

1X

**For Review and Inspection Only
Letting August 1, 2025**

Notice to Bidders, Specifications and Proposal



**Contract No. 68C55
TAZEWELL County
Section 13[(HVB,HB-3)]BR-1;R
Route FAP 669,FAP 673
Project NHPP-7VBW(880)
District 4 Construction Funds**

Prepared by

F

Checked by

(Printed by authority of the State of Illinois)



**Illinois Department
of Transportation**

NOTICE TO BIDDERS

1. **TIME AND PLACE OF OPENING BIDS.** Electronic bids are to be submitted to the electronic bidding system (iCX-Integrated Contractors Exchange). All bids must be submitted to the iCX system prior to 12:00 p.m. August 1, 2025 at which time the bids will be publicly opened from the iCX SecureVault.
2. **DESCRIPTION OF WORK.** The proposed improvement is identified and advertised for bids in the Invitation for Bids as:

**Contract No. 68C55
TAZEWELL County
Section 13[(HVB,HB-3)]BR-1;R
Project NHPP-7VBW(880)
Route FAP 669,FAP 673
District 4 Construction Funds**

Bridge replacement carrying IL 8/IL 116 (Cedar St. Ext.) over TZPR RR and over IL 29. This includes reconstruction of the IL 8/IL 116 interchange with IL 29/US 24 (S. Main St.) in East Peoria in Tazewell County.

3. **INSTRUCTIONS TO BIDDERS.** (a) This Notice, the invitation for bids, proposal and letter of award shall, together with all other documents in accordance with Article 101.09 of the Standard Specifications for Road and Bridge Construction, become part of the contract. Bidders are cautioned to read and examine carefully all documents, to make all required inspections, and to inquire or seek explanation of the same prior to submission of a bid.

(b) State law, and, if the work is to be paid wholly or in part with Federal-aid funds, Federal law requires the bidder to make various certifications as a part of the proposal and contract. By execution and submission of the proposal, the bidder makes the certification contained therein. A false or fraudulent certification shall, in addition to all other remedies provided by law, be a breach of contract and may result in termination of the contract.
4. **AWARD CRITERIA AND REJECTION OF BIDS.** This contract will be awarded to the lowest responsive and responsible bidder considering conformity with the terms and conditions established by the Department in the rules, Invitation for Bids and contract documents. The issuance of plans and proposal forms for bidding based upon a prequalification rating shall not be the sole determinant of responsibility. The Department reserves the right to determine responsibility at the time of award, to reject any or all proposals, to re-advertise the proposed improvement, and to waive technicalities.

By Order of the
Illinois Department of Transportation

Gia Biagi,
Secretary

INDEX
 FOR
 SUPPLEMENTAL SPECIFICATIONS
 AND RECURRING SPECIAL PROVISIONS

Adopted January 1, 2025

This index contains a listing of SUPPLEMENTAL SPECIFICATIONS and frequently used RECURRING SPECIAL PROVISIONS.

ERRATA Standard Specifications for Road and Bridge Construction (Adopted 1-1-22) (Revised 1-1-25)

SUPPLEMENTAL SPECIFICATIONS

| <u>Std. Spec. Sec.</u> | <u>Page No.</u> |
|---|-----------------|
| 202 Earth and Rock Excavation | 1 |
| 204 Borrow and Furnished Excavation..... | 2 |
| 207 Porous Granular Embankment | 3 |
| 211 Topsoil and Compost | 4 |
| 406 Hot-Mix Asphalt Binder and Surface Course | 5 |
| 407 Hot-Mix Asphalt Pavement (Full-Depth) | 7 |
| 420 Portland Cement Concrete Pavement | 8 |
| 502 Excavation for Structures | 9 |
| 509 Metal Railings | 10 |
| 540 Box Culverts | 11 |
| 542 Pipe Culverts | 31 |
| 550 Storm Sewers | 40 |
| 586 Granular Backfill for Structures | 47 |
| 630 Steel Plate Beam Guardrail | 48 |
| 632 Guardrail and Cable Road Guard Removal | 49 |
| 644 High Tension Cable Median Barrier | 50 |
| 665 Woven Wire Fence | 51 |
| 701 Work Zone Traffic Control and Protection | 52 |
| 781 Raised Reflective Pavement Markers | 54 |
| 782 Reflectors | 55 |
| 801 Electrical Requirements | 57 |
| 821 Roadway Luminaires | 60 |
| 1003 Fine Aggregates | 61 |
| 1004 Coarse Aggregates | 62 |
| 1010 Finely Divided Minerals | 63 |
| 1020 Portland Cement Concrete | 64 |
| 1030 Hot-Mix Asphalt | 67 |
| 1040 Drain Pipe, Tile, and Wall Drain | 68 |
| 1061 Waterproofing Membrane System | 69 |
| 1067 Luminaire | 70 |
| 1097 Reflectors | 77 |
| 1102 Hot-Mix Asphalt Equipment | 78 |

RECURRING SPECIAL PROVISIONS

The following RECURRING SPECIAL PROVISIONS indicated by an "X" are applicable to this contract and are included by reference:

| <u>CHECK SHEET #</u> | | <u>PAGE NO.</u> |
|----------------------|--|-----------------|
| 1 | X Additional State Requirements for Federal-Aid Construction Contracts | 79 |
| 2 | X Subletting of Contracts (Federal-Aid Contracts) | 82 |
| 3 | X EEO | 83 |
| 4 | Specific EEO Responsibilities Nonfederal-Aid Contracts | 93 |
| 5 | Required Provisions - State Contracts | 98 |
| 6 | Asbestos Bearing Pad Removal | 104 |
| 7 | Asbestos Waterproofing Membrane and Asbestos Hot-Mix Asphalt Surface Removal | 105 |
| 8 | Temporary Stream Crossings and In-Stream Work Pads | 106 |
| 9 | X Construction Layout Stakes | 107 |
| 10 | Use of Geotextile Fabric for Railroad Crossing | 110 |
| 11 | Subsealing of Concrete Pavements | 112 |
| 12 | Hot-Mix Asphalt Surface Correction | 116 |
| 13 | Pavement and Shoulder Resurfacing | 118 |
| 14 | Patching with Hot-Mix Asphalt Overlay Removal | 119 |
| 15 | Polymer Concrete | 121 |
| 16 | Reserved | 123 |
| 17 | Bicycle Racks | 124 |
| 18 | Temporary Portable Bridge Traffic Signals | 126 |
| 19 | X Nighttime Inspection of Roadway Lighting | 128 |
| 20 | English Substitution of Metric Bolts | 129 |
| 21 | Calcium Chloride Accelerator for Portland Cement Concrete | 130 |
| 22 | X Quality Control of Concrete Mixtures at the Plant | 131 |
| 23 | X Quality Control/Quality Assurance of Concrete Mixtures | 139 |
| 24 | Reserved | 155 |
| 25 | Reserved | 156 |
| 26 | Temporary Raised Pavement Markers | 157 |
| 27 | Restoring Bridge Approach Pavements Using High-Density Foam | 158 |
| 28 | Portland Cement Concrete Inlay or Overlay | 161 |
| 29 | Portland Cement Concrete Partial Depth Hot-Mix Asphalt Patching | 165 |
| 30 | Longitudinal Joint and Crack Patching | 168 |
| 31 | X Concrete Mix Design – Department Provided | 170 |
| 32 | X Station Numbers in Pavements or Overlays | 171 |

TABLE OF CONTENTS

| | |
|--|----|
| LOCATION OF PROJECT | 1 |
| DESCRIPTION OF PROJECT | 2 |
| DATE OF COMPLETION (PLUS WORKING DAYS)..... | 2 |
| CONSTRUCTION LAYOUT RESPONSIBILITY | 2 |
| CONSTRUCTION LAYOUT UTILIZING GPS EQUIPMENT..... | 3 |
| CONSTRUCTION LAYOUT EQUIPMENT | 3 |
| REMOVAL OF ABANDONED UNDERGROUND UTILITIES | 4 |
| UTILITIES – LOCATIONS/INFORMATION ON PLANS | 4 |
| LOCATION OF UNDERGROUND STATE MAINTAINED FACILITIES | 5 |
| EMBANKMENT (RESTRICTIONS)..... | 5 |
| MOWING | 6 |
| PROOF ROLLING | 6 |
| SUBGRADE TREATMENT | 6 |
| TEMPORARY PAVEMENT..... | 7 |
| HOT-MIX ASPHALT SURFACE COURSE SURFACE TESTS | 7 |
| PROTECTION OF FRAMES AND LIDS OF UTILITY STRUCTURES | 8 |
| HOT-MIX ASPHALT SURFACE REMOVAL, 2" | 8 |
| CONCRETE HEADWALL REMOVAL | 11 |
| GRANULAR BACKFILL FOR STRUCTURES..... | 11 |
| PCC PLACEMENT BY PUMP REQUIREMENTS | 11 |
| STORM SEWER, (WATER MAIN QUALITY PIPE)..... | 12 |
| BACKFILL, BUILDING REMOVAL..... | 14 |
| INLETS, TYPE G-1 | 15 |
| INLETS, TYPE G-1, SPECIAL | 15 |
| INLET-MANHOLE, TYPE G-1, 6' DIAMETER | 15 |
| INLET-MANHOLE, TYPE G-1, 8' DIAMETER..... | 16 |
| INLETS, TYPE G-1, DOUBLE..... | 16 |
| INLETS, TYPE G-1, DOUBLE, SPECIAL..... | 16 |
| INLET-MANHOLE, TYPE G-1, 5' (1.5 M) DIAMETER..... | 17 |
| INLET-MANHOLE, TYPE G-1, 8' (2.4 M) DIAMETER, DOUBLE, SPECIAL..... | 17 |
| GUARDRAIL AGGREGATE EROSION CONTROL | 17 |
| TRAFFIC BARRIER TERMINALS..... | 18 |
| EQUIPMENT VAULT FOR NUCLEAR TESTING EQUIPMENT..... | 18 |

| | |
|---|----|
| TRAFFIC CONTROL PLAN | 18 |
| TRAFFIC CONTROL AND PROTECTION (SPECIAL)..... | 20 |
| INCENTIVE/DISINCENTIVE FOR REPLACEMENT OF STRUCTURES OVER TPZ RAILROAD, MAIN STREET AND MAINLINE AND RAMP PAVEMENT REPLACEMENT | 21 |
| PROSECUTION OF WORK..... | 22 |
| WIDTH RESTRICTION SIGNING | 22 |
| TRAFFIC CONTROL AND PROTECTION BLR 21 | 24 |
| TRAFFIC CONTROL AND PROTECTION BLR 22 | 25 |
| TEMPORARY CONCRETE BARRIER REFLECTORS | 25 |
| PCC SLIPFORM PAVING AGGREGATE OPTIMIZATION | 25 |
| PCC SUPERSTRUCTURE AGGREGATE OPTIMIZATION..... | 25 |
| MEMBRANE CURING METHOD..... | 26 |
| PCC QMP ELECTRONIC REPORT SUBMITTALS | 26 |
| PCC AUTOMATIC BATCHING EQUIPMENT | 27 |
| RETAINING WALL REMOVAL | 27 |
| APPROACH SLAB REMOVAL | 27 |
| FENCE REMOVAL | 28 |
| PORTLAND CEMENT CONCRETE SIDEWALK 4 INCH (SPECIAL)..... | 28 |
| TEMPORARY PAVEMENT REMOVAL..... | 29 |
| CONCRETE GUTTER, TYPE A (SPECIAL) | 29 |
| EROSION CONTROL BLANKET (SPECIAL)..... | 29 |
| REMOVAL AND DISPOSAL OF REGULATED SUBSTANCES | 30 |
| CONNECTION TO EXISTING DRAINAGE STRUCTURE | 39 |
| REMOVE EXISTING FLARED END SECTION..... | 39 |
| INLETS, SPECIAL, NO. 1 | 39 |
| INLETS, SPECIAL, NO. 2 | 40 |
| INLETS, SPECIAL, NO. 3 | 40 |
| PROPOSED MANHOLE/CATCH BASIN CONNECTION OVER EXISTING STORM SEWER.. | 40 |
| UNDERDRAIN CONNECTION TO STRUCTURE..... | 41 |
| REMOVAL OF EXISTING STRUCTURES..... | 41 |
| FORM LINER LIMESTONE SURFACE..... | 42 |
| ANTI-GRAFFITI COATING | 45 |
| CHAIN LINK FENCE ATTACHED TO STRUCTURE | 46 |
| ERECTION OF COMPLEX STEEL STRUCTURES | 47 |
| DRAINAGE SCUPPERS..... | 48 |

| | |
|--|----|
| TEMPORARY FACILITIES INSTALLED BY CONTRACTOR..... | 48 |
| BAR SPLICERS..... | 48 |
| REMOVE AND RELOCATE SIGN PANEL..... | 49 |
| DELINEATOR REMOVAL..... | 49 |
| DRAINAGE STRUCTURE ADJUSTMENT (SPECIAL) | 49 |
| MAINHOLE FRAMES TO BE ADJUSTED | 50 |
| VALVE BOX FRAMES TO BE ADJUSTED..... | 50 |
| COOPERATION WITH UTILITIES..... | 51 |
| SANITARY SEWER 10"..... | 51 |
| STEEL CASINGS 24" | 55 |
| CARRIER SANITARY SEWER, 10" DIAMETER..... | 58 |
| SANITARY MANHOLES (SPECIAL)..... | 60 |
| SANITARY MANHOLES, SADDLE-TYPE, 5' DIAMETER..... | 63 |
| SANITARY SEWER STAGING AND BYPASS FLOW SYSTEM..... | 67 |
| INSPECTION AND TESTING OF SEWERS | 68 |
| SANITARY MANHOLE FRAME AND COVER | 71 |
| SANITARY MANHOLES TO BE RECONSTRUCTED..... | 71 |
| ABANDON AND FILL EXISING SANITARY MANHOLE | 74 |
| ELECTRIC SERVICE INSTALLATION, SPECIAL..... | 75 |
| REMOVAL OF UNDERPASS LIGHTING UNIT, NO SALVAGE..... | 75 |
| CONTRACT GUARANTEE | 75 |
| SYSTEM IMPLEMENTATION, EQUIPMENT INTEGRATION AND SUPPORT | 76 |
| CONSTRUCTION PERMITS | 76 |
| AS-BUILT DOCUMENTATION | 77 |
| HPS LAMP RECYCLING | 77 |
| POTHOLING FOR LOCATION OF EXISTING UNDERGROUND UTILITIES | 77 |
| HANDHOLE, PORTLAND CEMENT CONCRETE | 77 |
| WIDE AREA VIDEO DETECTION SYSTEM COMPLETE | 78 |
| CAT 5 ETHERNET CABLE | 83 |
| FIBER OPTIC CABLE 12 FIBERS, SINGLE MODE OR FIBER OPTIC CABLE 48 FIBERS, SINGLE MODE..... | 84 |
| FUSION SPLICING OF FIBER OPTIC CABLES..... | 88 |
| TERMINATION OF FIBER OPTIC CABLES WITH FUSION SPLICED ST CONNECTORS | 91 |
| COMMUNICATIONS VAULT | 92 |
| CLOSED-CIRCUIT TELEVISION DOME CAMERA, HD | 93 |

| | |
|---|-----|
| CAT 6 ETHERNET CABLE | 98 |
| WIRELESS ETHERNET RADIO | 99 |
| FLASHING BEACON ASSEMBLY | 101 |
| POWER OVER ETHERNET EXTENDER | 102 |
| FLASHER CONTROLLER, SPECIAL, WITHOUT CABINET..... | 106 |
| FIBER OPTIC ETHERNET DROP AND REPEAT SWITCH..... | 107 |
| CLOSED-CIRCUIT TELEVISION CABINET | 110 |
| MISCELLANEOUS ELECTRICAL WORK..... | 111 |
| REMOVE EXISTING ITS EQUIPMENT | 112 |
| INSTALLATION OF IDOT CONDUIT AND FIBER OPTIC CABLE | 113 |
| HANDHOLE TO BE ADJUSTED WITH NEW FRAME AND COVER | 113 |
| SEEDING, MINOR AREAS | 114 |
| RAILROAD PROTECTIVE LIABILITY INSURANCE (BDE)..... | 115 |
| REQUIREMENTS WHEN WORKING WITH THE RAILROAD | 115 |
| WORKING RESTRICTIONS..... | 116 |
| TZPR/GWRR RAILROAD COORDINATION AND PERMITTING REQUIREMENTS | 116 |
| BUILDING REMOVAL (BDE)..... | 118 |
| HIGH LOAD MULTI-ROTATIONAL BEARINGS..... | 119 |
| MODULAR EXPANSION JOINT | 125 |
| ERECTION OF CURVED STEEL STRUCTURES | 129 |
| DIAMOND GRINDING AND SURFACE TESTING BRIDGE SECTIONS | 130 |
| STRUCTURAL ASSESSMENT REPORTS FOR CONTRACTOR'S MEANS AND METHODS | 135 |
| BRIDGE DECK CONSTRUCTION..... | 137 |
| METALLIZING OF STRUCTURAL STEEL..... | 138 |
| HOT DIP GALVANIZING FOR STRUCTURAL STEEL | 153 |
| DRILLED SHAFTS..... | 157 |
| PREFORMED PAVEMENT JOINT SEAL | 166 |
| ERECTION OF BRIDGE GIRDERS OVER OR ADJACENT TO RAILROADS..... | 172 |
| BAR SPLICERS, HEADED REINFORCEMENT | 173 |
| AGGREGATE SUBGRADE IMPROVEMENT (BDE)..... | 174 |
| BITUMINOUS MATERIALS COST ADJUSTMENTS (BDE)..... | 176 |
| CEMENT, FINELY DIVIDED MINERALS, ADMIXTURES; CONCRETE, AND MORTAR (BDE) | 177 |
| COMPENSABLE DELAY COSTS (BDE) | 187 |

| | |
|--|-----|
| CONCRETE BARRIER (BDE) | 190 |
| CONCRETE SEALER (BDE) | 190 |
| DISADVANTAGED BUSINESS ENTERPRISE PARTICIPATION (BDE) | 191 |
| EROSION CONTROL BLANKET (BDE) | 194 |
| FUEL COST ADJUSTMENT (BDE) | 196 |
| HOT-MIX ASPHALT (BDE) | 198 |
| PAVEMENT MARKING INSPECTION (BDE) | 200 |
| PAVEMENT PATCHING (BDE) | 200 |
| PERFORMANCE GRADED ASPHALT BINDER (BDE) | 201 |
| REMOVAL AND DISPOSAL OF REGULATED SUBSTANCES (BDE) | 205 |
| SEEDING (BDE) | 207 |
| SHORT TERM AND TEMPORARY PAVEMENT MARKINGS (BDE) | 212 |
| SIGN PANELS AND APPURTENANCES (BDE) | 215 |
| SLOPE WALL (BDE) | 216 |
| SOURCE OF SUPPLY AND QUALITY REQUIREMENTS (BDE) | 216 |
| STEEL COST ADJUSTMENT (BDE) | 217 |
| SUBCONTRACTOR AND DBE PAYMENT REPORTING (BDE) | 220 |
| SUBCONTRACTOR MOBILIZATION PAYMENTS (BDE) | 220 |
| SUBMISSION OF BIDDERS LIST INFORMATION (BDE) | 221 |
| SUBMISSION OF PAYROLL RECORDS (BDE) | 221 |
| SURFACE TESTING OF PAVEMENTS – IRI (BDE) | 222 |
| SURVEYING SERVICES (BDE) | 227 |
| TRAINING SPECIAL PROVISIONS (BDE) | 228 |
| IDOT TRAINING PROGRAM GRADUATE ON-THE-JOB TRAINING SPECIAL PROVISION | 230 |
| VEHICLE AND EQUIPMENT WARNING LIGHTS (BDE) | 232 |
| WEEKLY DBE TRUCKING REPORTS (BDE) | 233 |
| WOOD SIGN SUPPORT (BDE) | 233 |
| WORK ZONE TRAFFIC CONTROL DEVICES (BDE) | 233 |
| PROJECT LABOR AGREEMENT | 236 |
| ASBESTOS ABATEMENT SURVEY | 254 |
| STORM WATER POLLUTION PREVENTION PLAN (BDE 2342) | 288 |
| CONTRACTOR'S CERTIFICATION STATEMENT (BDE 2342A) | 300 |

STATE OF ILLINOIS

SPECIAL PROVISIONS

The following Special Provisions supplement the "Standard Specifications for Road and Bridge Construction," adopted January 1, 2022 (revised January 1, 2025), the latest edition of the "Manual on Uniform Traffic Control Devices for Streets and Highways," and the "Manual of Test Procedures for Materials" in effect on the date of invitation for bids, and the Supplemental Specifications and Recurring Special Provisions indicated on the Check Sheet included herein which apply to and govern the construction of FAP Routes 669 & 673 (IL 116/IL 8 & US 24), Project NHPP-7VBW(880), Section 13[(HVB,HB-3)]BR-1;R, Tazewell County, Contract No. 68C55 and in case of conflict with any part or parts of said Specifications, the said Special Provisions shall take precedence and shall govern.

LOCATION OF PROJECT

The Cedar Street Bridge Extension project is the Section of FAP Route 669 (IL 8/IL 116) in Tazewell County, Illinois, and includes bridge Structure No. 090-0034 over Tazewell & Peoria Railroad (TZPR) and bridge Structure No. 090-0035 over Main Street (IL 29/US 24) in the city of East Peoria. The project limits also include Main Street (IL 29/US 24) from Ravine Street to Everett Street, a distance of approximately 0.5 mile.

The project starts just south of where Cedar Street crosses Center Street, runs southeast along the existing Cedar Street alignment, and includes the four interchange ramps allowing traffic to and from Center and Main Streets. The project length on Cedar Street and the interchange is approximately 0.5 mile. The interchange ramps are identified below:

- Ramp A: IL 8/IL 116 Eastbound exit ramp to Eastbound Main Street (IL 29/US 24)
- Ramp B: IL 8/IL 116 Westbound entrance ramp from eastbound Main Street (IL 29/US 24)
- Ramp C: IL 8/IL 116 Eastbound exist ramp to Westbound Main Street (IL 29/US 24)
- Ramp D: IL 8/IL 116 Westbound entrance ramp from Westbound Main Street (IL 29/US 24)

This section of roadway is functionally classified as an "Other Principal Arterial". It is marked as IL 8/IL 116 and locally known as Cedar Street. Cedar Street crosses the Illinois River approximately one mile to the north from the northern project termini. The interchange with Cedar Street and Main Street is approximately 0.7 mile southwest of the interchange of Main Street and Interstate 74. The Illinois Department of Transportation (IDOT) has jurisdiction of Cedar Street (IL 8/IL 116) and Main Street (IL 29/US 24). The city of East Peoria currently has a maintenance agreement with IDOT which includes roadway surface and lighting along Cedar Street and the interchange at Main Street. IDOT maintains the pavement and bridge structures for the mainline route. IDOT has a maintenance agreement with the city of East Peoria for the mainline pavement for winter snow and ice control. This section of Cedar Street is classified as a Class II truck route and is located on the National Highway System (NHS).

DESCRIPTION OF PROJECT

This project includes the reconstruction of the Cedar Street Extension bridges over TZPR and Main Street, reconstruction of the Cedar Street (IL 8/IL 116) at Main Street (IL 29/US 24) interchange, Main Street pavement replacement, reconstruction of the Silver Street access, retaining walls, and a multi-use path.

In addition, the project will include, but not limited to: earthwork, pavement removal and replacement, pavement widening, storm sewer and utility removal and replacement, curb and gutter replacement, sidewalk removal and reconstruction, lighting removal and reconstruction, signing and striping, erosion control, and various other items.

The project includes new construction of the following proposed structures:

1. S.N. 090-0182 Bridge – Cedar Street (IL 8/IL 116) over Tazewell & Peoria Railroad (TZPR)
2. S.N. 090-0183 Bridge – Cedar Street (IL 8/IL 116) over Main Street (IL 29/US 24)

The project includes the removal of the following structures:

1. S.N. 090-0034 Bridge – Cedar Street (IL 8/IL 116) over Tazewell & Peoria Railroad (TZPR)
2. S.N. 090-0035 Bridge – Cedar Street (IL 8/IL 116) over Main Street (IL 29/US 24)

DATE OF COMPLETION (PLUS WORKING DAYS)

Effective March 1, 1990 Revised August 3, 2018

The Contractor shall schedule his operations so as to complete all work, except as specified below, and open all the roadway to traffic on or before **November 30, 2027**. The Contractor shall note that this completion date is based on an expedited work schedule. The Contractor will be allowed **30 working days**, after the November 30, 2027 completion date, to complete punchlist and the following items: landscaping, lighting, and permanent pavement markings.

CONSTRUCTION LAYOUT RESPONSIBILITY

Effective April 26, 2015 Revised: January 1, 2022

This special provision is included in addition to Check Sheet #9 of the Recurring Special Provisions, Special Provision for Construction Layout Stakes, to clearly define the responsibility of the Contractor for construction layout.

As the Contractor is generating the survey layout model, all roadway elements shall be verified to fit within the final proposed slopes and right-of-way. If the Contractor determines a portion of the plans is incorrect or a portion does not agree with another portion, they shall contact the Engineer to have the problem resolved and additional work, if any, agreed upon. The Contractor shall not proceed until authority is received from the Engineer and problems are resolved. The Engineer shall contact the District Studies and Plans Section if need be.

The Contractor shall set all horizontal control points at the end of construction and provide cross ties in a hardback survey book to the Engineer.

The Contractor shall also set and provide the Engineer with a list of final benchmarks in a hardback survey book at the end of construction for future control.

No additional compensation will be allowed for complying with this Special Provision, but all costs shall be included in the contract Lump Sum price for CONSTRUCTION LAYOUT.

CONSTRUCTION LAYOUT UTILIZING GPS EQUIPMENT

Effective: April 26, 2015

Revised: January 1, 2022

If the Contractor opts to utilize GPS equipment for Construction Layout, the Contractor shall be required to complete the following in addition to the requirements of Check Sheet #9 of the Recurring Special Provisions and as directed by the Engineer.

1. Submit 3D drawings or show the Engineer the digital terrain model (or proof of some type) that the Contractor has generated all proposed information correctly for all parts of the job (Mainline, ramps, side roads, entrances, etc.) before starting any grading, structures, or paving work. This does not relieve the Contractor of responsibility of any possible errors made in the modeling.
2. The Contractor shall also submit a QC/QA written plan that they will be following to provide quality control on the actual layout and quality assurance checks of the layout during and after being completed. This will be required to be submitted at the beginning of construction and shall meet the approval of the Engineer.
3. Stationing lathes shall be placed and maintained along the rights-of-way lines, centerline of the median, and agreed offset from other baselines such as interchange ramps and side roads, throughout the duration of the contract.

No additional compensation will be allowed for complying with this special provision, but all costs shall be included in the contract Lump Sum price for CONSTRUCTION LAYOUT.

CONSTRUCTION LAYOUT EQUIPMENT

Effective: April 26, 2015

Revised: November 6, 2015

General. The Contractor shall furnish articles of survey equipment to be used by the Department for independent monitoring and verification of construction layout stakes, reference points, and any other horizontal and vertical control set by the Contractor. All equipment will be for the exclusive use of the Department throughout the duration of the contract and will be returned to the Contractor at the end of the contract.

Equipment. The equipment to be furnished by the Contractor shall consist of one precision GNSS rover and a secondary GPS handheld controller. The precision GNSS rover must meet or exceed the capabilities of and be compatible with the Contractor's equipment and meet the approval of the Engineer. The second GPS handheld controller shall also meet or exceed the capabilities of and be compatible with the Contractor's equipment and meet the approval of the Engineer. The equipment provided shall include all software, data and any additional equipment (base station, repeaters, etc.) necessary to find any point on the project in station, offset and elevation with precision. The project data included in the equipment will be consistent with the data used by the Contractor for layout and grading. Any data revisions or software updates to the Contractor's equipment will also be applied to the Department's equipment by the Contractor.

The Contractor will be responsible for providing training for three members of the Department's staff on use of the equipment and software.

Basis of Payment. This work will not be measured separately but shall be included in the contract Lump Sum price for CONSTRUCTION LAYOUT.

REMOVAL OF ABANDONED UNDERGROUND UTILITIES

Effective January 15, 1996 Revised November 21, 1996

This work shall be completed in accordance with Article 105.07 of the Standard Specifications and the items outlined herein:

The cost of removal of abandoned or to be abandoned underground utilities shown on the plans are the responsibility of the owner. The Contractor shall make arrangements with the utility owner for removal and payment. The utility owner is listed in the plans under Status of Utilities.

Prior to removal of the abandoned facility, the owner shall be notified so that representatives can be present during the removal operation.

If an unknown abandoned utility is encountered, the Contractor will be paid for any removal required by the Engineer as extra work in accordance with Article 109.04 of the Standard Specifications.

UTILITIES – LOCATIONS/INFORMATION ON PLANS

Effective: November 8, 2013

The locations of existing water mains, gas mains, sewers, electric power lines, telephone lines, and other utilities as shown on the plans are based on field investigation and locations provided by the utility companies, but they are not guaranteed. Unless elevations are shown, all utility locations shown on the cross sections are based on the approximate depth supplied by the utility company. It shall be the Contractor's responsibility to ascertain their exact location from the utility companies and by field inspection.

LOCATION OF UNDERGROUND STATE MAINTAINED FACILITIES

Effective: August 3, 2007 Revised: July 31, 2009

The Contractor shall be responsible for locating existing and proposed IDOT electrical facilities (traffic signal, overhead lighting, Intelligent Transportation System, etc.) prior to performing any work at his/her own expense if required. The Contractor shall also be liable for any damage to IDOT facilities resulting from inaccurate locating.

The Contractor may obtain, on request, plans for existing electrical facilities from the Department.

The Contractor shall also be responsible for locating and providing protection for IDOT facilities during all phases of construction. If at any time the facilities are damaged, the Contractor shall immediately notify the Department and make all necessary arrangements for repair to the satisfaction of the Engineer. This work will not be paid for separately but shall be included in the contract bid price.

EMBANKMENT (RESTRICTIONS)

Effective: January 21, 2005 Revised: August 5, 2022

Replace the sixth and seventh paragraphs of Article 205.04 with the following:

Alternating layers of suitable soil and restricted-use material will not be permitted. Restricted-use materials may only be incorporated into the embankment by using one of the following procedures:

- a. Restricted-use materials shall be placed in 4" lifts and disked with the underlying lift material until a uniform and homogenous material is formed having more than 35% passing the number 200 sieve.
- b. Sand, gravel or crushed stone embankment when placed on the existing ground surface will be drained using a 10' (3 m) by 10' (3 m) French drain consisting of nonwoven geotechnical fabric with 12" (0.3 m) of B-3 riprap. This shall be constructed on both sides of the embankment at the toe of the foreslope spaced 150' (46 m) apart. At locations requiring a French drain the 3' (1 m) cohesive cap shall not be installed within the 10' by 10' riprap area. If the Engineer determines that the existing ground is a granular free draining soil, the French drain may be deleted.
- c. Sand, gravel or crushed stone embankment when placed on top of a cohesive embankment will be drained with a permanent 4" (100 mm) underdrain system. The underdrain system shall consist of a longitudinal underdrain on both sides of the embankment and transverse underdrains spaced at 250' (75 m) centers. The underdrain shall consist of a 2' (0.6 m) deep by 1' (0.3 m) wide trench, backfilled with FA4 sand and a 4" (100 mm) diameter underdrain. In addition, both sides of the embankment will have a 6" (150 mm) diameter pipe drain which will drain the underdrain system and outletted into a permanent drainage structure or outletted by a headwall at the toe of the embankment.

The above work will not be paid for separately but shall be included in the cost of EARTH EXCAVATION, FURNISHED EXCAVATION, or BORROW EXCAVATION.

MOWING

Effective: December 11, 2001 Revised: August 2, 2013

This work shall consist of mowing the entire median up to 60' (20m) in width and the roadway foreslopes of the outside lanes to the ditchline or for a width of 15' (4.572 meters) from the edge of pavement or paved shoulder, whichever is less. At intersecting roadways, the mowing shall extend to the proposed right of way for a distance of 150' (45 m) on either side of the intersection. The height of the mowing shall not be more than 6" (150 mm). Equipment used shall be capable of completely severing all growth at the cutting height and distributing it evenly over the mowed area. The Contractor will not be required to mow continuously wet ditches and drainage ways, slopes greater than 1:3 (V:H), or areas which may be designated by the Engineer as not mowable. Mowing shall be done within the project limits during the construction of the project as directed by the Engineer and prior to the final inspection of the project. Any subsequent mowing required to disperse mowed material shall be considered as included in the cost of the mowing. Debris encountered during mowing, which interferes with the mowing operation or is visible from the roadway shall be removed and disposed of according to Article 202.03 of the Standard Specifications.

Method of Measurement: Mowing will be measured for payment in acres of surface area mowed.

Basis of Payment: This work will be paid for at the contract unit price per Acre for MOWING.

PROOF ROLLING

Effective April 23, 2004 Revised January 1, 2007

This work shall consist of proof rolling the subgrade with a fully loaded tandem axle dump truck and driver at the direction of the Engineer. The truck shall travel the subgrade in all of the proposed lanes of traffic in the presence of the Engineer.

This work will not be paid for separately but considered included in the various earthwork pay items.

SUBGRADE TREATMENT

Effective July 1, 1990 Revised January 1, 2022

Revise first sentence of first paragraph of Article 301.04 as follows:

"When compacted, the subgrade shall have a minimum dry density of 95 percent of the standard laboratory dry density and a minimum immediate bearing value (IBV) of 3."

Delete the second paragraph (including subparagraphs a, b, and c) of Article 301.04 of the Standard Specifications and replace it with the following:

"In cut sections the Contractor responsible for the rough grading shall obtain not less than 95% of the standard laboratory density and not more than 110% of the optimum moisture for the top 1' (300 mm) of the subgrade.

The Contractor may, at his/her option, add a drying agent to lower the moisture content as specified. The drying agent must be approved by the Engineer prior to use. Additional compensation will not be allowed for the use of a drying agent but will be considered as included in the cost of the various earthwork items."

TEMPORARY PAVEMENT

Effective October 1, 1995 Revised April 24, 2020

This item shall include all materials, labor and equipment necessary to construct temporary pavement in accordance with applicable sections of the Standard Specifications except as herein specified.

The Contractor shall have the option of constructing temporary pavement made of 8" hot-mix asphalt base course on 4" of aggregate base course Type B or 8" PCC base course on 4" of aggregate base course Type B.

Hot-Mix Asphalt base course shall be placed in accordance with applicable portions of Section 355. Material for Hot-Mix Asphalt base course shall be Hot-Mix Asphalt Binder Course in accordance with Sections 406 and 407 and as shown in the Mix Design Table. Tack Coat shall be in accordance with Section 406 and at the rate shown in the General Notes. PCC base course shall be in accordance with Section 353 of the Standard Specifications.

This work will be paid for at the contract unit price per Square Yard (Square Meter) for TEMPORARY PAVEMENT.

Removal of Temporary Pavement will be paid for separately in accordance with Section 440 of the Standard Specifications.

HOT-MIX ASPHALT SURFACE COURSE SURFACE TESTS

Effective: November 1, 2003 Revised January 1, 2007

The Contractor shall provide a person to operate the straight edge in accordance with Article 406.11 of the Standard Specifications and communicate with IDOT Personnel to minimize the surface course bumps. If surface course bumps cannot be removed at this time, IDOT personnel will record the locations and provide deductions as stated in Article 406.11.

PROTECTION OF FRAMES AND LIDS OF UTILITY STRUCTURES

Effective March 6, 1991 Revised January 1, 2007

This work shall consist of protecting frames and lids of utility structures in the pavement after the adjacent hot-mix asphalt surface has been removed to the required depth by cold milling or by hand methods.

After the area has been swept clean and before the lane is opened to traffic, a hot bituminous mixture shall be placed around the casting, flush with its surface and decreasing to a featheredge in a distance of 4 feet (1.2 m) around the entire surface of the casting. Cold mix or milled material will not be permitted. This mixture shall remain in place until the day surfacing operations are undertaken within the immediate area of the structure. Prior to placing the surface course, the temporary hot-mix asphalt mixture shall be removed and disposed of by the Contractor as specified in Article 202.03 of the Standard Specifications.

The temporary tapers and their removal shall be considered included in the contract unit price per Square Meter (Square Yard) for HOT-MIX ASPHALT SURFACE REMOVAL of the depth specified, and no additional compensation will be allowed.

HOT-MIX ASPHALT SURFACE REMOVAL, 2"

Effective: March 1, 1993
Revised: January 1, 2022

Description: This work shall consist of removing a portion of the existing hot-mix asphalt concrete surface course in accordance with the applicable portions of Section 440 and 1101 of the Standard Specifications, this special provision, details in the plans and as directed by the Engineer. The cold milled salvaged aggregate resulting from this operation shall become the property of the Contractor.

Equipment: The machine used for milling and planing shall be a self-propelled grinding machine having a minimum 12' (3.6 m) wide drum at least 28" (710 mm) in diameter. When a milling width in excess of 12' is required and the Contractor's milling machine is less than the required width shown in the plans, the remaining area shall be milled with a machine capable of meeting the requirements of this special provision. Milling attachments used with skid steer tractors will not be allowed for longitudinal areas to mill additional widths.

When the teeth become worn so that they do not produce a uniform surface texture, they shall all be changed at the same time (as a unit). Occasionally, individual teeth may be changed if they lock up or break, but this method shall not be used to avoid changing the set of teeth as a unit. Occasional gouges, due to deteriorated pavement condition, or separation of lifts will not be cause to replace all teeth. The Engineer will be the sole judge of the cause of the pavement gouging and the corrective work required. Corrective work due to negligence or poor workmanship shall be at the Contractor's expense.

The moldboard is critical in obtaining the desired surface texture. It shall be straight, true, and free of excessive nicks or wear, and it shall be replaced as necessary to uniformly produce the required surface texture. Gouging of the pavement by more than 1/4 inch (6 mm) shall be sufficient cause to require replacement of all teeth.

Add the following after the third sentence of Article 406.05 (c)(1):

"Vacuum sweeping shall be accomplished with a regenerative air sweeper."

Construction Requirements

General: Weather conditions, when milling work is performed, must be such that short term or temporary pavement markings can be placed the day the surface is milled in accordance with Section 703 "Work Zone Pavement Markings".

An automatic grade control device shall be used when milling mainline pavement and shall be capable of controlling the elevation of the drum relative to either a preset grade control stringline or a grade reference device traveling on the adjacent pavement surface. The automatic grade control device may be utilized only on one side of the machine with a automatic slope control device controlling the opposite side. The traveling grade reference device shall not be less than 30 feet (9 m) in length. When milling cross roads, turn lanes, intersections, crossovers, or other miscellaneous areas, the Engineer may permit the matching shoe. The Contractor, at his option, may also substitute an approved 6' wide (1.8 m) machine for areas other than mainline pavement.

The Contractor shall mill 2" inches at the centerline and project the proposed cross slope to the edge of pavement. In the event the milling at the outer edge of the lane would exceed 2" inches; then the Contractor shall reduce the cut at the centerline to provide the maximum cut of 2" inches at the edge of pavement. If deemed necessary, the Contractor may reduce the cross slope from normal 1.5% to 1%.

Surface tests will be performed in accordance with Article 406.11 of the Standard Specifications. The longitudinal profile will be taken 3 ft. (0.9 m) from and parallel to each edge of pavement and 3 ft. (0.9 m) from and parallel to the centerline on each side. If a shadow area is found at the 3 ft. (0.9 m) points the pavement smoothness tester will be moved sufficient distance either side to measure the Contractor's milling efforts. Any surface variations exceeding the tolerance in Article 406.11 shall be corrected by reprofiling at no additional expense to the Department. In addition, the Contractor shall be responsible for refilling with approved hot-mix asphalt mixtures any area that lowered the pavement profile as a result of faulty milling operations if directed by the Engineer. The Contractor shall be responsible for providing the pavement smoothness tester described elsewhere to retest the pavement profile obtained.

If the milling depth is intended to expose the original concrete pavement, then additional hand or machine work may be necessary to remove any remaining veneer of bituminous pavement which may be left in place behind the milling machine. Such work will be at the direction of the Engineer and at no extra cost to the Department.

The Contractor shall provide a 10-foot (3 m) straightedge equipped with a carpenter's level or a 7-foot (2.1 m) electronic straightedge to check the cross slope of the roadway at regular intervals as directed by the Engineer.

Surface Texture: Each tooth on the cutting drum shall produce a series of discontinuous longitudinal striations. There shall be 16 to 20 striations (tooth marks) for each tooth for each 6 feet (1.8 m) in the longitudinal direction, and each striation shall be 1.7 inches ± 0.2 inch (43 ± 5 mm) in length after the area is planed by the moldboard. Thus, the planed length between each pair of striations shall be 2.3 inches ± 0.2 inch (58 ± 5 mm). There shall be 80 to 96 rows of discontinuous longitudinal striations for each 5 feet (1.5 m) in the transverse dimension. The areas between the striations in both the longitudinal and transverse directions shall be flat topped and coplaner. The moldboard shall be used to cut this plane; and any time the operation fails to produce this flat plane interspersed with a uniform pattern of discontinuous longitudinal striations, the operation shall be stopped and the cause determined and corrected before recommencing. Other similar patterns of uniform discontinuous longitudinal striations interspersed on a flat plane may be approved by the Engineer. The drawing titled "Hot-Mix Asphalt Surface Removal" showing the desired surface texture is included in the plans.

The start-up milling speed shall be limited to a maximum of 50-foot (15 m) per minute. The Contractor shall limit his operations to this speed to demonstrate his ability to obtain the striations and ride ability as described above. If the Contractor is able to demonstrate that he can consistently obtain the desired striations and ride ability at a greater speed he will be permitted to run at the increased speed.

Cleanup: After cold milling a traffic lane and before opening the lane to traffic, the pavement shall be swept by a regenerative air sweeper to prevent compaction of the cuttings onto the pavement. All loose material shall be removed from the roadway. Before the prime coat is placed, the pavement shall be cleaned of all foreign material to the satisfaction of the Engineer.

This cleanup work shall be considered included in the contract unit price per Square Meter (Square Yard) for HOT-MIX ASPHALT SURFACE REMOVAL of the depth specified, and no additional compensation will be allowed.

Method of Measurement:

- (a) Contract Quantities. The requirements for the use of Contract Quantities shall be Article 202.07(a) of the Standard Specifications.
- (b) Measured Quantities. Cold milling and planing will be measured and the area computed in Square Yards (Square Meters) of surface.

Areas not milled (shadowed areas) due to rutting in the existing pavement surface will be included in the area measured for payment.

Basis of Payment: The cold milling and planing will be paid for at the contract unit price per Square Yard (Square Meter) for HOT-MIX ASPHALT SURFACE REMOVAL of the depth specified. Payment as specified will include variations in depth of cuts due to rutting, superelevations, and pavement crown and no additional compensation will be allowed.

CONCRETE HEADWALL REMOVAL

Effective July 1, 1990

This work shall consist of the removal of existing concrete headwalls at various locations as shown on the plans and shall be done in accordance with the applicable portions of Section 501 of the Standard Specifications.

The above work shall include the removal of the first section of pipe with the headwall. The removal of the first section of pipe will not be paid for separately but shall be included in the unit price Each for CONCRETE HEADWALL REMOVAL, and no additional compensation will be allowed due to the various sizes of pipes and headwalls.

GRANULAR BACKFILL FOR STRUCTURES

Effective August 4, 2017 Revised November 6, 2020

The aggregate shall be one of the following gradations:

CA7, CA11, or CA13 thru CA16, according to Sections 1003 and 1004 of the Standard Specifications.

PCC PLACEMENT BY PUMP REQUIREMENTS

Effective: January 1, 2022

These provisions are required for concrete structures and drilled shaft construction.

Revise the 7th paragraph of Article 503.07 to read:

"When air entrained concrete is pumped, a reduction hose at point of placement will be utilized. In addition, the pump shall be operated with sufficient minimum pressure and flow rate to create a steady stream of material at the point of placement. The maximum allowable air loss caused by the pumping operation shall be 3.0 percent with the minimum air content at the point of discharge meeting the requirements of Article 1020.04. The initial air test utilized to determine the air content correction factor shall not be conducted within the confines of the pour. A pneumatic or mechanical shut-off device shall be incorporated in the pump apparatus as close as practical to point of placement; the device shall be utilized to maintain a full surcharge of material in the pump during pump stoppage."

Revise the 4th paragraph of Article 503.08 to read:

"At the Contractor's option, pumping equipment may be used in lieu of a tremie to deposit concrete underwater. The Engineer will approve the concrete pumping equipment and its piping before the work is started. If pumping equipment is used to deliver concrete to a tremie and hopper, a reduction hose at point of placement will be utilized. In addition, the pump shall be operated with sufficient minimum pressure and flow rate to create a steady stream of material at the point of placement. The maximum allowable air loss caused by the pumping operation shall be 3.0 percent with the minimum air content at the point of discharge meeting the requirements of Article 1020.04. The initial air test utilized to determine the air content correction factor shall not be conducted within the confines of the pour. A pneumatic or mechanical shut-off device shall be incorporated in the pump apparatus as close as practical to point of placement; the device shall be utilized to maintain a full surcharge of material in the pump during pump stoppage."

STORM SEWER, (WATER MAIN QUALITY PIPE)

Effective January 1, 2011

Revised January 1, 2021

This work consists of constructing storm sewer to meet water main standards, as required by the IEPA or when otherwise specified. The work shall be performed in accordance with applicable parts of Section 550 of the Standard Specifications, applicable sections of the current edition of the IEPA Regulations (Title 35 of the Illinois Administrative Code, Subtitle F, Chapter II, Section 653.119), the applicable sections of the current edition of the "Standard Specifications for Water and Sewer Main Construction in Illinois", and as herein specified.

This provision shall govern the installation of all storm sewers which do not meet IEPA criteria for separation distance between storm sewers and water mains. Separation criteria for storm sewers placed adjacent to water mains and water service lines are as follows:

- (1) Water mains and water service lines shall be located at least 10 feet (3.05 meters) horizontally from any existing or proposed drain, storm sewer, sanitary sewer, or sewer service connections.
- (2) Water mains and water service lines may be located closer than 10 feet (3.05 meters) to a sewer line when:
 - (a) Local conditions prevent a lateral separation of 10 feet (3.05 meters); and
 - (b) The water main or water service invert is 18 inches (460 mm) above the crown of the sewer; and
 - (c) The water main or water service is either in a separate trench or in the same trench on an undisturbed earth shelf located to one side of the sewer.
- (3) A water main or water service shall be separated from a sewer so that its invert is a minimum of 18 inches (460 mm) above the crown of the drain or sewer whenever water mains or services cross storm sewers, sanitary sewers or sewer service connections. The vertical separation shall be maintained for that portion of the water main or water services located within 10 feet (3.05 meters) horizontally of any sewer or drain crossed.

When it is impossible to meet (1), (2) or (3) above, the storm sewer shall be constructed of concrete pressure pipe, slip-on or mechanical joints ductile iron pipe, or PVC pipe equivalent to water main standards of construction. Construction shall extend on each side of the crossing until the perpendicular distance from the water main or water service to the sewer or drain line is at least 10 feet (3.05 meters). Storm sewer meeting water main requirements shall be constructed of the following pipe materials:

Concrete Pressure Pipe

Concrete pressure pipe shall conform to the latest ANSI/AWWA C300, C301, or C303.

Joints shall conform to Article 41-2.07B of the "Standard Specifications for Water and Sewer Main Construction in Illinois."

Ductile Iron Pipe

Ductile Iron pipe shall conform to ANSI A 21.51 (AWWA C151), class or thickness designed per ANSI A 2150 (AWWA C150), tar (seal) coated and/or cement lined per ANSI A 21.4 (AWWA C104), with a mechanical or rubber ring (slip seal or push on) joints.

Joints for ductile iron pipe shall be in accordance with the following applicable specifications.

- | | | |
|----------------------|---|--------------------|
| 1. Mechanical Joints | - | AWWA C111 and C600 |
| 2. Push-On Joints | - | AWWA C111 and C600 |

Plastic Pipe

Plastic pipe shall be marked with the manufacturer's name (or trademark); ASTM or AWWA specification; Schedule Number, Dimension Ratio (DR) Number or Standard Dimension Ratio (SDR) Number; and Cell Class. The pipe and fittings shall also meet NSF Standard 14 and bear the NSF seal of approval. Fittings shall be compatible with the type of pipe used. The plastic pipe options shall be in accordance with the following:

1. Polyvinyl Chloride (PVC) conforming to ASTM Standard D 1785. Schedule 80 is the minimum required for all pipe sizes, except when the pipe is to be threaded, and then it shall be Schedule 120. It shall be made from PVC compound meeting ASTM D 1784, Class 12454C.
2. Polyvinyl Chloride (PVC) conforming to ASTM D 2241. A minimum wall thickness of SDR 26 is required for all pipe sizes (Note: The lower the SDR number, the higher the wall thickness and pressure rating). It shall be made from PVC compound meeting ASTM D 1784, Class 12454B.
3. Chlorinated Polyvinyl Chloride (CPVC) conforming to ASTM F 441. A minimum of Schedule 80 is required for all pipe sizes. Threaded joints are not allowed. It shall be made from CPVC compound meeting ASTM D 1784, Class 23447B.
4. Chlorinated Polyvinyl Chloride (CPVC) conforming to ASTM F 442M/F422M. A minimum wall thickness of SDR 26 is required for all pipe sizes (Note: The lower the SDR number, the higher the wall thickness and pressure rating). It shall be made from CPVC compound meeting ASTM D 1784.

5. Polyvinyl Chloride (PVC) conforming to ANSI/AWWA C900. A minimum of wall thickness of DR 25 is required for all pipe sizes (Note: The lower the DR number, the higher the wall thickness and pressure rating). It shall be made from PVC compound meeting ASTM D 1784, Class 12454.
6. Polyvinyl Chloride (PVC) conforming to ANSI/AWWA C905. A minimum of wall thickness of DR 26 is required for all pipe sizes (Note: The lower the DR number, the higher the wall thickness and pressure rating). It shall be made from PVC compound meeting ASTM D 1784, Class 12454.

Joining of plastic pipe shall be by push-on joint, solvent welded joint, heat welded joint, flanged joint, or threaded joint, butt fused or electro fused, in accordance with the pipe manufacturer's instructions and industry standards. Special precautions shall be taken to insure clean, dry contact surfaces when making solvent or heat welded joints. Adequate setting time shall be allowed for maximum strength.

Elastometric seals (gaskets) used for push-on joints shall comply with ASTM F477.

Solvent cement shall be specific for the plastic pipe material and shall comply with ASTM D 2564 (PVC) or ASTM F 493 (CPVC) and be approved by NSF.

This work will be measured and paid for at the contract unit price per Foot (Meter) for STORM SEWER (WATER MAIN QUALITY PIPE) of the diameter and type specified.

BACKFILL, BUILDING REMOVAL

Effective August 20, 1991

Revised January 1, 2007

All material furnished for backfilling holes and basements for building removal shall satisfy Article 1003.04 or 1004.05 of the Standard Specifications.

The cavities under the proposed roadway shall be backfilled as outlined under Article 550.07 Method 1, 2, or 3 of the Standard Specifications.

Aggregate used shall contain no frozen matter nor shall the aggregate be placed on snow or ice. Jetting or inundating shall not be done during freezing weather.

After the filling of the void, the site shall be graded and cleaned-up to the satisfaction of the Engineer.

If there is a possibility of trapping of sub-surface drainage, basement floors shall be broken to comply with local building codes to prevent entrapment of water.

A suitable earth cap, minimum 12 inches (300 mm) thick, shall be placed as the final backfill lift on all cavity areas outside the proposed embankment or pavement structure.

This work will not be paid for separately but shall be included in the cost of the building removal pay items included in the contract.

INLETS, TYPE G-1

Effective October 1, 1995

Revised January 1, 2007

This work shall consist of furnishing all labor, equipment, and material for the construction of Type G-1 Inlets and Combination Concrete Curb and Gutter in accordance with Sections 602 and 606 of the Standard Specifications and the details in the plans.

Add "INLETS, TYPE G-1" to Article 602.16 of the Standard Specifications. Delete the first paragraph in Articles 606.14 and 606.15.

Payment for transitional Combination Concrete Curb and Gutter will be included in "INLETS, TYPE G-1" in accordance with details shown in the plans.

This work will be paid for at the contract unit price Each for INLETS, TYPE G-1.

INLETS, TYPE G-1, SPECIAL

Effective October 1, 1995

Revised January 1, 2007

This work shall consist of furnishing all labor, equipment, and material for the construction of Type G-1, Special inlets and Combination Concrete Curb and Gutter in accordance with Sections 602 and 606 of the Standard Specifications and the details in the plans.

Add "INLETS, G-1, SPECIAL" to Article 602.16 of the Standard Specifications. Delete the first paragraph in Articles 606.14 and 606.15.

Payment for transitional Combination Concrete Curb and Gutter will be included in "INLETS, TYPE G-1, SPECIAL" in accordance with details shown in the plans.

This work will be paid for at the contract unit price Each for INLETS, TYPE G-1, SPECIAL.

INLET-MANHOLE, TYPE G-1, 6' DIAMETER

This work shall consist of furnishing all labor, equipment, and materials for the construction of Inlet-Manhole, Type G-1, 6' Diameter and Combination Concrete Curb and Gutter in accordance with Sections 602 and 606 of the Standard Specifications and the details in the plans.

Add "INLET-MANHOLE, TYPE G-1, 6' DIAMETER" to Article 602.16 of the Standard Specifications. Delete the first paragraph of Articles 606.14 and 606.15 of the Standard Specifications.

Payment for transitional Combination Concrete Curb and Gutter will be included in "INLET-MANHOLE, TYPE G-1, 6' DIAMETER" in accordance with details shown in the plans.

The work will be paid for at the contract unit price Each for INLET-MANHOLE, TYPE G-1, 6' DIAMETER.

INLET-MANHOLE, TYPE G-1, 8' DIAMETER

This work shall consist of furnishing all labor, equipment, and materials for the construction of Inlet-Manhole, Type G-1, 8' Diameter and Combination Concrete Curb and Gutter in accordance with Sections 602 and 606 of the Standard Specifications and the details in the plans.

Add "INLET-MANHOLE, TYPE G-1, 8' DIAMETER" to Article 602.16 of the Standard Specifications. Delete the first paragraph of Articles 606.14 and 606.15 of the Standard Specifications.

Payment for transitional Combination Concrete Curb and Gutter will be included in "INLET-MANHOLE, TYPE G-1, 8' DIAMETER" in accordance with details shown in the plans.

The work will be paid for at the contract unit price Each for INLET-MANHOLE, TYPE G-1, 8' DIAMETER.

INLETS, TYPE G-1, DOUBLE

Effective July 31, 2009

This work shall consist of furnishing equipment, labor, and materials for the construction of Type G-1, Double Inlets and Combination Concrete Curb and Gutter in accordance with Section 602 and 606 of the Standard Specifications and the details in the plans.

Add "INLETS, TYPE G-1, DOUBLE" to Article 602.16 of the Standard Specifications. Delete the first paragraph in Articles 606.14 and 606.15 of the Standard Specifications.

Payment for transitional Combination Concrete Curb and Gutter will be included in "INLETS, TYPE G-1, DOUBLE" in accordance with details shown in the plans.

This work will be paid for at the contract unit price Each for INLETS, TYPE G-1, DOUBLE.

INLETS, TYPE G-1, DOUBLE, SPECIAL

Effective October 1, 1995

Revised January 1, 2007

This work shall consist of furnishing equipment, labor, and materials for the construction of Type G-1, Double, Special Inlets and Combination Concrete Curb and Gutter in accordance with Section 602 and 606 of the Standard Specifications and the details in the plans.

Add "INLETS, TYPE G-1, DOUBLE, SPECIAL" to Article 602.16 of the Standard Specifications. Delete the first paragraph in Articles 606.14 and 606.15.

Payment for transitional Combination Concrete Curb and Gutter will be included in "INLETS, TYPE G-1, DOUBLE SPECIAL" in accordance with details shown in the plans.

This work will be paid for at the contract unit price Each for INLETS, TYPE G-1, DOUBLE, SPECIAL.

INLET-MANHOLE, TYPE G-1, 5' (1.5 M) DIAMETER

Effective October 1, 1995

Revised January 1, 2007

This work shall consist of furnishing all labor, equipment, and materials for the construction of Inlet-Manhole, Type G-1, 5' (1.5 m) Diameter and Combination Concrete Curb and Gutter in accordance with Sections 602 and 606 of the Standard Specifications and the details in the plans.

Add "INLET-MANHOLE, TYPE G-1, 5' (1.5 m) DIAMETER" to Article 602.16 of the Standard Specifications. Delete the first paragraph of Articles 606.14 and 606.15 of the Standard Specifications.

Payment for transitional Combination Concrete Curb and Gutter will be included in "INLET-MANHOLE, TYPE G-1, 5' (1.5 m) DIAMETER" in accordance with details shown in the plans.

The work will be paid for at the contract unit price Each for INLET-MANHOLE, TYPE G-1, 5' (1.5 m) DIAMETER.

INLET-MANHOLE, TYPE G-1, 8' (2.4 M) DIAMETER, DOUBLE, SPECIAL

Effective October 1, 1995

Revised January 1, 2007

This work shall consist of furnishing all labor, equipment, and materials to construct the Inlet-Manhole, Type G-1, 8' (2.4 m) Diameter, Double, Special and Combination Concrete Curb and Gutter in accordance with Sections 602 and 606 of the Standard Specifications and details in the plans.

Add "INLET-MANHOLE, TYPE G-1, 8' (2.4 m) DIAMETER, DOUBLE, SPECIAL" to Article 602.16 of the Standard Specifications. Delete the first paragraph of Articles 606.14 and 606.15 of the Standard Specifications.

Payment for transitional Combination Concrete Curb and Gutter will be included in "INLET-MANHOLE, TYPE G-1, 8' (2.4 m) DIAMETER, DOUBLE, SPECIAL" in accordance with details shown in the plans.

This work will be paid for at the contract unit price Each for INLET-MANHOLE, TYPE G-1, 8' (2.4 m) DIAMETER, DOUBLE, SPECIAL.

GUARDRAIL AGGREGATE EROSION CONTROL

Effective: February 1, 1993

Revised: January 1, 2007

This work shall consist of furnishing, placing, and shaping crushed aggregate placed around and behind guardrail posts in accordance with plan details.

Method of Measurement: The aggregate for constructing the Guardrail Aggregate Erosion Control will be measured in Tons (Metric Tons).

The Geotextile Fabric will not be measured for payment.

Basis of Payment: Guardrail Aggregate Erosion Control will be paid for at the contract unit price per Ton (Metric Ton) for GUARDRAIL AGGREGATE EROSION CONTROL measured as specified herein. The Geotextile Fabric will not be measured for payment but shall be included in the cost per Ton (Metric Ton) for GUARDRAIL AGGREGATE EROSION CONTROL.

TRAFFIC BARRIER TERMINALS

Effective: February 1, 1996

Revised: November 5, 2004

Widening of existing shoulders/slopes for the construction of Traffic Barrier Terminals shall be completed as directed by the Engineer and paid for as specified in Article 109.04 of the Standard Specifications.

EQUIPMENT VAULT FOR NUCLEAR TESTING EQUIPMENT

Effective June 24, 1993

Revised November 8, 2019

Add the following to the list of equipment and furniture to be furnished under Article 670.05 Engineer's Field Laboratory.

A cabinet or vault shall be provided for the nuclear density equipment which shall have a suitable barrier system of concrete, steel, lead, or other radiation barrier material and shall remain at the job site. The vault shall be located in a secure, weather-proof location and be a minimum of 15' from any work station and approved by the RSO. It shall have a dimension capable of holding the number of units being stored at the site and shall have a lock for security to prevent intruders from gaining access to this equipment. All walls and doors of the unit shall be sufficient thickness to prevent any radiation leakage from the equipment should a malfunction result which would allow this leakage.

The cost of furnishing the equipment vault will not be paid for separately but shall be considered as included in the unit cost for ENGINEER'S FIELD LABORATORY.

TRAFFIC CONTROL PLAN

Effective: May 1, 2025

Traffic control shall be in accordance with the applicable sections of the "Standard Specifications for Road and Bridge Construction," the applicable guidelines contained in the "Illinois Manual on Uniform Traffic Control Devices for Streets and Highways," these Special Provisions, and any special details and Highway Standards contained herein and in the plans.

Special attention is called to Section 701 and Articles 107.09 and 107.14 of the "Standard Specifications for Road and Bridge Construction" and the following Highway Standards relating to traffic control:

| | | | | | |
|--------|--------|--------|--------|--------|--------|
| 701101 | 701106 | 701411 | 701422 | 701423 | 701426 |
| 701427 | 701451 | 701456 | 701601 | 701606 | 701611 |
| 701701 | 701801 | 701901 | BLR 21 | BLR 22 | |

Special attention is called to the detour plans in the plans. The Contractor shall close IL Route 116/IL Route 8 (Cedar Street extension) and detour traffic as detailed in the detour plans.

Contractor Access: At road closure locations where Type III Barricades are installed in a manner that will not allow Contractor access to the project without relocation of one or more of the barricades, the arrangement of the barricades at the beginning of each workday may be relocated, when approved by the Engineer, in the manner shown on Highway Standard 701901 for "Road Closed to Through Traffic". "Road Closed" signs (R11-2), supplemented by "Except Authorized Vehicles" signs (R3-I101), shall be mounted on both the near-right and far-left barricade(s). At the end of each workday, the barricades shall be returned to their in-line positions. This work will be included in the cost of the contract, and no extra compensation will be allowed.

Traffic Control Notes and Working Restrictions

1. The Contractor shall notify the IDOT Resident Engineer two (2) weeks prior to the closure of IL 8/IL 116 (Cedar Street extension).
2. The IL 116/IL 8 and the Main Street interchange shall not be closed prior to January 15, 2026 and shall be opened to traffic by November 15, 2027. See Special Provisions for Incentive/Disincentive included herein.
3. No permanent lane closures shall be allowed during the allowable working days after the November 15, 2027 opening date.
4. The removal, delivery, and erection of bridge beams for S.N. 090-0183 which carries IL Routes 116/8 over US Route 24 (Main Street), on the project shall be limited to the hours of 10:00 P.M. to 6:00 A.M. from Sunday through Thursday or as specified by the Engineer. The Contractor may temporarily stop traffic on US Route 24 (Main Street) for a maximum of 20minutes at a time to remove or set the beams. The Engineer shall be notified a minimum of seven (7) calendar days prior to work operations that will stop traffic.
5. During the winter period from November 1, 2025 to March of 2026, only daytime lane closures are allowed on Main Street.
6. Within the winter period from November 15, 2026 to January 14, 2027, two lanes of traffic shall be maintained in each direction along Main Street and no daytime lane closures shall be allowed. From January 15, 2027 to February 28, 2027, two lanes of traffic shall be maintained in each direction along Main Street and daytime lane closures shall be allowed.

TRAFFIC CONTROL AND PROTECTION (SPECIAL)

Specific traffic control plan details and Special Provisions have been prepared for this contract. This work shall include all labor, materials, transportation, handling and incidental work necessary to furnish, install, maintain and remove all traffic control devices required as indicated in the plans and as approved by the Engineer.

This work shall be completed in accordance with Article 107.14 and Section 701 of the Standard Specifications, the staging details and notes in the plans, applicable Highway Standards, applicable District 4 Standards, the Special Provisions and as specified herein.

The plan details present a suggested means for implementing the necessary traffic control for this project. The plans do not attempt to detail or define all construction conditions which may require installation of traffic controls and items to meet unforeseen needs. The Contractor may revise or modify the traffic control as shown in the plans with the written permission of the Engineer. The cost of any traffic control devices that must remain upon completion of the contract shall be included in this work.

Existing regulatory traffic signing shall be relocated as needed for each stage of construction. In addition, the Contractor shall furnish and install temporary regulatory signing at the locations shown in the plans. The Contractor shall maintain all temporarily relocated signs until the new permanent signing has been installed. The furnishing, installation, relocation and removal of all temporary regulatory or warning traffic signs will not be paid for separately but shall be governed by Article 107.25 of the Standard Specifications.

Method of Measurement: All traffic control (except Temporary Pavement Markings) indicated on the traffic control plan details and specified in the Special Provisions will be measured for payment on a lump sum basis. No additional compensation will be made for any alterations, modifications or additions necessary to accommodate the traffic control to construct the various work items shown in the plans.

Basis of Payment: All traffic control (including pedestrian) and protection will be paid for at the contract lump sum price for TRAFFIC CONTROL AND PROTECTION (SPECIAL). Temporary pavement markings will be measured and paid for according to Section 703 and Section 780. Pavement marking removal will be measured and paid for according to Section 703 and Section 783 of the Standard Specifications.

Temporary concrete barrier and relocate temporary concrete barrier will be measured and paid for according to Section 704 of the Standard Specifications.

Changeable message signs not referenced within a traffic control and protection highway standard will be measured for payment separately.

**INCENTIVE/DISINCENTIVE FOR REPLACEMENT OF STRUCTURES OVER TPZ RAILROAD,
MAIN STREET AND MAINLINE AND RAMP PAVEMENT REPLACEMENT**

The Contractor will be allowed a bridge closure from Monday, January 15, 2026 through Friday, November 15, 2027 for the reconstruction of two (2) structures carrying IL 116/IL 8 over the TPZ Railyard and IL 29/US 24 (Main Street) as well as the replacement of the mainline and ramp pavements. The Contractor shall schedule his/her operations so as to complete all work, except as specified below, and open all the roadway to traffic within the allowed closure. The Contractor shall note that this completion date is based on an expedited work schedule. The Contractor will be allowed 30 working days after the November 15, 2027 completion date to complete the punch list, landscaping, highway lighting and permanent pavement markings.

Failure to Complete the Work on Time: Should the Contractor fail to complete the work on or before the specified date of completion or within such extended time allowed by the Department, the Contractor shall be liable to the Department in the amount of \$16,500 not as a penalty but as liquidated and ascertained damages for Each Calendar Day beyond the November 15, 2027 completion date or extended time as may be allowed. Such damages may be deducted by the Department from the monies due to the Contractor. Working days shall be governed by the provision of Article 108.09 of the Standard Specifications.

In fixing the damages as set out herein, the desire is to establish a certain mode of calculation for the work because the Department's actual loss, in the event of delay, cannot be predetermined, would be difficult of ascertainment, and a matter of argument and unprofitable litigation. This mode is an equitable rule for measurement of the Department's actual loss and fairly takes into account the loss of use of the roadway if the project is delayed in completion. The Department shall not be required to provide any actual losses to recover these liquidated damages provided herein, as these damages are very difficult to ascertain. Furthermore, no provision of this clause shall be construed as a penalty, as such is not the intention of the parties.

A Calendar Day is every day on the calendar and starts at 12:00 midnight and ends at the following 12:00 midnight, twenty-four hours later. Liquidated damages will not be assessed for any less than twenty-four hours.

Incentive Payment Plan: The nature of this project is such that the use of this roadway cannot be safely and efficiently used until all specified work is complete. On this basis, the Contractor shall be entitled to an Incentive Payment for the completion of all work as set forth by the date of completion.

The Incentive Payment shall be paid at a rate of \$15,000 per Calendar Day for Each Day of early completion. The maximum payment under this incentive pay will be limited to 14 Calendar Days.

A Calendar Day is every day on the Calendar and starts at 12:00 midnight and ends at the following 12:00 midnight, twenty-four hours later. No payment will be paid for any day less than twenty-four hours.

PROSECUTION OF WORK

In order to assure the timely completion of the work involved in this project, it may be necessary for the Contractor to work extended work hours including weekends and winter seasons.

Any expenses incurred by the Contractor in order to comply with this special provision will not be paid for separately but shall be included in the contract bid price.

Should the Contractor be delayed in the commencement, prosecution, or completion of the work for any reason, there shall be no extension of the incentive payment calculation days even though there may be granted an extension of time for completion of the work unless significant extra work is added to the contract by the Department. No Incentive Payment will be made if the Contractor fails to complete the work before the specified date of completion or within such extended time allowed by the Department. Failure of the Contractor to complete all work as required by the contract before the November 3, 2027 completion date, shall release and discharge the State, the Department and all its officers, agents and employees from and all claims and demands for the payment of any incentive amount of damages arising from the refusal to pay any incentive amount.

If the contract is part of a combination award, no Incentive Payment shall commence on this contract which is part of the combination until all work on contracts which are part of the combination award has been completed.

WIDTH RESTRICTION SIGNING

Effective: November 1, 2007 Revised: January 1, 2019

Description. This work shall consist of providing, placing, maintaining, and removing width restriction signing as shown on the plans and special provisions. Width restriction signing shall be required when the roadway width will be less than 17'-6" as measured from face to face of temporary concrete barrier and a concrete parapet, guardrail or other fixed, immovable barrier. Width restriction signing may be required when the roadway width will be less than 17'-6" as measured from movable traffic control devices and a fixed object (concrete parapet, guardrail or other immovable roadside barrier). The contractor shall provide signing if the traffic control devices cannot be shifted (in areas of a fixed object and patching/paving/centerline work etc.) to accommodate a traveled way opening of 17'-6". The Contractor shall furnish all materials, equipment, labor, and other essentials necessary to accomplish this work and all other work described herein and as directed by the Engineer.

Materials. All sign post materials shall be in accordance with Articles/Sections: 1093.01(a), 10007.05. Galvanizing will not be required. The nominal size of wood posts shall be 4 in. x 4 in. (100 mm x 100 mm).

Equipment. All equipment shall be in accordance with Article/Section 1106.01.

Notification. The Contractor shall notify the Traffic Control Supervisor, in writing, when the Contractor receives an award letter for the contract. The letter shall state the anticipated start date of lane width restrictions. The twenty-one (21) day notice will start from the Award date. No width restrictions will be allowed until twenty-one (21) days after receiving notice from the Contractor. The Contractor may elect to provide the anticipated start date of lane width restrictions at the Preconstruction meeting so long as there is a minimum of twenty-one (21) days advanced notice.

Traffic Control Supervisor Don Hoffman (309) 671-4488

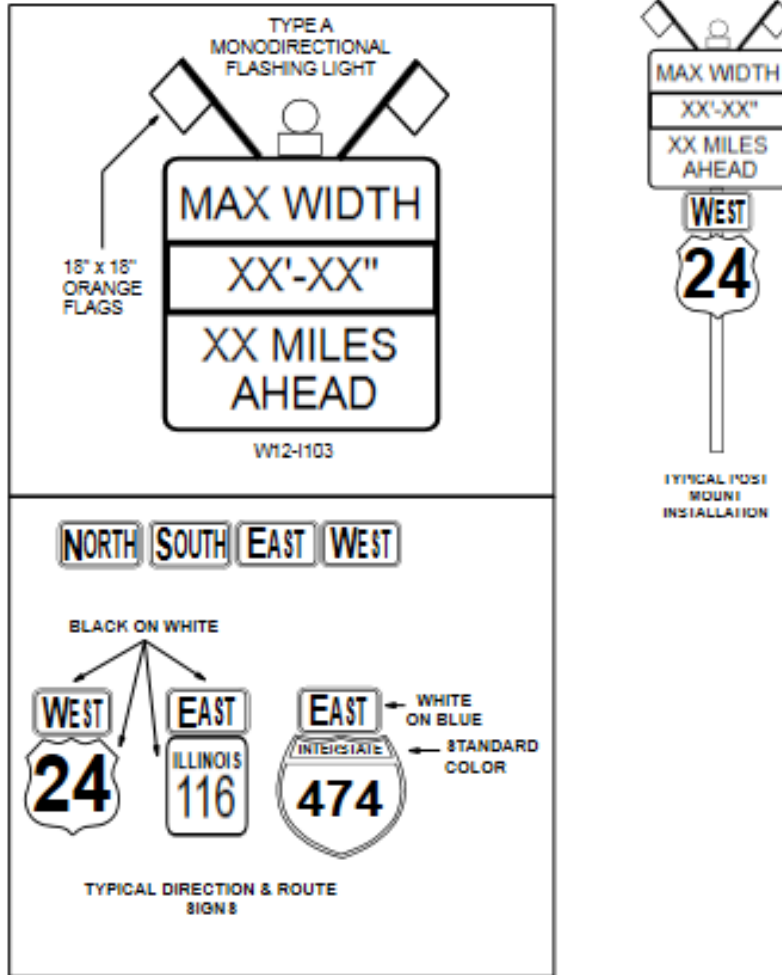
Failure to provide required advanced notice may delay project at the expense of the Contractor.

General. The Contractor shall provide the route and directional (North, South, East and West) signage. The route and directional signage shall be placed, maintained, and removed by the Contractor. The route sign shall visually be the same as the existing route signs as posted by IDOT. The directional signage shall be black lettering on a white background. Interstate signs shall have the cardinal direction signs with white on a blue background. The route and directional signage shall be placed below Sign W12-I103.

Locations, distances and quantity of signs and shall be as shown on the plan sheets or in the Traffic Control Plan. All final field locations will be marked by the Bureau of Operations, Traffic Control Supervisor.

It shall be the Contractor's responsibility to make arrangements for the J.U.L.I.E. locates.

Basis of Payment: This work will not be paid for separately but will be included in the cost of Traffic Control and Protection pay items. This work shall consist of providing, placing, maintaining, and removing width restriction signing as shown on the plans and special provisions and no additional compensation will be allowed.



TRAFFIC CONTROL AND PROTECTION BLR 21

Effective: April 25, 2008 Revised: April 24, 2020

This work shall be in accordance with Section 701 of the Standard Specifications and shall include all material, equipment, and labor necessary to install the traffic control items as shown on the Highway Standard BLR 21 or as shown and described in the plans and specifications.

Add the following to the first paragraph of Article 701.20(b):

"Traffic Control and Protection Standard BLR 21".

TRAFFIC CONTROL AND PROTECTION BLR 22

Effective: April 25, 2008 Revised: April 24, 2020

This work shall be in accordance with Section 701 of the Standard Specifications and shall include all material, equipment, and labor necessary to install the traffic control items as shown on the Highway Standard BLR 22 or as shown and described in the plans and specifications.

Add the following to the first paragraph of Article 701.20(b):

"Traffic Control and Protection Standard BLR 22".

TEMPORARY CONCRETE BARRIER REFLECTORS

Effective: January 21, 2005 Revised: November 6, 2020

Installation of reflectors shall be in accordance with the Traffic Control Standards, plan details, and specifications, except that the reflector shall be fastened to the concrete using a mechanical fastener such as a screw suitable for use in concrete.

Reflectors mounted on temporary concrete barrier will not be measured for payment and shall be included in the cost of pay items associated with temporary concrete barrier.

PCC SLIPFORM PAVING AGGREGATE OPTIMIZATION

Effective: August 3, 2012 Revised: January 1, 2022

Delete Note 7/ of Article 1004.01(c) and replace Article 1004.02(d)(1) with the following:

For the slipform paving of concrete pavement, the Class PV concrete shall be uniformly graded. This may be accomplished by using a uniformly graded single coarse aggregate, or by blending two or more coarse aggregate sizes. As a minimum for multiple coarse aggregate sizes, CA 7 or CA 11 shall be blended with CA 13, CA 14, or CA 16. The final single coarse aggregate or combined coarse aggregate gradation shall have minimum 45 percent and maximum 60 percent passing the 1/2 in. (12.5 mm) sieve. However, the Contractor may propose for approval by the Engineer an alternate uniformly graded concrete mixture using the information in the "Portland Cement Concrete Level III Technician Course – Manual of Instructions for Design of Concrete Mixtures".

PCC SUPERSTRUCTURE AGGREGATE OPTIMIZATION

Effective: August 4, 2006 Revised: January 1, 2022

Delete Note 7/ of Article 1004.01(c) and replace Article 1004.02(d)(1) with the following:

For the bridge superstructure and bridge approach slab, the Class BS concrete shall be uniformly graded.

This may be accomplished by using a uniformly graded single coarse aggregate, or by blending two or more coarse aggregate sizes. As a minimum for multiple coarse aggregate sizes, CA 7 or CA 11 shall be blended with CA 13, CA 14, or CA 16. The final single coarse aggregate or combined coarse aggregate gradation shall have minimum 45 percent and maximum 60 percent passing the 1/2 in. (12.5 mm) sieve. However, the Contractor may propose for approval by the Engineer an alternate uniformly graded concrete mixture using the information in the "Portland Cement Concrete Level III Technician Course – Manual of Instructions for Design of Concrete Mixtures".

For bridge decks and bridge approach slabs, the as-placed water cement ratio shall be between 0.39 and 0.41. The coarse aggregate shall be listed on the Department's Bureau of Materials and Physical Research "Freeze Thaw Rating List".

Concrete Superstructures Aggregate Optimization will not be paid for separately but shall be considered as included in the unit cost of CONCRETE SUPERSTRUCTURES.

MEMBRANE CURING METHOD

Effective: July 29, 2016

Revised: November 17, 2017

Revise Article 1020.13(a)(4) paragraph 2 to read:

"After all finishing work to the concrete surface has been completed, the surface and all exposed edges shall be sealed with membrane curing compound of the type specified within ten minutes. The seal shall be maintained for the specified curing period. The edges of the concrete shall, likewise, be sealed within ten minutes after the forms are removed. Two separate applications, applied at least one minute and no more than fifteen (15) minutes apart, each at the rate of not less than 1 gal./250 sq. ft. (0.16L/sq. m) will be required upon the surfaces and edges of the concrete. These applications shall be made with the mechanical equipment specified. Type III compound shall be agitated immediately before and during the application.

