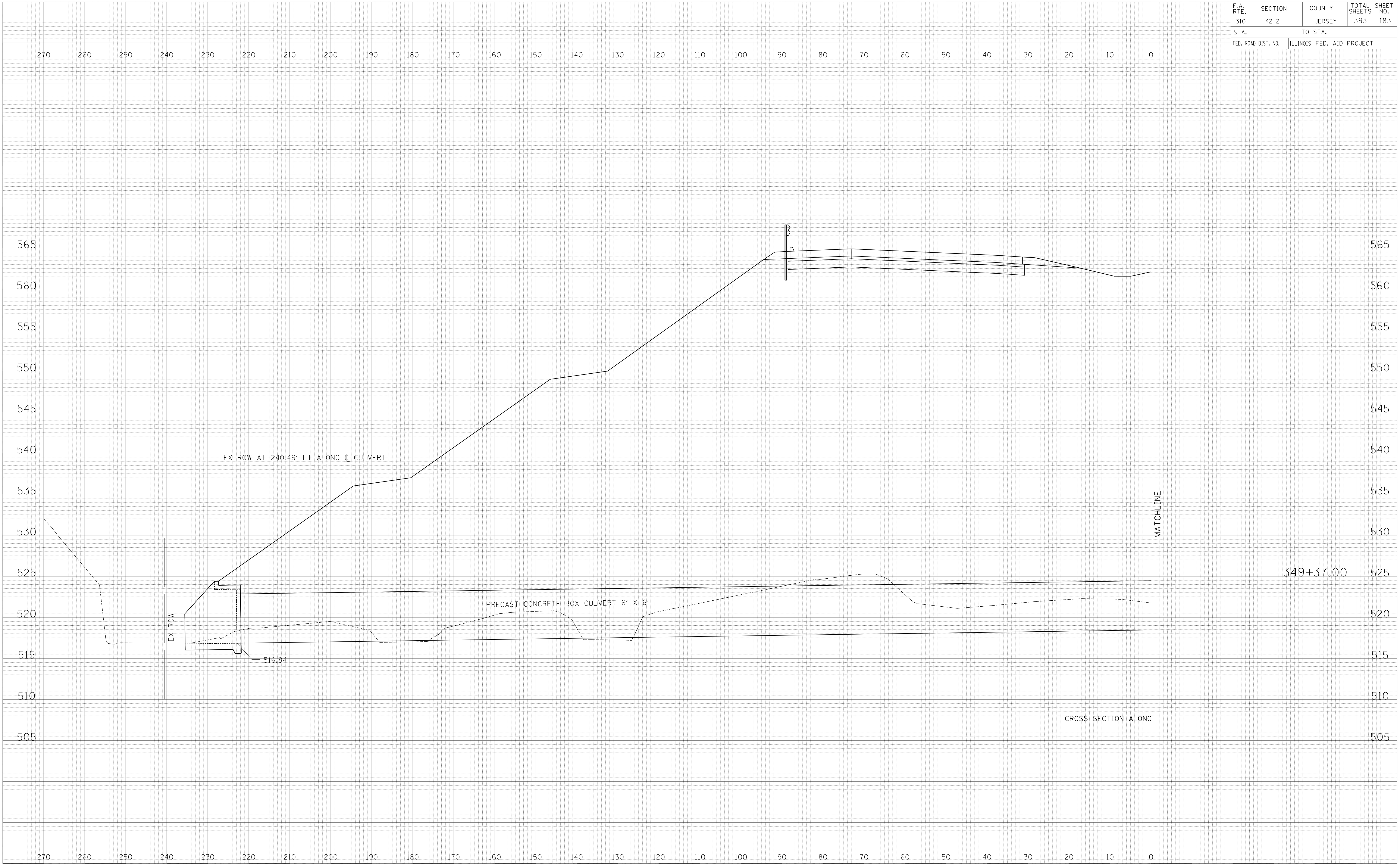
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NO.	NO.
AREAS CHECKED	AREAS CHECKED
PLOTTED DATE	PLOTTED DATE
NOTE BOOK	NOTE BOOK
SURVEYED	SURVEYED
FINAL SURVEY	FINAL SURVEY

HOOPER & CHASTAIN
& ASSOCIATES, LLP
 100 N. LAUREL STREET
 CHICAGO, ILLINOIS 60602
 (773) 714-0050
 (618) 489-0000
 JPL-00137

DATE	BY
NO.	NO.
AREAS CHECKED	AREAS CHECKED
PLOTTED DATE	PLOTTED DATE
NOTE BOOK	NOTE BOOK
SURVEYED	SURVEYED
ORIGINAL SURVEY	ORIGINAL SURVEY

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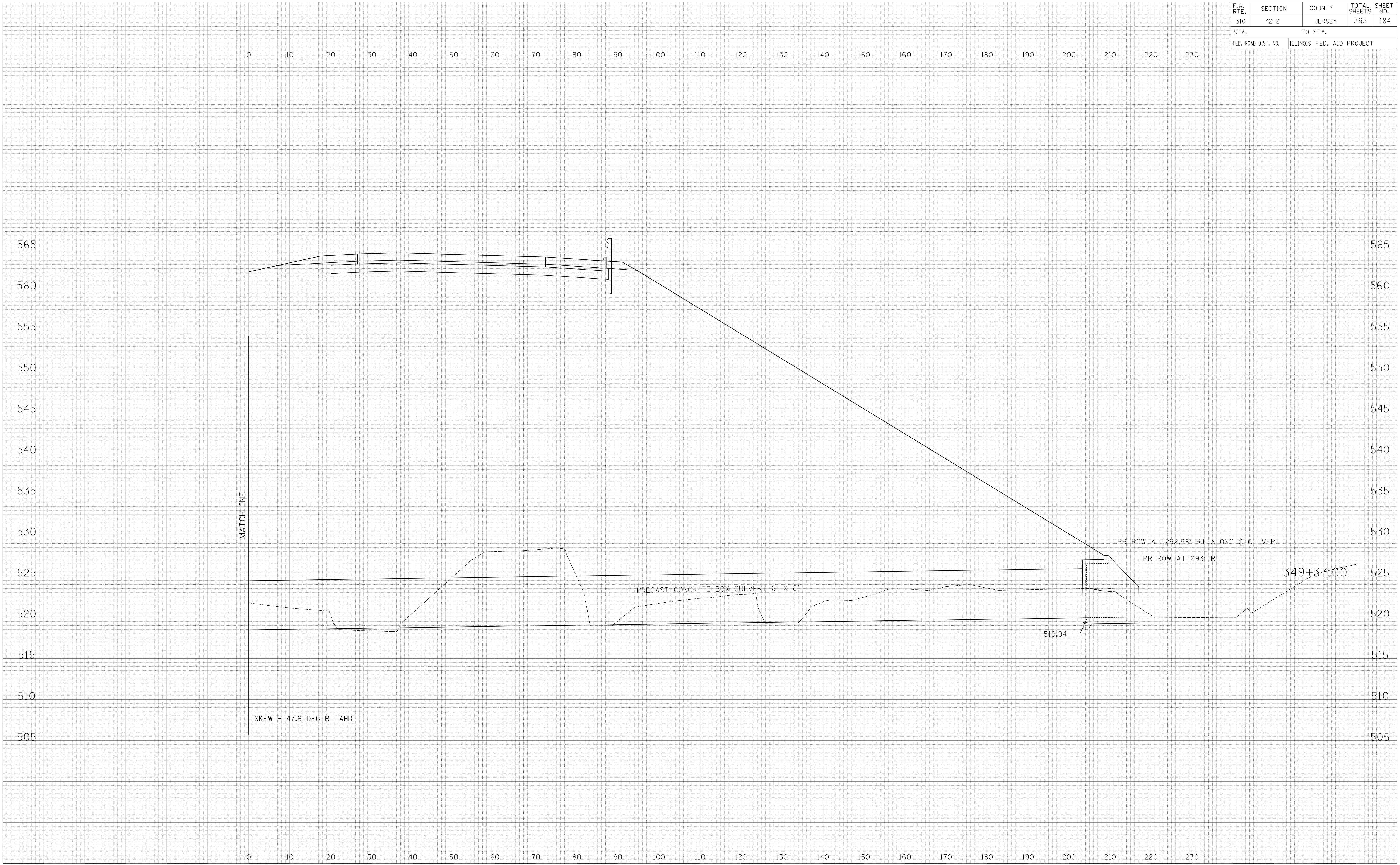


DATE	BY
NO.	NO.
AREAS CHECKED	AREAS CHECKED
PLATE	PLATE
PLotted	PLotted
Surveyed	Surveyed
Final	Final
Survey	Survey

HOOPER & CHASTAIN
 & ASSOCIATES, LLP
 100 N. LAUREL STREET
 CHICAGO, IL 60602
 (773) 714-0860
 (616) 889-0000
 10/1/17

DATE	BY
NO.	NO.
AREAS CHECKED	AREAS CHECKED
PLATE	PLATE
PLotted	PLotted
Surveyed	Surveyed
Original	Original
Survey	Survey

F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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STA.		TO STA.		
FED. ROAD DIST. NO.		ILLINOIS	FED. AID PROJECT	

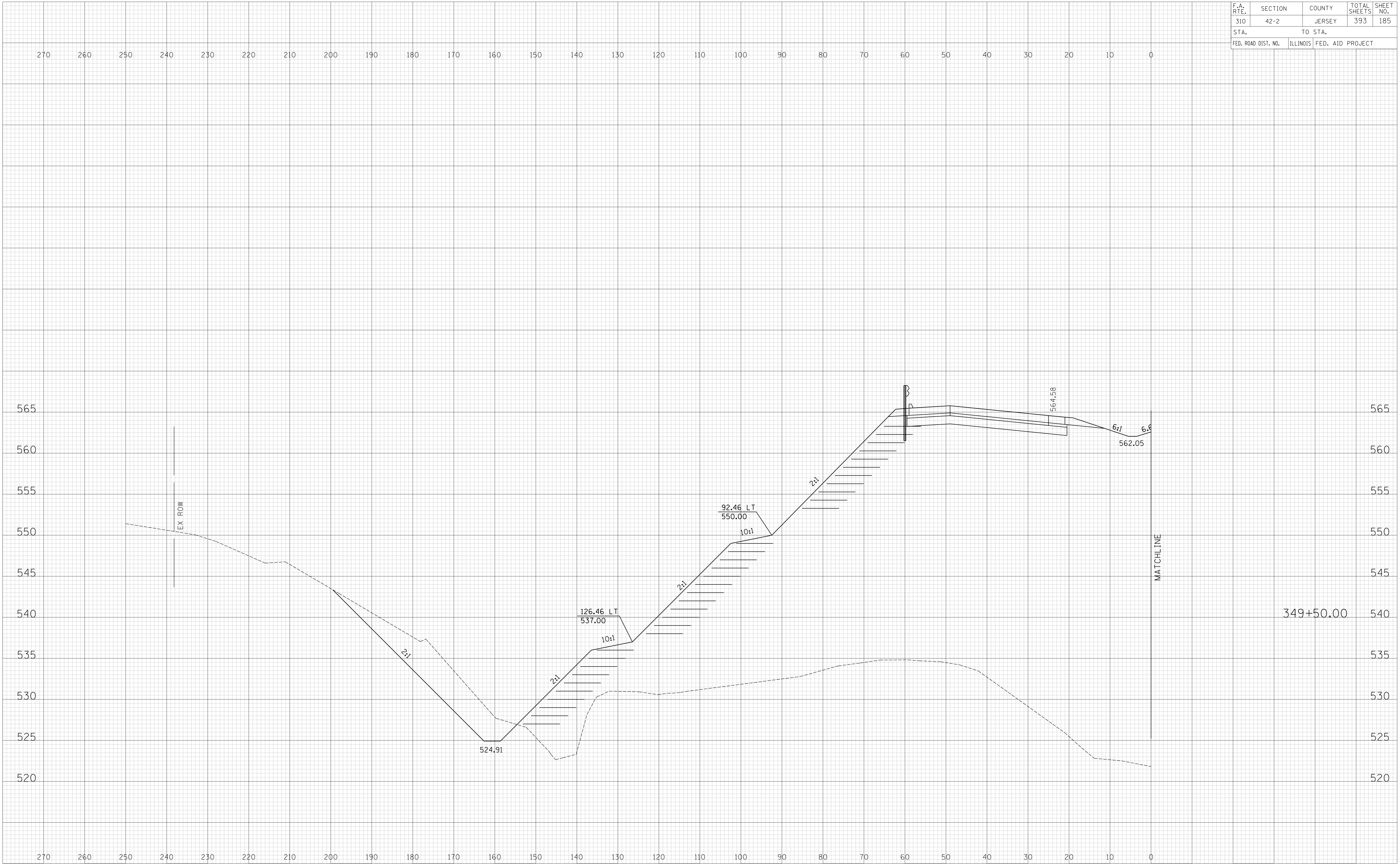


DATE	BY
DATE	BY
DATE	BY

HOOPER & CHASTAIN & ASSOCIATES, LLP
 SURVEYORS & ENGINEERS
 CHICAGO, ILLINOIS
 (773) 714-0050
 (618) 489-0000
 ROCKFORD, ILLINOIS

DATE	BY
DATE	BY
DATE	BY

F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
310	42-2	JERSEY	393	185
STA.	TO STA.			
FED. ROAD DIST. NO.	ILLINOIS	FED. AID PROJECT		



DATE	BY
NO.	NO.
AREAS CHECKED	AREAS CHECKED
AREAS	AREAS
PLOTTED DATE	PLOTTED DATE
NOTE BOOK	NOTE BOOK
SURVEYED	SURVEYED
FINAL SURVEY	FINAL SURVEY

HOOPER & CHASTAIN
 & ASSOCIATES, LLP
 100 N. LAKE ST., SUITE 2000
 CHICAGO, IL 60601
 (773) 714-0050
 (616) 889-0000
 hcr@hca.com

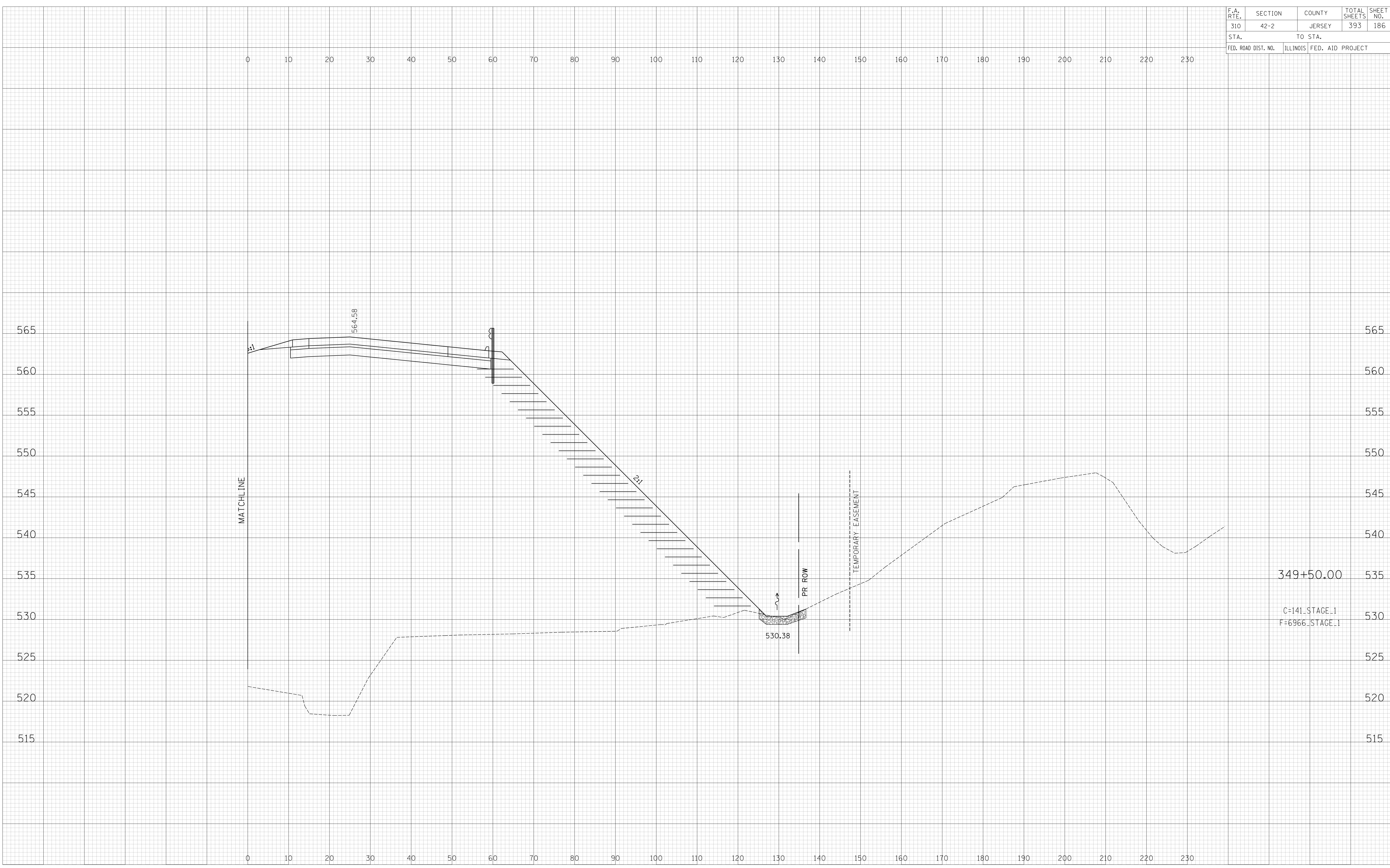
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ORIGINAL SURVEY	ORIGINAL SURVEY

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STA.	TO STA.			
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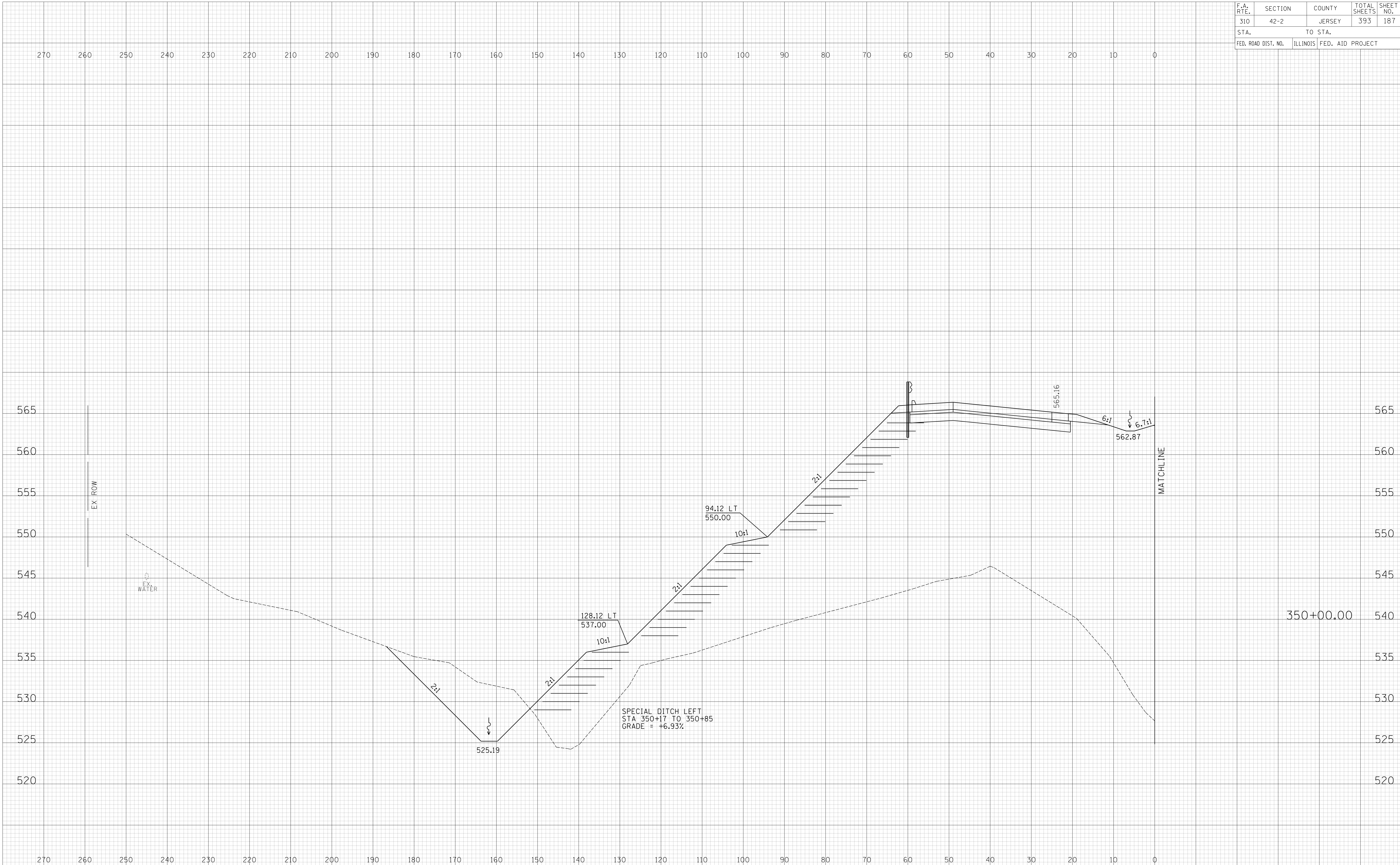
HOOPER & CHASTAIN ASSOCIATES, LLP
 SURVEYING & ENGINEERING
 100 W. WASHINGTON ST.
 CHICAGO, IL 60601
 (773) 714-0850
 (616) 889-0080
 JH-00137

ORIGINAL SURVEY	SURVEYED	DATE
NOTE BOOK NO.	PLOTTED	
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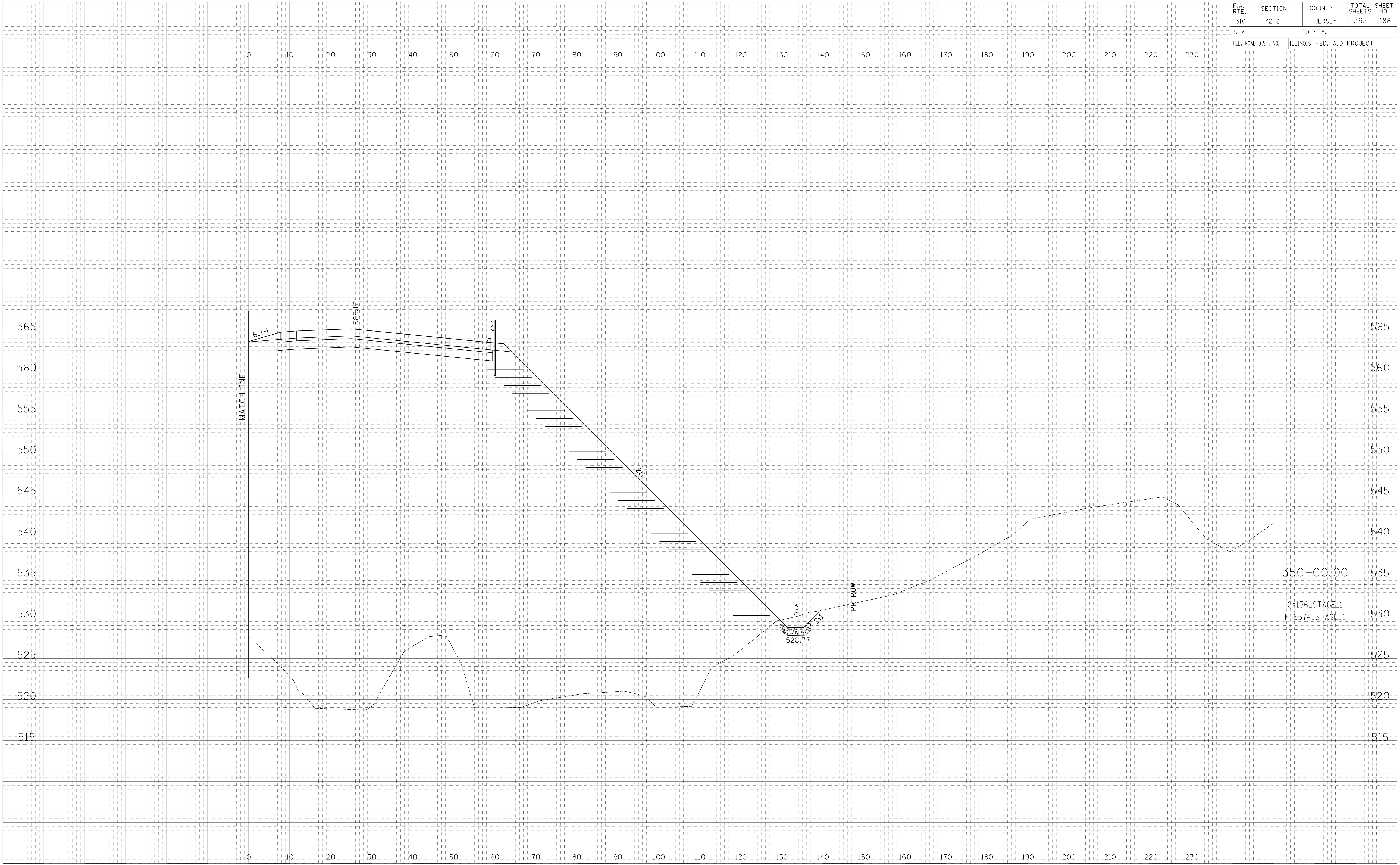
DATE	BY

HOOPER & CHASTAIN & ASSOCIATES, LLP
 100 WEST WASHINGTON STREET
 CHICAGO, IL 60601
 (773) 714-0050
 (616) 889-0000
 FAX (773) 714-0050
 hca-00137

DATE	BY

HOOPER & CHASTAIN & ASSOCIATES, LLP
 100 WEST WASHINGTON STREET
 CHICAGO, IL 60601
 (773) 714-0050
 (616) 889-0000
 FAX (773) 714-0050
 hca-00137

F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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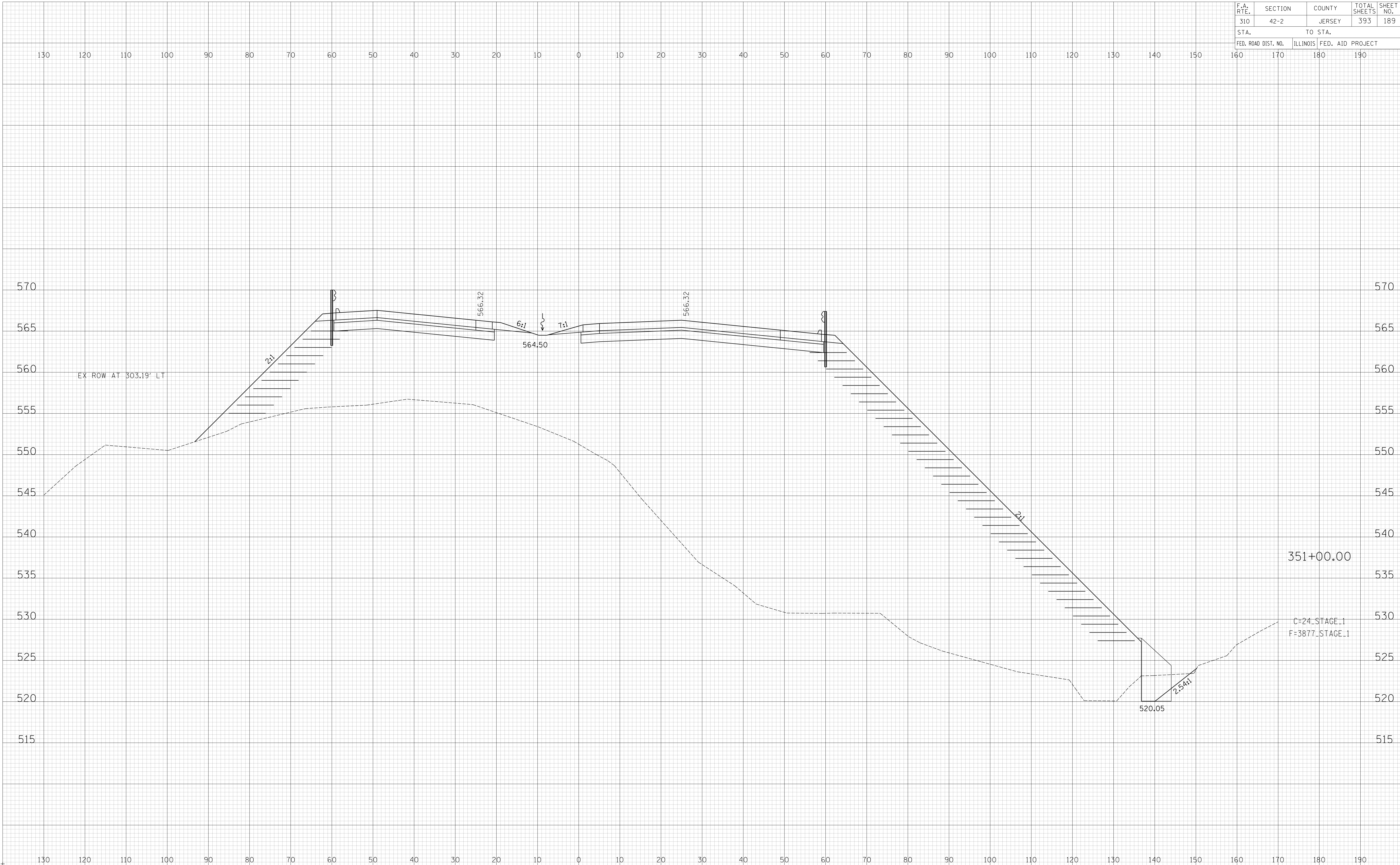


DATE	BY
NO.	AREAS CHECKED
NO.	AREAS CHECKED

HOOPER & CHASTAIN & ASSOCIATES, LLP
 100 N. LAUREL STREET
 CHICAGO, IL 60602
 (773) 714-0050
 (616) 889-0000
 10/10/17

DATE	BY
NO.	AREAS CHECKED
NO.	AREAS CHECKED

F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
310	42-2	JERSEY	393	189
STA.		TO STA.		
FED. ROAD DIST. NO.		ILLINOIS	FED. AID PROJECT	



FINAL SURVEY

DATE

BY

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PLOTTED

NOTE BOOK

AREAS CHECKED

NO.