PCC QMP ELECTRONIC REPORT SUBMITTALS

Effective: January 13, 2022

The Contractor's QC personnel shall be responsible for electronically submitting the following reports to the Department: PRO and IND data for BMPR MI654 "Air, Slump, & Quantity"; PRO data for BMPR MI655 "PCC Strength"; and PRO data for BMPR MI504 "Field/Lab Gradation". The format for the electronic submittals will be the "QMP" reporting program which will be provided by the Department. Microsoft Office 2007 or newer is required for this program which must be provided by the Contractor.

PCC AUTOMATIC BATCHING EQUIPMENT

Effective: April 23, 2010 Revised: August 1, 2023

Portland cement concrete provided shall be produced from batch plants that conform to the requirements of Article 1103.03 (a) and (b) of the Standard Specifications for Road and Bridge Construction. Semi-automatic batching will not be allowed.

Plants shall have computerized batching interfaced with a printer. IDOT Producer Number, IDOT Concrete Material Code, batch weights, aggregate mixtures, water added, amount of each admixture or additive, and percent variance from design shall be printed for each batch. The ticket shall state the actual water-cement ratio as batched, and the amount of water that can be added to the batch without exceeding the maximum water-cement ratio. Truck delivery tickets will still be required as per Article 1020.11 (a)(7) of the Standard Specifications.

RETAINING WALL REMOVAL

This work will consist of the removal and satisfactory disposal of the existing concrete or block walls and/or foundations in accordance with applicable portions of Section 501 of the Standard Specifications, as shown in the plans, and as directed by the Engineer.

No adjustments shall be made for variations in thickness, width, and/or length.

Required saw cuts will not be measured for payment.

Basis of Payment: This work will be paid for at the contract unit price per Foot for RETAINING WALL REMOVAL, which price includes the cost of the saw cuts.

APPROACH SLAB REMOVAL

This work shall consist of the complete removal of the existing approach slab as shown in the plans. All pavement and appurtenances within the limits of the approach slab shall be removed. This includes, but not limited to, Portland Cement Concrete pavement, hot-mix asphalt overlays, reinforcement, curbs, inlets, stabilized subbase, approach footing (sleeper slab), connector pavement, and expansion joint material adjacent to the approach slab.

Approach slab removal shall be performed in accordance with Section 440 of the Standard Specifications.

Approach Slab Removal will be measured for payment in place and the area computed in square yards.

This work will be paid for at the contract unit price per Square Yard for APPROACH SLAB REMOVAL.

FENCE REMOVAL

Description. This work shall consist of the complete removal and disposal of existing fence, regardless of type, as shown in the plans or as directed by the Engineer.

Construction Requirements. All associated hardware and appurtenances of the existing fence including but not limited to post, foundations, fittings, gates, and accessories shall be removed and disposed of by the Contractor. All post or foundation holes shall be backfilled and compacted to the satisfaction of the Engineer. Any part of the fence that is not called out to be removed and damaged by the Contractor's work shall be replaced to the satisfaction of the Engineer at the expense of the Contractor

Method of Measurement. Fence removal shall be measured for payment per Foot and measured along the top of the fence from center to center of end posts, including length occupied by gates.

Basis of Payment. Removal of the existing fence will be paid for at the contract unit price per Foot for FENCE REMOVAL.

PORTLAND CEMENT CONCRETE SIDEWALK 4 INCH (SPECIAL)

This work shall consist of furnishing all labor equipment, material necessary for the construction of ADA curb ramps in accordance with Section 424 of the Standard Specifications and plan details at the specified locations. This work also includes the construction of side curbs, curb walls, side flares, and transitional sidewalk panels as show in the individual ADA curb ramp details.

The Contractor shall verify removal limits and field elevations versus plan elevations to ensure that ADA standards are met prior to placement of concrete.

The Contractor shall utilize radial detectable warning panels around any radius and shall be responsible for measuring the radius prior to ordering panels.

Method of Measurement: This work will be measured for payment in-place and the area computed in Square Feet. No deduction will be made, or detectable warnings located within the ramp.

Temporary ramps needed for pedestrian accommodations at ADA ramp construction locations shall be paid in accordance with Article 109.04 of the Standard Specifications at Each location.

Basis of Payment: This work shall not be paid for separately but shall be considered included in the contract unit price per Square Foot for PORTLAND CEMENT CONCRETE SIDEWALK 4 INCH, (SPECIAL).

TEMPORARY PAVEMENT REMOVAL

Description: This work shall be performed in accordance with Section 440 of the Standard Specifications, except as herein modified.

Method of Measurement: This work shall be measured for payment in Square Yards, regardless of the type of temporary pavement removed, and shall include removal of patching as used for traffic staging.

Basis of Payment: This work shall be paid for at the contract unit price, per Square Yard for TEMPORARY PAVEMENT REMOVAL.

CONCRETE GUTTER, TYPE A (SPECIAL)

This work shall consist of constructing a new Concrete Gutter, Type A at the locations shown in the plans, as directed by the Engineer, in accordance with the plan details, applicable portions of Section 606 of the Standard Specifications, and as described herein.

Any excavation and disposal of material and reinforcement shall not be paid for separately but shall be included in the cost of CONCRETE GUTTER, TYPE A (SPECIAL).

Transitions pieces shall be measured and paid for as CONCRETE GUTTER, TYPE A (SPECIAL).

Method of Measurement. This work will be measured for payment in Feet in the flow line of the gutter.

Basis of Payment. This work will be paid for at the contract unit price per Foot for CONCRETE GUTTER, TYPE A (SPECIAL), which price shall include all labor, equipment, and materials necessary to complete the work as herein specified.

EROSION CONTROL BLANKET (SPECIAL)

This Special Provision revises Section 251 of the Standard Specifications for Road and Bridge Construction to eliminate the use of Excelsior Blanket for Erosion Control Blanket. This work shall consist of furnishing, transporting, and placing 100 % biodegradable leno weave erosion control blanket over seeded areas with biodegradable anchors, according to Section 251 except as modified herein.

Delete Article 1081.10(a) Excelsior Blanket.

Delete Article 1081.10 (b) Knitted Straw Mat and substitute the following:

Knitted Straw Mat. Knitted straw mat shall be a machine-produced mat of 100% clean, weed free agricultural straw. The blanket shall be of consistent thickness with the straw evenly distributed over the entire area of the blanket with a functional longevity of up to 12 months. The blanket shall be covered on top and bottom sides with a 100% biodegradable woven natural organic fiber netting. No plastic netting will be allowed.

The top and bottom netting shall consist of machine directional strands formed from two intertwined yarns with cross directional strands interwoven through the twisted machine strands (leno weave) to form an approximate 0.50 x 1.0 (1.27 x 2.54 cm) mesh. Joints shall be movable joints, not fixed or welded.

The blanket shall be sewn together on 1.50-inch (3.81 cm) to 2-inch centers with 100% biodegradable thread. The blanket shall be manufactured with a colored thread stitched along both outer edges (approximately 2-5 inches (5-12.5cm) from the edge) as an overlap guide for adjacent mats.

Delete Article 1081.10(d) Wire Staples.

Add the following to Article 1081.10 (e) Wood Stakes:

The 100% biodegradable hardwood stake shall exhibit ample rigidity to enable being driven into hard ground, with sufficient flexibility to resist breakage. The wooden stake shall be a minimum of 6 inches in length with a 1.25-inch head to hold the blanket in place. Biodegradable plastic stakes nor metal wire stakes will be allowed.

Add the following to Article 251.06 Method of Measurement:

EROSION CONTROL BLNAKET (SPECIAL) will be measured for payment in place in Square Yards of actual surface areas covered.

Add the following to Article 251.07 Basis of Payment:

This work will be paid for at the contract unit price per Square Yard for EROSION CONTROL BLANKET (SPECIAL).

REMOVAL AND DISPOSAL OF REGULATED SUBSTANCES

Description. This work shall consist of the removal and disposal of regulated substances according to Section 669 of the Standard Specifications as revised below.

Contract Specific Work Areas. The excavated soil and groundwater within the work areas listed below shall be managed as either "uncontaminated soil", hazardous waste, special waste or non-special waste. For stationing, the lateral distance is measured from centerline and the farthest distance is the offset distance or construction limit, whichever is less.

Soil Disposal Analysis. When the waste material requires sampling for landfill disposal acceptance, the Contractor shall secure a written list of the specific analytical parameters and analytical methods required by the landfill. The Contractor shall collect and analyze the required number of samples for the parameters required by the landfill using the appropriate analytical procedures. A copy of the required parameters and analytical methods (from landfill email or on landfill letterhead) shall be provided as Attachment 4A of the BDE 2733 (Regulated Substances Final Construction Report). The price shall include all sampling materials and effort necessary for collection and management of the samples, including transportation of samples from the job site to the laboratory. The Contractor shall be responsible for determining the specific disposal facilities to be utilized; and collect and analyze any samples required for disposal facility acceptance using a NELAP certified analytical laboratory registered with the State of Illinois.

The following contract specific work areas shall be monitored by the Environmental Firm for soil contamination and workers protection.

ISGS Site 3368-2/3369-3 – ROW , 200-900 blocks of Cedar St., East Peoria, Tazewell County:

- Station 42+00 to Station 43+00; 0 to 50 feet LT. The Engineer has determined this material from 0 to 5 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(c). Contaminants of concern sampling parameters include: iron and manganese.
- Station 42+00 to Station 43+00; 0 to 50 feet LT. The Engineer has determined this material from 5 to 8 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(2). Contaminants of concern sampling parameters include: iron and manganese.
- Station 42+00 to Station 43+00; 0 to 50 feet RT. The Engineer has determined this material from 0 to 5 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(3). Contaminants of concern sampling parameters include: SVOCs and metals.
- Station 42+00 to Station 43+00; 0 to 50 feet RT. The Engineer has determined this material from 5 to 8 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(c). Contaminants of concern sampling parameters include: iron.
- Station 43+00 to Station 49+00; 0 to 60 feet LT and RT. The Engineer has determined this material from 0 to 1 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(5). Contaminants of concern sampling parameters include: SVOCs and metals.
- Station 49+00 to Station 51+00; 0 to 60 feet LT. The Engineer has determined this material from 0 to 1 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(1). Contaminants of concern sampling parameters include: SVOCs and metals.
- Station 49+00 to Station 51+00; 0 to 50 feet RT. The Engineer has determined this material from 0 to 1 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(3). Contaminants of concern sampling parameters include: SVOCs.
- Station 51+00 to Station 53+00; 0 to 60 feet LT. The Engineer has determined this material from 0 to 1 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(3). Contaminants of concern sampling parameters include: SVOCs.
- Station 51+00 to Station 53+00; 0 to 50 feet RT. The Engineer has determined this material from 0 to 1 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(5). Contaminants of concern sampling parameters include: SVOCs and metals.

- Station 53+00 to Station 55+00; 0 to 60 feet LT. The Engineer has determined this material from 0 to 5 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(5). Contaminants of concern sampling parameters include: SVOCs and metals.
- Station 53+00 to Station 55+00; 0 to 60 feet LT. The Engineer has determined this material from 5 to 8 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(2). Contaminants of concern sampling parameters include: metals.
- Station 53+00 to Station 55+00; 0 to 50 feet RT. The Engineer has determined this material from 0 to 5 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(5). Contaminants of concern sampling parameters include: SVOCs and metals.
- Station 53+00 to Station 55+00; 0 to 50 feet RT. The Engineer has determined this material from 5 to 8 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(3). Contaminants of concern sampling parameters include: SVOCs and metals.
- Station 57+00 to Station 59+00; 0 to 60 feet LT. The Engineer has determined this material from 0 to 1 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(3). Contaminants of concern sampling parameters include: SVOCs and metals.
- Station 57+00 to Station 59+00; 0 to 50 feet RT. The Engineer has determined this material from 0 to 1 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(5). Contaminants of concern sampling parameters include: SVOCs and metals.
- Station 59+00 to Station 61+00; 0 to 60 feet LT and RT. The Engineer has determined this material from 0 to 1 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(2). Contaminants of concern sampling parameters include: metals.
- Station 61+00 to Station 63+00; 0 to 60 feet LT. The Engineer has determined this material from 0 to 1 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(3). Contaminants of concern sampling parameters include: SVOCs and metals.
- Station 61+00 to Station 65+00; 0 to 50 feet RT. The Engineer has determined this material from 0 to 1 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(5). Contaminants of concern sampling parameters include: SVOCs and metals.
- Station 63+00 to Station 65+00; 0 to 60 feet LT. The Engineer has determined this material from 0 to 1 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(5). Contaminants of concern sampling parameters include: SVOCs and metals.

- Station 65+00 to Station 67+00; 0 to 60 feet LT. The Engineer has determined this material from 0 to 5 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(3). Contaminants of concern sampling parameters include: SVOCs and metals.
- Station 65+00 to Station 67+00; 0 to 60 feet LT. The Engineer has determined this material from 5 to 8 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(c). Contaminants of concern sampling parameters include: iron.
- Station 65+00 to Station 67+00; 0 to 60 feet RT. The Engineer has determined this material from 0 to 5 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(c). Contaminants of concern sampling parameters include: iron.
- Station 67+00 to Station 69+00; 0 to 60 feet LT and RT. The Engineer has determined this material from 0 to 3 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(5). Contaminants of concern sampling parameters include: metals.
- Station 69+00 to Station 71+00; 0 to 60 feet LT. The Engineer has determined this material from 0 to 3 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed 26 in accordance with Article 669.05(a)(1). Contaminants of concern sampling parameters include: metals.
- Station 71+00 to Station 73+00; 0 to 60 feet LT. The Engineer has determined this material from 0 to 1 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(5). Contaminants of concern sampling parameters include: SVOCs and metals.
- Station 71+00 to Station 73+00; 0 to 50 feet RT. The Engineer has determined this material from 0 to 1 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(3). Contaminants of concern sampling parameters include: SVOCs and metals.
- Station 73+00 to Station 75+00; 0 to 60 feet RT and LT. The Engineer has determined this material from 0 to 1 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(5). Contaminants of concern sampling parameters include: SVOCs and metals.

ISGS Site 3369-3 – ROW , 0-200 blocks of Cedar St., East Peoria, Tazewell County:

- Station 81+00 to Station 82+50; 0 to 50 feet RT. The Engineer has determined this material from 0 to 6 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(2). Contaminants of concern sampling parameters include: SVOCs and metals.
- Station 307+50 to Station 309+00; 0 to 30 feet LT. The Engineer has determined this material from 0 to 13 feet bgs in the vicinity of the station and off-set shall be risk managed in accordance with Article 669.05(a)(1). Contaminants of concern sampling parameters include: SVOCs and metals.

- Station 309+00 to Station 308+50; 0 to 50 feet LT. The Engineer has determined this material from 0 to 13 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(2). Contaminants of concern sampling parameters include: metals.
- Station 87+00 to Station 191+00; 0 to 40 feet RT. The Engineer has determined this material from 0 to 20 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(2). Contaminants of concern sampling parameters include: metals.
- Station 87+00 to Station 191+00; 0 to 40 feet LT. The Engineer has determined this material from 0 to 20 feet bgs in the vicinity of the station and off-set shall be risk managed in accordance with Article 669.05(a)(1). Contaminants of concern sampling parameters include: SVOCs and metals.
- Station 191+00 to Station 192+50; 0 to 40 feet RT. The Engineer has determined this material from 0 to 15 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(2). Contaminants of concern sampling parameters include: metals.
- Station 191+00 to Station 192+50; 0 to 40 feet RT. The Engineer has determined this material from 15 to 20 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(c). Contaminants of concern sampling parameters include: manganese.
- Station 192+50 to Station 194+00; 0 to 40 feet RT. The Engineer has determined this material from 0 to 15 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(2). Contaminants of concern sampling parameters include: metals.
- Station 192+50 to Station 194+00; 0 to 40 feet RT. The Engineer has determined this material from 15 to 20 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(1). Contaminants of concern sampling parameters include: metals.
- Station 82+50 to Station 83+50; 0 to 25 feet RT. The Engineer has determined this material from 0 to 6 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(2). Contaminants of concern sampling parameters include: metals.
- Station 85+00 to Station 87+00; 0 to 40 feet LT. The Engineer has determined this material from 0 to 20 feet bgs in the vicinity of the station and off-set shall be risk managed in accordance with Article 669.05(a)(1). Contaminants of concern sampling parameters include: SVOCs and metals.
- Station 85+00 to Station 87+00; 0 to 40 feet RT. The Engineer has determined this material from 0 to 20 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(2). Contaminants of concern sampling parameters include: metals.

- Station 81+00 to Station 83+00; 0 to 50 feet LT. The Engineer has determined this material from 0 to 6 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(2). Contaminants of concern sampling parameters include: metals.
- Station 406+50 to Station 408+50; 0 to 50 feet LT. The Engineer has determined this material from 0 to 13 feet bgs in the vicinity of the station and off-set shall be risk managed in accordance with Article 669.05(a)(1). Contaminants of concern sampling parameters include: SVOCs and metals.
- Station 408+00 to Station 410+00; 0 to 50 feet RT. The Engineer has determined this material from 0 to 13 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(2). Contaminants of concern sampling parameters include: metals.
- Station 195+00 to Station 196+00; 0 to 25 feet RT. The Engineer has determined this material from 15 to 20 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(1). Contaminants of concern sampling parameters include: metals.
- Station 408+50 to Station 410+00; 0 to 50 feet L. The Engineer has determined this material from 0 to 13 feet bgs in the vicinity of the station and off-set meets the criteria of and off-set shall be risk managed in accordance with Article 669.05(a)(1). Contaminants of concern sampling parameters include: SVOCs and metals.
- Station 410+00 to Station 411+50; 0 to 50 feet LT. The Engineer has determined this material from 0 to 5 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(3). Contaminants of concern sampling parameters include: SVOCs and metals.
- Station 410+00 to Station 411+50; 0 to 50 feet LT. The Engineer has determined this material from 5 to 13 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(2). Contaminants of concern sampling parameters include: metals.
- Station 101+00 to Station 103+00; 0 to 50 feet LT. The Engineer has determined this material from 0 to 11 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(3). Contaminants of concern sampling parameters include: SVOCs and metals.

ISGS Site 3369-9 – Community Plaza, 210-270 S. Main St., East Peoria, Tazewell County:

- Station 411+50 to Station 413+50; 0 to 30 feet LT. The Engineer has determined this material from 0 to 6 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(2). Contaminants of concern sampling parameters include: metals.
- Station 97+00 to Station 99+00; 0 to 50 feet RT. The Engineer has determined this material from 0 to 6 feet bgs in the vicinity of the station and off-set meets the criteria of and off-set shall be risk managed in accordance with Article 669.05(a)(1). Contaminants of concern sampling parameters include: SVOCs and metals.

ISGS Site 3369-15 – Commercial Building, 604 S. Main St. East Peoria, Tazewell County:

- Station 310+00 to Station 311+50; 0 to 25 feet RT. The Engineer has determined this material from 0 to 25 feet bgs in the vicinity of the station and off-set meets the criteria of and off-set shall be risk managed in accordance with Article 669.05(a)(1). Contaminants of concern sampling parameters include: SVOCs and metals.
- Station 310+00 to Station 311+50; 0 to 25 feet RT. The Engineer has determined this material from 25 to 32 feet bgs in the vicinity of the station and off-set meets the criteria of and off-set shall be risk managed in accordance with Article 669.05(a)(5). Contaminants of concern sampling parameters include: VOCs, SVOCs, PID Readings and metals.
 - **During due-diligence activities, high PID readings were encountered at this depth. BDE has determined that a WORK ZONE is required.**
- Station 311+50 to Station 313+00; 0 to 25 feet RT. The Engineer has determined this material from 0 to 5 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(2). Contaminants of concern sampling parameters include: metals.
- Station 311+50 to Station 313+00; 0 to 25 feet RT. The Engineer has determined this material from 5 to 10 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(3). Contaminants of concern sampling parameters include: SVOCs and metals.
- Station 311+50 to Station 313+00; 0 to 25 feet RT. The Engineer has determined this material from 10 to 25 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(2). Contaminants of concern sampling parameters include: metals.
- Station 311+50 to Station 313+00; 0 to 25 feet RT. The Engineer has determined this material from 25 to 32 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(5). Contaminants of concern sampling parameters include: VOCs, SVOCs, and metals.
- Station 313+00 to Station 314+75; 0 to 25 feet RT. The Engineer has determined this material from 10 to 25 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(2). Contaminants of concern sampling parameters include: metals.
- Station 313+00 to Station 314+75; 0 to 25 feet RT. The Engineer has determined this material from 5 to 10 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(5). Contaminants of concern sampling parameters include: VOCs, SVOCs, and metals.
- Station 313+00 to Station 314+75; 0 to 25 feet RT. The Engineer has determined this material from 10 to 25 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(2). Contaminants of concern sampling parameters include: metals.

- Station 313+00 to Station 314+75; 0 to 25 feet RT. The Engineer has determined this material from 25 to 32 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(5). Contaminants of concern sampling parameters include: VOCs, SVOCs, PID Reading and metals.
 - **During due-diligence activities, high PID readings were encountered at this depth. BDE has determined that a WORK ZONE is required.**
- Station 314+75 to Station 318+75; 0 to 25 feet RT. The Engineer has determined this material from 0 to 32 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(2). Contaminants of concern sampling parameters include: metals.
- Station 117+00 to Station 119+00; 0 to 50 feet RT. The Engineer has determined this material from 0 to 10 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(2). Contaminants of concern sampling parameters include: metals.
- Station 117+00 to Station 119+00; 0 to 50 feet RT. The Engineer has determined this material from 10 to 15 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(5). Contaminants of concern sampling parameters include: metals.
- Station 117+00 to Station 119+00; 0 to 50 feet RT. The Engineer has determined this material from 15 to 20 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(c). Contaminants of concern sampling parameters include: iron.
- Station 117+00 to Station 119+00; 0 to 50 feet RT. The Engineer has determined this material from 20 to 32 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(2). Contaminants of concern sampling parameters include: metals.
- Station 119+00 to Station 121+00; 0 to 50 feet RT. The Engineer has determined this material from 0 to 15 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(2). Contaminants of concern sampling parameters include: metals.
- Station 119+00 to Station 121+00; 0 to 50 feet RT. The Engineer has determined this material from 15 to 20 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(3). Contaminants of concern sampling parameters include: SVOCs and metals.
- Station 119+00 to Station 121+00; 0 to 50 feet RT. The Engineer has determined this material from 20 to 32 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(2). Contaminants of concern sampling parameters include: metals.

- Station 121+00 to Station 123+00; 0 to 50 feet RT. The Engineer has determined this material from 0 to 20 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(2). Contaminants of concern sampling parameters include: metals.
- Station 121+00 to Station 123+00; 0 to 50 feet RT. The Engineer has determined this material from 25 to 32 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(2). Contaminants of concern sampling parameters include: metals.

ISGS Site 3943-COV-2 – Residences and Vacant Land, 700 block of S. Main St., East Peoria, Tazewell County:

- Station 295+00 to Station 297+00; 15 feet LT. The Engineer has determined this material from 0 to 2 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(2). Contaminants of concern sampling parameters include: metals.
- Station 297+00 to Station 301+00; 15 feet LT. The Engineer has determined this material from 0 to 2 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(5). Contaminants of concern sampling parameters include: metals.

ISGS Site 3943-COV-4 – Dave's Garage and Auto Sales, 727 S. Main St. East Peoria, Tazewell County:

- Station 121+25 to Station 123+00; 45 feet LT. The Engineer has determined this material from 0 to 2 feet bgs in the vicinity of the station and off-set meets the criteria of and shall be managed in accordance with Article 669.05(a)(3). Contaminants of concern sampling parameters include: SVOCs and metals.

Work Zones

Three distinct OSHA HAZWOPER work zones (exclusion, decontamination, and support) shall apply to projects adjacent to or within sites with documented leaking underground storage tank (LUST) incidents, or sites under management in accordance with the requirements of the Site Remediation Program (SRP), Resource Conservation and Recovery Act (RCRA), or Comprehensive Environmental Response, Compensation and Liability Act (CERCLA), or as deemed necessary. For this project, the work zones apply for the following ISGS PESA Sites:

Site 3369-15 (Commercial Building) 25 to 32 feet below ground surface)

Additional information on the contract specific work areas listed above collected during the regulated substances due-diligence process is available through the District's Environmental Studies Unit (DESU).

CONNECTION TO EXISTING DRAINAGE STRUCTURE

Description. This work shall consist of connecting new pipe drain to the existing drainage structure at the location as shown in the plans.

General. Contractor shall carefully core drill existing drainage structure at the proposed pipe drain invert and diameter of proposed pipe drain. The protrusion of the proposed pipe drain into the drainage structure shall not exceed one-inch.

After the pipe drain is installed, the drainage structure connection shall be made watertight by mortaring with a non-shrink concrete grout. The downstream pipes in drainage structures shall be screened to prevent entry of mortar or other debris from entering the system. Any mortar or other debris that has entered drainage structure as a result of the work shall be removed.

This work shall be performed as detailed on the plans and according to Sections 601 and 602 of the Standard Specifications.

Basis of Payment. This work will be paid for at the contract unit price per Each for CONNECTION TO EXISTING DRAINAGE STRUCTURE.

REMOVE EXISTING FLARED END SECTION

Description. This work shall consist of the removal and proper disposal of existing flared sections and grates/fittings at the locations shown in the plans.

General. This work shall be performed as detailed on the plans and according to Sections 440, 501, 550, and 605 of the Standard Specifications.

Basis of Payment. This work will be paid for at the contract unit price per Each for REMOVE EXISTING FLARED END SECTION.

INLETS, SPECIAL, NO. 1

Description. This work shall consist of constructing Precast Manhole Type A, 4' (1.22m) Diameter [IDOT Standard 602401-07] with one Type 37 Grate [IDOT D4 Standard 604301-D4] for each manhole at the locations and elevations described in the contract plans.

General. This work shall be performed as detailed on the plans and according to Section 602 of the Standard Specifications. The Contractor shall submit shop drawings per Articles 1042.03(b) and 105.04 of the Standard Specifications of the proposed frame and grate to the Resident Engineer for approval.

Basis of Payment. This work will be paid for at the contract unit price per Each for INLETS, SPECIAL, NO. 1.

INLETS, SPECIAL, NO. 2

Description. This work shall consist of constructing Precast Manhole Type A, 6' (1.83m) Diameter [IDOT Standard 602406-11] with one Type 37 Grate [IDOT D4 Standard 604301-D4] for each manhole at the locations and elevations described in the contract plans.

General. This work shall be performed as detailed on the plans and according to Section 602 of the Standard Specifications. The Contractor shall submit shop drawings per Articles 1042.03(b) and 105.04 of the proposed frame and grate to the Resident Engineer for approval.

Basis of Payment. This work will be paid for at the contract unit price per Each for INLETS, SPECIAL, NO. 2.

INLETS, SPECIAL, NO. 3

Description. This work shall consist of constructing Inlets, Type B [IDOT Standard 602306-03] with one Type 37 Grate [IDOT D4 Standard 604301-D4] for each inlet at the locations and elevations described in the contract plans.

General. This work shall be performed as detailed on the plans and according to Section 602 of the Standard Specifications. Eccentric elevation shall be utilized by contractor per details defined on Structural Plans. The Contractor shall submit shop drawings per Articles 1042.03(b) and 105.04 of the Standard Specifications of the proposed frame and grate to the Resident Engineer for approval.

Trench backfills of the drainage inlets adjacent to proposed retaining walls shall be completed with Controlled Low-Strength Material (CLSM), as defined in Section 593 of the Standard Specifications. Installation of the CLSM shall be completed in accordance with Section 522 of the Standard Specifications and Structural Plans.

Basis of Payment. This work will be paid for at the contract unit price per Each for INLETS, SPECIAL, NO. 3.

The cost of Controlled Low-Strength Material (CLSM) will not be paid for separately but shall be included in the contract unit price for INLETS, SPECIAL, NO. 3.

PROPOSED MANHOLE/CATCH BASIN CONNECTION OVER EXISTING STORM SEWER

Description. This work shall consist of installing a manhole/catch basin over an existing sewer at the locations as shown in the plans.

General. Contractor shall carefully remove existing manhole / catch basin / inlet / storm sewer and install new structure at existing storm sewer. The protrusion of the existing storm sewer into the manhole shall not exceed one-inch. After the storm structure is installed, the manhole shall be mortared with a non-shrink concrete grout.

This work shall be performed as detailed on the plans and according to Section 602 of the Standard Specifications.

Basis of Payment. This work will be paid for at the contract unit price per Each for PROPOSED MANHOLE/CATCH BASIN CONNECTION OVER EXISTING STORM SEWER.

UNDERDRAIN CONNECTION TO STRUCTURE

Description. This work shall consist of connecting new underdrains to Inlets/Manholes at the locations as shown in the plans.

General. Contractor shall carefully core drill inlet/manhole at the diameter of the proposed underdrain. The protrusion of the underdrain shall not exceed one-inch. After the underdrain is installed, the Inlets/Manholes connection shall be made watertight by mortaring with a non-shrink concrete grout. The downstream pipes in manholes shall be screened to prevent entry of mortar or other debris from entering the system. Any mortar or other debris that has entered the existing manhole as a result of the work shall be removed.

This work shall be performed as detailed on the plans and according to Sections 601 and 602 of the Standard Specifications.

Basis of Payment. This work will be paid for at the contract unit price per Each for UNDERDRAIN CONNECTION TO STRUCTURE.

REMOVAL OF EXISTING STRUCTURES

Description. This work shall consist of removal of various bridge structures in accordance with Section 501 of the Standard Specifications for Road and Bridge Construction except as herein modified, as shown on the Plans, all project permits and as directed by the Engineer. The structures to be removed are described as follows:

REMOVAL OF EXISTING STRUCTURE NO. 1. This structure is defined as existing Structure No. 090-0034 carrying Illinois Route 8/116 traffic over the Tazewell and Peoria Railroad. The structure is approximately 705'-10" long and varies in width from 23'-0" to 88'-0" wide. The overall structure consists of a 4-span steel 84" web welded plate girder superstructure unit, a two-span steel 33WF superstructure unit along Ramp A & B, a three-span steel 27WF superstructure unit along Ramp C, and a two-span steel 30WF superstructure unit along Ramp D. The superstructure is supported by pile bent abutments founded on concrete piles and concrete column piers founded on concrete piles.

REMOVAL OF EXISTING STRUCTURE NO. 2. This structure is defined as existing Structure No. 090-0035 carrying Illinois Route 8/116 traffic over Main Street. The structure is approximately 157'-11" long and 48'-0" wide. This overall structure is a three-span steel 36WF superstructure. The superstructure is supported by pile bent abutments founded on concrete piles and concrete column piers founded on concrete piles at the fixed pier and spread footing at the expansion pier.

Existing Plans. Available plans for the existing structures will be posted on the IDOT website for bidding for Structure No. 090-0034 and Structure No. 090-0035 or are available for examination at the IDOT District Four Office in Peoria, Illinois by prior appointment. The completeness of these plans is not guaranteed, and no responsibility is assumed by the Department for their accuracy. Information is furnished for whatever value may be derived by the Contractor and is to be used solely at the Contractor's risk.

Submittals. Demolition plans, procedures and timelines for each individual structure shall be prepared and sealed by an Illinois Licensed Structural Engineer and submitted to the Engineer for review and approval. Such plans, procedures and timelines shall also be subject to review and approval of the IDOT Bureau of Bridges and Structures and the Tazewell and Peoria Railroad. Structural plans and procedures for any proposed temporary structures used to facilitate access shall be prepared and sealed by an Illinois Licensed Structural Engineer. (Refer to the Special Provision "Temporary Facilities Installed by Contractor.") The Contractor is advised that the existing Structure No. 090-0034 contains members that are in a deteriorated condition with reduced load carrying capacity. It is the Contractor's responsibility to account for the condition of the existing structure when developing construction procedures for the complete removal or partial removal, or replacement of the structure.

Maintenance of Traffic. Existing Structure No. 090-0034 and Structure No. 090-0035 will be closed to traffic during removal of the existing structure.

Rail traffic on the Tazewell and Peoria Railroad shall be maintained and protected during removal of the existing structure.

Method of Measurement. The removal of structures will be paid as Each, for Each location specified to the limits shown on the plans and as described herein.

Basis of Payment. This work will be paid for at the contract unit price per Each for REMOVAL OF EXISTING STRUCTURES, of the number indicated which payment shall constitute full compensation for all labor, materials, tools and equipment required for removal and disposal of existing structure and incidental items, as detailed in the plans, described herein and as directed by the Engineer.

FORM LINER LIMESTONE SURFACE

Description of Work. This work shall consist of the design and installation of form liners and forming concrete using form liners to achieve the concrete treatments as shown in the drawings and specifications. Form lined surfaces shall include areas of retaining walls as shown in plan details. Work shall comply with Section 503 of the Standard Specifications and as specified herein.

Fabricator Requirements. The following form liner manufacturers have been pre-approved to provide the listed pattern for the limestone surface form liner. Only the following manufacturers and patterns will be permitted to supply the limestone pattern form liners:

- a) Custom Rock International, St. Paul, MN, (1-800-637-2447)
Pattern No. 1208 – Drystack Form Liner
- b) Increte Systems, Odessa, FL, (1-800-752-4626)
Pattern No. WDRY-F001-000 - Dry Stack Form Liner
- c) Spec Form Liners, Inc., Santa Ana, CA, (1-888-429-9550)
Pattern No. 1508 – Large Dry Stack Fieldstone Form Liner

Shop Drawings. Shop drawings of the concrete facing patterns shall be submitted for each area of textured concrete. Shop drawing submittals shall include:

1. Individual form liner pattern descriptions, dimensions, and sequencing of form liner sections. Include details showing typical cross sections, joints, corners, step footings, stone relief, stone size, pitch/working line, mortar joint and bed depths, joint locations, end and edge treatments, and any other special conditions.
2. Elevation views of the form liner panel layouts for the limestone textures showing the full length and height of the structures with each form liner panel outlined. The arrangement of the form liner panels shall provide a continuous pattern without visual disruption.
3. The shop drawings shall depict enough different individual form sections, proposed orientation, and arrangement such that the proposed limestone wall's appearance will be free from a repetition of a distinguishable pattern.

To minimize the possibility of preparing an unsatisfactory test sample as described herein, the Contractor may elect to provide shop drawings for the test sample.

Materials. Form liners shall be of high quality, highly reusable and capable of withstanding anticipated concrete pour pressures without causing leakage or causing physical defects. Form liners shall attach easily to pour-in-place forms and be removable without causing concrete surface damage or weakness in the substrate. Liners used for the limestone textures shall be made from high-strength elastomeric urethane material which shall not compress more than 1/4" when poured at a rate of ten (10) vertical feet-per-hour. Form release agents shall be non-staining, non-residual, non-reactive and shall not contribute to the degradation of the form liner material.

Forms for smooth faced surfaces shall be plastic coated or metal to provide a smooth surface free of any impression or pattern. If the Contractor elects to use form ties for concrete forming, only fiberglass form ties will be permitted. Use of removable metallic form ties will not be allowed.

Cast Concrete Test Sample. The Contractor shall provide a cast concrete test sample containing the limestone patterns and smooth form liner surface. A 2 ft. x 2 ft. (600 x 600 mm) test sample that includes the proposed textured surface shall be cast and supplied to the Engineer for his/her approval 30 days prior to pouring the cast-in-place concrete.

Upon receipt of comments from inspection of the test sample, adjustments or corrections shall be made to the molds where imperfections are found. If required, additional test samples shall be prepared when the initial test sample is found to be unsatisfactory.

Installation. Form liners shall be installed in accordance with the manufacturers' recommendations to achieve the highest quality concrete appearance possible. Form liners shall withstand concrete placement pressures without leakage causing physical or visual defects. A form release agent shall be applied to all surfaces of the liner which will encounter concrete as per the manufacturer's recommendations. After each use, liners shall be cleaned and made free of build-up prior to the next placement, and visually inspected for blemishes or tears. If necessary, the form liners shall be repaired in accordance with the manufacturer's recommendations. All form liner panels that will not perform as intended or are no longer repairable shall be replaced. An on-site inventory of each panel type shall be established based on the approved form liner shop drawings and anticipated useful life for each form liner type.

The liner shall be securely attached to the forms according to the manufacturer's recommendations. Liners shall be attached to each other with flush seams and seams filled as necessary to eliminate visible evidence in cast concrete. Liner butt joints shall be blended into the pattern to create no visible vertical or horizontal seams or conspicuous form butt joint marks. Liner joints must fall within pattern joints or reveals. Finished textures shall be continuous without visual disruption and properly aligned over adjacent and multiple liner panels. Continuous or single liner panels shall be used where liner joints may interrupt the intended pattern. Panel remnants shall not be pieced together.

The Contractor shall coordinate concrete pours to prevent visible differences between individual pours or batches. Concrete pours shall be continuous between construction or expansion joints. Cold joints shall not occur within continuous form liner pattern fields. Wall ties shall be coordinated with the liner and form to achieve the least visible result. Liners shall be stripped between 12 and 24 hours as recommended by the manufacturer. Curing methods shall be compatible with the desired aesthetic result. Use of curing compounds will not be allowed. Concrete slump requirements shall meet the form liner manufacturers' recommendations for optimizing the concrete finish.

The Contractor shall employ proper consolidation methods to ensure the highest quality finish. Internal vibration shall be achieved with a vibrator of appropriate size, the highest frequency and low to moderate amplitude.

Concrete placement shall be in lifts not to exceed 1'-6". Internal vibrator operation shall be at appropriate intervals and depths and withdrawn slowly enough to assure the minimum amount of surface air voids and the best possible finish without causing segregation. External form vibrators may be required to assure the proper results. Any use of external form vibrators must be approved by the form liner manufacturer. It is the intention of this specification that no rubbing of flat areas or other repairs shall be required after form removal. The finished exposed formed concrete surfaces shall be free of visible vertical seams, horizontal seams, and butt joint marks. Grinding and chipping of finished formed surfaces shall be avoided.

Guidelines for Use of Form Liners to Achieve Desired Architectural Results. Form liners are being used on this project to achieve very specific architectural results. The Contractor shall not deviate from the guidelines contained herein unless authorized by the Engineer in writing.

Limestone rustications shall be placed on retaining walls as per the details shown in the plans. The proposed limestone wall's appearance shall be free from a repetition of a distinguishable pattern. Smooth faced border strips and crash surfaces shall be formed using smooth faced form liner material and placed as per the details shown in the plans.

Method of Measurement. This work shall be measured and paid for in place and the area computed in square foot (meters) of actual concrete surface area formed with concrete form liners.

The pay area for limestone surface shall be the actual cast area of limestone surface plus the actual area of any adjacent smooth form lined horizontal cap faces measured in square foot (meters).

Add "No deductions will be made for the volume of concrete displaced by limestone form lined surfaces." after the last sentence of the first paragraph of Article 503.21 (b) of the Standard Specifications.

Cast concrete form liner test samples will not be measured separately. Required adjustments or corrections needed to address test sample comments and the cost of additional test samples, if required, will not be paid for separately, but shall be included in the contract unit price Square Foot for the pay item FORM LINER LIMESTONE SURFACE.

Basis of Payment. Wall form liner will be paid for at the contract unit price per Square Foot for FORM LINER LIMESTONE SURFACE. Form Liner design, construction, and required adjustments or corrections shall be at no additional cost to the Owner.

ANTI-GRAFFITI COATING

Description: This work shall consist of the furnishing and application of an anti-graffiti coating to exposed concrete surfaces designated on the plans.

General: Product features shall include: Zero VOC, 10-year unlimited warranty for graffiti removals, binary prime coat, non-yellowing, non-chalking and breathable. The anti-graffiti coating shall consist of a permanent, color stable, UV, stain, chemical and abrasion resistant coating. The removal of graffiti from the protected surfaces shall be accomplished by applying a separate removal agent as recommended by the manufacturer of the permanent coating. The removal agent shall have the capability of completely removing all types of paints and stains. After graffiti removal there shall be no damage to the anti-graffiti coating or the surface to which it is applied. Additionally, there shall be no evidence of ghosting, shadowing, or staining of the protected surface.

Qualifications: The anti-graffiti coating shall be a product that has been commercially available for a period of at least five (5) years. Contractor shall apply the material to a test patch following the manufacturer's recommendation. After the manufacturer's recommended curing period, the Engineer will apply various types of graffiti materials to the coating. After three (3) days the removal agent shall be used to remove the graffiti. If after graffiti removal the anti-graffiti coating is clean and undamaged, with no evidence of ghosting, shadowing or staining, then the anti-graffiti coating is approved for use.

Preparation: Prior to application of the anti-graffiti coating, all designated surfaces shall be cleaned of loose debris, previous coatings (except staining) and all foreign matter by a method as recommended by the coating manufacturer and approved by the Engineer. All surfaces shall be thoroughly clean, dry and free of dust that might prevent penetration of the coating. New concrete should be thoroughly cured before application of the coating. Glossy, glazed and slick troweled surfaces of unstained concrete should be lightly etched or abraded before application of the coating. Concrete surfaces shall be properly sealed according to the manufacturer's recommendations, so the application of the system does not produce any noticeable long-term change in color of the surfaces being treated. A technical representative of the manufacturer shall be present to approve surface preparation and application of the anti-graffiti coating.

Weather Conditions: Coatings shall not be applied in the rain, snow, fog or mist, nor shall they be applied if these conditions are expected within twelve (12) hours of application. Coatings shall not be applied when the surface or air temperatures are less than 40° F nor greater than 100° F or is expected to exceed these temperatures within twelve (12) hours of application.

Application: The manufacturer's product data sheets and application guides shall be submitted to the Engineer prior to coating application. All information contained in the data sheets and application guides shall be strictly followed. All coatings shall be applied in the presence of the Engineer. Film thickness shall be measured by the Contractor in the presence of the Engineer and shall be according to the manufacturer's recommendation. Application of the clear protective coating shall take place after the application and curing of the concrete sealer as appropriate for the surface to be treated (see the standard specifications for CONCRETE SEALER). In a contrasting color of the same anti-graffiti system, the name of the system used, and the date of application shall be stenciled in letters not to exceed 2 inches high. The location of the stencil shall be near one end of the work at the bottom of the surface to be protected. For projects greater than 3,000 Square Feet the stencil shall be periodically repeated once for every 3,000 Square Feet near the bottom at the locations designated by the Engineer.

Cleaning Agent: The Contractor shall supply the Engineer with an initial quantity of the removal agent and written instructions for its use, as recommended by the manufacturer for graffiti removal. The amount shall be furnished at a rate of one (1) Gallon per 81 Square Yard of treated surface.

Measurement: This work will be measured in place per Square Foot of surface area upon which the anti-graffiti coating has been applied and accepted by the Engineer. No surface area will be measured for payment for areas below final grade. Applying an anti-graffiti coating to mock-up will not be measured for payment.

Basis of Payment: This Work will be paid for at the contract unit price per Square Foot for ANTI-GRAFFITI COATING which shall be payment in full for the cleaning of designated surfaces, the application of the anti-graffiti coating, supplying the manufacturer's technical representative and supplying the initial quantity of cleaning agent.

CHAIN LINK FENCE ATTACHED TO STRUCTURE

Description: This work shall consist of all labor, materials and equipment necessary for the mounting of a chain link fence on retaining walls, in accordance with the details and locations shown on the plans and the requirements of Section 664 of the Standard Specifications.

Construction Requirements: All posts shall be vertical when erected; the base plate must be welded to the post at the proper angle to account for any slope along the top of the wall.

The fence fabric shall be Type I, Class D and shall be in accordance with Article 1006.27 of the Standard Specifications.

The steel base plate and fence posts shall be galvanized in accordance with the requirements of AASHTO M111. The steel base plate shall meet the requirements of AASHTO M183.

Method of Measurement: Measurement shall be made along the top of the fence center-to-center of the end posts, in Meters, completed in place.

Basis of Payment: The work under this item will be paid at the contract unit price per Foot for CHAIN LINK FENCE, (of the height specified) ATTACHED TO STRUCTURE, measured in place, which price shall be payment for all work necessary to complete the work as herein specified, as shown on the plans or as directed by the Engineer.

ERECTION OF COMPLEX STEEL STRUCTURES

Effective: April 11, 2007

Description: In addition to the requirements of Article 505.08(e), the following shall apply.

The Contractor or sub-Contractor performing the erection of the structural steel is herein referred to as the Erection Contractor.

Erector Qualifications: The Erection Contractor shall be certified as an Advanced Certified Steel Erector (ACSE), by the AISC Certification Program. The Erection Contractor shall submit evidence of current ACSE certification to the Engineer with the submittal of the proposed erection plan.

Erection Plan: The Erection Contractor shall retain the services of an engineering firm, prequalified with the Illinois Department of Transportation in the Complex Structures category, for the completion of a project-specific erection plan. An Illinois Licensed Structural Engineer employed by this pre-qualified engineering firm, herein referred to as the Erection Engineer, shall sign and seal the erection plan, drawings, and calculations for the proposed erection of the structural steel.

The erection plan shall be complete in detail for all phases, stages, and conditions anticipated during erection. The erection plan shall include structural calculations and supporting documentation necessary to completely describe and document the means, methods, temporary support positions, and loads necessary to safely erect the structural steel in conformance with the contract documents and as outlined herein. The erection plans shall address and account for all items pertinent to the steel erection including such items as sequencing, falsework, temporary shoring and/or bracing, girder stability, crane positioning and movement, means of access, pick points, girder shape, permissible deformations and roll, interim/final plumbness, cross frame/diaphragm placement and connections, bolting and anchor bolt installation sequences and procedures, and blocking and anchoring of bearings. The Erection Contractor shall be responsible for the stability of the partially erected steel structure during all phases of the steel erection.

The erection plans and procedures shall be submitted to the Engineer for review and acceptance prior to starting the work. Review, acceptance and/or comments by the department shall not be construed to guarantee the safety or final acceptability of the work or compliance with all applicable specifications, codes, or contract requirements, and shall neither relieve the Contractor of the responsibility and liability to comply with these requirements, nor create liability for the Department. Significant changes to the erection plan in the field must be approved by the Erection Engineer and accepted by the Engineer for the Department.

Basis of Payment: This work shall not be paid for separately but shall be included in the applicable pay items according to Article 505.13 of the Standard Specifications.

DRAINAGE SCUPPERS

Description: Drainage scuppers of the type and size specified shall be furnished and installed as shown on the plans.

Method of Measurement: Drainage scuppers shall be measured for payment based on Each unit furnished and installed. Each unit includes the grate, frame, downspout, anchor studs, bolts, washers, nuts, and accessories required by the drainage scupper specified in the contract plans.

Basis of Payment: This work will be paid for at the contract unit price per Each for DRAINAGE SCUPPERS, DS-11.

TEMPORARY FACILITIES INSTALLED BY CONTRACTOR

Description of Work. This work shall consist of the design, installation, and removal of temporary facilities required and installed by the contractor to enable access to the work area.

General Construction Requirements. All means and methods employed for the installation and any subsequent removal of the temporary facilities to be installed by the Contractor for access or for any other reason shall be in compliance with all project permits.

Haul roads and crane mats constitute part of the temporary facilities installed and subsequently removed by the contractor. Haul roads shall be constructed with materials (i.e., coarse aggregate) meeting the requirements of Article 1004.04 of the Standard Specification, except if pit run gravel is used, prior approval of the source may be required by the Engineer. Upon completion of the work, the haul road shall be removed, and the railyard returned to its original contours, or the contours called for in the plans.

Submittals. Submittals for temporary facilities shall be prepared in accordance with the requirements set forth in the TZPR Public Project Manual and submitted to the specified parties for review and approval.

Basis of Payment. Temporary facilities installed by the Contractor and their subsequent removal will not be measured nor paid for separately but shall be considered as included in the unit cost of the various related pay items in the contract.

BAR SPLICERS

Add the following to Article 508.08(b):

When bar splicers are epoxy-coated, all damaged or uncoated areas near the threaded ends shall be coated with a two-part epoxy according to ASTM D 3963 (D 3963M). All threaded ends of Stage II construction threaded splicer bars shall be coated according to ASTM D 3963 or dipped in an epoxy-mastic primer prior to joining the Stage II construction threaded splicer bar to the threaded coupler.

Add the following to Article 1006.10(a)(1)g:

For bar splicers with welded connections between the threaded coupler and threaded rod, the Stage I construction threaded splicer bar shall be welded to the threaded coupler using an all-around fillet weld.

REMOVE AND RELOCATE SIGN PANEL

This work shall consist of removing and relocating the existing pole and street sign shown in the plans in accordance with applicable portions of Section 720 of the Standard Specifications and as directed by the Engineer.

Basis of Payment: This work shall be paid for at the contract unit price per Each for a REMOVE AND RELOCATE SIGN PANEL.

DELINEATOR REMOVAL

This work shall consist of the removal and proper disposal of existing delineator and hardware/appurtenances at the locations shown in the plans and the resultant void be filled and tamped.

This work shall be performed as detailed on the plans and according to Section 635 of the Standard Specifications. This work will be paid for at the contract unit price per Each for DELINEATOR REMOVAL.

DRAINAGE STRUCTURE ADJUSTMENT (SPECIAL)

Description: This work shall be performed as directed by the Engineer in conformance with applicable provisions of Sections 353, 406, 602, 603, and 1006 of the "Standard Specifications for Road and Bridge Construction".

This work shall be completed in accordance with the detail in the Plans, and include the following:

- Proposed drainage structures on Main Street which are installed in stages prior to placing final pavement, as indicated on the plans, shall be installed with a temporary steel plate to allow for maintenance of traffic during staged construction.
- The top of the new structure will be set such that a steel plate, plus the depth of temporary pavement, or patching, will retain a flush pavement surface across existing pavement to be used for temporary traffic control.
- The method setting top of structure could include casting the box with an adjusted top, or temporarily using a flat slab top instead of cone section, or other suitable methods as approved by the ENGINEER.

- At the locations specified on the plans, or as directed by the Engineer, the Contractor shall place and install a 3-foot square steel plate over the structure opening instead of the final casting and grate. The steel plate shall be a minimum of on (1) inch in thickness and a minimum ASTM-A-36 grade steel.
- Temporary pavement or patching over the steel plate shall utilize hot-mix asphalt.
- The removal of the steel plate shall be included in this work. The plates shall remain the property of the Contractor.

Method of Measurement: This work will be measured for payment per Each.

Basis of Payment: Payment for this work shall be at the contract unit price per Each for DRAINAGE STRUCTURE ADJUSTMENT (SPECIAL), which will include all labor, material, and equipment necessary to complete this item of work. Furnishing and installing of the new frame and lids shall be paid for separately.

MAINHOLE FRAMES TO BE ADJUSTED

Description. This work shall consist of adjusting manhole frame elevations for each communication or sanitary manhole at the locations and elevations described in the contract plans.

General. This work shall be performed as detailed on the plans and according to Sections 602 and 603 of the Standard Specifications.

The frame and lid shall be set in a full bituminous mastic bed or approved rubber gasket seal. The frame and lid shall be set accurately to the finished elevation so that no subsequent adjustment will be necessary.

Adjusting rings shall be in accordance with Section 1042, Article 1043.02 or Article 1043.03 of the Standard Specifications. The minimum thickness for concrete adjusting rings shall be 2".

Basis of Payment. This work will be paid for at the contract unit price per Each for MANHOLE FRAMES TO BE ADJUSTED.

VALVE BOX FRAMES TO BE ADJUSTED

Description. This work shall consist of adjusting valve box frame elevations for each gas or water valve at the locations and elevations described in the contract plans.

General. This work shall be performed as detailed on the plans and according to Sections 602 and 603 of the Standard Specifications.

The frame and lid shall be set accurately to the finished elevation so that no subsequent adjustment will be necessary.

Basis of Payment. This work will be paid for at the contract unit price per Each for VALVE BOX FRAMES TO BE ADJUSTED.

COOPERATION WITH UTILITIES

Add to Article 105.07.

The City of East Peoria plans to abandon water main from approximately Station 99+10 to 120+50 along US 24/IL 29. Fire hydrants along Johnson and Pekin would be relocated. It will be necessary for the Contractor to accommodate up to 14 consecutive calendar days within their schedule for this work to be completed by others. These tasks shall be completed prior to the completion of temporary pavement and permanent subgrade improvements. No additional compensation shall be made to the Contractor for this scheduling requirement. The Contractor shall contact the City of East Peoria at telephone number (309) 427-7653 (Mr. Cord Crisler) to coordinate the scheduling of removal and relocation work of the water main.

SANITARY SEWER 10"

The work in this special provision includes furnishing all labor, materials, and equipment required to install sanitary sewer pipe in accordance with the details in the plans, notes in the plans, and this special provision.

The term Engineer as referenced within this specific special provision is defined as the Illinois Department of Transportation's Resident Engineer and City of East Peoria's Water and Wastewater Supervisor.

Items in this work include, but are not limited to:

- All excavation and embankment,
- Pipe cradle,
- Initial backfill,
- Backfill (refer to detail(s) in the plans),
- Removal of sufficient existing sanitary sewer pipe in direct conflict with proposed sanitary sewer pipe,
- Sealing of abandoned sanitary sewer lines at locations other than at manholes,
- Direct bury sanitary sewer PVC pipe shall be AWWA C900 ASTM D2241 DR14/PS815 with ASTM D3139 integral bell joints and ASTM F477 gaskets,
- Shoring to be designed and sealed by an Illinois Licensed Structural Engineer,
- Connection to proposed and existing structures,
- All ancillary materials required to construct fully functional sanitary sewer pipe, and
- Inspection and testing of sewers (refer to Inspection and Testing of Sewers Special Provision).

Prior to ordering sanitary sewer pipe, Contractor shall submit to Engineer written specifications from pipe manufacturer the pipe to be installed, including joints and gaskets, are rated/approved for burial depths up to 35 feet.

Pipe shall be homogenous throughout and free from cracks, holes, foreign inclusions or other injurious defects. Pipe shall be uniform as practicable in: color, opacity, density and any other physical property. Routine inspection, sampling and testing shall be performed during pipe and fitting production to assure a product quality in accordance with applicable standards. Materials certifications for the specified casing, fittings and pipe appurtenances shall be submitted to the Engineer for approval prior to installation.

Pipe will be legibly marked in with its required certification or standard. Pipe not bearing the required certification or standard may be rejected.

Upon delivery to the site(s) of work, both the outside and inside surfaces of all pipes shall be inspected for damage such as, but not limited to, cuts, scrapes, gouges, tears, cracks, punctures. If any damages are found, the Engineer shall be the sole judge of the damages and the acceptability of the pipe. If rejected, the Contractor shall be responsible for removing the defective pipe from the site(s) of work and replacing it with new.

Prior to and during construction, pipe shall not be dragged, pushed or rolled over the ground surface. Pipe shall be moved using other means in accordance with the recommendations of the manufacturer of the pipe and the Engineer.

All aspects of pipe installation using excavation methods, including, but not limited to, joint construction, bedding, pipe material, concrete thrust block design and construction, backfilling, trench construction, maximum loading imposed on pipe in the trench, and material testing shall also conform to the Manufacturer's Specification for the particular type of pipe specified; however, when in conflict, these Specifications shall govern.

Connections between pipes of dissimilar materials or of unequal outside diameters shall be made using Fernco-brand flexible-type couplings. The flexible type coupling shall fit over plain ends of both pipes and then be tightened to make a water tight seal. Couplings for pipes under twelve-inches in diameter shall be 5000 Series, Strong Back (RC) shielded flexible-type couplings as manufactured by Fernco.

The Contractor shall make all excavations to the width and depth necessary for proper construction of the sewers and other structures in accordance with the Plans.

The bottom of the trench shall be smooth and cleared of stones or protruding hard objects.

Trench widths shall be sufficiently wide to permit tamping around the pipe. The following specifications for trench dimensions shall apply to all conditions except where pipe construction is within strata of rock. Rock shall be defined as that material occurring in a natural state which requires blasting, barring, or wedging for removal from its original bed and having an unconfined compressive strength in excess of 1,800 pounds per square inch. It also includes boulders larger than 46 cubic feet in volume.

Trench widths measured at the top elevation of the pipe and extending to the elevation of initial backfill shall be a minimum of 16" plus the outside diameter of the pipe and a maximum of 36" plus the outside diameter of the pipe. Pipes shall be laid in the center of the trench. Whenever the trench widths measured at the top of the pipe exceed these limitations, the Contractor shall at his own expense remove any disturbed earth and shall refill the excavated trench from wall to wall with approved granular bedding, concrete cradle, concrete encasement or a combination thereof as directed by the Engineer.

Bedding dimensions and compaction requirements shall be per the details in the plans and shall apply to all conditions except where pipe construction is within strata of rock. Bedding shall be constructed and maintained to evenly support loading throughout pipe construction to avoid non-uniform loading at any point.

If the ground conditions are not suitable for bedding as outlined, the Contractor must excavate and dispose of the unsuitable material and add approved granular bedding material to support the pipe, as determined by Engineer. The removal of unsuitable material and furnishing and installing of approved granular bedding will be considered extra work and paid on a force account basis.

Granular pipe cradle and envelope shall be constructed with granular materials from approved local deposits graded to Illinois Department of Transportation Standard Specifications for Road and Bridge Construction, latest edition. Acceptable graduations for the granular pipe cradle and envelope are CA-7 and CA-11. The material shall be crushed gravel or crushed stone as per IDOT's Coarse Aggregate Standards with a minimum of 75% fractured material, from IDOT-approved sources. When constructing PVC pipe using excavation methods at depths greater than sixteen (16) feet, granular materials used for bedding purposes shall be CA-7 or CA-11 white rock crushed stone.

Pipe shall be constructed in conformity with the alignment and elevations. All pipe shall be constructed using a pipe laser and with the bell located at the upstream end. Pipe shall be installed beginning at the downstream end and proceeding upstream unless written authorization is obtained from the Engineer.

There are a number of lasers used in construction; hence, the method used to set up the laser prior to laying the sewer shall be approved. However, an above-ground spinning laser is unacceptable as the only laser used to check the grade of the sewer. Beginning at the first manhole, the laser will be leveled and set on line and grade. As the sewer construction reaches the next manhole, the laser will be moved to that new manhole, leveled, and line and grade reset for the next reach of sewer with the percent of grade given on the Plans. The laser will be checked for level, line and grade each morning and noon or at such other times as the construction is resumed after any delay in the work or at such times as in the opinion of the Engineer the line and grade is in question as to its accuracy and conformance with the Plans. Upstream or downstream of each manhole, using a method independent of the laser and approved by the Engineer, the elevation of pipe will be checked at the end of the first pipe laid, twenty-five (25) feet from the manhole and at seventy-five (75) -foot intervals thereafter.

The Contractor shall have a ventilation system on site, ready and available for use by the construction crew. The system shall be of adequate size to ventilate to prevent laser drift.

Before adjoining pipes are connected, the interior of the pipe already constructed, including the bell thereof, shall be thoroughly cleared of all solids. A watertight plug shall be placed in the last pipe placed and not removed except to connect another pipe which in turn must be plugged. Pipes shall not be constructed where water has not been removed from the trench. Foreign materials inside constructed pipes shall be removed by the Contractor to the satisfaction of the Engineer.

The Contractor shall at all times during construction provide and maintain ample means and devices with which to promptly remove and properly dispose of all water entering excavations or other parts of the work and shall keep said excavations dry until the structures to be built therein are complete. No water or unauthorized sewage shall be drained into work built or under construction.

When a Contractor is directed to connect an outlet sewer to an existing sewer, he shall immediately provide a temporary bulkhead at the closest downstream manhole. Connections to existing sewers shall be performed using methods detailed in these Specifications. Appropriate management of flows shall be maintained by the Contractor.

For direct buried applications, all joints between non-restrained and restrained joint pipe, ductile iron fittings, or pressure pipe connections of dissimilar materials shall be by mechanical joint. Mechanical joints shall be constructed on plain-end pipe using either Series 2000PV mechanical joint restraint for PVC pipe as manufactured by EBAA Iron, Inc., or PVC Stargrip, Series 4000, mechanical joint wedge action restraint as manufactured by Star Pipe Products. In no case shall a mechanical joint adapter be used that is not specifically designed for the pipe material for which it is to be attached. When joint adapters are utilized with HDPE pipe, the pipe shall be fitted with a correctly sized stainless steel pipe stiffener. Pipe stiffeners shall be 230 /231 Stainless Steel HDPE Stiffeners as manufactured by JCM Industries or approved equivalent.

For non-direct buried applications (i.e., above grade piping or piping contained in an enclosed vault or building), all joints between non-restrained and restrained joint pipe, ductile iron fittings, or pressure pipe connections of dissimilar materials shall be by flanged joint unless the design pressure exceeds 125 psi. At locations where the design pressure exceeds 125 psi mechanical joints shall be used. Flanged joints shall be constructed by weld on slip joint flanges for steel pipe, integral cast flanges, or thread on flanges for ductile iron pipe, or flange adapter for all non-metallic pipe types. Where a retrofit connection to existing steel or ductile iron pipe is required, the connection to the existing pipe shall be by flange adapter. Flange adapters shall be Super Flange Series 7200 Adapters as manufactured by Star Pipe Products. When joint adapters are utilized with HDPE pipe, the pipe shall be fitted with a correctly sized stainless steel pipe stiffener. Pipe stiffeners shall be 230 /231 Stainless Steel HDPE Stiffeners as manufactured by JCM Industries or approved equivalent.

Unless directed otherwise by the Engineer, joint locations shall be planned and located to minimize the total number of joints. Joints for pipe installed by jacking and tunneling or HDD shall be of the restrained type.

Trench shall not be backfilled until sanitary sewer pipe between upstream and downstream manhole structures has passed infiltration, exfiltration, or exfiltration of air under pressure, and deflection of pipe tests.

The ends of the sanitary sewer pipes to be abandoned in place shall be sealed per the existing detail in the plans.

Pipe is measured from inside face of manhole structure to inside face of manhole structure.
Trench backfill will be paid for separately.

This work will be paid for at the contract unit price per Foot for SANITARY SEWER of the type and diameter specified.

STEEL CASINGS 24"

The work in this special provision includes furnishing all labor, materials, and equipment required to install steel casing pipe in accordance with the details in the plans, notes in the plans, and this special provision.

The term Engineer as referenced within this specific special provision is defined as the Illinois Department of Transportation's Resident Engineer and City of East Peoria's Water and Wastewater Supervisor.

Items in this work include, but are not limited to:

- All excavation and embankment,
- Pipe cradle,
- Initial backfill,
- Backfill (refer to detail(s) in the plans),
- Steel casing pipe be A36 pipe with 3/4" minimum wall thickness,
- Welding,
- Shoring to be designed and sealed by an Illinois Licensed Structural Engineer,
- Casing spacers,
- Sealing ends of casing, and
- All ancillary materials required to construct fully functional steel casing pipe.

Steel casing shall be homogenous throughout and free from cracks, holes, foreign inclusions or other injurious defects. Pipe shall be uniform as practicable in: color, opacity, density and any other physical property. Materials certifications for the specified steel casing, fittings and pipe appurtenances shall be submitted to the Engineer for approval prior to installation. The steel casing certifications shall include, but shall not be limited to steel casing manufacturer, the date and location of its manufacture as well as the steel casing's specifications.

Steel casing will be legibly marked with its required certification or standard. Steel casing not bearing the required certification or standard may be rejected.

Upon delivery to the site(s) of work, both the outside and inside surfaces of all steel casing shall be inspected for damage such as, but not limited to, cuts, scrapes, gouges, tears, cracks, punctures. If any damages are found, the Engineer shall be the sole judge of the damages and the acceptability of the pipe. If rejected, the Contractor shall be responsible for removing the defective steel casing from the site(s) of work and replacing it with new.

Prior to and during construction, steel casing shall not be dragged, pushed or rolled over the ground surface. Steel casing shall be moved using other means in accordance with the recommendations of the manufacturer of the pipe and the Engineer.

All aspects of steel casing installation using excavation methods, including, but not limited to, joint construction, bedding, pipe material, concrete thrust block design and construction, backfilling, trench construction, maximum loading imposed on steel casing in the trench, and material testing shall also conform to the Manufacturer's Specification for the particular type of steel casing specified; however, when in conflict, these Specifications shall govern.

The Contractor shall make all excavations to the width and depth necessary for proper construction of the steel casing, sewer pipe, and other structures in accordance with the Plans.

The bottom of the trench shall be smooth and cleared of stones or protruding hard objects.

Trench widths shall be sufficiently wide to permit tamping around the pipe. The following specifications for trench dimensions shall apply to all conditions except where pipe construction is within strata of rock. Rock shall be defined as that material occurring in a natural state which requires blasting, barring, or wedging for removal from its original bed and having an unconfined compressive strength in excess of 1,800 pounds per square inch. It also includes boulders larger than 46 cubic feet in volume.

Trench widths measured at the top elevation of the pipe and extending to the elevation of initial backfill shall be a minimum of 16" plus the outside diameter of the steel casing and a maximum of 36" plus the outside diameter of the steel casing. Steel casing shall be laid in the center of the trench. Whenever the trench widths measured at the top of the pipe exceed these limitations, the Contractor shall at his own expense remove any disturbed earth and shall refill the excavated trench from wall to wall with approved granular bedding, concrete cradle, concrete encasement or a combination thereof as directed by the Engineer.

Bedding dimensions and compaction requirements shall be per the details in the plans and shall apply to all conditions except where steel casing construction is within strata of rock. Bedding shall be constructed and maintained to evenly support loading throughout steel casing construction to avoid non-uniform loading at any point.

If the ground conditions are not suitable for bedding as outlined, the Contractor must excavate and dispose of the unsuitable material and add approved granular bedding material to support the steel casing, as determined by Engineer. The removal of unsuitable material and furnishing and installing of approved granular bedding will be considered extra work and paid on a force account basis.

Granular steel casing cradle and envelope shall be constructed with granular materials from approved local deposits graded to Illinois Department of Transportation Standard Specifications for Road and Bridge Construction, latest edition. Acceptable graduations for the granular steel casing cradle and envelope are CA-7 and CA-11. The material shall be crushed gravel or crushed stone as per IDOT's Coarse Aggregate Standards with a minimum of 75% fractured material, from IDOT-approved sources. When constructing steel casing using excavation methods at depths greater than sixteen (16) feet, granular materials used for bedding purposes shall be CA-7 or CA-11 white rock crushed stone.

Steel casing shall be constructed in conformity with the alignment and elevations. All steel casing shall be constructed using a pipe laser.

There are a number of lasers used in construction; hence, the method used to set up the laser prior to laying the sewer shall be approved. However, an above-ground spinning laser is unacceptable as the only laser used to check the grade of the sewer. Beginning at the manhole, the laser will be leveled and set on line and grade. The laser will be checked for level, line and grade each morning and noon or at such other times as the construction is resumed after any delay in the work or at such times as in the opinion of the Engineer the line and grade is in question as to its accuracy and conformance with the Plans. Upstream or downstream of each manhole, using a method independent of the laser and approved by the Engineer, the elevation of the steel casing will be checked at intervals as directed by the Engineer.

The Contractor shall have a ventilation system on site, ready and available for use by the construction crew. The system shall be of adequate size to ventilate to prevent laser drift. Before adjoining steel casing sections are connected, the interior of the pipe already installed shall be thoroughly cleared of all solids. A watertight plug shall be placed in the last section of steel casing and not removed except to connect another section of steel casing which in turn must be plugged. Steel casing shall not be constructed where water has not been removed from the trench. Foreign materials inside constructed steel casing shall be removed by the Contractor to the satisfaction of the Engineer.

The Contractor shall at all times during construction provide and maintain ample means and devices with which to promptly remove and properly dispose of all water entering excavations or other parts of the work and shall keep said excavations dry until the structures to be built therein are complete. No water or unauthorized sewage shall be drained into work built or under construction.

Sections of steel pipe shall be welded uniformly throughout the circumference of each joint. Welds shall be free of leaks and defects.

For all welders who will assist towards the completion of the Project, the Contractor shall furnish a certificate issued by an approved testing laboratory that demonstrates the welders' abilities to make groove and fillet welds in all positions for the type and grade of steel specified. Welder's certification shall be submitted to the Engineer prior to installation.

The Contractor shall provide the Engineer with documentation showing that all participating welders have practiced welding continuously since certification. A proposed welder shall not participate in welding operations necessary towards the completion of the Project until a certification demonstrating compliance with these Specifications is submitted to and accepted by the Engineer.

After installation of the casing pipe and prior to the installation of the carrier sewer pipe, the Contractor shall inspect the casing pipe using a pipe laser and CCTV to verify the installed pipe meets the requirements of the Construction Documents. CCTV inspection shall be performed in the presence of the Engineer and video recorded and submitted to Engineer for review and acceptance.

Trench for installation of steel casing shall not be backfilled until carrier sewer pipe has been installed inside steel casing and connected to upstream and downstream manhole.

Trench for installation of steel casing shall not be backfilled until carrier sewer pipe between upstream and downstream manhole structures has passed infiltration, exfiltration, or exfiltration of air under pressure, and deflection of pipe tests.

Trench for installation of steel casing shall not be backfilled until the Contractor inspects the carrier pipe using CCTV and submits the video to the Engineer for review and acceptance.

Steel casing pipe will be measured for payment in-place in Feet.

Trench backfill will be paid for separately.

This work will be paid for at the contract unit price per Foot for STEEL CASINGS of the type, grade, and diameter specified.

CARRIER SANITARY SEWER, 10" DIAMETER

The work in this special provision includes furnishing all labor, materials, and equipment required to install sanitary sewer pipe in accordance with the details in the plans, notes in the plans, and this special provision.

The term Engineer as referenced within this specific special provision is defined as the Illinois Department of Transportation's Resident Engineer and City of East Peoria's Water and Wastewater Supervisor.

Items in this work include, but are not limited to:

- All excavation and embankment beyond what is required for steel casing,
- Pipe cradle beyond steel casing limits,
- Initial backfill beyond steel casing limits,
- Backfill beyond steel casing limits (refer to detail(s) in the plans),
- Carrier sewer PVC pipe shall be ASTM D2241 DR14/PS815 with ASTM D3139 integral bell joints and ASTM F477 gaskets (Certa-Lok).
- Shoring to be designed and sealed by an Illinois Licensed Structural Engineer,
- Connection to proposed and existing structures,
- All ancillary materials required to construct fully functional sanitary sewer pipe, and
- Inspection and testing of sewers (refer to Inspection and Testing of Sewers Special Provision).

Prior to ordering sanitary sewer pipe, Contractor shall submit to Engineer written specifications from pipe manufacturer the pipe to be installed, including joints and gaskets, are rated/approved for burial depths up to 35 feet.

Carrier Pipe shall be installed in accordance with sanitary sewer pipe special provision and per the following:

Carrier Pipe to be installed inside casing pipe shall be installed at depths and grade specified on the Project plan sheets or as specified by the Engineer. The carrier sewer pipe must be installed freely within the casing pipe without binding or resistance that requires excess force to overcome. The carrier pipe shall be installed without vertical or horizontal deflection.

Casing spacers shall be attached to the carrier sewer pipe so that the completed sewer will be at the required line and grade, centered within the casing pipe, and restrained against flotation. Spacers shall be selected from the following list:

- Model S stainless steel casing isolators by Pipeline Seal and Insulator (PSI), Inc.,
- Model BWM SS Stainless Steel Casing Spacers by the BWM Company, or
- CCS-JR Stainless Steel centered & restrained joints manufactured by Cascade Waterworks Manufacturing.

Widths and sizes for spacers and isolators shall be based upon both the manufacturer's recommendations and the specific application. Spacers and isolators shall be placed in accordance with the following guidelines:

1. Spacers or isolators shall be evenly spaced and placed at minimum intervals of 12-inches around the circumference of the pipe, with a minimum of four individual spacers or isolators at each location
2. Spacers shall be located no more than one (1) foot from both ends of the casing pipe.
3. Spacers or isolators shall be placed on both sides of a joint between two sections of carrier pipe. Said spacers or isolators shall be placed no more than one (1) foot from the joint.
4. Spacing shall meet manufacturer requirements but in no case shall be more than ten (10) feet along the length of the pipe. apart.

After the carrier sewer pipe has been installed and centered within the casing, unless specified otherwise by the Engineer, the ends of the casing pipe shall be brick and mortared per the details in the plans.

Carrier pipe trench shall not be backfilled until the Contractor inspects the carrier pipe using CCTV and submits the video to the Engineer for review & acceptance.

Carrier pipe trench shall not be backfilled until sewer pipe between upstream and downstream manhole structures has passed infiltration, exfiltration, or exfiltration of air under pressure, and deflection of pipe tests.

Pipe is measured from inside face of manhole structure to inside face of manhole structure.
Trench backfill will be paid for separately.

This work will be paid for at the contract unit price per Foot for CARRIER SANITARY SEWER of the type and diameter specified.

SANITARY MANHOLES (SPECIAL)

The work in this special provision includes furnishing all labor, materials, and equipment required to install precast flat top manholes in accordance with the details in the plans, notes in the plans, and this special provision.

The term Engineer as referenced within this specific special provision is defined as the Illinois Department of Transportation's Resident Engineer and City of East Peoria's Water and Wastewater Supervisor.

Items included in this work include, but are not limited to:

- All excavation and embankment,
- Shoring to be designed and sealed by an Illinois Licensed Structural Engineer,
- Backfill (refer to detail(s) in the plans),
- Connection to proposed and existing pipe,
- Manhole structure,
- Adjusting rings,
- Engineering design,
- All ancillary materials required to construct fully functional flat top manhole, and
- Testing and inspection for leakage in accordance with ASTM C1244-93 or ASTM C969-94.

A structural engineer, licensed in Illinois, shall certify the proper design of manhole structures at depths greater than twenty (20) feet using the manhole standard detail included in the plans as the basis of design.

Sanitary manholes shall be precast manholes with a minimum compressive strength as designated by the Illinois licensed structural engineer of record. Concrete compressive strength tests using cores taken from manhole products shall not be allowed; rather, concrete compressive strength tests shall be performed using compression test cylinders a set of which shall be constructed no less than daily per concrete mix. Furthermore, in contrast to that allowable within the text of applicable ASTM guidelines, in no case shall any cylinder tested fall below one hundred percent (100%) of the specified strength of the concrete. All test results shall be the property of the Engineer and provided by the Contractor immediately after

the completion of testing. If compression test results are less than allowable, the Engineer shall determine necessary measures to be taken including potential rejection of constructed precast, manhole components.

Manhole bottoms shall be pre-cast with cast-in fillets (Moorbase), except where saddle-type manholes are specified in the plans.

Illinois licensed structural engineer of record shall also approve the following manhole joint configurations or design a custom joint:

| Nominal Diameter | Inside Diameter | Wall Thickness | Joint Height | Joint Type |
|------------------|-----------------|----------------|--------------|-------------------------|
| 4" | 48" | 5" | 2.75" | One-step Forsheda Joint |
| 5" | 60" | 6" | 4.75" | Single Offset Joint |
| 6" | 72" | 7" | 6.00" | 3M Pipe Joint |

Additionally, the Illinois licensed structural engineer of record shall approve the following or recommend an alternate joint material between manhole structure components:

Joints between concrete manhole components shall be constructed using butyl rubber. Butyl rubber sealants shall be the CS-202 Butyl Rubber Sealant as manufactured by ConSeal Concrete Sealants, Inc. The ends of each butyl rubber sealant shall overlap to ensure closure. The exterior side of the manhole joints shall be sealed with CS-212 Polyolefin Backed Exterior Joint Wrap as manufactured by ConSeal Concrete Sealants, Inc.

Manhole connectors to seal the connection between pipes and the manholes shall be constructed using the following connector systems.

- Approved boot connectors shall include the following:
 - o G3 Boot System as manufactured by A-Lok Products, Inc.
 - o PSX Direct Drive system as manufactured by the Press-Seal Gasket Corporation
 - o PSX Positive Seal system as manufactured by the Press-Seal Gasket Corporation

If approved boot connectors are not rated for the installation depths, Contractor may propose alternate boot connectors for approval by Engineer. Submittal documents shall include boot connector manufacturer's written recommendations and specifications.

- Approved compression connectors shall include the following:
 - o Econoseal system as manufactured by the Press-Seal Gasket Corporation
 - o A-Lok Premium system as manufactured by A-Lok Products, Inc.
 - o X-Cel system as manufactured by A-Lok Products, Inc.

If approved compression connectors are not rated for the installation depths, Contractor may propose alternate compression connectors for approval by Engineer. Submittal documents shall include compression connector manufacturer's written recommendations and specifications.

The height of the barrel shall be suitable to fit the various depths of the manholes as shown on the plans. The top of manhole castings and lids shall be flush and consistent with the existing surface surrounding the manhole. A twelve-inch (12") or sixteen-inch (16") long barrel section shall be required immediately beneath a flat-top lid.

The maximum height of adjusting rings to be allowed for use under the manhole frame shall be eight (8") inches. Rubber adjusting rings shall be used for adjustments where the raise is less than or equal to three (3") inches; for all adjustments, at least two (2") inches of rubber adjusting rings shall be used immediately below the manhole frame. Manhole casting adjusting rings may be used for minor height adjustments not exceeding eight (8") inches; however, concrete adjusting rings of thickness two (2") inches or less shall not be allowed. If the surface surrounding the manhole is uneven, tapered rubber adjusting rings as provided by the manufacturer may be used. Adjusting rings shall be concentric to the manhole access opening with a deviation no greater than one eighth of an inch (1/8").

Rubber adjusting rings shall be either Infra-Riser Multi-Purpose Rubber Composite Adjustment Risers as manufactured by East Jordan Iron Works, Inc. or rubber adjusting rings as manufactured by American Highway Products, Ltd.