ORIGINAL SURVEY

DATE

BY

SURVEYED

PLOTTED

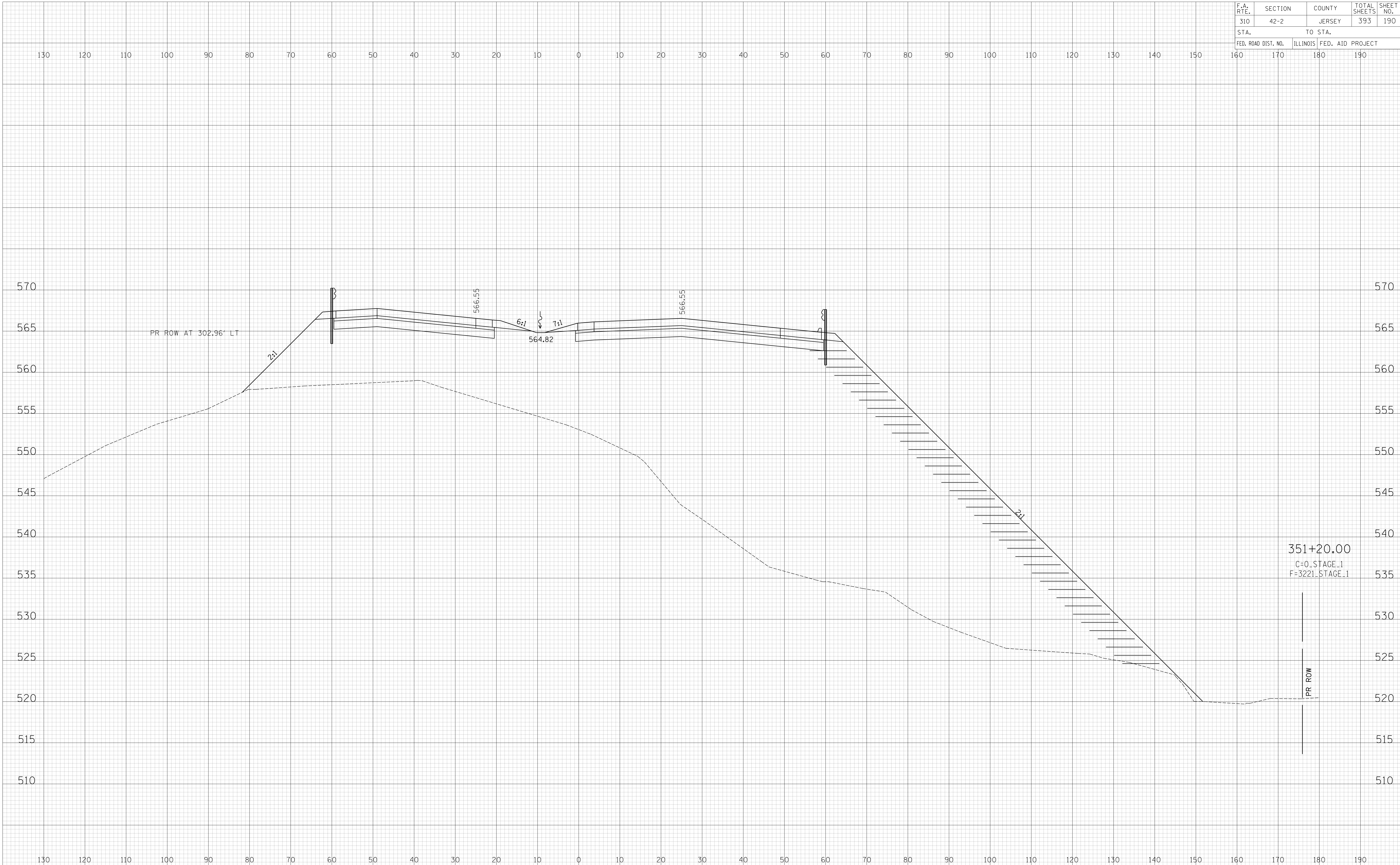
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AREAS CHECKED

NO.

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 100 W. WASHINGTON ST. SUITE 200
 CHICAGO, IL 60601
 (773) 714-0050
 (616) 889-0000
 hca@hca.com

F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
310	42-2	JERSEY	393	190
STA. 160		TO STA. 170		FED. AID PROJECT



DATE	
BY	
FINAL SURVEY	
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NOTE BOOK	
NO.	
AREAS CHECKED	

HOOPER & CHASTAIN & ASSOCIATES, LLP
 SURVEYORS & ENGINEERS
 100 N. LAUREL ST., SUITE 200
 CHICAGO, IL 60610
 (773) 714-0050
 (616) 889-0000
 hcr-001377

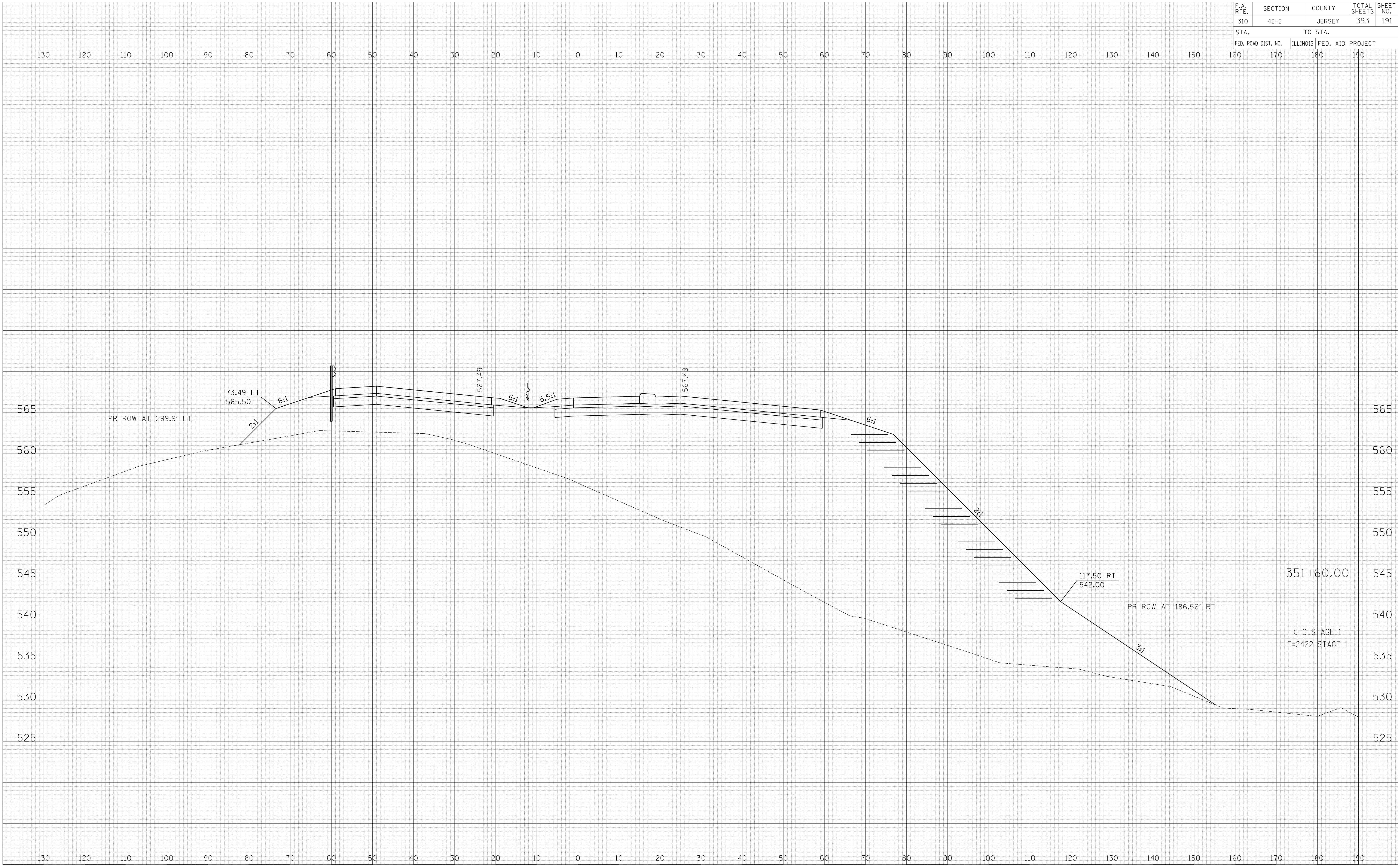
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BY	
ORIGINAL SURVEY	
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NOTE BOOK	
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AREAS CHECKED	

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STA.	TO STA.			
FED. ROAD DIST. NO.	ILLINOIS	FED. AID PROJECT		

FINAL SURVEY	SURVEYED	DATE
NOTE BOOK	PLOTTED	
NO.	AREAS	
	CHECKED	

ORIGINAL SURVEY	SURVEYED	DATE
NOTE BOOK	PLOTTED	
NO.	AREAS	
	CHECKED	

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 100 N. LAUREL STREET
 CHICAGO, IL 60602
 (773) 714-0050
 (616) 889-0000
 ROCKFORD, IL 61107



F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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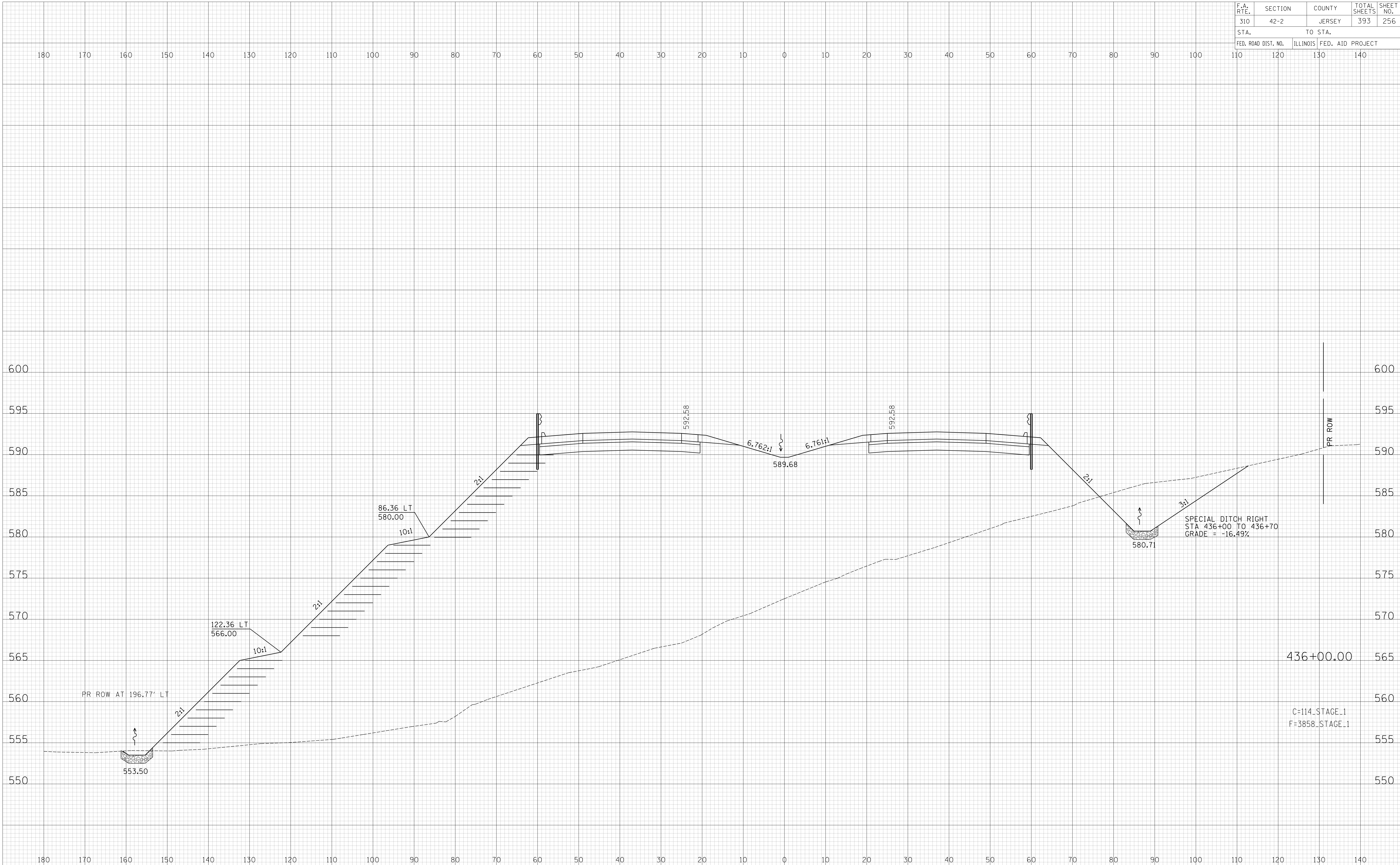
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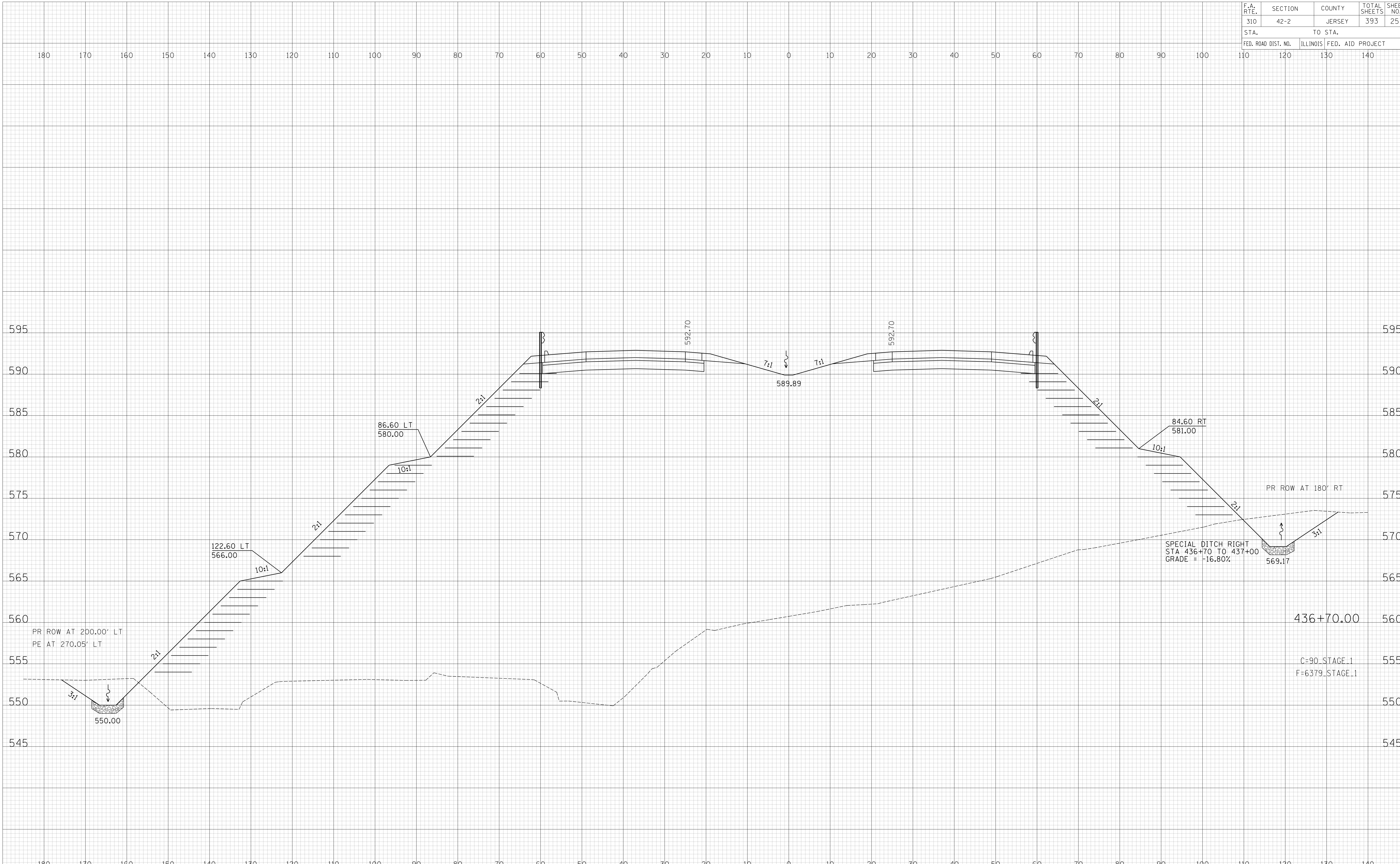
HOOPER L. CHASTAIN & ASSOCIATES, LLP
 SURVEYORS & ENGINEERS
 CHICAGO, ILLINOIS 60604
 (773) 714-0000
 (815) 489-9000
 10/14/2017

DATE	BY

NO.	AREAS CHECKED



F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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FED. ROAD DIST. NO.	ILLINOIS	FED. AID PROJECT		

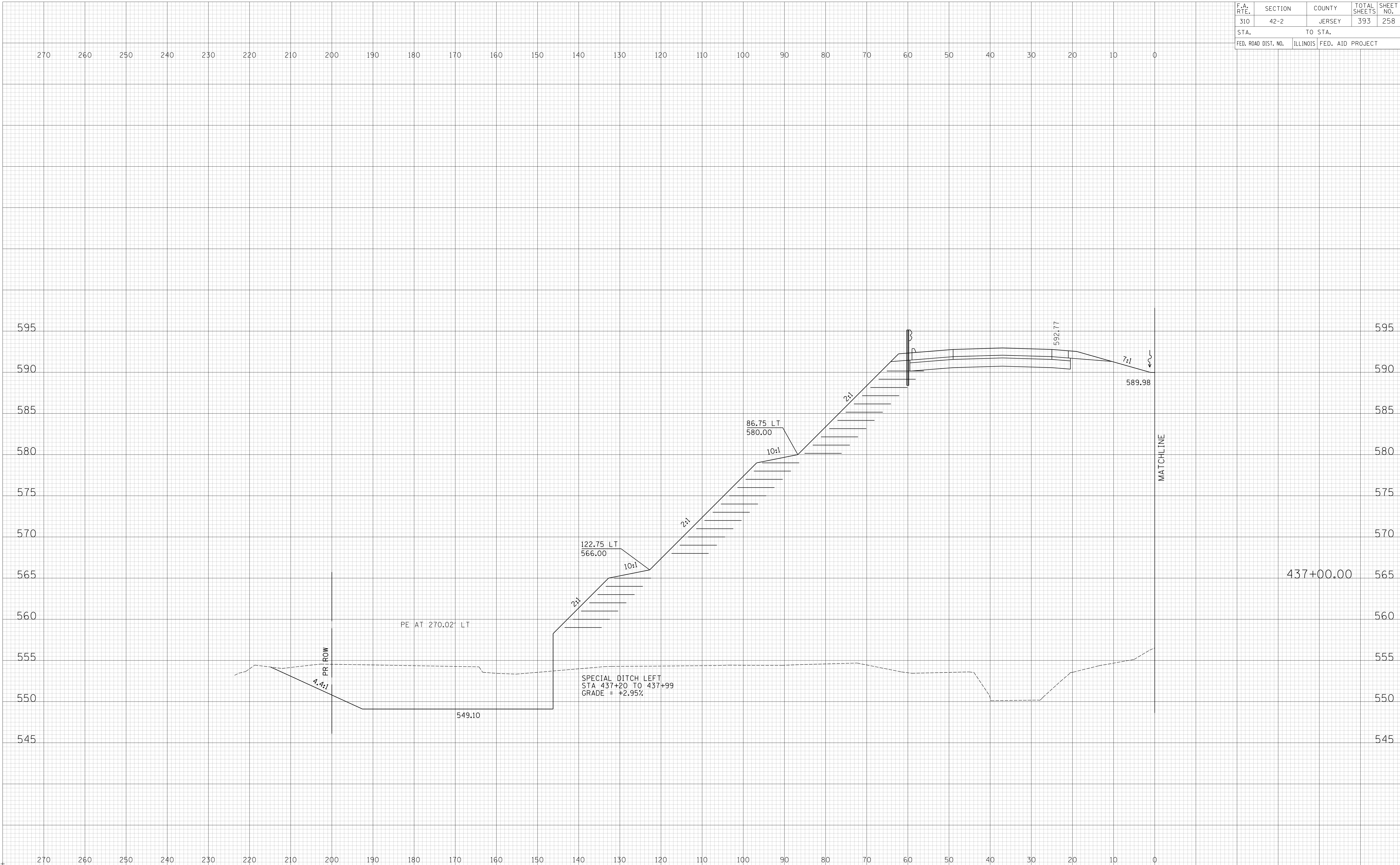


DATE	
BY	
NO.	
FINAL SURVEY	SURVEYED
NOTE BOOK	PLOTTED
	AREAS CHECKED

HOWER L. CHASTAIN & ASSOCIATES, LLP
 SURVEYING & ENGINEERING
 CHICAGO, ILLINOIS 60604
 (773) 714-0000
 (618) 489-0000
 hlc@hlc.com

DATE	
BY	
NO.	
ORIGINAL SURVEY	SURVEYED
NOTE BOOK	PLOTTED
	AREAS CHECKED

F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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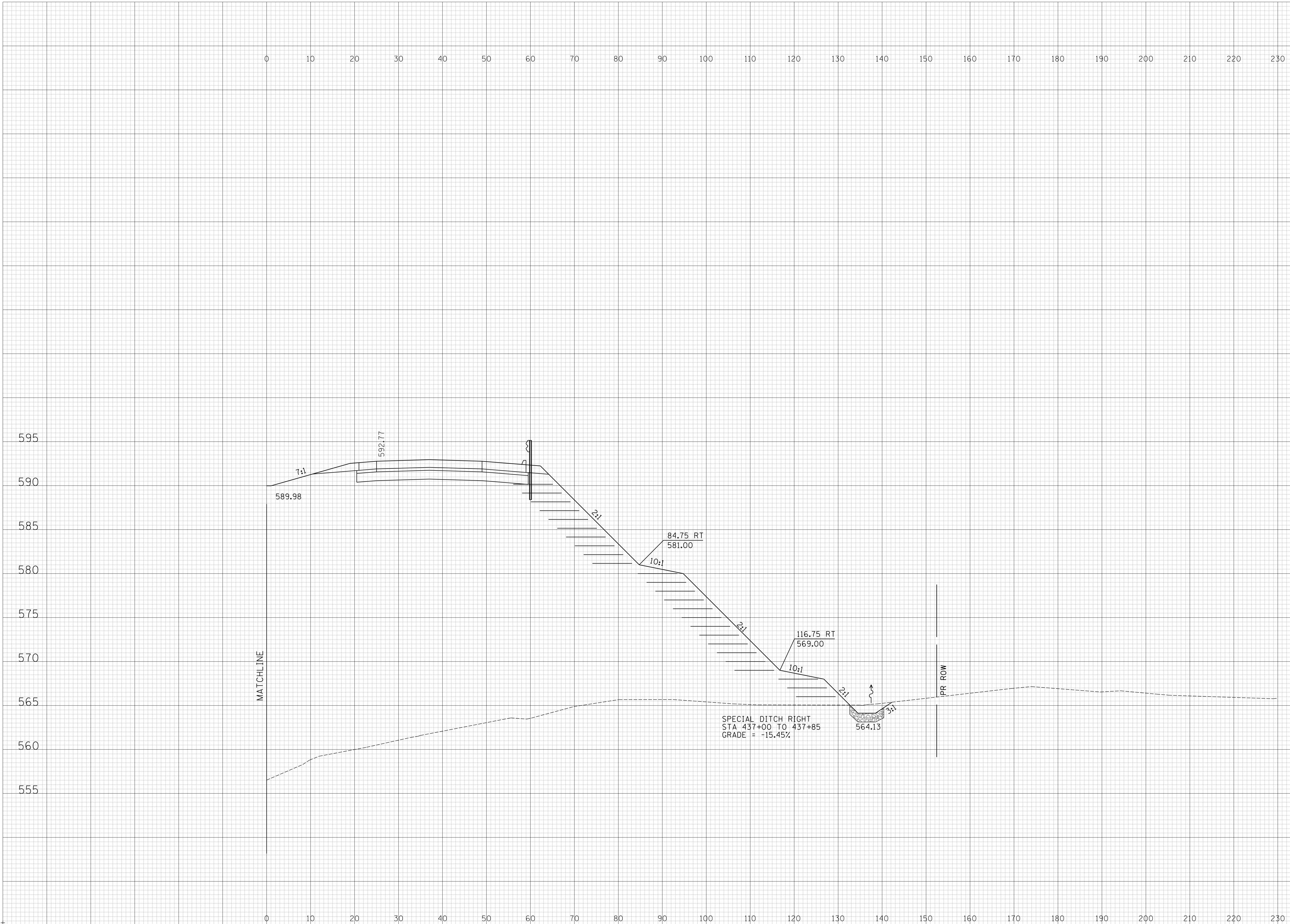


DATE	BY

POWER L. CHASTAIN & ASSOCIATES, LLP
 SURVEYORS
 CHICAGO, ILLINOIS 60601
 TEL: 773-714-0000
 FAX: 773-714-0000
 ROCKFORD, ILLINOIS 61107

DATE	BY

F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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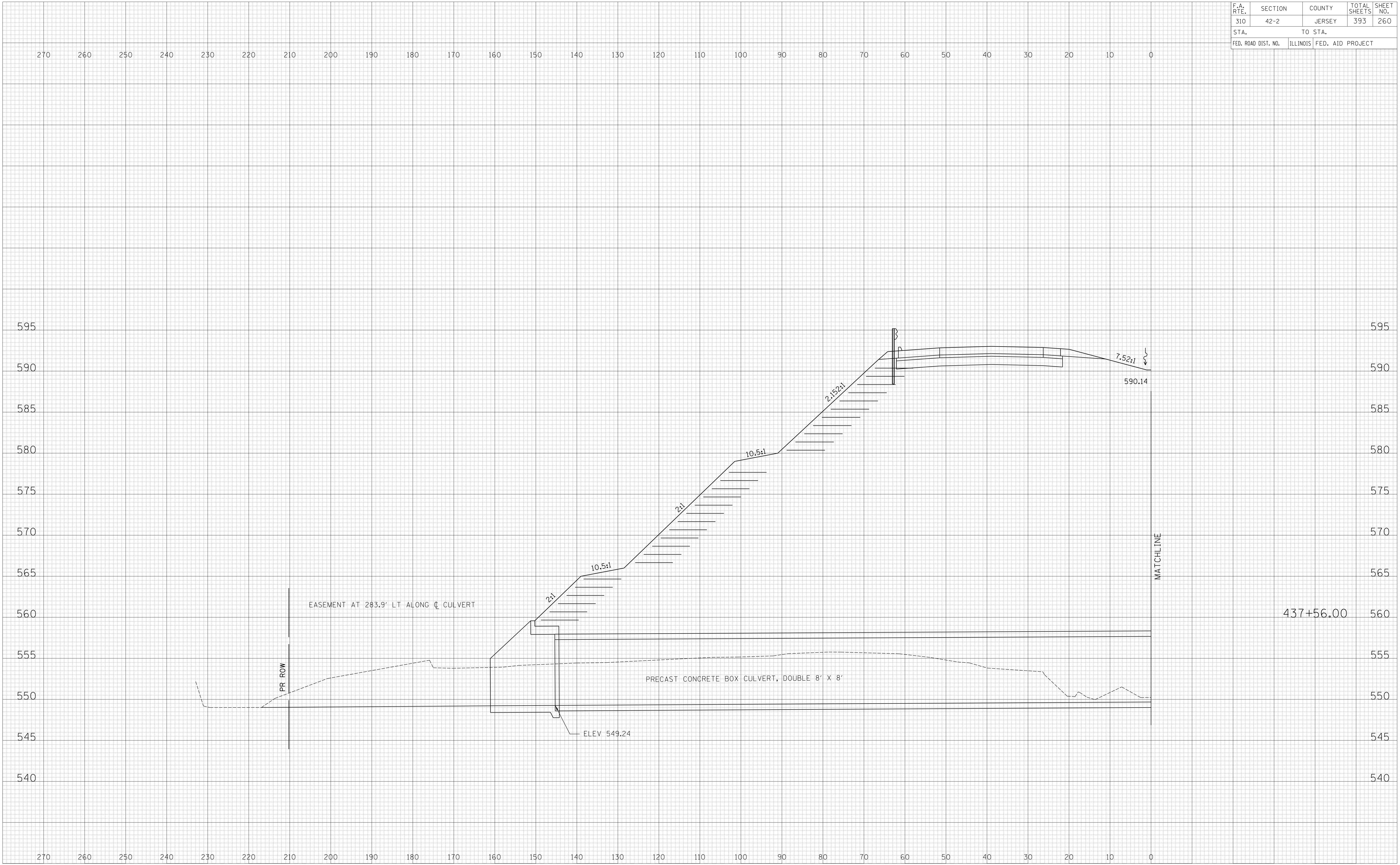
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 NOTE BOOK NO. _____
 AREAS CHECKED: _____

POWER L. CHASTAIN & ASSOCIATES, LLP
 100 N. LAKE ST. SUITE 1000
 CHICAGO, IL 60601
 (773) 714-0000
 (616) 489-9000
 fax: (773) 714-0000

F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
310	42-2	JERSEY	393	260
STA.	TO STA.			
FED. ROAD DIST. NO.	ILLINOIS	FED. AID PROJECT		