Steps in the manhole shall be equal to M.A. Industries, Inc. molded step PS-1-PF and shall be placed as specified on the applicable Detail Drawing as provided in Section 095 of these Specifications. Manhole steps shall be centered over the outlet pipe on eight (8") inch to and including twelve (12") inch diameter sewers. Steps shall be installed at ninety (90°) degrees to the outlet pipe on sewers larger than twelve (12") inches.

Manhole Safety Ramps as manufactured by American Highway Products, Ltd. May be used as directed.

Concrete anchoring systems shall be either Trubolt Wedge Anchors, Type 316 stainless steel, part of the Red Head Concrete Anchoring Systems manufactured by Illinois Tool Works (ITW), Inc. or Type 316 Stainless Steel threaded rod bedded by an adhesive anchoring system in boreholes. Where utilized boreholes shall be freshly drilled and cleaned with compressed air prior to installation of the adhesive anchoring system. If precast or previously drilled boreholes are to be utilized, they shall also be cleaned with a non-residue degreasing agent and allowed to completely dry prior to installation of the adhesive anchoring system. Boreholes shall have a diameter of not less than 1.25 times nor more than 1.5 times the diameter of the threaded rod. Boreholes shall have a minimum embedment depth of not less than three inches or five times the diameter of the threaded rod, whichever is greater.

The type of anchor used as well as the thread length and anchor diameter shall be as specified on the project plans. Where indicated on the project plans, concrete anchoring shall be coupled with an adhesive anchoring system. Adhesive anchoring systems shall be EPCON, two-part epoxy, Adhesive Anchoring Systems as manufactured by ITW, Inc.

Capsule anchors shall be of the size and length required as outlined on the Plans and Specifications and shown on the Shop Drawings unless otherwise stated. Anchors shall be Molly Parabolt type M24-1, Ramset Chemset CTR10. Installation shall be per manufacturer's recommendations.

Sanitary manhole shall not be backfilled until structure has passed infiltration, exfiltration, or exfiltration of air under pressure tests.

This work will be paid for at the contract unit price per Each for SANITARY MANHOLES (SPECIAL) of the size specified.

SANITARY MANHOLES, SADDLE-TYPE, 5' DIAMETER

The work in this special provision includes furnishing all labor, materials, and equipment required to install precast flat top saddle-type manholes in accordance with the details in the plans, notes in the plans, and this special provision.

The term Engineer as referenced within this specific special provision is defined as the Illinois Department of Transportation's Resident Engineer and City of East Peoria's Water and Wastewater Supervisor.

Items included in this work include, but are not limited to:

- All excavation and embankment,
- Shoring to be designed and sealed by an Illinois Licensed Structural Engineer,
- Solid concrete block,
- Coarse aggregate CA7 or CA11,
- Non-shrink grout,
- All Concrete
- Saw cut opening in existing pipe,
- Compacted bedding stone,
- Controlled low strength material,
- Concrete collar,
- Backfill (refer to detail(s) in the plans),
- Manhole structure,
- Manhole bench and trough reconstruction
- Adjusting rings,
- Engineering design,
- All ancillary materials required to construct fully functional saddle-type manhole, and
- Testing and inspection for leakage in accordance with ASTM C1244-93 or ASTM C969-94.

Sanitary saddle-type manholes shall be installed in accordance with the special provision for sanitary manholes and per the following:

A structural engineer, licensed in Illinois, shall certify the proper design of saddle-type manhole structures at depths greater than twenty (20) feet using the saddle-type manhole standard detail included in the plans as the basis of design.

Saddle-type manholes shall be constructed using precast concrete manhole components in accordance with these Specifications except that a saddle-type manhole shall be precast to conform to the dimensions and operation of the existing sewer over which it is to be constructed. Likewise, the existing sewer shall be field modified so that the constructed manhole is fully operational.

A saddle-type manhole shall be constructed to straddle an existing sewer by modifying an appropriately sized precast concrete manhole barrel section. The dimensions of the chosen precast concrete manhole barrel section shall be chosen based upon field conditions including the size of the existing sewer and consideration for the structural integrity of the constructed manhole. The opening cut into the barrel shall be large enough to allow placement over the existing sewer; however, excessive cutting beyond that necessary to place the modified barrel section over the sewer shall not be allowed.

The manhole-base section shall be properly constructed in accordance with these Specifications and per the Illinois licensed structural engineer of record for the design. Manhole-base sections shall be adequately supported upon concrete blocks and sewer bedding materials as designed by the Illinois licensed structural engineer of record. After properly supporting the manhole base, ready-mix, Portland cement Class SI concrete as detailed in the Illinois Department of Transportation (IDOT) Standard Specifications shall be poured around the manhole-base section. No further construction upon or around the constructed base section shall be allowed until the concrete has reached minimum compressive strength tests designated by the Illinois licensed structural engineer of record. The use of early age or high early accelerators is allowed.

Joints between the newly placed and modified manhole barrel section that will serve as the manhole bottom and the existing sewer shall be made using ready-mix concrete in accordance Illinois licensed structural engineer of record. The joints shall be watertight.

Manhole benches within saddle-type manholes shall be constructed in accordance with the directions for manhole bench and trough reconstruction provided in Section 075 of these Specifications.

The crown of existing sewer pipes shall be removed to the top of the newly cast bench. The pipe shall be cut true and even with the top of the constructed manhole bench without excessive pipe removed below or remaining above the top of the bench.

Bench and trough reconstruction shall be performed immediately after the removal of the existing bench and trough and before commencement of manhole rehabilitation. Water shall not be allowed to either collect in the bottom of the manhole or exit by seepage through the manhole bottom; at all times, the Contractor shall maintain a system to ensure that water is either removed from the manhole or allowed to flow into the exit piping of the manhole.

Concrete shall be placed with tubes so that the drop is not greater than 2' feet maximum.

Trough reconstruction shall be by the method called for in the plans. When a specific method is not called for in the plans then the reconstruction shall be by any of the approved methods:

Method 1 – Use of PVC Pipe and Fittings to Construct Flow-Through Trough with Flexible Coupler Connection Seals.

This reconstruction method consists of the following steps:

- Carefully chip out the manhole pipe connections in such a way that the connecting carrier pipes are not damaged.
- Create sufficient clearance around the connecting pipes to permit the installation of a flexible coupler to the existing connecting sewer pipes.

- Construct a flow through trough using approved piping materials - sizing and configuration of replacement piping shall be governed by the size of the sewers connecting to the respective manhole as well as the angles of sewer pipes connecting to the manhole at the trough.
- Pipe and fittings used to reconstruct troughs shall be the same as sanitary sewer pipe used for this project.
- Depending upon the configuration of the sewers connecting to the manhole at the trough, tees and wyes may be used to reconstruct a trough that will convey all sewage coming into a manhole out through the exit piping.
- Connect the constructed flow through trough piping to the existing connecting sewer piping utilizing flexible rubber couplers (i.e. Fernco Coupler).
- Shim and support the flow through trough piping to maintain consistent slope between connecting pipes and resist floatation of the piping during bench and trough construction.
- Place and consolidate concrete from the base of the manhole to the springline of the flow through trough piping to reconstruct the base, bench and trough. Concrete shall be IDOT Class PP-1 concrete with the addition of 4 lb./cu. yd. of macro fibers. Approved macro fibers are Strux 90/40 Synthetic Macro Fibers as manufactured by GCP Applied Technologies Inc.
- During finishing of the benches, the concrete shall be finished smooth and sloped in accordance with bench detailing for new manholes detail in the plans.
- After 24 hours of cure time has elapsed on the concrete trough and benches, the flow through trough piping shall be modified by having the top of the piping from the spring line up to and including the crown removed by careful trimming.

Method 2 – Use of PVC Pipe and Fittings to Construct Flow-Through Trough with Hydraulic Cement.

This reconstruction method consists of the following steps:

- Carefully chip out around the manhole pipe connections in such a way that the connecting carrier pipes are not damaged – chip only as required to reconstruct the watertight seal at the manhole with hydraulic cement packing (approximately 1"-2" around the pipe and 2" deep along the pipe outside circumference).
- Construct a flow through trough using approved piping material - sizing and configuration of replacement piping shall be governed by the size of the sewers connecting to the respective manhole as well as the angles of sewer pipes connecting to the manhole at the trough.
- Pipe and fittings used to reconstruct troughs shall be the same as sanitary sewer pipe used for this project.
- Depending upon the configuration of the sewers connecting to the manhole at the trough, tees and wyes may be used to reconstruct a trough that will convey all sewage coming into a manhole out through the exit piping.

- Exercise care in trimming the pipe(s) to length such that a tight fit with minimal clearance exists between the existing connecting pipes and the constructed flow through trough piping.
- The manhole pipe connections shall be sealed to the manhole by packing hydraulic cement in the void created during the chipping and additional hydraulic cement shall be packed around the trough piping to form a continuous fillet of at least 1.5" inches in width around the trough pipe at the connection points – this work shall be performed at the same time such that the connection and the fillet are monolithic.
- Shim and support the flow through trough piping to maintain consistent slope between connecting pipes and resist floatation of the piping during bench and trough construction.
- Place and consolidate concrete from the base of the manhole to the spring line of the flow through trough piping to reconstruct the base, bench and trough. Concrete shall be IDOT Class PP-1 concrete with the addition of 4 lb./yd. of macro fibers. Approved macro fibers are Strux 90/40 Synthetic Macro Fibers as manufactured by GCP Applied Technologies Inc.
- During finishing of the benches, the concrete shall be finished smooth and sloped in accordance with bench detailing for new manholes detail in the plans.
- After 24 hours of cure time has elapsed on the concrete trough and benches, the flow through trough piping shall be modified by having the top of the piping from the spring line up to and including the crown removed by careful trimming.

Method 3 – Hand Form and Trowel Flow-Through Trough with Hydraulic Cement Connection Seals

This reconstruction method consists of the following steps:

- Carefully chip out around the manhole pipe connections in such a way that the connecting carrier pipes are not damaged – chip only as required to reconstruct the watertight seal at the manhole with hydraulic cement packing (approximately 1"-2" around the pipe and 2" deep along the pipe outside circumference).
- The manhole pipe connections shall be sealed to the manhole by packing hydraulic cement in the void created during the chipping.
- Reconstruct the bench and trough by placement and consolidation of concrete from the base of the manhole to the bench to reconstruct the base, bench and trough.
- Form and trowel a smooth half pipe trough and bench - sizing and configuration of replacement half pipe trough shall be governed by the size of the sewers connecting to the respective manhole as well as the angles of sewer pipes connecting to the manhole at the trough.
- During finishing of the benches, the concrete shall be finished smooth and sloped in accordance with bench detailing for new manholes detail in the plans.

- Concrete shall be IDOT Class PP-1 concrete with the addition of 4 lb./yd. of macro fibers. Approved macro fibers are Strux 90/40 Synthetic Macro Fibers as manufactured by GCP Applied Technologies Inc.

The slope of the reconstructed trough shall be consistent and continuous throughout the entire reconstructed trough. The slope shall be governed by the differential between the lowest manhole outlet pipe invert and the invert of the lowest manhole influent pipe that introduces flow into the manhole. If the manhole has two outlets, the trough shall be reconstructed relative to the lowest outlet pipe invert. If there are multiple influent pipes, those influent pipes that are more than four (4") inches above the lowest influent pipe shall not be connected to the replacement trough.

Where specified by GPSD, the Contractor shall construct inside drop systems in accordance with GPSD specifications. If possible, internal drop systems shall be constructed such that their bottoms are allowed to rest on the manhole bench; however, reconstructed manhole benches must be completely constructed with all materials cured prior to the placement of internal drop systems.

If there are no pipes other than that effluent pipe to which the replacement trough is joined, then the Contractor shall construct a trough half pipe from the manhole outlet to the interior of the manhole at a minimum slope of one (1) percent. The length of the half pipe shall be equal to the interior width of the manhole or 4' feet, whichever is less. The trough pipe shall be blocked at the end opposite of its connection to the outlet piping to prevent materials from the bench reconstruction from slipping into the trough and to provide a solid end around which a bench can be reconstructed.

The minimum concrete thickness below the trough shall be not less than 4" or the depth of the manhole wall or barrel section, whichever is greater. In the event the manhole wall or barrel section is shallower than 4" below the invert of the trough, the Contractor shall carefully excavate a sub-trough in the underlying soils to permit the placement of the minimum thickness of concrete.

Sanitary saddle-type manhole shall not be backfilled until structure has passed infiltration, exfiltration, or exfiltration of air under pressure tests.

This work will be paid for at the contract unit price per Each for SANITARY MANHOLES, SADDLE – TYPE of the size specified.

SANITARY SEWER STAGING AND BYPASS FLOW SYSTEM

The work in this special provision includes furnishing all labor, materials, and equipment required to stage construction of proposed sanitary sewer improvements and to provide a sanitary sewer by-pass flow system in accordance with details in the plans and notes in the plans.

This work will be paid for at the contract Lump Sum price for SANITARY SEWER STAGING AND BY-PASS FLOW SYSTEM.

INSPECTION AND TESTING OF SEWERS

The work in this special provision includes furnishing all labor, materials, and equipment required to inspect and test sanitary sewer pipe.

The term Engineer as referenced within this specific special provision is defined as the Illinois Department of Transportation's Resident Engineer and City of East Peoria's Water and Wastewater Supervisor.

Following sewer construction, but prior to acceptance, all sewers, appurtenances and work shall be tested and inspected according to the following table. The Contractor shall notify the Engineer when the work is ready for inspection and testing. All testing shall be witnessed by the Engineer. The method or combination of methods to be used shall be approved by the Engineer. Air testing shall be the basic standard of acceptance testing; however, the Engineer may require that sections of sewers and manholes be tested using infiltration or exfiltration methods.

| Metric | Method | Reference | Applicability/Exceptions | Responsible Party |
|--|---|--|--------------------------------|-------------------|
| Infiltration/ Exfiltration (Use one or combination of methods) | Pressure Testing Using Compressed Air | ASTM F1417 | Plastic Non-pressure Sewer | Contractor |
| | Low Pressure Air Joint Testing | ASTM F3058 | Gravity Sewers $\geq 30"$ Dia. | Contractor |
| | Exfiltration of Water | Standard Specifications for Water and Sewer Construction in Illinois | | Contractor |
| | Infiltration of Water | Standard Specifications for Water and Sewer Construction in Illinois | | Contractor |
| Deflection | Mandrel/Rigid Ball | ASTM D3034 | For flexible pipe | Contractor |
| Line/Grade & Cleanliness | CCTV Camera | GPSD Specification 093 | | Contractor |

Before the commencement of testing, the sewer shall be cleaned and all wyes, tees, and stubs shall be sealed with Cherne Gripper Mechanical Plugs or approved equal. Testing procedures shall conform to the following:

1. Procedure for Pressure Testing using Compressed Air: Prior to testing, the section of sewer to be tested shall have been trench backfilled. Pneumatic plugs with a sealing length equal to or greater than the diameter of the pipe to be tested shall be placed in both ends of the pipe to be tested and inflated in accordance with manufacturer pressure rating and guidelines. The sealed sewer pipe shall then be pressurized to four (4) psi above the average back pressure of ground water over the sewer pipe, but not to less than 3.5 psi.(Note: Air test pressures shall be increased 0.43 psi for every foot of depth of groundwater over the sewer line at the upstream end. If the ground water level is 2 feet or more above the top of the pipe or if the air pressure required for the test is greater than 9 psi-gauge, the air testing practice should not be used.) The air pressure shall then be allowed to stabilize for at least two (2) minutes. After the stabilization period the line shall be pressurized to 3.5 psi and the time measured in minutes and seconds until the pressure

drops to 2.5 psi. Air leakage shall not be less than the time per inch of pipe diameter per length of sewer pipe as specified below (next page):

MINIMUM AIR TEST TIME FOR VARIOUS PIPE SIZES
(Per Standard Specifications for Water & Sewer Main Construction in Illinois
and ASTM 1417; current as of December 2022)

| Pipe Diameter (in.) | Specification Time for Pressure drop from 3.5 to 2.5 psig (min:sec) | | | | | | |
|---------------------------|---|--------|--------|--------|--------|--------|--------|
| | 100 ft | 150 ft | 200 ft | 250 ft | 300 ft | 350 ft | 400 ft |
| 6 | 5:40 | 5:40 | 5:40 | 5:40 | 5:40 | 5:40 | 5:42 |
| 8 | 7:34 | 7:34 | 7:34 | 7:34 | 7:36 | 8:52 | 10:08 |
| 10 | 9:26 | 9:26 | 9:26 | 9:53 | 11:52 | 13:51 | 15:49 |
| 12 | 11:20 | 11:20 | 11:24 | 14:15 | 17:05 | 19:56 | 22:47 |
| 15 | 14:10 | 14:10 | 17:48 | 22:15 | 26:42 | 31:09 | 35:36 |
| 18 | 17:00 | 19:13 | 25:38 | 32:03 | 38:27 | 44:52 | 51:16 |
| 21 | 19:50 | 26:10 | 34:54 | 43:37 | 52:21 | 61:00 | 69:48 |
| 24 | 22:47 | 34:11 | 45:34 | 56:58 | 68:22 | 76:46 | 91:10 |
| 27 | 28:51 | 43:16 | 57:41 | 72:07 | 86:32 | 100:57 | 115:22 |
| 30 | 35:37 | 53:25 | 71:13 | 89:02 | 106:50 | 124:38 | 142:26 |
| 33 | 43:05 | 64:38 | 86:10 | 107:43 | 129:16 | 150:43 | 172:21 |
| 36 | 51:17 | 76:55 | 102:34 | 128:12 | 153:50 | 179:29 | 205:07 |
| 42 | 69:48 | 104:42 | 139:37 | 174:30 | 209:24 | 244:19 | 279:13 |
| 48 | 91:10 | 136:45 | 182:21 | 227:55 | 273:31 | 319:06 | 364:42 |
| 54 | 115:24 | 173:05 | 230:47 | 288:29 | 346:11 | 403:53 | 461:34 |
| 60 | 142:28 | 213:41 | 284:55 | 356:09 | 427:23 | 498:37 | 569:50 |

Note: Consult with pipe manufacturer for maximum test pressure for pipe >30 in. diameter.

2. Low Pressure Air Joint Testing: For gravity sewers 27 inches diameter and larger, using low-pressure air in accordance with ASTM F3058 is permitted.
 - a. Position joint test apparatus over the joint to be tested. Make sure the end element sealing tubes straddle both sides of the joint and the hoses are attached.
 - b. Inflate end element sealing tubes with air in accordance with equipment manufacturer's instructions. Introduce low pressure air slowly until the void pressure reaches the starting pressure of 3.5 psi greater than the average back pressure of any groundwater above the pipe, but not greater than 6 psi.
 - c. After the starting air pressure is reached, throttle the air supply to maintain that internal pressure for at least 10 seconds to allow the air pressure and temperature to stabilize before starting the test.
 - d. After the system has been allowed to stabilize, disconnect the air supply and allow pressure to drop. If pressure holds, or drops less than 1 psi in 5 seconds, the joint is acceptable.
 - e. If the joint being tested fails, it shall be retested, or repaired if necessary, and retested until joint passes.

3. **Exfiltration of Water Procedure:** Prior to testing, the section of sewer to be tested shall have been trench backfilled and cleaned. The section of sewer to be tested shall be sealed by inserting inflatable rubber bags into the pipes, or by other means approved by GPSD, and then water shall be introduced into a manhole until the section is completely filled. The Contractor shall fill the pipe to the test level prior to the time of exfiltration testing to permit normal absorption into the pipe walls. Throughout the test period of at least one (1) hour, the water level in the upper manhole shall be maintained at least twenty-four (24) inches above the crown of the upper end of the pipe or at least twenty-four (24) inches above the ground water table, whichever is higher. The length of pipe tested shall be limited so that the pressure on the centerline of the lower end of the section tested shall not exceed six (6) feet of water column. The exfiltration leakage shall not exceed 240 gallons per inch of pipe diameter per mile per day of sewer pipe, including manholes in the test section.
4. **Infiltration of Water Procedure:** Prior to testing, the section of sewer to be tested shall have been trench backfilled and cleaned. Infiltration testing shall be conducted by inducing infiltration conditions by jetting the sewer trench for a sufficient length of time to ensure that the water level in the trench is a minimum of twenty-four (24) inches over the crown of the sewer pipe. The test must be performed before existing sewers are connected and before sewage flow is allowed into the sewers. Infiltration leakage shall not exceed 200 gallons per inch of pipe diameter per mile per day of sewer pipe, including manholes in the test section.
5. **Deflection Testing Procedure:** All gravity sewers constructed of flexible piping materials shall be tested for deflection. The deflection tests shall not be performed prior to thirty (30) days after installation and backfilling of the sewer trench. Deflection testing shall be performed by pulling a mandrel or rigid ball through the pipeline. The diameter of the device shall be ninety-five (95) percent of the un-deflected inside diameter of the pipe. Deflection testing shall be performed without mechanical pulling devices. Whenever possible, the testing shall initiate at the downstream manhole and proceed upstream. The maximum allowable deflection shall be five (5) percent.

Where the deflection is found to be in excess of five (5) percent, the Contractor shall excavate to the point of excess deflection, remove the excessive deflection from the installed pipe and carefully re-compact bedding and backfill materials around the repaired pipe. The line shall then again be re-tested for excessive deflection. After subsequent testing, should the pipe continue to exhibit unacceptable deflection, the line shall be replaced by the Contractor at no additional cost to the project.

When infiltration or exfiltration occurs in excess of the specified amounts, defective pipe or joints shall be located and repaired at the expense of the Contractor. If defective portions cannot be located, the Contractor, at his own expense, shall remove and reconstruct as much of the original work as necessary to obtain a sewer within the allowable infiltration limits. All methods for repairing defects shall be approved by the Engineer.

This work will not be paid for separately but shall be considered included in the contract unit price per Foot for SANITARY SEWER of the type and diameter specified and included in the contract unit price per Foot for CARRIER SANITARY SEWER of the type and diameter specified.

SANITARY MANHOLE FRAME AND COVER

The work in this special provision includes furnishing all labor, materials, and equipment required to furnish and install sanitary manhole frames and covers.

Sanitary sewer manhole covers shall be of the concealed pick type castings shall be self-sealing, extra heavy duty with the City of East Peoria logo as manufactured by the East Jordan Works Foundry (or equal). Non-Rocking castings will not be permitted.

All castings shall be of uniform quality, free from blowholes, porosity, hard spots, shrinkage, distortion or other defects. They shall be smooth and well-cleaned by shot blasting and shall be coated with asphalt paint. Castings shall be sealed to the top of the manhole with butyl rubber sealant.

This work will be paid for at the contract unit price per Each for SANITARY MANHOLE FRAME AND COVER.

SANITARY MANHOLES TO BE RECONSTRUCTED

The work in this special provision includes furnishing all labor, materials, and equipment required to remove existing corbel and barrel sections down to an existing joint. The entire vertical length of the corbel and barrel section(s) shall be removed. New precast barrel section(s) with a precast flat top manhole shall be installed such that the sanitary manhole frame and cover meets the proposed finished grade.

The replacement of the removed corbel and barrel section(s) shall be constructed utilizing the existing joint.

If the barrel section to remain does not have a joining system that is able to be utilized the replacement precast concrete barrel sections shall be constructed on top of the remaining barrel. The new circular sections shall be centered and leveled on top of the remaining barrel as well as possible. The remaining barrel section shall provide support for the proposed manhole sections and, where in contact, two rows of butyl-rubber rings shall be used to form the joint between the poured, concrete collar and the replacement barrel sections. If this is not possible a precast barrel section with an inside diameter larger than the outside diameter of the brick manhole shall be furnished and set on a concrete foundation in accordance with GPSD's standard details.

A poured-in-place concrete collar is to be constructed around the joint of the new and remaining barrel sections to both secure the position of the new barrel sections and to provide tapered surfaces. Where the position of the existing barrel deviates from the replacement sections, the concrete collar shall be tapered to promote conveyance of any water and solids down into the manhole bench and trough. Sharp angles and benches created at the location of the joint shall not be allowed. Tapered surfaces shall be made part of the concrete collar and sloped at a grade of no less than one-inch (1") vertical for each twelve-inches (12") horizontal away from the new wall section. The concrete collar is to be constructed using material in accordance with these Specifications.

The concrete collar shall be sufficiently sized to encompass the horizontal cross-sections of both the existing and proposed barrel sections by six-inches (6") beyond the outer-circumference. Additionally, the concrete collar shall vertically extend at least twelve-inches (12") below and above the joint of the remaining and new barrel sections.

The term Engineer as referenced within this specific special provision is defined as the Illinois Department of Transportation's Resident Engineer and City of East Peoria's Water and Wastewater Supervisor.

Items included in this work include, but are not limited to:

- All excavation and embankment,
- Shoring to be designed and sealed by an Illinois Licensed Structural Engineer,
- Backfill (refer to detail(s) in the plans),
- Connection to existing manhole structure,
- Removal of existing corbel and barrel section(s),
- New precast barrel section(s),
- New precast flat top section,
- Adjusting rings,
- Concrete blocks, if applicable,
- Concrete Collar if applicable
- All ancillary materials required to construct fully functional flat top manhole, and
- Testing and inspection for leakage in accordance with ASTM C1244-93 or ASTM C969-94.

Sanitary manholes shall be precast manholes with a minimum compressive strength as designated by the Illinois licensed structural engineer of record. Concrete compressive strength tests using cores taken from manhole products shall not be allowed; rather, concrete compressive strength tests shall be performed using compression test cylinders a set of which shall be constructed no less than daily per concrete mix. Furthermore, in contrast to that allowable within the text of applicable ASTM guidelines, in no case shall any cylinder tested fall below one hundred percent (100%) of the specified strength of the concrete. All test results shall be the property of the Engineer and provided by the Contractor immediately after the completion of testing. If compression test results are less than allowable, the Engineer shall determine necessary measures to be taken including potential rejection of constructed precast, manhole components.

Manhole joint configurations for new section(s):

| Nominal Diameter | Inside Diameter | Wall Thickness | Joint Height | Joint Type |
|------------------|-----------------|----------------|--------------|-------------------------|
| 4" | 48" | 5" | 2.75" | One-step Forsheda Joint |
| 5" | 60" | 6" | 4.75" | Single Offset Joint |
| 6" | 72" | 7" | 6.00" | 3M Pipe Joint |

Joints between concrete manhole components shall be constructed using butyl rubber. Butyl rubber sealants shall be the CS-202 Butyl Rubber Sealant as manufactured by ConSeal Concrete Sealants, Inc. The ends of each butyl rubber sealant shall overlap to ensure closure. The exterior side of the manhole joints shall be sealed with CS-212 Polyolefin Backed Exterior Joint Wrap as manufactured by ConSeal Concrete Sealants, Inc.

The height of the barrel shall be suitable such that the top of manhole castings and lids shall be flush and consistent with the existing surface surrounding the manhole. A twelve-inch (12") or sixteen-inch (16") long barrel section shall be required immediately beneath a flat-top lid.

The maximum height of adjusting rings to be allowed for use under the manhole frame shall be eight (8") inches. Rubber adjusting rings shall be used for adjustments where the raise is less than or equal to three (3") inches; for all adjustments, at least two (2") inches of rubber adjusting rings shall be used immediately below the manhole frame. Manhole casting adjusting rings may be used for minor height adjustments not exceeding eight (8") inches; however, concrete adjusting rings of thickness two (2") inches or less shall not be allowed. If the surface surrounding the manhole is uneven, tapered rubber adjusting rings as provided by the manufacturer may be used. Adjusting rings shall be concentric to the manhole access opening with a deviation no greater than one eighth of an inch (1/8").

Rubber adjusting rings shall be either Infra-Riser Multi-Purpose Rubber Composite Adjustment Risers as manufactured by East Jordan Iron Works, Inc. or rubber adjusting rings as manufactured by American Highway Products, Ltd.

Steps in the manhole shall be equal to M.A. Industries, Inc. molded step PS-1-PF and shall be placed as specified on the applicable Detail Drawing as provided in Section 095 of these Specifications. Manhole steps shall be centered over the outlet pipe on eight (8") inch to and including twelve (12") inch diameter sewers. Steps shall be installed at ninety (90°) degrees to the outlet pipe on sewers larger than twelve (12") inches.

Manhole Safety Ramps as manufactured by American Highway Products, Ltd. May be used as directed.

Concrete anchoring systems shall be either Trubolt Wedge Anchors, Type 316 stainless steel, part of the Red Head Concrete Anchoring Systems manufactured by Illinois Tool Works (ITW), Inc or Type 316 Stainless Steel threaded rod bedded by an adhesive anchoring system in boreholes. Where utilized boreholes shall be freshly drilled and cleaned with compressed air prior to installation of the adhesive anchoring system. If precast or previously drilled boreholes are to be utilized, they shall also be cleaned with a non-residue degreasing agent and allowed to completely dry prior to installation of the adhesive anchoring system. Boreholes shall have a diameter of not less than 1.25 times nor more than 1.5 times the diameter of the threaded rod. Boreholes shall have a minimum embedment depth of not less than three inches or five times the diameter of the threaded rod, whichever is greater.

The type of anchor used as well as the thread length and anchor diameter shall be as specified on the project plans. Where indicated on the project plans, concrete anchoring shall be coupled with an adhesive anchoring system. Adhesive anchoring systems shall be EPCON, two-part epoxy, Adhesive Anchoring Systems as manufactured by ITW, Inc.

Capsule anchors shall be of the size and length required as outlined on the Plans and Specifications and shown on the Shop Drawings unless otherwise stated. Anchors shall be Molly Parabol type M24-1, Ramset Chemset CTR10. Installation shall be per manufacturer's recommendations.

Sanitary manhole shall not be backfilled until structure has passed infiltration, exfiltration, or exfiltration of air under pressure tests.

This work will be paid for at the contract unit price per Each for SANITARY MANHOLES TO BE RECONSTRUCTED of the size specified.

ABANDON AND FILL EXISTING SANITARY MANHOLE

The work in this special provision includes furnishing all labor, materials, and equipment required to fill existing sanitary sewer manholes.

Existing sanitary manhole structure shall be removed to an existing joint that is a minimum 2.0' below proposed finished grade.

The ends of the sanitary sewer pipes to be abandoned in place and shall be sealed per the existing detail in the plans.

After the mortar has set, the sanitary manhole structure shall be filled with controlled low strength material. The first lift of controlled low strength material shall be 3' feet thick maximum and allowed to cure for 24 hours minimum prior to filling the remainder of the manhole structure with controlled low strength material. The controlled low strength material shall be placed with tubes such that the drop is not greater than 5' feet.

This work also includes all earthwork required to meet the proposed finished grades around the sanitary manhole to be filled.

All material resulting from the filling of existing sanitary manholes shall be disposed of by the Contractor according to Article 202.03 of the Standard Specifications.

This work, including controlled low strength material, sealing abandoned sanitary sewer pipes, and earthwork will be paid for at the contract unit price per Each for ABANDON AND FILL EXISTING SANITARY MANHOLE.

ELECTRIC SERVICE INSTALLATION, SPECIAL

Description. This work shall consist of installing, modifying, or extending an electric service installation according to the Standard Specifications as applicable, as shown on the plans and as described herein.

Materials. Materials shall be in accordance with Article 804.02 of the Standard Specifications as applicable and as shown on the plans.

CONSTRUCTION REQUIREMENTS

General. Utility coordination and installation shall be according to Articles 804.03 and 804.04 of the Standard Specifications and as shown on the plans.

Method Of Measurement. Electric Service Installation, Special shall be counted, Each.

Basis Of Payment. This work will be paid for at the contract unit price Each for ELECTRIC SERVICE INSTALLATION, SPECIAL which shall be payment in full for the work specified herein.

REMOVAL OF UNDERPASS LIGHTING UNIT, NO SALVAGE

Description. This work shall consist of the removal and disposal of existing underpass lighting units according to the Standard Specifications as applicable, as shown on the plans and as described herein.

CONSTRUCTION REQUIREMENTS

General. Removal work shall be according to Articles 842.02 and 842.03 of the Standard Specifications.

Method Of Measurement. Each lighting unit which is removed and delivered to a Department storage facility, or disposed of as indicated, will be counted as a unit for payment.

Basis Of Payment. This work will be paid for at the contract unit price Each for REMOVAL OF UNDERPASS LIGHTING UNIT, NO SALVAGE which shall be payment in full for the work specified herein.

CONTRACT GUARANTEE

The Contractor shall guarantee all electrical equipment, apparatus, materials, and workmanship provided under the contract for a period of six (6) months after the date of final inspection according to Article 801.14 of the Standard Specifications.

All instruction sheets required to be furnished by the manufacturer for materials and supplies and for operations shall be delivered to the Engineer prior to the acceptance of the project, with the following warranties and guarantees:

1. The manufacturer's standard written warranty for each piece of electrical equipment or apparatus furnished under the contract.
2. The Contractor's written guarantee that, for a period of six (6) months after the date of final inspection of the project, all necessary repairs to or replacement of said warranted equipment, or apparatus shall be made by the Contractor at no cost to the Department.
3. The Contractor's written guarantee for satisfactory operation of all electrical systems furnished and constructed under the contract for a period of 6 months after final inspection of the project.

SYSTEM IMPLEMENTATION, EQUIPMENT INTEGRATION AND SUPPORT

The Contractor shall install the CCTV cameras and wireless radios at the locations indicated on the plans.

The CCTV cameras and wireless radios along with all related components shall be subject to a 30-day burn-in period. During the "burn-in" period, all components shall perform continuously, without any interruption of operation, for a period of thirty days. In the event that there are operational problems during the burn-in period, the burn-in period shall reset back to day one.

After the successful completion of the burn-in period, the system will have completed final acceptance.

The Department will program the cameras and integrate them into the existing ITS system.

The Contractor shall be responsible for installing the proposed CCTV cameras in accordance with the plans, specifications, and manufacturers recommended practices.

This work will not be paid for separately but shall be included in the contract bid price.

CONSTRUCTION PERMITS

The Contractor shall be responsible for obtaining all required permits from counties, municipalities, and other entities prior to beginning work. The Contractor shall pay all costs associated with obtaining the permits.

Basis of Payment. This work will not be paid for separately but shall be included in the contract bid price.

AS-BUILT DOCUMENTATION

The Contractor shall locate all proposed conduit, communication vaults, handholes, junction boxes, light poles, and camera poles every 100 feet using a GIS locating device that is accurate to the nearest Foot.

The Contractor shall provide a GIS based map of the conduit route and a complete listing of all of map coordinates in an electronic format (Google Earth KML or KMZ shape file).

Basis of Payment. This work will not be paid for separately but shall be included in the contract bid price.

HPS LAMP RECYCLING

The Contractor shall recycle high-pressure sodium lamps through a certified recycling company. The Contractor shall submit detailed information pertaining to recycling to the Department for review along with the electrical material submittals. The Contractor shall submit proof of recycling to the Department.

Basis of Payment: This work will not be paid for separately but shall be included in the contract unit price for the lighting removal pay items.

POTHOLING FOR LOCATION OF EXISTING UNDERGROUND UTILITIES

Potholing to locate existing underground utilities shall be included in the contract bid price for the conduit pay items.

Removal and replacement of existing sidewalk, pavement, and islands only for utility locating purposes will not be paid for separately but shall also be included in the contract bid price for the conduit pay items.

HANDHOLE, PORTLAND CEMENT CONCRETE

This work shall consist of furnishing the materials and constructing a handhole in accordance with the applicable Articles of Section 814 and 1088 of the Standard Specifications with the following modifications:

The lift ring for the cover shall consist of a solid closed ring of stainless steel at least 3/8 inch in diameter. The lift ring shall be attached to the cover by a loop of stainless steel at least 3/8 inch in diameter. The lift ring and loop shall be recessed in the cover.

The Contractor shall install heavy-duty, fully galvanized hooks, with a minimum diameter of 1/2" in the proposed handhole. The Contractor shall submit this material to the Engineer prior to construction of the handholes.

The lid shall be marked with the legend "Traffic Signals".

Pre-cast handholes are not allowed.

All unsuitable materials shall be disposed of by the Contractor outside the job limits.

Basis of Payment: This work will be paid for at the contract unit price Each for HANDHOLE, PORTLAND CEMENT CONCRETE which price shall be payment in full for all labor, materials, and equipment required to provide the handhole described above as well as any necessary excavating, backfilling, disposal of unsuitable materials, and furnishing all materials within the limits of the handhole.

WIDE AREA VIDEO DETECTION SYSTEM COMPLETE

The following video detection systems are approved for use within District Four:

Gridsmart (1 Camera System for 4 Approaches equipped with High-Capacity Storage Option).

The video detection system shall be a Gridsmart GS-3 system for integration into the existing ITS System.

The video vehicle detection system shall include all necessary electric cable, electrical junction boxes, electrical and communications surge suppression, brackets, hardware, software, programming, and all other items that are required for installation and configuration. These items should be taken into consideration and shall be included in the bid price for the video detection system.

All CAT 5 Ethernet cable shall meet the requirements contained in the special provisions (outdoor rated, gel-filled, shielded, etc.).

All vehicle video detection systems shall be equipped with the latest software or firmware revisions.

The video vehicle system shall be configured and installed to NEMA TS2 Standards (use of the SDLC port and BIU). Installation conforming to NEMA TS1 standards will not be allowed.

The Contractor shall furnish and install a SDLC splitter cable and connect the proposed video detection processor to the SDLC splitter cable.

The Department will program all video detection systems.

The video detection cameras shall be installed on the strain pole of the mast arm that is located closest to the traffic signal controller cabinet or at the locations shown on the plan sheets.

The Contractor shall install the camera at a 45 ft. minimum height and the maximum height shall not exceed 47' feet.

The camera mast shall be secured to the mast arm strain pole using two brackets (one at top of camera mast and one at the bottom) with stainless steel banding. The Contractor shall furnish and install two brackets per intersection.

All CAT 5 cable runs shall not exceed 300' feet. The Contractor shall measure the distance of the cable and test the cable for continuity by using a handheld tester that shows the length of each cable pair.

The Contractor shall install the system components in accordance with the manufacturer's recommendations. The Contractor shall install a green insulated #12 AWG wire from the camera surge suppressor to the ground bus inside the cabinet and connect the drain shield from the CAT 5 ethernet cable to the ground lug located inside the surge arrestor.

The Contractor shall measure the distance from the bottom of the camera to the roadway and record this information inside the cabinet.

The minimum requirements for a video vehicle detection system are listed below:

1.0 General

This Specification sets forth the minimum requirements for a system that monitors vehicles on a roadway via processing of video images and provides detector outputs to a traffic controller or similar device.

1.1 System Hardware

The system shall consist of one video camera and an automatic control unit (ACU). The ACU shall process all detected calls and shall be equipped with the latest firmware revisions.

1.2 System Software

The system shall be able to detect either approaching or receding vehicles in multiple traffic lanes. A minimum of twenty-four (24) detection zones shall be user-definable per camera. The user shall be able to modify and delete previously defined detection zones. The software shall provide remote access operation and shall be the latest revision.

2.0 Functional Capabilities

2.1 Real-Time Detection

2.2 The ACU shall be capable of simultaneously processing information from up to four (4) digital video sources. The video shall be digitized and analyzed at a rate of 30 times per second.

2.3 The system shall be able to detect the presence of vehicles in a minimum of 96 detection zones within the combined field of view of the image sensors.

3.0 Vehicle Detection

3.1 Detection Zone Placement

The video detection system shall provide flexible detection zone placement anywhere and at any orientation within the combined field of view of the image sensors. In addition, detection zones shall have the capability of implementing logical functions including AND and/or.

3.2 Optimal Detection

The video detection system shall reliably detect vehicle presence when the image sensor is mounted 10m (30 ft.) or higher above the roadway, when the image sensor is adjacent to the desired coverage area, and when the length of the detection area or field of view (FOV) is not greater than ten (10) times the mounting height of the image sensor. The image sensor shall not be required to be mounted directly over the roadway. A single image sensor, placed at the proper mounting height with the proper lens, shall be able to monitor six (6) to eight (8) traffic lanes simultaneously.

3.3 Detection Performance

Overall performance of the video detection system shall be comparable to inductive loops. Using standard image sensor optics and in the absence of occlusion, the system shall be able to detect vehicle presence with 98% accuracy under normal conditions, (day and night) and 96% accuracy under adverse conditions (fog, rain, snow). The ACU shall output a constant call for each enabled detector output channel if a loss of video signal occurs in any camera.

The ACU shall be capable of processing a minimum of twenty detector zones placed anywhere in the field of view of the camera.

4.0 ACU Hardware

4.1 ACU Mounting

The ACU shall be shelf or rack mountable. Nominal outside dimensions excluding connectors shall not exceed 180mm (7.25") x 475mm (19") x 260mm (10.5") (H" x W" x D").

4.2 ACU Environmental

The ACU shall be designed to operate reliably in the adverse environment found in the typical roadside traffic cabinet. It shall meet the environmental requirements set forth by the NEMA (National Electrical Manufacturers Association) TS1 and TS2 standards as well as the environmental requirements for Type 170 and Type 179 controllers. The minimum operating temperature range shall be from -35°C to +74°C at 0% to 95% relative humidity, non-condensing.

5.0 ACU Electrical

- 5.1 The ACU shall be modular in design and provide processing capability equivalent to the Intel Pentium microprocessor. The bus connections used to interconnect the modules of the ACU shall be gold-plated DIN connectors.
- 5.2 The ACU shall be powered by 89 - 135 VAC, 60 Hz, single phase, and draw 0.25 amps, or by 190 - 270 VAC, 50 Hz, single phase and draw 0.12 amps. If a rack mountable ACU is supplied, it shall be capable of operating from 10 to 28 VDC. The power supply shall automatically adapt to the input power level. Surge ratings shall be as set forth in the NEMA TS1 and TS2 specifications.
- 5.3 Communications to a remote computer equipped with remote monitoring software shall be through a RJ-45 Ethernet port.
- 5.4 The ACU shall be equipped with a NEMA TS2 RS-485 SDLC interface for communicating input and output information. Front panel LEDs shall provide status information when communications are open.
- 5.5 The ACU and/or camera hookup panel shall be equipped with four RJ-45 connector based/terminal block connections for cameras so that signals from four image sensors can be processed in real-time.
- 5.6 The ACU shall be equipped with USB ports, and Ethernet ports to provide communications to a computer running the configuration and remote access software.
- 5.7 The ACU and/or camera hookup panels used for a rack mountable ACU shall be equipped with a video output port.
- 5.8 The ACU shall be equipped with viewable front panel detection LED indications.

6.0 Camera

- 6.1 The video detection system shall use a high resolution, color, camera as the video source for real-time vehicle detection. As a minimum, each image sensor shall provide the following capabilities:
 - a. H.264 video compression and transport
 - b. Support video streaming that is viewable with an adjustable frame rates of 5/15/30 fps
 - c. Images shall be produced with a CCD sensing element with horizontal resolution of at least 720 lines and vertical resolution of at least 480 lines.
 - d. Useable video and resolvable features in the video image shall be produced when those features have luminance levels as low as 0.1 lux at night.

- e. Useable video and resolvable features in the video image shall be produced when those features have luminance levels as high as 10,000 lux during the day.
 - f. Automatic gain, automatic iris, and absolute black reference controls shall be furnished.
 - g. An optical filter and appropriate electronic circuitry shall be included in the image sensor to suppress "blooming" effects at night.
- 6.2 The image sensor shall be equipped with an integrated zoom lens with zoom and focus capabilities that can be changed using either configuration computer software or handheld controller. The machine vision processor (MVP) may be enclosed within the camera.
- 6.3 The image sensor and lens assembly shall be housed in an environmental enclosure that provides the following capabilities:
- a. The enclosure shall be waterproof and dust-tight to NEMA-4 specifications. The camera shall be IP-67 rated.
 - b. The enclosure shall allow the image sensor to operate satisfactorily over an ambient temperature range from -34°C to +74°C while exposed to precipitation as well as direct sunlight.
 - c. The enclosure shall allow the image sensor horizon to be rotated in the field during installation.
 - d. A heater shall be at the front of the enclosure to prevent the formation of ice and condensation in cold weather, as well as to assure proper operation of the lens' iris mechanism. The heater shall not interfere with the operation of the image sensor electronics, and it shall not cause interference with the video signal.
 - f. The enclosure shall be light-colored and shall include a sun shield to minimize solar heating. The front edge of the sunshield shall protrude beyond the front edge of the environmental enclosure and shall include provision to divert water flow to the sides of the sunshield. The amount of overhang of the sun shield shall be adjustable to prevent direct sunlight from entering the lens or hitting the faceplate.
 - g. The total weight of the image sensor in the environmental enclosure with sunshield shall be less than 2.7 kg (6 pounds).
 - h. When operating in the environmental enclosure with power and video signal cables connected, the image sensor shall meet FCC class B requirements for electromagnetic interference emissions.
- 6.3 The video output of the image sensor shall be isolated from earth ground. All video connections from the image sensor to the video interface panel shall also be isolated from earth ground.

- 6.4 The video output, communication, and power to the image sensor shall include transient protection to prevent damage to the sensor due to transient voltages occurring on the cable leading from the image sensor to other field locations.
- 6.5 A stainless-steel junction box shall be available as an option with each image sensor for installation on the structure used for image sensor mounting. The junction box shall contain a terminal block for terminating power to the image sensor and connection points for cables from the image sensor and from the ACU.
- 6.6 Software
 - 7.1 The system shall include the remote access software that is used to setup and configure the video detection system. The software shall be of the latest revision.
 - 7.2 All necessary cable, adapters, and other equipment shall be included with the system.
- 8.0 Installation and Training
 - 8.1 The supplier of the video detection system shall supervise the installation and testing of the video and video vehicle detection equipment. A factory certified representative from the supplier shall be on-site during installation.
- 9.0 Warranty, Maintenance, and Support
 - 9.1 The video detection system shall be warranted by its supplier for a minimum of three (3) years from date of turn-on. This warranty shall cover all material defects and shall also provide all parts and labor as well as unlimited technical support.
 - 9.2 Ongoing software support by the supplier shall include updates of the ACU and supervisor software. These updates shall be provided free of charge during the warranty period.
 - 9.3 The supplier shall maintain a program for technical support and software updates following expiration of the warranty period. This program shall be made available to the contracting agency in the form of a separate agreement for continuing support.

Basis of Payment: This work will be paid for at the contract unit price Each for WIDE AREA VIDEO DETECTION SYSTEM COMPLETE which price shall be payment in full for all labor, equipment, and materials required to furnish, install, and test the video vehicle detection system described above, complete.

CAT 5 ETHERNET CABLE

This work shall be in accordance with Sections 873, 1076, and 1088 of the Standard Specifications except as modified herein.

This work shall consist of furnishing and installing an outdoor rated CAT5E cable in conduits, handholes, and poles.

The cable shall be rated for outdoor use and conform to the following specifications:

- Outdoor CMX Rated Jacket (climate/oil resistant jacket)
- UV Resistant Outer Jacket Material (PVC-UV, UV Stabilized)
- Outer Jacket Ripcord
- Designed for Outdoor Above- Ground or Conduit Duct applications
- Cat5E rated to 350MHz (great for 10/100 or even 1000mbps Gigabit Ethernet)
- Meets TIA/EIA 568b.2 Standard
- Shielded Twist Pair
- 4 Pairs, 8 Conductors
- 24AWG, Solid Core Copper
- UL 444 ANSI TIA/EIA-568.2 ISO/IEC 11801
- RoHS Compliant
- Water Blocking Gel

Basis of Payment: This work will be paid for at the contract unit price per Foot for CAT 5 ETHERNET CABLE, which shall be payment in full for all labor, equipment, and materials required to provide and install the cable described above, complete.

FIBER OPTIC CABLE 12 FIBERS, SINGLE MODE OR FIBER OPTIC CABLE 48 FIBERS, SINGLE MODE

This work shall be in accordance with Sections 801, 864, 871, and 1076 of the Standard Specifications except as modified herein.

The Contractor shall furnish and install a 48F SM fiber optic cable for lateral splicing into a mainline fiber. The Contractor shall splice all 48 strands to each end of the existing fiber optic cable using weatherproof splice kits located inside the existing and proposed communications vaults. All individual fibers within the fiber optic cable shall be spliced continuously using a fusion splicer. Proposed fiber strands shall be spliced to existing fiber strands to restore fiber continuity.

THE CONTRACTOR SHALL SCHEDULE AND CONDUCT ALL MAINLINE FUSION SPLICING WITHIN THE MAINTENANCE WINDOW OF 12:00 A.M. TO 5:00 A.M. THE CONTRACTOR SHALL SPLICE ALL LIVE FIBER STRANDS FIRST TO RESTORE CONNECTIVITY AND REDUCE OUTAGE TIME, FOLLOWED BY ALL UNUSED FIBER STRANDS. THE CONTRACTOR SHALL NOTIFY THE DEPARTMENT A MINIMUM OF FOURTEEN DAYS PRIOR TO SPLICING. THE COST OF THIS WORK SHALL BE INCLUDED IN THE COST OF THE FIBER OPTIC CABLE.

The Contractor shall furnish and install a 12F SM fiber optic cable for communications with a CCTV equipment cabinet. The Contractor shall terminate 12 single mode fiber stands with ST connectors inside the existing traffic signal cabinet at IL Route 116 (Main Street) & Gold Street and inside the proposed CCTV equipment cabinet.

The Contractor may elect to furnish a fiber optic cables that have greater fiber counts at no additional cost to the Department.

Each cable shall be clearly labeled in each cabinet utilizing a durable computer-generated label. The label shall contain information in regard to the location where the cable is going to or coming from, buffer tube, and fiber color. The Contractor shall provide numerical foot marking data at each handhole, vault, and cabinet to the Department.

Unused buffer tubes shall be readily accessible for future use. Each buffer tube shall be neatly coiled inside each traffic signal and CCTV cabinet with a minimum length of eight (8') feet.

Fibers not being used shall be labeled "spare", and fibers not attached to a distribution enclosure shall be capped and sealed.

All ancillary components, required to complete the fiber optic cable plant, including but not limited to, moisture and water sealants, cable caps, fan-out kits, weather-proof splice kits, boots, cable trays, splice enclosures, termination panels, etc., shall be supplied under this pay item and will not be paid for separately. These items shall be submitted to the Department for approval.

The fiber optic cable shall be clearly marked in each handhole, communication vault, and cabinet with a brightly colored (orange or yellow) weather resistant label securely attached to the cable.

The Contractor shall provide and install a 12 Ga., stranded (EPR-TYPE RHW or THHN), insulated tracer cable in all conduits that contain fiber optic cable and do not contain an existing tracer wire. This work shall be done at the same time the fiber optic cable is pulled. There will be no additional compensation for this work.

Materials. The single-mode, fiber optic cable shall incorporate a loose, buffer-tube design. The cable shall conform to the requirements of RUS 7 CFR1755.900 (PE-90) for a single sheathed, non-armored cable, and shall be new, unused and of current design and manufacture. The number of fibers in each cable shall be as specified on the plans.

CONSTRUCTION REQUIREMENTS

Experience Requirements:

Personnel involved in the installation, splicing and testing of the fiber optic cables shall meet the following requirements:

- a. A minimum of three (3) years' experience in the installation of fiber optic cables, including splicing, terminating and testing single mode fibers.
- b. Install two systems where fiber optic cables are outdoors in conduit and where the systems have been in continuous satisfactory operation for at least two years. The Contractor shall submit as proof, photographs or other supporting documents, and the names, addresses and telephone numbers of the operating personnel who can be contacted regarding the installed fiber optic systems.
- c. One fiber optic cable system (which may be one of the two in the preceding paragraph), which the Contractor can arrange for demonstration to the Department representatives and the Engineer.

Installers shall be familiar with the cable manufacturer's recommended procedures for installing the cable. This shall include knowledge of splicing procedures for, and equipment being used on this project and knowledge of all hardware such as breakout (furcation) kits and splice closures. The Contractor shall submit documented procedures to the Engineer for approval and to be used by Construction inspectors.

Personnel involved in testing shall have been trained by the manufacturer of the fiber optic cable test equipment to be used, in fiber optic cable testing procedures. Proof of this training shall be submitted to the Engineer for approval. In addition, the Contractor shall submit documentation of the testing procedures for approval by the Engineer.

Installation in Conduit.

During cable pulling operations, the Contractor shall ensure that the minimum bending of the cable is maintained during the unreeling and pulling operations. Entry guide chutes shall be used to guide the cable into the handhole conduit ports. Lubricating compound shall be used to minimize friction. Corner rollers (wheels), if used, shall not have radii less than the minimum installation-bending radius of the cable. A series array of smaller wheels can be used for accomplishing the bend if the cable manufacturers specifically approve the array.

The pulling tension shall be continuously measured and shall not be allowed to exceed the maximum tension specified by the manufacturer of the cable. Fuse links and breaks can be used to ensure that the cable tensile strength is not exceeded. The pulling system shall have an audible alarm that sounds whenever a pre-selected tension level is reached. Tension levels shall be recorded continuously and shall be given to the Engineer upon request.

The cable shall be pulled into the conduit as a single component, absorbing the pulling force in all tension elements. The central strength member and Aramid yarn shall be attached directly to the pulling eye during cable pulling. "Basket grip" or "Chinese-finger type" attachments, which only attach to the cable's outer jacket, shall not be permitted. A breakaway swivel, rated at 95% of the cable manufacturer's approved maximum tensile loading, shall be used on all pulls. When simultaneously pulling fiber optic cable with other cables, separate grooved rollers shall be used for each cable.

Splicing Requirements: Splices shall be made at locations shown on the Plans. Any other splices shall be permitted only with the approval of the Engineer. The Contractor shall submit a splicing plan to the Department for approval.

Operation And Maintenance Documentation:

After the fiber optic cable plant has been installed, two (2) complete sets of Operation and Maintenance Documentation shall be provided. The documentation shall, as a minimum, include the following:

- Complete and accurate as-built diagrams showing the entire fiber optic cable plant including locations of all splices.
- Final copies of all approved test procedures.
- Complete performance data of the cable plant showing the losses at each terminal connector.

- Complete parts list including names of vendors.
- Electronic Testing Files (OTDR traces, power meter data, etc.)

Testing Requirements:

Testing shall be in accordance with Article 801.13 except where modified by this special provision.

The Contractor shall submit detailed test procedures for approval by the Engineer. All continuous fiber runs shall be tested bi-directionally at both 1310 nm and 1550 nm with a power meter and optical source and OTDR. For testing, intermediate breakout fibers may be concatenated and tested end-to-end. Any discrepancies between the measured results and these specifications will be resolved to the satisfaction of the Engineer.

The Contractor shall provide the date, time and location of any tests required by this specification to the Engineer at least 5 days before performing the test. Upon completion of the cable installation, splicing, and termination, the Contractor shall test all fibers in each link for continuity and attenuation. The test procedure shall be as follows:

A Certified Technician utilizing an Optical Source/Power Meter and OTDR shall conduct the testing. The Technician is directed to conduct the test using the standard operating procedures defined by the manufacturer of the test equipment. All fibers installed shall be tested in both directions.

At the completion of the test, the Contractor shall provide two copies of documentation of the test results to the Engineer. The test documentation shall be bound and shall include the following:

Cable & Fiber Identification:

Cable ID
Cable Location - beginning and end point
Fiber ID, including tube and fiber color
Operator Name
Date & Time
Setup Parameters
Wavelength
Pulse width (OTDR)
Refractory index (OTDR)
Range (OTDR)
Scale (OTDR)
Setup Option chosen to pass OTDR "dead zone"

Test Results:

Optical Source/Power Meter

Total Attenuation
Attenuation (dB/km)

These results shall be provided in tabular form. The following shall be the criteria for the acceptance of the cable:

The test results shall show that the dB/km loss does not exceed +3% of the factory test or 1% of the cable's published production loss. However, no event shall exceed 0.10 dB. If any event is detected above 0.10 dB, the Contractor shall replace or repair the proposed fiber and/or fusion splice and connector including that event point.

The total dB loss of the cable, less events, shall not exceed the manufacturer's production specifications as follows: 0.5 dB/km at both 1310 and 1550 nm.

If the total loss exceeds these specifications, the Contractor shall replace or repair that cable run at the Contractor's expense, both labor and materials. Elevated attenuation due to exceeding the pulling tension during installation shall require the replacement of the cable run at the Contractor's expense, including labor and materials.

The Contractor shall label the destination of each trunk cable onto the cable in each handhole and termination panel.

Slack Storage of Fiber Optic Cables.

A part of this pay item, slack fiber shall be supplied as necessary to allow splicing the fiber optic cables in a controlled environment, such as a splicing van or tent. After splicing has been completed, the slack fiber shall be stored underground in handholes and in the traffic controller cabinets.

The amount of slack cable listed in Article 873.03 shall be revised as follows:

| <u>Location</u> | <u>Length of Slack Cable (Ft.)</u> |
|------------------------|------------------------------------|
| Communications Vault | 200.0 |
| Double Handhole | 60.0 |
| Handhole | 30.0 |
| CCTV or Signal Cabinet | 10.0 |
| Junction Box | 10.0 |
| Equipment Cabinet | 3.0 |

Basis of Payment: This work will be paid for at the contract unit price per Foot for FIBER OPTIC CABLE 12 FIBERS, SINGLE MODE or FIBER OPTIC CABLE 48 FIBERS, SINGLE MODE and shall be payment in full for all labor, equipment, and materials required to provide, install, terminate, splice, and test the fiber optic cable described above, complete.

FUSION SPLICING OF FIBER OPTIC CABLES

Description. The Contractor will splice optical fibers from different cable sheaths and protect them with a splice closure at the locations shown on the Plans. Fiber splicing consists of in-line fusion splices for all fibers described in the cable plan at the particular location.

Two types of splices are identified. A mainline splice includes selected fibers from each cable run as shown in the plan sheets. In a lateral splice, the buffer tubes in the mainline cable are dressed out and those fibers identified on the plans are accessed in and spliced to lateral cables.

Materials.

Splice Closures:

Splice closures shall be designed for use under the most severe conditions such as moisture, vibration, impact, cable stress and flex temperature extremes as demonstrated by successfully passing the factory test procedures and minimum specifications listed below:

Physical Requirements:

The closures shall provide ingress for up to four cables in a butt configuration.

The closure shall prevent the intrusion of water without the use of encapsulates.

The closure shall be capable of accommodating splice organizer trays that accept mechanical, or fusion splices. The splice closure shall have provisions for storing fiber splices in an orderly manner, mountings for splice organizer assemblies, and space for excess or un-spliced fiber. Splice organizers shall be re-enterable. The splice case shall be UL rated.

Closure re-entry and subsequent reassembly shall not require specialized tools or equipment. Further, these operations shall not require the use of additional parts.

The splice closure shall have provisions for controlling the bend radius of individual fibers to a minimum of 1.5 in (38 mm).

Factory Testing of Splice Closures:

Compression Test: The closure shall not deform more than 10% in its largest cross-sectional dimension when subjected to a uniformly distributed load of 1335 N at a temperature of 0°F and 100°F (-18°C and 38°C). The test shall be performed after stabilizing at the required temperature for a minimum of two hours. It shall consist of placing an assembled closure between two flat parallel surfaces, with the longest closure dimension parallel to the surfaces. The weight shall be placed on the upper surface for a minimum of 15 minutes. The measurement shall then be taken with weight in place.

Impact Test: The assembled closure shall be capable of withstanding an impact of 28 N-M at temperatures of 0°F and 100°F (-18°C and 38°C). The test shall be performed after stabilizing the closure at the required temperature for a minimum of 2 hours. The test fixture shall consist of 20 lb (9 kg) cylindrical steel impacting head with a 2 in (5 cm) spherical radius at the point where it contacts the closure. It shall be dropped from a height of 12 in (30 cm). The closure shall not exhibit any cracks or fractures to the housing that would preclude it from passing the water immersion test. There shall be no permanent deformation to the original diameter or characteristic vertical dimension by more than 5%.

Cable Gripping and Sealing Testing: The cable gripping and sealing hardware shall not cause an increase in fiber attenuation in excess of 0.05 dB/fiber @ 1550 nm when attached to the cables and the closure assembly. The test shall consist of measurements from six fibers, one from each buffer tube or channel, or randomly selected in the case of a single fiber bundle. The measurements shall be taken from the test fibers before and after assembly to determine the effects of the cable gripping and sealing hardware on the optical transmission of the fibers.

Vibration Test: The splice organizers shall securely hold the fiber splices and store the excess fiber. The fiber splice organizers and splice retaining hardware shall be tested per EIA Standard FOTP-II, Test Condition I. The individual fibers shall not show an increase in attenuation in excess of 0.1 dB/fiber.

Water Immersion Test: The closure shall be capable of preventing a 10 ft (3 m) water head from intruding into the splice compartment for a period of 7 days. Testing of the splice closure is to be accomplished by the placing of the closure into a pressure vessel and filling the vessel with tap water to cover the closure. Apply continuous pressure to the vessel to maintain a hydrostatic head equivalent to 10 ft (3 m) on the closure and cable. This process shall be continued for 30 days. Remove the closure and open to check for the presence of water. Any intrusion of water in the compartment containing the splices constitutes a failure.

Certification: It is the responsibility of the Contractor to insure that either the manufacturer, or an independent testing laboratory has performed all of the above tests, and the appropriate documentation has been submitted to the Department. Manufacturer certification is required for the model(s) of closure supplied. It is not necessary to subject each supplied closure to the actual tests described herein.

CONSTRUCTION REQUIREMENTS

The closure shall be installed according to the manufacturer's recommended guidelines. For all splices, the cables shall be fusion spliced.

The Contractor shall prepare the cables and fibers in accordance with the closure and cable manufacturers' installation practices. A copy of these practices shall be provided to the Engineer 21 days prior to splicing operations.

Using a fusion splicer, the Contractor shall optimize the alignment of the fibers and fuse them together. The Contractor shall recoat the fused fibers and install mechanical protection over them.

Upon completing all splicing operations for a cable span, the Contractor shall measure the mean bi-directional loss at each splice using an Optical Time Domain Reflectometer. This loss shall not exceed 0.1 dB.

The Contractor shall measure the end-to-end attenuation of each fiber optic link, from connector to connector, using an optical power meter and source. This loss shall be measured from both directions and shall not exceed 0.5 dB per installed kilometer of single mode cable. Measurements shall be made at both 1300 and 1550 nm for single mode cable. For multimode cable, power meter measurements shall be made at 850 and 1300 nm. The end-to-end attenuation shall not exceed 3.8 dB/installed kilometers at 850nm or 1.8 dB per installed kilometer at 1300nm for multimode fibers.

As directed by the Engineer, the Contractor at no additional cost to the Department shall replace any cable splice not satisfying the required objectives.

The Contractor shall secure the Splice Closure to the side of the splice facility using cable support brackets. All cables shall be properly dressed and secured to rails or racks within the handhole or traffic signal cabinet. No cables or enclosures will be permitted to lie on the floor of the splice facility. Cables that are spliced inside a building will be secured to the equipment racks or walls as appropriate and indicated on the Plans.

Basis of Payment. This work will not be paid for separately but shall be included in the bid price for the fiber optic cable pay items.

TERMINATION OF FIBER OPTIC CABLES WITH FUSION SPLICED ST CONNECTORS

Description. The Contractor shall terminate a single mode fiber by fusion splicing a factory-formed ST connector (from a pre-formed fiber optic pigtail) onto a field fiber at the locations shown on the Plans.

Materials. The Contractor shall be responsible for ensuring that the pre-formed pigtail fiber is compatible with the field fiber that it will be fusion splice to.

The splice shall be protected with a protection sleeve/enclosure that will secure both cables and prevent cable movement.

The fiber optic patch cords shall meet or exceed the following specifications:

- High-quality 125um fiber optics
- 900um tight buffer construction
- Aramid yarn individually protected
- Duplex construction
- Stress relief boots color coded (Tx/Rx)
- ST connectors with high-grade zirconia ferrule
- Insertion Loss < 0.2 dB @ 1310 / 1550 nm
- Return Loss < -58 dB @ 1310 / 1550 nm
- Compliant with ANSI/TIA/EIA 568-B.3
- TIA/EIA-604, FOCIS-2

The Contractor shall submit a shop drawing of all proposed components to the Engineer for approval prior to commencing construction.

CONSTRUCTION REQUIREMENTS

The Contractor shall prepare the cables and fibers in accordance with the cable manufacturers' installation practices. A copy of these practices shall be provided to the Engineer 21 days prior to splicing operations.

Using a fusion splicer, the Contractor shall optimize the alignment of the fibers and fuse them together. The Contractor shall recoat the fused fibers and install mechanical protection over them.

Upon completing all splicing operations for a cable span, the Contractor shall measure the mean bi-directional loss at each connector using an Optical Time Domain Reflectometer. This loss shall not exceed the loss of the fusion splice (0.1 dB) plus the loss of the connector (typically 0.75 dB).

As directed by the Engineer, the Contractor at no additional cost to the Department shall replace any cable splice and/or connector not satisfying the required objectives.

Basis of Payment: This work will not be paid for separately but shall be included in the bid price for the fiber optic cable pay items.

COMMUNICATIONS VAULT

This work shall be in accordance with Sections 814 and 1088 of the Standard Specifications except as modified herein.

This work shall consist of furnishing and installing a communications vault constructed of polymer concrete.

The following items are approved for use in District Four:

Hubbel, Quazite, Part Numbers: PG2436HA00 (Cover) and PG2436BA30 (Box) or Approved Equal

The communications vault and lid shall conform to the following specifications:

Cover:

Material: Polymer Concrete
Nominal Dimensions: 24"W x 36"L
Gasketed, Heavy Duty Lid with 2 Bolts
Design/Test Load: 15,000/22,500 lbs.
ANSI Tier: 15
Gasketed

Box:

Material: Polymer Concrete
Nominal Dimensions: 24"W x 36"L x 42"D
Open Bottom
Design/Test Load: 22,500/33,750 lbs.
ANSI Tier: 22

The location of the handhole shall be excavated so that the top of the handhole is set flush with the sidewalk or paved surface. When installed in earth shoulder away from the pavement edge, the top surface of the handhole shall be 1" in. (25 mm) above the finished grade. The excavation shall be deep enough to accommodate the depth of the box and French drain.

The French drain shall be constructed underneath the proposed handhole according to Article 601.06 of the Standard Specifications and in accordance with Highway Standard 814006.

The conduits shall enter the vault at between 33" and 36" and the Contractor shall install 6" inches of CA 5 or CA 7 in the bottom of the vault.

The Contractor shall submit testing reports to verify that the communications vaults and lids meet the requirements of ANSI Tier 15 and ANSI Tier 22 loading.

A fiber optic marker shall be installed next to each communication vault.

The locating cable shall be continuous and accessible on the outside of each communication vault through the use of a locating cap that is installed on top of the fiber optic marker.

The Contractor shall utilize appropriate corrosion resistant hardware (stainless steel) and connections to the locating wire. The Contractor shall submit material and installation methods to the Department for review.

Basis of Payment: This work will be paid for at the contract unit price of Each for COMMUNICATIONS VAULT, which shall be payment in full for all labor, equipment, and materials required to provide and install the equipment described above, complete.

CLOSED-CIRCUIT TELEVISION DOME CAMERA, HD

Description. This work shall consist of furnishing and installing an integrated Closed-Circuit Television (CCTV) Dome Camera Assembly, camera bracket, and all other items required for installation and operation. This assembly shall contain all components identified in the Materials Section and shall be configured as indicated on the plan sheets.

Materials.

The CCTV camera shall be an Axis Model Q6075-E Dome Camera Assembly for integration into the existing District 4 ITS system.

The Contractor shall provide all materials required to install the proposed camera on the proposed sign structure camera mast as shown on the plan sheets.

The Contractor shall submit catalog cut sheets to the Department for all items (mounting brackets, hardware, etc.) that will be utilized for review prior to commencing work.

The existing CAT 5 cable will be used with the proposed CCTV cameras unless otherwise noted on the plan sheets.

The Contractor shall remove the existing cameras and camera brackets and deliver them to the IDOT traffic building located at 1025 West Detweiler Drive, Peoria. The Contractor shall notify the District 4 ITS Technician, Mr. Greg Dixon at (309) 258-7481 forty-eight (48) hours prior to delivery. The Contractor shall place the existing cameras inside the boxes and packaging that came with the new cameras to prevent damaging the cameras during transport.

The Contractor shall furnish and install new camera brackets at all locations. The existing CAT 5 ethernet cable shall be re-terminated in the bracket terminal block (use IDC connector and pre-formed RJ-45 connector that is furnished with the camera bracket) in accordance with the manufacturer's instructions.

The Department will program the cameras prior to installation.

The camera shall meet or exceed the following specifications:

CAMERA

| | |
|-----------------------|--|
| VIDEO: | 60 Hz (NTSC), 50 Hz (PAL) |
| IMAGE SENSOR: | 1/2.8" progressive scan CMOS |
| LENS: | 4.44–142.6 mm, F1.6–4.41 Horizontal angle of view: 62.8°–2.23° Vertical angle of view: 36.8°–1.3° Autofocus, auto-iris |
| DAY AND NIGHT: | Automatically removable infrared-cut filter |
| MINIMUM ILLUMINATION: | Color: 0.3 lux at 30 IRE F1.6 B/W: 0.03 lux at 30 IRE F1.6 Color: 0.5 lux at 50 IRE F1.6 B/W: 0.04 lux at 50 IRE F1.6 |
| SHUTTER TIME: NTSC: | 1/33000 s to 1/3 s with 50 Hz 1/33000 s to 1/4 s with 60 Hz |
| PAN/TILT/ZOOM: | Pan: 360° endless, 0.05° - 450°/s Tilt: 220°, 0.05°-450°/s 32x optical zoom and 12x digital zoom, total 384x zoom E-flip, 256 preset positions, Tour recording, Guard tour, Control queue, On-screen directional indicator, Set new pan 0°, Adjustable zoom speed |

VIDEO

| | |
|---------------------|---|
| VIDEO COMPRESSION: | H.264 (MPEG-4 Part 10/AVC), Motion JPEG |
| RESOLUTIONS: | HDTV 1080p 1920x1080 to 320x180 HDTV 720p 1280x720 to 320x180 |
| FRAME RATE (H.264): | Up to 60/50 fps (60/50 Hz) in HDTV 720p Up to 30/25 fps (60/50 Hz) in HDTV 1080p |
| VIDEO STREAMING: | Multiple, individually configurable streams in H.264 and Motion JPEG, Axis' Zipstream technology, Controllable frame rate and bandwidth, VBR/MBR H.264 |
| IMAGE SETTING: | Manual shutter time, compression, color, brightness, sharpness, white balance, exposure control, exposure zones, fine tuning of behavior at low light, rotation: 0°, 180°, text and image overlay, 32 individual 3D privacy masks, image freeze on PTZ, automatic defog, backlight compensation |

Wide Dynamic Range (WDR): Up to 120 dB depending on scene, highlight compensation

NETWORK

SECURITY: Password protection, IP address filtering, HTTPSa encryption, IEEE 802.1Xa network access control, Digest authentication, User access log, Centralized Certificate Management

PROTOCOLS: IPv4/v6, HTTP, HTTPSa, SSL/TLSa, QoS Layer 3 DiffServ, FTP, CIFS/SMB, SMTP, Bonjour, UPnP, SNMP v1/v2c/v3 (MIB-II), DNS, DynDNS, NTP, RTSP, RTP, SFTP, TCP, UDP, IGMP, RTCP, ICMP, DHCP, ARP, SOCKS, SSH, NTCIP

SYSTEM INTEGRATION

APPLICATION PROGRAM INTERFACE: Open API for software integration, including VAPIX® and AXIS Camera Application Platform; specifications at www.axis.com, AXIS Video Hosting System (AVHS) with One-Click Connection, ONVIF Profile S, specification at www.onvif.org

ANALYTICS: Video motion detection, Autotracking, Active Gatekeeper Basic Analytics (not to be compared with third-party analytics): Object removed, Enter/Exit detector, Fence detector, Object Counter, Highlight compensation, Support for AXIS Camera Application Platform enabling installation of third-party applications, see www.axis.com/acap

EVENT TRIGGERS: Detectors: Live stream accessed, Video motion detection, Shock Detection, Object removed, Enter/Exit detector, Fence detector, Object counter; Hardware: Fan, Network, Temperature, Casing Open; PTZ: Autotracking, Error, Moving, Ready, Preset Reached; Storage: Disruption, Recording; System: System Ready; Time: Recurrence, Use Schedule; Input signal: Manual trigger, Virtual input

EVENT ACTIONS: Day/night mode, overlay text, video recording to edge storage, pre- and post-alarm video buffering, send SNMP trap
PTZ: PTZ preset, start/stop guard tour
File upload via FTP, SFTP, HTTP, HTTPS network share and Email; Notification via email, HTTP, HTTPS and TCP

DATA STREAMING Event data

BUILT IN INSTALLATION AIDS Pixel Counter

GENERAL

| | |
|--------------------------|---|
| CASING: | IP66-, NEMA 4X- and IK10-rated Metal casing (aluminum), polycarbonate (PC) clear dome, sunshield (PC/ASA) |
| SUSTAINABILITY: | PVC Ffree |
| MEMORY: | 512 MB RAM, 128 MB Flash |
| POWER CAMERA: | Axis High PoE midspan 1-port: 100–240 V AC, max 74 W Camera consumption: typical 16 W, max 60 W |
| CONNECTORS: | RJ45 10BASE-T/100BASE-TX PoE, RJ45 Push-pull Connector (IP66) included |
| EDGE STORAGE: | Support for SD/SDHC/SDXC card Support for recording to dedicated network-attached storage (NAS); For SD card and NAS recommendations see www.axis.com |
| OPERATING CONDITIONS: | With 30 W midspan: -20 °C to 50 °C (-4 °F to 122 °F) With 60 W midspan: -50 °C to 50 °C (-58 °F to 122 °F) Maximum temperature (intermittent): 60 °C (140 °F) Arctic Temperature Control: Start-up as low as -40 °C (-40 °F) Humidity 10–100% RH (condensing) |
| APPROVALS: | EMC: EN 55022 Class A, EN 61000-3-2, EN 61000-3-3, EN 61000- 6-1, EN 61000-6-2, EN 55024, FCC Part 15 Subpart B Class A, ICES-003 Class A, VCCI Class A, RCM AS/NZS CISPR 22 Class A, KCC KN32 Class A, KN35 Safety: IEC/EN/UL 60950-1, IEC/EN/UL 60950-22 Environment: EN 50121-4, IEC 62236-4, IEC 60068-2-1, IEC 60068-2-2, IEC 60068-2-6, IEC 60068-2-14, IEC 60068-2-27, IEC 60721-4-3, NEMA 250 Type 4X, IEC 60068-2-30, IEC 60068-2-60, IEC 60068-2-78, IEC/EN 60529 IP66, NEMA TS-2-2003 v02.06, Subsection 2.2.7, 2.2.8, 2.2.9; IEC 62262 IK10, ISO 4892-2 Midspan: EN 60950-1, GS, UL, cUL, CE, FCC, VCCI, CB, KCC, UL-AR |
| WEIGHT: | 3.7 kg (8.2 lb.) |
| INCLUDED ACCESSORIES: | Axis High PoE 60 W midspan 1-port, RJ45 Push-pull Connector (IP66), Sunshield, Installation Guide, Windows decoder 1-user license |

VIDEO MANAGEMENT: AXIS Camera Companion, AXIS Camera Station, Video
SOFTWARE: management software from Axis' Application Development
Partners available on www.axis.com/techsup/software

WARRANTY: Axis 3-year warranty and AXIS Extended Warranty option

Environmental Enclosure/Housing

The environmental enclosure shall be designed to physically protect the integrated camera from the outdoor environment and moisture via a sealed enclosure. If the option exists in the standard product line of the manufacturer, the assembly shall be supplied with an integral sun shield. The enclosure shall be fully water and weather resistant with a NEMA 4 rating or better.

The camera dome shall be constructed of distortion free acrylic or equivalent material that must not degrade from environmental conditions. The environmental housing shall include a camera-mounting bracket. In addition, the environmental housing shall include a heater, blower, and power surge protector. An integral fitting compatible with a standard 1-1/2 in (38.1 mm) NPT pipe, suitable for outdoor pendant mounting shall also be provided.

The enclosure shall be equipped with a heater controlled by a thermostat. The heater shall turn on when the temperature within the enclosure falls below 40°F (4.4°C). The heater shall turn off when the temperature exceeds 60°F (15.6°C). The heater will minimize internal fogging of the dome faceplate when the assembly is operated in cold weather.

In addition, a fan shall be provided as part of the enclosure. The fan will provide airflow to ensure effective heating and to minimize condensation.

The enclosure shall be equipped with a hermetically sealed, weatherproof connector, located near the top for external interface with power, video, and control feeds.

CCTV Dome Camera Mounting Supports

The Contractor shall furnish and install an Axis Pole Mount Bracket T91L61 (Part Number 5801-721) for camera installation on traffic signal mast arms and CCTV camera poles and stainless-steel banding as required.

Mounting supports shall be configured as shown on the camera support detail plans and as approved by the Engineer. Mount shall be of aluminum construction with enamel or polyester powder coat finish. Braces, supports, and hardware shall be stainless steel. Wind load rating shall be designed for sustained gusts up to 90 mph (145 km/hr.), with a 30% gust factor. Load rating shall be designed to support up to 75 lb. (334 N). For roof or structural post/light pole mounting, mount shall have the ability to swivel inward for servicing. The mounting flange shall use standard 1-1/2-inch (38.1 mm) NPT pipe thread.