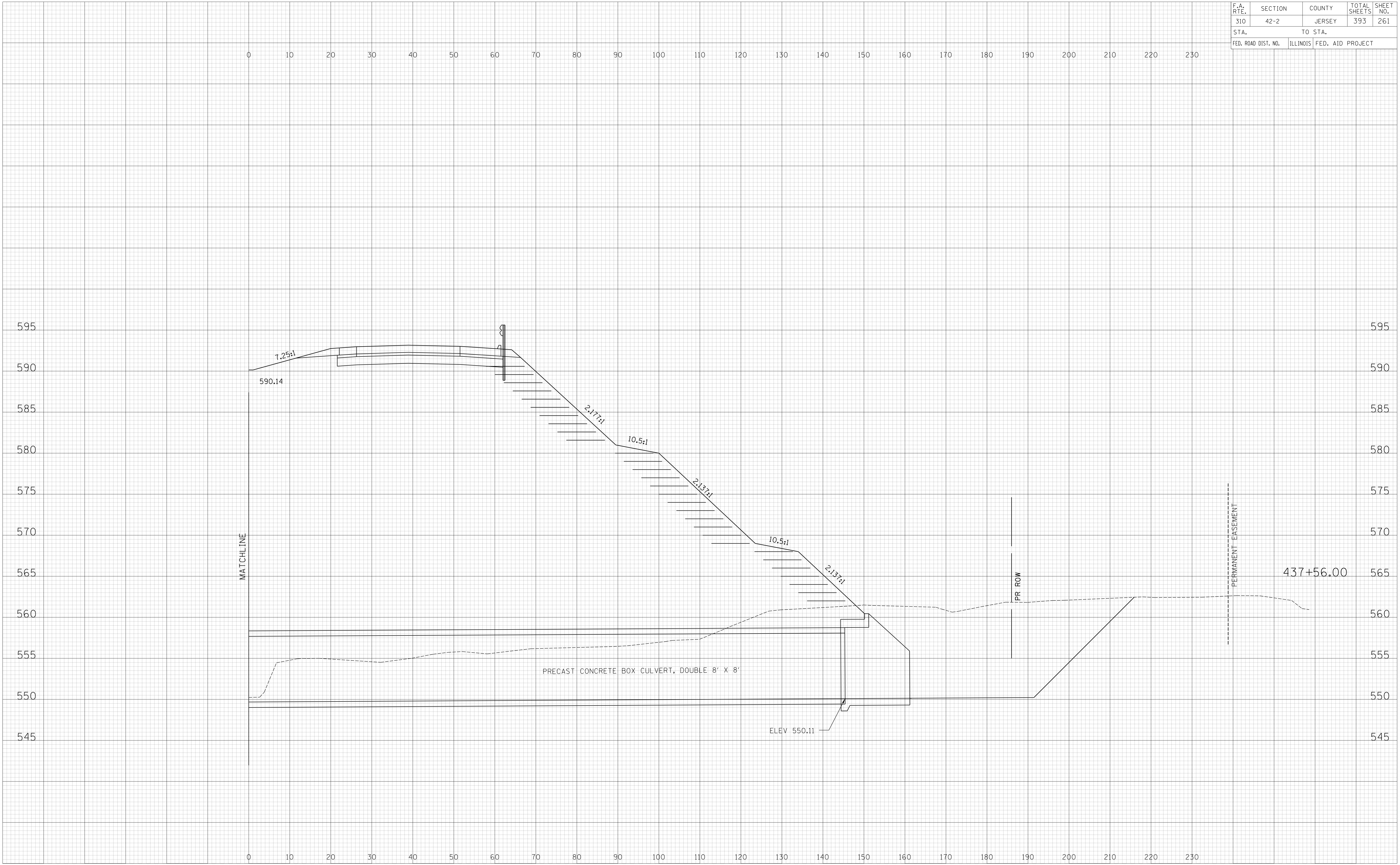


DATE	BY
NO.	NO.
AREAS CHECKED	AREAS CHECKED
PLOTTED DATE	PLOTTED DATE
NOTE BOOK	NOTE BOOK
NO.	NO.
FINAL SURVEY	FINAL SURVEY
SURVEYED	SURVEYED

POWER L
CHASTAIN
 & ASSOCIATES, LLP
 100 WEST WASHINGTON
 CHICAGO, ILLINOIS 60601
 (773) 714-0000
 (616) 489-9000
 104-00137

DATE	BY
NO.	NO.
AREAS CHECKED	AREAS CHECKED
PLOTTED DATE	PLOTTED DATE
NOTE BOOK	NOTE BOOK
NO.	NO.
ORIGINAL SURVEY	ORIGINAL SURVEY
SURVEYED	SURVEYED

F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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STA.	TO STA.			
FED. ROAD DIST. NO.	ILLINOIS	FED. AID PROJECT		

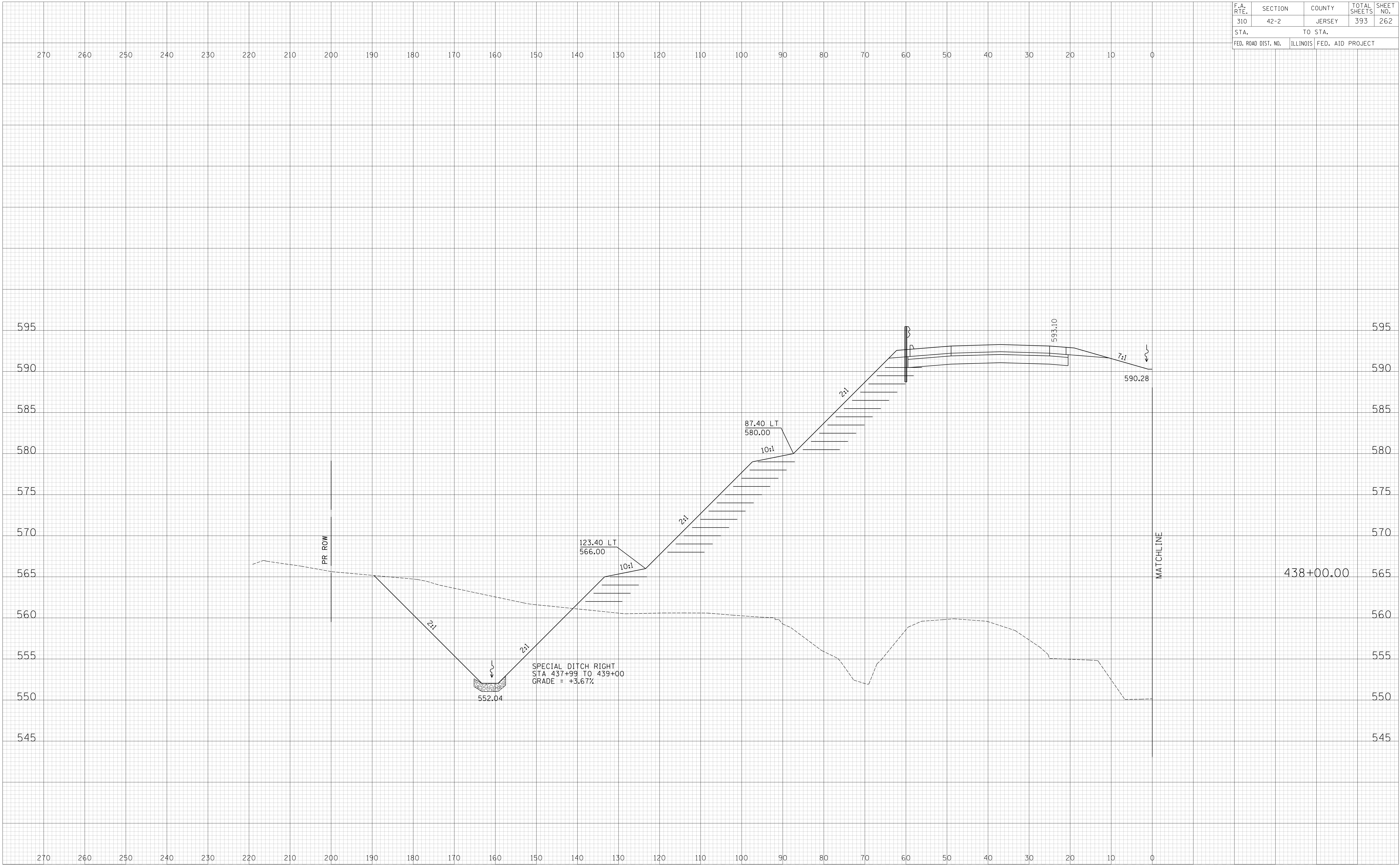


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NO.	NO.
AREAS CHECKED	AREAS CHECKED
PLOTTED DATE	PLOTTED DATE
NOTE BOOK	NOTE BOOK
SURVEYED	SURVEYED
FINAL SURVEY	FINAL SURVEY

POWER L. CHASTAIN & ASSOCIATES, LLP
 100 N. LAUREL STREET, SUITE 200
 CHICAGO, ILLINOIS 60610
 (773) 714-0000
 (815) 489-9000
 (312) 461-1377

DATE	BY
NO.	NO.
AREAS CHECKED	AREAS CHECKED
PLOTTED DATE	PLOTTED DATE
NOTE BOOK	NOTE BOOK
SURVEYED	SURVEYED
ORIGINAL SURVEY	ORIGINAL SURVEY

F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
310	42-2	JERSEY	393	262
STA.		TO STA.		
FED. ROAD DIST. NO.		ILLINOIS	FED. AID PROJECT	

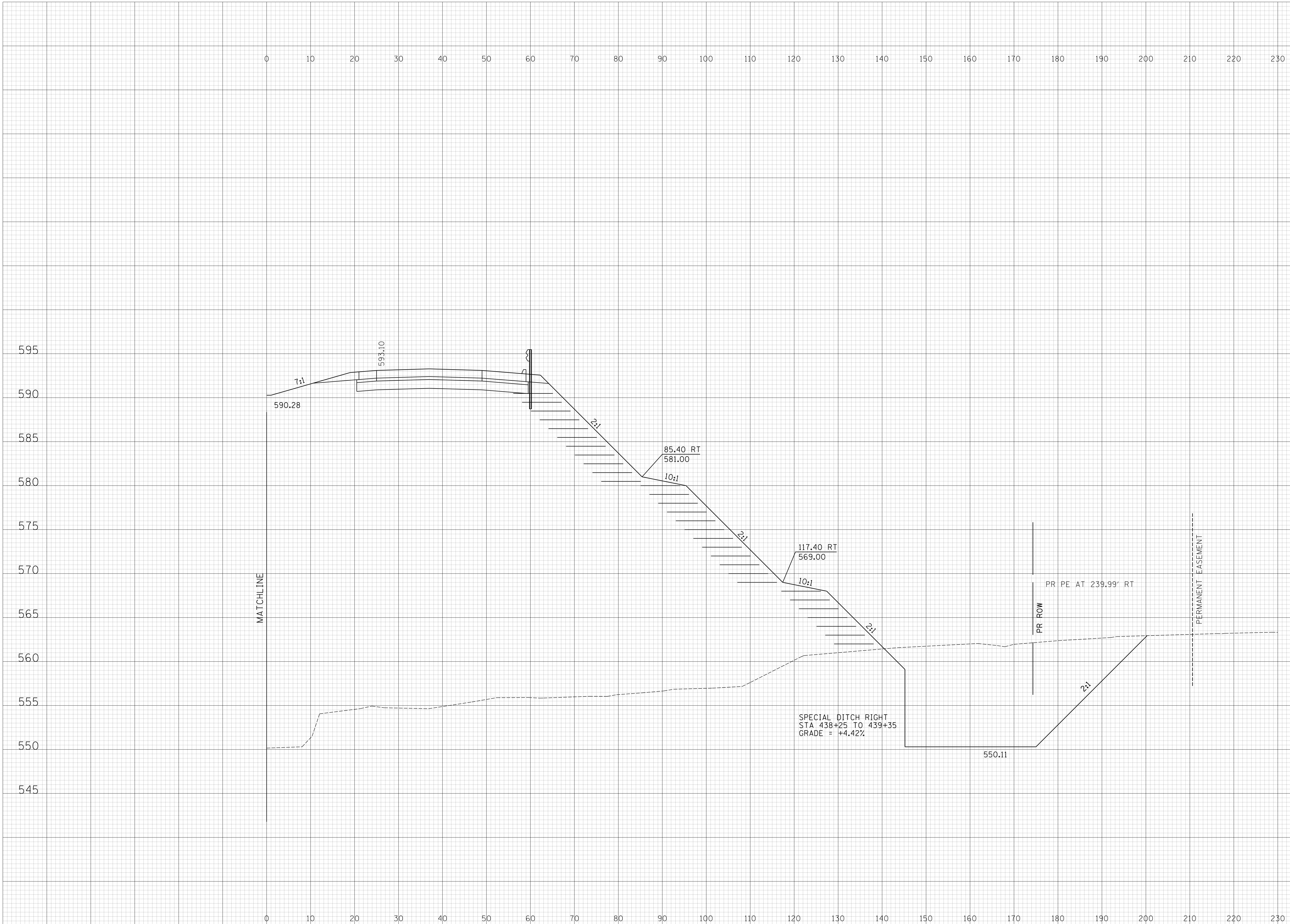


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SURVEYED
PLOTTED
NOTE BOOK
AREAS CHECKED
NO.

POWER L
CHASTAIN
 & ASSOCIATES, LLP
 SURVEYORS
 CHICAGO
 ROCKFORD
 (773) 714-0000
 (815) 489-0000
 04-00137

DATE
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SURVEYED
PLOTTED
NOTE BOOK
AREAS CHECKED
NO.

F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
310	42-2	JERSEY	393	263
STA.	TO STA.			
FED. ROAD DIST. NO.	ILLINOIS	FED. AID PROJECT		



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 F=7255_STAGE_1

SPECIAL DITCH RIGHT
 STA 438+25 TO 439+35
 GRADE = +4.42%

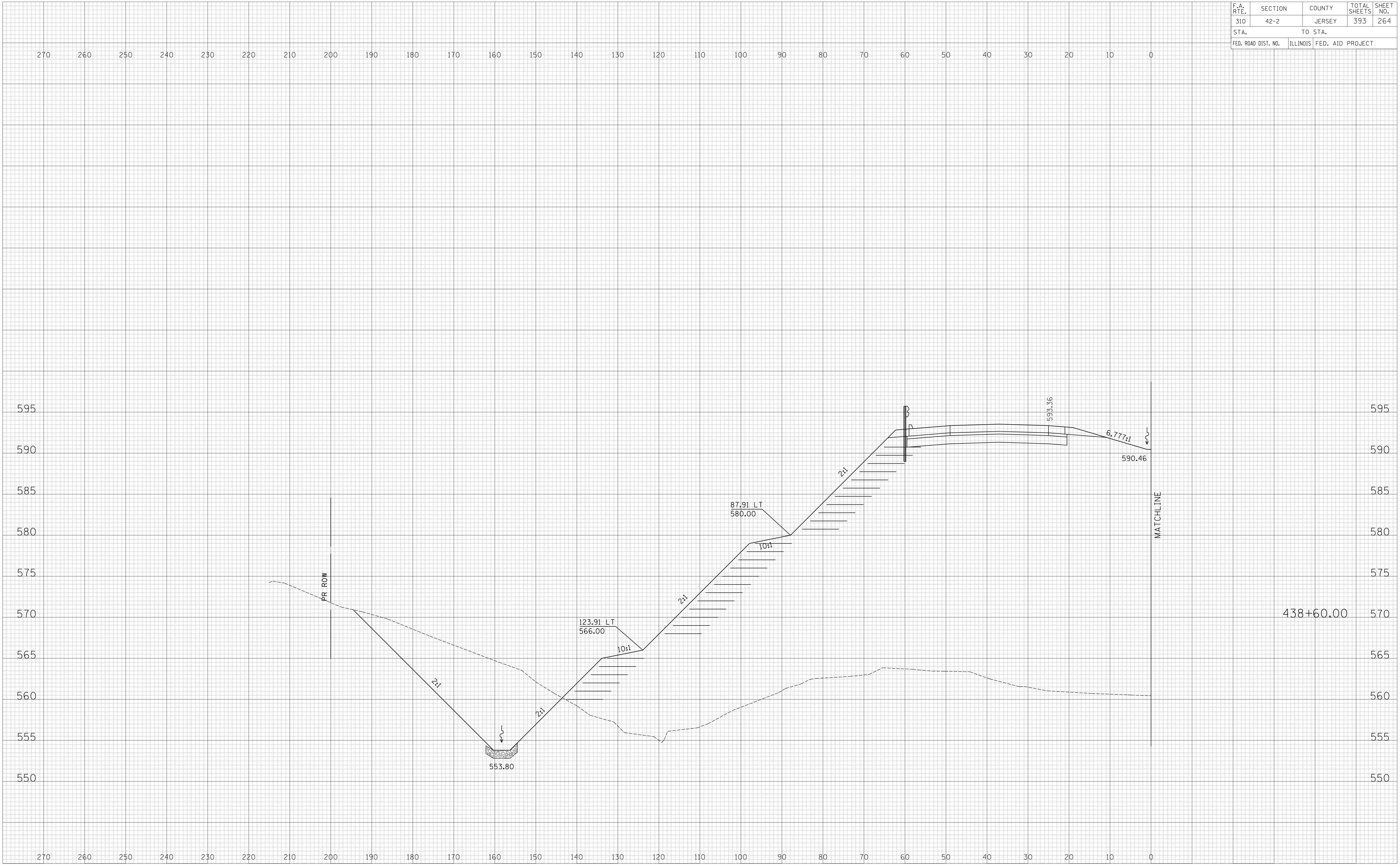
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DATE	BY

FINAL SURVEY
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 NOTE BOOK
 NO. _____
 AREAS CHECKED _____

DATE	BY
DATE	BY

ORIGINAL SURVEY
 SURVEYED
 PLOTTED
 NOTE BOOK
 NO. _____
 AREAS CHECKED _____

F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
310	42-2	JERSEY	393	264
STA.	TO STA.			
FED. ROAD DIST. NO.	ILLINOIS	FED. AID PROJECT		



DATE	BY
NO.	AREAS CHECKED
NO.	AREAS CHECKED

POWER L. CHASTAIN & ASSOCIATES, LLP
 100 N. LAUREL STREET, SUITE 200
 CHICAGO, ILLINOIS 60610
 (773) 714-0000
 (773) 714-0000
 (773) 714-0000

DATE	BY
NO.	AREAS CHECKED
NO.	AREAS CHECKED

F.A. RT.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
310	42-2	JERSEY	393	265
STA.	TO STA.			
FED. ROAD DIST. NO.	ILLINOIS	FED. AID PROJECT		



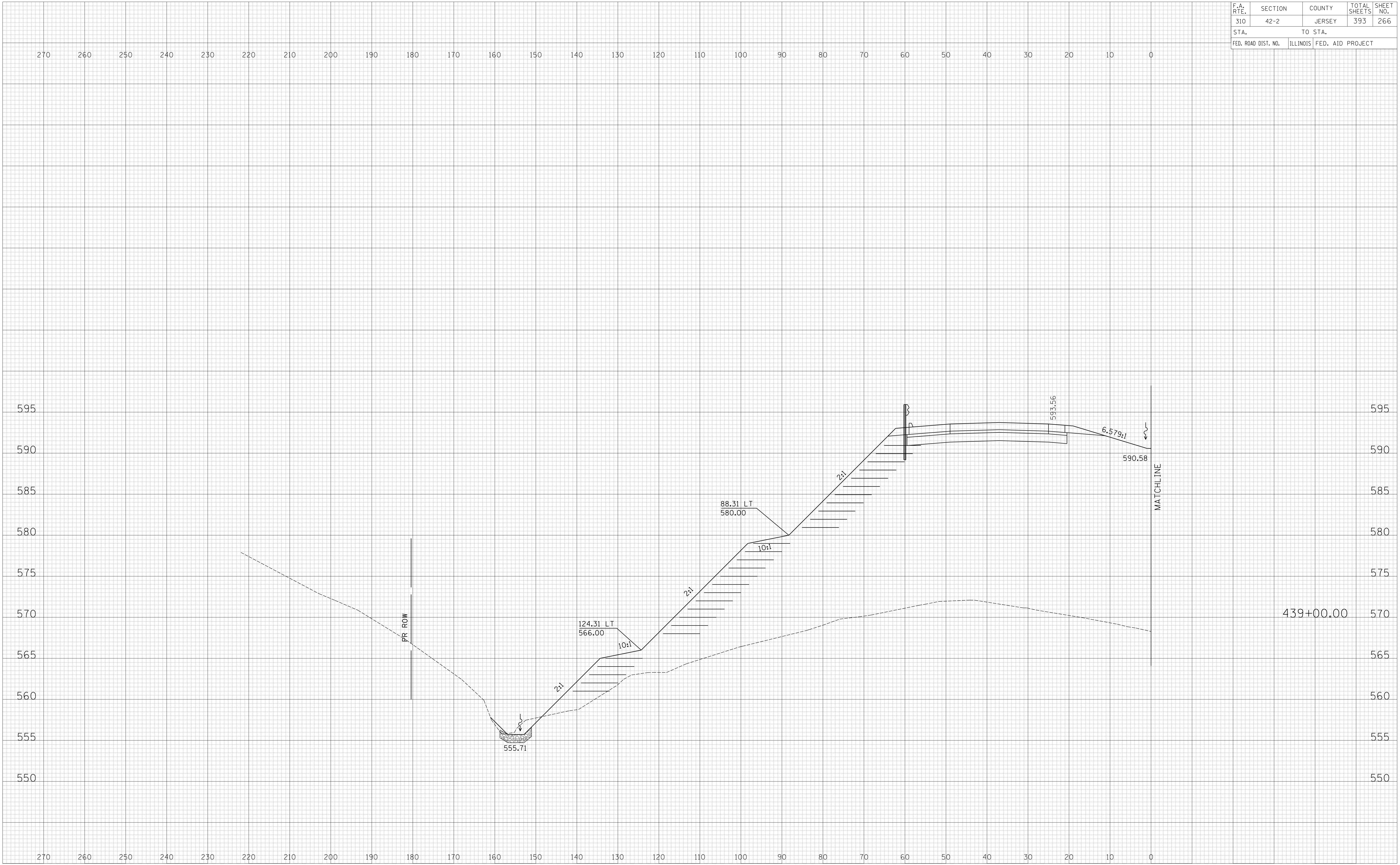
DATE	BY
NO.	NO.
NO.	NO.

POWER L
CHASTAIN
& ASSOCIATES, LLP
SURVEYORS
CHICAGO
ROCKFORD
TEL: 815-499-0000
FAX: 815-499-0000
WWW.CHASTAIN-PA.COM

DATE	BY
NO.	NO.
NO.	NO.

ORIGINAL
SURVEY
PLOTTED
DATE
AREAS CHECKED

F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
310	42-2	JERSEY	393	266
STA.	TO STA.			
FED. ROAD DIST. NO.	ILLINOIS	FED. AID PROJECT		



DATE	BY
NO.	AREAS CHECKED
NO.	AREAS CHECKED

POWER L
CHASTAIN
 &
ASSOCIATES, LLP
 SURVEYORS
 CHICAGO
 ROCKFORD
 815-426-4444
 815-998-9888
 10/4/2017

DATE	BY
NO.	AREAS CHECKED
NO.	AREAS CHECKED

F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
310	42-2	JERSEY	393	267
STA.	TO STA.			
FED. ROAD DIST. NO.	ILLINOIS	FED. AID PROJECT		

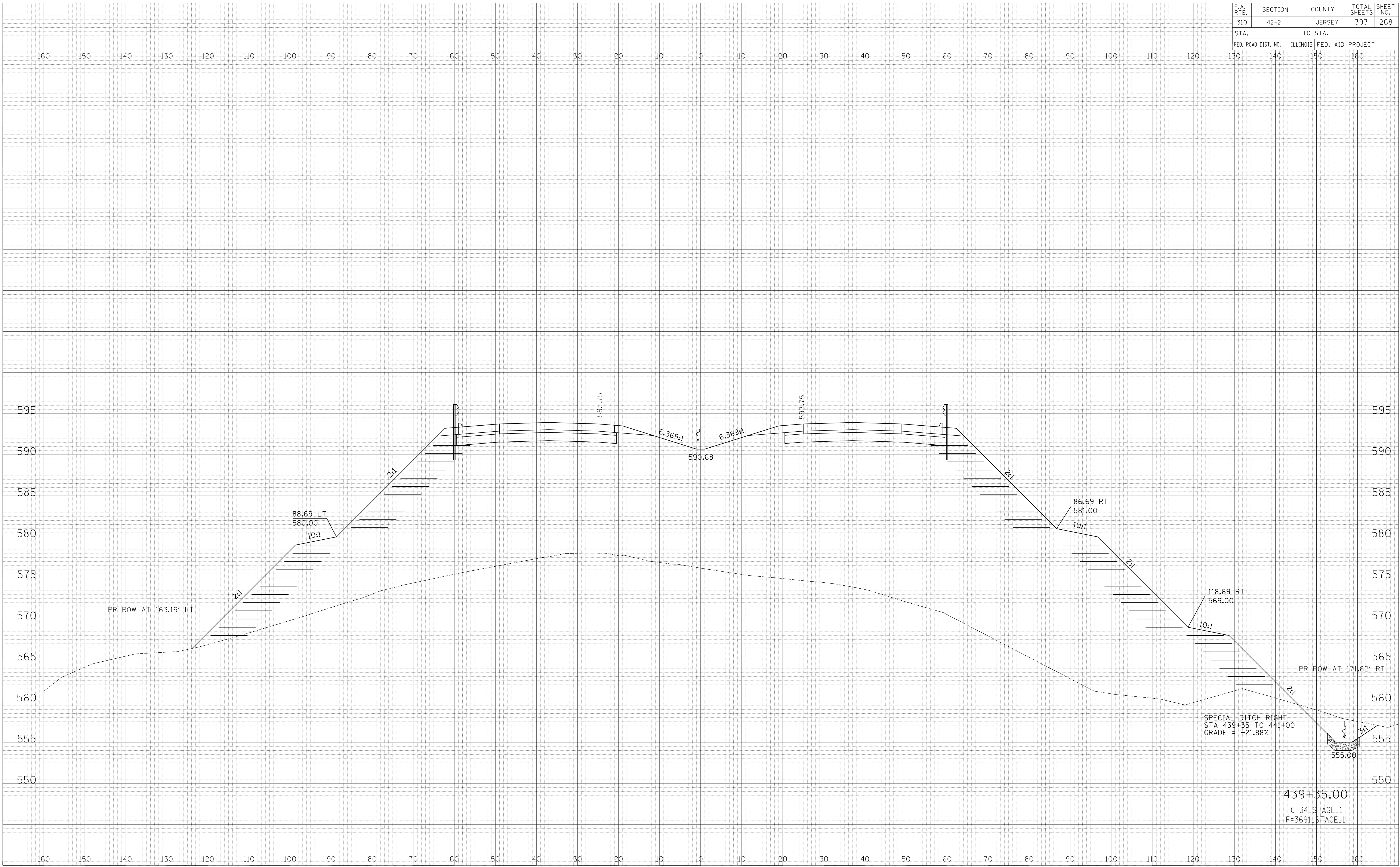


FINAL SURVEY	SURVEYED	DATE
NOTE BOOK	PLOTTED	
NO.	AREAS	
	CHECKED	

POWER L. CHASTAIN & ASSOCIATES, LLP
 100 N. LAUREL STREET, SUITE 1000
 CHICAGO, ILLINOIS 60602
 (773) 714-0000
 (815) 489-9000
 (312) 461-1377

ORIGINAL SURVEY	SURVEYED	DATE
NOTE BOOK	PLOTTED	
NO.	AREAS	
	CHECKED	

F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
310	42-2	JERSEY	393	268
STA.	TO STA.			
FED. ROAD DIST. NO.	ILLINOIS	FED. AID PROJECT		



DATE	BY

POWER L. CHASTAIN & ASSOCIATES, LLP
REGISTERED PROFESSIONAL ENGINEERS
CHICAGO
ILLINOIS
773.714.0000
www.chastain.com

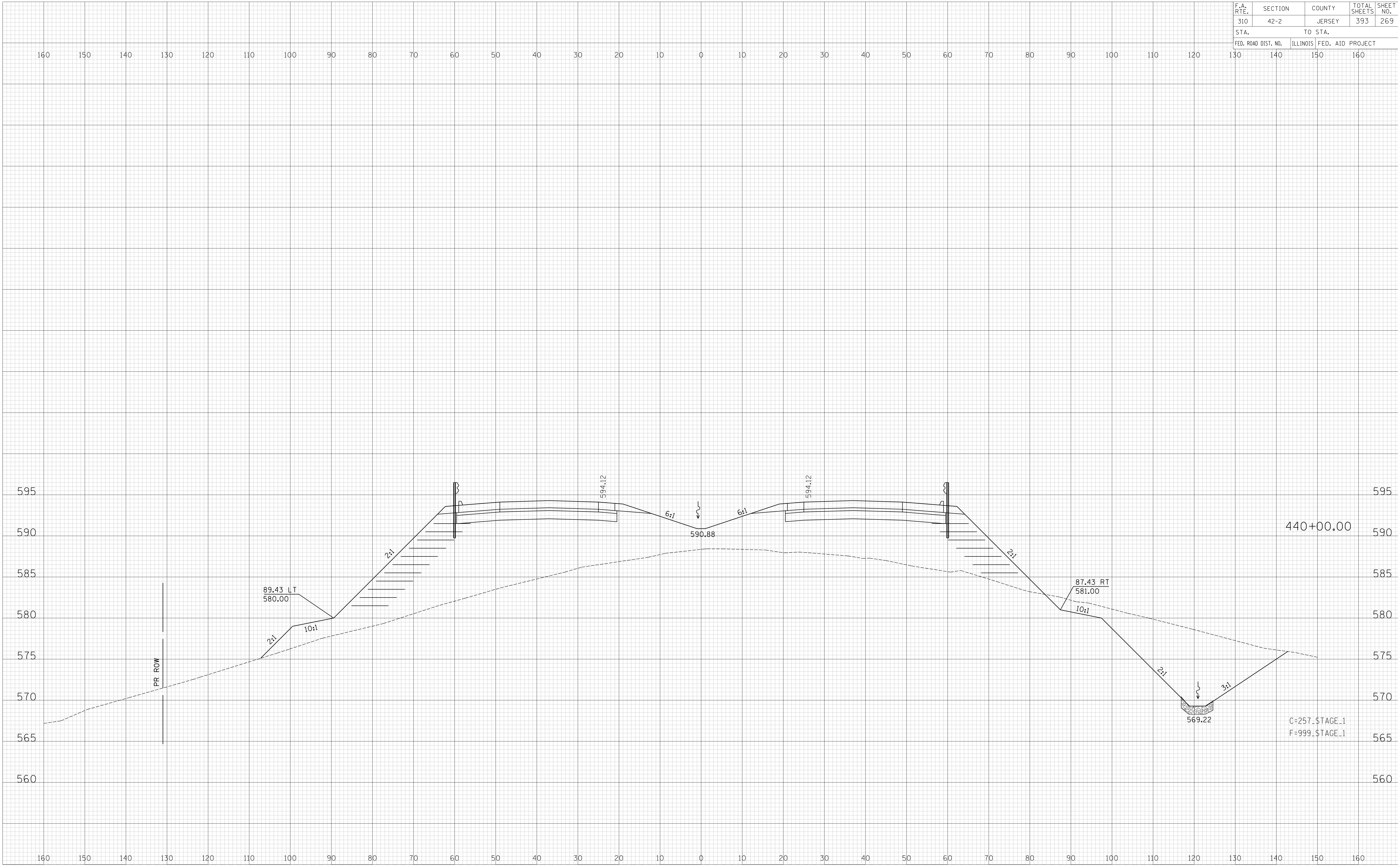
DATE	BY

F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
310	42-2	JERSEY	393	269
STA.	TO STA.			
FED. ROAD DIST. NO.	ILLINOIS	FED. AID PROJECT		