Connecting Cables

The Contractor shall furnish and install outdoor rated, shielded CAT 5E cable at the locations shown on the plan sheets. The cable shall be terminated using the terminal block inside the camera bracket and the IDC connector and pre-formed IP66 rated RJ-45 connector on the camera end and a shielded RJ-45 connector in the cabinet. The Contractor shall test the cable prior after termination.

Cable will be paid for separately under the pay item for CAT 5 ETHERNET CABLE.

CONSTRUCTION REQUIREMENTS

General

The Contractor shall prepare a shop drawing detailing the complete CCTV Dome Camera Assembly and installation of all components to be supplied for approval of the Engineer. Particular emphasis shall be given to the cabling and the interconnection of all of the components.

The Contractor shall install the CCTV dome camera assembly at the locations indicated in the Plans. The CCTV Dome Camera Assembly shall be mounted on a pole, wall, or other structure.

Testing

The Contractor shall test each installed CCTV Dome Camera Assembly. The test shall be conducted from the field cabinet using the standard communication protocol and a laptop computer. The Contractor shall verify that the camera can be fully exercised and moved through the entire limits of Pan, Tilt, Zoom, Focus and Iris adjustments, using both the manual control and presets. The Contractor shall maintain a log of all testing and the results. A representative of the Contractor and a representative of the Engineer shall sign the log as witnessing the results. Records of all tests shall be submitted to the Engineer prior to accepting the installation.

Method of Measurement. The closed-circuit television dome camera bid item will be measured for payment by the actual number of CCTV dome camera assemblies furnished, installed, tested, and accepted.

Basis of Payment. Payment will be made at the contract unit price for each CLOSED-CIRCUIT TELEVISION DOME CAMERA, HD including all equipment, material, testing, documentation, and labor detailed in the contract documents for this bid item.

CAT 6 ETHERNET CABLE

This work shall be in accordance with Sections 873, 1076, and 1088 of the Standard Specifications except as modified herein.

This work shall consist of furnishing and installing an outdoor rated CAT6E cable in conduits, handholes, and poles.

The cable shall meet the requirements for use in the installation of the wireless ethernet radios.

Approved Cable: Belden 7953A or approved equal.

The cable shall be rated for outdoor use and conform to the following specifications:

- 600V Rated
- Outdoor CMX Rated Jacket (climate/oil resistant jacket)
- UV Resistant Outer Jacket Material (PVC-UV, UV Stabilized)
- Outer Jacket Ripcord
- Designed for Outdoor Above- Ground or Conduit Duct applications
- Meets TIA/EIA 568b.2 Standard
- Shielded Twist Pair
- 4 Pairs, 8 Conductors
- 23AWG, Solid Core Copper
- UL 444 ANSI TIA/EIA-568.2 ISO/IEC 11801
- RoHS Compliant
- Water Blocking Gel

Basis of Payment: This work will be paid for at the contract unit price per Foot for CAT 6 ETHERNET CABLE, which shall be payment in full for all labor, equipment, and materials required to provide and install the cable described above, complete.

WIRELESS ETHERNET RADIO

The Contractor shall furnish a wireless ethernet radio system and install it on an existing traffic signal mast arm or mast arm strain pole at the locations shown in the plans.

The Contractor shall furnish and install the wireless radio, surge arrestors, mounting brackets, hardware, and all other items required for installation.

CAT6 Ethernet cable will be paid for separately under the pay item for CAT 6 ETHERNET CABLE.

The Contractor shall install the radios in accordance with the manufacturer's recommendations and aim the radio antennas to ensure optimal signal strength and connectivity.

The wireless ethernet radio shall be an Ubiquiti Networks airFiber 60 LR 60 GHz radio or approved equal that meets or exceeds the following minimum specifications:

Features:

- 60 GHz radio
- Low-interference 60 Ghz spectrum
- Long range, up to 12 km
- Up to 1.9 Gbps with low latency
- Integrated GPS
- Full and half bandwidth support

Mechanical:

Dimensions: Ø413 x 360 mm (Ø16.3 x 14.2")
Weight: Without mount: 1.5 kg (3.3 lb), With mount: 2.7 kg (6 lb)
Enclosure materials: Aluminum, UV stabilized polycarbonate
Mount material: Galvanized steel
Mounting Precision Alignment Kit (included)
Pole compatibility: Ø25.4-76.2 mm (Ø1-3")
Wind loading: 420 N at 200 km/h (94.4 lbf at 125 mph)

Hardware:

Processor: Quad-Core ARM® Cortex® A7
Memory: 256 MB DDR3
RF connections: Internal
Max. power consumption: 18W
Power method: Passive PoE 4-pairs (1, 2+; 3, 6-) (4, 5+; 7, 8-) or 2-pairs (4, 5+; 7, 8-)
Power supply: 48VDC, 0.65A gigabit PoE adapter (included)
Supported voltage range: 48VDC ± 10%
ESD/EMP protection: Air/contact: ± 24kV
Operating temperature: -40 to 60° C (-40 to 140° F)
Operating humidity: 5% to 95% noncondensing
Certifications: FCC, IC, CE

LEDs:

Power: Flashing white: bootup in progress; White: not connected to UISP™ console; Blue: connected to UISP console
Ethernet: Flashing blue: ethernet activity
GPS: Blue: receiving at least (4) GPS satellite signals
60G: Blue: active connection

Software:

OS: airOS®
Operating mode: PtP only
Ubiquiti specific features: Integrated 60 GHz radio, discovery protocol, Wave technology
Network: Bridge mode
Services: UISP, ping watchdog, NTP client, device discovery
Tools: Antenna alignment, discovery utility, ping, trace route, speed test
Software management: Bluetooth management for easy setup over UISP app, WEB UI
Minimum software requirements: Any modern WEB browser/iOS or Android based smartphone

System:

Maximum throughput: 1.95 Gbps
Maximum range: 12+ km
Encryption: WPA2-PSK (AES)

RF

Operating Frequency: 57~71 GHz (Depends on regulatory region)

GPS: Yes

Channel Bandwidth: 2160, 1080 MHz

Installation: The Contractor shall ensure that there is a clear line of sight between radios. The Contractor shall furnish and install outdoor, shielded Category 6 (or above) cabling and shielded RJ45 connectors. The Contractor shall furnish two Ethernet Surge Protectors (model ETH-SP-G2) and install one at each end of the cabling. The Contractor shall test all CAT6 cables after installation. The Department will program and configure the radios.

The Contractor shall install the stabilizer arms on the antennas and aim them towards the receiving antenna. The Contractor shall make adjustments to the antenna aiming to ensure optimal signal strength and radio link connectivity. The Contractor shall furnish all hardware and brackets required to install the radio antennas on the existing mast arm or strain pole.

Basis of Payment: This work will be paid for at the contract unit price per Each for WIRELESS ETHERNET RADIO which price shall be payment in full for all labor, materials, and equipment required to furnish the wireless ethernet radio and install it on an existing traffic signal mast arm or strain pole at the locations shown in the plans.

FLASHING BEACON ASSEMBLY

Description. This work shall consist of furnishing and installing a low voltage DC powered advance flashing beacon system consisting of a metal screw in foundation, galvanized steel traffic signal post with galvanized steel breakaway base, roadway sign, 12V DC yellow LED flashing beacon, brackets and all other items shown on the plan sheet detail.

Materials. The signal post shall consist of a 4-1/2" diameter galvanized steel pole, galvanized steel breakaway base, mounting brackets and banding, hardware, and pole top cap installed on a metal screw in foundation.

The Contractor shall furnish and install including but not limited to hardware, brackets, stainless steel banding, nipples, grommets, wire, and all other items required for installation to make the system fully operational.

Rust, corrosion, and anti-seize protection shall be provided at all threaded assemblies by coating the mating surfaces with an approved compound.

Basis of Payment. This work will be paid for at the contract unit price of Each for FLASHING BEACON ASSEMBLY, which shall be payment in full for furnishing and installing the flashing beacon assembly as described above and shown on the plan sheet detail, and for all labor, materials, and equipment required to complete this work.

POWER OVER ETHERNET EXTENDER

Description. This work shall consist of furnishing and installing an outdoor rated industrial power over ethernet extender, and all other components required for installation and operation. This assembly shall contain all components identified in the Materials Section and shall be configured as indicated on the plan sheets.

Materials.

The ethernet extender shall be an industrial outdoor rated ethernet extender that is IEEE 802.3af, IEEE 802.3at, and high PoE 60W compatible.

The Contractor shall provide all materials required to install the ethernet extender inside an existing handhole or mast arm strain pole.

The Contractor shall submit catalog cut sheets to the Department for all items that will be utilized for review prior to commencing work.

The ethernet extender shall be compatible with the proposed Wide Area Video Detection System and CCTV cameras and meet or exceed the following specifications:

PART 1 GENERAL

1.01 SYSTEM DESCRIPTION

A. General Requirements

1. The specified unit shall be of manufacturer's official product line, designed for commercial and/or industrial 24/7/365 use.

2. The specified unit shall be based upon standard components and proven technology using open and published protocols.

3. Related Requirements

- a. 28 05 07.21 PoE Power Sources for Electronic Safety and Security
- b. 28 05 11 Cyber Security Requirements for Electronic Safety and Security
- c. 28 05 19 Storage Appliances for Electronic Safety and Security
- d. 28 05 21 Network Attached Storage for Electronic Safety and Security
- e. 28 05 23 Storage Area Network for Electronic Safety and Security
- f. 28 23 11 Video Management System Analytics
- g. 28 23 13 Video Management System Interfaces

B. Sustainability

1. The specified unit shall be manufactured in accordance with ISO 14001.
2. The specified unit shall be compliant with the EU directives 2011/65/EU (RoHS) and 2012/19/EU (WEEE).
3. The specified unit shall be compliant with the EU regulation 1907/2006 (REACH).
4. The specified unit, including all its components, shall not contain any added PVC.
5. The manufacturer shall have signed and support the UN Global Compact initiative as defined by United Nations.

1.02 CERTIFICATIONS AND STANDARDS

A. General abbreviations and acronyms

1. FTP: File Transfer Protocol
2. IEEE 802.1x: Authentication framework for network devices
3. IP: Internet Protocol
4. ISO: International Standards Organization
5. LED: Light Emitting Diode
6. PoE: Power over Ethernet (IEEE 802.3af/at) standard for providing power over network cable

B. The specified unit and its components shall carry the following EMC approvals:

1. EN 55032 Class B
2. EN 55024
3. FCC Part 15 Subpart B Class B
4. ICES-003 Class A
5. VCCI Class B
6. RCM AS/NZS CISPR 32 Class B
7. KC KN32 Class A
8. KC KN35

C. The specified unit shall meet the following product safety standards:

1. IEC/EN 62368-1

D. The specified unit shall meet the following standards

1. Networking:

- a. IEEE 802.3af/802.3at (Power over Ethernet)

1.03 QUALITY ASSURANCE

- A. The contractor or security sub-contractor shall be a licensed security contractor with a minimum of five (5) years' experience installing and servicing systems of similar scope and complexity and evidence that is completed at least three (3) projects of similar design and is currently engaged in the installation and maintenance of systems herein described.
- B. All installation, configuration, setup, program and related work shall be performed by electronic technicians thoroughly trained by the manufacturer in the installation and service of the equipment provided.
- C. The contractor or designated sub-contractor shall submit credentials of completed manufacturer certification, verified by a third party organization, as proof of the knowledge.
- D. The specified unit shall be manufactured in accordance with ISO9001.

1.04 WARRANTY

- A. The manufacturer shall provide a five (3) year limited hardware warranty for product that is free from defects in design, workmanship and materials under substantiated normal use. Defective products under the warranty period will be either repaired or replaced by the manufacturer.

PART 2 PRODUCTS

2.01 GENERAL

- A. Units shall be IP-based and comply with established network and video standards.

2.03 VIDEO SURVEILLANCE UNITS

- A. The Outdoor PoE Extender
 - 1. The Outdoor PoE Extender shall meet or exceed the following design specifications:
 - a. The product shall be able to provide PoE and High PoE data rate.
 - b. The product shall provide full functionality, including 100/100Mbps symmetrical data for up to 300 m/984 ft.
 - c. The product shall support Plug-and-Play installation and shall automatically detect PoE and High PoE enabled devices, supplying in-line power.
 - d. The product shall be equipped with Green/Yellow LED display that provides information of network connectivity and power status.
 - e. The product shall provide extended network connectivity and power supply to connected products.
 - f. Hardware interfaces

1. Power
 - a. The product shall provide outdoor extension for power over Ethernet IEEE 802.3af/802.3at Type 1 Class 3 with maximum power consumption at 13W.
2. Network interface
 - a. The product shall be equipped with shielded RJ45, EIA 568A and 568B connectors.
 - b. The product shall be manufactured with an IP66/IP67-rated PVC free, polycarbonate casing.
 - c. Enclosure
 - d. Environmental
1. The product shall:
 - a. Operate in a temperature range of -40°C to 60°C (-40°F to 140°F).
 - b. Operate in a humidity range of 0 to 85% RH (non-condensing).

PART 3 EXECUTION

3.01 INSTALLATION

- A. The contractor's or subcontractor's main resources within the project shall carry proper professional certification issued by the manufacturer and verified by a third-party organization to confirm sufficient product and technology knowledge.
- B. The contractor shall carefully follow instructions in documentation provided by the manufacturer to ensure all steps have been taken to provide a reliable, easy-to-operate system.
- C. All equipment shall be tested and configured in accordance with instructions provided by the manufacturer prior to installation.
- D. All firmware found in products shall be the latest and most up-to-date version as specified by the manufacturer, or by the product component provider.
- E. All equipment requiring users to log on using a password shall be configured with user/site-specific password/passwords. No system/product default passwords shall be allowed.

- F. A proper installation shall meet NEC (National Electrical Code – US only) per the guidelines of that year's revision. When properly installed equipment meets Low Voltage, Class 2 classification of the NEC.

Basis of Payment: This work will be paid for at the contract unit price per Each for POWER OVER ETHERNET EXTENDER which shall be payment in full for all labor, equipment, and materials required to provide and install the equipment described above, complete.

FLASHER CONTROLLER, SPECIAL, WITHOUT CABINET

This work shall be in accordance with the applicable Articles of Section 858 and 1073 of the Standard Specifications with the following modifications:

This pay item consists of furnishing and installing a 12V DC flasher controller inside a proposed lighting controller.

The Contractor shall perform the following work items:

- Furnish and install one new 12VDC 1AMP DIN Rail Mounted Power Supply, Solid State DC Flasher Module, terminal blocks, fusing for flasher circuit (4A Slow Blow), control wiring, and all other miscellaneous components that are required for low voltage 12VDC operation.
- Furnish and install a terminal block and terminate the field wiring on the terminal block. The solid-state flasher module shall be connected to the terminal block as shown on the plan sheet detail.
- The Contractor shall mount all of components to a DIN rail or secure them to the back plate of the lighting controller cabinet.
- Furnish and install one duplex non-GFCI equipment receptacle to power the flasher DC 12V power supply.
- Furnish and install all hardware, brackets, and other items required for low voltage 12VDC flashing beacon operation.

The power supply shall be a Meanwell MDR-60-12 12VDC 5A DIN Rail mounted power supply that or approved equal that meets or exceeds the following minimum specifications:

- 60W DIN-RAIL SWITCHING POWER SUPPLY
- 12 Volts @ 5 Amps
- Universal AC input range
- Protections: Short-circuit, Overload, Overvoltage
- 100% Full load burn-in test
- Soft-start circuit, limiting AC surge current
- Input Voltage: 85-264VAC @ 47-63Hz, or 120-370VDC
- Input Current @ Full load: 1.8A(115V) / 1.0A(230V)
- Cooling by free air convection
- Can be installed on DIN rail TS-35/7.5 or 15
- Class I, Div 2 Hazardous Locations T4
- LED indicator for power on
- DC OK relay contact
- No load power consumption<0.75W
- Size: 3.9" x 1.6" x 3.5"
- Weight: 0.7 lbs.
- 3-year warranty

The solid state flasher shall be a Lights to Go Inc Model AFDC2 56 FPM Alternating Flasher for 12V DC Signal Lights with the following specifications:

Description: 2 Channel Alternating Light Flasher

Voltage: 10 - 24VDC (Incandescent or LED)

Output Current: 4A Max load, .010A Minimum 50W @ 12VDC, 100W @ 24V

Fuse: 6A std/4A slow blow recommended

Sequence: 56 fpm Alternating Flash

Dimensions: 2.8in x1.3in x.45in, (73x33x11mm)

Temp Rating: -40F to 150Deg F (-40C to +70C)

Mounting: 2 flange. 156 holes, 2.45 in spacing

Connection: 3x 6 inch tinned leads

Format: Sealed, encapsulated, IP67

Mass: 0.9 oz, 25gm

Basis of Payment: This work will be paid for at the contract unit price Each for FLASHER CONTROLLER, SPECIAL, WITHOUT CABINET which price shall be payment in full for all labor, materials, and equipment required to furnish and install the low voltage DC flasher controller components inside a proposed lighting controller cabinet as described above.

FIBER OPTIC ETHERNET DROP AND REPEAT SWITCH

The Contractor shall furnish a fiber optic drop and repeat switch (material only) complete with the accessories specified below and deliver it to the Department.

The fiber optic drop and repeat switch shall meet or exceed the following minimum specifications:

Approved Models: Antaira (Aaxeon) Technologies Model LNX-0702G-SFP-T (7-Port (5-port 10/100/1000T + 2 10/100/1000T SFP ports Industrial Ethernet Switch, Wide Operating Temperature) or approved equal.

- | | |
|----------------------|---|
| Features: | <ul style="list-style-type: none">• 5-Port 10/100/1000TX + 2-Port 10/100/1000T/Mini-GBIC Combo• Store-and-Forward Switching Architecture• 10Gbps Back-Plane (Switching Fabric)• 1 Mbits Memory Buffer• 8K MAC Address Table• Wide-Range Redundant Power Design• Power Polarity Reserve Protect• Provides EFT Protection 3000 VDC for Power Line• Supports 6000 VDC Ethernet ESD Protection• IP30 Rugged Aluminum Case Design• 5-Year Warranty |
| Standard: | <ul style="list-style-type: none">• IEEE 802.3 10BaseT Ethernet• IEEE 802.3u 100BaseTX Fast Ethernet• IEEE 802.z Gigabit Fiber• IEEE 802.3x Flow Control and Back-Pressure |
| Protocol: | <ul style="list-style-type: none">• CSMA/CD |
| Switch Architecture: | <ul style="list-style-type: none">• Back-Plane (Switching Fabric): 10Gbps |
| Transfer Rate: | <ul style="list-style-type: none">• 14,880pps for Ethernet Port• 148,800pps for Fast Ethernet Port• 1,488,000pps for Gigabit Fiber Ethernet Port |
| MAC Address: | <ul style="list-style-type: none">• 8K MAC Address Table |
| Memory Buffer: | <ul style="list-style-type: none">• 7,926 pps (default) |
| LED: | <ul style="list-style-type: none">• Unit: Power 1, Power 2, Fault• 10/100 TX: Link/Activity, Full Duplex/Collision• Gigabit Copper: Link/Activity, Speed• SFP: Link/Activity |
| Connector: | <ul style="list-style-type: none">• 10/100/1000T: 5 x RJ-45• 100/1000T: 2 x 100/1000 SFP Sockets |

- Network Cable:
- 10BaseT: 2-pair UTP/STP Cat. 3, 4, 5 cable EIA/TIA-568 100-ohm (100m)
 - 100BaseTX: 2-pair UTP/STP Cat. 5 cable EIA/TIA-568 100-ohm (100m)
- Power Supply:
- DC 12 ~ 48V, Redundant Power with Polarity Reverse Protect Function and Removable Terminal Block
- Power Consumption:
- 6 Watts
- Reverse Polarity Protection:
- Present
- Overload Current Protection:
- Present
- Mechanical:
- Casing: IP30 Metal Case
 - Dimension (W x H x D): 30 x 99 x 142 mm
 - Installation: DIN-Rail/Wall Mountable
- Weight:
- Unit Weight: 1.3 lbs.
 - Shipping Weight: 1.7 lbs.
- Operation Temperature:
- Wide Operating Temperature: -40°C to 75°C (-40°F to 176°F)
- Operation Humidity:
- 5% to 95% (Non-condensing)
- Storage Temperature:
- -40°C to 85°C
- EMI:
- FCC Class A
 - CE EN6100-4-2/EN6100-4-3/EN6100-4-4/EN6100-4-5/EN6100-4-6
 - /EN6100-4-8/EN6100-4-11/EN6100-4-12/EN6100-6-2/EN6100-6-4
- Stability Testing:
- Shock: IEC60068-2-27
 - Free Fall: IEC60068-2-32
 - Vibration: IEC60068-2-6
- Warranty:
- 5-Year Warranty

The following items shall also be included with each switch:

- SFP Fiber Optic Module – Qty. 2 (Aaxeon SFP-S20-T, 1.25Gbps Ethernet SFP Transceiver, Single Mode 20KM / LC / 1310nm, -40°C~85°C)
- Fiber Optic Patch Cables – Qty. 2 (single mode fiber, 1 meter length, duplex, LC/ST connectors)

Basis of Payment: This work will be paid for at the contract unit price per Each for FIBER OPTIC ETHERNET DROP AND REPEAT SWITCH which price shall be payment in full for all labor, materials, and equipment required to provide the fiber optic Ethernet drop and repeat switch and associated equipment and deliver it to the Department.

CLOSED-CIRCUIT TELEVISION CABINET

Description. This work consists of furnishing and installing a pole mounted equipment cabinet and peripheral equipment at locations indicated in the Plans. These cabinets will be utilized to house critical electrical, optical, and communications equipment as defined in other contract pay items.

Materials. Materials shall be in accordance with the following specifications.

General. The equipment cabinet shall conform to the details shown on the plan sheet. Equipment cabinets shall be mounted and anchored on the poles and structures at locations indicated in the Plans. In addition, all mounting hardware and brackets required to install the equipment cabinet on the pole shall be stainless steel and provided by the Contractor. The mounting heights and pole diameters shall be as specified by the Engineer.

The cabinet shall be a NEMA 3R Single Door Enclosure, constructed from .125" thick aluminum, with nominal outside dimensions of 24" (H) x 14" (W) x 10" (D). The cabinet shall have a natural finish.

The cabinet shall be furnished with a slam lock, neoprene door gasket, vent slots, continuous stainless steel door hinge, and all stainless-steel hardware. The cabinet shall also have a Corbin #2 dead bolt lock or skeleton key. The key shall be removable in the lock position only. Two keys shall be supplied for each lock, and all equipment cabinet locks shall be keyed the same. All cables shall be labeled utilizing marking tags.

The cabinet shall be equipped with a main power panel as shown on the cabinet plan detail sheet. The power panel shall include one 15A main breaker, power terminal blocks, and one six outlet power strip with integral surge protection. The power panel shall include a plexi-glass safety shield that covers the power panel.

Power Strip

The cabinet power strip shall have a minimum of six (6) outlets and integral surge suppression that meets or exceeds the following minimum specifications:

- Let Through Voltage: <85 Volts
- Operating Voltage: 120VAC, 50/60H
- UL Suppressed Voltage Rating: 330V
- Energy Rating: 320J
- Peak Current NM/CM: 13k Amps NM, 13k Amps CM
- EMI/RFI Noise Filtration: >25-60dB

The power strip shall be wired directly to the protected power terminals on the cabinet surge arrester.

CONSTRUCTION REQUIREMENTS

The Contractor shall prepare and submit shop drawings that detail all of the components to be supplied, along with associated mounting hardware for the pole mounted equipment cabinet. The shop drawings must be approved by the Engineer prior installation of the completed cabinet in the field.

The Engineer reserves the right to inspect and/or factory test any completed cabinet assemblies prior to shipment of the material to the project site. Any deviations from these specifications that are identified during such testing shall be corrected prior to delivery of the assembly to the project site.

The Contractor shall install the cabinet to an existing or proposed light pole at the locations show on the plan sheets.

The AC power service to be run to the equipment cabinet shall be terminated. In addition, the cabinet shall be connected to an adequate ground following the Standard Specifications.

The Contractor shall terminate any inbound and outbound fiber optic, CAT5E cables, or wireless antenna leads in the equipment cabinet as shown in the Plans. The Contractor shall terminate any twisted pair communication cable on the termination panel in the equipment cabinet as shown in the Plans. Lugs shall be installed at the end of each conductor suitable for connection to the barrier terminal blocks.

Method of Measurement. This item shall be measured for payment by Each pole mounted equipment cabinet in-place.

Basis of Payment. This work shall be paid for at the contract unit price Each for CLOSED CIRCUIT TELEVISION CABINET, and shall include all equipment, material and labor required to furnish the cabinet and install it as described above, complete.

MISCELLANEOUS ELECTRICAL WORK

The Contractor shall perform the following items:

East Peoria

The Contractor shall remove the flasher radio equipment from the existing flasher controller cabinet and relocate it to the proposed lighting controller as shown on the plan sheet.

The Contractor shall remove the radio antenna from the existing flasher post and relocate it to the proposed light pole as shown on the plan sheets.

The Contractor shall furnish new antenna cable, brackets, hardware, and all other items required to relocate the equipment and antenna to the new locations.

The Contractor shall plug the equipment into the equipment receptacle that will be installed as part of the flasher controller pay item.

The Contractor shall aim the antenna towards the City of East Peoria radio communications tower located at the East Peoria Public Safety building 201 West Washington Street, East Peoria, Illinois 61611.

The Contractor shall test the radio system after relocation.

The Contractor shall submit shop drawings for all items for approval prior to ordering any materials.

The Contractor shall verify all field conditions prior to bidding. There will be no additional compensation for this work.

Basis of Payment: This work will be paid for at the contract unit price per Lump Sum for MISCELLANEOUS ELECTRICAL WORK and shall be payment in full for all labor, materials, and equipment required to terminate and splice the fiber optic cable described above, complete.

REMOVE EXISTING ITS EQUIPMENT

This work shall be in accordance with Section 895 of the Standard Specifications except as modified herein.

The Contractor shall remove and dispose of the following items off of the right of way:

| Removal Items | IL 78 (Main) & Vine |
|--|---------------------|
| ELECTRIC CABLE FOR CCTV EQUIPMENT CABINET | ALL |
| 12F SM FIBER OPTIC CABLE FOR CCTV EQUIPMENT CABINET | ALL |
| 48F SM FIBER OPTIC CABLE BACKBONE (REMAINING CABLE NOT USED FOR SLACK FIBER) | ALL |

The Contractor shall remove the following items and deliver it to the IDOT Peoria West Maintenance Facility at 6500 West Highway 150 Edwards, Illinois 61529:

| Removal Items | IL 116 & IL 29 Cedar St. Extension |
|---------------------------------|------------------------------------|
| 55 FT. CCTV CAMERA POLE | 1.0 |
| CCTV EQUIPMENT CABINET COMPLETE | 1.0 |

The above list should represent an accurate listing of removal items; however, it is the Contractor's responsibility to verify all quantities prior to bidding. There will be no additional compensation.

The Contractor shall dispose of all items off the Right-of-Way and reflect the salvage value of the material in the contract bid price.

Basis of Payment: The above work will be paid for at the contract unit price Each for REMOVE EXISTING ITS EQUIPMENT and shall be payment in full for removing and disposing of the equipment described above, complete. No additional compensation will be allowed.

INSTALLATION OF IDOT CONDUIT AND FIBER OPTIC CABLE

The Contractor shall coordinate the installation of the relocated IDOT owned communications duct and fiber optic cable with the Resident Engineer prior to installing any proposed facilities.

The Contractor shall submit JULIE tickets for all areas where the proposed conduit and fiber optic cable will be installed to have the existing utilities marked.

After the existing utilities have been marked, the Contractor shall meet with the Resident Engineer to determine the location and mark the proposed conduit path and communications vault locations based on the locations of the existing utilities and proposed roadway construction.

The Contractor shall ensure that the installation of the proposed conduit and placement of the proposed communications vaults do not interfere with the proposed construction.

The existing fiber optic cable shall remain in operation until the proposed fiber has been installed and is ready to be spliced into the existing fiber.

Splicing of new fiber into existing fiber will only be permitted during the maintenance window of 12AM-5AM in accordance with the requirements contained in the special provision for FIBER OPTIC CABLE 48 FIBERS, SINGLE MODE

Basis of Payment: This work will not be paid for separately but shall be included in the cost of the proposed conduit and fiber optic cable.

HANDHOLE TO BE ADJUSTED WITH NEW FRAME AND COVER

This work shall consist of furnishing the materials and construction to adjust an existing handhole in accordance with the applicable Articles of Section 814, 895, 1020, and 1088 of the Standard Specifications with the following modifications:

CONSTRUCTION REQUIREMENTS

The Contractor shall perform the following items:

- The Contractor shall remove the existing handhole casting, lid, and hardware and dispose of them off the Right-of-Way.
- The Contractor shall remove a minimum of 12" inches of concrete from the existing handhole walls and shall furnish and install a new Handhole casting and lid. The new concrete shall be dowelled into the existing concrete.
- The handhole installation shall be in accordance with the latest revision of Highway Standard 814006.
- The Contractor shall provide protection for the handhole cables during all phases of construction.

- The Contractor shall ground the handhole lids and frame in accordance with NEC requirements using #6 XLP-USE green copper conductor cable which shall be bonded to all items and their associated ground rods utilizing mechanical lugs and bolts. The use of split bolts will not be allowed.

Material Requirements.

- The lift ring for the cover shall consist of a solid closed ring of stainless steel at least 10mm (3/8 inch) in diameter. The lift ring shall be attached to the cover by a loop of stainless-steel at least 10 mm (3/8 inch) in diameter. The lift ring and loop shall be recessed in the cover.
- The lid shall be marked with the legend "Traffic Signals".

Pre-cast handholes are not allowed.

All unsuitable materials shall be disposed of by the Contractor outside the job limits.

Basis of Payment: This work will be paid for at the contract unit price Each for HANDHOLE TO BE ADJUSTED WITH NEW FRAME AND COVER, which price shall be payment in full for all labor, materials, and equipment required to remove the existing handhole casting and lid and rebuild the handhole as described above as well as any necessary excavating, backfilling, disposal of unsuitable materials, and furnishing all materials within the limits of the handhole.

SEEDING, MINOR AREAS

Effective July 1, 1990

Revised January 1, 2007

Seeding, fertilizing, and mulching shall be done in accordance with Article 250 of the Standard Specifications except for the following revisions:

All areas disturbed by the work performed shall be seeded, fertilized, and mulched in accordance with Article 251.03(a). The materials may be purchased locally and placed as directed by the Engineer.

The estimated area is approximately 0.001 acre. The seed mixture shall be applied at 100 pounds/acre (110 kg/ha). The mixture shall be one that contains a high percentage of Kentucky Blue Grass. All seeds shall meet the purity and noxious weed requirements of Article 1081.04 of the Standard Specifications and be approved by the Engineer.

The fertilizer nutrients shall be applied at a rate of 270 lbs. (300 kg) of actual nutrients per acre (hectare). The fertilizer furnished shall be ready mixed material having a ratio of (1-1-1).

The contractor shall provide the engineer with the test results from the seed container and the chemical analysis of the fertilizer nutrients.

The seed, fertilizer, and mulch will not be measured for payment but shall be included in the contract bid price for the pay items for UNDERGROUND CONDUIT of the size and type specified.

RAILROAD PROTECTIVE LIABILITY INSURANCE (BDE)

Effective: December 1, 1986

Revised: January 1, 2022

Description. Railroad Protective Liability and Property Damage Liability Insurance shall be carried according to Article 107.11 of the Standard Specifications. A separate policy is required for each railroad unless otherwise noted.

| NAMED INSURED & ADDRESS | NUMBER & SPEED OF PASSENGER TRAINS | NUMBER & SPEED OF FREIGHT TRAINS |
|--|---|-------------------------------------|
| Tazewell & Peoria Railroad, Inc. (TZPR) 301 Wesley Road Creve Coeur, Illinois 61610 | 0 | 46/day @ 2-10 MPH |
| Class 1 RR (Y or N): N DOT/AAR No.: 862851B RR Division: Northern | RR Mile Post: 1.23 RR Sub-Division: Nickel Plate | |
| For Freight/Passenger Information Contact: Greg Breaston Phone: (904) 999-3378 Email: greg.breaston@gwrr.com | | |
| For Insurance Information Contact: Crystal Galbreath Phone: (904) 596-7782 Email: crystal.galbreath@gwrr.com | | |

Basis of Payment. Providing Railroad Protective Liability and Property Damage Liability Insurance will be paid for at the contract unit price per Lump Sum for RAILROAD PROTECTIVE LIABILITY INSURANCE.

REQUIREMENTS WHEN WORKING WITH THE RAILROAD

Special attention is brought to Division 100 of the "*Standard Specifications for Road and Bridge Construction*" regarding working with the Railroad and the authority of the Railroad Engineer as defined. The Contractor shall make themselves aware of all the rules and regulations the railroad may have regarding, but not limited to, working restrictions, safety training, safety procedures and flagger scheduling and call-off requirements. The Contractor shall also submit, to the Railroad, copies, for review and approval, of the structure demolition plans, beam erection plans and other work plans that may directly impact the Railroad facilities. This submittal shall happen concurrently when submitting to the Department. The cost to comply with any requirements the Railroad may have in order to perform work on this project shall be considered included in the cost of the contract items and no additional compensation will be allowed.

The State will be responsible for payment of engineering review costs incurred by the TZPR for review of Contractor submittals to the TZPR for this contract. The State has entered into a Construction and Maintenance Agreement with TZPR that includes provision for the State to pay TZPR directly for the engineering review costs of Contractor submittals. The Construction and Maintenance Agreement also includes provision for the State to pay TZPR directly for the services of Railroad flaggers as required for protection of railroad traffic in conjunction with the work to be performed by the State or its contractors under this contract, and to pay TZPR directly for costs the TZPR incurs for conducting periodic site inspections.

WORKING RESTRICTIONS

The Contractor shall coordinate with the Tazewell & Peoria Railroad, Inc. (TZPR) to determine allowable times for the removal of the superstructure and Piers 1,2,3,4 and 5 of existing Structure No. 090-0034. The Contractor shall coordinate with TZPR to determine allowable times for the construction of the new bridge piers and placement of new superstructure to determine allowable times for these activities. The TZPR will not make any commitments in advance to provide allowable work windows at specific days or times; however, the TZPR has indicated that their railcar traffic volume has historically been lower on weekends.

No existing guardrail shall be removed until Illinois Route 8/116 is closed to traffic.

TZPR/GWRR RAILROAD COORDINATION AND PERMITTING REQUIREMENTS

All work performed on, above, or adjacent to railroad property shall be in accordance with the Geneseo & Wyoming Company's Public Project Manual, current edition. Work plans shall be submitted for review to the Railroad for all work that presents the potential to affect railroad property or operations. All work plans shall be prepared and submitted to the Railroad in adherence with the Public Project Manual, Section 1.11 Construction Submission Criteria. Per Section 1.08 of the Public Project Manual, reviews of submittals are expected to take 30 to 45 days depending on the level of effort needed. Revised submittals are also expected to take 30 to 45 days for review.

The Contractor will be required to contact Geneseo & Wyoming Real Estate Department for a Right-of-Entry (ROE) application and agreement for work to take place on the Geneseo &

Wyoming Right-of-Way. ROE application and information are located at the following website:

https://www.gwrr.com/real_estate/accessing_property

The Contractor shall notify the Genesee & Wyoming Public Projects Department 30 days prior to starting construction.

Geneseo & Wyoming Flagging Services will be required for all work within Geneseo & Wyoming Right-of-Way or for any work that has a potential to foul.

The Contractor shall not store their materials or equipment on railroad property or where they may potentially interfere with operations unless the Contractor has received prior written permission from the railroad. Any materials or equipment for which the Contractor has received permission from the railroad to store on railroad property shall be stored in accordance with Section 1.10 of the Geneseo & Wyoming Company's Public Project Manual, current edition.

The Contractor must plan and perform the work in a manner such that the railroad tracks at the project location remain fully capable of operating rail traffic throughout the work period and rail traffic is not delayed or otherwise impacted due to the work being performed.

In addition to the requirements of Article 107.10 of the IDOT Standard Specifications for Road and Bridge Construction for Temporary Railroad Grade Crossing, the Contractor is advised that if they desire access across the Railroad's property or tracks at a location other than an existing and open public road crossing in or adjacent to the construction of the project, the Contractor must first obtain the permission of the TZPR and shall execute any applicable contracts as described in Section 1.09 of the Geneseo & Wyoming Company's Public Project Manual.

The Contractor is advised that the TZPR operates a maintenance access road for TZPR vehicular traffic which runs below existing Structure No. 090-0034. The Contractor shall be required to maintain the access road through the Contractor's work areas in such a condition that it is fully capable of operating for TZPR vehicular throughout the duration of the project, as directed by the Railroad Engineer. The access road shall be maintained at a minimum width of 12-feet, with the nearest edge of the access road a minimum of 8-feet from the nearest track centerline. Upon completion of the project, the Contractor shall leave the maintenance access road in a condition satisfactory to the Railroad Engineer. The cost to comply with the requirements for maintaining the access road shall be considered included in the cost of the contract items and no additional compensation will be allowed.

BUILDING REMOVAL (BDE)

Effective: September 1, 1990

Revised: August 1, 2022

Description. This work shall consist of the removal and disposal of building(s), including all foundations, retaining walls, and piers, down to a plane 1 ft. (300 mm) below the ultimate bottom of building elevation or proposed bottom of construction elevation. The building(s) are identified as follows:

| <u>Bldg. No.</u> | <u>Parcel No.</u> | <u>Location</u> | <u>Description</u> |
|------------------|-------------------|--|---|
| 1 | 4ABL011 | 600 South Main Street East Peoria, Illinois 61611 | Commercial garage/shop building with partial upper level constructed in 1945 (Approx. 2,880 SF) |

CONSTRUCTION REQUIREMENTS

General. The IEPA's "State of Illinois Demolition/Renovation/Asbestos Project Notification Form" shall be submitted and a copy sent to the Engineer. It shall be updated if there is a change in the start and/or finish date or if asbestos is found to be present in the building(s) to be removed.

Discontinuance of Utilities. The Contractor shall arrange for the discontinuance of all utility services and the removal of the metering devices that serve the building(s) according to the respective requirements and regulations of the city, county, and utility companies involved. The Contractor shall disconnect and seal the service outlets.

Posting. Upon execution of the contract and prior to the removal of any buildings, the Contractor shall paint or stencil, in contrasting colors of an oil base paint, on all sides of each building or structure, the following posting:

NO TRESPASSING
VIOLATORS WILL BE PROSECUTED

The postings shall be positioned prominently on the structure so they can be easily read and at a sufficient height to prevent defacing.

Any holes, such as basements, shall be backfilled according to Article 502.10.

Basis of Payment. This work will be paid for at the contract lump sum unit price for BUILDING REMOVAL NO. 1.

HIGH LOAD MULTI-ROTATIONAL BEARINGS

Effective: October 13, 1988

Revised: June 28, 2024

Description. This work shall consist of furnishing and installing High Load Multi-Rotational type bearing assemblies at the locations shown on the plans.

High Load Multi-Rotational (HLMR) bearings shall be the type as shown on the plans, which will be one of the following:

- a) Pot Bearings. These bearings shall be manufactured so that the rotational capability is provided by an assembly having a rubber disc of proper thickness, confined in a manner so it behaves like a fluid. The disc shall be installed, with a snug fit, into a steel cylinder and confined by a tight fitting piston. The outside diameter of the piston shall be no more than 0.03 in. (750 microns) less than the inside diameter of the cylinder at the interface level of the piston and rubber disc. The sides of the piston shall be beveled. PTFE sheets, or silicone grease shall be utilized to facilitate rotation of the rubber disc. Suitable brass sealing rings shall be provided to prevent any extrusion between piston and cylinder.
- b) Shear Inhibited Disc Type Bearing. The Structural Element shall be restricted from shear by the pin and ring design and need not be completely confined as with the Pot Bearing design. The disc shall be a molded monolithic Polyether Urethane compound.

These bearings shall be further subdivided into one or more of the following classes:

- 1) Fixed. These allow rotation in any direction but are fixed against translation.
- 2) Guided Expansion. These allow rotation in any direction but translation only in limited directions.
- 3) Non-Guided Expansion. These allow rotation and translation in any direction.

Suppliers: The Department maintains a pre-qualified list of proprietary structural systems allowed for High Load Multi-Rotational Bearings. This list can be found on the Departments web site under Prequalified Structural Systems. The Contractor's options are limited to those systems pre-qualified by the Department on the date that the contract was bid. These systems have been reviewed for structural feasibility and adequacy only. Presence on this list shall in no case relieve the Contractor of the site-specific design or QC/QA requirements stated herein.

The supplier shall notify the Department at least two weeks in advance of fabrication of the fabrication shop address. The fabricator shall provide evidence of current certification by AISC according to Article 106.08(e) of the Standard Specifications.

The overall depth dimension for the HLMR bearings shall be as specified on the plans. The horizontal dimensions shall be limited to the available bearing seat area.

Any modifications required to accommodate the bearings chosen shall be submitted to the Engineer for approval prior to ordering materials. Modifications may include the addition of steel filler plates or the adjustment of beam seat elevations. Adjustments to bridge seat elevations and accompanying reinforcement details shall be approved by the Structural Engineer of Record. Modifications required shall be made at no additional cost to the State. Inverted bearing or center-guided bearing configurations will not be permitted.

The Contractor shall comply with all manufacturer's material, fabrication and installation requirements specified.

Submittals. Shop drawings shall be submitted to the Engineer for approval according to Article 105.04 of the Standard Specifications. All steel filler plate details shall be included in the shop drawings. In addition the Contractor shall furnish certified copies of the bearing manufacturer's test reports on the physical properties of the component materials for the bearings to be furnished and a certification by the bearing manufacturer stating the bearing assemblies furnished conform to all the requirements shown on the plans and as herein specified. Submittals with insufficient test data and supporting certifications will be rejected.

Materials. The materials for the HLMR bearing assemblies shall be according to the following:

- (a) Elastomeric Materials. The rubber disc for Pot bearings shall be according to Article 1083.02(a) of the Standard Specifications.
- (b) Polytetrafluoroethylene (PTFE) Material. The PTFE material shall be according to Article 1083.02(b) of the Standard Specifications, except that it shall be dimpled lubricated with a maximum coefficient of friction of 0.02 on stainless steel. The dimpled and lubricated PTFE surface shall comply with AASHTO 14.7.2. The friction requirement shall be as specified in the Long Term Deterioration Test required for prequalification and the Sliding Friction Test as specified below.
- (c) Stainless Steel Sheets. The stainless steel sheets shall be of the thickness specified and shall be according to Article 1083.02(c).
- (d) Structural Steel. All structural steel used in the bearing assemblies shall be according to AASHTO M 270, Grade 50 (M 270M Grade 345), unless otherwise specified.
- (e) Threaded studs. The threaded stud, when required, shall conform to the requirements of Article 1083.02(d)(4) of the Standard Specifications.

- (f) Polyether Urethane for Disc bearings shall be according to all of the following requirements:

| PHYSICAL PROPERTY | ASTM TEST METHOD | REQUIREMENTS | |
|---|------------------|--------------------------|--------------------------|
| Hardness, Type D durometer | D 2240 | 45 Min | 65 Max |
| Tensile Stress, psi (kPa) At 100% elongation, min | D 412 | 1500 psi (10,350 kPa) | 2300 psi (15,900 kPa) |
| Tensile Stress, psi (kPa) At 200% elongation, min | D 412 | 2800 psi (19,300 kPa) | 4000 psi (27,600 kPa) |
| Tensile Strength, psi (kPa), min | D 412 | 4000 psi (27,600 kPa) | 6000 psi (41,400 kPa) |
| Ultimate Elongation, %, min | D 412 | 350 | 220 |
| Compression Set 22 hr. at 158 °F (70 °C), Method B %, max | D 395 | 40 | 40 |

The physical properties for a durometer hardness between the minimum and maximum values shown above shall be determined by straight line interpolation.

Design. The HLMR bearings shall be of the type and class specified and designed for the loads shown on the plans. Bearing details shown on the contract plans are a schematic representation of the bearing. Actual design of the bearing shall be by the supplier according to:

- the exact parameters specified in the Design Data table noted on the bridge plans,
- the appropriate AASHTO LRFD Bridge Design Specifications, and
- the IDOT Bridge Manual.

The design of the masonry and sole bearing plates are based on detail assumptions which may require modifications depending on the supplier chosen by the Contractor.

Fabrication. The bearings shall be complete factory-produced assemblies. They shall provide for rotation in all directions and for sliding, when specified, in directions as indicated on the plans. All bearings shall be furnished as a complete unit from one manufacturing source. All material used in the manufacture shall be new and unused with no reclaimed material incorporated into the finished assembly.

The translation capability for both guided and non-guided expansion bearings shall be provided by means of a polished stainless steel sliding plate that bears on a PTFE sheet bonded and recessed to the top surface of the piston or disc. The sliding element of expansion bearings shall be restrained against movement in the fixed direction by exterior guide bars capable of resisting the horizontal forces or 20 percent of the vertical design load on the bearing applied in any direction, whichever is greater. The sliding surfaces of the guide bar shall be of PTFE sheet and stainless steel. Guiding off of the fixed base, or any extension of the base, will not be permitted.

Structural steel plates shall be fabricated according to Article 505.04(l) of the Standard Specifications. Prior to shipment the exposed edges and other exposed portions of the structural steel plates shall be cleaned and given a corrosion protection coating as specified on the plans and according to the applicable Special Provisions and Articles 506.03 and 506.04 of the Standard Specifications. During cleaning and coating the stainless steel, PTFE sheet and neoprene shall be protected from abrasion and coating material.

PTFE sheets shall be bonded to steel under factory controlled conditions using heat and pressure for the time required to set the epoxy adhesive used. The PTFE sheet shall be free from bubbles and the sliding surface shall be burnished to an absolutely smooth surface.

The steel piston and the steel cylinder for pot bearings shall each be machined from a solid piece of steel. The steel base cylinder shall be either integrally machined, recessed into with a snug fit, or continuously welded to its steel masonry plate. If the sole plate and piston are not one piece, the piston shall be recessed $\frac{3}{8}$ inch into the sole plate.

If the bottom disc plate or base cylinder is recessed into the masonry plate, the designed thickness of the masonry plate shall take into account the depth of the recess. If the top disc plate is recessed into the sole plate, the designed thickness of the sole plate shall take into account the depth of the recess.

The shear resisting mechanism shall be machined from a solid piece of steel. Connection of the shear resisting mechanism to top and bottom disc plate shall be determined by the bearing fabricator.

Packaging. Each HLMR bearing assembly shall be fully assembled at the manufacturing plant and delivered to the construction site as complete units. The assemblies shall be packaged, crated or wrapped so the assemblies will not be damaged during handling, transporting and shipping. The bearings shall be held together with removable restraints so sliding surfaces are not damaged.

Centerlines shall be marked on both masonry and sole plates for alignment in the field. The bearings shall be shipped in moisture-proof and dust-proof covers.

Performance Testing. The following performance tests are required per lot on the project. A lot size shall be the number of bearings per class (fixed, guided expansion, non-guided expansion) on the project, but not to exceed 25 bearings per class. When multiple sizes of bearings are used on the same contract, they shall be grouped by class when determining lot sizes and amount of bearings to be tested. All tests shall be performed by the manufacturer prior to shipment.

Dimension Check. Each bearing shall be checked dimensionally to verify all bearing components are within tolerances. Failure to satisfy any dimensional tolerance shall be grounds for rejecting the bearing component or the entire bearing assembly.

Clearance Test. This test shall be performed on one bearing per lot. The bearing selected for this test shall be the one with the least amount of clearance based on the dimension check. The bearing assembly shall be loaded to its service limit state rated capacity at its full design rotation but not less than 0.02 radians to verify the required clearances exist. This test shall be performed twice for each bearing with the rotation oriented longitudinally with the bridge once in each direction. Any visual signs of rubbing or binding shall be grounds for rejection of the lot.

Proof Load Test. This test shall be performed on one bearing per lot. The bearing assembly shall be load tested to 150 percent of the service limit state rated capacity at a rotation of 0.02 radians. The load shall be maintained for 5 minutes, removed then reapplied for 5 minutes. If the load drops below the required value during either application, the test shall be restarted from the beginning. This test shall be performed twice for each bearing with the rotation oriented longitudinally with the bridge once in each direction.

The bearing shall be visually examined both during the test and upon disassembly after the test. Any resultant visual defects include, but are not limited to:

1. Extruded or deformed elastomer, polyether urethane, or PTFE.
2. Insufficient clearances such as evidence of metal to metal contact between the pot wall and the top or sole plate.
3. Damaged components such as cracked steel, damaged seal rings, or damaged limiting rings.
4. Bond failure.

If any of the above items are found it shall be grounds for rejection of the lot.

Sliding Friction Test. For expansion bearings, this test shall be performed on one bearing per lot. The sliding surfaces shall be thoroughly cleaned with a degreasing solvent. No lubrication other than that specified for the bearing shall be used. The bearing shall be loaded to its service limit state rated capacity for 1 hour prior to and throughout the duration of the sliding test. At least 12 cycles of plus and minus sliding with an amplitude equaling the smaller of the design displacement and 1 inch (25 mm) shall then be applied. The average sliding speed shall be between 0.1 inch and 1.0 inches (2.5 mm and 25 mm) per minute. The sliding friction coefficient shall be computed for each direction of each cycle and its mean and standard deviation shall be computed for the sixth through twelfth cycles.

The friction coefficient for the first movement and the mean plus two standard deviations for the sixth through twelfth cycles shall not exceed the design value used. In addition, the mean value for the sixth through twelfth cycles shall not exceed 2/3 of the design value used. Failure of either of these shall result in rejection of the lot.

The bearing shall also be visually examined both during and after the testing, any resultant defects, such as bond failure, physical destruction, or cold flow of the PTFE shall also be cause for rejection of the lot.

The Contractor shall furnish a notarized certification from the bearing manufacturer stating the HLMR bearings have been performance tested as specified, and a. purchase order prior to fabrication. The purchase order shall contain, as a minimum, the quantity and size of each type of bearing furnished. The notarized certifications and the purchase order shall be submitted in one package to the Engineer of Tests at the Bureau of Materials and Physical Research (126 East Ash Springfield, IL 62704). The Department reserves the right to perform any of the specified tests on one or more of the furnished bearings. If the tested bearing shows failure it shall be replaced and the remaining bearings shall be similarly tested for acceptance at the Contractor's expense.

The manufacturer shall furnish samples of component materials used in the bearings, for testing by the Department, to the Engineer of Tests at the Bureau of Materials and Physical Research (126 East Ash Springfield, IL 62704). The required components shall be those components of HLMR bearings that are consistent with elastomeric bearing components according to Article 1083.04 of the Standard Specifications.

Installation. The HLMR bearings shall be erected according to Article 521.05 of the Standard Specifications.

Exposed edges and other exposed portions of the structural steel plates shall be field painted as specified for Structural Steel.

Basis of Payment. This work will be paid for at the contract unit price each for HIGH LOAD MULTI-ROTATIONAL BEARINGS, POT , FIXED; HIGH LOAD MULTI-ROTATIONAL BEARINGS, POT, GUIDED EXPANSION; HIGH LOAD MULTI-ROTATIONAL BEARINGS, POT, NON-GUIDED EXPANSION; HIGH LOAD MULTI-ROTATIONAL BEARINGS, DISC, FIXED; HIGH LOAD MULTI-ROTATIONAL BEARINGS, DISC, GUIDED EXPANSION; or HIGH LOAD MULTI-ROTATIONAL BEARINGS, DISC, NON-GUIDED EXPANSION of the load capacity specified.

When the fabrication and erection of HLMR bearings is accomplished under separate contracts, the applicable requirements of Article 505.09 shall apply.

Fabricated HLMR bearings and other materials complying with the requirements of this item, furnished and accepted, will be paid for at the contract unit price each for FURNISHING HIGH LOAD MULTI-ROTATIONAL BEARINGS, POT, FIXED; FURNISHING HIGH LOAD MULTI-ROTATIONAL BEARINGS, POT, GUIDED EXPANSION; FURNISHING HIGH LOAD MULTI-ROTATIONAL BEARINGS, POT, NON-GUIDED EXPANSION; FURNISHING HIGH LOAD MULTI-ROTATIONAL BEARINGS, DISC, FIXED; FURNISHING HIGH LOAD MULTI-ROTATIONAL BEARINGS, DISC, GUIDED EXPANSION; or FURNISHING HIGH LOAD MULTI-ROTATIONAL BEARINGS, DISC, NON-GUIDED EXPANSION of the load capacity specified.

Storage and care of fabricated HLMR bearings and other materials complying with the requirements of this item by the Fabrication Contractor beyond the specified storage period, will be paid for at the contract unit price per calendar day for STORAGE OF HIGH LOAD MULTI-ROTATIONAL BEARINGS if a pay item is provided for in the contract, or will be paid for according to Article 109.04 if a pay item is not provided in the contract.

HLMR bearings and other materials fabricated under this item erected according to the requirements of the specifications, and accepted, will be paid for at the contract unit price each for ERECTING HIGH LOAD MULTI-ROTATIONAL BEARINGS, POT, FIXED; ERECTING HIGH LOAD MULTI-ROTATIONAL BEARINGS, POT, GUIDED EXPANSION; ERECTING HIGH LOAD MULTI-ROTATIONAL BEARINGS, POT, NON-GUIDED EXPANSION; ERECTING HIGH LOAD MULTI-ROTATIONAL BEARINGS, DISC, FIXED; ERECTING HIGH LOAD MULTI-ROTATIONAL BEARINGS, DISC, GUIDED EXPANSION; or ERECTING HIGH LOAD MULTI-ROTATIONAL BEARINGS, DISC, NON-GUIDED EXPANSION of the load capacity specified.

MODULAR EXPANSION JOINT

Effective: May 19, 1994

Revised: October 27, 2023

Description. This work shall consist of furnishing and installing a modular expansion joint(s) as shown on the plans, and according to applicable portions of Section 520 of the Standard Specifications.

General. The expansion joint device shall be capable of handling the specified longitudinal movement. In addition, when specified, the joint shall also be capable of handling the differential non-parallel longitudinal movement. The expansion joint device shall effectively seal the joint opening in the deck surface and barrier curbs against the entrance of water and foreign materials. There shall be no appreciable change in the deck surface plane with the expansion and contraction movements of the bridge.

The device shall consist of a shop-fabricated modular assembly of transverse elastomeric seals, edge and center beams, bearing on support bars spanning the joint opening. The assembly shall maintain equal distances between intermediate support rails, at any cross section, for the entire length of the joint. The assembly shall be stable under all conditions of expansion and contraction.

The noise level of the joint in service shall meet all Federal and State of Illinois noise requirements.

At sidewalks, concrete median barriers and concrete parapet joints, a sliding steel plate shall be fabricated and installed according to the plans. Painting or galvanizing of sliding steel plates shall be as specified on the plans.

Suppliers: The Department maintains a pre-qualified list of proprietary structural systems allowed for modular expansion joints. This list can be found on the Departments web site under Prequalified Structural Systems. The Contractor's options are limited to those systems pre-qualified by the Department on the date that the contract was bid. These systems have been reviewed for structural feasibility and adequacy only. Presence on this list shall in no case relieve the Contractor of the site-specific design or QC/QA requirements stated herein.

The supplier shall notify the Department at least two weeks in advance of fabrication of the fabrication shop address. The fabricator shall provide evidence of current certification by AISC according to Article 106.08(e) of the Standard Specifications.

Submittals: Shop drawings and a copy of the calculations and support documents shall be submitted to the Engineer for approval according to Article 105.04 of the Standard Specifications. Calculations shall be sealed by an Illinois Licensed Professional Engineer. Submittals will be required for each modular expansion joint device specified. In addition, the Contractor shall provide the Department with a certification of compliance by the manufacturer listing all materials in the system. The certification shall attest that the system conforms to the design requirements, material requirements, and that all components of the joint are the same as what was included in the prequalification submittal that was successfully tested in the OMV, seal push out, and fatigue tests of Section 19, Appendix 19, Article 5.1, 5.2, and 5.3 of the AASHTO LRFD Bridge Construction Specifications. Submittals with insufficient test data and supporting certifications will be rejected.

The shop drawings shall include tables showing the total anticipated movements for each joint and the required setting width of the joint assemblies at various temperatures.

The shop drawings shall include installation drawings or details showing locations and details of temporary installation supports, and joint assembly components, in relation to the adjacent primary structural beams, girders, or members. These details shall demonstrate that the proposed modular expansion joint is designed to fit and operate around all primary structural members within the space provided on the contract plans.

Fabrication: Fabricators of the modular expansion device(s) are required to meet the following tolerances:

| | |
|---|---|
| Allowable variation in straightness of center beam rails Length < 30' Length 30' to 45' Length > 45' | 1/8" per 10' total length 3/8" $3/8" + 1/8" * (\text{total length (feet)} - 45') / 10'$ |
| Allowable lateral variation in specified location of support boxes | $\pm 1/4"$ |
| Allowable lateral variation in specified location of stirrup or other attachments to center beam | $\pm 1/16"$ |
| Allowable variation in total depth | $\pm 1/8"$ |
| Allowable vertical dimension variation of all components | $\pm 1/8"$ |
| Allowable variation from specified elevation end squareness or skew | $\pm 1/8"$ |
| Allowable variation in overall length of joint | $\pm 1"$ |

Metallic attachments used to secure elastomeric seals to centerbeams, if welded to the centerbeams and edge beams, shall be welded continuously along either their top or bottom edges.

Run off tabs shall be used for stirrup or other attachments to the center beam full penetration welds.

Design Requirements: The maximum vertical, transverse and horizontal rotations and displacements shall be defined and included in the design.

The expansion joint device(s) shall be designed, detailed and successfully tested, according to Section 14 of the AASHTO LRFD Bridge Design Specifications.

The design forces used for centerbeam to support bar analysis shall be taken at the centerline of the centerbeam.

The maximum fatigue resistance of any detail shall not exceed that associated with the fatigue category prescribed in the table below.

| Type of Detail | Maximum Permitted Category |
|---|----------------------------|
| Welded Multiple Centerbeam to Support Bar Connections | C |
| Weld Stirrup Attachments for Single Support Bar Systems | C |
| Bolted Stirrup Attachments for Single Support Bar Systems | D |
| Groove Welded Centerbeam Splices | B |
| Miscellaneous Welded Connections ¹ | C |
| Miscellaneous Bolted Connections | D |

¹Miscellaneous connections include attachments for equidistant devices and any metallic attachments to the centerbeams or edge beams that are used to secure the elastomeric seals.

In addition, expansion joint device(s) shall be designed for the vehicular live load as specified on the General, Plan, and Elevation sheet of the plans across the entire width of the structure.

Top, bottom and sides of support bars shall be restrained to prevent uplift, transmit bearing loads, and maintain the lateral position of the bars.

The total service movement of each individual sealing element shall not exceed 3 in. (75 mm).

The joint supplier shall design, layout, and detail the modular expansion joint assembly and components to miss existing or proposed structural beams, girders, or members. Cutting of structural members to install joint assemblies shall not be permitted.

Materials:

- (a) Metals. Structural Steel. All structural steel, except stainless steel, shall be according to AASHTO M 270, Grade 50 or 50W (M 270M Grade 345), unless otherwise specified. All structural steel, except stainless steel, shall be hot-dip galvanized according to ASTM A123 or A153 as applicable.

Stainless steel sheets for the sliding surfaces of the support bars shall conform to the requirements of ASTM A240 (A240M) type 302 or 304. Stainless steel mating surfaces shall require a No. 8 finish. For non-mating surfaces a 2B finish is required.

The use of aluminum components in the modular joint will not be allowed.

- (b) Preformed Elastomeric Seals. The elastomeric sealing element shall be according to ASTM D5973.

Lubricant/Adhesive for installing the preformed elastomeric elements in place shall be a one-part, moisture-curing, polyurethane and hydrocarbon solvent mixture as recommended by the manufacturer and containing not less than 65 percent solids.

- (c) Support Bar Bearings. Support bar bearings shall be fabricated from elastomeric pads with polytetrafluorethylene (PTFE) surfacing or from polyurethane compound with PTFE sliding surfaces. The elastomeric and PTFE materials shall meet the requirements of Section 1083 of the Standard Specifications.

- (d) Support Bars. Support bars shall incorporate stainless steel sliding surfaces to permit joint movement.

Construction Requirements

General. Installation of expansion devices shall be according to the plans and shop drawings.

The fabricator of the modular joint assembly shall be AISC certified according to Article 106.08 for Bridge and Highway Metal Component Manufacturers. In lieu of AISC certification, the Contractor may have all welding on main members (support bars and center beams) observed and inspected by independent (third party) personnel at the Contractor's expense. Welding shall then be observed by a Certified Welding Inspector (CWI) in addition to the manufacturer's own welding inspection. Third-party Non-Destructive Examination (NDE) shall be performed by inspector(s), certified as level II in applicable methods, and all complete penetration beam-to-bar welds and butt joints in beams shall be UT inspected and 10 percent of fillet and partial pen welds shall be MT inspected.

The manufacturer of the expansion device shall provide a qualified technical service representative to supervise installation. Modular expansion joint devices shall be factory prefabricated assemblies, preset by the manufacturer prior to shipment with provisions for field adjustment for the ambient temperature at the time of installation.

Unless otherwise shown on the plans, the neoprene seals shall be continuous without any field splices. Installation of the joint seals shall be performed by a trained representative of the Manufacturer.

The metal surfaces in direct contact with the neoprene seals shall be blast cleaned to permit a high strength bond of the lubricant/adhesive between the neoprene seal and mating metal surfaces.

The Contractor shall anticipate and make all necessary adjustments to existing or plan-specified reinforcement bars, subject to the approval of the Engineer, in order to prevent interferences with placement of the selected joint in the structure. Any adjustments to reinforcement bars interfering with the joint installation shall be the responsibility of the Contractor and preapproved by the Engineer prior to installation of the joint. Cutting of reinforcement shall be minimized, and any bars that are cut shall be replaced in-kind at no additional cost.

The prefabricated joint assembly shall be properly positioned and attached to the structure according to the manufacturer's approved shop drawings. The attachment shall be sufficiently rigid to prevent non-thermal rotation, distortion, or misalignment of the joint system relative to the deck prior to casting the concrete. The joints shall be adjusted to the proper opening based on the ambient temperature at the time of installation and then all restraints preventing thermal movement shall be immediately released and/or removed. The joint upturn may be recessed 1 inch into the parapet to allow for lateral adjustment. The joint assembly units shall be straight, parallel and in proper vertical alignment or reworked until proper adjustment is obtained prior to casting of the concrete around the joint.

After the joint system is installed, the joint area shall be flooded with water and inspected, from below for leakage. If leakage is observed, the joint system shall be repaired, at the expense of the Contractor, as recommended by the manufacturer and approved by the Engineer.

Method of Measurement. This work will be measured for payment in place, in feet (meters), along the centerline of the joint. All sliding plate assemblies at the sidewalks, parapets and median barriers will not be measured for payment. The size will be defined as the specified longitudinal movement rounded up to the nearest 3 inch (75 mm) increment.

Basis of Payment: When only a longitudinal movement is specified, this work will be paid for at the contract unit price per foot (meter) for the MODULAR EXPANSION JOINT, of the size specified. When a differential non-parallel movement is also specified, this work will be paid for at the contract unit price per foot (meter) for the MODULAR EXPANSION JOINT-SWIVEL, of the size specified.

All materials, equipment and labor required to fabricate, paint and install the sliding plate assemblies at the sidewalks, parapets and median barriers will not be paid for separately but shall be included in the price for the expansion joint specified.

When the fabrication and erection of modular expansion joint is accomplished under separate contracts, the applicable requirements of Article 505.09 shall apply, except the furnishing pay items shall include storage and protection of fabricated materials up to 75 days after the completion dates.

Fabricated modular expansion joints and other materials complying with the requirements of this item, furnished and accepted, will be paid for at the contract unit price per foot (meter) for FURNISHING MODULAR EXPANSION JOINT or FURNISHING MODULAR EXPANSION JOINT – SWIVEL of the size specified.

Storage and care of fabricated joints and other materials complying with the requirements of this item by the Fabrication Contractor beyond the specified storage period, will be paid for at the contract unit price per calendar day for STORAGE OF MODULAR EXPANSION JOINTS if a pay item is provided for in the contract, or will be paid for according to Article 109.04 if a pay item is not provided in the contract.

Modular expansion joints and other materials erected according to the requirements of the specifications, and accepted, will be paid for at the contract unit price per foot (meter) for ERECTING MODULAR EXPANSION JOINT or ERECTING MODULAR EXPANSION JOINT - SWIVEL of the size specified.

ERECTION OF CURVED STEEL STRUCTURES

Effective: June 1, 2007

Description: In addition to the requirements of Article 505.08(e), the following shall apply.

The Contractor or sub-Contractor performing the erection of the structural steel is herein referred to as the Erection Contractor.

Erection Plan: The Erection Contractor shall retain the services of an Illinois Licensed Structural Engineer, experienced in the analysis and preparation of curved steel girder erection plans, for the completion of a project-specific erection plan. The structural engineer, herein referred to as the Erection Engineer, shall sign and seal the erection plan, drawings, and calculations for the proposed erection of the structural steel.

The erection plan shall be complete in detail for all phases, stages, and conditions anticipated during erection. The erection plan shall include structural calculations and supporting documentation necessary to completely describe and document the means, methods, temporary support positions, and loads necessary to safely erect the structural steel in conformance with the contract documents and as outlined herein. The erection plans shall address and account for all items pertinent to the steel erection including such items as sequencing, falsework, temporary shoring and/or bracing, girder stability, crane positioning and movement, means of access, pick points, girder shape, permissible deformations and roll, interim/final plumbness, cross frame/diaphragm placement and connections, bolting and anchor bolt installation sequences and procedures, and blocking and anchoring of bearings. The Erection Contractor shall be responsible for the stability of the partially erected steel structure during all phases of the steel erection.

The erection plans and procedures shall be submitted to the Engineer for review and acceptance prior to starting the work. Review, acceptance and/or comments by the Department shall not be construed to guarantee the safety or final acceptability of the work or compliance with all applicable specifications, codes, or contract requirements, and shall neither relieve the Contractor of the responsibility and liability to comply with these requirements, nor create liability for the Department. Significant changes to the erection plan in the field must be approved by the Erection Engineer and accepted by the Engineer for the Department.

Basis of Payment: This work shall not be paid for separately but shall be included in the applicable pay items according to Article 505.13 of the Standard Specifications.

DIAMOND GRINDING AND SURFACE TESTING BRIDGE SECTIONS

Effective: December 6, 2004

Revised: April 15, 2022

Description. This work shall consist of diamond grinding and surface testing bridge sections.

The bridge section shall consist of the bridge deck plus the bridge approach slab and pavement connector, if present, at each end of the bridge.

Equipment. Equipment shall be according to the following.

- (a) Diamond Grinder. The diamond grinder shall be a self-propelled planing machine specifically designed for diamond saw grinding. It shall be capable of accurately establishing the profile grade and controlling the grinding cross slope. It shall also have an effective means for removing excess material and slurry from the surface and for preventing dust from escaping into the air. The removal of slurry shall be continuous throughout the grinding operation. The slurry shall be disposed of according to Article 202.03.

The grinding head shall be a minimum of 4 ft. (1.2 m) wide and the diamond saw blades shall be gang mounted on the grinding head at a rate of 50 to 60 blades / ft. (164 to 197 blades/m).

- (b) Surface Testing Equipment. Required surface testing and analysis equipment and their jobsite transportation shall be provided by the Contractor. The Profile Testing Device shall be according to Illinois Test Procedure 701 except the trace analysis shall be based on traces from bridge sections.

CONSTRUCTION REQUIREMENTS

General. After all components have been properly cured, the bridge section shall be ground over its entire length and over a width that extends to within 2 ft. (600 mm) of the curbs or parapets. Grinding shall be done separately before any saw cut grooving, and no concurrent combination of the two operations will be permitted. Whenever possible, each subsequent longitudinal grinding pass shall progress down the cross slope from high to low. The maximum thickness removed shall be 1/4 inch (6 mm); however, when the bridge deck thickness noted on the plans can be maintained, as a minimum, additional removal thickness may be permitted.

The grinding process shall produce a pavement surface that is true in grade and uniform in appearance with longitudinal line-type texture. The line-type texture shall contain corrugations parallel to the outside pavement edge and present a narrow ridge corduroy type appearance. The peaks of the ridges shall be 1/8-inch +/- 1/16-inch (3 mm +/- 1.5 mm) higher than the bottom of the grinding with evenly spaced ridges. It shall be the Contractor's responsibility to select the actual number of blades per foot (meter) to be used to provide the proper surface finish for the aggregate type and concrete present on the project within the limits specified above.

The vertical difference between longitudinal passes shall be 1/8 inch (3 mm) maximum. The grinding at the ends of the bridge section shall be diminished uniformly at a rate of 1:240 over the pavement connectors.

Grinding shall be continuous through all joints. All expansion joints and bridge components under the joints shall be protected from damage or contact with the grinding slurry.

Surface Testing. The diamond ground bridge section shall be surface tested in the presence of the Engineer prior to opening to traffic.

A copy of the approval letter and recorded settings from the Profile Equipment Verification (PEV) Program shall be submitted to the Engineer prior to testing.

The Contractor shall notify the Engineer a minimum of 24 hours prior to commencement of measurements. All objects and debris shall be removed from the bridge section surface prior to testing. During surface testing, joint openings may be temporarily filled with material approved by the Engineer.

Profiles shall be taken in both wheel paths of each lane, 3 ft. (1 m) from, and parallel to, the planned lane lines.

The profile report shall have stationing indicated every 500 ft. (150 m) at a minimum. The profile report shall include the following information: contract number, structure number, beginning and ending stationing, which lane was tested, direction of travel on the trace, date of collection, time of collection, ambient air temperature at time of collection, and the device operator name(s). The data file created from the testing will be submitted to the Engineer and the Bureau of Research for analysis. The file shall be in a format that is compatible with ProVAL software (ERD, PPF).

Trace Reduction and Bump Locating Procedure. All traces shall be reduced using ProVal. This software shall calculate the Mean International Roughness Index (MRI) in inches/mile (mm/km) and indicate any areas of localized roughness in excess of 200 inches/mile (3105 mm/km) on a continuous 25 feet (8 meters) basis.

The average MRI and locations with deviations exceeding the 200 inches/mile (3105 mm/km) limit will be recorded on the Profile Report for Bridge Deck Smoothness.

All ProVAL files shall be provided to the Engineer within two working days of completing the testing. Bureau of Construction Form BC 2450 shall be provided to the Engineer. An example Form BC 2450 is attached. All files shall contain serial numbers for the vehicle and profiling equipment, the approved settings from the PEV program. The Engineer will compare these settings with the approved settings from the PEV Program. If the settings do not match, the results will be rejected and the section shall be retested/reanalyzed with the appropriate settings.

Corrective Actions. Within the bridge section, all deviations in excess of 200 inches/mile (1575 mm) within any continuous length of 25 ft. (8 m) shall be corrected. Correction of deviations shall not result in the deck thickness being less than the minimum. Where corrective work is performed, the bridge section shall be retested to verify that corrections have produced a MRI of 200 inch/mile (3105 mm/km) within an continuous length of 25 ft (8 m) or less for each lane. The Contractor shall furnish and Form BC 2450 the ProVAL files to the Engineer and the Bureau of Research within two working days after any corrections are made.

Corrective actions shall be performed at no additional cost to the department.

The Engineer may perform profile testing on the surface at any time for monitoring and comparison purposes.

Method of Measurement. This work will be measured for payment in place and the area computed in square yards (square meters) of diamond grinding performed.

Basis of Payment. This work will be paid for at the contract unit price per square yard (square meter) for DIAMOND GRINDING (BRIDGE SECTION).

Instructions for Completing Bridge Deck Smoothness Assessment Summary ALR

This form shall be prepared and submitted, along with the raw data files, to the Engineer.

Report Type:

Initial – Testing of bridge section prior to any smoothness grinding.

Intermediate – After initial pass of smoothness grinding has been completed.

Final – All smoothness grinding has been completed.

Other information:

Submission Date – Date in which it has been submitted to the Engineer

Project Type – New Deck, Microsilica Overlay, Latex Overlay, Fly Ash Overlay

Specification Effective Date – revision date of the specification in the contract

Begin ALR Section 1 – beginning station of ALR finding

End ALR Section 1 – end station of ALR finding

Distance – End ALR minus the Begin ALR station number

MRI – The value of the ALR at that location.



Bridge Deck Smoothness Assessment Summary

Areas of Localized Roughness

| This worksheet is intended as a reference for documenting Areas of Localized Roughness (ALR) as described in GBSP-59. | | | | | |
|---|------------------------|------------------------|--|---------------|-------------|
| Contract Information | | Contact Info | | | |
| Contract | 60111 | IDOT RE Name | Jerry Jones | | |
| District | 1 | IDOT RE E-Mail | Jerry.Jones2@illinois.gov | | |
| Letting Date | 1/15/2022 | IDOT RE Phone | 217-555-4183 | | |
| Item # | 26 | Contractor Rep. Name | Bob Builder | | |
| Route | IL 164 | Contractor Rep. E-Mail | Bob.Builder@RTBRConstr.com | | |
| Report Type (Initial or Post Grinding) | Initial | Contractor Rep. Phone | 217-555-2822 | | |
| Lane | Driving | General Comments | | | |
| Direction | Eastbound | | | | |
| Begin Station | 13+45.00 | | | | |
| End Station | 14+65.00 | | | | |
| Contractor | Bob the Bridge Builder | | | | |
| Submission Date | 4/1/2022 | | | | |
| Overlay Type | Microsilica | | | | |
| Specification Effective Date | 1/1/2022 | | | | |
| Begin ALR Section 1 | 13+56.00 | | | Distance (ft) | MRI (in/mi) |
| End ALR Section 1 | 13+64.20 | | | 8.2 | 256.40 |
| Begin ALR Section 2 | 14+04.60 | 1.4 | 278.90 | | |
| End ALR Section 2 | 14+06.00 | | | | |
| Begin ALR Section 3 | | | | | |
| End ALR Section 3 | | | | | |
| Begin ALR Section 4 | | | | | |
| End ALR Section 4 | | | | | |
| Begin ALR Section 5 | | | | | |
| End ALR Section 5 | | | | | |
| Begin ALR Section 6 | | | | | |
| End ALR Section 6 | | | | | |
| Begin ALR Section 7 | | | | | |
| End ALR Section 7 | | | | | |
| Begin ALR Section 8 | | | | | |
| End ALR Section 8 | | | | | |
| Begin ALR Section 9 | | | | | |
| End ALR Section 9 | | | | | |
| Begin ALR Section 10 | | | | | |
| End ALR Section 10 | | | | | |

STRUCTURAL ASSESSMENT REPORTS FOR CONTRACTOR'S MEANS AND METHODS

Effective: March 6, 2009

Revised October 5, 2015

Description. This item shall consist of preparing and submitting, to the Engineer for approval, Structural Assessment Reports (SARs) for proposed work on structure(s) or portions thereof. Unless noted otherwise, a SAR shall be required when the Contractor's means and methods apply loads to the structure or change its structural behavior. A SAR shall be submitted and approved prior to beginning the work covered by that SAR. Separate portions of the work may be covered by separate SARs which may be submitted at different times or as dictated by the Contractor's schedule.

Existing Conditions. An Existing Structure Information Package (ESIP) will be provided by the Department to the Contractor upon request. This package will typically include existing or "As-Built" plans, and the latest National Bridge Inspection Standards (NBIS) inspection report. The availability of structural information from the Department is solely for the convenience and information of the Contractor and shall not relieve the Contractor of the duty to make, and the risk of making, examinations and investigations as required to assess conditions affecting the work. Any data furnished in the ESIP is for information only and does not constitute a part of the Contract. The Department makes no representation or warranty, express or implied, as to the information conveyed or as to any interpretations made from the data.

Removal SARs. A SAR for removal of existing structures, or portions thereof, shall demonstrate that the Contractor's proposed means and methods to accomplish the work do not compromise the structural adequacy of the bridge, or portions thereof that are to remain in service, at any time during the work activities being performed. Each phase of the operation shall be accounted for, as well as the existing condition of the structure.

Construction SARs. A SAR for new construction or for construction utilizing existing components shall demonstrate that the Contractor's proposed means and methods to accomplish the work do not compromise the structural adequacy of the bridge or portions thereof at any time during the work activities being performed. For construction activities applying less than 10 tons (9 metric tons) of total combined weight of equipment and stockpiled materials on the structure at any one time, a SAR submittal shall not be required provided the Contractor submits written verification to the Engineer stating the applied loads do not exceed this threshold. The verification shall be submitted prior to the start of the activity. This SAR exemption shall not relieve the Contractor from responsibility for the structure. A SAR shall be submitted in all cases where the existing structure is posted for less than legal loads or the Contract plans indicate a live load restriction is in place.

Requirements

a) General. All work specified shall be performed according to the Contract plans, Special Provisions and/or Standard Specifications governing that work.

Submittals for falsework and forming for concrete construction shall be according to Articles 503.05 and 503.06 and does not require a SAR. Moving construction equipment across a structure, or portions thereof, open to traffic shall be addressed according to Article 107.16 and does not require a SAR. Operating equipment on an in-service structure and/or using a portion of an in-service structure as a work platform shall require a SAR and Article 107.16 shall not apply.

The Contractor may move vehicles across the existing bridge without a SAR after closure and prior to removal of any portion of the structure provided:

- The vehicles satisfy the requirements of Section 15-111 of the Illinois Vehicle Code (described in the IDOT document "Understanding the Illinois Size & Weight Laws") or of the Federal Highway Administration document "Bridge Formula Weights" (available at: http://www.ops.fhwa.dot.gov/freight/publications/brdg_frm_wghts/index.htm)
- The Contractor submits written verification to the Engineer stating the vehicles meet these requirements. The verification shall be submitted prior to allowing the vehicles on the structure.

This SAR exemption shall not relieve the Contractor from responsibility for the structure. This SAR exemption shall not be allowed where the existing structure is posted for less than legal loads or the Contract plans indicate a live load restriction is in place. No stockpiling of material is allowed under this exemption.

All SARs shall detail the procedures and sequencing necessary to complete the work in a safe and controlled manner. When appropriate, supporting design calculations shall be provided verifying the following:

- The effects of the applied loads do not exceed the capacity at Operating level for any portions of the structure being utilized in the demolition of the structure provided those portions are not to be reused.
- The effects of the applied loads do not exceed the capacity at Inventory level for new construction or for portions of the existing structure that are to be reused.
- The condition of the structure and/or members has been considered.

See AASHTO Manual for Bridge Evaluation for further information on determining the available capacities at the Operating and Inventory levels.

- b) Confidential Documents. Due to the sensitivity of the inspection reports and bridge condition reports to bridge security, the following confidentiality statement applies to these reports:

"Reports used by the Contractor and the contents thereof are the property of the Department, and are subject to the control of the Department in accordance with State and Federal law. The distribution, dissemination, disclosure, duplication or release of these reports or the content thereof in any manner, form or format without the express permission of the keeper of this record is prohibited. The owner is the official keeper of these records, except for state owned bridges, where the official keeper of these records is the Regional Engineer."

- c) Submittals. The Contractor shall be pre-approved to prepare SAR(s) or shall retain the services of a pre-qualified engineering firm to provide these services. Pre-approval of the Contractor will be determined by the Illinois Department of Transportation and will allow SAR(s) preparation by the Contractor unless otherwise noted on the plans. For engineering firms, pre-qualification shall be according to the Department in the category of "Highway Bridges-Typical" unless otherwise noted on the plans. Firms involved in any part of the project (plan development or project management) will not be eligible to provide these services. Evidence of pre-approval/pre-qualification shall be submitted with all SAR(s). The SAR(s) shall be prepared and sealed by an Illinois Licensed Structural Engineer. The Contractor shall submit SAR(s), complete with working drawings and supporting design calculations, to the Engineer for approval, at least 30 calendar days prior to start of that portion of the work.

At a minimum a Structural Assessment Report shall include the following:

1. A plan outlining the procedures and sequence for the work, including staging when applicable.
2. A demolition plan (when removal is included as an item of work in the contract) including details of the proposed methods of removal.
3. A beam erection plan (when beam erection is included as an item of work in the contract) including details of the proposed methods of erection.
4. Pertinent specifications for equipment used during the work activity.
5. The allowable positions for that equipment during the work activity.
6. The allowable positions and magnitudes of stockpiled materials and/or spoils, if planned to be located on the structure.
7. Design and details for temporary shoring and/or bracing, if required by the Contractor's means and methods.

Approval or acceptance of a Structural Assessment Report shall not relieve the Contractor of any responsibility for the successful completion of the work.

Revisions to the Contractor's means and methods resulting in no increased load effects to the structure, as determined by the Contractor's Structural Engineer, shall not require a SAR resubmittal. However, the Contractor's Structural Engineer shall submit to the Engineer written verification that there is no increased load effect. The written verification shall specify the revisions and shall be submitted prior to the start of the revised activities.

The Contractor shall be responsible for following the approved SAR related to the work involved.

Method of Measurement. Structural Assessment Reports will not be measured for payment.

Basis of payment. Structural Assessment Reports will not be paid for separately but shall be considered as included in the contract unit price(s) for the work item(s) specified.

BRIDGE DECK CONSTRUCTION

Effective: October 22, 2013

Revised: December 21, 2016

When Diamond Grinding of Bridge Sections is specified, hand finishing of the deck surface shall be limited to areas not finished by the finishing machine and to address surface corrections according to Article 503.16(a)(2). Hand finishing shall be limited as previously stated solely for the purpose of facilitating a more timely application of the curing protection. In addition the requirements of 503.16(a)(3)a. and 503.16(a)(4) will be waived.

Revise the Second Paragraph of Article 503.06(b) to read as follows.

"When the Contractor uses cantilever forming brackets on exterior beams or girders, additional requirements shall be as follows."

Revise Article 503.06(b)(1) to read as follows.

- “(1) Bracket Placement. The spacing of brackets shall be per the manufacturer’s published design specifications for the size of the overhang and the construction loads anticipated. The resulting force of the leg brace of the cantilever bracket shall bear on the web within 6 inches (150 mm) of the bottom flange of the beam or girder.”

Revise Article 503.06(b)(2) to read as follows.

- “(2) Beam Ties. The top flange of exterior steel beams or girders supporting the cantilever forming brackets shall be tied to the bottom flange of the next interior beam. The top flange of exterior concrete beams supporting the cantilever forming brackets shall be tied to the top flange of the next interior beam. The ties shall be spaced at 4 ft (1.2 m) centers. Permanent cross frames on steel girders may be considered a tie. Ties shall be a minimum of 1/2 inch (13 mm) diameter threaded rod with an adjusting mechanism for drawing the tie taut. The ties shall utilize hanger brackets or clips which hook onto the flange of steel beams. No welding will be permitted to the structural steel or stud shear connectors, or to reinforcement bars of concrete beams, for the installation of the tie bar system. After installation of the ties and blocking, the tie shall be drawn taut until the tie does not vary from a straight line from beam to beam. The tie system shall be approved by the Engineer.”

Revise Article 503.06(b)(3) to read as follows.

- “(3) Beam Blocks. Suitable beam blocks of 4 in x 4 in (100 x 100 mm) timbers or metal structural shapes of equivalent strength or better, acceptable to the Engineer, shall be wedged between the webs of the two beams tied together, within 6 inches (150 mm) of the bottom flange at each location where they are tied. When it is not feasible to have the resulting force from the leg brace of the cantilever brackets transmitted to the web within 6 inches (150 mm) of the bottom flange, then additional blocking shall be placed at each bracket to transmit the resulting force to within 6 inches (150 mm) of the bottom flange of the next interior beam or girder.”

Delete the last paragraph of Article 503.06(b).

METALLIZING OF STRUCTURAL STEEL

Effective: October 4, 2016

Revised: October 20, 2017

Description: This work consists of furnishing all materials, equipment, labor, and other essentials necessary to accomplish the surface preparation and application of thermal spray metallizing to all new structural steel, or portions thereof as detailed in the plans, in the shop. Also included in this work, when specified on the Contract plans, is the application of a paint system over the metallizing in the shop and/or in the field.

Materials: Materials shall be according to the following.

Metallizing Wire: All thermal spray feedstock (metallizing wire) shall be the products of a single manufacturer, meet the requirements below, and meet the thermal spray equipment manufacturer's specifications.

- a. The metallizing wire shall consist of 99.9% zinc or 85/15 zinc/aluminum complying with ASTM B-833 and ANSI/AWS C2.25/C2.25M
- b. The Contractor shall provide a certificate of chemical composition of the proposed metallizing wire from the metallizing wire manufacturer.

Paint: All materials to be used on an individual structure shall be produced by the same manufacturer.

The Bureau of Materials and Physical Research has established a list of all paint products that have met preliminary requirements. Each batch of material, except for the clear aliphatic urethane and the penetrating sealer shall be tested and approved for use. The specified colors shall be produced in the coating manufacturer's facility. Tinting of coating after it leaves the manufacturing facility is not allowed.

The paint materials shall meet the following requirements of the Standard Specification and as noted below:

| <u>Item</u> | <u>Article</u> |
|--|----------------|
| (a) Waterborne Acrylic | 1008.04 |
| (b) Aluminum Epoxy Mastic (Note 1) | 1008.03 |
| (c) Epoxy/ Aliphatic Urethane (Note 1) | 1008.05 |
| (d) Penetrating Sealer (Note 2) | |
| (e) Clear Aliphatic Urethane (Note 3) | |

Note 1: If the finish coats are being applied in the field over a shop applied epoxy, select an epoxy intermediate for shop application with a recoat window that is long enough to support the construction schedule.

Note 2: The Epoxy Penetrating Sealer shall be a cross-linked multi component sealer. The sealer shall have the following properties:

- (a) The volume solids shall be 98 percent (plus or minus 2 percent).
- (b) Shall be clear or slightly tinted color.

Note 3: The Clear Aliphatic Urethane material shall be one of the following products:

- (a) Carbothane Clear Coat by Carboline Company
- (b) Pitthane Ultra Clear 95-8000 by Pittsburgh Paints (PPG)
- (c) ArmorSeal Rexthane I MCU by Sherwin-Williams

Shop Prequalification: The Contractor performing the shop work shall have either an SSPC-QP 3 Certification or an AISC Sophisticated Paint Endorsement certification. The certification(s) shall remain current throughout the duration of the contract.

The Contractor performing the shop work shall have satisfactorily performed a minimum of three (3) previous projects involving abrasive blast cleaning, metallizing, and paint application. At least one project within the past two (2) years shall have involved a bridge or similar industrial type application. The suitability of the Contractor's qualifications and prior experience will be considered by the Department before granting approval to proceed.

Submittals: The Contractor performing the shop work shall submit the following plans and information for Engineer review and acceptance within 30 days of contract execution (unless written permission from the Engineer states otherwise). When full coats are being applied in the field, the field painting contractor shall comply with the submittal requirements of Article 506.03. Work in the shop or field shall not proceed until submittals are accepted by the Engineer.

- (a) Contractor Personnel Qualifications: Evidence of experience and the names and qualifications/experience/training of the personnel managing and implementing the Quality Control program, and for those performing the quality control tests. QC personnel qualification requirements are found under "Quality Control (QC) Inspection."

All metallizing applicators shall be qualified in accordance with AWS C2.16/C2.16M.

- (b) Quality Control (QC) Plan: A Quality Control Plan that identifies: test instruments to be used, a schedule of required measurements and observations, procedures for correcting unacceptable work, and procedures for improving surface preparation and metallizing/painting quality as a result of quality control findings. The program shall incorporate the IDOT Quality Control Daily Report Forms as supplied by the Engineer, or equivalent information on Engineer-approved Shop Contractor-designed forms.
- (c) Surface Preparation Plan: The surface preparation plan shall include the methods of surface preparation and types of equipment that will be used to prepare the surfaces as specified herein. Also any solvents proposed for solvent cleaning shall be identified and MSDS provided.
- (d) Abrasives: Identify the type and brand name of the abrasive proposed for use, provide MSDS and manufacturer's data indicating that the abrasive meets requirements of the SSPC-AB 1 or AB 3 standards as specified herein.
- (e) Metallizing Plan: Written procedures for the shop application of metallizing, including the brand name and type of metallizing wire and application equipment to be used. Proof that the metallizing wire complies with ASTM B-833 and ANSI/AWS C2.25/C2.25M shall also be provided. Provide written documentation verifying that all metallizing applicators are qualified in accordance with ANSI/AWS C2.16/C2.16M.
- (f) Painting Plan: If shop painting is specified to be applied over the metallizing or if galvanizing is used in lieu of metallizing on minor bridge members, procedures for the application of the coating system shall be provided along with MSDS and product data sheets. A description of the application equipment to be used shall be included. The plan shall include the requirements to be followed by the field contractor for field touch up.
- (g) Shipping and Handling Plan: A written plan outlining the precautions that shall be taken for the protection of the finished surface during shipping and handling. The plan shall address the steps to be taken, such as insulating padding, wood dunnage, load securing strapping, binding apparatus, etc.

- (h) Galvanizing Option: At the Contractor's option, hot dip galvanizing may be proposed as a substitute for shop metallizing of bearings, typical cross frames, or diaphragms on non-curved structures; expansion joint assemblies; and other elements not carrying calculated stress. Submittal requirements are found under "Hot Dip Galvanizing Option." Include the proposed cleaning and painting plan.

The Engineer will provide written notification to the Contractor when submittals are complete and acceptable. No surface preparation work shall begin until that notification is received. This acceptance shall not be construed to imply approval of any particular method or sequence for conducting the work, or for addressing health and safety concerns. Acceptance does not relieve the Contractor from the responsibility to conduct the work according to the requirements of Federal, State, or Local regulations and this specification, or to adequately protect the health and safety of all workers involved in the project and any members of the public who may be affected by the project. The Contractor remains solely responsible for the adequacy and completeness of the programs and work practices, and adherence to them.

Quality Control (QC) Inspections: The Contractor performing the shop work shall perform first line, in process QC inspections. The Contractor shall implement the accepted QC Program to insure that the work complies with these specifications. The designated Quality Control inspector shall be onsite full time during any operations that affect the quality of the system (e.g., surface preparation, metallizing application, paint application, and final inspection at project completion). The Contractor shall use the IDOT Contractor Daily (QC) Metallizing & Painting Report form (supplied by the Engineer, or Engineer-approved Contractor-designed forms that contain the same information, to record the results of quality control tests and inspections. The completed reports shall be given to the Engineer before work resumes the following day.

QC inspections shall include, but are not limited to the following:

- Ambient conditions.
- Surface preparation (solvent cleaning, abrasive blast cleanliness, surface profile depth, etc.).
- Metallizing application (specified materials used, bend test, continuity and coverage, adhesion, dry film thickness).
- Verification that the MISTIC test ID number for the paint system has been issued when painting is specified.
- Paint Application (when specified)(specified materials used, continuity and coverage, dry film thickness, freedom from overspray, dry spray, pinholes, skips, misses, etc.).

The personnel managing the QC Program shall possess a minimum classification as a NACE CIP Level 2, or shall provide evidence of successful inspection of three projects of similar or greater complexity and scope completed in the last two years. References shall include the name, address, and telephone number of a contact person employed by the facility owner.

The personnel performing the QC tests shall be trained in all tests, inspections, and instrument use required for the inspection of surface preparation, metallizing and paint application. Documentation of training shall be provided. The QC personnel shall be solely dedicated to quality control activities and shall not perform any production work. QC personnel shall take the lead in all inspections, but applicators shall perform wet film thickness measurements during application of the coatings, with QC personnel conducting random spot checks. The Contractor shall not replace the QC personnel assigned to the project without advance notice to the Engineer, and acceptance of the replacement(s), by the Engineer.

The Contractor performing the shop work shall supply all necessary equipment to perform the QC tests and inspections as specified. Equipment shall include the following at a minimum:

- Psychrometer or comparable equipment for measurement of dew point and relative humidity, including weather bureau tables or psychrometric charts
- Surface temperature thermometer
- SSPC Visual Standard VIS 1
- Surface profile replica tape and spring micrometer or electronic micrometer designed for use with replica tape; or electronic profilometer designed for measuring blast profile.
- Blotter paper for compressed air cleanliness checks
- Type 2 Electronic Dry Film Thickness Gage
- Calibration standards for dry film thickness gage
- Bend test coupons and bend test mandrel
- Adhesion testing instrument
- Companion panels for adhesion testing (if that option is selected)
- All applicable ASTM, ANSI, AWS, and SSPC Standards used for the work (reference list attached)

The instruments shall be verified for accuracy and adjusted by the Contractor's personnel in accordance with the equipment manufacturer's recommendations and the Contractor's QC Program. All inspection equipment shall be made available to the Engineer for QA observations as needed.

Hold Point Notification: Specific inspection and testing requirements within this specification are designated as Hold Points. Unless other arrangements are made, the Contractor shall provide the Engineer with a minimum four-hour notification in advance of the Hold Point. If four-hour notification is provided and the work is ready for inspection at that time, the Engineer will conduct the necessary observations. If the work is not ready at the appointed time, unless other arrangements are made, an additional four-hour notification is required. Permission to proceed beyond a Hold Point without a QA inspection will be at the sole discretion of the Engineer and will only be granted on a case-by-case basis.

Quality Assurance (QA) Observations: The Engineer will conduct QA observations of any or all phases of the work. The presence or activity of Engineer observations in no way relieves the Contractor of the responsibility to perform all necessary daily QC inspections of their own and to comply with all requirements of this Specification.

The Engineer has the right to reject any work that was performed without adequate provision for QA observations.

CONSTRUCTION REQUIREMENTS

The surface preparation and metallizing shall be according to the SSPC Specification for the Application of Thermal Spray Coatings (Metallizing) of Aluminum, Zinc and their Alloys and Composites for the Corrosion Protection of Steel, SSPC-CS 23.00/AWS C2.23M/NACE No. 12 except as modified herein. In the event of a conflict, the requirements of this specification shall prevail.

Hot Dip Galvanizing Option: At the Contractor's option, hot dip galvanizing may be substituted for shop metallizing of bearings, typical cross frames, or diaphragms on non-curved structures; expansion joint assemblies; and other elements not carrying calculated stress. Galvanized surfaces which shall have concrete poured against them shall be chemically passivated or otherwise protected by a method approved by the Engineer. Galvanized bearings for exterior members and elements readily visible after erection shall be prepared for field painting, but galvanized items obscured from public view will not require field painting. The Contractor shall submit a proposal for substituting galvanizing to the Engineer, showing items to be field painted, applicable provisions of AASHTO M 111 (ASTM A 123), drain/vent holes and any other necessary modifications.

Notification: The Contractor shall notify the Engineer 24-hours in advance of beginning surface preparation operations.

Surface Preparation, Metallizing and Painting Equipment: The Contractor shall provide surface preparation, metallizing, and painting equipment as needed to perform the work as specified herein.

Metallizing application equipment shall be portable electric arc thermal spray units that are set-up, adjusted and operated in accordance with the manufacturer's written instructions.

All cleaning and painting equipment shall include gages capable of accurately measuring fluid and air pressures and shall have valves capable of regulating the flow of air, water or paint as recommended by the equipment manufacturer. The equipment shall be maintained in proper working order.

Diesel or gasoline powered equipment shall be positioned or vented in a manner to prevent deposition of combustion contaminants on any part of the structure.

Hand tools, power tools, pressure washing, water jetting, abrasive blast cleaning equipment, brushes, rollers, and spray equipment shall be of suitable size and capacity to perform the work required by this specification. Appropriate filters, traps and dryers shall be provided for the compressed air used for abrasive blast cleaning and conventional spray application. Paint pots shall be equipped with air operated continuous mixing devices unless prohibited by the coating manufacturer.

Test Areas (Sections): Prior to proceeding with production work on the project, the Contractor shall prepare test sections of at least 10 square feet (0.93 sq. m). More than one test section may be needed to represent the various design configurations of the structure. The test section(s) shall be blast cleaned, metallized and painted (if specified) in accordance with the requirements specified herein using the same equipment, materials and procedures that will be used for the production.

During the blast cleaning, metallizing, and painting of the test section(s), in the presence of the Engineer, the Contractor shall perform all quality control tests and inspections required by this specification including complete documentation. In addition, the Contractor shall allow sufficient time for the Engineer to perform any or all quality assurance tests and inspections desired.

Production work shall not proceed until the Engineer agrees that the blast cleaning, metallizing, and painting work, along with the quality control testing, inspection, and documentation are acceptable.

No additional compensation will be paid for the preparation of the test section(s).

Protective Coverings and Damage: The Contractor shall apply protective coverings to all surfaces of the structural steel that are not scheduled for surface preparation, metallizing, and painting. The coverings shall be maintained and remain in place until the work is completed and then shall be removed prior to shipping.

Metallized or painted surfaces damaged by any Contractor's operation shall be repaired, and re-metallized and/or re-painted, as directed by the Engineer, at no additional cost to the Department.

Ambient Conditions: Surfaces prepared for metallizing or painting shall be free of moisture and other contaminants. The Contractor shall control operations to insure that dust, dirt, or moisture do not come in contact with surfaces on which work will take place. The surface temperature shall be at least 5°F (3°C) above the dew point during final surface preparation operations, and the application of metallizing. Metallizing shall only be applied when the surface and air temperatures are above 32°F (0°C). The manufacturers' published literature shall be followed for specific temperature, dew point, and humidity restrictions during the application of each paint coat. Metallizing or paint shall not be applied in rain, wind, snow, fog or mist. Ambient conditions shall be maintained during the drying period specified by the manufacturer.

Compressed Air Cleanliness: Prior to using compressed air for abrasive blast cleaning, blowing down surfaces, and metallizing or painting application, the Contractor shall verify that the compressed air is free of moisture and oil contamination according to the requirements of ASTM D 4285. The tests shall be conducted at least one time per shift for each compressor system in operation. If air contamination is evident, the Contractor shall change filters, clean traps, add moisture separators or filters, or make other adjustments as necessary to achieve clean, dry air. The Contractor shall also examine the work performed since the last acceptable test for evidence of defects or contamination caused by the contaminated compressed air. Contaminated work shall be repaired at no additional cost to the Department.

Solvent Cleaning (HOLD POINT): All traces of oil, grease, and other detrimental contaminants on the steel surfaces to be metallized shall be removed by solvent cleaning in accordance with SSPC-SP 1. The brand name of proposed cleaning solvent(s) and/or proprietary chemical cleaners including manufacturers' product data sheet and MSDS shall be submitted for Engineer acceptance prior to use.

Under no circumstances shall blast cleaning be performed in areas containing surface contaminants or in areas where the Engineer has not accepted the solvent cleaning. Rejected surfaces shall be re-cleaned to the specified requirements at no additional cost to the Department.

Abrasives: Abrasive blast cleaning shall be performed using either expendable abrasives or recyclable steel grit abrasives. Expendable abrasives shall be used one time and discarded. The abrasive shall be angular in shape. Acceptable angular shaped abrasives include, but are not limited to, aluminum oxide, steel grit, and crushed slag. Silica sand shall not be used. Steel shot and other abrasives producing a rounded surface profile are not acceptable, even if mixed with angular grit abrasives.

Abrasive suppliers shall provide written certification that expendable abrasives and recyclable steel grit abrasives meet the requirements of SSPC-AB 1 and AB 3, respectively. Abrasive suppliers shall certify that abrasives are not oil contaminated and shall have a water extract pH value within the range of 6 to 8. On a daily basis, the Contractor shall verify that recycled abrasives are free of oil and contamination by performing a vial test in accordance with SSPC-AB 2.

All surfaces that are found to have been prepared using abrasives not meeting the SSPC-AB 1, AB 2, or AB 3 requirements, as applicable, are oil contaminated, or have a pH outside the specified range, shall be solvent cleaned or low pressure water cleaned, and re-blast cleaned at no cost to the Department.

Surface Preparation (HOLD POINT): The following method of surface preparation shall be used:

- (a) **Flame Cut Steel:** Prior to blast cleaning, all flame cut edges shall be ground to remove hardened steel and any sharp or irregular shapes.
- (b) **Near-White Metal Blast Cleaning:** All steel surfaces to be metallized shall be near white metal blast cleaned in accordance with SSPC-SP 10 using dry abrasive blast cleaning methods.

- (c) Galvanized Minor Bridge Members: If galvanizing of minor bridge members is selected in lieu of metallizing, prepare all galvanized surfaces for painting by brush-off blast cleaning in accordance with SSPC-SP 16 or by using proprietary solutions that are specifically designed to clean and etch (superficially roughed) galvanized steel for painting. If cleaning and etching solutions are selected, submit manufacturer's technical product literature and MSDS for Engineer's review and written acceptance prior to use.
- (d) Base Metal Irregularities: If hackles, burrs, or slivers in the base metal are visible on the steel surface after cleaning, the Contractor shall remove them by grinding followed by re-blast cleaning.

Surface Profile (HOLD POINT): Blast cleaning abrasives shall be of the size and grade that will produce a uniform angular surface profile depth of 3.5 to 4.5 mils (89 to 114 microns). If the metallizing wire manufacturer's profile requirements are more restrictive, the Contractor shall advise the Engineer and comply with those requirements. For recycled abrasives, an appropriate operating mix shall be maintained in order to control the profile within these limits.

The average surface profile shall be determined each work day with a minimum frequency of one location per every 200 sq ft (18.6 sq m) per piece of equipment. All surfaces, including flame cut edges, shall be tested in accordance with SSPC-PA 17. Surface profile replica tape or electronic profilometer shall be used. The tape shall be retained and included with the daily QC report. Single measurements less than 3.5 mils (89 microns) are unacceptable. In that event, additional testing shall be done to determine the limits of the deficient area and, if it is not isolated, work will be suspended. The Contractor shall submit a plan for making the necessary adjustments to insure that the specified surface profile is achieved on all surfaces. Work shall not resume until the Engineer provides written acceptance.

Surface Condition Prior to Metallizing (HOLD POINT): Prepared surfaces shall meet the requirements of SSPC-SP 10 immediately prior to metallizing, and shall be metallized within six hours of blast cleaning. If rust appears or bare steel has been exposed for more than six hours, the affected area shall be re-blasted at no additional cost to the Department.

All dust and surface preparation residue on steel surfaces shall be removed prior to metallizing.

The quality of surface preparation and cleaning of surface dust and debris shall be accepted by the Engineer prior to metallizing.

The Engineer has the right to reject any work that was performed without adequate provision for QA observations to accept the degree of cleaning. Rejected metallizing work shall be removed and replaced at no additional cost to the Department.

Daily Metallizing Operator-Equipment Qualification – Bend Tests: Unless directed otherwise by the Engineer, each day that metallizing will be applied, the Contractor shall perform bend testing prior to beginning production work. For each metallizing applicator, five carbon steel coupons measuring 2 inch wide x 8 inch long x 0.05 inch (50mm x400 mm x 1.3 mm) thick shall be blast cleaned using the same equipment and abrasive used for the production work. Each applicator shall apply the metallizing to five coupons in accordance with the requirements of this Specification to a dry film thickness of 8.0 to 12.0 mils (200 to 300µm). 180 degree bend testing shall be performed on all five coupons using a 13mm (1/2") mandrel in accordance with the requirements and acceptance criteria of SSPC-CS 23/AWS C2.23M/NACE 12. Minor cracks that cannot be lifted from the substrate with knife blade are acceptable. If lifting occurs on any coupon, the surface preparation and/or metallizing process shall be modified until acceptable results are achieved before proceeding with production work.

Application of Metallizing: Application shall be done in overlapping passes in a cross-hatch pattern (i.e., a second set of overlapping passes shall be applied at right angles to the first set of overlapping passes) to ensure uniform coverage. The gun shall be held at such a distance from the work surfaces that the metal is still molten on impact. The metallizing shall be applied as a continuous film of uniform thickness, firmly adherent, and free from thin spots, misses, lumps or blisters, and have a fine sprayed texture. Thin spots and misses shall be re-metallized. If touch up metallizing or the application of additional metallizing to previously applied metallizing does not occur within 24 hours, the surface of the metallizing shall be brush off blast cleaned according to SSPC-SP7 to remove oxidation and surface contaminants prior to the application of additional metallizing. The final appearance of the metallizing when left un-top coated or top coated with System 1 shall be uniform without excessive blotchiness or contrast in color. If the surface does not have a uniform appearance, remove and replace the metallizing at no cost to the Department. If the configuration of the surface being metallized does not allow for a proper gun-to-work piece standoff distance, the Contractor shall notify the Engineer.

Unless required by the contract plans, the top of the top flanges shall not be metallized or painted. If the contract plans indicate that the top flange is to be metallized, only the first coat of the paint system shall be applied to the top flange.

Metallizing Thickness: The thickness of the metallizing shall be 8.0 to 12.0 mils (200-300 microns). Thickness shall be measured as specified by SSPC-PA 2 (use a Type 2 Electronic Gauge only).

Metallizing Adhesion: Adhesion testing of metallizing applied each day shall be determined with a self-adjusting adhesion tester in accordance with ASTM D 4541. Unless otherwise directed by the Engineer, a minimum of one test shall be conducted for every 500 sq ft (46sq m) of metallized surface. The tests shall be conducted prior to application of any coating. If any of the tests exhibit less than 700 psi (4.83 MPa) for 85/15 or less than 500 psi (3.45 MPa) for zinc, additional tests shall be conducted to determine the extent of the deficient material. All deficient metallizing shall be removed by blast cleaning and re-applied at no additional cost to the Department.

At the discretion of the Engineer, a representative blast cleaned test panel (or steel companion panel approximately 12 inch x 12 inch x ¼ inch thick) can be metallized at the same time each 500 sq ft (46sq m) of surface area, or portion thereof, is metallized. Adhesion testing can be performed on the companion panel rather than on the structure. If the adhesion tests on the panels are acceptable, the metallizing on the structure is considered acceptable and testing on the structure is not required. If adhesion testing of the panels fails, testing shall be conducted on the structure. If adhesion testing on the structure is acceptable, the metallizing on the structure is considered to be acceptable. If tests on the structure are unacceptable, complete removal of the failing metallizing and re-metallizing in accordance with this Specification shall be performed at no additional cost to the Department.

Application of Paint Systems Over Metallizing:

When painting over the metallizing is specified, three painting system options exist for application over the metallizing as shown below. Systems, or components of systems, specified to be shop applied shall not be applied to the faying surfaces of bolted connections. The system to be applied shall be as designated on the plans.

- (a) **System 1** is a single coat system consisting of a full clear aliphatic urethane coat shop applied to all metallized surfaces except as noted above.

The thickness of the clear coat to be applied is dependent on the product selected and shall be as follows:

TABLE 1

CLEAR URETHANE COAT (SINGLE COAT SYSTEM)

| MANUFACTURER | SEALER COAT ONLY (DFT) |
|----------------------------|---|
| Carboline Company | Carbothane Clear Coat (3.0 to 5.0 mils) (75 to 125 microns) |
| Pittsburgh Paints (PPG) | Pitthane Ultra Clear 95-8000 (2.0 to 3.0 mils) (50 to 75 microns) |
| Sherwin-Williams | AarmorSeal Rexthane I MCU (3.0 to 5.0 mils) (75 to 125 microns) |

The clear urethane shall be applied in a 2 step process. The first step shall be to apply a “mist coat” that is thinned at the maximum allowable thinning rate as listed on the manufacturer’s product data sheet that is compliant with VOC regulations. The intent of the mist coat is to saturate the porous metallizing surface and displace entrapped air within the porosity of the metallizing. After allowing the mist coat to flash off for 20 minutes, the full coat of clear urethane shall be applied to achieve the manufacturer’s recommended dry film thickness.

- (b) **System 2** is a four coat system consisting of a full shop coat of epoxy penetrating sealer coat, a full shop coat of an extended recoat epoxy and two full field applied coats of waterborne acrylic.

The epoxy penetrating sealer shall be applied in accordance with the coating manufacturer's instructions at a coverage rate designed to achieve a theoretical dry film thickness of 1.5 mils (38 microns). The intent of the epoxy penetrating sealer coat is to saturate the metallizing and cover the surface rather than to build a film thickness; therefore, dry film thickness measurement of the epoxy penetrating sealer coat is not required. The top of top flanges that are specified to be metallized and embedded in concrete shall receive the epoxy penetrating sealer only.

The thicknesses of the epoxy and waterborne acrylic coats shall be according to Article 506.09(f)(1).

- (c) **System 3** is a three coat system consisting of a full epoxy penetrating sealer coat, a full epoxy intermediate coat, and a full urethane finish coat. All coats shall be shop-applied unless specified otherwise. If the urethane is field-applied, an extended recoat epoxy shall be applied in the shop.

The epoxy penetrating sealer shall be applied in accordance with the coating manufacturer's instructions at a coverage rate designed to achieve a theoretical dry film thickness of 1.5 mils (38 microns). The intent of the epoxy penetrating sealer coat is to saturate the metallizing and cover the surface rather than to build a film thickness; therefore, dry film thickness measurement of the epoxy penetrating sealer coat is not required. The top of top flanges that are specified to be metallized and embedded in concrete shall receive the epoxy penetrating sealer only.

The thicknesses of the epoxy and urethane coats shall be according to Article 506.09(f)(2).

The single clear urethane coat or the epoxy penetrating sealer coat shall be applied within 24 hours of metallizing providing that the immediate work environment is controlled. If temperature and humidity cannot be controlled, that time frame shall be reduced to within 8 hours. The metallizing shall be dry and free of any visible debris or oxidation (zinc oxide) at the time of application. Visible oxidation shall be removed by mechanical methods such as stiff bristle or wire brushing. Contact surfaces for bolted connections shall consist of bare, uncoated metallizing only and shall be masked off prior to the application of any shop applied coatings.

The clear urethane coat or the epoxy penetrating sealer shall be applied in accordance with the manufacturer's instructions and in such a manner to assure thorough wetting and sealing of the metallizing.

For systems 2 and 3, prior to application of any subsequent coat, the surface of the previous coat shall be dry in accordance with the manufacturer's instructions and free of any visible contamination. If the manufacturer's specified recoat times are exceeded, the effected coat(s) shall be completely roughened or removed and replaced, according to the manufacturer's instructions, at no cost to the Department. The same restrictions regarding film appearance and continuity for the seal coat apply to the intermediate coat and topcoat.

All coats shall be applied to achieve a smooth, uniform appearance that is free of dryspray, overspray, and orange peel. Shadow-through, pinholes, bubbles, skips, misses, lap marks between applications, runs, sags, or other visible discontinuities are unacceptable.

Masked off areas around field connections shall be coated in the field after the steel is fully erected according to the touch-up procedure for the completed system.

When the application of field coat(s) is required, the existing shop applied coats shall be prepared and field painting performed according to the applicable provisions of Article 506.10. If any coat has exceeded its recoat time, the surface shall be completely roughened or removed and replaced according to the manufacturer's instructions, prior to the application of the topcoat.

All coatings shall be applied by spray, supplemented with brushing or rolling, if needed. Special attention shall be given to obtaining complete coverage and proper coating thickness in crevices, on welds and edges, and in hard to reach areas.

Application of Paint System over Galvanizing: If galvanizing is used in lieu of metallizing and Paint System 1, no further painting is required. If galvanizing is used in lieu of metallizing and Paint System 2, apply a two-coat system consisting of a full waterborne acrylic intermediate coat and a full waterborne acrylic finish coat from System 2. If galvanizing is used in lieu of metallizing and Paint System 3, apply a full epoxy intermediate coat and a full urethane coat from System 3. To minimize handling and erection damage the acrylic coats of System 2 shall be applied in the field. Except as noted on the plans, the epoxy and urethane coats of System 3 can be applied in the shop or field.

Touch-Up of Completed Coating System: The Contractor shall repair all damaged and/or unacceptable areas of the completed coating system (all metallizing, galvanizing, and paint layers) prior to shipment as defined below. The same process shall be followed for the repair of shipping, handling, and erection damage.

Damage to the metallizing, galvanizing, and/or paint that does not expose the substrate shall be prepared by solvent cleaning in accordance with SSPC-SP 1 followed by power tool cleaning in accordance with SSPC-SP 3 to remove loose material. For the repair of damaged metallizing or galvanizing that exposes the substrate, the surface shall be spot blast cleaned in accordance with SSPC-SP 10. If blast cleaning cannot be performed, as authorized by the Engineer, the damage shall be spot power tool cleaned to SSPC-SP11.

The metallizing, galvanizing and/or paint surrounding each repair area shall be feathered for a distance of 1 to 2 inches (25 to 50 mm) to provide a smooth, tapered transition into the existing intact material. The surrounding intact paint shall be roughened to promote adhesion of the repair coats.

Damage to metallizing or galvanizing extends to the substrate shall be repaired. For metallizing it is critical that all remnants of sealer or paint have been removed from the porosity of the metallizing before applying new metallizing or an adhesion failure can occur. If it is no longer feasible to apply metallizing, spot-apply an organic zinc primer meeting the requirements of Section 1008. For galvanizing, spot apply organic zinc. After priming, for both the metallizing and galvanizing, apply the same intermediate and finish coats used on the surrounding steel. If the damage does not expose the substrate, only the effected paint coat(s) shall be applied.

Surface Preparation and Painting of Galvanized Fasteners: All ASTM A 325 or ASTM F 3125 high strength steel bolts, nuts and washers shall be hot dip galvanized according to AASHTO M232, except in areas where the metallized surfaces are to be top coated, in which case they shall be mechanically galvanized according to Article 1006.08(a) of the Standard Specifications.

The Contractor shall prepare all fasteners (i.e., galvanized nuts, bolts, etc.) by power tool cleaning in accordance with SSPC-SP 3. Following power tool cleaning and prior to painting, the surfaces shall be solvent cleaned according to SSPC-SP 1. Slight stains of torqueing compound dye may remain after cleaning provided the dye is not transferred to a cloth after vigorous rubbing. If any dye is transferred to a cloth after vigorous rubbing, additional cleaning is required.

Spot paint the fasteners with one coat of an aluminum epoxy mastic coating meeting the requirements of Article 1008.03 of the Standard Specifications.

Shipping and Handling: The Contractor shall take special care in handling the steel in the shop and when loading for shipment. Painted, metallized, or galvanized steel shall not be moved or handled until sufficient cure time has elapsed to prevent handling damage. During shipping, the steel shall be insulated from the moving apparatus (i.e., chains, cables, hooks, clamps, etc.) by softeners approved by the Engineer. Apparatus used to hoist the steel shall be padded. Steel shall be placed on wood dunnage and spaced in such a manner that no rubbing will occur during shipment that could damage the paint, metallizing or galvanizing.

Special Instructions: At the completion of the work, the Contractor shall stencil on the bridge, using a contrasting colored paint, the date of metallizing and painting. The letters shall be capitals, not less than 2 inches (50 mm) and not more than 3 inches (75 mm) in height. The information defined below shall be stenciled on the exterior face of the first girders at the bridge abutments (approximately 1 or 2 feet outward from the abutment end of the girders). The Engineer will identify the bridge member(s) to be stenciled.

When all coats are applied in the shop with the exception of touch-up, the shop Contractor shall do the stenciling. The stencil shall contain the following words on four lines: "METALLIZED BY" on the first line; name of the Contractor on the second line; and the month and year in which the coating was completed on the third line; and the applicable system Code on the fourth line.

When the finish coat is applied in the field, the Contractor shall do the stenciling as described above, but insert "PAINTED BY" and the Contractor's name after the fourth line.

Basis of Payment: This work shall not be paid for separately but shall be included in the unit price bid for furnishing and/or erecting structural steel according to Article 505.13.

Appendix 1 – Reference List

The Shop and Field Contractor(s) shall maintain the following regulations and references on site for the duration of the project:

Illinois Environmental Protection Act

American Society of Testing Material

- ASTM D 4285, Standard Test Method for Indicating Oil or Water in Compressed Air
- ASTM B833, Standard Specifications for Zinc Wire for Thermal Spraying (Metallizing)
- ASTM D4541, Standard Test Method for Pull-Off Strength of Coatings Using Portable Adhesion Testers

Society of Protective Coatings

- SSPC-AB 1, Mineral and Slag Abrasives
- SSPC-AB 2, Specification for Cleanliness of Recycled Ferrous Metallic Abrasives
- SSPC-AB 3, Newly Manufactured or Re-Manufactured Steel Abrasives
- SSPC-PA 2, Measurement of Dry Coating Thickness with Magnetic Gages
- SSPC-QP 1, Standard Procedure for Evaluating Painting Shop Contractors (Field Application to Complex Structures)
- SSPC-QP 2, Standard Procedure for Evaluating the Qualifications of Painting Shop Contractors to Remove Hazardous Paint
- SSPC-SP 1, Solvent Cleaning
- SSPC-SP 5/NACE No. 1, White Metal Blast Cleaning
- SSPC-SP 11, Power Tool Cleaning to Bare Metal
- SSPC-SP 12/NACE No. 5, Surface Preparation and Cleaning of Metals by Water Jetting Prior to Recoating
- SSPC-SP 16, Brush-Off Blast Cleaning of Coated and Uncoated Galvanized Steel, Stainless Steels, and Non-Ferrous Metals
- SSPC-PA 17, Procedure for Determining Conformance to Steel Profile/Surface Roughness/Peak Count Requirements.
- SSPC-VIS 1, Guide and Reference Photographs for Steel Surfaces Prepared by Dry Abrasive Blast Cleaning

- SSPC-VIS 5, Guide and Reference Photographs for Steel Prepared by Wet Abrasive Blast Cleaning
- SSPC-Guide 15, Field Methods for Retrieval and Analysis of Soluble Salts on Steel and Other Nonporous Surfaces
- SSPC-CS 23.00/AWS C2.23M/NACE No. 12, Specification for the Application of Thermal Spray Coatings (Metallizing) of Aluminum, Zinc, and Their Alloys and Composites for the Corrosion Protection of Steel

American National Standards Institute/American Welding Society

- ANSI/AWS C2.25/C2.25M, Specification for Solid and Composite Wires, and Ceramic Rods for Thermal Spraying
- AWS C2.6/C2.6M, Guide for Thermal-Spray Operator Qualification

Metallizing wire and coating manufacturer's application instructions, MSDS and product data sheets

HOT DIP GALVANIZING FOR STRUCTURAL STEEL

Effective: June 22, 1999

Revised: June 28, 2024

Description. This work shall consist of surface preparation and hot dip galvanizing all structural steel specified on the plans and painting of galvanized structural steel when specified on the plans.

Materials. Fasteners shall be ASTM F 3125, Grade 325, Type 1, High Strength bolts with matching nuts and washers.

Fabrication Requirements. Hot-dip galvanizing shall be indicated on the shop drawings. The fabricator shall coordinate with the galvanizer to incorporate additional steel details required to facilitate galvanizing of the steel. These additional details shall be indicated on the shop drawings.

Additional temporary stiffeners may be added at the contractor's expense as necessary to prevent distortion of the girders during galvanizing. The contractor shall coordinate with the fabricator and the galvanizer to determine if additional stiffeners are necessary, and where these shall be placed. Any proposed changes shall be submitted to the Engineer for approval prior to making any changes and documented on the shop drawings.

Temporary stiffener angles shall be bolted to each side of the splice ends of each girder segment to prevent distortion during galvanizing. Temporary stiffener angles shall bolt or fit tight against top and bottom flanges and include spacer tubes to minimize damage to galvanizing during removal.

To ensure identification after galvanizing, piece marks shall be supplemented with metal tags for all items where fit-up requires matching specific pieces.

After fabrication (cutting, welding, drilling, etc.) is complete, all holes shall be deburred and all fins, scabs or other surface/edge anomalies shall be ground or repaired per ASTM A6. The items shall then be cleaned per Steel Structures Painting Council's Surface Preparation Specification SSPC-SP1 (Solvent Cleaning) and SSPC-SP6 (Commercial Blast Cleaning). All surfaces shall be inspected to verify no fins, scabs or other similar defects are present.

The Contractor shall consult with the galvanizer to ensure proper removal of grease, paint and other deleterious materials prior to galvanizing.

Surface Preparation and Hot Dip Galvanizing

General. Surfaces of the structural steel specified on the plans shall be prepared and hot dip galvanized as described herein.

Cleaning Structural Steel. If rust, mill scale, dirt, oil, grease or other foreign substances have accumulated prior to galvanizing, steel surfaces shall be cleaned by a combination of caustic cleaning and cleaning according to SSPC-SP8 (Pickling).

Special attention shall be given to the cleaning of corners and reentrant angles.

Surface Preparation. A flux shall be applied to all steel surfaces to be galvanized. Any surfaces which will receive field-installed stud shear connectors shall not be galvanized within 2 in. (50 mm) of the stud location. Either the entire area receiving studs or just individual stud locations may be left ungalvanized. The following steel surfaces of bearings shall not be galvanized: stainless steel surfaces, surfaces which will be machined (except for fixed bearing sole plates), and surfaces which will have TFE, elastomer, or stainless steel parts bonded to them.

The cleaned surfaces shall be galvanized within 24 hours after cleaning, unless otherwise authorized by the Engineer.

Application of Hot Dip Galvanized Coating. Steel members, fabrications and assemblies shall be galvanized by the hot dip process in the shop according to AASHTO M 111.

Bolts, nuts, and washers shall be galvanized according to ASTM F 2329.

All steel shall be safeguarded against embrittlement according to ASTM A 143. Water quenching or chromate conversion coating shall not be used on any steel work that is to be painted. All galvanized steel work shall be handled in such a manner as to avoid any mechanical damage and to minimize distortion.

Beams and girders shall be handled, stored and transported with their webs vertical and with proper cushioning to prevent damage to the member and coating. Members shall be supported and externally stiffened during galvanizing to prevent permanent distortion.

Hot Dip Galvanized Coating Requirements. Coating weight, surface finish, appearance and adhesion shall conform to requirements of ASTM A 385, ASTM F2329, AASHTO M 111 or AASHTO M 232, as appropriate.

Any high spots of zinc coating, such as metal drip lines and rough edges, left by the galvanizing operation in areas that are to be field connected or in areas that are to be painted shall be removed by cleaning per SSPC-SP2 (Hand Tool Cleaning) or SSPC-SP3 (Power Tool Cleaning). The zinc shall be removed until it is level with the surrounding area, leaving at least the minimum required zinc thickness.

Shop assemblies producing field splices shall provide 1/8 in. (3 mm) minimum gaps between ends of members to be galvanized. At field splices of beams or girders, galvanizing exceeding 0.08 in. (2 mm) on the cross-sectional (end) face shall be partially removed until it is 0.04 in. to 0.08 in. (1 to 2 mm) thick.

Testing of Hot Dip Galvanized Coating. Inspection and testing of hot dip galvanized coatings shall follow the guidelines provided in the American Galvanizers Association publication "*Inspection of Products Hot Dip Galvanized After Fabrication*". Sampling, inspection, rejection and retesting for conformance with requirements shall be according to AASHTO M 111 or AASHTO M 232, as applicable. Coating thickness shall be measured according to AASHTO M 111, for magnetic thickness gage measurement or AASHTO M 232, as applicable.

All steel shall be visually inspected for finish and appearance.

Bolts, nuts, washers, and steel components shall be packaged according to ASTM F 2329. Identity of bolts, nuts and washers shall be maintained for lot-testing after galvanizing according to Article 505.04(f)(2) for high strength steel bolts.

A notarized certificate of compliance with the requirements listed herein shall be furnished. The certificate shall include a detailed description of the material processed and a statement that the processes used met or exceeded the requirements for successful galvanizing of the surface, where applicable. The certificate shall be signed by the galvanizer.

Repair of Hot Dip Galvanized Coating. Surfaces with inadequate zinc thickness shall be repaired in the shop according to ASTM A 780 and AASHTO M 111.

Surfaces of galvanized steel that are damaged after the galvanizing operation shall be repaired according to ASTM A 780 whenever damage exceeds 3/16 in. (5 mm) in width and/or 4 in. (100 mm) in length. Damage that occurs in the shop shall be repaired in the shop. Damage that occurs during transport or in the field shall be repaired in the field.

Connection Treatment. All bolt holes shall be reamed or drilled to their specified diameters after galvanizing. All bolts shall be installed after galvanizing.

Surface Preparation and Painting

Surface Preparation. When galvanized steel surfaces are specified to be painted they shall be clean and free of oil, grease, and other foreign substances. Surface preparation necessary to provide adequate adhesion of the coating shall be performed according to ASTM D6386. Surface preparation shall include, but not be limited to the following:

- All galvanized steel surfaces that are to be painted shall be cleaned according to SSPC-SP1 (Solvent Cleaning). After cleaning, all chemicals shall be thoroughly rinsed from the surface with a suitable solvent. The steel shall be allowed to completely dry prior to coating application.
- All galvanized steel surfaces that are to be painted shall be checked for the presence of chromate conversion coating according to ASTM D 6386 Appendix X1. Surfaces where chromate conversion coating is found shall be cleaned according to the same appendix and blown down with clean, compressed air according to ASTM D 6386 Section 6.1.
- All galvanized steel surfaces that are to be painted shall be checked for the presence of wet storage stain. Surfaces where wet storage stain is found shall be cleaned, rinsed and completely dried according to ASTM D 6386 Section 6.2.
- Following galvanizing, thickness readings shall verify the acceptable thickness of the galvanizing according to AASHTO M111/ASTM A123.

Paint Requirements. The paint materials (epoxy intermediate coat and aliphatic urethane finish coat) shall meet the requirements of the Articles 1008.05(d) and (e) of the Standard Specification.

All paint materials for the shop and field shall be supplied by the same manufacturer, and samples of components submitted for approval by the Department, before use.

Paint storage, mixing, and application shall be according to Section 506 of the Standard Specifications and the paint manufacturer's written instructions and product data sheets. In the event of a conflict the Contractor shall advise the Engineer and comply with the Engineer's written resolution. Until a resolution is provided, the most restrictive conditions shall apply.

Shop Application of the Paint System. The areas to be painted shall receive one full coat of an epoxy intermediate coat and one full coat of an aliphatic urethane finish coat. The film thickness of each coat shall be according to Article 506.09(f)(2).

Construction Requirements. The contact surfaces of splice flange connections (mating flange faces and areas under splice bolt heads and nuts) shall be free of paint prior to assembly. If white rust is visible on the mating flange surfaces, the steel shall be prepared by hand wire brushing or brush-off blasting according to SSPC-SP7. Power wire brushing is not allowed.

After field erection, the following areas shall be prepared by cleaning according to SSPC-SP1 (Solvent Cleaning), tie- or wash-coated if applicable, and then painted or touched up with the paint specified for shop application (the intermediate coat and/or the finish coat):

- exposed unpainted areas at bolted connections
- areas where the shop paint has been damaged
- any other unpainted, exposed areas as directed by the Engineer.

Special Instructions. Painting Date/System Code. At the completion of the work, the Contractor shall stencil in contrasting color paint the date of painting the bridge and the paint type code from the Structure Information and Procedure Manual for the system used according to Article 506.10(i). The code designation for galvanizing is "V". If painting of the structural steel is not specified then the word "PAINTED" may be omitted, the month and year shall then correspond to the date the stencil is applied.

Basis of Payment. The cost of all surface preparation, galvanizing, painting and all other work described herein shall be considered as included in the unit price bid for the applicable pay items to be galvanized and painted, according to the Standard Specifications.

DRILLED SHAFTS

Effective: October 5, 2015

Revised: October 27, 2023

Revise Section 516 of the Standard Specifications to read:

“SECTION 516. DRILLED SHAFTS

516.01 Description. This work shall consist of constructing drilled shaft foundations.

516.02 Materials. Materials shall be according to the following.

| Item | Article/Section |
|---|-----------------|
| (a) Portland Cement Concrete (Note 1) | 1020 |
| (b) Reinforcement Bars..... | 1006.10 |
| (c) Grout (Note 2)..... | 1024.01 |
| (d) Permanent Steel Casing..... | 1006.05(d) |
| (e) Slurry (Note 3) | |

Note 1. When the soil contains sulfate contaminates, ASTM C 1580 testing will be performed to assess the severity of sulfate exposure to the concrete. If the sulfate contaminate is >0.10 to < 0.20 percent by mass, a Type II (MH) cement shall be used. If the sulfate contaminate is >0.20 to < 2.0 percent by mass, a Type V cement shall be used. If the sulfate contaminate is ≥ 2.0 percent by mass, refer to ACI 201.2R for guidance.

Note 2. The sand-cement grout mix shall be according to Section 1020 and shall be two to five parts sand and one part Type I or II cement. The maximum water cement ratio shall be sufficient to provide a flowable mixture with a typical slump of 10 in. (250 mm).

Note 3. Slurry shall be bentonite, emulsified polymer, or dry polymer, and shall be approved by the Engineer.

516.03 Equipment. Equipment shall be according to the following.

| Item | Article/Section |
|---|-----------------|
| (a) Concrete Equipment | 1020.03 |
| (b) Drilling Equipment (Note 1) | |
| (c) Hand Vibrator | 1103.17(a) |
| (d) Underwater Concrete Placement Equipment | 1103.18 |

Note 1. The drilling equipment shall have adequate capacity, including power, torque and down thrust, to create a shaft excavation of the maximum diameter specified to a depth of 20 percent beyond the depths shown on the plans.

516.04 Submittals. The following information shall be submitted on form BBS 133.

(a) Qualifications. At the time of the preconstruction conference, the Contractor shall provide the following documentation.

(1) References. A list containing at least three projects completed within the three years prior to this project's bid date which the Contractor performing this work has installed drilled shafts of similar diameter, length, and site conditions to those shown in the plans. The list of projects shall contain names and phone numbers of owner's representatives who can verify the Contractor's participation on those projects.

(2) Experience. Name and experience record of the drilled shaft supervisor, responsible for all facets of the shaft installation, and the drill operator(s) who will be assigned to this project. The supervisor and operator(s) shall each have a minimum of three years experience in the construction of drilled shafts.

(b) Installation Procedure. A detailed installation procedure shall be submitted to the Engineer for acceptance at least 28 days prior to drilled shaft construction and shall address each of the following items unless otherwise directed by the Engineer in writing.

(1) Equipment List. List of proposed equipment to be used including cranes, drill rigs, augers, belling tools, casing, vibratory hammers, core barrels, bailing buckets, final cleaning equipment, slurry equipment, tremies, or concrete pumps, etc.

(2) General Sequence. Details of the overall construction operation sequence, equipment access, and the sequence of individual shaft construction within each substructure bent or footing group. The submittal shall address the Contractor's proposed time delay and/or the minimum concrete strength necessary before initiating a shaft excavation adjacent to a recently installed drilled shaft.

(3) Shaft Excavation. A site specific step by step description of how the Contractor anticipates the shaft excavation to be advanced based on their evaluation of the subsurface data and conditions expected to be encountered. This sequence shall note the method of casing advancement, anticipated casing lengths, tip elevations and diameters, the excavation tools used and drilled diameters created. The Contractor shall indicate whether wet or dry drilling conditions are expected and if groundwater will be sealed from the excavation.

(4) Slurry. When the use of slurry is proposed, details on the types of additives to be used and their manufacturers shall be provided. In addition, details covering the measurement and control of the hardness of the mixing water, agitation, circulation, de-sanding, sampling, testing, and chemical properties of the slurry shall be submitted.

(5) Shaft Cleaning. Method(s) and sequence proposed for the shaft cleaning operation.

(6) Reinforcement Cage and Permanent Casing. Details of reinforcement placement including rolling spacers to be used and method to maintain proper elevation and location of the reinforcement cage within the shaft excavation during concrete placement. The method(s) of adjusting the reinforcement cage length and permanent casing if rock is encountered at an elevation other than as shown on the

plans. As an option, the Contractor may perform soil borings and rock cores at the drilled shaft locations to determine the required reinforcement cage and permanent casing lengths.

- (7) Concrete Placement. Details of concrete placement including proposed operational procedures for free fall, tremie or pumping methods. The sequence and method of casing removal shall also be stated along with the top of pour elevation, and method of forming through water above streambed.
- (8) Mix Design. The proposed concrete mix design(s).
- (9) Disposal Plan. Containment and disposal plan for slurry and displaced water. Containment and disposal plan for contaminated concrete pushed out of the top of the shaft by uncontaminated concrete during concrete placement.
- (10) Access and Site Protection Plan. Details of access to the drilled shafts and safety measures proposed. This shall include a list of casing, scaffolding, work platforms, temporary walkways, railings, and other items needed to provide safe access to the drilled shafts. Provisions to protect open excavations during non-working hours shall be included.

The Engineer will evaluate the drilled shaft installation procedure and notify the Contractor of acceptance, need for additional information, or concerns with the installation's effect on the existing or proposed structure(s).

CONSTRUCTION REQUIREMENTS

516.05 General. Excavation for drilled shaft(s) shall not proceed until written authorization is received from the Engineer. The Contractor shall be responsible for verification of the dimensions and alignment of each shaft excavation as directed by the Engineer.

Unless otherwise approved in the Contractor's installation procedure, no shaft excavation, casing installation, or casing removal with a vibratory hammer shall be made within four shaft diameters center to center of a shaft with concrete that has a compressive strength less than 1500 psi (10,300 kPa). The site-specific soil strengths and installation methods selected will determine the actual required minimum spacing, if any, to address vibration and blow out concerns.

Lost tools shall not remain in the shaft excavation without the approval of the Engineer.

Blasting shall not be used as a method of shaft excavation.

516.06 Shaft Excavation Protection Methods. The construction of drilled shafts may involve the use of one or more of the following methods to support the excavation during the various phases of shaft excavation, cleaning, and concrete placement dependent on the site conditions encountered. Surface water shall not flow uncontrolled into the shaft excavation, however water may be placed into the shaft excavation in order to meet head pressure requirements according to Articles 516.06(c) and 516.13.

The following are general descriptions indicating the conditions when these methods may be used.

- (a) Dry Method. The dry construction method shall only be used at sites where the groundwater and soil conditions are suitable to permit the drilling and dewatering of the excavation without causing subsidence of adjacent ground, boiling of the base soils, squeezing, or caving of the shaft side walls. The dry method shall consist of drilling the shaft excavation, removing accumulated water, cleaning the shaft base, and placing the reinforcement cage and concrete in a predominately dry excavation.

Slurry Method. The slurry construction method may be used at sites where dewatering the excavation would cause collapse of the shaft sidewalls or when the volume and head of water flowing into the shaft is likely to contaminate the concrete during placement resulting in a shaft defect. This method uses slurry, or in rare cases water, to maintain stability of the shaft sidewall while advancing the shaft excavation. After the shaft excavation is completed, the slurry level in the shaft shall be kept at an elevation to maintain stability of the shaft sidewall, maintain stability of the shaft base, and prevent additional groundwater from entering the shaft. The shaft base shall be cleaned, the reinforcement cage shall be set, and the concrete shall be discharged at the bottom of the shaft excavation, displacing the slurry upwards.

- (b) Temporary Casing Method. Temporary casing shall be used when either the dry or slurry methods provide inadequate support to prevent sidewall caving or excessive deformation of the shaft excavation. Temporary casing may be used with slurry or be used to reduce the flow of water into the excavation to allow dewatering and concrete placement in a dry shaft excavation. Temporary casing shall not be allowed to remain permanently without the approval of the Engineer.

During removal of the temporary casing, the level of concrete in the casing shall be maintained at a level such that the head pressure inside the casing is a minimum of 1.25 times the head pressure outside the casing, but in no case is less than 5 ft (1.5 m) above the bottom of the casing. Casing removal shall be at a slow, uniform rate with the pull in line with the shaft axis. Excessive rotation of the casing shall be avoided to limit deformation of the reinforcement cage. In addition, the slump requirements during casing removal shall be according to Article 516.12.

When called for on the plans, the Contractor shall install a permanent casing as specified. Permanent casing may be used as a shaft excavation support method or may be installed after shaft excavation is completed using one of the above methods. After construction, if voids are present between the permanent casing and the drilled excavation, the voids shall be filled with grout by means of tremie(s) or concrete pump which shall be lowered to the bottom of the excavation. The contractor's means and methods for grout placement shall fill the annular void(s) between the permanent casing and the surrounding earth material to restore and provide lateral earth resistance to the shaft. Grout yield checks shall be performed by the contractor for submittal to the Engineer. Permanent casing shall not remain in place beyond the limits shown on the plans without the specific approval of the Engineer.

When the shaft extends above the streambed through a body of water and permanent casing is not shown, the portion above the streambed shall be formed with removable casings, column forms, or other forming systems as approved by the Engineer. The forming system shall not scar or spall the finished concrete or leave in place any forms or casing within the removable form limits as shown on the plans unless approved as part of the installation procedure. The forming system shall not be removed until the concrete has attained a minimum compressive strength of 2500 psi (17,200 kPa) and cured for a minimum of 72 hours. For shafts extending through water, the concrete shall be protected from water action after placement for a minimum of seven days.

516.07 Slurry. When slurry is used, the Contractor shall provide a technical representative of the slurry additive manufacturer at the site prior to introduction of the slurry into the first shaft where slurry will be used, and during drilling and completion of a minimum of one shaft to adjust the slurry mix to the specific site conditions. During construction, the level of the slurry shall be maintained a minimum of 5 feet (1.5 m) above the height required to prevent caving of the shaft excavation. In the event of a sudden or significant loss of slurry in the shaft excavation, the construction of that foundation shall be stopped and the shaft excavation backfilled or supported by temporary casing, until a method to stop slurry loss, or an alternate construction procedure, has been approved by the Engineer.

- (a) General Properties. The material used to make the slurry shall not be detrimental to the concrete or surrounding ground. Mineral slurries shall have both a mineral grain size that remains in suspension and sufficient viscosity and gel characteristics to transport excavated material to a suitable screening system. Polymer slurries shall have sufficient viscosity and gel characteristics to transport excavated material to suitable screening systems or settling tanks. The percentage and specific gravity of the material used to make the slurry shall be sufficient to maintain the stability of the excavation and to allow proper concrete placement.

If approved by the Engineer, the Contractor may use water and excavated soils as drilling slurry. In this case, the range of acceptable values for density, viscosity and pH, as shown in the following table for bentonite slurry shall be met.

When water is used as the slurry to construct rock sockets in limestone, dolomite, sandstone or other formations that are not erodible, the requirements for slurry testing shall not apply if the entire fluid column is replaced with fresh water after drilling. To do so, fresh water shall be introduced at the top of the shaft excavation and existing water used during drilling shall be pumped out of the shaft excavation from the bottom of the shaft excavation until the entire volume of fluid has been replaced.

- (b) Preparation. Prior to introduction into the shaft excavation, the manufactured slurry admixture shall be pre-mixed thoroughly with clean, fresh water and for adequate time in accordance with the slurry admixture manufacturer's recommendations. Slurry tanks of adequate capacity shall be used for slurry mixing, circulation, storage and treatment. No excavated slurry pits will be allowed in lieu of slurry tanks without approval from the Engineer. Adequate desanding equipment shall be provided to control slurry properties during the drilled shaft excavation in accordance with the values provided in Table 1.

- (c) Quality Control. Quality control tests shall be performed on the slurry to determine density, viscosity, sand content and pH of freshly mixed slurry, recycled slurry and slurry in the shaft excavation. Tests of slurry samples from within two feet of the bottom and at mid-height of the shaft excavation shall be conducted in each shaft excavation during the excavation process to measure the consistency of the slurry. A minimum of four sets of tests shall be conducted during the first eight hours of slurry use on the project. When a series of four test results do not change more than 1% from the initial test, the testing frequency may be decreased to one set every four hours of slurry use. Reports of all tests, signed by an authorized representative of the Contractor, shall be furnished to the Engineer upon completion of each drilled shaft. The physical properties of the slurry shall be as shown in Table 1.

The slurry shall be sampled and tested less than 1 hour before concrete placement. Any heavily contaminated slurry that has accumulated at the bottom of the shaft shall be removed. The contractor shall perform final shaft bottom cleaning after suspended solids have settled from the slurry. Concrete shall not be placed if the slurry does not have the required physical properties.

| Table 1 – SLURRY PROPERTIES | | | | |
|---|--|------------------------|------------------------|----------------|
| | Bentonite | Emulsified Polymer | Dry Polymer | Test Method |
| Density, lb/cu ft (kg/cu m) (at introduction) | 65.2 ± 1.6 ¹ (1043.5 ± 25.6) | 63 (1009.0) max. | 63 (1009.0) max. | ASTM D 4380 |
| Density, lb/cu ft (kg/cu m) (prior to concrete placement) | 67.0 ± 3.5 ¹ (1073.0 ± 56.0) | 63 (1009.0) max. | 63 (1009.0) max. | ASTM D 4380 |
| Viscosity ² , sec/qt (sec/L) | 46 ± 14 (48 ± 14) | 38 ± 5 (40 ± 5) | 65 ± 15 (69 ± 16) | ASTM D 6910 |
| pH | 9.0 ± 1.0 | 9.5 ± 1.5 | 9.0 ± 2.0 | ASTM D 4972 |
| Sand Content, percent by volume (at introduction) | 4 max. | 1 max. | 1 max. | ASTM D 4381 |
| Sand Content, percent by volume (prior to concrete placement) | 10 max. | 1 max. | 1 max. | ASTM D 4381 |
| Contact Time ³ , hours | 4 max. | 72 max. | 72 max | |

Note 1. When the slurry consists of only water and excavated soils, the density shall not exceed 70 lb/cu ft (1121 kg/cu m).

Note 2. Higher viscosities may be required in loose or gravelly sand deposits.

Note 3. Contact time is the time without agitation and sidewall cleaning.

516.08 Obstructions. An obstruction is an unknown isolated object that causes the shaft excavation method to experience a significant decrease in the actual production rate and requires the Contractor to core, break up, push aside, or use other means to mitigate the obstruction. Subsurface conditions such as boulders, cobbles, or logs and buried infrastructure such as footings, piling, or abandoned utilities, when shown on the plans, shall not constitute an obstruction. When an obstruction is encountered, the Contractor shall notify the Engineer immediately and upon concurrence of the Engineer, the Contractor shall mitigate the obstruction with an approved method.

516.09 Top of Rock. The top of rock will be considered as the point where rock, defined as bedded deposits and conglomerate deposits exhibiting the physical characteristics and difficulty of rock removal as determined by the Engineer, is encountered which cannot be drilled with augers and/or underreaming tools configured to be effective in the soils indicated in the contract documents.

516.10 Design Modifications. If the top of rock elevation differs from that shown on the plans by more than 10 percent of the length of the drilled shaft above the rock, the Engineer shall be contacted to determine if any drilled shaft design changes may be required. In addition, if the type of soil or rock encountered is not similar to that shown in the subsurface exploration data, the Contractor may be required to extend the drilled shaft length(s) beyond those specified in the plans. In either case, the Engineer will determine if revisions are necessary and the extent of the modifications required.

516.11 Excavation Cleaning and Inspection. Materials removed or generated from the shaft excavations shall be disposed of according to Article 202.03.

After excavation, each shaft shall be cleaned. For a drilled shaft terminating in soil, the depth of sediment or debris shall be a maximum of 1 1/2 in. (38 mm). For a drilled shaft terminating in rock, the depth of sediment or debris shall be a maximum of 1/2 in. (13 mm).

A shaft excavation shall be overreamed when, in the opinion of the Engineer, the sidewall has softened, swelled, or has a buildup of slurry cake. Overreaming may also be required to correct a shaft excavation which has been drilled out of tolerance. Overreaming may be accomplished with a grooving tool, overreaming bucket, or other approved equipment. Overreaming thickness shall be a minimum of 1/2 in. (13 mm) and a maximum of 3 in. (75 mm).

516.12 Reinforcement. This work shall be according to Section 508 and the following.

The shaft excavation shall be cleaned and inspected prior to placing the reinforcement cage. The reinforcement cage shall be completely assembled prior to drilling and be ready for adjustment in length as required by the conditions encountered. The reinforcement cage shall be lifted using multiple point sling straps or other approved methods to avoid reinforcement cage distortion or stress. Cross frame stiffeners may be required for lifting or to keep the reinforcement cage in proper position during lifting and concrete placement.

The Contractor shall attach rolling spacers to keep the reinforcement cage centered within the shaft excavation during concrete placement and to ensure that at no point will the finished shaft have less than the minimum concrete cover(s) shown on the plans. The rolling spacers or other approved non-corrosive spacing devices shall be installed within 2 ft (0.6 m) of both the top and bottom of the drilled shaft and at intervals not exceeding 10 ft (3 m) throughout the length of the shaft to ensure proper reinforcement cage alignment and clearance for the entire

shaft. The number of rolling spacers at each level shall be one for each 1.0 ft (300 mm) of shaft diameter, with a minimum of four rolling spacers at each level. For shafts with different shaft diameters throughout the length of the excavation, different sized rolling spacers shall be provided to ensure the reinforcement cage is properly positioned throughout the entire length of the shaft.

When a specific concrete cover between the base of the drilled shaft and the reinforcement cage is shown on the plans, the bottom of the reinforcement cage shall be supported so that the proper concrete cover is maintained.

If the conditions differ such that the length of the shaft is increased, additional longitudinal bars shall be either mechanically spliced or lap spliced to the lower end of the reinforcement cage and confined with either hoop ties or spirals. The Contractor shall have additional reinforcement available or fabricate the reinforcement cages with additional length as necessary to make the required adjustments in a timely manner as dictated by the encountered conditions. The additional reinforcement may be non-epoxy coated.

516.13 Concrete Placement. Concrete work shall be performed according to the following.

Throughout concrete placement the head pressure inside the drilled shaft shall be at least 1.1 times the head pressure outside the drilled shaft.

Concrete placement shall begin within 1 hour of shaft cleaning and inspection. The pour shall be made in a continuous manner from the bottom to the top elevation of the shaft as shown on the contract plan or as approved in the Contractor's installation procedure. Concrete placement shall continue after the shaft excavation is full and until 18 in. (450 mm) of good quality, uncontaminated concrete is expelled at the top of shaft. Vibration of the concrete will not be allowed when the concrete is displacing slurry or water. In dry excavations, the concrete in the top 10 ft (3 m) of the shaft shall be vibrated.

When using temporary casing or placing concrete under water or slurry, a minimum of seven days prior to concrete placement, a 4 cu yd (3 cu m) trial batch of the concrete mixture shall be performed to evaluate slump retention. Temporary casing shall be withdrawn before the slump of the concrete drops below 6 in. (150 mm). For concrete placed using the slurry method of construction, the slump of all concrete placed shall be a minimum of 6 in. (150 mm) at the end of concrete placement.

Devices used to place concrete shall have no aluminum parts in contact with concrete.

When the top of the shaft is at the finished elevation and no further concrete placement above the finished elevation is specified, the top of the shaft shall be level and finished according to Article 503.15(a).

Concrete shall be placed by free fall, tremie, or concrete pump subject to the following conditions.

- (a) Free Fall Placement. Concrete shall only be placed by free fall when the rate of water infiltration into the shaft excavation is less than 12 in. (300 mm) per hour and the depth of water in the shaft excavation is less than 3 in. (75 mm) at the time of concrete placement.

Concrete placed by free fall shall fall directly to the base without contacting the reinforcement cage, cross frame stiffeners, or shaft sidewall. Drop chutes may be used to direct concrete to the base during free fall placement.

Drop chutes used to direct placement of free fall concrete shall consist of a smooth tube. Concrete may be placed through either a hopper at the top of the tube or side openings as the drop chute is retrieved during concrete placement. The drop chute shall be supported so that free fall does not exceed 60 ft (18.3 m) for conventional concrete or 30 ft (9.1 m) for self-consolidating concrete. If placement cannot be satisfactorily accomplished by free fall in the opinion of the Engineer, either a tremie or pump shall be used to accomplish the pour.

- (b) Tremie and Concrete Pump Placement. Concrete placement shall be according to Article 503.08, except the discharge end of the steel pipe shall remain embedded in the concrete a minimum of 10 ft (3.0 m) throughout concrete placement when displacing slurry or water.

516.14 Construction Tolerances. The following construction tolerances shall apply to all drilled shafts.

- (a) Center of Shaft. The center of the drilled shaft shall be within 3 in. (75 mm) of the plan station and offset at the top of the shaft.
- (b) Center of Reinforcement Cage. The center of the reinforcement cage shall be within 1 1/2 in. (40 mm) of plan station and offset at the top of the shaft.
- (c) Vertical Plumbness of Shaft. The out of vertical plumbness of the shaft shall not exceed 1.5 percent.
- (d) Vertical Plumbness of Reinforcement Cage. The out of vertical plumbness of the shaft reinforcement cage shall not exceed 0.83 percent.
- (e) Top of Shaft. The top of the shaft shall be no more than 1 in. (25 mm) above and no more than 3 in. (75 mm) below the plan elevation.
- (f) Top of Reinforcement Cage. The top of the reinforcement cage shall be no more than 1 in. (25 mm) above and no more than 3 in. (75 mm) below the plan elevation.
- (g) Bottom of shaft. Excavation equipment and methods used to complete the shaft excavation shall have a nearly planar bottom. The cutting edges of excavation equipment used to create the bottom of shafts in rock shall be normal to the vertical axis of the shaft within a tolerance of 6.25 percent.

516.15 Method of Measurement. This work will be measured for payment in place and the volume computed in cubic yards (cubic meters). The volume will be computed using the plan diameter of the shaft multiplied by the measured length of the shaft. The length of shaft in soil will be computed as the difference in elevation between the top of the drilled shaft shown on the plans, or as installed as part of the Contractor's installation procedure, and the bottom of the shaft or the top of rock (when present) whichever is higher. The length of shaft in rock will be computed as the difference in elevation between the measured top of rock and the bottom of the shaft.

When permanent casing is specified, it will be measured for payment in place, in feet (meters). Permanent casing installed at the Contractor's option will not be measured for payment.

Reinforcement furnished and installed will be measured for payment according to Article 508.07.

516.16 Basis of Payment. This work will be paid for at the contract unit price per cubic yard (cubic meter) for DRILLED SHAFT IN SOIL, and/or DRILLED SHAFT IN ROCK.

Permanent casing will be paid for at the contract unit price per foot (meter) for PERMANENT CASING.

Reinforcement furnished and installed will be paid for according to Article 508.08.

Obstruction mitigation will be paid for according to Article 109.04."

PREFORMED PAVEMENT JOINT SEAL

Effective: October 4, 2016

Revised: March 24, 2023

Description. This work shall consist of furnishing all labor, equipment and materials necessary to prepare the joint opening and install pavement joint seal(s) at the locations specified. Unless otherwise detailed on the plans, the joint shall be sized for a rated movement of 2 inches (50 mm).

Materials: Unless otherwise specified, one of the following prefabricated joint seals will be permitted.

- (a) Preformed Elastomeric Joint Seal. This material shall be according to Section 1053.01.
- (b) Preformed Pre-compressed, Silicone Coated, Self-Expanding Sealant System. This Sealant system shall be comprised of three components: 1) cellular polyurethane foam impregnated with hydrophobic 100% acrylic, water-based emulsion, factory coated with highway-grade, fuel resistant silicone; 2) field-applied epoxy adhesive primer, 3) field-injected silicone sealant bands.

The preformed, pre-compressed silicone joint seal shall, as a minimum, be according to the following:

- The joint seal shall be held in place by a non-sag, high modulus silicone adhesive.
- The joint seal shall be compatible with the epoxy and header material.
- The joint seal shall withstand the effects of vertical and lateral movements, skew movements and rotational movement without adhesive or cohesive failure.
- The joint seal shall be designed so that, the material is capable of movement of +50%, -50% (100% total) of nominal material size.
- The gland shall not contain any open, unsealed joints along its length in its final condition.
- Changes in plane and direction shall be executed using factory fabricated 90 degree transition assemblies. The transitions shall be watertight at the inside and outside corners through the full movement of the product.
- The depth of the joint shall be recessed 3/4 in. (19 mm) below the riding surface throughout the normal limits of joint movement.

- The joint seal shall be resistant to ultraviolet rays.
- The joint seal shall be resistant to abrasion, oxidation, oils, gasoline, salt, and other materials that may be spilled on or applied to the surface.
- The manufacturer shall certify that the joint composition shall be free of any waxes or wax compounds; asphalts or asphalt compounds.

The joint material shall meet the following physical properties:

| Property | Requirement | Test Method |
|--|-------------------------------|-------------|
| Tensile Strength of Silicone Coating (min) | 140 psi | ASTM D 412 |
| UV Resistance of Joint System | No Changes--2000 Hours | ASTM C793 |
| Density of Cellular Polyurethane Foam | 4.0 lb/ cu ft (200kg/cu m) | ASTM D545 |
| Heat Aging Effects (Silicone Coating) | No cracking, chalking | ASTM C 792 |
| Joint System Operating temp range (min) | -40° F to 185° F | ASTM C 711 |

The adhesive shall be a two-component, 100% solid, modified epoxy meeting the requirements of ASTM C881, Type I, Grade 3, Class B & C. The adhesive shall also have the following properties:

| Property | Requirement | Test method |
|--------------------------|-------------------------|-------------|
| Tensile Strength | 2,500 psi (24 MPa) min. | ASTM D638 |
| Compressive Strength | 7000 psi (48 MPa) min. | ASTM D695 |
| Bond Strength (Dry Cure) | 2000 psi (28MPa) min | ASTM C882 |
| Water Absorption | 0.1% by weight | ASTM D570 |

The silicone band adhesive shall have the following properties:

| Property | Requirement | Test Method |
|-------------------------|---|-------------|
| Movement Capability | +50/-50% | ASTM C 719 |
| Elongation at Break | >600% | ASTM D 5893 |
| Slump | ≤0.3" | ASTM D 2202 |
| Hardness (Shore A) max. | 20 | ASTM C 661 |
| Tack free time (max) | 60 minutes | ASTM C 679 |
| Heat Aging Effects | No cracking, chalking | ASTM C 792 |
| Resilience | ≥ 75% | ASTM D5329 |
| Bond | 0% Adhesive or Cohesive Failure after 5 cycles @100%extension | ASTM D 5329 |

- (c) Performed Silicone Joint Seal. The preformed silicone joint seal used for this item shall conform to the following specifications:

Table 1
Physical Properties of Preformed Silicone Gland

| Property | Requirement | Test Method |
|----------------------------------|---------------------------|-------------|
| Rated Movement Capability | +2 ¼ inch total | N/A |
| Tensile Strength, psi. | 1000 min | ASTM D 412 |
| Elongation | 400% min | ASTM D 412 |
| Tear (die B) | 100 ppi. min | ASTM D 624 |
| Hardness Durometer (Shore A). | 55 +/- 5 max | ASTM D 2240 |
| Compression set at 212°F, 70 hrs | 30% max | ASTM D 395 |
| Heat Aged Properties | 5pt max loss on Durometer | ASTM D 573 |
| Tensile and Elongation % Loss | 10 % max | |

The color of the preformed silicone seal shall be black, made by the addition of Carbon Black fillers which increases UV resistance, tensile strength, and abrasion wear properties.