DATE	BY
NO.	NO.
AREAS CHECKED	AREAS CHECKED
PLOTTED DATE	PLOTTED DATE
NOTE BOOK	NOTE BOOK
SURVEYED	SURVEYED

POWER L
CHASTAIN
& ASSOCIATES, LLP
REGISTERED PROFESSIONAL ENGINEER
CHICAGO
773-714-0000
104-00137

DATE	BY
NO.	NO.
AREAS CHECKED	AREAS CHECKED
PLOTTED DATE	PLOTTED DATE
NOTE BOOK	NOTE BOOK
SURVEYED	SURVEYED



C=257_STAGE_1
F=999_STAGE_1

GRID REINFORCED EMBANKMENT

LEFT SIDE	DIST.	WIDTH FT.	AREA SQ.FT.	QUANTITY (1' vertical Spacing)	TOTAL SQ. FT.	RIGHT SIDE	DIST.	WIDTH FT.	AREA SQ.FT.	QUANTITY (1' vertical Spacing)	TOTAL SQ. FT.
346+90	10	9	90	16	1440	346+90	10	9	90	0	0
347+00	100	9	900	16	22950	347+00	100	9	900	0	0
348+00	100	9	900	35	27450	348+00	100	9	900	0	0
349+00	100	9	900	26	25650	349+00	100	9	900	24	23850
350+00	100	9	900	31	0	350+00	100	9	900	29	29250
351+00	25	9	225	0	0	351+00	25	9	225	36	8100
351+25				0	0	351+25				36	8100
435+90	10	9	90	31	2790	435+90	10	9	90	0	0
436+00	100	9	900	31	27450	436+00	100	9	900	0	9000
437+00	100	9	900	30	24750	437+00	100	9	900	20	20250
438+00	100	9	900	25	21600	438+00	100	9	900	25	25200
439+00	100	9	900	23	14400	439+00	100	9	900	31	13950
440+00	10	9	90	9	810	440+00	10	9	90	0	0
440+10				9		440+10				0	
TOTAL (SQ. FT.):					309690	FT ²					140400
TOTAL (SQ. YD.):					34410 YD ²						

Appendix G

Settlement Analyses

t₉₀ Calculations

ASSUMPTIONS

T = 0.848
 Single Drainage
 C_{VF} from Holtz & Kovacs, Page 404, Figure 9.10
 LL = Moisture Content

FORMULAS

$t_{90} = (T * H_{DR}^2) / C_{VF}$ *For a single layer*
 $t_{90} = [\sum (t_{90} * \text{settlement})] / \sum \text{settlement}$ *For the entire soil column*

Location	Station	Layer	Layer Height (ft)	C _{VF} (ft ² /d)	t ₉₀ (days)	Settlement (ft)
Mainline	347+50	1	1.8	0.9300	2.95	0.0050
		2	1.8	0.9300	2.95	0.0033
		3	1.8	0.9300	2.95	0.0033
		4	2.0	0.9300	3.65	0.0167
		5	2.0	0.6975	4.86	0.1250
		6	2.2	0.7905	5.19	0.0000
		7	2.2	0.9300	4.41	0.0000
		8	2.2	0.9300	4.41	0.0100
		Total				
Mainline	423+20	1	2.6	0.8370	6.85	0.0558
		2	2.6	0.5115	11.21	0.2575
		3	2.6	0.9300	6.16	0.0167
		4	1.0	0.9300	0.91	0.0000
		Total				
Mainline	423+67	1	3.8	0.6975	17.56	0.0533
		2	2.5	0.6975	7.60	0.0283
		3	3.5	0.9300	11.17	0.0150
		4	1.5	0.9300	2.05	0.0000
		5	3.0	0.9300	8.21	0.0000
		6	2.3	0.9300	4.82	0.0000
		7	2.3	0.9300	4.82	0.0000
		8	2.3	0.9300	4.82	0.0000
		9	2.3	0.9300	4.82	0.0000
		10	2.3	0.9300	4.82	0.0000
		11	2.3	0.9300	4.82	0.0000
Total					13.65	0.0967
US 67 Connector	26+65	1	2.4	0.6231	7.84	0.2525
		2	1.8	0.8835	3.11	0.0167
		3	1.8	0.9300	2.95	0.0150
		4	2.5	0.9300	5.70	0.0000
		5	1.8	0.9300	2.95	0.0000
		6	1.8	0.9300	2.95	0.0000
		7	1.8	0.9300	2.95	0.0000
		Total				

Settlement Analyses

Station 333+19.67 to Station 463+50

SETTLEMENT ANALYSIS

I.D.O.T. BBS FOUNDATIONS AND GEOTECHNICAL UNIT

Modified on 1/31/05

UNIT OR ENGLISH ===== 1 (1=ENGLISH, 2=METRIC)
 TYPE OF SURCHARGE ===== 2 (1=bridge cone, 2=continuous embankment, 3=rectangular)
 DEPTH TO WATER TABLE (below surf. of exist. embank. or exist. rectang. surch.) ===== FT.

NEW EMBANKMENT:

NEW EMBANKMENT FILL UNIT WEIGHT ===== 120 PCF.
 NEW EMBANKMENT FILL HEIGHT ===== 20.38 FT.
 PROPOSED WIDTH AT TOP ===== 124 FT.
 PROPOSED WIDTH AT BOTTOM ===== 244 FT. WHICH WOULD BE A 2.9:1 SIDE SLOPE
 PROPOSED LENGTH OF EMBANK. OR SURCHARGE (RECTANGULAR ONLY) ===== 0 FT.

EXISTING EMBANKMENT (IF ANY):

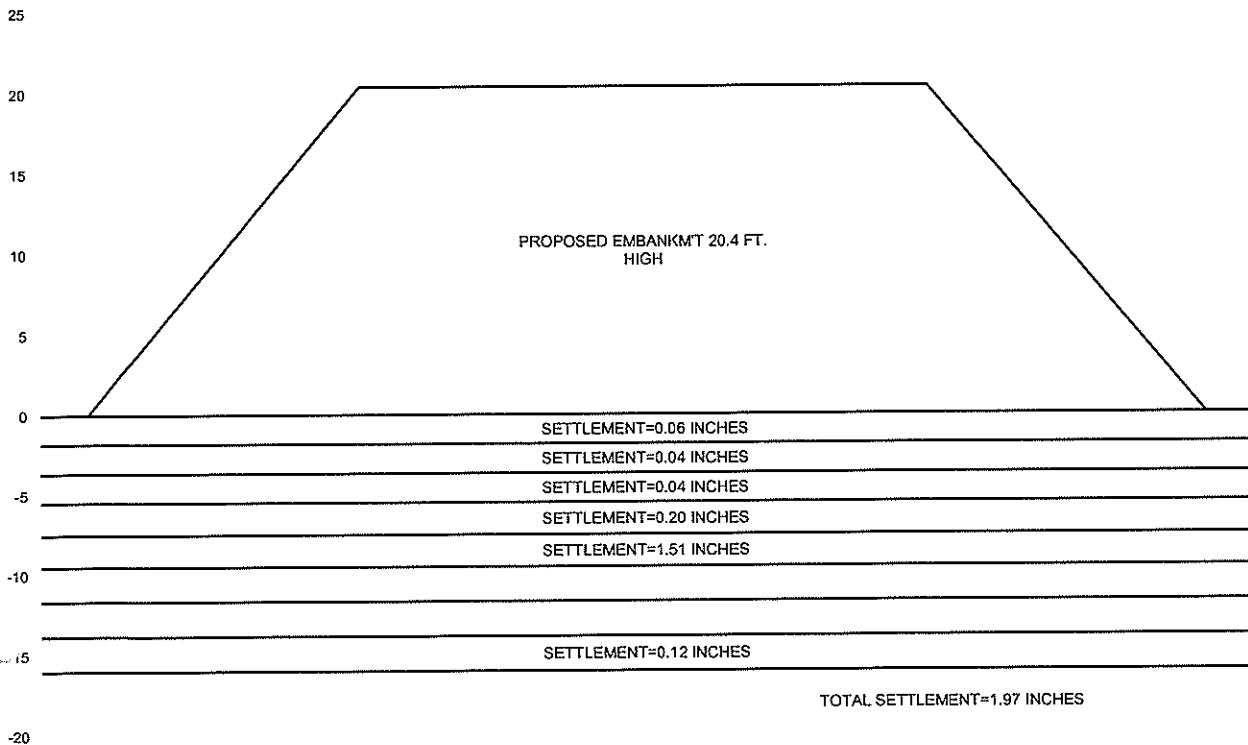
EXIST. EMBANKMENT UNIT WEIGHT ===== PCF.
 EXIST. EMBANKMENT HEIGHT ===== FT.
 WIDTH AT TOP ===== FT.
 WIDTH AT BASE ===== FT. WHICH WOULD BE A 0.0:1 SIDE SLOPE
 EXISTING LENGTH OF EMBANK. OR SURCHARGE (RECTANGULAR ONLY) ===== FT.

LAYER THICK (FT.)	TOTAL UNIT WT. (PCF.)	UNCONFINED COMP. STR. (TSF.)	MOIST. CONTENT (%)	OVERBURDEN PRESSURE (KSF.)	PRESSURE INCREASE (KSF.)	INITIAL VOID RATIO	COMPRESSION INDEX, Cc	LAYER SETTLEMENT (IN.)
1.8	120	0.60	11	0.053	2.446	0.297	0.002	0.06
1.8	120	0.60	11	0.158	2.446	0.297	0.002	0.04
1.8	120	0.60	11	0.264	2.445	0.297	0.002	0.04
2.0	120	0.80	17	0.374	2.445	0.459	0.014	0.20
2.0	120	0.38	25	0.489	2.445	0.675	0.135	1.51
2.2	120	4.00	23	0.609	2.444	0.621	0.000	0.00
2.2	120	2.40	20	0.734	2.442	0.540	0.000	0.00
2.2	120	1.50	19	0.859	2.440	0.513	0.012	0.12

ASSUMPTIONS:
 SOIL IS NORMALLY CONSOLIDATED
 SOIL IS SATURATED
 $E_o = 2.7 * (\text{MOIST CONT.} \%) / 100$
 $C_c = 0.009 * (LL - 10)$
 LL = MOIST CONT. %
 SOIL HAS A LOW SENSITIVITY

TOTAL SETTLEMENT UNDER CENTER OF CONTINUOUS SURCHARGE = 1.97 IN

EMBANKMENT AND SOIL PROFILE



SETTLEMENT ANALYSIS

I.D.O.T. BBS FOUNDATIONS AND GEOTECHNICAL UNIT

Modified on 1/31/05

UNIT SYSTEM (1=ENGLISH, 2=METRIC) ===== 1 (1=ENGLISH, 2=METRIC)
 TYPE OF SURCHARGE ===== 2 (1=bridge cone, 2=continuous embankment, 3=rectangular)
 DEPTH TO WATER TABLE (below surf. of exist. embank. or exist. rectang. surch.) == N/A FT.

NEW EMBANKMENT:

NEW EMBANKMENT FILL UNIT WEIGHT ===== 120 PCF.
 NEW EMBANKMENT FILL HEIGHT ===== 16.5 FT.
 PROPOSED WIDTH AT TOP ===== 118 FT.
 PROPOSED WIDTH AT BOTTOM ===== 250 FT. WHICH WOULD BE A 4.0:1 SIDE SLOPE
 PROPOSED LENGTH OF EMBANK. OR SURCHARGE (RECTANGULAR ONLY) ===== 0 FT.

EXISTING EMBANKMENT (IF ANY):

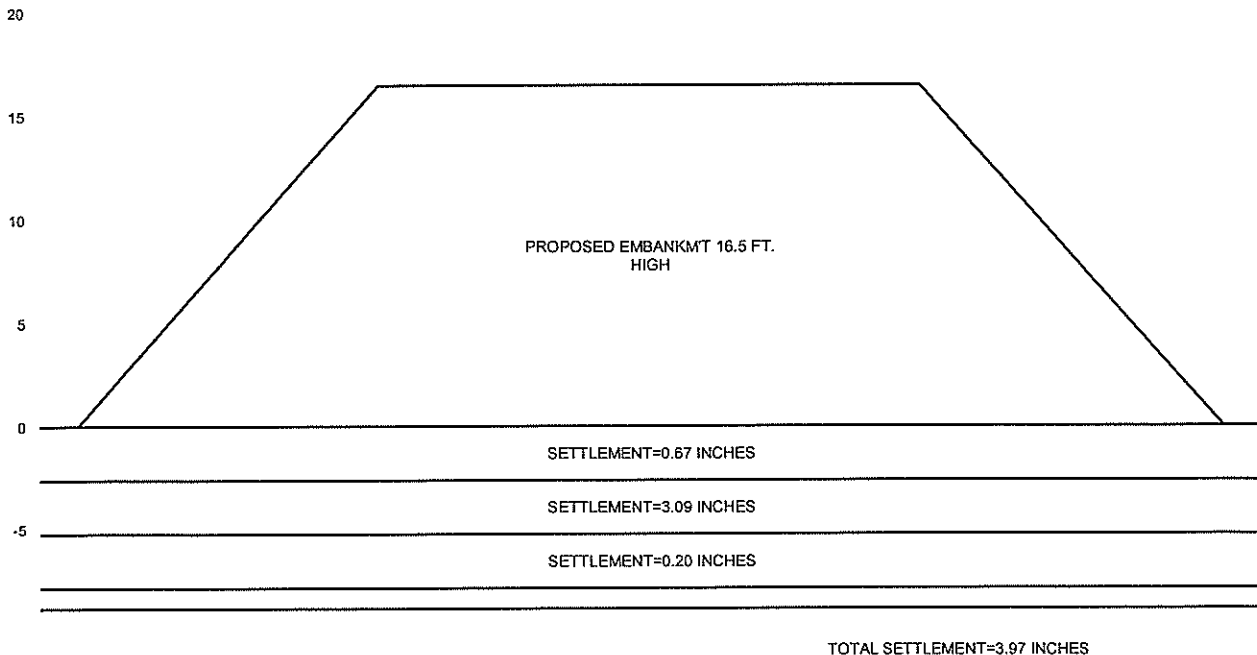
EXIST. EMBANKMENT UNIT WEIGHT ===== PCF.
 EXIST. EMBANKMENT HEIGHT ===== FT.
 WIDTH AT TOP ===== FT.
 WIDTH AT BASE ===== FT. WHICH WOULD BE A 0.0:1 SIDE SLOPE
 EXISTING LENGTH OF EMBANK. OR SURCHARGE (RECTANGULAR ONLY) ===== FT.