The locking adhesive shall be non-sag, high modulus silicone adhesive conforming to the following specifications:

Table 2
Physical Properties of the Silicone Locking Adhesive

| Property | Requirement | Test Method |
|--------------------------|---------------------------------------|-------------|
| Tensile Strength, psi. | 200 min | ASTM D 412 |
| Elongation, % | 450 min | ASTM D 412 |
| Tack Free Time, minutes. | 20 max. | ASTM C 679 |
| Cure Time ¼" bead, hrs | 24 max | ASTM C 679 |
| Resistance to U.V. | No cracking, chalking, or degradation | ASTM C793 |
| VOC (g/L) | 0 | ASTM D 3960 |

Any rips, tears, or bond failure will be cause for rejection.

The two part epoxy primer shall be supplied for application to the vertical faces of the joint opening. The supplied primer shall be equally as effective when bonded to concrete or steel. This primer shall meet the following criteria:

Table 3
Physical Properties of Preformed Silicone Joint System Primer

| Property | Requirement | Test Method |
|----------------------------------|-------------|---------------------------------------|
| Viscosity (cps) | 44 | ASTM D 2196 |
| Color | Light Amber | Visual |
| Solids (%) | 41 | ASTM D 4209 |
| Specific Gravity | 0.92 | ASTM D 1217 |
| Product Flash Point (°F, T.C.C.) | 48 | ASTM D 56 |
| Package Stability | N/A | One year in tightly sealed containers |
| Cleaning | N/A | Mineral Spirits |
| VOC (g/L) | 520 | ASTM D 3960 |

- (d) Preformed Inverted EPDM Joint Seal. The preformed inverted EPDM joint seal used for this item shall conform to the following specifications:

Table 1
Physical Properties of Preformed Silicone Gland

| Property | Requirement | Test Method |
|----------------------------------|--------------------|-------------|
| Rated Movement Capability | Up To 5 inch total | N/A |
| Tensile Strength, psi. | 1200 psi min | ASTM D 412 |
| Elongation | 400 % min | ASTM D 412 |
| Tear (Die C) | 150 pli. min | ASTM D 624 |
| Durometer Content | 50 +/- 5 max | ASTM D 2240 |
| Water Resistance (70 hrs @ 100c) | 10% max | ASTM D 471 |
| Ozone Resistance | 100 min | ASTM D 1171 |

Table 2
Physical Properties of the V-Epoxy-R

V-Epoxy-R adhesive meets the requirements of ASTM C881 Type III, Grade 2. The adhesive shall also have the following properties:

| Property | Requirement | Test Method |
|---|---------------------------------------|-------------|
| Color | Gray | Visual |
| Viscosity | 45,000 CP (typ.) | N/A |
| Gel Time (minutes) | 30 min. | ASTM C 881 |
| Shelf Life (Separate Sealed Containers) | 12 Months | N/A |
| Resistance to U.V. | No cracking, chalking, or degradation | ASTM C793 |
| VOC (g/L) | 0 | ASTM D 3960 |

Any rips, tears, or bond failure will be cause for rejection.

- (e) Bonded Preformed Joint Seal. This joint system shall consist of preformed elastomeric seal bonded to the side walls of the joint opening using an adhesive as specified by the Manufacturer of the joint seal.

The bonded preformed joint seal shall be according to Table 1 of ASTM D2628 with the following exceptions: Compression set shall not be over 40 percent when tested according to Method B (Modified) of ASTM D 395 after 70 hours at 212 °F (100 °C). The Compression-Deflection requirement will not apply to the bonded preformed joint seal.

The adhesive shall be epoxy base, dual component, which resists salt, diluted acids, alkalis, solvents, greases, oils, moisture, sunlight and weathering. Temperatures up to 200 °F (93 °C) shall not reduce bond strength. At 68 °F (20 °C), the bond strength shall be a minimum of 1000 psi (6.9 MPa) within 24 hours.

Any primers or cleaning solutions used on the faces of the joint or on the profile of the sides of the bonded preformed joint seal shall be supplied by the manufacturer of the bonded preformed joint seal.

Any additional installation materials and adhesive for splicing joint sections shall be as supplied by the manufacturer of the preformed joint seal.

The Contractor shall submit the Manufacturer's material certification documentation stating that their materials meet the applicable requirements of this specification for the joint seal(s) installed.

CONSTRUCTION REQUIREMENTS

General. The Contractor shall furnish the Engineer with the manufacturer's product information and installation procedures at least two weeks prior to installation.

The minimum ambient air temperature in which the joint seal can be installed is 40° F (4.4° C) and rising, except for bonded preformed joint seals which shall not be installed when temperatures below 50 °F (10 °C) are predicted within a 48 hour period.

The joint surface shall be completely dry before installing the Joint Seal. For newly placed concrete, the concrete shall be fully cured and allowed to dry out a minimum of seven additional days prior to placement of the seal. Cold, wet, inclement weather will require an extended drying time.

The Joint Seal shall not be installed immediately after precipitation or if precipitation is forecasted for the day. Joint preparation and installation of Joint Seal shall be done during the same day.

Surface Preparation. Surface preparation shall be according to the joint seal manufacturer's written instructions.

After surface preparation is completed, the joint shall be cleaned of debris using compressed air with a minimum pressure of 90 psi (620 kPa). The air compressor shall be equipped with traps to prevent the inclusion of water and/or oil in the air line. The compressed air shall be according to the cleanliness requirements of ASTM D 4285.

When priming is required per the manufacturer's instruction, this operation shall immediately follow cleaning.

Joint Installation. The Joint installation shall be per the manufacturer's instructions; special attention shall be given to ensure the joint seal is properly recessed below the top of the riding surface as recommended by the manufacturer.

For bonded joint seals the seal shall be inserted into the joint and held tightly against both sides of the joint until sufficient bond strength has been developed to resist the expected expansion forces.

Opening to traffic. As these joint systems are supposed to be recessed below the top of the riding surface, there should be no restriction, based on the joint seal installation, on when these joints can be reopened to traffic.

Method of Measurement. The installed prefabricated joint seal will not be measured for payment.

Basis of Payment. The prefabricated joint seal will not be paid for separately but shall be considered included in the cost of the adjacent concrete work involved.

ERECTION OF BRIDGE GIRDERS OVER OR ADJACENT TO RAILROADS

Effective: August 9, 2019

Description: In addition to the requirements of Article 504.06(d) and 505.08(e), the following shall apply.

The Contractor or sub-Contractor performing the erection of steel or concrete beams or girders over, or adjacent to (within 25 ft. of), active railroad tracks shall submit an erection plan to the Engineer for approval prior to starting the work.

Erection Plan: The Erection Contractor shall retain the services of an Illinois Licensed Structural Engineer for the completion of a project-specific erection plan. The structural engineer, herein referred to as the Erection Engineer, shall sign and seal the erection plan, drawings, and calculations for the proposed erection of the structural beams or girders.

The erection plan shall be complete in detail for all phases, stages, and conditions anticipated during erection. The erection plan shall include structural calculations and supporting documentation necessary to completely describe and document the means, methods, temporary support positions, and loads necessary to safely erect the structural members in conformance with the contract documents and as outlined herein. The erection plans shall address and account for all items pertinent to the erection including such items as sequencing, falsework, temporary shoring and/or bracing, girder stability, crane positioning and movement, means of access, pick points, girder shape, permissible deformations and roll, interim/final plumbness, cross frame/diaphragm placement and connections, bolting and anchor bolt installation sequences and procedures, and blocking and anchoring of bearings. The Erection Contractor shall be responsible for the stability of the partially erected structure during all phases of erection.

The erection plans and procedures shall be submitted to the Engineer for review and acceptance prior to starting the work. Review, acceptance and/or comments by the Department shall not be construed to guarantee the safety or final acceptability of the work or compliance with all applicable specifications, codes, or contract requirements, and shall neither relieve the Contractor of the responsibility and liability to comply with these requirements, nor create liability for the Department. Significant changes to the erection plan in the field must be approved by the Erection Engineer and accepted by the Engineer for the Department.

Basis of Payment: This work shall not be paid for separately but shall be included in the applicable pay items according to Article 504.08 or 505.13 of the Standard Specifications.

BAR SPLICERS, HEADED REINFORCEMENT

Effective: September 2, 2022

Revised: October 27, 2023

Add the following to Article 508.08(b):

When bar splicers are epoxy-coated, all damaged or uncoated areas near the threaded ends shall be coated with a two-part epoxy according to ASTM D 3963 (D 3963M). All threaded ends of Stage II construction threaded splicer bars shall be coated according to ASTM D 3963 or dipped in an epoxy-mastic primer prior to joining the Stage II construction threaded splicer bar to the threaded coupler.

Add the following Article 508.02 (d)

Bar Terminators1006.10(a)(1)h

Add the following paragraph after Article 508.08 (c):

Bar terminators are threaded, headed attachments to reinforcement to form headed reinforcement. When specified on the plans, a bar terminator shall be attached to the designated reinforcement for development.

Add the following 4th paragraph to Article 508.11:

Bar Terminators will be paid for at the contract unit price per each for BAR TERMINATORS.

Add the following to Article 1006.10(a)(1)g:

For bar splicers with welded connections between the threaded coupler and threaded rod, the Stage I construction threaded splicer bar shall be welded to the threaded coupler using an all-around fillet weld.

Add the following Article 1006.10(a)(1)h:

Bar Terminators. Designated bars shall use a bar terminator to form headed reinforcement. Headed reinforcement shall conform to ASTM A970 with threaded attachment; Class HA; and reinforcement bars conforming to ASTM A706, except the connection strength of the bar terminator to the reinforcement bar shall meet, in tension, at least 125 percent of the specified yield strength of the reinforcement bar. The bar terminator shall be on the Department's qualified product list.

When the reinforcement bar to receive the bar terminator is epoxy coated, the bar terminator shall also be epoxy coated according to ASTM A 775 (A 775M)

AGGREGATE SUBGRADE IMPROVEMENT (BDE)

Effective: April 1, 2012

Revised: April 1, 2022

Add the following Section to the Standard Specifications:

“SECTION 303. AGGREGATE SUBGRADE IMPROVEMENT

303.01 Description. This work shall consist of constructing an aggregate subgrade improvement (ASI).

303.02 Materials. Materials shall be according to the following.

| Item | Article/Section |
|--|-----------------|
| (a) Coarse Aggregate | 1004.07 |
| (b) Reclaimed Asphalt Pavement (RAP) | 1031.09 |

303.03 Equipment. The vibratory roller shall be according to Article 1101.01, or as approved by the Engineer. Vibratory machines, such as tampers, shall be used in areas where rollers do not fit.

303.04 Soil Preparation. The minimum immediate bearing value (IBV) of the soil below the improved subgrade shall be according to the Department’s “Subgrade Stability Manual” for the aggregate thickness specified.

303.05 Placing and Compacting. The maximum nominal lift thickness of aggregate gradations CA 2, CA 6, and CA 10 when compacted shall be 9 in. (225 mm). The maximum nominal lift thickness of aggregate gradations CS 1, CS 2, and RR 1 when compacted shall be 24 in. (600 mm).

The top surface of the aggregate subgrade improvement shall consist of a layer of capping aggregate gradations CA 6 or CA 10 that is 3 in. (75 mm) thick after compaction. Capping aggregate will not be required when aggregate subgrade improvement is used as a cubic yard pay item for undercut applications.

Each lift of aggregate shall be compacted to the satisfaction of the Engineer. If the moisture content of the material is such that compaction cannot be obtained, sufficient water shall be added so that satisfactory compaction can be obtained.

303.06 Finishing and Maintenance. The aggregate subgrade improvement shall be finished to the lines, grades, and cross sections shown on the plans, or as directed by the Engineer. The aggregate subgrade improvement shall be maintained in a smooth and compacted condition.

303.07 Method of Measurement. This work will be measured for payment according to Article 311.08.

303.08 Basis of Payment. This work will be paid for at the contract unit price per cubic yard (cubic meter) or ton (metric ton) for AGGREGATE SUBGRADE IMPROVEMENT or at the contract unit price per square yard (square meter) for AGGREGATE SUBGRADE IMPROVEMENT, of the thickness specified.”

Add the following to Section 1004 of the Standard Specifications:

“1004.07 Coarse Aggregate for Aggregate Subgrade Improvement (ASI). The aggregate shall be according to Article 1004.01 and the following.

- (a) Description. The coarse aggregate shall be crushed gravel, crushed stone, or crushed concrete. In applications where greater than 24 in. (600 mm) of ASI material is required, gravel may be used below the top 12 in (300 mm) of ASI.
- (b) Quality. The coarse aggregate shall consist of sound durable particles reasonably free of deleterious materials.
- (c) Gradation.
 - (1) The coarse aggregate gradation for total ASI thickness less than or equal to 12 in. (300 mm) shall be CA 2, CA 6, CA 10, or CS 1.

The coarse aggregate gradation for total ASI thickness greater than 12 in. (300 mm) shall be CS 1 or CS 2 as shown below or RR 1 according to Article 1005.01(c).

| Grad No. | COARSE AGGREGATE SUBGRADE GRADATIONS | | | | |
|----------|--------------------------------------|--------|---------|---------|---------|
| | Sieve Size and Percent Passing | | | | |
| | 8" | 6" | 4" | 2" | #4 |
| CS 1 | 100 | 97 ± 3 | 90 ± 10 | 45 ± 25 | 20 ± 20 |
| CS 2 | | 100 | 80 ± 10 | 25 ± 15 | |

| Grad No. | COARSE AGGREGATE SUBGRADE GRADATIONS (Metric) | | | | |
|----------|---|--------|---------|---------|---------|
| | Sieve Size and Percent Passing | | | | |
| | 200 mm | 150 mm | 100 mm | 50 mm | 4.75 mm |
| CS 1 | 100 | 97 ± 3 | 90 ± 10 | 45 ± 25 | 20 ± 20 |
| CS 2 | | 100 | 80 ± 10 | 25 ± 15 | |

- (2) Capping aggregate shall be gradation CA 6 or CA 10.”

Add the following to Article 1031.09 of the Standard Specifications:

“(b) RAP in Aggregate Subgrade Improvement (ASI). RAP in ASI shall be according to Articles 1031.01(a), 1031.02(a), 1031.06(a)(1), and 1031.06(a)(2), and the following.

- (1) The testing requirements of Article 1031.03 shall not apply.
- (2) Crushed RAP used for the lower lift may be mechanically blended with aggregate gradations CS 1, CS 2, and RR 1 but it shall be no greater than 40 percent of the total product volume. RAP agglomerations shall be no greater than 4 in. (100 mm).
- (3) For capping aggregate, well graded RAP having 100 percent passing the 1 1/2 in. (38 mm) sieve may be used when aggregate gradations CS 1, CS 2, CA 2, or RR 1 are used in the lower lift. FRAP will not be permitted as capping material.

Blending shall be through calibrated interlocked feeders or a calibrated blending plant such that the prescribed blending percentage is maintained throughout the blending process. The calibration shall have an accuracy of ± 2.0 percent of the actual quantity of material delivered."

BITUMINOUS MATERIALS COST ADJUSTMENTS (BDE)

Effective: November 2, 2006

Revised: August 1, 2017

Description. Bituminous material cost adjustments will be made to provide additional compensation to the Contractor, or credit to the Department, for fluctuations in the cost of bituminous materials when optioned by the Contractor. The bidder shall indicate with their bid whether or not this special provision will be part of the contract.

The adjustments shall apply to permanent and temporary hot-mix asphalt (HMA) mixtures, bituminous surface treatments (cover and seal coats), and preventative maintenance type surface treatments that are part of the original proposed construction, or added as extra work and paid for by agreed unit prices. The adjustments shall not apply to bituminous prime coats, tack coats, crack filling/sealing, joint filling/sealing, or extra work paid for at a lump sum price or by force account.

Method of Adjustment. Bituminous materials cost adjustments will be computed as follows.

$$CA = (BPI_P - BPI_L) \times (\%AC_V / 100) \times Q$$

Where: CA = Cost Adjustment, \$.

BPI_P = Bituminous Price Index, as published by the Department for the month the work is performed, \$/ton (\$/metric ton).

BPI_L = Bituminous Price Index, as published by the Department for the month prior to the letting for work paid for at the contract price; or for the month the agreed unit price letter is submitted by the Contractor for extra work paid for by agreed unit price, \$/ton (\$/metric ton).

%AC_V = Percent of virgin Asphalt Cement in the Quantity being adjusted. For HMA mixtures, the % AC_V will be determined from the adjusted job mix formula. For bituminous materials applied, a performance graded or cutback asphalt will be considered to be 100% AC_V and undiluted emulsified asphalt will be considered to be 65% AC_V.

Q = Authorized construction Quantity, tons (metric tons) (see below).

For HMA mixtures measured in square yards: $Q, \text{ tons} = A \times D \times (G_{mb} \times 46.8) / 2000$. For HMA mixtures measured in square meters: $Q, \text{ metric tons} = A \times D \times (G_{mb} \times 1) / 1000$. When computing adjustments for full-depth HMA pavement, separate calculations will be made for the binder and surface courses to account for their different G_{mb} and % AC_V.

For bituminous materials measured in gallons: $Q, \text{ tons} = V \times 8.33 \text{ lb/gal} \times SG / 2000$

For bituminous materials measured in liters: $Q, \text{ metric tons} = V \times 1.0 \text{ kg/L} \times SG / 1000$

Where: A = Area of the HMA mixture, sq yd (sq m).
 D = Depth of the HMA mixture, in. (mm).
 G_{mb} = Average bulk specific gravity of the mixture, from the approved mix design.
 V = Volume of the bituminous material, gal (L).
 SG = Specific Gravity of bituminous material as shown on the bill of lading.

Basis of Payment. Bituminous materials cost adjustments may be positive or negative but will only be made when there is a difference between the BPI_L and BPI_P in excess of five percent, as calculated by:

$$\text{Percent Difference} = \{(BPI_L - BPI_P) \div BPI_L\} \times 100$$

Bituminous materials cost adjustments will be calculated for each calendar month in which applicable bituminous material is placed; and will be paid or deducted when all other contract requirements for the work placed during the month are satisfied. The adjustments shall not apply during contract time subject to liquidated damages for completion of the entire contract.

CEMENT, FINELY DIVIDED MINERALS, ADMIXTURES; CONCRETE, AND MORTAR (BDE)

Effective: January 1, 2025

Revise the first paragraph of Article 285.05 of the Standard Specifications to read:

“285.05 Fabric Formed Concrete Revetment Mat. The grout shall consist of a mixture of cement, fine aggregate, and water so proportioned and mixed as to provide a pumpable slurry. Fly ash or ground granulated blast furnace (GGBF) slag, and concrete admixtures may be used at the option of the Contractor. The grout shall have an air content of not less than 6.0 percent nor more than 9.0 percent of the volume of the grout. The mix shall obtain a compressive strength of 2500 psi (17,000 kPa) at 28 days according to Article 1020.09.”

Revise Article 302.02 of the Standard Specifications to read:

“302.02 Materials. Materials shall be according to the following.

| Item | Article/Section |
|---|-----------------|
| (a) Cement | 1001 |
| (b) Water | 1002 |
| (c) Hydrated Lime | 1012.01 |
| (d) By-Product, Hydrated Lime | 1012.02 |
| (e) By-Product, Non-Hydrated Lime | 1012.03 |
| (f) Lime Slurry | 1012.04 |
| (g) Fly Ash | 1010 |
| (h) Soil for Soil Modification (Note 1) | 1009.01 |
| (i) Bituminous Materials (Note 2) | 1032 |

Note 1. This soil requirement only applies when modifying with lime (slurry or dry).

Note 2. The bituminous materials used for curing shall be emulsified asphalt RS-2, CRS-2, HFE 90, or HFE 150; rapid curing liquid asphalt RC-70; or medium curing liquid asphalt MC-70 or MC-250.”

Revise Article 312.07(c) of the Standard Specifications to read:

“(c) Cement1001”

Add Article 312.07(i) of the Standard Specifications to read:

“(i) Ground Granulated Blast Furnace (GGBF) Slag1010”

Revise the first paragraph of Article 312.09 of the Standard Specifications to read:

“312.09 Proportioning and Mix Design. At least 60 days prior to start of placing CAM II, the Contractor shall submit samples of materials to be used in the work for proportioning and testing. The mixture shall contain a minimum of 200 lb (120 kg) of cement per cubic yard (cubic meter). Cement may be replaced with fly ash or ground granulated blast furnace (GGBF) slag according to Article 1020.05(c)(1) or 1020.05(c)(2), respectively, however the minimum cement content in the mixture shall be 170 lbs/cu yd (101 kg/cu m). Blends of coarse and fine aggregates will be permitted, provided the volume of fine aggregate does not exceed the volume of coarse aggregate. The Engineer will determine the proportions of materials for the mixture according to the “Portland Cement Concrete Level III Technician Course” manual. However, the Contractor may substitute their own mix design. Article 1020.05(a) shall apply, and a Level III PCC Technician shall develop the mix design.”

Revise Article 352.02 of the Standard Specifications to read:

“352.02 Materials. Materials shall be according to the following.

| Item | Article/Section |
|--|-----------------|
| (a) Cement (Note 1) | 1001 |
| (b) Soil for Soil-Cement Base Course | 1009.03 |
| (c) Water | 1002 |
| (d) Bituminous Materials (Note 2) | 1032 |

Note 1. Bulk cement may be used for the traveling mixing plant method if the equipment for handling, weighing, and spreading the cement is approved by the Engineer.

Note 2. The bituminous materials used for curing shall be emulsified asphalt RS-2, CRS-2, HFE 90, or HFE 150; rapid curing liquid asphalt RC-70; or medium curing liquid asphalt MC-70 or MC-250.”

Revise Article 404.02 of the Standard Specifications to read:

“404.02 Materials. Materials shall be according to the following.

| Item | Article/Section |
|---|-----------------|
| (a) Cement | 1001 |
| (b) Water | 1002 |
| (c) Fine Aggregate | 1003.08 |
| (d) Bituminous Material (Tack Coat) | 1032.06 |
| (e) Emulsified Asphalts (Note 1) (Note 2) | 1032.06 |
| (f) Fiber Modified Joint Sealer | 1050.05 |
| (g) Additives (Note 3) | |

Note 1. When used for slurry seal, the emulsified asphalt shall be CQS-1h according to Article 1032.06(b).

Note 2. When used for micro-surfacing, the emulsified asphalt shall be CQS-1hP according to Article 1032.06(e).

Note 3. Additives may be added to the emulsion mix or any of the component materials to provide the control of the quick-traffic properties. They shall be included as part of the mix design and be compatible with the other components of the mix.

Revise the last sentence of the fourth paragraph of Article 404.08 of the Standard Specifications to read:

“When approved by the Engineer, the sealant may be dusted with fine sand, cement, or mineral filler to prevent tracking.”

Revise Note 2 of Article 516.02 of the Standard Specifications to read:

“Note 2. The sand-cement grout mix shall be according to Section 1020 and shall be a 1:1 blend of sand and cement comprised of a Type I, IL, or II cement at 185 lb/cu yd (110 kg/cu m). The maximum water cement ratio shall be sufficient to provide a flowable mixture with a typical slump of 10 in. (250 mm).”

Revise Note 2 of Article 543.02 of the Standard Specifications to read:

“Note 2. The grout mixture shall be 6.50 hundredweight/cu yd (385 kg/cu m) of cement plus fine aggregate and water. Fly ash or ground granulated blast furnace (GGBF) slag may replace a maximum of 5.25 hundredweight/cu yd (310 kg/cu m) of the cement. The water/cement ratio, according to Article 1020.06, shall not exceed 0.60. An air-entraining admixture shall be used to produce an air content, according to Article 1020.08, of not less than 6.0 percent nor more than 9.0 percent of the volume of the grout. The Contractor shall have the option to use a water-reducing or high range water-reducing admixture.”

Revise Article 583.01 of the Standard Specifications to read:

“583.01 Description. This work shall consist of placing cement mortar along precast, prestressed concrete bridge deck beams as required for fairing out any unevenness between adjacent deck beams prior to placing of waterproofing membrane and surfacing.”

Revise Article 583.02(a) of the Standard Specifications to read:

“(a) Cement1001”

Revise the first paragraph of Article 583.03 of the Standard Specifications to read:

“ 583.03 General. This work shall only be performed when the air temperature is 45 °F (7 °C) and rising. The mixture for cement mortar shall consist of three parts sand to one part cement by volume. The amount of water shall be no more than that necessary to produce a workable, plastic mortar.”

Revise Note 2/ in Article 1003.01(b) of the Standard Specifications to read:

“2/ Applies only to sand. Sand exceeding the colorimetric test standard of 11 (Illinois Modified AASHTO T 21) will be checked for mortar making properties according to Illinois Modified ASTM C 87 and shall develop a compressive strength at the age of 14 days when using Type I, IL, or II cement of not less than 95 percent of the comparable standard.

Revise the second sentence of Article 1003.02(e)(1) of the Standard Specifications to read:

“The test will be performed with Type I, IL, or II portland cement having a total equivalent alkali content ($\text{Na}_2\text{O} + 0.658\text{K}_2\text{O}$) of 0.90 percent or greater.”

Revise the first sentence of the second paragraph of Article 1003.02(e)(3) of the Standard Specifications to read:

“The ASTM C 1293 test shall be performed with Type I, IL, or II portland cement having a total equivalent alkali content ($\text{Na}_2\text{O} + 0.658\text{K}_2\text{O}$) of 0.80 percent or greater.”

Revise the second sentence of Article 1004.02(g)(1) of the Standard Specifications to read:

“The test will be performed with Type I, IL, or II portland cement having a total equivalent alkali content ($\text{Na}_2\text{O} + 0.658\text{K}_2\text{O}$) of 0.90 percent or greater.”

Revise Article 1017.01 of the Standard Specifications to read:

“1017.01 Requirements. The mortar shall be high-strength according to ASTM C 387 and shall have a minimum 80.0 percent relative dynamic modulus of elasticity when tested by the Department according to Illinois Modified AASHTO T 161 or AASHTO T 161 when tested by an independent lab. The high-strength mortar shall have a water-soluble chloride ion content of less than 0.40 lb/cu yd (0.24 kg/cu m). The test shall be performed according to ASTM C 1218, and the high-strength mortar shall have an age of 28 to 42 days at the time of test. The ASTM C 1218 test shall be performed by an independent lab a minimum of once every five years, and the test results shall be provided to the Department. Mixing of the high-strength mortar shall be according to the manufacturer’s specifications. The Department will maintain a qualified product list.”

Revise the fourth sentence of Article 1018.01 of the Standard Specifications to read:

“The ASTM C 1218 test shall be performed by an independent lab a minimum of once every five years, and the test results shall be provided to the Department.”

Revise Article 1019.02 of the Standard Specifications to read:

“1019.02 Materials. Materials shall be according to the following.

| Item | Article/Section |
|--|-----------------|
| (a) Cement | 1001 |
| (b) Water | 1002 |
| (c) Fine Aggregate for Controlled Low-Strength Material (CLSM) | 1003.06 |
| (d) Fly Ash | 1010 |
| (e) Ground Granulated Blast Furnace (GGBF) Slag..... | 1010 |

(f) Admixtures (Note 1)

Note 1. The air-entraining admixture may be in powder or liquid form. Prior to approval, a CLSM air-entraining admixture will be evaluated by the Department. The admixture shall be able to meet the air content requirements of Mix 2. The Department will maintain a qualified product list.”

Revise Article 1019.05 of the Standard Specifications to read:

“1019.05 Department Mix Design. The Department mix design shall be Mix 1, 2, or 3 and shall be proportioned to yield approximately one cubic yard (cubic meter).

| Mix 1 | |
|--|-----------------------|
| Cement | 50 lb (30 kg) |
| Fly Ash – Class C or F, and/or GGBF Slag | 125 lb (74 kg) |
| Fine Aggregate – Saturated Surface Dry | 2900 lb (1720 kg) |
| Water | 50-65 gal (248-322 L) |
| Air Content | No air is entrained |

| Mix 2 | |
|--|-----------------------|
| Cement | 125 lb (74 kg) |
| Fine Aggregate – Saturated Surface Dry | 2500 lb (1483 kg) |
| Water | 35-50 gal (173-248 L) |
| Air Content | 15-25 % |

| Mix 3 | |
|--|-----------------------|
| Cement | 40 lb (24 kg) |
| Fly Ash – Class C or F, and/or GGBF Slag | 125 lb (74 kg) |
| Fine Aggregate – Saturated Surface Dry | 2500 lb (1483 kg) |
| Water | 35-50 gal (179-248 L) |
| Air Content | 15-25 %” |

Revise Article 1020.04, Table 1, Note (8) of the Standard Specifications to read:

“(8) In addition to the Type III portland cement, 100 lb/cu yd of ground granulated blast-furnace slag and 50 lb/cu yd of microsilica (silica fume) shall be used. For an air temperature greater than 85 °F, the Type III portland cement may be replaced with Type I, IL, or II portland cement.”

Revise Article 1020.04, Table 1 (Metric), Note (8) of the Standard Specifications to read:

“(8) In addition to the Type III portland cement, 60 kg/cu m of ground granulated blast-furnace slag and 30 kg/cu m of microsilica (silica fume) shall be used. For an air temperature greater than 30 °C, the Type III portland cement may be replaced with Type I, IL, or II portland cement.”

Revise the second paragraph of Article 1020.05(a) of the Standard Specifications to read:

“For a mix design using a portland-pozzolan cement, portland blast-furnace slag cement, portland-limestone cement, or replacing portland cement with finely divided minerals per Articles 1020.05(c) and 1020.05(d), the Contractor may submit a mix design with a minimum portland cement content less than 400 lbs/cu yd (237 kg/cu m), but not less than 375 lbs/cu yd (222 kg/cu m), if the mix design is shown to have a minimum relative dynamic modulus of elasticity of 80 percent determined according to AASHTO T 161. Testing shall be performed by an independent laboratory accredited by AASHTO re:source for Portland Cement Concrete.”

Revise the first sentence of the first paragraph of Article 1020.05(b) of the Standard Specifications to read:

“Corrosion inhibitors and concrete admixtures shall be according to the qualified product lists.”

Delete the fourth and fifth sentences of the second paragraph of Article 1020.05(b) of the Standard Specifications.

Revise the third sentence of the second paragraph of Article 1020.05(b)(5) of the Standard Specifications to read:

“The qualified product lists of concrete admixtures shall not apply.”

Revise second paragraph of Article 1020.05(b)(10) of the Standard Specifications to read:

“When calcium nitrite is used, it shall be added at the rate of 4 gal/cu yd (20 L/cu m) and shall be added to the mix immediately after all compatible admixtures have been introduced to the batch. Other corrosion inhibitors shall be added per the manufacturer’s specifications.”

Delete the third paragraph of Article 1020.05(b)(10) of the Standard Specifications.

Revise Article 1020.15(b)(1)c. of the Standard Specifications to read:

“c. The minimum portland cement content in the mixture shall be 375 lbs/cu yd (222 kg/cu m). When the total of organic processing additions, inorganic processing additions, and limestone addition exceed 5.0 percent in the cement, the minimum portland cement content in the mixture shall be 400 lbs/cu yd (237 kg/cu m). For a drilled shaft, foundation, footing, or substructure, the minimum portland cement may be reduced to as low as 330 lbs/cu yd (196 kg/cu m) if the concrete has adequate freeze/thaw durability. The Contractor shall provide freeze/thaw test results according to AASHTO T 161, and the relative dynamic modulus of elasticity of the mix design shall be a minimum of 80 percent. Testing shall be performed by an independent laboratory accredited by AASHTO re:source for Portland Cement Concrete. Freeze/thaw testing will not be required for concrete that will not be exposed to freezing and thawing conditions as determined by the Engineer.”

Revise Article 1021.01 of the Standard Specifications to read:

“1021.01 General. Admixtures shall be furnished in liquid or powder form ready for use. The admixtures shall be delivered in the manufacturer's original containers, bulk tank trucks or such containers or tanks as are acceptable to the Engineer. Delivery shall be accompanied by a ticket which clearly identifies the manufacturer, the date of manufacture, and trade name of the material. Containers shall be readily identifiable as to manufacturer, the date of manufacture, and trade name of the material they contain.

Concrete admixtures shall be on one of the Department's qualified product lists. Unless otherwise noted, admixtures shall have successfully completed and remain current with the AASHTO Product Eval and Audit Concrete Admixture (CADD) testing program. For admixture submittals to the Department; the product brand name, manufacturer name, admixture type or types, an electronic link to the product's technical data sheet, and the NTPEP testing number which contains an electronic link to all test data shall be provided. In addition, a letter shall be submitted certifying that no changes have been made in the formulation of the material since the most current round of tests conducted by AASHTO Product Eval and Audit. After 28 days of testing by AASHTO Product Eval and Audit, air-entraining admixtures may be provisionally approved and used on Departmental projects. For all other admixtures, unless otherwise noted, the time period after which provisionally approved status may be earned is 6 months.

The manufacturer shall include the following in the submittal to the AASHTO Product Eval and Audit CADD testing program: the manufacturing range for specific gravity, the midpoint and manufacturing range for residue by oven drying, and manufacturing range of pH. The submittal shall also include an infrared spectrophotometer trace no more than five years old.

For air-entraining admixtures according to Article 1021.02, the specific gravity allowable manufacturing range established by the manufacturer shall be according to AASHTO M 194. For residue by oven drying and pH, the allowable manufacturing range and test methods shall be according to AASHTO M 194.

For admixtures according to Articles 1021.03, 1021.04, 1021.05, 1021.06, 1021.07, and 1021.08, the pH allowable manufacturing range established by the manufacturer shall be according to ASTM E 70. For specific gravity and residue by oven drying, the allowable manufacturing range and test methods shall be according to AASHTO M 194.

All admixtures, except chloride-based accelerators, shall contain a maximum of 0.3 percent chloride by weight (mass) as determined by an appropriate test method. To verify the test result, the Department will use Illinois Modified AASHTO T 260, Procedure A, Method 1.

Prior to final approval of an admixture, the Engineer reserves the right to request a sample for testing. The test and reference concrete mixtures tested by the Engineer will contain a cement content of 5.65 cwt/cu yd (335 kg/cu m). For freeze-thaw testing, the Department will perform the test according to Illinois Modified AASHTO T 161. The flexural strength test will be performed according to AASHTO T 177. If the Engineer decides to test the admixture, the manufacturer shall submit AASHTO T 197 water content and set time test results on the standard cement used by the Department. The manufacturer may select their lab or an independent lab to perform this testing. The laboratory is not required to be accredited by AASHTO.

Random field samples may be taken by the Department to verify an admixture meets specification. A split sample will be provided to the manufacturer if requested. Admixtures that do not meet specification requirements or an allowable manufacturing range established by the manufacturer shall be replaced with new material.”

Revise Article 1021.03 of the Standard Specifications to read:

“1021.03 Retarding and Water-Reducing Admixtures. The admixture shall be according to the following.

- (a) Retarding admixtures shall be according to AASHTO M 194, Type B (retarding) or Type D (water-reducing and retarding).
- (b) Water-reducing admixtures shall be according to AASHTO M 194, Type A.
- (c) High range water-reducing admixtures shall be according to AASHTO M 194, Type F (high range water-reducing) or Type G (high range water-reducing and retarding).”

Revise Article 1021.05 of the Standard Specifications to read:

“1021.05 Self-Consolidating Admixtures. Self-consolidating admixture systems shall consist of either a high range water-reducing admixture only or a high range water-reducing admixture combined with a separate viscosity modifying admixture. The one or two component admixture system shall be capable of producing a concrete that can flow around reinforcement and consolidate under its own weight without additional effort and without segregation.

High range water-reducing admixtures shall be according to AASHTO M 194, Type F.

Viscosity modifying admixtures shall be according to AASHTO M 194, Type S (specific performance).”

Revise Article 1021.06 of the Standard Specifications to read:

“1021.06 Rheology-Controlling Admixture. Rheology-controlling admixtures shall be capable of producing a concrete mixture with a lower yield stress that will consolidate easier for slipform applications used by the Contractor. Rheology-controlling admixtures shall be according to AASHTO M 194, Type S (specific performance).”

Revise Article 1021.07 of the Standard Specifications to read:

“1021.07 Corrosion Inhibitor. The corrosion inhibitor shall be according to one of the following.

- (a) Calcium Nitrite. Corrosion inhibitors shall contain a minimum 30 percent calcium nitrite by weight (mass) of solution and shall comply with either the requirements of AASHTO M 194, Type C (accelerating) or the requirements of ASTM C 1582. The corrosion inhibiting performance requirements of ASTM C 1582 shall not apply.

(b) Other Materials. The corrosion inhibitor shall be according to ASTM C 1582.

For submittals requiring testing according to ASTM M 194, Type C (accelerating), the admixture shall meet the requirements of the AASHTO Product Eval and Audit CADD testing program according to Article 1021.01.

For submittals requiring testing according to ASTM C 1582, a report prepared by an independent laboratory accredited by AASHTO re:source for portland cement concrete shall be provided. The report shall show the results of physical tests conducted no more than five years prior to the time of submittal, according to applicable specifications. However, ASTM G 109 test information specified in ASTM C 1582 is not required to be from an independent accredited lab. All other information in ASTM C 1582 shall be from an independent accredited lab. Test data and other information required to be submitted to AASHTO Product Eval and Audit according to Article 1021.01, shall instead be submitted directly to the Department.”

Add Article 1021.08 of the Standard Specifications as follows:

“1021.08 Other Specific Performance Admixtures. Other specific performance admixtures shall, at a minimum, be according to AASHTO M 194, Type S (specific performance). The Department also reserves the right to require other testing, as determined by the Engineer, to show evidence of specific performance characteristics.

Initial testing according to AASHTO M 194 may be conducted under the AASHTO Product Eval and Audit CADD testing program according to Article 1021.01, or by an independent laboratory accredited by AASHTO re:source for Portland Cement Concrete. In either case, test data and other information required to be submitted to AASHTO Product Eval and Audit according to Article 1021.01, shall also be submitted directly to the Department. The independent accredited lab report shall show the results of physical tests conducted no more than five years prior to the time of submittal, according to applicable specifications.”

Revise Article 1024.01 of the Standard Specifications to read:

“1024.01 Requirements for Grout. The grout shall be proportioned by dry volume, thoroughly mixed, and shall have a minimum temperature of 50 °F (10 °C). Water shall not exceed the minimum needed for placement and finishing.

Materials for the grout shall be according to the following.

| Item | Article/Section |
|--|-----------------|
| (a) Cement | 1001 |
| (b) Water | 1002 |
| (c) Fine Aggregate | 1003.02 |
| (d) Fly Ash | 1010 |
| (e) Ground Granulated Blast Furnace (GGBF) Slag..... | 1010 |
| (f) Concrete Admixtures | 1021” |

Revise Note 1 of Article 1024.02 of the Standard Specifications to read:

“Note 1. Nonshrink grout shall be according to Illinois Modified ASTM C 1107.

The nonshrink grout shall have a water-soluble chloride ion content of less than 0.40 lb/cu yd (0.24 kg/cu m). The test shall be performed according to ASTM C 1218, and the grout shall have an age of 28 to 42 days at the time of test. The ASTM C 1218 test shall be performed by an independent lab a minimum of once every five years, and the test results shall be provided to the Department. Mixing of the nonshrink grout shall be according to the manufacturer's specifications. The Department will maintain a qualified product list."

Revise Article 1029.02 of the Standard Specifications to read:

" **1029.02 Materials.** Materials shall be according to the following.

| Item | Article/Section |
|---|-----------------|
| (a) Cement..... | 1001 |
| (b) Fly Ash | 1010 |
| (c) Ground Granulated Blast Furnace (GGBF) Slag | 1010 |
| (d) Water..... | 1002 |
| (e) Fine Aggregate..... | 1003 |
| (f) Concrete Admixtures | 1021 |
| (g) Foaming Agent (Note 1) | |

Note 1. The manufacturer shall submit infrared spectrophotometer trace and test results indicating the foaming agent meets the requirements of ASTM C 869 in order to be on the Department's qualified product list. Submitted data/results shall not be more than five years old."

Revise the second paragraph of Article 1103.03(a)(4) the Standard Specifications to read:

"The dispenser system shall provide a visual indication that the liquid admixture is actually entering the batch, such as via a transparent or translucent section of tubing or by independent check with an integrated secondary metering device. If approved by the Engineer, an alternate indicator may be used for admixtures dosed at rates of 25 oz/cwt (1630 mL/100 kg) or greater, such as accelerating admixtures, corrosion inhibitors, and viscosity modifying admixtures."

Revise the first two sections of Check Sheet #11 of the Supplemental Specifications and Recurring Special Provisions to read:

"Description. This work shall consist of filling voids beneath rigid and composite pavements with cement grout.

Materials. Materials shall be according to the following Articles of Division 1000 - Materials of the Standard Specifications:

| Item | Article/Section |
|---|-----------------|
| (a) Cement | 1001 |
| (b) Water | 1002 |
| (c) Fly Ash | 1010 |
| (d) Ground Granulated Blast Furnace (GGBF) Slag..... | 1010 |
| (e) Admixtures | 1021 |
| (f) Packaged Rapid Hardening Mortar or Concrete | 1018" |

Revise the third paragraph of Materials Note 2 of Check Sheet #28 of the Supplemental Specifications and Recurring Special Provisions to read:

“The Department will maintain a qualified product list of synthetic fibers, which will include the minimum required dosage rate. For the minimum required fiber dosage rate based on the Illinois Modified ASTM C 1609 test, a report prepared by an independent laboratory accredited by AASHTO re:source for Portland Cement Concrete shall be provided. The report shall show results of tests conducted no more than five years prior to the time of submittal.”

COMPENSABLE DELAY COSTS (BDE)

Effective: June 2, 2017

Revised: April 1, 2019

Revise Article 107.40(b) of the Standard Specifications to read:

“(b) Compensation. Compensation will not be allowed for delays, inconveniences, or damages sustained by the Contractor from conflicts with facilities not meeting the above definition; or if a conflict with a utility in an unanticipated location does not cause a shutdown of the work or a documentable reduction in the rate of progress exceeding the limits set herein. The provisions of Article 104.03 notwithstanding, compensation for delays caused by a utility in an unanticipated location will be paid according to the provisions of this Article governing minor and major delays or reduced rate of production which are defined as follows.

- (1) Minor Delay. A minor delay occurs when the work in conflict with the utility in an unanticipated location is completely stopped for more than two hours, but not to exceed two weeks.
- (2) Major Delay. A major delay occurs when the work in conflict with the utility in an unanticipated location is completely stopped for more than two weeks.
- (3) Reduced Rate of Production Delay. A reduced rate of production delay occurs when the rate of production on the work in conflict with the utility in an unanticipated location decreases by more than 25 percent and lasts longer than seven calendar days.”

Revise Article 107.40(c) of the Standard Specifications to read:

“(c) Payment. Payment for Minor, Major, and Reduced Rate of Production Delays will be made as follows.

- (1) Minor Delay. Labor idled which cannot be used on other work will be paid for according to Article 109.04(b)(1) and (2) for the time between start of the delay and the minimum remaining hours in the work shift required by the prevailing practice in the area.

Equipment idled which cannot be used on other work, and which is authorized to standby on the project site by the Engineer, will be paid for according to Article 109.04(b)(4).

- (2) Major Delay. Labor will be the same as for a minor delay.

Equipment will be the same as for a minor delay, except Contractor-owned equipment will be limited to two weeks plus the cost of move-out to either the Contractor's yard or another job and the cost to re-mobilize, whichever is less. Rental equipment may be paid for longer than two weeks provided the Contractor presents adequate support to the Department (including lease agreement) to show retaining equipment on the job is the most economical course to follow and in the public interest.

- (3) Reduced Rate of Production Delay. The Contractor will be compensated for the reduced productivity for labor and equipment time in excess of the 25 percent threshold for that portion of the delay in excess of seven calendar days. Determination of compensation will be in accordance with Article 104.02, except labor and material additives will not be permitted.

Payment for escalated material costs, escalated labor costs, extended project overhead, and extended traffic control will be determined according to Article 109.13."

Revise Article 108.04(b) of the Standard Specifications to read:

"(b) No working day will be charged under the following conditions.

- (1) When adverse weather prevents work on the controlling item.
- (2) When job conditions due to recent weather prevent work on the controlling item.
- (3) When conduct or lack of conduct by the Department or its consultants, representatives, officers, agents, or employees; delay by the Department in making the site available; or delay in furnishing any items required to be furnished to the Contractor by the Department prevents work on the controlling item.
- (4) When delays caused by utility or railroad adjustments prevent work on the controlling item.
- (5) When strikes, lock-outs, extraordinary delays in transportation, or inability to procure critical materials prevent work on the controlling item, as long as these delays are not due to any fault of the Contractor.
- (6) When any condition over which the Contractor has no control prevents work on the controlling item."

Revise Article 109.09(f) of the Standard Specifications to read:

"(f) Basis of Payment. After resolution of a claim in favor of the Contractor, any adjustment in time required for the work will be made according to Section 108. Any adjustment in the costs to be paid will be made for direct labor, direct materials, direct equipment, direct jobsite overhead, direct offsite overhead, and other direct costs allowed by the resolution. Adjustments in costs will not be made for interest charges, loss of anticipated profit, undocumented loss of efficiency, home office overhead and unabsorbed overhead other than as allowed by Article 109.13, lost opportunity, preparation of claim expenses and other consequential indirect costs regardless of method of calculation.

The above Basis of Payment is an essential element of the contract and the claim cost recovery of the Contractor shall be so limited.”

Add the following to Section 109 of the Standard Specifications.

“109.13 Payment for Contract Delay. Compensation for escalated material costs, escalated labor costs, extended project overhead, and extended traffic control will be allowed when such costs result from a delay meeting the criteria in the following table.

| Contract Type | Cause of Delay | Length of Delay |
|-----------------|--|---|
| Working Days | Article 108.04(b)(3) or Article 108.04(b)(4) | No working days have been charged for two consecutive weeks. |
| Completion Date | Article 108.08(b)(1) or Article 108.08(b)(7) | The Contractor has been granted a minimum two week extension of contract time, according to Article 108.08. |

Payment for each of the various costs will be according to the following.

- (a) Escalated Material and/or Labor Costs. When the delay causes work, which would have otherwise been completed, to be done after material and/or labor costs have increased, such increases will be paid. Payment for escalated material costs will be limited to the increased costs substantiated by documentation furnished by the Contractor. Payment for escalated labor costs will be limited to those items in Article 109.04(b)(1) and (2), except the 35 percent and 10 percent additives will not be permitted.
- (b) Extended Project Overhead. For the duration of the delay, payment for extended project overhead will be paid as follows.
 - (1) Direct Jobsite and Offsite Overhead. Payment for documented direct jobsite overhead and documented direct offsite overhead, including onsite supervisory and administrative personnel, will be allowed according to the following table.

| Original Contract Amount | Supervisory and Administrative Personnel |
|--|--|
| Up to \$5,000,000 | One Project Superintendent |
| Over \$ 5,000,000 - up to \$25,000,000 | One Project Manager, One Project Superintendent or Engineer, and One Clerk |
| Over \$25,000,000 - up to \$50,000,000 | One Project Manager, One Project Superintendent, One Engineer, and One Clerk |
| Over \$50,000,000 | One Project Manager, Two Project Superintendents, One Engineer, and One Clerk |

(2) Home Office and Unabsorbed Overhead. Payment for home office and unabsorbed overhead will be calculated as 8 percent of the total delay cost.

(c) Extended Traffic Control. Traffic control required for an extended period of time due to the delay will be paid for according to Article 109.04.

When an extended traffic control adjustment is paid under this provision, an adjusted unit price as provided for in Article 701.20(a) for increase or decrease in the value of work by more than ten percent will not be paid.

Upon payment for a contract delay under this provision, the Contractor shall assign subrogation rights to the Department for the Department's efforts of recovery from any other party for monies paid by the Department as a result of any claim under this provision. The Contractor shall fully cooperate with the Department in its efforts to recover from another party any money paid to the Contractor for delay damages under this provision."

CONCRETE BARRIER (BDE)

Effective: January 1, 2025

Revise the second paragraph of Article 637.12 of the Standard Specifications to read:

"When a double face concrete barrier with a variable cross-section is required, and the variation exceeds 1/2 in. (13 mm), the barrier will be paid for at the contract unit price per foot (meter) for CONCRETE BARRIER, VARIABLE CROSS-SECTION, of the height specified."

CONCRETE SEALER (BDE)

Effective: November 1, 2023

Replace Section 1026 of the Standard Specifications with the following:

"SECTION 1026. CONCRETE SEALER

1026.01 General. Sealer types shall be according to the listing in AASHTO M 224. All concrete sealer types shall meet the sealer requirements of AASHTO M 224 when tested in accordance with AASHTO T 384. The sealer shall be listed on the Department's qualified product list.

The sealer shall have a clear or amber color when dry.

The Department will perform the sealer characterization properties of ATR-FTIR spectra, total solids, and specific gravity in accordance with AASHTO M 224."

DISADVANTAGED BUSINESS ENTERPRISE PARTICIPATION (BDE)

Effective: September 1, 2000

Revised: January 2, 2025

1. OVERVIEW AND GENERAL OBLIGATION. The Department of Transportation, as a recipient of federal financial assistance, is required to take all necessary and reasonable steps to ensure nondiscrimination in the award and administration of contracts. Consequently, the federal regulatory provisions of 49 CFR Part 26 apply to this contract concerning the utilization of disadvantaged business enterprises. For the purposes of this Special Provision, a disadvantaged business enterprise (DBE) means a business certified in accordance with the requirements of 49 CFR Part 26 and listed in the Illinois Unified Certification Program (IL UCP) DBE Directory. Award of the contract is conditioned on meeting the requirements of 49 CFR Part 26, and failure by the Contractor to carry out the requirements of Part 26 is a material breach of the contract and may result in the termination of the contract or such other remedies as the Department deems appropriate.
2. CONTRACTOR ASSURANCE. All assurances set forth in FHWA 1273 are hereby incorporated by reference and will be physically attached to the final contract and all subcontracts.
3. CONTRACT GOAL TO BE ACHIEVED BY THE CONTRACTOR. The Department has determined the work of this contract has subcontracting opportunities that may be suitable for performance by DBE companies and that, in the absence of unlawful discrimination and in an arena of fair and open competition, DBE companies can be expected to perform **2.00%** of the work. This percentage is set as the DBE participation goal for this contract. Consequently, in addition to the other award criteria established for this contract, the Department will only award this contract to a bidder who makes a good faith effort to meet this goal of DBE participation in the performance of the work in accordance with the requirements of 49 CFR 26.53 and SBE Memorandum No. 24-02.
4. IDENTIFICATION OF CERTIFIED DBE. Information about certified DBE Contractors can be found in the Illinois UCP Directory. Bidders can obtain additional information and assistance with identifying DBE-certified companies at the Department's website or by contacting the Department's Bureau of Small Business Enterprises at (217) 785-4611.
5. BIDDING PROCEDURES. Compliance with this Special Provision and SBE Policy Memorandum 24-02 is a material bidding requirement. The following shall be included with the bid.
 - (a) DBE Utilization Plan (form SBE 2026) documenting enough DBE participation has been obtained to meet the goal, or a good faith effort has been made to meet the goal even though the efforts did not succeed in obtaining enough DBE participation to meet the goal.
 - (b) Applicable DBE Participation Statement (form SBE 2023, 2024, and/or 2025) for each DBE firm the bidder has committed to perform the work to achieve the contract goal.

The required forms and documentation shall be submitted as a single .pdf file using the "Integrated Contractor Exchange (iCX)" application within the Department's "EBids System".

The Department will not accept a bid if it does not meet the bidding procedures set forth herein and the bid will be declared non-responsive. A bidder declared non-responsive for failure to meet the bidding procedures will not give rise to an administrative reconsideration. In the event the bid is declared non-responsive, the Department may elect to cause the forfeiture of the penal sum of the bidder's proposal guaranty and may deny authorization to bid the project if re-advertised for bids.

6. UTILIZATION PLAN EVALUATION. The contract will not be awarded until the Utilization Plan is approved. All information submitted by the bidder must be complete, accurate, and adequately document the bidder has committed to DBE participation sufficient to meet the goal, or that the bidder has made good faith efforts to do so, in the event the bidder cannot meet the goal, in order for the Department to commit to the performance of the contract by the bidder.

The Utilization Plan will be approved by the Department if the Utilization Plan documents sufficient commercially useful DBE work to meet the contract goal or the Department determines, based upon the documentation submitted, that the bidder has made a good faith effort to meet the contract goal pursuant to 49 CFR Part 26, Appendix A and the requirements of SBE 2026.

If the Department determines that a good faith effort has not been made, the Department will notify the responsible company official designated in the Utilization Plan of that determination in accordance with SBE Policy Memorandum 24-02.

7. CALCULATING DBE PARTICIPATION. The Utilization Plan values represent work the bidder commits to have performed by the specified DBEs and paid for upon satisfactory completion. The Department is only able to count toward the achievement of the overall goal and the contract goal the value of payments made for the work actually performed by DBE firms. In addition, a DBE must perform a commercially useful function on the contract to be counted. A commercially useful function is generally performed when the DBE is responsible for the work and is carrying out its responsibilities by actually performing, managing, and supervising the work involved. The Department and Contractor are governed by the provisions of 49 CFR Part 26.55(c) on questions of commercially useful functions as it affects the work. Specific guidelines for counting goal credit are provided in 49 CFR Part 26.55. In evaluating Utilization Plans for award the Department will count goal credit as set forth in Part 26 and in accordance with SBE Policy Memorandum 24-02.
8. CONTRACT COMPLIANCE. The Contractor must utilize the specific DBEs listed to perform the work and supply the materials for which each DBE is listed in the Contractor's approved Utilization Plan, unless the Contractor obtains the Department's written consent to terminate the DBE or any portion of its work. The DBE Utilization Plan approved by SBE is a condition-of-award, and any deviation to that Utilization Plan, the work set forth therein to be performed by DBE firms, or the DBE firms specified to perform that work, must be approved, in writing, by the Department in accordance with federal regulatory requirements. Deviation from the DBE Utilization Plan condition-of-award without such written approval is a violation of the contract and may result in termination of the contract or such other remedy the Department deems appropriate. The following administrative procedures and remedies govern the compliance by the Contractor with the contractual obligations established by the Utilization Plan.

- (a) NOTICE OF DBE PERFORMANCE. The Contractor shall provide the Engineer with at least three days advance notice of when all DBE firms are expected to perform the work committed under the Contractor's Utilization Plan.
- (b) SUBCONTRACT. If awarded the contract, the Contractor is required to enter into written subcontracts with all DBE firms indicated in the approved Utilization Plan and must provide copies of fully executed DBE subcontracts to the Department upon request. Subcontractors shall ensure that all lower tier subcontracts or agreements with DBEs to supply labor or materials be performed in accordance with this Special Provision.
- (c) PAYMENT TO DBE FIRMS. The Department is prohibited by federal regulations from crediting the participation of a DBE included in the Utilization Plan toward either the contract goal or the Department's overall goal until the amount to be applied toward the goal has been paid to the DBE. The Contractor shall document and report all payments for work performed by DBE certified firms in accordance with Article 109.11 of the Standard Specifications. All records of payment for work performed by DBE certified firms shall be made available to the Department upon request.
- (d) FINAL PAYMENT. After the performance of the final item of work or trucking, or delivery of material by a DBE and final payment to the DBE by the Contractor, but not later than 30 calendar days after payment has been made by the Department to the Contractor for such work or material, the Contractor shall submit a DBE Payment Agreement (form SBE 2115) to the Engineer. If the Contractor does not have the full amount of work indicated in the Utilization Plan performed by the DBE companies indicated in the Utilization Plan and after good faith efforts are reviewed, the Department may deduct from contract payments to the Contractor the amount of the goal not achieved as liquidated and ascertained damages.
- (g) ENFORCEMENT. The Department reserves the right to withhold payment to the Contractor to enforce the provisions of this Special Provision. Final payment shall not be made on the contract until such time as the Contractor submits sufficient documentation demonstrating achievement of the goal in accordance with this Special Provision or after liquidated damages have been determined and collected.

EROSION CONTROL BLANKET (BDE)

Effective: August 1, 2025

Revise Article 251.02 of the Standard Specifications to read:

“251.02 Materials. Materials shall be according to the following.

| Item | Article/Section |
|---|-----------------|
| (a) Compost | 1081.05(b) |
| (b) Mulch | 1081.06(a) |
| (c) Chemical Mulch Binder | 1081.06(a)(3) |
| (d) Chemical Compost Binder | 1081.06(a)(4) |
| (e) Erosion Control Blanket | 1081.10(a) |
| (f) Wildlife Friendly Erosion Control Blanket | 1081.10(b) |
| (g) Wire Staples | 1081.10(c) |
| (h) Wood Stakes | 1081.10(d) |
| (i) Turf Reinforcement Mat | 1081.10(e)” |

Revise the first and second sentences of Article 251.04 of the Standard Specifications to read:

“251.04 Erosion Control Blanket. All erosion control blanket materials shall be placed on the areas specified within 24 hours of seed placement.”

Revise the second paragraph of Article 251.04 of the Standard Specifications to read:

“After the area has been properly shaped, fertilized (when applicable), and seeded, the blanket shall be laid out flat, evenly, and smoothly, without stretching the material. The erosion control blanket shall be placed according to the manufacture’s recommendations.”

Revise the second sentence of Article 251.06(b) of the Standard Specifications to read:

“Erosion control blanket, wildlife friendly erosion control blanket, and turf reinforcement mat will be measured for payment in square yards (square meters).”

Revise Article 251.07 of the Standard Specifications to read:

“251.07 Basis of Payment. This work will be paid for at the contract unit price per acre (hectare) for MULCH, of the method specified; and at the contract unit price per square yard (square meter) for EROSION CONTROL BLANKET, WILDLIFE FRIENDLY EROSION CONTROL BLANKET, or TURF REINFORCEMENT MAT.”

Revise first sentence of Article 280.04(h) of the Standard Specifications to read:

“This system consists of temporarily installing erosion control blanket or wildlife friendly erosion control blanket over areas that are to be reworked during a later construction phase.”

Revise Article 280.08(g) of the Standard Specifications to read:

- “(g) Temporary Erosion Control Blanket. Temporary erosion control blanket will be paid for at the contract unit price per square yard (square meter) for TEMPORARY EROSION CONTROL BLANKET or TEMPORARY WILDLIFE FRIENDLY EROSION CONTROL BLANKET.

The work of removing, storing, and reinstalling the blanket over areas to be reworked more than once will not be paid for separately but shall be included in the cost of the temporary erosion control blanket or temporary wildlife friendly erosion control blanket.”

Revise Article 1081.10 of the Standard Specifications to read:

“**1081.10 Erosion Control Blankets.** The manufacturer shall furnish a certificate with each shipment stating the amount of product furnished and that the material complies with these requirements.

- (a) Erosion Control Blanket. Erosion control blanket shall be covered on top and bottom, also known as double net, with a 100 percent biodegradable woven, natural fiber or jute net meeting the following.

| Material | Minimum Value |
|-----------------------|---------------------------------|
| Excelsior | 80% |
| Straw | 100% |
| Coconut or Coir | 100% Coconut or Coir |
| Straw/Coconut or Coir | 70% Straw / 30% Coconut or Coir |

- (b) Wildlife Friendly Erosion Control Blanket. Wildlife friendly erosion control blanket shall be according to Article 1081.10(a) except the netting shall be loose weave, also known as leno weave or gauze weave, with a moveable joint.
- (c) Wire Staples. Staples shall be made from No. 11 gauge or heavier uncoated black carbon steel wire, a minimum of 1 in. (25 mm) wide at the top and a minimum overall length of 8 in. (200 mm).
- (d) Wood Stakes. Hardwood blanket anchors shall be nominally 7 in. (180 mm) long from neck of hook to tip of anchor. The anchor shall have a minimum 1/2 in. (13 mm) curving hook to hold the blanket in place.
- (e) Turf Reinforcement Mat (TRM). The TRM shall be comprised of non-degradable, ultraviolet stabilized synthetic fibers, filaments, netting, and/or wire mesh processed into a three-dimensional reinforced mat. The mats may include degradable material to assist with vegetation establishment. Soil filled mats will not be allowed.

The TRM shall meet the following physical and performance properties:

| Property | Value | Test Method |
|--|-----------------|---|
| Tensile Strength, lb/ft (kN/m) | 150 (2.19) min. | ASTM D 6818 |
| UV Stability, (% Tensile Retained) | 80 min. | ASTM D 4355 (1000 Hour Exposure) |
| Resiliency, (% Thickness Retained) | 80 min. | ASTM D 6524 |
| Allowable Shear Stress, lb/sq ft (Pa) ^{1/} | 8 (384) | ECTC approved test method and independent laboratory |

1/ Minimum shear stress the TRM (fully vegetated) can sustain without physical damage or excess erosion (> 1/2 in. (13 mm) soil loss) during a 30 minute flow event in large scale testing.

For TRMs containing degradable components, all property values shall be obtained on the non-degradable portion of the matting alone.”

FUEL COST ADJUSTMENT (BDE)

Effective: April 1, 2009

Revised: August 1, 2017

Description. Fuel cost adjustments will be made to provide additional compensation to the Contractor, or a credit to the Department, for fluctuations in fuel prices when optioned by the Contractor. The bidder shall indicate with their bid whether or not this special provision will be part of the contract. Failure to indicate “Yes” for any category of work will make that category of work exempt from fuel cost adjustment.

General. The fuel cost adjustment shall apply to contract pay items as grouped by category. The adjustment shall only apply to those categories of work checked “Yes”, and only when the cumulative plan quantities for a category exceed the required threshold. Adjustments to work items in a category, either up or down, and extra work paid for by agreed unit price will be subject to fuel cost adjustment only when the category representing the added work was subject to the fuel cost adjustment. Extra work paid for at a lump sum price or by force account will not be subject to fuel cost adjustment. Category descriptions and thresholds for application and the fuel usage factors which are applicable to each are as follows:

(a) Categories of Work.

- (1) Category A: Earthwork. Contract pay items performed under Sections 202, 204, and 206 including any modified standard or nonstandard items where the character of the work to be performed is considered earthwork. The cumulative total of all applicable item plan quantities shall exceed 25,000 cu yd (20,000 cu m). Included in the fuel usage factor is a weighted average 0.10 gal/cu yd (0.50 liters/cu m) factor for trucking.

- (2) Category B: Subbases and Aggregate Base Courses. Contract pay items constructed under Sections 311, 312 and 351 including any modified standard or nonstandard items where the character of the work to be performed is considered construction of a subbase or aggregate, stabilized or modified base course. The cumulative total of all applicable item plan quantities shall exceed 5000 tons (4500 metric tons). Included in the fuel usage factor is a 0.60 gal/ton (2.50 liters/metric ton) factor for trucking.
- (3) Category C: Hot-Mix Asphalt (HMA) Bases, Pavements and Shoulders. Contract pay items constructed under Sections 355, 406, 407 and 482 including any modified standard or nonstandard items where the character of the work to be performed is considered HMA bases, pavements and shoulders. The cumulative total of all applicable item plan quantities shall exceed 5000 tons (4500 metric tons). Included in the fuel usage factor is 0.60 gal/ton (2.50 liters/metric ton) factor for trucking.
- (4) Category D: Portland Cement Concrete (PCC) Bases, Pavements and Shoulders. Contract pay items constructed under Sections 353, 420, 421 and 483 including any modified standard or nonstandard items where the character of the work to be performed is considered PCC base, pavement or shoulder. The cumulative total of all applicable item plan quantities shall exceed 7500 sq yd (6000 sq m). Included in the fuel usage factor is 1.20 gal/cu yd (5.94 liters/cu m) factor for trucking.
- (5) Category E: Structures. Structure items having a cumulative bid price that exceeds \$250,000 for pay items constructed under Sections 502, 503, 504, 505, 512, 516 and 540 including any modified standard or nonstandard items where the character of the work to be performed is considered structure work when similar to that performed under these sections and not included in categories A through D.

(b) Fuel Usage Factors.

| English Units | | |
|--|--------|--------------|
| Category | Factor | Units |
| A - Earthwork | 0.34 | gal / cu yd |
| B – Subbase and Aggregate Base courses | 0.62 | gal / ton |
| C – HMA Bases, Pavements and Shoulders | 1.05 | gal / ton |
| D – PCC Bases, Pavements and Shoulders | 2.53 | gal / cu yd |
| E – Structures | 8.00 | gal / \$1000 |

| Metric Units | | |
|--|--------|---------------------|
| Category | Factor | Units |
| A - Earthwork | 1.68 | liters / cu m |
| B – Subbase and Aggregate Base courses | 2.58 | liters / metric ton |
| C – HMA Bases, Pavements and Shoulders | 4.37 | liters / metric ton |
| D – PCC Bases, Pavements and Shoulders | 12.52 | liters / cu m |
| E – Structures | 30.28 | liters / \$1000 |

(c) Quantity Conversion Factors.

| Category | Conversion | Factor |
|----------|--------------------|--------------------------------------|
| B | sq yd to ton | 0.057 ton / sq yd / in depth |
| | sq m to metric ton | 0.00243 metric ton / sq m / mm depth |
| C | sq yd to ton | 0.056 ton / sq yd / in depth |
| | sq m to metric ton | 0.00239 m ton / sq m / mm depth |
| D | sq yd to cu yd | 0.028 cu yd / sq yd / in depth |
| | sq m to cu m | 0.001 cu m / sq m / mm depth |

Method of Adjustment. Fuel cost adjustments will be computed as follows.

$$CA = (FPI_P - FPI_L) \times FUF \times Q$$

Where: CA = Cost Adjustment, \$
 FPI_P = Fuel Price Index, as published by the Department for the month the work is performed, \$/gal (\$/liter)
 FPI_L = Fuel Price Index, as published by the Department for the month prior to the letting for work paid for at the contract price; or for the month the agreed unit price letter is submitted by the Contractor for extra work paid for by agreed unit price, \$/gal (\$/liter)
 FUF = Fuel Usage Factor in the pay item(s) being adjusted
 Q = Authorized construction Quantity, tons (metric tons) or cu yd (cu m)

The entire FUF indicated in paragraph (b) will be used regardless of use of trucking to perform the work.

Basis of Payment. Fuel cost adjustments may be positive or negative but will only be made when there is a difference between the FPI_L and FPI_P in excess of five percent, as calculated by:

$$\text{Percent Difference} = \{(FPI_L - FPI_P) \div FPI_L\} \times 100$$

Fuel cost adjustments will be calculated for each calendar month in which applicable work is performed; and will be paid or deducted when all other contract requirements for the items of work are satisfied. The adjustments shall not apply during contract time subject to liquidated damages for completion of the entire contract.

HOT-MIX ASPHALT (BDE)

Effective: January 1, 2024

Revised: January 1, 2025

Revise the first and second paragraphs of Articles 1030.06(c)(2) of the Standard Specifications to read:

“(2) Personnel. The Contractor shall provide a QC Manager who shall have overall responsibility and authority for quality control. This individual shall maintain active certification as a Hot-Mix Asphalt Level II technician.

In addition to the QC Manager, the Contractor shall provide sufficient personnel to perform the required visual inspections, sampling, testing, and documentation in a timely manner. Mix designs shall be developed by personnel with an active certification as a Hot-Mix Asphalt Level III technician. Technicians performing mix design testing and plant sampling/testing shall maintain active certification as a Hot-Mix Asphalt Level I technician. The Contractor may provide a technician trainee who has successfully completed the Department's "Hot-Mix Asphalt Trainee Course" to assist in the activities completed by a Hot-Mix Asphalt Level I technician for a period of one year after the course completion date. The Contractor may also provide a Gradation Technician who has successfully completed the Department's "Gradation Technician Course" to run gradation tests only under the supervision of a Hot-Mix Asphalt Level II Technician. The Contractor shall provide a Hot-Mix Asphalt Density Tester who has successfully completed the Department's "Nuclear Density Testing" course to run all nuclear density tests on the job site."

Revise the second paragraph of Articles 1030.07(a)(11) and 1030.08(a)(9) of the Standard Specifications to read:

"When establishing the target density, the HMA maximum theoretical specific gravity (G_{mm}) will be based on the running average of four available Department test results for that project. If less than four G_{mm} test results are available, an average of all available Department test results for that project will be used. The initial G_{mm} will be the last available Department test result from a QMP project. If there is no available Department test result from a QMP project, the Department mix design verification test result will be used as the initial G_{mm} ."

Revise Article 1030.09(g)(2) of the Standard Specifications to read:

"(2) The Contractor shall complete split verification sample tests listed in the Limits of Precision table in Article 1030.09(h)(1)."

In the Supplemental Specifications, replace the revision for the end of the third paragraph of Article 1030.09(h)(2) with the following:

"When establishing the target density, the HMA maximum theoretical specific gravity (G_{mm}) will be the Department mix design verification test result."

Revise the tenth paragraph of Article 1030.10 of the Standard Specifications to read:

"Production is not required to stop after a test strip has been constructed."

PAVEMENT MARKING INSPECTION (BDE)

Effective: April 1, 2025

Revise the second sentence of the first paragraph of Article 780.13 of the Standard Specifications to read:

“In addition, thermoplastic, preformed plastic, epoxy, preformed thermoplastic, polyurea, and modified urethane pavement markings will be inspected following a winter performance period that extends from November 15 to April 1 of the next year.”

PAVEMENT PATCHING (BDE)

Effective: August 1, 2025

Revise the first sentence of the last paragraph of Article 442.06(a)(2) of the Standard Specifications to read:

“Type IV patches shall be reinforced with welded wire reinforcement according to the details shown on the plans.”

Revise Article 442.06(a)(3) of the Standard Specifications to read:

“(3) Class C Patching. Patches adjacent to a new lane of pavement, new portland cement concrete shoulder, or new curb and gutter of more than 20 ft (6 m) in length shall be tied with No. 6 (No. 19) tie bars, 24 in. (600 mm) long, embedded 8 in. (200 mm) at 36 in. (900 mm) centers according to Article 420.05(b).

When the patched pavement is not to be resurfaced, transverse contraction joints shall be formed on 15 ft (4.5 m) to 20 ft (6 m) centers by sawing in all patches that are more than 20 ft (6 m) in length. They shall be placed in line with joints or cracks in the existing slab whenever possible.”

Revise the eighth paragraph of Article 442.11 of the Standard Specifications to read:

“Pavement tie bars for patches will be paid for at the contract unit price per each for TIE BARS, of the diameter specified.”

PERFORMANCE GRADED ASPHALT BINDER (BDE)

Effective: January 1, 2023

Revise Article 1032.05 of the Standard Specifications to read:

“1032.05 Performance Graded Asphalt Binder. These materials will be accepted according to the Bureau of Materials Policy Memorandum, “Performance Graded Asphalt Binder Qualification Procedure.” The Department will maintain a qualified producer list. These materials shall be free from water and shall not foam when heated to any temperature below the actual flash point. Air blown asphalt, recycle engine oil bottoms (ReOB), and polyphosphoric acid (PPA) modification shall not be used.

When requested, producers shall provide the Engineer with viscosity/temperature relationships for the performance graded asphalt binders delivered and incorporated in the work.

- (a) Performance Graded (PG) Asphalt Binder. The asphalt binder shall meet the requirements of AASHTO M 320, Table 1 “Standard Specification for Performance Graded Asphalt Binder” for the grade shown on the plans and the following.

| Test | Parameter |
|---|------------|
| Small Strain Parameter (AASHTO PP 113) BBR, ΔT_c , 40 hrs PAV (40 hrs continuous or 2 PAV at 20 hrs) | -5 °C min. |

- (b) Modified Performance Graded (PG) Asphalt Binder. The asphalt binder shall meet the requirements of AASHTO M 320, Table 1 “Standard Specification for Performance Graded Asphalt Binder” for the grade shown on the plans.

Asphalt binder modification shall be performed at the source, as defined in the Bureau of Materials Policy Memorandum, “Performance Graded Asphalt Binder Qualification Procedure.”

Modified asphalt binder shall be safe to handle at asphalt binder production and storage temperatures or HMA construction temperatures. Safety Data Sheets (SDS) shall be provided for all asphalt modifiers.

- (1) Polymer Modification (SB/SBS or SBR). Elastomers shall be added to the base asphalt binder to achieve the specified performance grade and shall be either a styrene-butadiene diblock, triblock copolymer without oil extension, or a styrene-butadiene rubber. The polymer modified asphalt binder shall be smooth, homogeneous, and be according to the requirements shown in Table 1 or 2 for the grade shown on the plans.

| Table 1 - Requirements for Styrene-Butadiene Copolymer (SB/SBS) Modified Asphalt Binders | | |
|---|---|---|
| Test | Asphalt Grade SB/SBS PG 64-28 SB/SBS PG 70-22 | Asphalt Grade SB/SBS PG 64-34 SB/SBS PG 70-28 SB/SBS PG 76-22 SB/SBS PG 76-28 |
| Separation of Polymer ITP, "Separation of Polymer from Asphalt Binder" Difference in °F (°C) of the softening point between top and bottom portions | 4 (2) max. | 4 (2) max. |
| TESTS ON RESIDUE FROM ROLLING THIN FILM OVEN TEST (AASHTO T 240) | | |
| Elastic Recovery ASTM D 6084, Procedure A, 77 °F (25 °C), 100 mm elongation, % | 60 min. | 70 min. |

| Table 2 - Requirements for Styrene-Butadiene Rubber (SBR) Modified Asphalt Binders | | |
|---|---|---|
| Test | Asphalt Grade SBR PG 64-28 SBR PG 70-22 | Asphalt Grade SB/SBS PG 64-34 SB/SBS PG 70-28 SBR PG 76-22 SBR PG 76-28 |
| Separation of Polymer ITP, "Separation of Polymer from Asphalt Binder" Difference in °F (°C) of the softening point between top and bottom portions | 4 (2) max. | 4 (2) max. |
| Toughness ASTM D 5801, 77 °F (25 °C), 20 in./min. (500 mm/min.), in.-lbs (N-m) | 110 (12.5) min. | 110 (12.5) min. |
| Tenacity ASTM D 5801, 77 °F (25 °C), 20 in./min. (500 mm/min.), in.-lbs (N-m) | 75 (8.5) min. | 75 (8.5) min. |
| TESTS ON RESIDUE FROM ROLLING THIN FILM OVEN TEST (AASHTO T 240) | | |
| Elastic Recovery ASTM D 6084, Procedure A, 77 °F (25 °C), 100 mm elongation, % | 40 min. | 50 min. |

- (2) Ground Tire Rubber (GTR) Modification. GTR modification is the addition of recycled ground tire rubber to liquid asphalt binder to achieve the specified performance grade. GTR shall be produced from processing automobile and/or truck tires by the ambient grinding method or micronizing through a cryogenic process. GTR shall not exceed 1/16 in. (2 mm) in any dimension and shall not contain free metal particles, moisture that would cause foaming of the asphalt, or other foreign materials. A mineral powder (such as talc) meeting the requirements of AASHTO M 17 may be added, up to a maximum of four percent by weight of GTR to reduce sticking and caking of the GTR

particles. When tested in accordance with Illinois Modified AASHTO T 27 "Standard Method of Test for Sieve Analysis of Fine and Coarse Aggregates" or AASHTO PP 74 "Standard Practice for Determination of Size and Shape of Glass Beads Used in Traffic Markings by Means of Computerized Optical Method", a 50 g sample of the GTR shall conform to the following gradation requirements.

| Sieve Size | Percent Passing |
|----------------------|-----------------|
| No. 16 (1.18 mm) | 100 |
| No. 30 (600 μ m) | 95 \pm 5 |
| No. 50 (300 μ m) | > 20 |

GTR modified asphalt binder shall be tested for rotational viscosity according to AASHTO T 316 using spindle S27. GTR modified asphalt binder shall be tested for original dynamic shear and RTFO dynamic shear according to AASHTO T 315 using a gap of 2 mm.

The GTR modified asphalt binder shall meet the requirements of Table 3.

| Table 3 - Requirements for Ground Tire Rubber (GTR) Modified Asphalt Binders | | |
|--|---|---|
| Test | Asphalt Grade GTR PG 64-28 GTR PG 70-22 | Asphalt Grade GTR PG 76-22 GTR PG 76-28 GTR PG 70-28 |
| TESTS ON RESIDUE FROM ROLLING THIN FILM OVEN TEST (AASHTO T 240) | | |
| Elastic Recovery ASTM D 6084, Procedure A, 77 °F (25 °C), 100 mm elongation, % | 60 min. | 70 min. |

- (3) Softener Modification (SM). Softener modification is the addition of organic compounds, such as engineered flux, bio-oil blends, modified vegetable oils, glycol amines, and fatty acid derivatives, to the base asphalt binder to achieve the specified performance grade. Softeners shall be dissolved, dispersed, or reacted in the asphalt binder to enhance its performance and shall remain compatible with the asphalt binder with no separation. Softeners shall not be added to modified PG asphalt binder as defined in Articles 1032.05(b)(1) or 1032.05(b)(2).

An Attenuated Total Reflectance-Fourier Transform Infrared spectrum (ATR-FTIR) shall be collected for both the softening compound as well as the softener modified asphalt binder at the dose intended for qualification. The ATR-FTIR spectra shall be collected on unaged softener modified binder, 20-hour Pressurized Aging Vessel (PAV) aged softener modified binder, and 40-hour PAV aged softener modified binder. The ATR-FTIR shall be collected in accordance with Illinois Test Procedure 601. The electronic files spectral files (in one of the following extensions or equivalent: *.SPA, *.SPG, *.IRD, *.IFG, *.CSV, *.SP, *.IRS, *.GAML, *. [0-9], *.IGM, *.ABS, *.DRT, *.SBM, *.RAS) shall be submitted to the Central Bureau of Materials.

Softener modified asphalt binders shall meet the requirements in Table 4.

| Table 4 - Requirements for Softener Modified Asphalt Binders | | |
|---|---------------|-------------|
| Test | Asphalt Grade | |
| | SM PG 46-28 | SM PG 46-34 |
| | SM PG 52-28 | SM PG 52-34 |
| | SM PG 58-22 | SM PG 58-28 |
| | SM PG 64-22 | |
| Small Strain Parameter (AASHTO PP 113) BBR, ΔT_c , 40 hrs PAV (40 hrs continuous or 2 PAV at 20 hrs) | -5°C min. | |
| Large Strain Parameter (Illinois Modified AASHTO T 391) DSR/LAS Fatigue Property, $\Delta G^* _{peak}$, 40 hrs PAV (40 hrs continuous or 2 PAV at 20 hrs) | ≥ 54 % | |

The following grades may be specified as tack coats.

| Asphalt Grade | Use |
|------------------------------|------------|
| PG 58-22, PG 58-28, PG 64-22 | Tack Coat" |

Revise Article 1031.06(c)(1) and 1031.06(c)(2) of the Standard Specifications to read:

“(1) RAP/RAS. When RAP is used alone or RAP is used in conjunction with RAS, the percentage of virgin ABR shall not exceed the amounts listed in the following table.

| HMA Mixtures - RAP/RAS Maximum ABR % ^{1/ 2/} | | | |
|---|--------|---------|---|
| Ndesign | Binder | Surface | Polymer Modified Binder or Surface ^{3/} |
| 30 | 30 | 30 | 10 |
| 50 | 25 | 15 | 10 |
| 70 | 15 | 10 | 10 |
| 90 | 10 | 10 | 10 |

- 1/ For Low ESAL HMA shoulder and stabilized subbase, the RAP/RAS ABR shall not exceed 50 percent of the mixture.
- 2/ When RAP/RAS ABR exceeds 20 percent, the high and low virgin asphalt binder grades shall each be reduced by one grade (i.e. 25 percent ABR would require a virgin asphalt binder grade of PG 64-22 to be reduced to a PG 58-28).
- 3/ The maximum ABR percentages for ground tire rubber (GTR) modified mixes shall be equivalent to the percentages specified for SBS/SBR polymer modified mixes.

- (2) FRAP/RAS. When FRAP is used alone or FRAP is used in conjunction with RAS, the percentage of virgin asphalt binder replacement shall not exceed the amounts listed in the following table.

| HMA Mixtures - FRAP/RAS Maximum ABR % ^{1/ 2/} | | | |
|--|--------|---------|--|
| Ndesign | Binder | Surface | Polymer Modified Binder or Surface ^{3/} |
| 30 | 55 | 45 | 15 |
| 50 | 45 | 40 | 15 |
| 70 | 45 | 35 | 15 |
| 90 | 45 | 35 | 15 |
| SMA | - - | - - | 25 |
| IL-4.75 | - - | - - | 35 |

- 1/ For Low ESAL HMA shoulder and stabilized subbase, the FRAP/RAS ABR shall not exceed 50 percent of the mixture.
- 2/ When FRAP/RAS ABR exceeds 20 percent for all mixes, the high and low virgin asphalt binder grades shall each be reduced by one grade (i.e. 25 percent ABR would require a virgin asphalt binder grade of PG 64-22 to be reduced to a PG 58-28).
- 3/ The maximum ABR percentages for GTR modified mixes shall be equivalent to the percentages specified for SBS/SBR polymer modified mixes.”

Add the following to the end of Note 2 of Article 1030.03 of the Standard Specifications.

“A dedicated storage tank for the ground tire rubber (GTR) modified asphalt binder shall be provided. This tank shall be capable of providing continuous mechanical mixing throughout and/or recirculation of the asphalt binder to provide a uniform mixture. The tank shall be heated and capable of maintaining the temperature of the asphalt binder at 300 °F to 350 °F (149 °C to 177 °C). The asphalt binder metering systems of dryer drum plants shall be calibrated with the actual GTR modified asphalt binder material with an accuracy of ± 0.40 percent.”

REMOVAL AND DISPOSAL OF REGULATED SUBSTANCES (BDE)

Effective: January 1, 2024

Revised: April 1, 2024

Revise the first paragraph of Article 669.04 of the Standard Specifications to read:

“669.04 Regulated Substances Monitoring. Regulated substances monitoring includes environmental observation and field screening during regulated substances management activities. The excavated soil and groundwater within the work areas shall be managed as either uncontaminated soil, hazardous waste, special waste, or non-special waste.

As part of the regulated substances monitoring, the monitoring personnel shall perform and document the applicable duties listed on form BDE 2732 “Regulated Substances Monitoring Daily Record (RSMDBR)”.

Revise the first two sentences of the nineteenth paragraph of Article 669.05 of the Standard Specifications to read:

“The Contractor shall coordinate waste disposal approvals with the disposal facility and provide the specific analytical testing requirements of that facility. The Contractor shall make all arrangements for collection, transportation, and analysis of landfill acceptance testing.”

Revise the last paragraph of Article 669.05 of the Standard Specifications to read:

“The Contractor shall select a permitted landfill facility or CCDD/USFO facility meeting the requirements of 35 Ill. Admin. Code Parts 810-814 or Part 1100, respectively. The Department will review and approve or reject the facility proposed by the Contractor based upon information provided in BDE 2730. The Contractor shall verify whether the selected facility is compliant with those applicable standards as mandated by their permit and whether the facility is presently, has previously been, or has never been, on the United States Environmental Protection Agency (U.S. EPA) National Priorities List or the Resource Conservation and Recovery Act (RCRA) List of Violating Facilities. The use of a Contractor selected facility shall in no manner delay the construction schedule or alter the Contractor's responsibilities as set forth.”

Revise the first paragraph of Article 669.07 of the Standard Specifications to read:

“669.07 Temporary Staging. Soil classified according to Articles 669.05(a)(2), (b)(1), or (c) may be temporarily staged at the Contractor's option. All other soil classified according to Articles 669.05(a)(1), (a)(3), (a)(4), (a)(5), (a)(6), or (b)(2) shall be managed and disposed of without temporary staging to the greatest extent practicable. If circumstances beyond the Contractor's control require temporary staging of these latter materials, the Contractor shall request approval from the Engineer in writing.

Topsoil for re-use as final cover which has been field screened and found not to exhibit PID readings over daily background readings as documented on the BDE 2732, visual staining or odors, and is classified according to Articles 669.05(a)(2), (a)(3), (a)(4), (b)(1), or (c) may be temporarily staged at the Contractor's option.”

Add the following paragraph after the sixth paragraph of Article 669.11 of the Standard Specifications.

“The sampling and testing of effluent water derived from dewatering discharges for priority pollutants volatile organic compounds (VOCs), priority pollutants semi-volatile organic compounds (SVOCs), or priority pollutants metals, will be paid for at the contract unit price per each for VOCS GROUNDWATER ANALYSIS using EPA Method 8260B, SVOCs GROUNDWATER ANALYSIS using EPA Method 8270C, or RCRA METALS GROUNDWATER ANALYSIS using EPA Methods 6010B and 7471A. This price shall include transporting the sample from the job site to the laboratory.”

Revise the first sentence of the eight paragraph of Article 669.11 of the Standard Specifications to read:

“Payment for temporary staging of soil classified according to Articles 669.05(a)(1), (a)(3), (a)(4), (a)(5), (a)(6), or (b)(2) to be managed and disposed of, if required and approved by the Engineer, will be paid according to Article 109.04.”

SEEDING (BDE)

Effective: November 1, 2022

Revise Article 250.07 of the Standard Specifications to read:

“250.07 Seeding Mixtures. The classes of seeding mixtures and combinations of mixtures will be designated in the plans.

When an area is to be seeded with two or more seeding classes, those mixtures shall be applied separately on the designated area within a seven day period. Seeding shall occur prior to placement of mulch cover. A Class 7 mixture can be applied at any time prior to applying any seeding class or added to them and applied at the same time.

TABLE 1 - SEEDING MIXTURES

| Class - Type | Seeds | lb/acre (kg/hectare) |
|--|---|--|
| 1 Lawn Mixture 1/ | Kentucky Bluegrass Perennial Ryegrass <i>Festuca rubra</i> ssp. <i>rubra</i> (Creeping Red Fescue) | 100 (110) 60 (70) 40 (50) |
| 1A Salt Tolerant Lawn Mixture 1/ | Kentucky Bluegrass Perennial Ryegrass <i>Festuca rubra</i> ssp. <i>rubra</i> (Creeping Red Fescue) <i>Festuca brevipilla</i> (Hard Fescue) <i>Puccinellia distans</i> (Fults Saltgrass or Salty Alkaligrass) | 60 (70) 20 (20) 20 (20) 20 (20) 60 (70) |
| 1B Low Maintenance Lawn Mixture 1/ | Turf-Type Fine Fescue 3/ Perennial Ryegrass Red Top <i>Festuca rubra</i> ssp. <i>rubra</i> (Creeping Red Fescue) | 150 (170) 20 (20) 10 (10) 20 (20) |
| 2 Roadside Mixture 1/ | <i>Lolium arundinaceum</i> (Tall Fescue) Perennial Ryegrass <i>Festuca rubra</i> ssp. <i>rubra</i> (Creeping Red Fescue) Red Top | 100 (110) 50 (55) 40 (50) 10 (10) |
| 2A Salt Tolerant Roadside Mixture 1/ | <i>Lolium arundinaceum</i> (Tall Fescue) Perennial Ryegrass <i>Festuca rubra</i> ssp. <i>rubra</i> (Creeping Red Fescue) <i>Festuca brevipilla</i> (Hard Fescue) <i>Puccinellia distans</i> (Fults Saltgrass or Salty Alkaligrass) | 60 (70) 20 (20) 30 (20) 30 (20) 60 (70) |
| 3 Northern Illinois Slope Mixture 1/ | <i>Elymus canadensis</i> (Canada Wild Rye) 5/ Perennial Ryegrass Alsike Clover 4/ <i>Desmanthus illinoensis</i> (Illinois Bundleflower) 4/ 5/ <i>Schizachyrium scoparium</i> (Little Bluestem) 5/ <i>Bouteloua curtipendula</i> (Side-Oats Grama) 5/ <i>Puccinellia distans</i> (Fults Saltgrass or Salty Alkaligrass) Oats, Spring Slender Wheat Grass 5/ Buffalo Grass 5/ 7/ | 5 (5) 20 (20) 5 (5) 2 (2) 12 (12) 10 (10) 30 (35) 50 (55) 15 (15) 5 (5) |
| 3A Southern Illinois Slope Mixture 1/ | Perennial Ryegrass <i>Elymus canadensis</i> (Canada Wild Rye) 5/ <i>Panicum virgatum</i> (Switchgrass) 5/ <i>Schizachyrium scoparium</i> (Little Blue Stem) 5/ <i>Bouteloua curtipendula</i> (Side-Oats Grama) 5/ <i>Dalea candida</i> (White Prairie Clover) 4/ 5/ <i>Rudbeckia hirta</i> (Black-Eyed Susan) 5/ Oats, Spring | 20 (20) 20 (20) 10 (10) 12 (12) 10 (10) 5 (5) 5 (5) 50 (55) |

| Class – Type | | Seeds | lb/acre (kg/hectare) |
|--------------|--|--|----------------------|
| 4 | Native Grass 2/ 6/ | <i>Andropogon gerardi</i> | 4 (4) |
| | | (Big Blue Stem) 5/ | |
| | | <i>Schizachyrium scoparium</i> | 5 (5) |
| | | (Little Blue Stem) 5/ | |
| | | <i>Bouteloua curtipendula</i> | 5 (5) |
| | | (Side-Oats Grama) 5/ | |
| | | <i>Elymus canadensis</i> | 1 (1) |
| | | (Canada Wild Rye) 5/ | |
| | | <i>Panicum virgatum</i> (Switch Grass) 5/ | 1 (1) |
| | | <i>Sorghastrum nutans</i> (Indian Grass) 5/ | 2 (2) |
| 4A | Low Profile Native Grass 2/ 6/ | Annual Ryegrass | 25 (25) |
| | | Oats, Spring | 25 (25) |
| | | Perennial Ryegrass | 15 (15) |
| | | <i>Schizachyrium scoparium</i> | 5 (5) |
| | | (Little Blue Stem) 5/ | |
| | | <i>Bouteloua curtipendula</i> | 5 (5) |
| | | (Side-Oats Grama) 5/ | |
| | | <i>Elymus canadensis</i> | 1 (1) |
| | | (Canada Wild Rye) 5/ | |
| | | <i>Sporobolus heterolepis</i> | 0.5 (0.5) |
| 4B | Wetland Grass and Sedge Mixture 2/ 6/ | (Prairie Dropseed) 5/ | |
| | | Annual Ryegrass | 25 (25) |
| | | Oats, Spring | 25 (25) |
| | | Perennial Ryegrass | 15 (15) |
| | | Wetland Grasses (species below) 5/ | 6 (6) |
| | | <u>Species:</u> | <u>% By Weight</u> |
| | | <i>Calamagrostis canadensis</i> (Blue Joint Grass) | 12 |
| | | <i>Carex lacustris</i> (Lake-Bank Sedge) | 6 |
| | | <i>Carex slipata</i> (Awl-Fruited Sedge) | 6 |
| | | <i>Carex stricta</i> (Tussock Sedge) | 6 |
| | | <i>Carex vulpinoidea</i> (Fox Sedge) | 6 |
| | | <i>Eleocharis acicularis</i> (Needle Spike Rush) | 3 |
| | | <i>Eleocharis obtusa</i> (Blunt Spike Rush) | 3 |
| | | <i>Glyceria striata</i> (Fowl Manna Grass) | 14 |
| | | <i>Juncus effusus</i> (Common Rush) | 6 |
| | | <i>Juncus tenuis</i> (Slender Rush) | 6 |
| | | <i>Juncus torreyi</i> (Torrey's Rush) | 6 |
| | | <i>Leersia oryzoides</i> (Rice Cut Grass) | 10 |
| | | <i>Scirpus acutus</i> (Hard-Stemmed Bulrush) | 3 |
| | | <i>Scirpus atrovirens</i> (Dark Green Rush) | 3 |
| | | <i>Bolboschoenus fluviatilis</i> (River Bulrush) | 3 |
| | | <i>Schoenoplectus tabernaemontani</i> (Softstem Bulrush) | 3 |
| | | <i>Spartina pectinata</i> (Cord Grass) | 4 |

| Class – Type | Seeds | lb/acre (kg/hectare) |
|--------------|---|----------------------|
| 5 | Forb with | 1 (1) |
| | Annuals Mixture 2/ 5/ 6/ | 10 (10) |
| | Annuals Mixture (Below) | |
| | Forb Mixture (Below) | |
| | Annuals Mixture - Mixture not exceeding 25 % by weight of any one species, of the following: | |
| | <i>Coreopsis lanceolata</i> (Sand Coreopsis) | |
| | <i>Leucanthemum maximum</i> (Shasta Daisy) | |
| | <i>Gaillardia pulchella</i> (Blanket Flower) | |
| | <i>Ratibida columnifera</i> (Prairie Coneflower) | |
| | <i>Rudbeckia hirta</i> (Black-Eyed Susan) | |
| | Forb Mixture - Mixture not exceeding 5 % by weight PLS of any one species, of the following: | |
| | <i>Amorpha canescens</i> (Lead Plant) 4/ | |
| | <i>Anemone cylindrica</i> (Thimble Weed) | |
| | <i>Asclepias tuberosa</i> (Butterfly Weed) | |
| | <i>Aster azureus</i> (Sky Blue Aster) | |
| | <i>Symphyotrichum leave</i> (Smooth Aster) | |
| | <i>Aster novae-angliae</i> (New England Aster) | |
| | <i>Baptisia leucantha</i> (White Wild Indigo) 4/ | |
| | <i>Coreopsis palmata</i> (Prairie Coreopsis) | |
| | <i>Echinacea pallida</i> (Pale Purple Coneflower) | |
| | <i>Eryngium yuccifolium</i> (Rattlesnake Master) | |
| | <i>Helianthus mollis</i> (Downy Sunflower) | |
| | <i>Heliopsis helianthoides</i> (Ox-Eye) | |
| | <i>Liatris aspera</i> (Rough Blazing Star) | |
| | <i>Liatris pycnostachya</i> (Prairie Blazing Star) | |
| | <i>Monarda fistulosa</i> (Prairie Bergamot) | |
| | <i>Parthenium integrifolium</i> (Wild Quinine) | |
| | <i>Dalea candida</i> (White Prairie Clover) 4/ | |
| | <i>Dalea purpurea</i> (Purple Prairie Clover) 4/ | |
| | <i>Physostegia virginiana</i> (False Dragonhead) | |
| | <i>Potentilla arguta</i> (Prairie Cinquefoil) | |
| | <i>Ratibida pinnata</i> (Yellow Coneflower) | |
| | <i>Rudbeckia subtomentosa</i> (Fragrant Coneflower) | |
| | <i>Silphium laciniatum</i> (Compass Plant) | |
| | <i>Silphium terebinthinaceum</i> (Prairie Dock) | |
| | <i>Oligoneuron rigidum</i> (Rigid Goldenrod) | |
| | <i>Tradescantia ohimensis</i> (Spiderwort) | |
| | <i>Veronicastrum virginicum</i> (Culver's Root) | |

| Class – Type | | Seeds | lb/acre (kg/hectare) |
|--------------|--|---|----------------------|
| 5A | Large Flower Native Forb Mixture 2/ 5/ 6/ | Forb Mixture (see below) | 5 (5) |
| | | <u>Species:</u> | <u>% By Weight</u> |
| | | <i>Aster novae-angliae</i> (New England Aster) | 5 |
| | | <i>Echinacea pallida</i> (Pale Purple Coneflower) | 10 |
| | | <i>Helianthus mollis</i> (Downy Sunflower) | 10 |
| | | <i>Heliopsis helianthoides</i> (Ox-Eye) | 10 |
| | | <i>Liatris pycnostachya</i> (Prairie Blazing Star) | 10 |
| | | <i>Ratibida pinnata</i> (Yellow Coneflower) | 5 |
| | | <i>Rudbeckia hirta</i> (Black-Eyed Susan) | 10 |
| | | <i>Silphium laciniatum</i> (Compass Plant) | 10 |
| | | <i>Silphium terebinthinaceum</i> (Prairie Dock) | 20 |
| | | <i>Oligoneuron rigidum</i> (Rigid Goldenrod) | 10 |
| 5B | Wetland Forb 2/ 5/ 6/ | Forb Mixture (see below) | 2 (2) |
| | | <u>Species:</u> | <u>% By Weight</u> |
| | | <i>Acorus calamus</i> (Sweet Flag) | 3 |
| | | <i>Angelica atropurpurea</i> (Angelica) | 6 |
| | | <i>Asclepias incarnata</i> (Swamp Milkweed) | 2 |
| | | <i>Aster puniceus</i> (Purple Stemmed Aster) | 10 |
| | | <i>Bidens cernua</i> (Beggarticks) | 7 |
| | | <i>Eutrochium maculatum</i> (Spotted Joe Pye Weed) | 7 |
| | | <i>Eupatorium perfoliatum</i> (Boneset) | 7 |
| | | <i>Helenium autumnale</i> (Autumn Sneeze Weed) | 2 |
| | | <i>Iris virginica shrevei</i> (Blue Flag Iris) | 2 |
| | | <i>Lobelia cardinalis</i> (Cardinal Flower) | 5 |
| | | <i>Lobelia siphilitica</i> (Great Blue Lobelia) | 5 |
| | | <i>Lythrum alatum</i> (Winged Loosestrife) | 2 |
| | | <i>Physostegia virginiana</i> (False Dragonhead) | 5 |
| | | <i>Persicaria pensylvanica</i> (Pennsylvania Smartweed) | 10 |
| | | <i>Persicaria lapathifolia</i> (Curlytop Knotweed) | 10 |
| | | <i>Pychanthemum virginianum</i> (Mountain Mint) | 5 |
| | | <i>Rudbeckia laciniata</i> (Cut-leaf Coneflower) | 5 |
| | | <i>Oligoneuron riddellii</i> (Riddell Goldenrod) | 2 |
| | | <i>Sparganium eurycarpum</i> (Giant Burreed) | 5 |
| 6 | Conservation Mixture 2/ 6/ | <i>Schizachyrium scoparium</i> (Little Blue Stem) 5/ | 5 (5) |
| | | <i>Elymus canadensis</i> (Canada Wild Rye) 5/ | 2 (2) |
| | | Buffalo Grass 5/ 7/ | 5 (5) |
| | | Vernal Alfalfa 4/ | 15 (15) |
| | | Oats, Spring | 48 (55) |
| 6A | Salt Tolerant Conservation Mixture 2/ 6/ | <i>Schizachyrium scoparium</i> (Little Blue Stem) 5/ | 5 (5) |
| | | <i>Elymus canadensis</i> (Canada Wild Rye) 5/ | 2 (2) |
| | | Buffalo Grass 5/ 7/ | 5 (5) |
| | | Vernal Alfalfa 4/ | 15 (15) |
| | | Oats, Spring | 48 (55) |
| | | <i>Puccinellia distans</i> (Fults Saltgrass or Salty Alkaligrass) | 20 (20) |
| 7 | Temporary Turf Cover Mixture | Perennial Ryegrass | 50 (55) |
| | | Oats, Spring | 64 (70) |

Notes:

- 1/ Seeding shall be performed when the ambient temperature has been between 45 °F (7 °C) and 80 °F (27 °C) for a minimum of seven (7) consecutive days and is forecasted to be the same for the next five (5) days according to the National Weather Service.
- 2/ Seeding shall be performed in late fall through spring beginning when the ambient temperature has been below 45 °F (7 °C) for a minimum of seven (7) consecutive days and ending when the ambient temperature exceeds 80 °F (27 °C) according to the National Weather Service.
- 3/ Specific variety as shown in the plans or approved by the Engineer.
- 4/ Inoculation required.
- 5/ Pure Live Seed (PLS) shall be used.
- 6/ Fertilizer shall not be used.
- 7/ Seed shall be primed with KNO₃ to break dormancy and dyed to indicate such.

Seeding will be inspected after a period of establishment. The period of establishment shall be six (6) months minimum, but not to exceed nine (9) months. After the period of establishment, areas not exhibiting 75 percent uniform growth shall be interseeded or reseeded, as determined by the Engineer, at no additional cost to the Department."

SHORT TERM AND TEMPORARY PAVEMENT MARKINGS (BDE)

Effective: April 1, 2024

Revised: April 2, 2024

Revise Article 701.02(d) of the Standard Specifications to read:

"(d) Pavement Marking Tapes (Note 3)1095.06"

Add the following Note to the end of Article 701.02 of the Standard Specifications:

"Note 3. White or yellow pavement marking tape that is to remain in place longer than 14 days shall be Type IV tape."

Revise Article 703.02(c) of the Standard Specifications to read:

"(c) Pavement Marking Tapes (Note 1)1095.06"

Add the following Note to the end of Article 703.02 of the Standard Specifications:

"Note 1. White or yellow pavement marking tape that is to remain in place longer than 14 days shall be Type IV tape."

Revise Article 1095.06 of the Standard Specifications to read:

"1095.06 Pavement Marking Tapes. Type I white or yellow marking tape shall consist of glass spheres embedded into a binder on a foil backing that is precoated with a pressure sensitive adhesive. The spheres shall be of uniform gradation and distributed evenly over the surface of the tape.

Type IV tape shall consist of white or yellow tape with wet reflective media incorporated to provide immediate and continuing retroreflection in wet and dry conditions. The wet retroreflective media shall be bonded to a durable polyurethane surface. The patterned surface shall have approximately 40 ± 10 percent of the surface area raised and presenting a near vertical face to traffic from any direction. The channels between the raised areas shall be substantially free of exposed reflective elements or particles.

Blackout tape shall consist of a matte black, non-reflective, patterned surface that is precoated with a pressure sensitive adhesive.

- (a) Color. The white and yellow markings shall meet the following requirements for daylight reflectance and color, when tested, using a color spectrophotometer with 45 degrees circumferential/zero degree geometry, illuminant D65, and two degree observer angle. The color instrument shall measure the visible spectrum from 380 to 720 nm with a wavelength measurement interval and spectral bandpass of 10 nm.

| Color | Daylight Reflectance %Y |
|----------|-------------------------|
| White | 65 min. |
| Yellow * | 36 - 59 |

*Shall match Aerospace Material Specification Standard 595 33538 (Orange Yellow) and the chromaticity limits as follows.

| | | | | |
|---|-------|-------|-------|-------|
| x | 0.490 | 0.475 | 0.485 | 0.530 |
| y | 0.470 | 0.438 | 0.425 | 0.456 |

- (b) Retroreflectivity. The white and yellow markings shall be retroreflective. Reflective values measured in accordance with the photometric testing procedure of ASTM D 4061 shall not be less than those listed in the table below. The coefficient of retroreflected luminance, R_L , shall be expressed as average millicandelas/footcandle/sq ft (millicandelas/lux/sq m), measured on a 3.0 x 0.5 ft (900 mm x 150 mm) panel at 86 degree entrance angle.

| Coefficient of Retroreflected Luminance, R_L , Dry | | | | | |
|--|-------|--------|-------------------|-------|--------|
| Type I | | | Type IV | | |
| Observation Angle | White | Yellow | Observation Angle | White | Yellow |
| 0.2° | 2700 | 2400 | 0.2° | 1300 | 1200 |
| 0.5° | 2250 | 2000 | 0.5° | 1100 | 1000 |

Wet retroreflectance shall be measured for Type IV under wet conditions according to ASTM E 2177 and meet the following.

| Wet Retroreflectance, Initial R_L | |
|-------------------------------------|------------------|
| Color | R_L 1.05/88.76 |
| White | 300 |
| Yellow | 200 |

- (c) Skid Resistance. The surface of Type IV and blackout markings shall provide a minimum skid resistance of 45 BPN when tested according to ASTM E 303.
- (d) Application. The pavement marking tape shall have a precoated pressure sensitive adhesive and shall require no activation procedures. Test pieces of the tape shall be applied according to the manufacturer's instructions and tested according to ASTM D 1000, Method A, except that a stiff, short bristle roller brush and heavy hand pressure will be substituted for the weighted rubber roller in applying the test pieces to the metal test panel. Material tested as directed above shall show a minimum adhesion value of 750 g/in. (30 g/mm) width at the temperatures specified in ASTM D 1000. The adhesive shall be resistant to oils, acids, solvents, and water, and shall not leave objectionable stains or residue after removal. The material shall be flexible and conformable to the texture of the pavement.
- (e) Durability. Type IV and blackout tape shall be capable of performing for the duration of a normal construction season and shall then be capable of being removed intact or in large sections at pavement temperatures above 40 °F (4 °C) either manually or with a roll-up device without the use of sandblasting, solvents, or grinding. The Contractor shall provide a manufacturer's certification that the material meets the requirements for being removed after the following minimum traffic exposure based on transverse test decks with rolling traffic.
- (1) Time in place - 400 days
 - (2) ADT per lane - 9,000 (28 percent trucks)
 - (3) Axle hits - 10,000,000 minimum

Samples of the material applied to standard specimen plates will be measured for thickness and tested for durability in accordance with ASTM D 4060, using a CS-17 wheel and 1000-gram load, and shall meet the following criteria showing no significant change in color after being tested for the number of cycles indicated.

| Test | Type I | Type IV | Blackout |
|--------------------------------------|-----------|--|--|
| Minimum Initial Thickness, mils (mm) | 20 (0.51) | 65 (1.65) ^{1/} 20 (0.51) ^{2/} | 65 (1.65) ^{1/} 20 (0.51) ^{2/} |
| Durability (cycles) | 5,000 | 1,500 | 1,500 |

1/ Measured at the thickest point of the patterned surface.

2/ Measured at the thinnest point of the patterned surface.

The pavement marking tape, when applied according to the manufacturer's recommended procedures, shall be weather resistant and shall show no appreciable fading, lifting, or shrinkage during the useful life of the marking. The tape, as applied, shall be of good appearance, free of cracks, and edges shall be true, straight, and unbroken.

(f) Sampling and Inspection.

- (1) Sample. Prior to approval and use of Type IV pavement marking tape, the manufacturer shall submit a notarized certification from an independent laboratory, together with the results of all tests, stating that the material meets the requirements as set forth herein. The independent laboratory test report shall state the lot tested, the manufacturer's name, and the date of manufacture.

After initial approval by the Department, samples and certification by the manufacturer shall be submitted for each subsequent batch of Type IV tape used. The manufacturer shall submit a certification stating that the material meets the requirements as set forth herein and is essentially identical to the material sent for qualification. The certification shall state the lot tested, the manufacturer's name, and the date of manufacture.

- (2) Inspection. The Contractor shall provide a manufacturer's certification to the Engineer stating the material meets all requirements of this specification. All material samples for acceptance tests shall be taken or witnessed by a representative of the Bureau of Materials and shall be submitted to the Engineer of Materials, 126 East Ash Street, Springfield, Illinois 62704-4766 at least 30 days in advance of the pavement marking operations."

SIGN PANELS AND APPURTENANCES (BDE)

Effective: January 1, 2025

Revised: April 1, 2025

Add Article 720.02(c) of the Standard Specifications to read:

"(c) Aluminum Epoxy Mastic1008.03"

Revise the second and third paragraphs of Article 720.02 of the Standard Specifications to read:

"The sign mounting support channel shall be manufactured from steel or aluminum and shall be according to Standard 720001.

Steel support channels shall be according to ASTM A 1011 (A 1011M), ASTM A 635 (A 635M), ASTM A 568 (A 568M), or ASTM A 684 (A 684M), and shall be galvanized. Galvanizing shall be according to ASTM A 653 (A 653M) when galvanized before fabrication, and AASHTO M 111 (M 111M) when galvanized after fabrication. Field or post fabricated drilled holes shall be spot painted with one coat of aluminum epoxy mastic paint prior to installation."

Revise the fifth paragraph of Article 720.02 of the Standard Specifications to read:

"The stainless steel banding for mounting signs or sign support channels to light or signal standards shall be according to ASTM A 240 (A 240M) Type 302 stainless steel."

SLOPE WALL (BDE)

Effective: August 1, 2025

Revise Article 511.02(b) of the Standard Specifications to read:

“(b) Welded Wire Reinforcement (Note 3) 1006.10”

Add the following note to the end of Article 511.02 of the Standard Specifications.

“Note 3. Welded wire reinforcement used for concrete slope wall shall be epoxy coated or galvanized according to AASHTO M 111 (M 111M).”

Revise the second sentence of Article 511.03(a) of the Standard Specifications to read:

“Concrete slope walls shall be reinforced with welded wire reinforcement consisting of 6 x 6 in. (150 x 150 mm) mesh, #4 gauge (5.74 mm), 58 lb (26 kg) per 100 sq ft (9 sq m) and supported 2 in. (50 mm) below the upper surface of the slope wall by concrete blocks.”

Add the following to the end of Article 511.03(a) of the Standard Specifications as the last paragraph.

“The concrete slope wall shall be given a broom finish. The broom shall be drawn vertically along the slope wall surface, with adjacent strokes slightly overlapping, producing a uniform, slightly rough surface with parallel broom marks.”

SOURCE OF SUPPLY AND QUALITY REQUIREMENTS (BDE)

Effective: January 2, 2023

Add the following to Article 106.01 of the Standard Specifications:

“The final manufacturing process for construction materials and the immediately preceding manufacturing stage for construction materials shall occur within the United States. Construction materials shall include an article, material, or supply that is or consists primarily of the following.

- (a) Non-ferrous metals;
- (b) Plastic and polymer-based products (including polyvinylchloride, composite building materials, and polymers used in fiber optic cables);
- (c) Glass (including optic glass);
- (d) Lumber;
- (e) Drywall.

Items consisting of two or more of the listed construction materials that have been combined through a manufacturing process, and items including at least one of the listed materials combined with a material that is not listed through a manufacturing process shall be exempt.”

STEEL COST ADJUSTMENT (BDE)

Effective: April 2, 2004

Revised: January 1, 2022

Description. Steel cost adjustments will be made to provide additional compensation to the Contractor, or a credit to the Department, for fluctuations in steel prices when optioned by the Contractor. The bidder shall indicate with their bid whether or not this special provision will be part of the contract. Failure to indicate "Yes" for any item of work will make that item of steel exempt from steel cost adjustment.

Types of Steel Products. An adjustment will be made for fluctuations in the cost of steel used in the manufacture of the following items:

- Metal Piling (excluding temporary sheet piling)
- Structural Steel
- Reinforcing Steel

Other steel materials such as dowel bars, tie bars, welded reinforcement, guardrail, steel traffic signal and light poles, towers and mast arms, metal railings (excluding wire fence), and frames and grates will be subject to a steel cost adjustment when the pay items they are used in have a contract value of \$10,000 or greater.

The adjustments shall apply to the above items when they are part of the original proposed construction, or added as extra work and paid for by agreed unit prices. The adjustments shall not apply when the item is added as extra work and paid for at a lump sum price or by force account.

Documentation. Sufficient documentation shall be furnished to the Engineer to verify the following:

- (a) The dates and quantity of steel, in lb (kg), shipped from the mill to the fabricator.
- (b) The quantity of steel, in lb (kg), incorporated into the various items of work covered by this special provision. The Department reserves the right to verify submitted quantities.

Method of Adjustment. Steel cost adjustments will be computed as follows:

$$SCA = Q \times D$$

Where: SCA = steel cost adjustment, in dollars
Q = quantity of steel incorporated into the work, in lb (kg)
D = price factor, in dollars per lb (kg)

$$D = MPI_M - MPI_L$$

Where: MPI_M = The Materials Cost Index for steel as published by the Engineering News-Record for the month the steel is shipped from the mill. The indices will be converted from dollars per 100 lb to dollars per lb (kg).

MPI_L = The Materials Cost Index for steel as published by the Engineering News-Record for the month prior to the letting for work paid for at the contract price; or for the month the agreed unit price letter is submitted by the Contractor for extra work paid for by agreed unit price,. The indices will be converted from dollars per 100 lb to dollars per lb (kg).

The unit weights (masses) of steel that will be used to calculate the steel cost adjustment for the various items are shown in the attached table.

No steel cost adjustment will be made for any products manufactured from steel having a mill shipping date prior to the letting date.

If the Contractor fails to provide the required documentation, the method of adjustment will be calculated as described above; however, the MPI_M will be based on the date the steel arrives at the job site. In this case, an adjustment will only be made when there is a decrease in steel costs.

Basis of Payment. Steel cost adjustments may be positive or negative but will only be made when there is a difference between the MPI_L and MPI_M in excess of five percent, as calculated by:

$$\text{Percent Difference} = \{(MPI_L - MPI_M) \div MPI_L\} \times 100$$

Steel cost adjustments will be calculated by the Engineer and will be paid or deducted when all other contract requirements for the items of work are satisfied. Adjustments will only be made for fluctuations in the cost of the steel as described herein. No adjustment will be made for changes in the cost of manufacturing, fabrication, shipping, storage, etc.

The adjustments shall not apply during contract time subject to liquidated damages for completion of the entire contract.

Attachment

| Item | Unit Mass (Weight) |
|--|--------------------------------|
| Metal Piling (excluding temporary sheet piling) | |
| Furnishing Metal Pile Shells 12 in. (305 mm), 0.179 in. (3.80 mm) wall thickness | 23 lb/ft (34 kg/m) |
| Furnishing Metal Pile Shells 12 in. (305 mm), 0.250 in. (6.35 mm) wall thickness | 32 lb/ft (48 kg/m) |
| Furnishing Metal Pile Shells 14 in. (356 mm), 0.250 in. (6.35 mm) wall thickness | 37 lb/ft (55 kg/m) |
| Other piling | See plans |
| Structural Steel | See plans for weights (masses) |
| Reinforcing Steel | See plans for weights (masses) |
| Dowel Bars and Tie Bars | 6 lb (3 kg) each |
| Welded Reinforcement | 63 lb/100 sq ft (310 kg/sq m) |
| Guardrail | |
| Steel Plate Beam Guardrail, Type A w/steel posts | 20 lb/ft (30 kg/m) |
| Steel Plate Beam Guardrail, Type B w/steel posts | 30 lb/ft (45 kg/m) |
| Steel Plate Beam Guardrail, Types A and B w/wood posts | 8 lb/ft (12 kg/m) |
| Steel Plate Beam Guardrail, Type 2 | 305 lb (140 kg) each |
| Steel Plate Beam Guardrail, Type 6 | 1260 lb (570 kg) each |
| Traffic Barrier Terminal, Type 1 Special (Tangent) | 730 lb (330 kg) each |
| Traffic Barrier Terminal, Type 1 Special (Flared) | 410 lb (185 kg) each |
| Steel Traffic Signal and Light Poles, Towers and Mast Arms | |
| Traffic Signal Post | 11 lb/ft (16 kg/m) |
| Light Pole, Tenon Mount and Twin Mount, 30 - 40 ft (9 – 12 m) | 14 lb/ft (21 kg/m) |
| Light Pole, Tenon Mount and Twin Mount, 45 - 55 ft (13.5 – 16.5 m) | 21 lb/ft (31 kg/m) |
| Light Pole w/Mast Arm, 30 - 50 ft (9 – 15.2 m) | 13 lb/ft (19 kg/m) |
| Light Pole w/Mast Arm, 55 - 60 ft (16.5 – 18 m) | 19 lb/ft (28 kg/m) |
| Light Tower w/Luminaire Mount, 80 - 110 ft (24 – 33.5 m) | 31 lb/ft (46 kg/m) |
| Light Tower w/Luminaire Mount, 120 - 140 ft (36.5 – 42.5 m) | 65 lb/ft (97 kg/m) |
| Light Tower w/Luminaire Mount, 150 - 160 ft (45.5 – 48.5 m) | 80 lb/ft (119 kg/m) |
| Metal Railings (excluding wire fence) | |
| Steel Railing, Type SM | 64 lb/ft (95 kg/m) |
| Steel Railing, Type S-1 | 39 lb/ft (58 kg/m) |
| Steel Railing, Type T-1 | 53 lb/ft (79 kg/m) |
| Steel Bridge Rail | 52 lb/ft (77 kg/m) |
| Frames and Grates | |
| Frame | 250 lb (115 kg) |
| Lids and Grates | 150 lb (70 kg) |

SUBCONTRACTOR AND DBE PAYMENT REPORTING (BDE)

Effective: April 2, 2018

Add the following to Section 109 of the Standard Specifications.

“109.14 Subcontractor and Disadvantaged Business Enterprise Payment Reporting.
The Contractor shall report all payments made to the following parties:

- (a) first tier subcontractors;
- (b) lower tier subcontractors affecting disadvantaged business enterprise (DBE) goal credit;
- (c) material suppliers or trucking firms that are part of the Contractor’s submitted DBE utilization plan.

The report shall be made through the Department’s on-line subcontractor payment reporting system within 21 days of making the payment.”

SUBCONTRACTOR MOBILIZATION PAYMENTS (BDE)

Effective: November 2, 2017

Revised: April 1, 2019

Replace the second paragraph of Article 109.12 of the Standard Specifications with the following:

“This mobilization payment shall be made at least seven days prior to the subcontractor starting work. The amount paid shall be at the following percentage of the amount of the subcontract reported on form BC 260A submitted for the approval of the subcontractor’s work.

| Value of Subcontract Reported on Form BC 260A | Mobilization Percentage |
|---|-------------------------|
| Less than \$10,000 | 25% |
| \$10,000 to less than \$20,000 | 20% |
| \$20,000 to less than \$40,000 | 18% |
| \$40,000 to less than \$60,000 | 16% |
| \$60,000 to less than \$80,000 | 14% |
| \$80,000 to less than \$100,000 | 12% |
| \$100,000 to less than \$250,000 | 10% |
| \$250,000 to less than \$500,000 | 9% |
| \$500,000 to \$750,000 | 8% |
| Over \$750,000 | 7%” |

SUBMISSION OF BIDDERS LIST INFORMATION (BDE)

Effective: January 2, 2025

Revised: March 2, 2025

In accordance with 49 CFR 26.11(c) all DBE and non-DBEs who bid as prime contractors and subcontractors shall provide bidders list information, including all DBE and non-DBE firms from whom the bidder has received a quote or bid to work as a subcontractor, whether or not the bidder has relied upon that bid in placing its bid as the prime contractor.

The bidders list information shall be submitted with the bid using the link provided within the "Integrated Contractor Exchange (iCX)" application of the Department's "EBids System".

SUBMISSION OF PAYROLL RECORDS (BDE)

Effective: April 1, 2021

Revised: November 2, 2023

FEDERAL AID CONTRACTS. Revise the following section of Check Sheet #1 of the Recurring Special Provisions to read:

"STATEMENTS AND PAYROLLS

The payroll records shall include the worker's name, social security number, last known address, telephone number, email address, classification(s) of work actually performed, hourly rates of wages paid (including rates of contributions or costs anticipated for bona fide fringe benefits or cash equivalents thereof), daily and weekly number of hours actually worked in total, deductions made, and actual wages paid.

The Contractor and each subcontractor shall submit certified payroll records to the Department each week from the start to the completion of their respective work, except that full social security numbers, last known addresses, telephone numbers, and email addresses shall not be included on weekly submittals. Instead, the payrolls need only include an identification number for each employee (e.g., the last four digits of the employee's social security number). The submittals shall be made using LCPTracker Pro software. The software is web-based and can be accessed at <https://lcptracker.com/>. When there has been no activity during a work week, a payroll record shall still be submitted with the appropriate option ("No Work", "Suspended", or "Complete") selected."

STATE CONTRACTS. Revise Item 3 of Section IV of Check Sheet #5 of the Recurring Special Provisions to read:

- "3. Submission of Payroll Records. The Contractor and each subcontractor shall, no later than the 15th day of each calendar month, file a certified payroll for the immediately preceding month to the Illinois Department of Labor (IDOL) through the Illinois Prevailing Wage Portal in compliance with the State Prevailing Wage Act (820 ILCS 130). The portal can be found on the IDOL website at <https://www2.illinois.gov/idol/Laws-Rules/CONMED/Pages/Prevailing-Wage-Portal.aspx>. Payrolls shall be submitted in the format prescribed by the IDOL.

In addition to filing certified payroll(s) with the IDOL, the Contractor and each subcontractor shall certify and submit payroll records to the Department each week from the start to the completion of their respective work, except that full social security numbers shall not be included on weekly submittals. Instead, the payrolls shall include an identification number for each employee (e.g., the last four digits of the employee's social security number). In addition, starting and ending times of work each day may be omitted from the payroll records submitted. The submittals shall be made using LCPtracker Pro software. The software is web-based and can be accessed at <https://lcptracker.com/>. When there has been no activity during a work week, a payroll record shall still be submitted with the appropriate option ("No Work", "Suspended", or "Complete") selected."

SURFACE TESTING OF PAVEMENTS – IRI (BDE)

Effective: January 1, 2021

Revised: January 1, 2023

Description. This work shall consist of testing the ride quality of the finished surface of pavement sections with new concrete pavement, PCC overlays, full-depth HMA, and HMA overlays with at least 2.25 in. (57 mm) total thickness of new HMA combined with either HMA binder or HMA surface removal, according to Illinois Test Procedure 701, "Ride Quality Testing Using the International Roughness Index (IRI)". Work shall be according to Sections 406, 407, or 420 of the Standard Specifications, except as modified herein.

Hot-Mix Asphalt (HMA) Overlays

Add the following to Article 406.03 of the Standard Specifications:

"(n) Pavement Surface Grinding Equipment.....1101.04"

Revise Article 406.11 of the Standard Specifications to read:

"406.11 Surface Tests. Prior to HMA overlay pavement improvements, the Engineer will measure the smoothness of the existing high-speed mainline pavement. The Contractor shall measure the smoothness of the finished high-speed mainline, low-speed mainline, and miscellaneous pavements after the pavement improvement is complete but within the same construction season. Testing shall be performed in the presence of the Engineer and according to Illinois Test Procedure 701. The pavement will be identified as high-speed mainline, low-speed mainline, or miscellaneous as follows.

(a) Test Sections.

- (1) **High-Speed Mainline Pavement.** High-speed mainline pavement consists of pavements, ramps, and loops with a posted speed limit greater than 45 mph. These sections shall be tested with an inertial profiling system (IPS).
- (2) **Low-Speed Mainline Pavement.** Low-speed mainline pavement consists of pavements, ramps, and loops with a posted speed limit of 45 mph or less. These sections shall be tested using a 16 ft (5 m) straightedge or with an IPS analyzed using the rolling 16 ft (5 m) straightedge simulation in ProVAL.

- (3) Miscellaneous Pavement. Miscellaneous pavement are segments that either cannot readily be tested by an IPS or conditions beyond the control of the Contractor preclude the achievement of smoothness levels typically achievable with mainline pavement construction. This may include the following examples or as determined by the Engineer.
- a. Pavement on horizontal curves with a centerline radius of curvature of less than or equal to 1,000 ft (300 m) and the pavement within the superelevation transition of such curves;
 - b. Pavement on vertical curves having a length less than or equal to 200 ft (60 m) in combination with an algebraic change in tangent grade greater than or equal to 3 percent as may occur on urban ramps or other constricted-space facilities;
 - c. The first and last 50 ft (15 m) of a pavement section where the Contractor is not responsible for the adjoining surface;
 - d. Intersections and the 25 ft (7.6 m) before and after an intersection or end of radius return;
 - e. Variable width pavements;
 - f. Side street returns, to the end of radius return;
 - g. Crossovers;
 - h. Pavement connector for bridge approach slab;
 - i. Bridge approach slab;
 - j. Pavement that must be constructed in segments of 600 ft (180 m) or less;
 - k. Pavement within 25 ft (7.6 m) of manholes, utility structures, at-grade railroad crossings, or other appurtenances;
 - l. Turn lanes; and
 - m. Pavement within 5 ft (1.5 m) of jobsite sampling locations for HMA volumetric testing that fall within the wheel path.

Miscellaneous pavement shall be tested using a 16 ft (5 m) straightedge.

- (4) International Roughness Index (IRI). An index computed from a longitudinal profile measurement using a quarter-car simulation at a simulation speed of 50 mph (80 km/h).
- (5) Mean Roughness Index (MRI). The average of the IRI values for the right and left wheel tracks.
- a. MRI₀. The MRI of the existing pavement prior to construction.

- b. MRI_I . The MRI value that warrants an incentive payment.
 - c. MRI_F . The MRI value that warrants full payment.
 - d. MRI_D . The MRI value that warrants a financial disincentive.
- (6) Areas of Localized Roughness (ALR). Isolated areas of roughness, which can cause significant increase in the calculated MRI for a given subplot.
- (7) Sublot. A continuous strip of pavement 0.1 mile (160 m) long and one lane wide. A partial subplot greater than or equal to 264 ft (80 m) will be subject to the same evaluation as a whole subplot. Partial sublots less than 264 ft (80 m) shall be included with the previous subplot for evaluation purposes.
- (b) Corrective Work. Corrective work shall be completed according to the following.
- (1) High-Speed Mainline Pavement. For high-speed mainline pavement, any 25 ft (7.6 m) interval with an ALR in excess of 200 in./mile (3,200 mm/km) will be identified by the Engineer and shall be corrected by the Contractor. Any subplot having a MRI greater than MRI_D , including ALR, shall be corrected to reduce the MRI to the MRI_F , or replaced at the Contractor's option.
 - (2) Low-Speed Mainline Pavement. Surface variations in low-speed mainline pavement which exceed the 5/16 in. (8 mm) tolerance will be identified by the Engineer and shall be corrected by the Contractor.
 - (3) Miscellaneous Pavements. Surface variations in miscellaneous pavement which exceed the 5/16 in. (8 mm) tolerance will be identified by the Engineer and shall be corrected by the Contractor.
- Corrective work shall be completed with pavement surface grinding equipment or by removing and replacing the pavement. Corrective work shall be applied to the full lane width. When completed, the corrected area shall have uniform texture and appearance, with the beginning and ending of the corrected area perpendicular to the centerline of the paved surface.
- Upon completion of the corrective work, the surface of the subplot(s) shall be retested. The Contractor shall furnish the data and reports to the Engineer within 2 working days after corrections are made. If the MRI and/or ALR still do not meet the requirements, additional corrective work shall be performed.
- Corrective work shall be at no additional cost to the Department.
- (c) Smoothness Assessments. Assessments will be paid to or deducted from the Contractor for each subplot of high-speed mainline pavement per the Smoothness Assessment Schedule. Assessments will be based on the MRI of each subplot prior to performing any corrective work unless the Contractor has chosen to remove and replace the pavement. For pavement that is replaced, assessments will be based on the MRI determined after replacement.

The upper MRI thresholds for high-speed mainline pavement are dependent on the MRI of the existing pavement before construction (MRI_0) and shall be determined as follows.

| Upper MRI Thresholds ^{1/} | MRI Thresholds (High-Speed, HMA Overlay) | |
|------------------------------------|--|--|
| | $MRI_0 \leq 125.0$ in./mile ($\leq 1,975$ mm/km) | $MRI_0 > 125.0$ in./mile ^{1/} ($> 1,975$ mm/km) |
| Incentive (MRI_I) | 45.0 in./mile (710 mm/km) | $0.2 \times MRI_0 + 20$ |
| Full Pay (MRI_F) | 75.0 in./mile (1,190 mm/km) | $0.2 \times MRI_0 + 50$ |
| Disincentive (MRI_D) | 100.0 in./mile (1,975 mm/km) | $0.2 \times MRI_0 + 75$ |

1/ MRI_0 , MRI_I , MRI_F , and MRI_D shall be in in./mile for calculation.

Smoothness assessments for high-speed mainline pavement shall be determined as follows.

| SMOOTHNESS ASSESSMENT SCHEDULE (High-Speed, HMA Overlay) | |
|--|--|
| Mainline Pavement MRI Range | Assessment Per Sublot ^{1/} |
| $MRI \leq MRI_I$ | $+ (MRI_I - MRI) \times \$20.00$ ^{2/} |
| $MRI_I < MRI \leq MRI_F$ | $+ \$0.00$ |
| $MRI_F < MRI \leq MRI_D$ | $- (MRI - MRI_F) \times \$8.00$ |
| $MRI > MRI_D$ | $- \$200.00$ |

1/ MRI , MRI_I , MRI_F , and MRI_D shall be in in./mile for calculation.

2/ The maximum incentive amount shall not exceed \$300.00.

Smoothness assessments will not be paid or deducted until all other contract requirements for the pavement are satisfied. Pavement that is corrected or replaced for reasons other than smoothness, shall be retested as stated herein."

Hot-Mix Asphalt (HMA) Pavement (Full-Depth)

Revise the first paragraph of Article 407.03 of the Standard Specifications to read:

"407.03 Equipment. Equipment shall be according to Article 406.03."

Revise Article 407.09 of the Standard Specifications to read:

"407.09 Surface Tests. The finished surface of the pavement shall be tested for smoothness according to Article 406.11, except as follows:

The testing of the existing pavement prior to improvements shall not apply and the smoothness assessment for high-speed mainline pavement shall be determined according to the following table.

| SMOOTHNESS ASSESSMENT SCHEDULE (High-Speed, Full-Depth HMA) | |
|---|--|
| Mainline Pavement MRI, in./mile (mm/km) | Assessment Per Sublot ^{1/} |
| ≤ 45.0 (710) | $+ (45 - \text{MRI}) \times \45.00 ^{2/} |
| > 45.0 (710) to 75.0 (1,190) | $+ \$0.00$ |
| > 75.0 (1,190) to 100.0 (1,580) | $- (\text{MRI} - 75) \times \20.00 |
| > 100.0 (1,580) | $- \$500.00$ |

1/ MRI shall be in in./mile for calculation.

2/ The maximum incentive amount shall not exceed \$800.00.”

Portland Cement Concrete Pavement

Delete Article 420.03(i) of the Standard Specifications.

Revise Article 420.10 of the Standard Specifications to read:

“420.10 Surface Tests. The finished surface of the pavement shall be tested for smoothness according to Article 406.11, except as follows.

The testing of the existing pavement prior to improvements shall not apply. The Contractor shall measure the smoothness of the finished surface of the pavement after the pavement has attained a flexural strength of 250 psi (3,800 kPa) or a compressive strength of 1,600 psi (20,700 kPa).

Membrane curing damaged during testing shall be repaired as directed by the Engineer at no additional cost to the Department.

- (a) Corrective Work. No further texturing for skid resistance will be required for areas corrected by grinding. Protective coat shall be reapplied to areas ground according to Article 420.18 at no additional cost to the Department.

Jointed portland cement concrete pavement corrected by removal and replacement, shall be corrected in full panel sizes.

- (b) Smoothness Assessments. Smoothness assessment for high-speed mainline pavement shall be determined as follows.

| SMOOTHNESS ASSESSMENT SCHEDULE (High-Speed, PCC) | |
|---|--|
| Mainline Pavement MRI, in./mile (mm/km) ^{3/} | Assessment Per Sublot ^{1/} |
| ≤ 45.0 (710) | $+ (45 - \text{MRI}) \times \60.00 ^{2/} |
| > 45.0 (710) to 75.0 (1,190) | $+ \$0.00$ |
| > 75.0 (1,190) to 100.0 (1,580) | $- (\text{MRI} - 75) \times \37.50 |
| > 100.0 (1,580) | $- \$750.00$ |

1/ MRI shall be in in./mile for calculation.

- 2/ The maximum incentive amount shall not exceed \$1200.00.
- 3/ If pavement is constructed with traffic in the lane next to it, then an additional 10 in./mile will be added to the upper thresholds.”

Removal of Existing Pavement and Appurtenances

Revise the first paragraph of Article 440.04 of the Standard Specifications to read:

“440.04 HMA Surface Removal for Subsequent Resurfacing. The existing HMA surface shall be removed to the depth specified on the plans with a self-propelled milling machine. The removal depth may be varied slightly at the discretion of the Engineer to satisfy the smoothness requirements of the finished pavement. The temperature at which the work is performed, the nature and condition of the equipment, and the manner of performing the work shall be such that the milled surface is not torn, gouged, shoved or otherwise damaged by the milling operation. Sufficient cutting passes shall be made so that all irregularities or high spots are eliminated to the satisfaction of the Engineer. When tested with a 16 ft (5 m) straightedge, the milled surface shall have no surface variations in excess of 3/16 in. (5 mm).”

General Equipment

Revise Article 1101.04 of the Standard Specifications to read:

“1101.04 Pavement Surface Grinding Equipment. The pavement surface grinding device shall have a minimum effective head width of 3 ft (0.9 m).

- (a) Diamond Saw Blade Machine. The machine shall be self-propelled with multiple diamond saw blades.
- (b) Profile Milling Machine. The profile milling machine shall be a drum device with carbide or diamond teeth with spacing of 0.315 in. (8 mm) or less and maintain proper forward speed for surface texture according to the manufacturer’s specifications.”

SURVEYING SERVICES (BDE)

Effective: April 1, 2025

Delete the fourth paragraph of Article 667.04 of the Standard Specifications.

Delete Section 668 of the Standard Specifications.

TRAINING SPECIAL PROVISIONS (BDE)

Effective: October 15, 1975

Revised: September 2, 2021

This Training Special Provision supersedes Section 7b of the Special Provision entitled "Specific Equal Employment Opportunity Responsibilities," and is in implementation of 23 U.S.C. 140(a).

As part of the Contractor's equal employment opportunity affirmative action program, training shall be provided as follows:

The Contractor shall provide on-the-job training aimed at developing full journeyman in the type of trade or job classification involved. The number of trainees to be trained under this contract will be **20**. In the event the Contractor subcontracts a portion of the contract work, it shall determine how many, if any, of the trainees are to be trained by the subcontractor, provided however, that the Contractor shall retain the primary responsibility for meeting the training requirements imposed by this special provision. The Contractor shall also ensure that this Training Special Provision is made applicable to such subcontract. Where feasible, 25 percent of apprentices or trainees in each occupation shall be in their first year of apprenticeship or training.

The number of trainees shall be distributed among the work classifications on the basis of the Contractor's needs and the availability of journeymen in the various classifications within the reasonable area of recruitment. Prior to commencing construction, the Contractor shall submit to the Illinois Department of Transportation for approval the number of trainees to be trained in each selected classification and training program to be used. Furthermore, the Contractor shall specify the starting time for training in each of the classifications. The Contractor will be credited for each trainee it employs on the contract work who is currently enrolled or becomes enrolled in an approved program and will be reimbursed for such trainees as provided hereinafter.

Training and upgrading of minorities and women toward journeyman status is a primary objective of this Training Special Provision. Accordingly, the Contractor shall make every effort to enroll minority trainees and women (e.g. by conducting systematic and direct recruitment through public and private sources likely to yield minority and women trainees) to the extent such persons are available within a reasonable area of recruitment. The Contractor will be responsible for demonstrating the steps it has taken in pursuance thereof, prior to a determination as to whether the Contractor is in compliance with this Training Special Provision. This training commitment is not intended, and shall not be used, to discriminate against any applicant for training, whether a member of a minority group or not.

No employee shall be employed as a trainee in any classification in which he or she has successfully completed a training course leading to journeyman status or in which he or she has been employed as a journeyman. The Contractor should satisfy this requirement by including appropriate questions in the employee application or by other suitable means. Regardless of the method used, the Contractor's records should document the findings in each case.

The minimum length and type of training for each classification will be as established in the training program selected by the Contractor and approved by the Illinois Department of Transportation and the Federal Highway Administration. The Illinois Department of Transportation and the Federal Highway Administration shall approve a program, if it is reasonably calculated to meet the equal employment opportunity obligations of the Contractor and to qualify the average trainee for journeyman status in the classification concerned by the end of the training period. Furthermore, apprenticeship programs registered with the U.S. Department of Labor, Bureau of Apprenticeship and Training, or with a State apprenticeship agency recognized by the Bureau and training programs approved by not necessarily sponsored by the U.S. Department of Labor Employment Training Administration shall also be considered acceptable provided it is being administered in a manner consistent with the equal employment obligations of Federal-aid highway construction contracts. Approval or acceptance of a training program shall be obtained from the State prior to commencing work on the classification covered by the program. It is the intention of these provisions that training is to be provided in the construction crafts rather than clerk-typists or secretarial-type positions. Training is permissible in lower level management positions such as office engineers, estimators, timekeepers, etc., where the training is oriented toward construction applications. Training in the laborer classification may be permitted provided that significant and meaningful training is provided and approved by the Illinois Department of Transportation and the Federal Highway Administration. Some offsite training is permissible as long as the training is an integral part of an approved training program and does not comprise a significant part of the overall training.

Except as otherwise noted below, the Contractor will be reimbursed 80 cents per hour of training given an employee on this contract in accordance with an approved training program. As approved by the Engineer, reimbursement will be made for training of persons in excess of the number specified herein. This reimbursement will be made even though the Contractor receives additional training program funds from other sources, provided such other source does not specifically prohibit the Contractor from receiving other reimbursement. Reimbursement for offsite training indicated above may only be made to the Contractor where he does one or more of the following and the trainees are concurrently employed on a Federal-aid project; contributes to the cost of the training, provides the instruction to the trainee or pays the trainee's wages during the offsite training period.

No payment shall be made to the Contractor if either the failure to provide the required training, or the failure to hire the trainee as a journeyman, is caused by the Contractor and evidences a lack of good faith on the part of the Contractor in meeting the requirement of this Training Special Provision. It is normally expected that a trainee will begin his training on the project as soon as feasible after start of work utilizing the skill involved and remain on the project as long as training opportunities exist in his work classification or until he has completed his training program.

It is not required that all trainees be on board for the entire length of the contract. A Contractor will have fulfilled his responsibilities under this Training Special Provision if he has provided acceptable training to the number of trainees specified. The number trained shall be determined on the basis of the total number enrolled on the contract for a significant period.

Trainees will be paid at least 60 percent of the appropriate minimum journeyman's rate specified in the contract for the first half of the training period, 75 percent for the third quarter of the training period, and 90 percent for the last quarter of the training period, unless apprentices or trainees in an approved existing program are enrolled as trainees on this project. In that case, the appropriate rates approved by the Departments of Labor or Transportation in connection with the existing program shall apply to all trainees being trained for the same classification who are covered by this Training Special Provision.

The Contractor shall furnish the trainee a copy of the program he will follow in providing the training. The Contractor shall provide each trainee with a certification showing the type and length of training satisfactorily complete.

The Contractor shall provide for the maintenance of records and furnish periodic reports documenting its performance under this Training Special Provision.

For contracts with an awarded contract value of \$500,000 or more, the Contractor is required to comply with the Illinois Works Apprenticeship Initiative (30 ILCS 559/20-20 to 20-25) and all applicable administrative rules to the extent permitted by Section 20-20(g). For federally funded projects, the number of trainees to be trained under this contract, as stated in the Training Special Provisions, will be the established goal for the Illinois Works Apprenticeship Initiative 30 ILCS 559/20-20(g). The Contractor shall make a good faith effort to meet this goal. For federally funded projects, the Illinois Works Apprenticeship Initiative will be implemented using the FHWA approved OJT procedures. The Contractor must comply with the recordkeeping and reporting obligations of the Illinois Works Apprenticeship Initiative for the life of the project, including the certification as to whether the trainee/apprentice labor hour goals were met.

Method of Measurement. The unit of measurement is in hours.

Basis of Payment. This work will be paid for at the contract unit price of 80 cents per hour for TRAINEES. The estimated total number of hours, unit price, and total price have been included in the schedule of prices.

IDOT TRAINING PROGRAM GRADUATE ON-THE-JOB TRAINING SPECIAL PROVISION

Effective: August 1, 2012

Revised: February 2, 2017

In addition to the Contractor's equal employment opportunity (EEO) affirmative action efforts undertaken as required by this Contract, the Contractor is encouraged to participate in the incentive program described below to provide additional on-the-job training to certified graduates of the IDOT pre-apprenticeship training program, as outlined in this Special Provision.

IDOT funds, and various Illinois community colleges operate, pre-apprenticeship training programs throughout the State to provide training and skill-improvement opportunities to promote the increased employment of minority groups, disadvantaged persons and women in all aspects of the highway construction industry. The intent of this IDOT Pre-Apprenticeship Training Program Graduate (TPG) special provision (Special Provision) is to place these certified program graduates on the project site for this Contract in order to provide the graduates with meaningful on-the-job training. Pursuant to this Special Provision, the Contractor must make every reasonable effort to recruit and employ certified TPG trainees to the extent such individuals are available within a practicable distance of the project site.

Specifically, participation of the Contractor or its subcontractor in the Program entitles the participant to reimbursement for graduates' hourly wages at \$15.00 per hour per utilized TPG trainee, subject to the terms of this Special Provision. Reimbursement payment will be made even though the Contractor or subcontractor may also receive additional training program funds from other non-IDOT sources for other non-TPG trainees on the Contract, provided such other source does not specifically prohibit the Contractor or subcontractor from receiving reimbursement from another entity through another program, such as IDOT through the TPG program. With regard to any IDOT funded construction training program other than TPG, however, additional reimbursement for other IDOT programs will not be made beyond the TPG Program described in this Special Provision when the TPG Program is utilized.

No payment will be made to the Contractor if the Contractor or subcontractor fails to provide the required on-site training to TPG trainees, as solely determined by IDOT. A TPG trainee must begin training on the project as soon as the start of work that utilizes the relevant trade skill and the TPG trainee must remain on the project site through completion of the Contract, so long as training opportunities continue to exist in the relevant work classification. Should a TPG trainee's employment end in advance of the completion of the Contract, the Contractor must promptly notify the IDOT District EEO Officer for the Contract that the TPG's involvement in the Contract has ended. The Contractor must supply a written report for the reason the TPG trainee involvement terminated, the hours completed by the TPG trainee on the Contract, and the number of hours for which the incentive payment provided under this Special Provision will be, or has been claimed for the separated TPG trainee.

Finally, the Contractor must maintain all records it creates as a result of participation in the Program on the Contract, and furnish periodic written reports to the IDOT District EEO Officer that document its contractual performance under and compliance with this Special Provision. Finally, through participation in the Program and reimbursement of wages, the Contractor is not relieved of, and IDOT has not waived, the requirements of any federal or state labor or employment law applicable to TPG workers, including compliance with the Illinois Prevailing Wage Act.

Method of Measurement: The unit of measurement is in hours.

Basis of Payment: This work will be paid for at the contract unit price of \$15.00 per hour for each utilized certified TPG Program trainee (TRAINEES TRAINING PROGRAM GRADUATE). The estimated total number of hours, unit price, and total price must be included in the schedule of prices for the Contract submitted by Contractor prior to beginning work. The initial number of TPG trainees for which the incentive is available for this contract is 20.

The Department has contracted with several educational institutions to provide screening, tutoring and pre-training to individuals interested in working as a TPG trainee in various areas of common construction trade work. Only individuals who have successfully completed a Pre-Apprenticeship Training Program at these IDOT approved institutions are eligible to be TPG trainees. To obtain a list of institutions that can connect the Contractor with eligible TPG trainees, the Contractor may contact: HCCTP TPG Program Coordinator, Office of Business and Workforce Diversity (IDOT OBWD), Room 319, Illinois Department of Transportation, 2300 S. Dirksen Parkway, Springfield, Illinois 62764. Prior to commencing construction with the utilization of a TPG trainee, the Contractor must submit documentation to the IDOT District EEO Officer for the Contract that provides the names and contact information of the TPG trainee(s) to be trained in each selected work classification, proof that the TPG trainee(s) has successfully completed a Pre-Apprenticeship Training Program, proof that the TPG is in an Apprenticeship Training Program approved by the U.S. Department of Labor Bureau of Apprenticeship Training, and the start date

for training in each of the applicable work classifications.

To receive payment, the Contractor must provide training opportunities aimed at developing a full journeyworker in the type of trade or job classification involved. During the course of performance of the Contract, the Contractor may seek approval from the IDOT District EEO Officer to employ additional eligible TPG trainees. In the event the Contractor subcontracts a portion of the contracted work, it must determine how many, if any, of the TPGs will be trained by the subcontractor. Though a subcontractor may conduct training, the Contractor retains the responsibility for meeting all requirements imposed by this Special Provision. The Contractor must also include this Special Provision in any subcontract where payment for contracted work performed by a TPG trainee will be passed on to a subcontractor.

Training through the Program is intended to move TPGs toward journeyman status, which is the primary objective of this Special Provision. Accordingly, the Contractor must make every effort to enroll TPG trainees by recruitment through the Program participant educational institutions to the extent eligible TPGs are available within a reasonable geographic area of the project. The Contractor is responsible for demonstrating, through documentation, the recruitment efforts it has undertaken prior to the determination by IDOT whether the Contractor is in compliance with this Special Provision, and therefore, entitled to the Training Program Graduate reimbursement of \$15.00 per hour.

Notwithstanding the on-the-job training requirement of this TPG Special Provision, some minimal off-site training is permissible as long as the offsite training is an integral part of the work of the contract, and does not compromise or conflict with the required on-site training that is central to the purpose of the Program. No individual may be employed as a TPG trainee in any work classification in which he/she has previously successfully completed a training program leading to journeyman status in any trade, or in which he/she has worked at a journeyman level or higher.

VEHICLE AND EQUIPMENT WARNING LIGHTS (BDE)

Effective: November 1, 2021

Revised: November 1, 2022

Add the following paragraph after the first paragraph of Article 701.08 of the Standard Specifications:

“The Contractor shall equip all vehicles and equipment with high-intensity oscillating, rotating, or flashing, amber or amber-and-white, warning lights which are visible from all directions. In accordance with 625 ILCS 5/12-215, the lights may only be in operation while the vehicle or equipment is engaged in construction operations.”

WEEKLY DBE TRUCKING REPORTS (BDE)

Effective: June 2, 2012

Revised: January 2, 2025

The following applies to all Disadvantaged Business Enterprise (DBE) trucks on the project, whether they are utilized for DBE goal credit or not.

The Contractor shall notify the Engineer at least three days prior to DBE trucking activity.

The Contractor shall submit a weekly report of DBE trucks hired by the Contractor or subcontractors (i.e. not owned by the Contractor or subcontractors) to the Engineer on Department form "SBE 723" within ten business days following the reporting period. The reporting period shall be Sunday through Saturday for each week reportable trucking activities occur.

Any costs associated with providing weekly DBE trucking reports shall be considered as included in the contract unit prices bid for the various items of work involved and no additional compensation will be allowed.

WOOD SIGN SUPPORT (BDE)

Effective: November 1, 2023

Add the following to Article 730.02 of the Standard Specifications:

"(c) Preservative Treatment1007.12"

Revise the first paragraph of Article 730.03 of the Standard Specifications to read:

" **730.03 General.** Wood sign supports shall be treated. When the 4 x 6 in. (100 x 150 mm) posts are used, they shall be modified to satisfy the breakaway requirements by drilling 1 1/2 in. (38 mm) diameter holes centered at 4 and 18 in. (100 and 450 mm) above the groundline and perpendicular to the centerline of the roadway."

WORK ZONE TRAFFIC CONTROL DEVICES (BDE)

Effective: March 2, 2020

Revised: January 1, 2025

Add the following to Article 701.03 of the Standard Specifications:

"(q) Temporary Sign Supports1106.02"

Revise the third paragraph of Article 701.14 of the Standard Specifications to read:

"For temporary sign supports, the Contractor shall provide a FHWA eligibility letter for each device used on the contract. The letter shall provide information for the set-up and use of the device as well as a detailed drawing of the device. The signs shall be supported within 20 degrees of vertical. Weights used to stabilize signs shall be attached to the sign support per the manufacturer's specifications."

Revise the first paragraph of Article 701.15 of the Standard Specifications to read:

“ **701.15 Traffic Control Devices.** For devices that must meet crashworthiness standards, the Contractor shall provide a manufacturer’s self-certification or a FHWA eligibility letter for each Category 1 device and a FHWA eligibility letter for each Category 2 and Category 3 device used on the contract. The self-certification or letter shall provide information for the set-up and use of the device as well as a detailed drawing of the device.”

Revise the first six paragraphs of Article 1106.02 of the Standard Specifications to read:

“ **1106.02 Devices.** Work zone traffic control devices and combinations of devices shall meet crashworthiness standards for their respective categories. The categories are as follows.

Category 1 includes small, lightweight, channelizing and delineating devices that have been in common use for many years and are known to be crashworthy by crash testing of similar devices or years of demonstrable safe performance. These include cones, tubular markers, plastic drums, and delineators, with no attachments (e.g. lights). Category 1 devices shall be MASH compliant.

Category 2 includes devices that are not expected to produce significant vehicular velocity change but may otherwise be hazardous. These include vertical panels with lights, barricades, temporary sign supports, and Category 1 devices with attachments (e.g. drums with lights). Category 2 devices shall be MASH compliant.

Category 3 includes devices that are expected to cause significant velocity changes or other potentially harmful reactions to impacting vehicles. These include crash cushions (impact attenuators), truck mounted attenuators, and other devices not meeting the definitions of Category 1 or 2. Category 3 devices manufactured after December 31, 2019 shall be MASH compliant. Category 3 devices manufactured on or before December 31, 2019, and compliant with NCHRP 350, may be used on contracts let before December 31, 2029. Category 3 devices shall be crash tested for Test Level 3 or the test level specified.

Category 4 includes portable or trailer-mounted devices such as sign supports, speed feedback displays, arrow boards, changeable message signs, temporary traffic signals, and area lighting supports. It is preferable for Category 4 devices manufactured after December 31, 2019 to be MASH-16 compliant; however, there are currently no crash tested devices in this category, so it remains exempt from the NCHRP 350 or MASH compliance requirement.

For each type of device, when no more than one MASH compliant is available, an NCHRP 350 compliant device may be used, even if manufactured after December 31, 2019.”

Revise Articles 1106.02(g), 1106.02(k), and 1106.02(l) to read:

“(g) Truck Mounted/Trailer Mounted Attenuators. The attenuator shall be approved for use at Test Level 3. Test Level 2 may be used for normal posted speeds less than or equal to 45 mph.

(k) Temporary Water Filled Barrier. The water filled barrier shall be a lightweight plastic shell designed to accept water ballast and be on the Department’s qualified product list.

Shop drawings shall be furnished by the manufacturer and shall indicate the deflection of the barrier as determined by acceptance testing; the configuration of the barrier in that test; and the vehicle weight, velocity, and angle of impact of the deflection test. The Engineer shall be provided one copy of the shop drawings.

(l) Movable Traffic Barrier. The movable traffic barrier shall be on the Department’s qualified product list.

Shop drawings shall be furnished by the manufacturer and shall indicate the deflection of the barrier as determined by acceptance testing; the configuration of the barrier in that test; and the vehicle weight, velocity, and angle of impact of the deflection test. The Engineer shall be provided one copy of the shop drawings. The barrier shall be capable of being moved on and off the roadway on a daily basis.”

PROJECT LABOR AGREEMENT

Effective: May 18, 2007

Revised: April 1, 2025

Description. The Illinois Project Labor Agreements Act, 30 ILCS 571, states that the State of Illinois has a compelling interest in awarding public works contracts so as to ensure the highest standards of quality and efficiency at the lowest responsible cost. A project labor agreement (PLA) is a form of pre-hire collective bargaining agreement covering all terms and conditions of employment on a specific project that is intended to support this compelling interest. It has been determined by the Department that a PLA is appropriate for the project that is the subject of this contract. The PLA document, provided below, only applies to the construction site for this contract. It is the policy of the Department on this contract, and all construction projects, to allow all contractors and subcontractors to compete for contracts and subcontracts without regard to whether they are otherwise parties to collective bargaining agreements.

The Department reserves the right to rescind the PLA requirement from this project in the event FHWA disapproves of the inclusion of the PLA terms for this project. The contractor, by bidding, agrees that any rescission of the PLA requirement shall not constitute grounds for the withdrawal of its bid and further agrees to remove the PLA requirement from this contract upon notice from the Department should such be necessary at a later date.

Execution of Letter of Assent. A copy of the PLA applicable to this project is included as part of this special provision. As a condition of the award of the contract, the successful bidder and each of its subcontractors shall execute a "Contractor Letter of Assent", in the form attached to the PLA as Exhibit A. The successful bidder shall submit a Subcontractor's Contractor Letter of Assent to the Department prior to the subcontractor's performance of work on the project. Upon request, copies of the applicable collective bargaining agreements will be provided by the appropriate signatory labor organization at the pre-job conference.

Quarterly Reporting. Section 37 of the Illinois Project Labor Agreements Act requires the Department to submit quarterly reports regarding the number of minorities and females employed under PLAs. To assist in this reporting effort, the Contractor shall provide a quarterly workforce participation report for all minority and female employees working under the PLA of this contract. The data shall be reported on Construction Form BC 820, Project Labor Agreement (PLA) Workforce Participation Quarterly Reporting Form available on the Department's website <https://idot.illinois.gov/content/dam/soi/en/web/idot/documents/idot-forms/bc/bc-820.pdf>.

The report shall be submitted no later than the 15th of the month following the end of each quarter (i.e., April 15 for the January – March reporting period). The form shall be emailed to DOT.PLA.Reporting@illinois.gov or faxed to (217) 524-4922.

Any costs associated with complying with this provision shall be considered as included in the contract unit prices bid for the various items of work involved and no additional compensation will be allowed.

Illinois Department of Transportation
PROJECT LABOR AGREEMENT

This Project Labor Agreement ("PLA" or "Agreement") is entered into this _____ day of

_____, 2025, by and between the Illinois Department of Transportation ("IDOT" or "Department") in its proprietary capacity, and each relevant Illinois AFL-CIO Building Trades signatory hereto as determined by the Illinois AFL-CIO Statewide Project Labor Agreement Committee on behalf of each of its affiliated members (individually and collectively, the "Unions"). This PLA shall apply to Construction Work (as defined herein) to be performed by IDOT's Prime Contractor and each of its subcontractors of whatever tier ("Subcontractor" or "Subcontractors") on Contract No. (hereinafter, the "Project").

ARTICLE I - INTENT AND PURPOSES

- 1.1 This PLA is entered into in accordance with the Project Labor Agreement Act ("Act", 30 ILCS 571). It is mutually understood and agreed that the terms and conditions of this PLA are intended to promote the public interest in obtaining timely and economical completion of the Project by encouraging productive and efficient construction operations; by establishing a spirit of harmony and cooperation among the parties; and by providing for peaceful and prompt settlement of any and all labor grievances or jurisdictional disputes of any kind without strikes, lockouts, slowdowns, delays, or other disruptions to the prosecution of the work. The parties acknowledge the obligations of the Contractors and Subcontractors to comply with the provisions of the Act. The parties will work with the Contractors and Subcontractors within the parameters of other statutory and regulatory requirements to implement the Act's goals and objectives.
- 1.2 As a condition of the award of the contract for performance of work on the Project, IDOT's Prime Contractor and each of its Subcontractors shall execute a "Contractor Letter of Assent", in the form attached hereto as Exhibit A, prior to commencing Construction Work on the Project. The Contractor shall submit a Subcontractor's Contractor Letter of Assent to the Department prior to the Subcontractor's performance of Construction Work on the Project. Upon request copies of the applicable collective bargaining agreements will be provided by the appropriate signatory labor organization consistent with this Agreement and at the pre-job conference referenced in Article III, Section 3.1.