LAYER THICK (FT.)	TOTAL UNIT WT. (PCF.)	UNCONFINED COMP. STR. (TSF.)	MOIST. CONTENT (%)	OVERBURDEN PRESSURE (KSF.)	PRESSURE INCREASE (KSF.)	INITIAL VOID RATIO	COMPRESSION INDEX, Cc	LAYER SETTLEMENT (IN.)
2.6	120	0.80	22	0.075	1.980	0.594	0.024	0.67
2.6	120	0.30	30	0.226	1.980	0.810	0.180	3.09
2.6	120	1.60	20	0.377	1.980	0.540	0.013	0.20
1.0	120	0.00	0	0.481	1.979	0.000	0.000	0.00

ASSUMPTIONS:
 SOIL IS NORMALLY CONSOLIDATED
 SOIL IS SATURATED
 $E_s = 2.7 * (\text{MOIST. CONT. \%}) / 100$
 $C_c = 0.009 * (LL - 10)$
 $LL = \text{MOIST. CONT. \%}$
 SOIL HAS A LOW SENSITIVITY

TOTAL SETTLEMENT UNDER CENTER OF CONTINUOUS SURCHARGE = 3.97 IN

EMBANKMENT AND SOIL PROFILE



SETTLEMENT ANALYSIS

I.D.O.T. BBS FOUNDATIONS AND GEOTECHNICAL UNIT

Modified on 1/31/05

UNIT OR ENGLISH ===== 1 (1=ENGLISH, 2=METRIC)
 TYPE OF SURCHARGE ===== 2 (1=bridge cone, 2=continuous embankment, 3=rectangular)
 DEPTH TO WATER TABLE (below surf. of exist. embank. or exist. rectang. surch.) == 12.80 FT.

NEW EMBANKMENT:

NEW EMBANKMENT FILL UNIT WEIGHT ===== 120 PCF.
 NEW EMBANKMENT FILL HEIGHT ===== 16 FT.
 PROPOSED WIDTH AT TOP ===== 118 FT.
 PROPOSED WIDTH AT BOTTOM ===== 248 FT. WHICH WOULD BE A 4.1:1 SIDE SLOPE
 PROPOSED LENGTH OF EMBANK. OR SURCHARGE (RECTANGULAR ONLY) ===== 0 FT.

EXISTING EMBANKMENT (IF ANY):

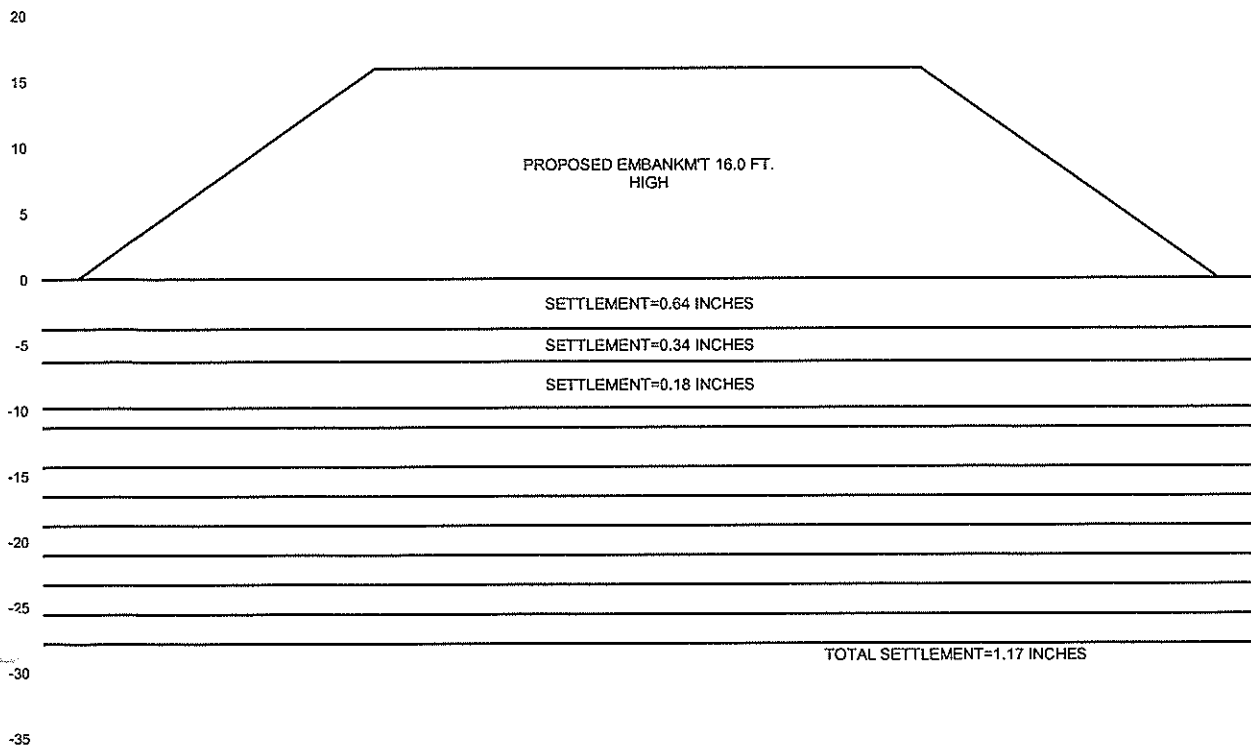
EXIST. EMBANKMENT UNIT WEIGHT ===== PCF.
 EXIST. EMBANKMENT HEIGHT ===== FT.
 WIDTH AT TOP ===== FT.
 WIDTH AT BASE ===== FT. WHICH WOULD BE A 0.0:1 SIDE SLOPE
 EXISTING LENGTH OF EMBANK. OR SURCHARGE (RECTANGULAR ONLY) ===== FT.

LAYER THICK (FT.)	TOTAL UNIT WT. (PCF.)	UNCONFINED COMP. STR. (TSF.)	MOIST. CONTENT (%)	OVERBURDEN PRESSURE (KSF.)	PRESSURE INCREASE (KSF.)	INITIAL VOID RATIO	COMPRESSION INDEX, Cc	LAYER SETTLEMENT (IN.)
3.8	120	1.20	25	0.228	1.920	0.675	0.024	0.64
2.5	120	0.70	25	0.605	1.920	0.675	0.031	0.34
3.5	120	1.30	19	0.965	1.919	0.513	0.014	0.18
1.5	120	0.00	0	1.265	1.918	0.000	0.000	0.00
3.0	120	0.00	0	1.535	1.917	0.000	0.000	0.00
2.3	120	6.80	14	1.687	1.915	0.378	0.000	0.00
2.3	120	4.90	17	1.816	1.913	0.459	0.000	0.00
2.3	120	5.20	16	1.946	1.910	0.432	0.000	0.00
2.3	120	5.90	14	2.075	1.906	0.378	0.000	0.00
2.3	120	4.20	16	2.205	1.902	0.432	0.000	0.00
2.3	120	2.90	20	2.335	1.897	0.540	0.000	0.00

ASSUMPTIONS:
 SOIL IS NORMALLY CONSOLIDATED
 SOIL IS SATURATED
 $E_o = 2.7 * (\text{MOIST CONT.} \%) / 100$
 $C_c = 0.009 * (LL - 10)$
 LL = MOIST CONT. %
 SOIL HAS A LOW SENSITIVITY

TOTAL SETTLEMENT UNDER CENTER OF CONTINUOUS SURCHARGE = 1.17 IN

EMBANKMENT AND SOIL PROFILE



Settlement Analyses

US 67 Connector

SETTLEMENT ANALYSIS

I.D.O.T. BBS FOUNDATIONS AND GEOTECHNICAL UNIT

Modified on 1/31/05

UNIT OF MEASURE (1=ENGLISH, 2=METRIC) ===== 1 (1=ENGLISH, 2=METRIC)
 TYPE OF SURCHARGE ===== 2 (1=bridge cone, 2=continuous embankment, 3=rectangular)
 DEPTH TO WATER TABLE (below surf. of exist. embank. or exist. rectang. surch.) == 8.43 FT.

NEW EMBANKMENT:

NEW EMBANKMENT FILL UNIT WEIGHT ===== 120 PCF.
 NEW EMBANKMENT FILL HEIGHT ===== 17 FT.
 PROPOSED WIDTH AT TOP ===== 32 FT.
 PROPOSED WIDTH AT BOTTOM ===== 133 FT. WHICH WOULD BE A 3.0:1 SIDE SLOPE
 PROPOSED LENGTH OF EMBANK. OR SURCHARGE (RECTANGULAR ONLY) ===== FT.

EXISTING EMBANKMENT (IF ANY):

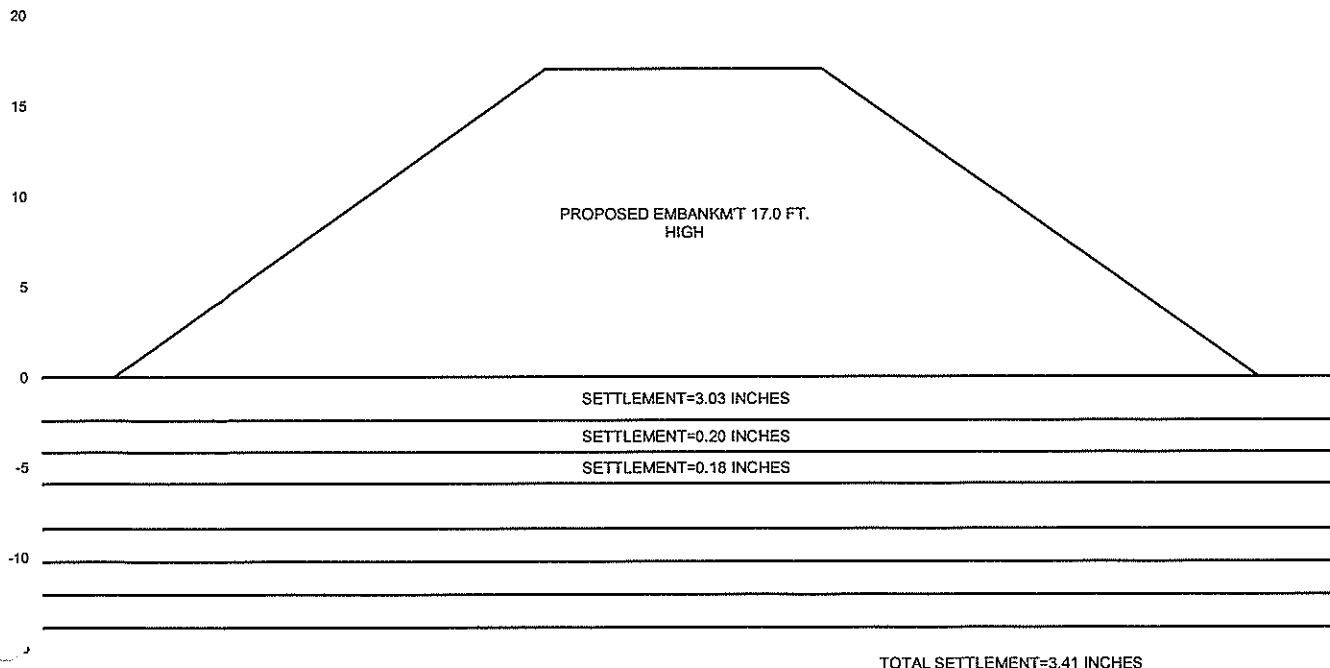
EXIST. EMBANKMENT UNIT WEIGHT ===== PCF.
 EXIST. EMBANKMENT HEIGHT ===== FT.
 WIDTH AT TOP ===== FT.
 WIDTH AT BASE ===== FT. WHICH WOULD BE A 0.0:1 SIDE SLOPE
 EXISTING LENGTH OF EMBANK. OR SURCHARGE (RECTANGULAR ONLY) ===== FT.

LAYER THICK (FT.)	TOTAL UNIT WT. (PCF.)	UNCONFINED COMP. STR. (TSF.)	MOIST. CONTENT (%)	OVERBURDEN PRESSURE (KSF.)	PRESSURE INCREASE (KSF.)	INITIAL VOID RATIO	COMPRESSION INDEX, Cc	LAYER SETTLEMENT (IN.)
2.4	120	0.00	27	0.146	2.040	0.729	0.153	3.03
1.8	120	1.10	21	0.397	2.039	0.567	0.019	0.20
1.8	120	0.50	19	0.607	2.036	0.513	0.020	0.18
2.5	120	0.00	0	0.862	2.030	0.000	0.000	0.00
1.8	120	4.00	12	1.064	2.019	0.324	0.000	0.00
1.8	120	7.50	12	1.170	2.006	0.324	0.000	0.00
1.8	120	4.50	12	1.275	1.991	0.324	0.000	0.00

ASSUMPTIONS:
 SOIL IS NORMALLY CONSOLIDATED
 SOIL IS SATURATED
 $E_o = 2.7 * (\text{MOIST CONT.} \%) / 100$
 $C_c = 0.009 * (LL - 10)$
 LL = MOIST CONT. %
 SOIL HAS A LOW SENSITIVITY

TOTAL SETTLEMENT UNDER CENTER OF CONTINUOUS SURCHARGE = 3.41 IN

EMBANKMENT AND SOIL PROFILE



SETTLEMENT ANALYSIS

I.D.O.T. BBS FOUNDATIONS AND GEOTECHNICAL UNIT

Modified on 1/31/05

LANGUAGE OR ENGLISH ===== 1 (1=ENGLISH, 2=METRIC)
 TYPE OF SURCHARGE ===== 2 (1=bridge cone, 2=continuous embankment, 3=rectangular)
 DEPTH TO WATER TABLE (below surf. of exist. embank. or exist. rectang. surch.) == 8.43 FT.

NEW EMBANKMENT:

NEW EMBANKMENT FILL UNIT WEIGHT ===== 120 PCF.
 NEW EMBANKMENT FILL HEIGHT ===== 17 FT.
 PROPOSED WIDTH AT TOP ===== 32 FT.
 PROPOSED WIDTH AT BOTTOM ===== 133 FT. WHICH WOULD BE A 3.0:1 SIDE SLOPE
 PROPOSED LENGTH OF EMBANK. OR SURCHARGE (RECTANGULAR ONLY) ===== FT.

EXISTING EMBANKMENT (IF ANY):

EXIST. EMBANKMENT UNIT WEIGHT ===== PCF.
 EXIST. EMBANKMENT HEIGHT ===== FT.
 WIDTH AT TOP ===== FT.
 WIDTH AT BASE ===== FT. WHICH WOULD BE A 0.0:1 SIDE SLOPE
 EXISTING LENGTH OF EMBANK. OR SURCHARGE (RECTANGULAR ONLY) ===== FT.

LAYER THICK (FT.)	TOTAL UNIT WT. (PCF.)	UNCONFINED COMP. STR. (TSF.)	MOIST. CONTENT (%)	OVERBURDEN PRESSURE (KSF.)	PRESSURE INCREASE (KSF.)	INITIAL VOID RATIO	COMPRESSION INDEX, Cc	LAYER SETTLEMENT (IN.)
2.4	120	0.00	0	0.146	2.040	0.000	0.000	0.00
1.8	120	1.10	21	0.397	2.039	0.567	0.019	0.20
1.8	120	0.50	19	0.607	2.036	0.513	0.020	0.18
2.5	120	0.00	0	0.862	2.030	0.000	0.000	0.00
1.8	120	4.00	12	1.064	2.019	0.324	0.000	0.00
1.8	120	7.50	12	1.170	2.006	0.324	0.000	0.00
1.8	120	4.50	12	1.275	1.991	0.324	0.000	0.00

ASSUMPTIONS:
 SOIL IS NORMALLY CONSOLIDATED
 SOIL IS SATURATED
 $E_o = 2.7 * (\text{MOIST CONT.} \%) / 100$
 $C_c = 0.009 * (LL - 10)$
 LL = MOIST CONT. %
 SOIL HAS A LOW SENSITIVITY

TOTAL SETTLEMENT UNDER CENTER OF CONTINUOUS SURCHARGE = 0.38 IN

EMBANKMENT AND SOIL PROFILE