- 1.3 Each Union affiliate and separate local representing workers engaged in Construction Work on the Project in accordance with this PLA are bound to this agreement by the Illinois AFL-CIO Statewide Project Labor Agreement Committee which is the central committee established with full authority to negotiate and sign PLAs with the State on behalf of all respective crafts. Upon their signing the Contractor Letter of Assent, the Prime Contractor, each Subcontractor, and the individual Unions shall thereafter be deemed a party to this PLA. No party signatory to this PLA shall, contract or subcontract, nor permit any other person, firm, company, or entity to contract or subcontract for the performance of Construction Work for the Project to any person, firm, company, or entity that does not agree in writing to become bound for the term of this Project by the terms of this PLA prior to commencing such work and to the applicable area-wide collective bargaining agreement(s) with the Union(s) signatory hereto.
- 1.4 It is understood that the Prime Contractor(s) and each Subcontractor will be considered and accepted by the Unions as separate employers for the purposes of collective bargaining, and it is further agreed that the employees working under this PLA shall constitute a bargaining unit separate and distinct from all others. The parties hereto also agree that this PLA shall be applicable solely with respect to this Project, and shall have no bearing on the interpretation of any other collective bargaining agreement or as to the recognition of any bargaining unit other than for the specific purposes of this Project.
- 1.5 In the event of a variance or conflict, whether explicit or implicit, between the terms and conditions of this PLA and the provisions of any other applicable national, area, or local collective bargaining agreement, the terms and conditions of this PLA shall supersede and control. For any work performed under the NTL Articles of Agreement, the National Stack/Chimney Agreement, the National Cooling Tower Agreement, the National Agreement of the International Union of Elevator Constructors, and for any instrument calibration work and loop checking performed under the UA/IBEW Joint National Agreement for Instrument and Control Systems Technicians, the preceding sentence shall apply only with respect to Articles I, II, V, VI, and VII.

- 1.6 Subject to the provisions of paragraph 1.5 of this Article, it is the parties' intent to respect the provisions of any other collective bargaining agreements that may now or hereafter pertain, whether between the Prime Contractor and one or more of the Unions or between a Subcontractor and one or more of the Unions. Accordingly, except and to the extent of any contrary provision set forth in this PLA, the Prime Contractor and each of its Subcontractors agrees to be bound and abide by the terms of the following in order of precedence: (a) the applicable collective bargaining agreement between the Prime Contractor and one or more of the Unions made signatory hereto; (b) the applicable collective bargaining agreement between a Subcontractor and one or more of the Unions made signatory hereto; or (c) the current applicable area collective bargaining agreement for the relevant Union that is the agreement certified by the Illinois Department of Labor for purposes of establishing the Prevailing Wage applicable to the Project. The Union will provide copies of the applicable collective bargaining agreements pursuant to part (c) of the preceding sentence to the Prime Contractor. Assignments by the Contractors or Subcontractors amongst the trades shall be consistent with area practices; in the event of unresolved disagreements as to the propriety of such assignments, the provisions of Article VI shall apply.
- 1.7 Subject to the limitations of paragraphs 1.4 to 1.6 of this Article, the terms of each applicable collective bargaining agreement as determined in accordance with paragraph 1.6 are incorporated herein by reference, and the terms of this PLA shall be deemed incorporated into such other applicable collective bargaining agreements only for purposes of their application to the Project.
- 1.8 To the extent necessary to comply with the requirements of any fringe benefit fund to which the Prime Contractor or Subcontractor is required to contribute under the terms of an applicable collective bargaining agreement pursuant to the preceding paragraph, the Prime Contractor or Subcontractor shall execute all "Participation Agreements" as may be reasonably required by the Union to accomplish such purpose; provided, however, that such Participation Agreements shall, when applicable to the Prime Contractor or Subcontractor solely as a result of this PLA, be amended as reasonably necessary to reflect such fact. Upon written notice in the form of a lien of a Contractor's or Subcontractor's delinquency from any applicable fringe benefit fund, IDOT will withhold from the Contractor's periodic pay request an amount sufficient to extinguish any delinquency obligation of the Contractor or Subcontractor arising out of the Project.
- 1.9 In the event that the applicable collective bargaining agreement between a Prime Contractor and the Union or between the Subcontractor and the Union expires prior to the completion of this Project, the expired applicable contract's terms will be maintained until a new applicable collective bargaining agreement is ratified. The wages and fringe benefits included in any new applicable collective bargaining agreement will apply on and after the effective date of the newly negotiated collective bargaining agreement, except to the extent wage and fringe benefit retroactivity is specifically agreed upon by the relevant bargaining parties.

ARTICLE II – APPLICABILITY, RECOGNITION, AND COMMITMENTS

- 2.1 The term Construction Work as used herein shall include all “construction, demolition, rehabilitation, renovation, or repair” work performed by a “laborer or mechanic” at the “site of the work” for the purpose of “building” the specific structures and improvements that constitute the Project. Terms appearing within quotation marks in the preceding sentence shall have the meaning ascribed to them pursuant to 29 CFR Part 5 and Illinois labor laws.
- 2.2 By executing the Letters of Assent, Prime Contractor and each of its Subcontractors recognizes the Unions signatory to this PLA as the sole and exclusive bargaining representatives for their craft employees employed on the jobsite for this Project. Unions who are signatory to this PLA will have recognition on the Project for their craft.
- 2.3 The Prime Contractor and each of its Subcontractors retains and shall be permitted to exercise full and exclusive authority and responsibility for the management of its operations, except as expressly limited by the terms of this PLA or by the terms and conditions of the applicable collective bargaining agreement.
- 2.4 Except to the extent contrary to an express provision of the relevant collective bargaining agreement, equipment or materials used in the Project may be pre-assembled or pre-fabricated, and there shall be no refusal by the Union to handle, transport, install, or connect such equipment or materials. Equipment or materials delivered to the job-site will be unloaded and handled promptly without regard to potential jurisdictional disputes; any such disputes shall be handled in accordance with the provisions of this PLA.
- 2.5 The parties are mutually committed to promoting a safe working environment for all personnel at the job-site. It shall be the responsibility of each employer to which this PLA applies to provide and maintain safe working conditions for its employees, and to comply with all applicable federal, state, and local health and safety laws and regulations.
- 2.6 The use or furnishing of alcohol or drugs and the conduct of any other illegal activity at the job-site is strictly prohibited. The parties shall take every practical measure consistent with the terms of applicable collective bargaining agreements to ensure that the job-site is free of alcohol and drugs.
- 2.7 All parties to this PLA agree that they will not discriminate against any employee based on race, creed, religion, color, national origin, union activity, age, gender or sexual orientation and shall comply with all applicable federal, state, and local laws.

- 2.8 In accordance with the Act and to promote diversity in employment, IDOT will establish, in cooperation with the other parties, the apprenticeship hours which are to be performed by minorities and females on the Project. IDOT shall consider the total hours to be performed by these underrepresented groups, as a percentage of the workforce, and create aspirational goals for each Project, based on the level of underutilization for the service area of the Project (together "Project Employment Objectives"). IDOT shall provide a quarterly report regarding the racial and gender composition of the workforce on the Project.

Persons currently lacking qualifications to enter apprenticeship programs will have the opportunity to obtain skills through basic training programs as have been established by the Department. The parties will endeavor to support such training programs to allow participants to obtain the requisite qualifications for the Project Employment Objectives.

The parties agree that all Contractors and Subcontractors working on the Project shall be encouraged to utilize the maximum number of apprentices as permitted under the terms of the applicable collective bargaining agreements to realize the Project Employment Objectives.

The Unions shall assist the Contractor and each Subcontractor in efforts to satisfy Project Employment Objectives. A Contractor or Subcontractor may request from a Union specific categories of workers necessary to satisfy Project Employment Objectives. The application of this section shall be consistent with all local Union collective bargaining agreements, and the hiring hall rules and regulations established for the hiring of personnel, as well as the apprenticeship standards set forth by each individual Union.

- 2.9 The parties hereto agree that engineering consultants and materials testing employees, to the extent subject to the terms of this PLA, shall be fully expected to objectively and responsibly perform their duties and obligations owed to the Department without regard to the potential union affiliation of such employees or of other employees on the Project.
- 2.10 This Agreement shall not apply to IDOT employees or employees of any other governmental entity.

ARTICLE III - ADMINISTRATION OF AGREEMENT

- 3.1 In order to assure that all parties have a clear understanding of the PLA, and to promote harmony, at the request of the Unions a post-award pre-job conference will be held among the Prime Contractor, all Subcontractors and Union representatives prior to the start of any Construction Work on the Project. No later than the conclusion of such pre-job conference, the parties shall, among other matters, provide to one another contact information for their respective representatives (including name, address, phone number, facsimile number, e-mail). Nothing herein shall be construed to limit the right of the Department to discuss or explain the purpose and intent of this PLA with prospective bidders or other interested parties prior to or following its award of the job.
- 3.2 Representatives of the Prime Contractor and the Unions shall meet as often as reasonably necessary following award until completion of the Project to assure the effective implementation of this PLA.
- 3.3 Any notice contemplated under Article VI and VII of this Agreement to a signatory labor organization shall be made in writing to the Local Union with copies to the local union's International Representative.

ARTICLE IV - HOURS OF WORK AND GENERAL CONDITIONS

- 4.1 The standard work day and work week for Construction Work on the Project shall be consistent with the respective collective bargaining agreements. In the event Project site or other job conditions dictate a change in the established starting time and/or a staggered lunch period for portions of the Project or for specific crafts, the Prime Contractor, relevant Subcontractors and business managers of the specific crafts involved shall confer and mutually agree to such changes as appropriate. If proposed work schedule changes cannot be mutually agreed upon between the parties, the hours fixed at the time of the pre-job meeting shall prevail.
- 4.2 Shift work may be established and directed by the Prime Contractor or relevant Subcontractor as reasonably necessary or appropriate to fulfill the terms of its contract with the Department. If used, shift hours, rates and conditions shall be as provided in the applicable collective bargaining agreement.
- 4.3 The parties agree that chronic and/or unexcused absenteeism is undesirable and must be controlled in accordance with procedures established by the applicable collective bargaining agreement. Any employee disciplined for absenteeism in accordance with such procedures shall be suspended from all work on the Project for not less than the maximum period permitted under the applicable collective bargaining agreement.

- 4.4 Except as may be otherwise expressly provided by the applicable collective bargaining agreement, employment begins and ends at the Project site; employees shall be at their place of work at the starting time; and employees shall remain at their place of work until quitting time.
- 4.5 Except as may be otherwise expressly provided by the applicable collective bargaining agreement, there shall be no limit on production by workmen, no restrictions on the full use of tools or equipment, and no restrictions on efficient use of manpower or techniques of construction other than as may be required by safety regulations.
- 4.6 The parties recognize that specialized or unusual equipment may be installed on the Project. In such cases, the Union recognizes the right of the Prime Contractor or Subcontractor to involve the equipment supplier or vendor's personnel in supervising the setting up of the equipment, making modifications and final alignment, and performing similar activities that may be reasonably necessary prior to and during the start-up procedure in order to protect factory warranties. The Prime Contractor or Subcontractor shall notify the Union representatives in advance of any work at the job-site by such vendor personnel in order to promote a harmonious relationship between the equipment vendor's personnel and other Project employees.
- 4.7 For the purpose of promoting full and effective implementation of this PLA, authorized Union representatives shall have access to the Project job-site during scheduled work hours. Such access shall be conditioned upon adherence to all reasonable visitor and security rules of general applicability that may be established for the Project site at the pre-job conference or from time to time thereafter.

ARTICLE V – GRIEVANCE PROCEDURES FOR DISPUTES ARISING UNDER A PARTICULAR COLLECTIVE BARGAINING AGREEMENT

- 5.1 In the event a dispute arises under a particular collective bargaining agreement specifically not including jurisdictional disputes referenced in Article VI below, said dispute shall be resolved by the Grievance/Arbitration procedure of the applicable collective bargaining agreement. The resulting determination from this process shall be final and binding on all parties bound to its process.
- 5.2 Employers covered under this Agreement shall have the right to discharge or discipline any employee who violates the provisions of this Agreement. Such discharge or discipline by a contractor or subcontractor shall be subject to Grievance/Arbitration procedure of the applicable collective bargaining agreement only as to the fact of such violation of this agreement. If such fact is established, the penalty imposed shall not be disturbed. Work at the Project site shall continue without disruption or hindrance of any kind as a result of a Grievance/Arbitration procedure under this Article.

- 5.3 In the event there is a deadlock in the foregoing procedure, the parties agree that the matter shall be submitted to arbitration for the selection and decision of an Arbitrator governed under paragraph 6.8.

ARTICLE VI –DISPUTES: GENERAL PRINCIPLES

- 6.1 This Agreement is entered into to prevent strikes, lost time, lockouts and to facilitate the peaceful adjustment of jurisdictional disputes in the building and construction industry and to prevent waste and unnecessary avoidable delays and expense, and for the further purpose of at all times securing for the employer sufficient skilled workers.

- 6.2 A panel of Permanent Arbitrators are attached as addendum (A) to this agreement. By mutual agreement between IDOT and the Unions, the parties can open this section of the agreement as needed to make changes to the list of permanent arbitrators.

The arbitrator is not authorized to award back pay or any other damages for a miss assignment of work. Nor may any party bring an independent action for back pay or any other damages, based upon a decision of an arbitrator.

- 6.3 The PLA Jurisdictional Dispute Resolution Process ("Process") sets forth the procedures below to resolve jurisdictional disputes between and among Contractors, Subcontractors, and Unions engaged in the building and construction industry. Further, the Process will be followed for any grievance or dispute arising out of the interpretation or application of this PLA by the parties except for the prohibition on attorneys contained in 6.11. All decisions made through the Process are final and binding upon all parties.

DISPUTE PROCESS

- 6.4 Administrative functions under the Process shall be performed through the offices of the President and/or Secretary-Treasurer of the Illinois State Federation of Labor, or their designated representative, called the Administrator. In no event shall any officer, employee, agent, attorney, or other representative of the Illinois Federation of Labor, AFL- CIO be subject to any subpoena to appear or testify at any jurisdictional dispute hearing.
- 6.5 There shall be no abandonment of work during any case participating in this Process or in violation of the arbitration decision. All parties to this Process release the Illinois State Federation of Labor ("Federation") from any liability arising from its action or inaction and covenant not to sue the Federation, nor its officers, employees, agents or attorneys.

- 6.6 In the event of a dispute relating to trade or work jurisdiction, all parties, including the employers, Contractors or Subcontractors, agree that a final and binding resolution of the dispute shall be resolved as follows:
- (a) Representatives of the affected trades and the Contractor or Subcontractor shall meet on the job site within two (2) business days after receiving written notice in an effort to resolve the dispute. (In the event there is a dispute between local unions affiliated with the same International Union, the decision of the General President, or his/her designee, as the internal jurisdictional authority of that International Union, shall constitute a final and binding decision and determination as to the jurisdiction of work.)
 - (b) If no settlement is achieved subsequent to the preceding Paragraph, the matter shall be referred to the local area Building & Construction Trades Council, which shall meet with the affected trades within two (2) business days subsequent to receiving written notice. In the event the parties do not wish to avail themselves of the local Building & Construction Trades Council, the parties may elect to invoke the services of their respective International Representatives with no extension of the time limitations. An agreement reached at this Step shall be final and binding upon all parties.
 - (c) If no settlement agreement is reached during the proceedings contemplated by Paragraphs "a" or "b" above, the matter shall be immediately referred to the Illinois Jurisdictional Dispute Process for final and binding resolution of said dispute. Said referral submission shall be in writing and served upon the Illinois State Federation of Labor, or the Administrator, pursuant to paragraph 6.4 of this agreement. The Administrator shall, within three (3) days, provide for the selection of an available Arbitrator to hear said dispute within this time period. Upon good cause shown and determined by the Administrator, an additional three (3) day extension for said hearing shall be granted at the sole discretion of the Administrator. Only upon mutual agreement of all parties may the Administrator extend the hearing for a period in excess of the time frames contemplated under this Paragraph. Business days are defined as Monday through Friday, excluding contract holidays.
- 6.7 The primary concern of the Process shall be the adjustment of jurisdictional disputes arising out of the Project. A sufficient number of Arbitrators shall be selected from list of approved Arbitrators as referenced Sec. 6.2 and shall be assigned per Sec. 6.8. Decisions shall be only for the Project and shall become effective immediately upon issuance and complied with by all parties. The authority of the Arbitrator shall be restricted and limited specifically to the terms and provisions of Article VI and generally to this Agreement as a whole.

- 6.8 Arbitrator chosen shall be randomly selected based on the list of Arbitrators in Sec. 6.2 and geographical location of the jurisdictional dispute and upon his/her availability, and ability to conduct a Hearing within two (2) business days of said notice. The Arbitrator may issue a "bench" decision immediately following the Hearing or he/she may elect to only issue a written decision, said decision must be issued within two (2) business days subsequent to the completion of the Hearing. Copies of all notices, pleadings, supporting memoranda, decisions, etc. shall be provided to all disputing parties and the Illinois State Federation of Labor.

Any written decision shall be in accordance with this Process and shall be final and binding upon all parties to the dispute and may be a "short form" decision. Fees and costs of the arbitrator shall be divided evenly between the contesting parties except that any party wishing a full opinion and decision beyond the short form decision shall bear the reasonable fees and costs of such full opinion. The decision of the Arbitrator shall be final and binding upon the parties hereto, their members, and affiliates.

In cases of jurisdictional disputes or other disputes between a signatory labor organization and another labor organization, both of which is an affiliate or member of the same International Union, the matter or dispute shall be settled in the manner set forth by their International Constitution and/or as determined by the International Union's General President whose decision shall be final and binding upon all parties. In no event shall there be an abandonment of work.

- 6.9 In rendering a decision, the Arbitrator shall determine:
- (a) First, whether a previous agreement of record or applicable agreement, including a disclaimer agreement, between National or International Unions to the dispute or agreements between local unions involved in the dispute, governs;
 - (b) Only if the Arbitrator finds that the dispute is not covered by an appropriate or applicable agreement of record or agreement between the crafts to the dispute, he shall then consider the established trade practice in the industry and prevailing practice in the locality. Where there is a previous decision of record governing the case, the Arbitrator shall give equal weight to such decision of record, unless the prevailing practice in the locality in the past ten years favors one craft. In that case, the Arbitrator shall base his decision on the prevailing practice in the locality. Except, that if the Arbitrator finds that a craft has improperly obtained the prevailing practice in the locality through raiding, the undercutting of wages or by the use of vertical agreements, the Arbitrator shall rely on the decision of record and established trade practice in the industry rather than the prevailing practice in the locality; and,

- (c) Only if none of the above criteria is found to exist, the Arbitrator shall then consider that because efficiency, cost or continuity and good management are essential to the well being of the industry, the interests of the consumer or the past practices of the employer shall not be ignored.
 - (d) The arbitrator is not authorized to award back pay or any other damages for a mis-assignment of work. Nor may any party bring an independent action for back pay or any other damages, based upon a decision of an arbitrator.
- 6.10 The Arbitrator shall set forth the basis for his/her decision and shall explain his/her findings regarding the applicability of the above criteria. If lower ranked criteria are relied upon, the Arbitrator shall explain why the higher-ranked criteria were not deemed applicable. The Arbitrator's decision shall only apply to the Project. Agreements of Record, for other PLA projects, are applicable only to those parties signatory to such agreements. Decisions of Record are those that were either attested to by the former Impartial Jurisdictional Disputes Board or adopted by the National Arbitration Panel.
- 6.11 All interested parties, as determined by the Arbitrator, shall be entitled to make presentations to the Arbitrator. Any interested labor organization affiliated to the PLA Committee and party present at the Hearing, whether making a presentation or not, by such presence shall be deemed to accept the jurisdiction of the Arbitrator and to agree to be bound by its decision. In addition to the representative of the local labor organization, a representative of the labor organization's International Union may appear on behalf of the parties. Each party is responsible for arranging for its witnesses. In the event an Arbitrator's subpoena is required, the party requiring said subpoena shall prepare the subpoena for the Arbitrator to execute. Service of the subpoena upon any witness shall be the responsibility of the issuing party.

Attorneys shall not be permitted to attend or participate in any portion of a Hearing.

The parties are encouraged to determine, prior to Hearing, documentary evidence which may be presented to the Arbitrator on a joint basis.

- 6.12 The Order of Presentation in all Hearings before an Arbitrator shall be
- I. Identification and Stipulation of the Parties
 - II. Unions(s) claiming the disputed work presents its case
 - III. Union(s) assigned the disputed work presents its case
 - IV. Employer assigning the disputed work presents its case
 - V. Evidence from other interested parties (i.e., general contractor, project manager, owner)
 - VI. Rebuttal by union(s) claiming the disputed work
 - VII. Additional submissions permitted and requested by Arbitrator
 - VIII. Closing arguments by the parties

- 6.13 All parties bound to the provisions of this Process hereby release the Illinois State Federation of Labor and IDOT, their respective officers, agents, employees or designated representatives, specifically including any Arbitrator participating in said Process, from any and all liability or claim, of whatsoever nature, and specifically incorporating the protections provided in the Illinois Arbitration Act, as amended from time to time.
- 6.14 The Process, as an arbitration panel, nor its Administrator, shall have any authority to undertake any action to enforce its decision(s). Rather, it shall be the responsibility of the prevailing party to seek appropriate enforcement of a decision, including findings, orders or awards of the Arbitrator or Administrator determining non-compliance with a prior award or decision.
- 6.15 If at any time there is a question as to the jurisdiction of the Illinois Jurisdictional Dispute Resolution Process, the primary responsibility for any determination of the arbitrability of a dispute and the jurisdiction of the Arbitrator shall be borne by the party requesting the Arbitrator to hear the underlying jurisdictional dispute. The affected party or parties may proceed before the Arbitrator even in the absence or one or more stipulated parties with the issue of jurisdiction as an additional item to be decided by the Arbitrator. The Administrator may participate in proceedings seeking a declaration or determination that the underlying dispute is subject to the jurisdiction and process of the Illinois Jurisdictional Dispute Resolution Process. In any such proceedings, the non-prevailing party and/or the party challenging the jurisdiction of the Illinois Jurisdictional Dispute Resolution Process shall bear all the costs, expenses and attorneys' fees incurred by the Illinois Jurisdictional Dispute Resolution Process and/or its Administrator in establishing its jurisdiction.

ARTICLE VII - WORK STOPPAGES AND LOCKOUTS

- 7.1 During the term of this PLA, no Union or any of its members, officers, stewards, employees, agents or representatives shall instigate, support, sanction, maintain, or participate in any strike, picketing, walkout, work stoppage, slow down or other activity that interferes with the routine and timely prosecution of work at the Project site or at any other contractor's or supplier's facility that is necessary to performance of work at the Project site. Hand billing at the Project site during the designated lunch period and before commencement or following conclusion of the established standard workday shall not, in itself, be deemed an activity that interferes with the routine and timely prosecution of work on the Project.

7.2 Should any activity prohibited by paragraph 7.1 of this Article occur, the Union shall undertake all steps reasonably necessary to promptly end such prohibited activities.

7.2.A No Union complying with its obligations under this Article shall be liable for acts of employees for which it has no responsibility or for the unauthorized acts of employees it represents. Any employee who participates or encourages any activity prohibited by paragraph 7.1 shall be immediately suspended from all work on the Project for a period equal to the greater of (a) 60 days; or (b) the maximum disciplinary period allowed under the applicable collective bargaining agreement for engaging in comparable unauthorized or prohibited activity.

7.2.B Neither the PLA Committee nor its affiliates shall be liable for acts of employees for which it has no responsibility. The principal officer or officers of the PLA Committee will immediately instruct, order and use the best efforts of his office to cause the affiliated union or unions to cease any violations of this Article. The PLA Committee in its compliance with this obligation shall not be liable for acts of its affiliates. The principal officer or officers of any involved affiliate will immediately instruct, order or use the best effort of his office to cause the employees the union represents to cease any violations of this Article. A union complying with this obligation shall not be liable for unauthorized acts of employees it represents. The failure of the Contractor to exercise its rights in any instance shall not be deemed a waiver of its rights in any other instance.

During the term of this PLA, the Prime Contractor and its Subcontractors shall not engage in any lockout at the Project site of employees covered by this Agreement.

7.3 Upon notification of violations of this Article, the principal officer or officers of the local area Building and Construction Trades Council, and the Illinois AFL-CIO Statewide Project Labor Agreement Committee as appropriate, will immediately instruct, order and use their best efforts to cause the affiliated union or unions to cease any violations of this Article. A Trades Council and the Committee otherwise in compliance with the obligations under this paragraph shall not be liable for unauthorized acts of its affiliates.

7.4 In the event that activities in violation of this Article are not immediately halted through the efforts of the parties, any aggrieved party may invoke the special arbitration provisions set forth in paragraph 7.5 of this Article.

- 7.5 Upon written notice to the other involved parties by the most expeditious means available, any aggrieved party may institute the following special arbitration procedure when a breach of this Article is alleged:
- 7.5.A The party invoking this procedure shall notify the individual designated as the Permanent Arbitrator pursuant to paragraph 6.8 of the nature of the alleged violation; such notice shall be by the most expeditious means possible. The initiating party may also furnish such additional factual information as may be reasonably necessary for the Permanent Arbitrator to understand the relevant circumstances. Copies of any written materials provided to the arbitrator shall also be contemporaneously provided by the most expeditious means possible to the party alleged to be in violation and to all other involved parties.
- 7.5.B Upon receipt of said notice the Permanent Arbitrator shall set and hold a hearing within twenty-four (24) hours if it is contended the violation is ongoing, but not before twenty-four (24) hours after the written notice to all parties involved as required above.
- 7.5.C The Permanent Arbitrator shall notify the parties by facsimile or any other effective written means, of the place and time chosen by the Permanent Arbitrator for this hearing. Said hearing shall be completed in one session. A failure of any party or parties to attend said hearing shall not delay the hearing of evidence or issuance of an Award by the Permanent Arbitrator.
- 7.5.D The sole issue at the hearing shall be whether a violation of this Article has, in fact, occurred. An Award shall be issued in writing within three (3) hours after the close of the hearing, and may be issued without a written opinion. If any party desires a written opinion, one shall be issued within fifteen (15) days, but its issuance shall not delay compliance with, or enforcement of, the Award. The Permanent Arbitrator may order cessation of the violation of this Article, and such Award shall be served on all parties by hand or registered mail upon issuance.
- 7.5.E Such Award may be enforced by any court of competent jurisdiction upon the filing of the Award and such other relevant documents as may be required. Facsimile or other hardcopy written notice of the filing of such enforcement proceedings shall be given to the other relevant parties. In a proceeding to obtain a temporary order enforcing the Permanent Arbitrator's Award as issued under this Article, all parties waive the right to a hearing and agree that such proceedings may be ex parte. Such agreement does not waive any party's right to participate in a hearing for a final order of enforcement. The Court's order or orders enforcing the Permanent Arbitrator's Award shall be served on all parties by hand or by delivery to their last known address or by registered mail.

- 7.6 Individuals found to have violated the provisions of this Article are subject to immediate termination. In addition, IDOT reserves the right to terminate this PLA as to any party found to have violated the provisions of this Article.
- 7.7 Any rights created by statute or law governing arbitration proceedings inconsistent with the above procedure or which interfere with compliance therewith are hereby waived by parties to whom they accrue.
- 7.8 The fees and expenses of the Permanent Arbitrator shall be borne by the party or parties found in violation, or in the event no violation is found, such fees and expenses shall be borne by the moving party.

ARTICLE VIII – TERMS OF AGREEMENT

- 8.1 If any Article or provision of this Agreement shall be declared invalid, inoperative or unenforceable by operation of law or by any of the above mentioned tribunals of competent jurisdiction, the remainder of this Agreement or the application of such Article or provision to persons or circumstances other than those as to which it has been held invalid, inoperative or unenforceable shall not be affected thereby.
- 8.2 This Agreement shall be in full force as of and from the date of the Notice of Award until the Project contract is closed.
- 8.3 This PLA may not be changed or modified except by the subsequent written agreement of the parties. All parties represent that they have the full legal authority to enter into this PLA. This PLA may be executed by the parties in one or more counterparts.
- 8.4 Any liability arising out of this PLA shall be several and not joint. IDOT shall not be liable to any person or other party for any violation of this PLA by any other party, and no Contractor or Union shall be liable for any violation of this PLA by any other Contractor or Union.
- 8.5 The failure or refusal of a party to exercise its rights hereunder in one or more instances shall not be deemed a waiver of any such rights in respect of a separate instance of the same or similar nature.

[The Balance of This Page Intentionally Left Blank]

Addendum A

IDOT Slate of Permanent Arbitrators

1. Bruce Feldacker
2. Thomas F. Gibbons
3. Edward J. Harrick
4. Brent L. Motchan
5. Robert Perkovich
6. Byron Yaffee
7. Glenn A. Zipp

Exhibit A - Contractor Letter of Assent

(Date)

To All Parties:

In accordance with the terms and conditions of the contract for Construction Work on [Contract No.], this Letter of Assent hereby confirms that the undersigned Prime Contractor or Subcontractor agrees to be bound by the terms and conditions of the Project Labor Agreement established and entered into by the Illinois Department of Transportation in connection with said Project.

It is the understanding and intent of the undersigned party that this Project Labor Agreement shall pertain only to the identified Project. In the event it is necessary for the undersigned party to become signatory to a collective bargaining agreement to which it is not otherwise a party in order that it may lawfully make certain required contributions to applicable fringe benefit funds, the undersigned party hereby expressly conditions its acceptance of and limits its participation in such collective bargaining agreement to its work on the Project.

(Authorized Company Officer)

(Company)

ASBESTOS ABATEMENT SURVEY



Illinois Department of Transportation

Asbestos Abatement Survey

Work Order No.: 39

Job Order No.: R-94-001-19

Route: FAP 669

Parcel No.: 4ABL011

600 South Main Street

East Peoria, Tazewell County, IL 61611

April 23, 2025

Asbestos Abatement Survey

Asbestos Abatement Survey

Work Order No.: 39
Job Order No.: R-94-001-19
Route: FAP 669
Parcel No.: 4ABL011
600 South Main Street
East Peoria, Tazewell County, IL 61611

April 23, 2025

Prepared By:

Arcadis U.S., Inc.
225 West Wacker Drive
Suite 2015
Chicago, Illinois 60606
Phone: 312 917 1000

Prepared For:

Illinois Department of Transportation
2300 South Dirksen Parkway
Springfield, IL 62764

Our Ref:

30272888



Paul Hoffman
Industrial Hygiene Specialist



Katherine Challberg
Senior Environmental Engineer / QA/QC



Brendan J. Quealy
Principal Environmental Engineer/Certified Project Manager

This document is intended only for the use of the individual or entity for which it was prepared and may contain information that is privileged, confidential and exempt from disclosure under applicable law. Any dissemination, distribution or copying of this document is strictly prohibited.

www.arcadis.com

Asbestos Abatement Survey

Contents

Acronyms and Abbreviations..... ii

1 Introduction 1

2 Survey Methodology 1

3 Analytical Methods 2

4 Findings 2

5 Recommendations..... 2

6 Cost Estimate..... 3

7 Additional Survey Limitations..... 4

Tables

Table 1 Asbestos Homogeneous Areas Summary

Figures

Figures 1-3 Asbestos Bulk Sampling Locations

Appendices

Appendix A Limitations and Service Constraints

Appendix B Accreditations - Inspector, Laboratory

Appendix C Laboratory Reports – Asbestos

Appendix D Photograph Log

Asbestos Abatement Survey

Acronyms and Abbreviations

| | |
|---------|--|
| ACM | asbestos-containing material |
| AHERA | Asbestos Hazard Emergency Response Act |
| Arcadis | Arcadis U.S., Inc. |
| ASTM | ASTM International |
| CFR | Code of Federal Regulations |
| COC | chain of custody |
| DS | dispersion staining |
| HA | homogeneous area |
| IAC | Illinois Administrative Code |
| IDPH | Illinois Department of Public Health |
| IDOT | Illinois Department of Transportation |
| IEPA | Illinois Environmental Protection Agency |
| NESHAP | National Emissions Standards for Hazardous Air Pollutant |
| NIOSH | National Institute for Occupational Safety and Health |
| NVLAP | National Voluntary Laboratory Accreditation Program |
| OSHA | Occupational Safety and Health Administration |
| PLM | polarized light microscopy |
| PPE | personal protective equipment |
| QA/QC | quality control/quality assurance |
| STAT | STAT Analysis Corporation of Chicago, Illinois |
| USEPA | United States Environmental Protection Agency |

Asbestos Abatement Survey

1 Introduction

Arcadis U.S., Inc. (Arcadis) conducted an asbestos abatement survey (survey) at Parcel No. 4ABL011 located at 600 South Main Street, East Peoria, Tazewell County, Illinois (Site) in accordance with Work Order No. 39 under the Prime Agreement for Consulting Services for PTB No. 202-043 between Illinois Department of Transportation (IDOT) and Arcadis. Arcadis' services and report are subject to the Limitations and Service Constraints provided in **Appendix A**.

The Site is improved with a commercial garage/shop building with a partial upper level, constructed in 1945, and covering approximately 2,880 square feet (SF).

The objective of the survey was to ascertain the general presence, quantity, and location of asbestos-containing materials (ACMs) at the site that may be impacted by the planned demolition and provide preliminary cost estimates for asbestos abatement that may be required prior to demolition activities.

The survey activities were conducted on March 11, 2025, by Paul Hoffman of Arcadis U.S., Inc. and Patrick Hook of A3 Environmental LLC. (A3E), an Arcadis sub-consultant. Copies of their accreditations are provided in **Appendix B**.

2 Survey Methodology

Arcadis conducted the asbestos survey in compliance with the United States Environmental Protection Agency (USEPA) National Emissions Standards for Hazardous Air Pollutants (NESHAPs), applicable State of Illinois and local asbestos regulations. This was accomplished by conducting a visual inspection of the structures to be impacted by the planned demolition and collecting samples of suspect ACM based on these observations.

Arcadis conducted the asbestos survey in general accordance with ASTM International (ASTM) E2356 Standard Practice for Comprehensive Building Asbestos Surveys. ASTM E2356 meets the applicable asbestos survey and/or sampling requirements of current USEPA NESHAP Standard [40 Code of Federal Regulations (CFR) 61, Subpart M (Asbestos)], USEPA Asbestos Hazard Emergency Response Act (AHERA) [40 CFR 763, Subpart E], and Occupational Safety and Health Administration (OSHA) Asbestos Construction Standard [29 CFR 1926.1101].

The survey included an inspection of the structures to be impacted by the planned demolition providing a general sense of the overall location, type, quantity, and condition of suspect ACMs present. The asbestos survey included a visual and physical assessment of each accessible space to locate suspect ACMs. Suspect materials were divided into "Homogeneous Areas" (HAs) (i.e., materials that were determined by the inspector to be homogeneous based on their color, texture, and assumed date of installation).

A representative number of samples were collected from each HA. Bulk material samples were collected in 4-milliliter plastic bags and tightly sealed for transport to STAT Analysis Corporation (STAT) in Chicago, Illinois. Each sample collected by Arcadis was assigned its own unique coded number and sent for laboratory analysis under a chain-of-custody (COC) protocol.

The survey included destructive, intrusive, and/or exploratory testing unless specifically prohibited by IDOT. Destructive sampling is performed in order to identify materials which are hidden or obstructed. Hidden or obstructed areas include but are not limited to wall cavities, pipe chases, spaces above fixed ceilings, materials located under carpeting or subfloors, and ceramic tile grout/adhesive.

Asbestos Abatement Survey

3 Analytical Methods

Asbestos bulk samples were analyzed for asbestos (reported in percent of type of asbestos) using polarized light microscopy with dispersion staining (PLM/DS) in accordance with the USEPA Method 600/R-93/116. Percentage estimates of each material's composition are based on the analyst's best visual judgment following PLM/DS analysis, and examination with a stereoscope.

STAT is accredited under the National Voluntary Laboratory Accreditation Program (NVLAP). A copy of their accreditation is provided in **Appendix B**. Laboratory analytical reports are provided in **Appendix C**.

4 Findings

Arcadis identified 12 HAs from which 36 samples were collected and analyzed. A listing of the identified HAs, including each HA material description, location, sample number, condition, asbestos content, and estimated quantity, are presented in Table 1. Approximate sampling locations are shown on Figures 1-3.

The USEPA defines ACM as a material containing greater than 1% asbestos. Materials containing less than 1% asbestos are not regulated by the USEPA or the State of Illinois, but their disturbance is regulated by OSHA.

The bulk samples collected by Arcadis as part of the survey were reported by the laboratory to be ND = Asbestos Not Detected (Not Present).

No HAs were identified as assumed ACM.

Representative photographs are provided in **Appendix D**.

5 Recommendations

ACMs identified at the site that may be disturbed during demolition activities, must be removed by a licensed asbestos abatement contractor utilizing industry standard work procedures in accordance with all federal, state, and local regulations governing asbestos.

If other suspect materials not referenced in this survey report are identified during demolition activities, we recommend these materials be assumed ACM until the materials are inspected by a licensed asbestos inspector, sampled, and submitted for laboratory analysis.

Asbestos Abatement Survey

6 Cost Estimate

Arcadis prepared this rough order of magnitude (ROM) opinion of cost to support planned asbestos abatement activities at the Site prior to building demolition.

| ROUGH ORDER OF MAGNITUDE OPINION OF ASBESTOS ABATEMENT COSTS | | |
|--|----------------|---------------------------|
| ASBESTOS ABATEMENT COSTS | | |
| Item | Estimated Cost | Estimated Duration (Days) |
| Removal/disposal of identified ACM | N/A | N/A |
| Removal/disposal of assumed ACM | N/A | N/A |
| Sub-Total Estimated Abatement Costs | N/A | N/A |
| Contingency (15%) | N/A | N/A |
| Total Estimated Abatement Costs | N/A | N/A |
| ASBESTOS OVERSIGHT COSTS | | |
| Item | Estimated Cost | Estimated Duration (Days) |
| Project Management/Coordination/Reporting | N/A | N/A |
| ACM Abatement Oversight (including equipment and analysis) | N/A | N/A |
| Sub-Total Estimated Oversight Costs | N/A | N/A |
| Contingency (15%) | N/A | N/A |
| Total Estimated Oversight Costs | N/A | N/A |
| TOTAL ESTIMATED PROJECT COST (WITH 15% CONTINGENCY) | N/A | N/A |

The above ROM opinion of cost is based on the following assumptions:

- Cost estimate is based on the information identified in Arcadis' survey report.
- Cost estimate includes permits, labor, equipment, waste transport and disposal.
- Cost estimate assumes work will be awarded in its entirety to one contractor and completed during a single mobilization.
- Cost estimate includes consulting fees such as project management, project oversight, air sampling, etc.
- Actual bids and schedule durations may vary.
- Oversight costs are dependent upon the project duration which may vary depending on the contractor's schedule and is outside of Arcadis' control.

Asbestos Abatement Survey

7 Additional Survey Limitations

Arcadis' survey is subject to the following survey limitations.

- The investigation did not include access or inspection of confined spaces, underground piping, conduits, and building footings, if any.
- If other suspect materials, not referenced in this survey report, are identified during renovation or demolition activities, Arcadis recommends that these materials be considered regulated until they are inspected by an appropriately certified inspector and proven otherwise.
- Utilities/services, including electric, water, and heat, were active in most areas surveyed. Materials associated with electrical components and energized equipment were not safely accessible and were not sampled.

Table

FAP Routes 669 & 673 (IL 116/IL 8 & US 24)
 Project NHPP-7VBW(880)
 Section 13[(HVB,HB-3)]BR-1;R
 Tazewell County
 Contract No. 68C55

Table 1. Asbestos Homogeneous Areas Summary
 IDOT - Asbestos Abatement Survey
 W0 # 39
 600 South Main Street, East Peoria, Illinois



| HA No. | Material Description | Material Location | Sample Number | Condition | Friability | Asbestos Content | Estimated Quantity | Unit | Notes |
|--------|--|----------------------------------|---------------|-----------|-------------|------------------|--------------------|------|---------------|
| 1 | Brown Laminare Flooring | 1st Floor Office, Front Entrance | 1, 2, 3 | Good | Non-Friable | NAD | 320 | SF | |
| 2 | Gray 12" x 12" Vinyl Floor Tile | 1st Floor Office, Front Entrance | 4, 5, 6 | Good | Non-Friable | NAD | 320 | SF | Beneath H.A.1 |
| 2M | Yellow Plastic associated with Gray 12" x 12" Floor Tile | 1st Floor Office, Front Entrance | 4M, 5M, 6M | Good | Non-Friable | NAD | 320 | SF | |
| 3 | White/Tan Gypsum Wall board | 1st Floor Office, Front Entrance | 7, 8, 9 | Good | Friable | NAD | 320 | SF | |
| 4 | White Joint Compound | 1st Floor Office, Front Entrance | 10, 11, 12 | Good | Friable | NAD | 320 | SF | |
| 5 | White/Tan 2' x 4' Ceiling Tile | 1st Floor Office, Front Entrance | 13, 14, 15 | Good | Friable | NAD | 320 | SF | |
| 6 | Black 12" x 12" Ceramic Tile | Garage Bathroom | 16, 17, 18 | Good | Non-Friable | NAD | 6 | SF | |
| 6G | Thinset associated with Black 12" x 12" Ceramic Floor Tile | Garage Bathroom | 16G, 17G, 18G | Good | Non-Friable | NAD | 6 | SF | |
| 7 | Gray Concrete | Garage/Basement | 19, 20, 21 | Good | Non-Friable | NAD | 1,900 | SF | |
| 8 | White Interior Window Caulk | Garage/Basement | 22, 23, 24 | Good | Non-Friable | NAD | 10 | LF | |
| 9 | White Exterior Window Caulk | Front Garage Window | 25, 26, 27 | Good | Non-Friable | NAD | 20 | LF | |
| 10 | Black Roof Casing Tile Sealant | Roof | 28, 29, 30 | Good | Non-Friable | NAD | 1,900 | SF | |
| 11 | Black Built Up Tar/Tar Paper Roof | Roof | 31, 32, 33 | Good | Non-Friable | NAD | 1,900 | SF | |
| 12 | Black Tar on Flashing | Basement | 34, 35, 36 | Good | Non-Friable | NAD | 100 | SF | |

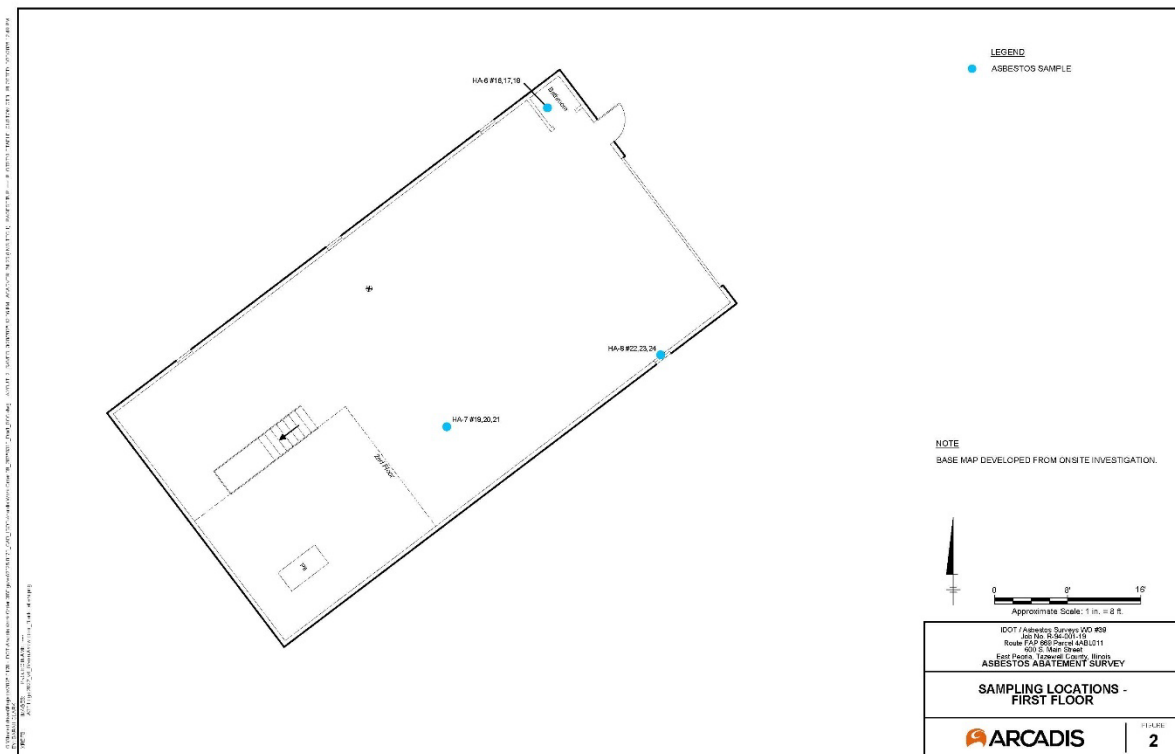
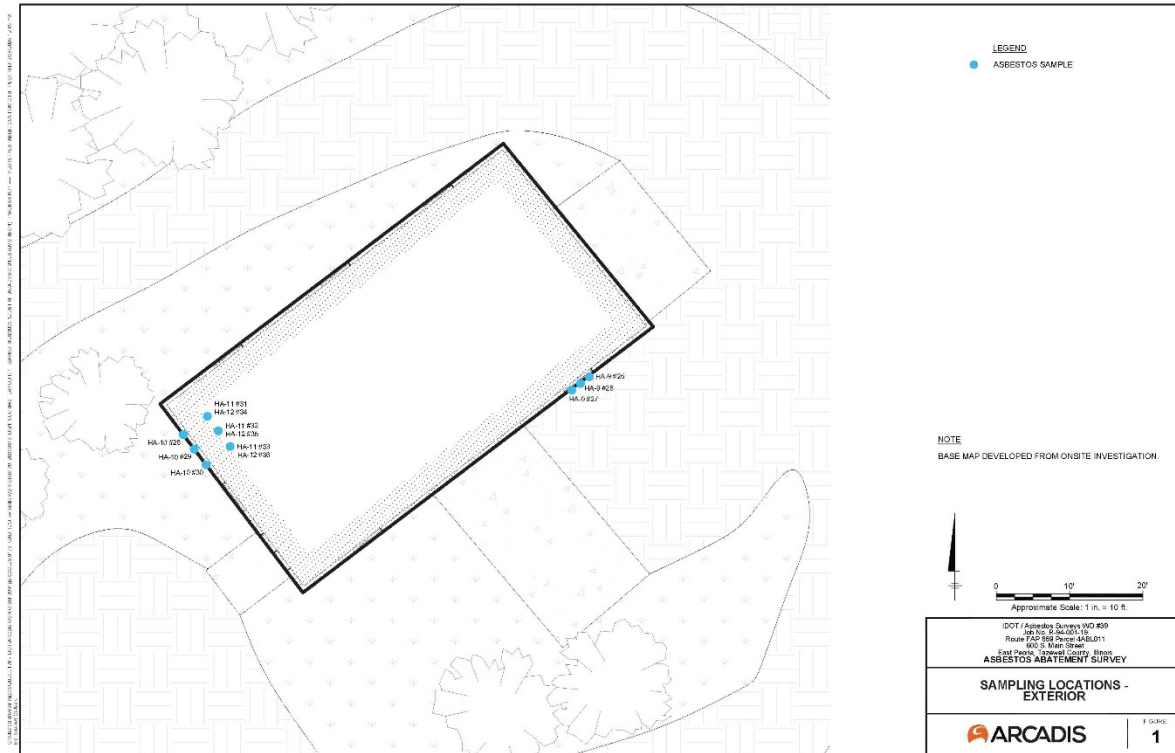
Notes:
 ACM = Asbestos Containing Material
 Assumed ACM = A material determined by the inspector to be suspected ACM and is considered asbestos-containing until laboratory analysis can prove otherwise.
 NAD = No Asbestos Detected
 EA = Each
 LF = Linear Foot
 SF = Square Foot

Table 1. Asbestos Homogeneous Areas Summary

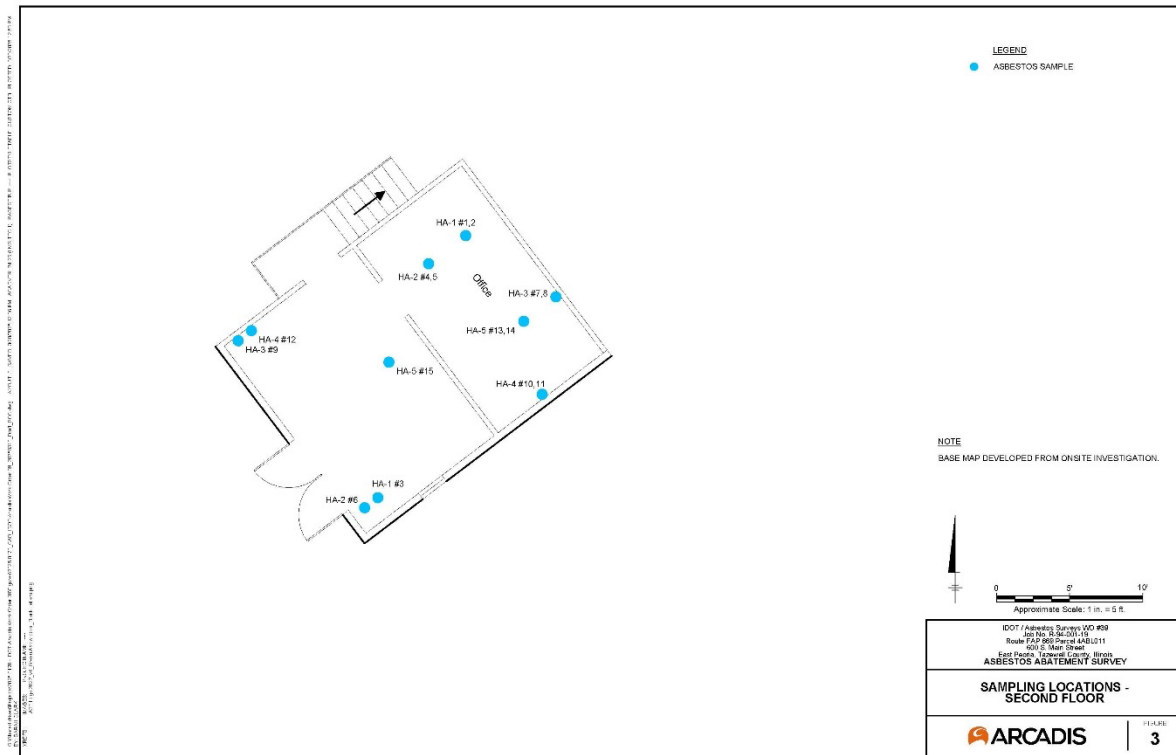
1/1

Figures

FAP Routes 669 & 673 (IL 116/IL 8 & US 24)
 Project NHPP-7VWB(880)
 Section 13[(HVB,HB-3)]BR-1;R
 Tazewell County
 Contract No. 68C55



FAP Routes 669 & 673 (IL 116/IL 8 & US 24)
 Project NHPP-7VVBW(880)
 Section 13[(HVB,HB-3)]BR-1;R
 Tazewell County
 Contract No. 68C55



Appendix A

Limitations and Service Constraints

Limitations and Service Constraints

The opinions, conclusions and recommendations presented in this report are limited to the information obtained during the performance of the specific scope of service identified in the report. To the extent that Arcadis relied upon any information prepared by other parties not under direct contract to Arcadis, no representation as to the accuracy or completeness of such information is made. This report is an instrument of professional service and the services described in the report were performed in accordance with generally accepted standards and level of skill and care ordinarily exercised by members of the profession working under similar conditions including comparable budgetary and schedule constraints. No warranty, guarantee or certification express or implied, is intended or given with respect to Arcadis' services, opinions, conclusions, or recommendations. This statement is in lieu of any other statement either expressed or implied.

Arcadis' observations, the results of testing and Arcadis' opinions, conclusions and recommendations apply solely to conditions existing at the specific times when and specific locations where Arcadis' investigative work was performed. Arcadis affirms that data gathered and presented by Arcadis in this report was collected in an appropriate manner in accordance with generally accepted methods and practices. Arcadis cannot be responsible for decisions made by our client solely on the basis of economic factors. Observation and testing activities such as those conducted by Arcadis are inherently limited and do not represent a conclusive or complete characterization. Arcadis analyzed only the substances, conditions and locations described in the report at the time indicated. Conditions in other parts of the project site, building or area may vary from conditions at the specific locations where observations were made and where testing was performed by Arcadis. Additionally, other building material hazards which were not identified by Arcadis, may also be present in inaccessible areas, including within walls and cavities, above ceilings, and below floors. Therefore, the extent of Arcadis' opinions, conclusions and recommendations are limited and 100% confidence in these opinions, conclusions and recommendations cannot reasonably be achieved. Nothing contained in this report shall relieve any other party of its responsibility to abide by contract documents and applicable laws, codes, regulations, or standards nor shall it be considered medical advice or consultation.

This report is expressly for the sole and exclusive use of the party for whom this report was originally prepared and for the particular purpose outlined in the report. Only the party for whom this report was originally prepared and/or other specifically named parties have the right to make use of and rely upon this report. Reuse of this report or any portion thereof for other than its intended purpose, or if modified, or if used by third parties, shall be at the user's sole risk. This report must be presented in its entirety.

Appendix B

Accreditations

Inspector

Laboratory



525-535 West Jefferson Street • Springfield, Illinois 62761-0001 • www.dph.illinois.gov

PATRICK W HOOK
2025 WOODLANE DR
LINDENHURST, IL 60046

3/11/2024



ASBESTOS PROFESSIONAL LICENSE ID NUMBER: 10967

Enclosed is your Asbestos Professional License. Please note the expiration date on the card and in the image depicted below.

COPY OF THE ASBESTOS PROFESSIONAL LICENSE

Front of License

Back of License

| ASBESTOS PROFESSIONAL LICENSE | | | ENDORSEMENTS | TC EXPIRES |
|--|-----------|------------|---|------------|
|  | | | INSPECTOR | 1/12/2025 |
| ID NUMBER | ISSUED | EXPIRES | PROJECT MANAGER | 1/13/2025 |
| 100 - 10967 | 3/11/2024 | 05/15/2025 | | |
| PATRICK W HOOK 2025 WOODLANE DR LINDENHURST, IL 60046 Environmental Health | | |  | |
| Alteration of this license shall result in legal action This license issued under authority of the State of Illinois Department of Public Health This license is valid only when accompanied by a valid training course certificate. | | | | |

If you have any questions or need further assistance, contact the Asbestos Program at (217)782-3517 or fax (217)785-5897.

Our WEB address is: dph.illinois.gov/topics-services/environmental-health-protection/asbestos
EMAIL Address: dph.asbestos@illinois.gov

PROTECTING HEALTH, IMPROVING LIVES
Nationally Accredited by PHAB



This is to certify that

Paul J. Hoffman

11 Precourt Lane, Norton, MA 02766

MA DLS Asbestos Inspector License# AI061473

*has completed requisite training by Video Conference, and has passed an examination for
reaccreditation as:*

Asbestos Inspector Refresher

pursuant to Title II of the Toxic Substance Control Act, 15 U.S.C. 2646

Course Location

Zoom Video Conference

Institute for Environmental Education 16 Upton Drive Wilmington, MA 01887

June 21, 2024

Course Dates

24-5332-106-213698

Certificate Number

June 21, 2024

Examination Date

June 21, 2025

Expiration Date

Training Director

16 Upton Drive, Wilmington, MA 01887

Telephone 978.658.5272

www.ieetrains.com

INSTITUTE FOR ENVIRONMENTAL EDUCATION

United States Department of Commerce
National Institute of Standards and Technology



Certificate of Accreditation to ISO/IEC 17025:2017

NVLAP LAB CODE: 101202-0

STAT Analysis Corporation
Chicago, IL

*is accredited by the National Voluntary Laboratory Accreditation Program for specific services,
listed on the Scope of Accreditation, for:*

Asbestos Fiber Analysis

*This laboratory is accredited in accordance with the recognized International Standard ISO/IEC 17025:2017.
This accreditation demonstrates technical competence for a defined scope and the operation of a laboratory quality
management system (refer to joint ISO-ILAC-IAF Communique on ISO/IEC 17025).*

2024-07-01 through 2025-06-30
Effective Dates




For the National Voluntary Laboratory Accreditation Program

**National Voluntary
Laboratory Accreditation Program**



SCOPE OF ACCREDITATION TO ISO/IEC 17025:2017

STAT Analysis Corporation

Sterling Labs
2242 W. Harrison St. Suite 200
Chicago, IL 60612
Carolyn Mazzuca
Phone: 312-733-0551
Email: cmazzuca@statanalysis.com

ASBESTOS FIBER ANALYSIS

NVLAP LAB CODE 101202-0

Bulk Asbestos Analysis

Code

Description

18/A01

EPA – 40 CFR Appendix E to Subpart E of Part 763, Interim Method of the Determination of Asbestos in Bulk Insulation Samples

18/A03

EPA 600/R-93/116: Method for the Determination of Asbestos in Bulk Building Materials

Airborne Asbestos Analysis

Code

Description

18/A02

U.S. EPA's "Interim Transmission Electron Microscopy Analytical Methods-Mandatory and Nonmandatory-and Mandatory Section to Determine Completion of Response Actions" as found in 40 CFR, Part 763, Subpart E, Appendix A.

A handwritten signature in blue ink, appearing to read "Dana S. Laman", is written over a horizontal line.

For the National Voluntary Laboratory Accreditation Program

Effective 2024-07-01 through 2025-06-30

Page 1 of 1

Appendix C

Laboratory Reports

STAT Analysis Corporation
2242 West Harrison St., Suite 200, Chicago, IL 60612-3766
Tel: (312) 733-0551 Fax: (312) 733-2386 STATinfo@STATAnalysis.com



NVLAP Lab Code 101202-0

ASBESTOS ANALYSIS BY POLARIZED LIGHT MICROSCOPY

Method: EPA/600/R-93/116

Arcadis US Inc.
225 W. Wacker Drive, Suite 2015
Chicago, IL 60606
Phone: (312) 575-3718

Reference: 30272888 IDOT
Location: 600 S Main St., Peoria, IL
Batch No.: 374118
Customer No.: 2530
Date Received: 03/13/2025
Date Analyzed: 03/20/2025
Date Reported: 03/20/2025
Turn Around Time: 5 Days

| Laboratory Sample | Customer Sample Number | Asbestos Components (%) | Non-Asbestos Components (%) |
|-------------------|------------------------|-------------------------|-----------------------------------|
| 374118001 | 1 | ND | Cellulose 1-5% Binder 95-99% |
| 374118002 | 2 | ND | Cellulose 1-5% Binder 95-99% |
| 374118003 | 3 | ND | Cellulose 1-5% Binder 95-99% |
| 374118004 | 4 | ND | Cellulose 1-5% Binder 95-99% |
| 374118005 | 5 | ND | Cellulose 1-5% Binder 95-99% |
| 374118006 | 6 | ND | Cellulose 1-5% Binder 95-99% |
| 374118007 | 4M | ND | Cellulose 1-5% Binder 95-99% |
| 374118008 | 5M | ND | Cellulose 1-5% Binder 95-99% |
| 374118009 | 6M | ND | Cellulose 1-5% Binder 95-99% |
| 374118010 | 7 | ND | Cellulose 10-15% Binder 85-90% |

ND = Asbestos Not Detected (Not Present) NA = Not Analyzed NS = Not Submitted

Components of inhomogeneous samples are analyzed per our Standard Operating Procedure, or per customer request.

The use of the NVLAP logo does not imply endorsement by NVLAP or any agency of the US Government.

The information contained in this report and any attachments is confidential information intended only for the use of the individual or entities named above. The results of this report relate only to the samples tested. If you have received this report in error, please notify us immediately by phone. This report shall not be reproduced, except in its entirety, unless written approval has been obtained from the laboratory. This report remains property of STAT Analysis until payment is received in full (see invoice).

Analyzed by Name :

Page 1 of 5

Daniel Mikos / Microscopist

Date: 03/20/2025

STAT Analysis Corporation
2242 West Harrison St., Suite 200, Chicago, IL 60612-3766
Tel: (312) 733-0551 Fax: (312) 733-2386 STATinfo@STATAnalysis.com



NVLAP Lab Code 101202-0

ASBESTOS ANALYSIS BY POLARIZED LIGHT MICROSCOPY

Method: EPA/600/R-93/116

Arcadis US Inc.
225 W. Wacker Drive, Suite 2015
Chicago, IL 60606
Phone: (312) 575-3718

Reference: 30272888 IDOT
Location: 600 S Main St., Peoria, IL
Batch No.: 374118
Customer No.: 2530
Date Received: 03/13/2025
Date Analyzed: 03/20/2025
Date Reported: 03/20/2025
Turn Around Time: 5 Days

| Laboratory Sample | Customer Sample Number | Asbestos Components (%) | Non-Asbestos Components (%) |
|-------------------|------------------------|-------------------------|-----------------------------------|
| 374118011 | 8 | ND | Cellulose 10-15% Binder 85-90% |
| 374118012 | 9 | ND | Cellulose 10-15% Binder 85-90% |
| 374118013 | 10 | ND | Cellulose 1-5% Binder 95-99% |
| 374118014 | 11 | ND | Cellulose 1-5% Binder 95-99% |
| 374118015 | 12 | ND | Cellulose 1-5% Binder 95-99% |
| 374118016 | 13 | ND | Cellulose 35-40% Binder 60-65% |
| 374118017 | 14 | ND | Cellulose 35-40% Binder 60-65% |
| 374118018 | 15 | ND | Cellulose 35-40% Binder 60-65% |
| 374118019 | 16 | ND | Cellulose 1-5% Binder 95-99% |
| 374118020 | 17 | ND | Cellulose 1-5% Binder 95-99% |

ND = Asbestos Not Detected (Not Present) NA = Not Analyzed NS = Not Submitted

Components of inhomogeneous samples are analyzed per our Standard Operating Procedure, or per customer request.

The use of the NVLAP logo does not imply endorsement by NVLAP or any agency of the US Government.

The information contained in this report and any attachments is confidential information intended only for the use of the individual or entities named above. The results of this report relate only to the samples tested. If you have received this report in error, please notify us immediately by phone. This report shall not be reproduced, except in its entirety, unless written approval has been obtained from the laboratory. This report remains property of STAT Analysis until payment is received in full (see invoice).

Analyzed by Name :

Page 2 of 5

Daniel Mikos / Microscopist

Date: 03/20/2025

STAT Analysis Corporation
2242 West Harrison St., Suite 200, Chicago, IL 60612-3766
Tel: (312) 733-0551 Fax: (312) 733-2386 STATinfo@STATAnalysis.com



NVLAP Lab Code 101202-0

ASBESTOS ANALYSIS BY POLARIZED LIGHT MICROSCOPY

Method: EPA/600/R-93/116

Arcadis US Inc.
225 W. Wacker Drive, Suite 2015
Chicago, IL 60606
Phone: (312) 575-3718

Reference: 30272888 IDOT
Location: 600 S Main St., Peoria, IL
Batch No.: 374118
Customer No.: 2530
Date Received: 03/13/2025
Date Analyzed: 03/20/2025
Date Reported: 03/20/2025
Turn Around Time: 5 Days

| Laboratory Sample | Customer Sample Number | Asbestos Components (%) | Non-Asbestos Components (%) |
|-------------------|------------------------|-------------------------|---------------------------------|
| 374118021 | 18 | ND | Cellulose 1-5% Binder 95-99% |
| 374118022 | 16G | ND | Cellulose 1-5% Binder 95-99% |
| 374118023 | 17G | ND | Cellulose 1-5% Binder 95-99% |
| 374118024 | 18G | ND | Cellulose 1-5% Binder 95-99% |
| 374118025 | 19 | ND | Cellulose 1-5% Binder 95-99% |
| 374118026 | 20 | ND | Cellulose 1-5% Binder 95-99% |
| 374118027 | 21 | ND | Cellulose 1-5% Binder 95-99% |
| 374118028 | 22 | ND | Cellulose 1-5% Binder 95-99% |
| 374118029 | 23 | ND | Cellulose 1-5% Binder 95-99% |
| 374118030 | 24 | ND | Cellulose 1-5% Binder 95-99% |

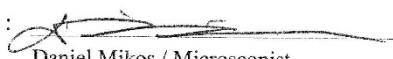
ND = Asbestos Not Detected (Not Present) NA = Not Analyzed NS = Not Submitted

Components of inhomogeneous samples are analyzed per our Standard Operating Procedure, or per customer request.

The use of the NVLAP logo does not imply endorsement by NVLAP or any agency of the US Government.

The information contained in this report and any attachments is confidential information intended only for the use of the individual or entities named above. The results of this report relate only to the samples tested. If you have received this report in error, please notify us immediately by phone. This report shall not be reproduced, except in its entirety, unless written approval has been obtained from the laboratory. This report remains property of STAT Analysis until payment is received in full (see invoice).

Analyzed by Name :


Daniel Mikos / Microscopist

Page 3 of 5

Date: 03/20/2025

STAT Analysis Corporation
2242 West Harrison St., Suite 200, Chicago, IL 60612-3766
Tel: (312) 733-0551 Fax: (312) 733-2386 STATinfo@STATAnalysis.com



NVLAP Lab Code 101202-0

ASBESTOS ANALYSIS BY POLARIZED LIGHT MICROSCOPY

Method: EPA/600/R-93/116

Arcadis US Inc.
225 W. Wacker Drive, Suite 2015
Chicago, IL 60606
Phone: (312) 575-3718

Reference: 30272888 IDOT
Location: 600 S Main St., Peoria, IL
Batch No.: 374118
Customer No.: 2530
Date Received: 03/13/2025
Date Analyzed: 03/20/2025
Date Reported: 03/20/2025
Turn Around Time: 5 Days

| Laboratory Sample | Customer Sample Number | Asbestos Components (%) | Non-Asbestos Components (%) |
|-------------------|------------------------|-------------------------|-----------------------------------|
| 374118031 | 25 | ND | Cellulose 1-5% Binder 95-99% |
| 374118032 | 26 | ND | Cellulose 1-5% Binder 95-99% |
| 374118033 | 27 | ND | Cellulose 1-5% Binder 95-99% |
| 374118034 | 28 | ND | Cellulose 10-15% Binder 85-90% |
| 374118035 | 29 | ND | Cellulose 10-15% Binder 85-90% |
| 374118036 | 30 | ND | Cellulose 10-15% Binder 85-90% |
| 374118037 | 31 | ND | Binder 85-90% Glass 10-15% |
| 374118038 | 32 | ND | Binder 85-90% Glass 10-15% |
| 374118039 | 33 | ND | Binder 85-90% Glass 10-15% |
| 374118040 | 34 | ND | Cellulose 10-15% Binder 85-90% |

ND = Asbestos Not Detected (Not Present) NA = Not Analyzed NS = Not Submitted

Components of inhomogeneous samples are analyzed per our Standard Operating Procedure, or per customer request.

The use of the NVLAP logo does not imply endorsement by NVLAP or any agency of the US Government.

The information contained in this report and any attachments is confidential information intended only for the use of the individual or entities named above. The results of this report relate only to the samples tested. If you have received this report in error, please notify us immediately by phone. This report shall not be reproduced, except in its entirety, unless written approval has been obtained from the laboratory. This report remains property of STAT Analysis until payment is received in full (see invoice).

Analyzed by Name :

Page 4 of 5

Daniel Mikos / Microscopist

Date: 03/20/2025

STAT Analysis Corporation
2242 West Harrison St., Suite 200, Chicago, IL 60612-3766
Tel: (312) 733-0551 Fax: (312) 733-2386 STATinfo@STATAnalysis.com



NVLAP Lab Code 101202-0

ASBESTOS ANALYSIS BY POLARIZED LIGHT MICROSCOPY

Method: EPA/600/R-93/116

Arcadis US Inc.
225 W. Wacker Drive, Suite 2015
Chicago, IL 60606
Phone: (312) 575-3718

Reference: 30272888 IDOT
Location: 600 S Main St., Peoria, IL
Batch No.: 374118
Customer No.: 2530
Date Received: 03/13/2025
Date Analyzed: 03/20/2025
Date Reported: 03/20/2025
Turn Around Time: 5 Days

| Laboratory Sample | Customer Sample Number | Asbestos Components (%) | Non-Asbestos Components (%) |
|-------------------|------------------------|-------------------------|-----------------------------------|
| 374118041 | 35 | ND | Cellulose 10-15% Binder 85-90% |
| 374118042 | 36 | ND | Cellulose 10-15% Binder 85-90% |


ND = Asbestos Not Detected (Not Present) NA = Not Analyzed NS = Not Submitted

Components of inhomogeneous samples are analyzed per our Standard Operating Procedure, or per customer request.

The use of the NVLAP logo does not imply endorsement by NVLAP or any agency of the US Government.

The information contained in this report and any attachments is confidential information intended only for the use of the individual or entities named above. The results of this report relate only to the samples tested. If you have received this report in error, please notify us immediately by phone. This report shall not be reproduced, except in its entirety, unless written approval has been obtained from the laboratory. This report remains property of STAT Analysis until payment is received in full (see invoice).

Analyzed by Name :


Daniel Mikos / Microscopist

Page 5 of 5

Date: 03/20/2025



sterling labs

CHAIN OF CUSTODY RECORD

Page: 1 of 4

280

Client:

IDOT

Building/Site No.:

E PEORIA
600 S MAIN ST
E. PEORIA, IL

Project No.:

30272888
Survey Date: 3/11/2025
Inspector: PATRICK HOOK374118
ARCADISfor natural and
built assets

| HA | Sample Number | Material Type (Circle) | Color Texture | Material Description | Floor: B, I, Mezz, Ext, Roof | Sample Location | Condition G, D, SD | Friable Y/N | Quantity SF, LF, EA | Photo |
|---|---------------|---|---------------|----------------------|------------------------------|--------------------------------------|--------------------|-------------|---------------------|-------|
| 1 | 1 | (M/S, TSI) | BROWN | BROWN LAMINATE | 1 | OFFICE - 1 ST FLR | D | N | 320 SF | |
| | 2 | Location: PLR | | FLOATING | 1 | ↓ | ↓ | ↓ | | |
| | 3 | Ceiling, Wall, Floor, Roof, Multiple, Shaftway, Other | | FLR (NO MASTIC) | 1 | FRONT ENTRANCE - 1 ST FLR | ↓ | ↓ | ↓ | |
| Mat. Location/QTY: FLOOR IS OVER HA-2 12"X12" F.T / YELL MASTIC | | | | | | | | | | |
| Notes: | | | | | | | | | | |
| #2 | 4 | (M/S, TSI) | WHITE/GRAY | 12"X12" | 1 | OFFICE - 1 ST FLOOR | D | N | 320 SF | |
| | 5 | Location: PLR | | FLOOR TILE | 1 | ↓ | ↓ | ↓ | | |
| | 6 | Ceiling, Wall, Floor, Roof, Multiple, Shaftway, Other | | AND YELLOW MASTIC | 1 | FRONT ENTRANCE - 1 ST FLR | ↓ | ↓ | ↓ | |
| Mat. Location/QTY: | | | | | | | | | | |
| Notes: | | | | | | | | | | |
| 3 | 7 | (M/S, TSI) | WHITE | WHITE/TAN | 1 | OFFICE - 1 ST FLR | D | NY | 320 SF | |
| | 8 | Location: WALL | | GYPSUM | 1 | ↓ | ↓ | ↓ | | |
| | 9 | Ceiling, Wall, Floor, Roof, Multiple, Shaftway, Other | | WALL BOARD | 1 | FRONT ENTRANCE - 1 ST FLR | ↓ | ↓ | ↓ | |
| Mat. Location/QTY: | | | | | | | | | | |
| Notes: | | | | | | | | | | |
| 4 | 10 | (M/S, TSI) | WHITE | WHITE JOINT | 1 | OFFICE - 1 ST FLR | D | N | 320 SF | |
| | 11 | Location: WALL | | THROUGH | 1 | ↓ | ↓ | ↓ | | |
| | 12 | Ceiling, Wall, Floor, Roof, Multiple, Shaftway, Other | | COMPOUND ON HA-3 | 1 | FRONT ENTRANCE - 1 ST FLR | ↓ | ↓ | ↓ | |
| Mat. Location/QTY: | | | | | | | | | | |
| Notes: | | | | | | | | | | |

Client:
IDOT

Building/Site No.: E PEORIA
 Address: 600 S MAIN ST
 E PEORIA, IL

Project No.: 30272888
 Survey Date: 3/11/2025
 Inspector: PATRICK HOOK

374118
 ARCADIS

| HA | Sample Number | Material Type (Circle) | Color Texture | Material Description | Floor: B, I, Mezz, Ext, Roof | Sample Location | Condition G, D, SD | Friable Y / N | Quantity SF, LF, EA | Photo |
|--------------------|---------------|------------------------|---------------|-------------------------|------------------------------|--------------------------------------|--------------------|---------------|---------------------|-------|
| 5 | 13 | (M) S. TSI | WHITE | WHITE / | 1 | OFFICE - 1 ST FLOOR | G | Y | 320 | |
| | 14 | | CEIL | TAN 2'X4' | 1 | ↓ | ↓ | ↓ | | |
| | 15 | | CEILING | TILE | 1 | FRONT ENTRANCE - 1 ST FLR | N | ↓ | ↓ | |
| Mat. Location/QTy: | | | | | | | | | | |
| Notes: | | | | | | | | | | |
| 6 | 16 | M. S. TSI | WHITE+BLACK | 12"X12" | B | GARAGE - BATHROOM | P | N | 6 TILES | |
| | 17 | | | CERAMIC | B | ↓ | ↓ | ↓ | 6 SF | |
| | 18 | | | FILE AND | B | ↓ | ↓ | ↓ | ↓ | |
| Mat. Location/QTy: | | | | | | | | | | |
| Notes: | | | | | | | | | | |
| 7 | 19 | (M) S. TSI | GRAY | GRAY CONCRETE | B | GARAGE / BASEMENT | G | N | 1900 SF | |
| | 20 | | FLR/WALL | FLR / WALL | ↓ | ↓ | ↓ | ↓ | ↓ | |
| | 21 | | | | ↓ | ↓ | ↓ | ↓ | ↓ | |
| Mat. Location/QTy: | | | | | | | | | | |
| Notes: | | | | | | | | | | |
| 8 | 22 | (M) S. TSI | WHITE | WHITE INTERIOR | B | GARAGE / BASEMENT | D | Y | 10 LF | |
| | 23 | | WINDOW | WINDOW | ↓ | ↓ | ↓ | ↓ | ↓ | |
| | 24 | | | CAULKING ON METAL FRAME | ↓ | ↓ | ↓ | ↓ | ↓ | |
| Mat. Location/QTy: | | | | | | | | | | |
| Notes: | | | | | | | | | | |

Client:

IDOT

Building/Site No.:

E. PEORIA
Address: 600 S MAIN ST
E. PEORIA, IL

Project No.:

30271888
Survey Date: 3/11/2025
Inspector: PATRICK HOOK

ARCADIS

for natural and
built assets

| HA | Sample Number | Material Type (Circle) | Color Texture | Material Description | Floor: S, I, Mezz, Ext, Roof | Sample Location | Condition G, D, SD | Friable Y/N | Quantity SF, LF, EA | Photo |
|--------------------|---------------|---|---------------|----------------------|------------------------------|-----------------------|--------------------|-------------|---------------------|-------|
| 9 | 25 | M/S TSI | WHITE | EXTERIOR | EXT | FRONT GARAGE WINDOW | D | N | 20 LF | |
| | 26 | Location: WINDOW | | WINDOW CAULK | | | | | | |
| | 27 | Ceiling, Wall, Floor, Roof, Multiple, Shaftway, Other | | | | | | | | |
| Mat. Location/Qty: | | | | | | | | | | |
| Notes: | | | | | | | | | | |
| 10 | 28 | M/S TSI | BLACK | BLACK | ROOF | ROOF - SOUTH SIDE | D | N | 1,900 SF | |
| | 29 | Location: ROOF | | ROOF COPING | | | | | | |
| | 30 | Ceiling, Wall, Floor, Roof, Multiple, Shaftway, Other | | TILE SEALANT | | | | | | |
| Mat. Location/Qty: | | | | | | | | | | |
| Notes: | | | | | | | | | | |
| 11 | 31 | M/S TSI | BLACK | BLACK | ROOF | ROOF - SOUTH SIDE | D | N | 1,900 SF | |
| | 32 | Location: ROOF | | BUILT-UP | | | | | | |
| | 33 | Ceiling, Wall, Floor, Roof, Multiple, Shaftway, Other | | TAR/TAR PAPER ROOF | | | | | | |
| Mat. Location/Qty: | | | | | | | | | | |
| Notes: | | | | | | | | | | |
| 12 | 34 | M/S TSI | BLACK | BLACK | EXT | FRONT - EXTERIOR WALL | D | N | 100 SF | |
| | 35 | Location: WALL | | TARON FLASHING | | | | | | |
| | 36 | Ceiling, Wall, Floor, Roof, Multiple, Shaftway, Other | | | | | | | | |
| Mat. Location/Qty: | | | | | | | | | | |
| Notes: | | | | | | | | | | |

Appendix D

Photo Log

IDOT

Asbestos Abatement Survey
Work Order No. 39 – 600 South Main Street
East Peoria, IL 61611



View of Building – Front



View of Building – Rear



HA-1: Brown Laminate Flooring
HA-2: Gray 12" x 12" Vinyl Floor Tile and Yellow Mastic



HA-3: White/Tan Gypsum Board
HA-4: White Joint Compound



HA-5: White/Tan 2' x 4' Ceiling Tile



HA-6: Black 12" x 12" Ceramic Floor Tile and Thinset

IDOT

Asbestos Abatement Survey
Work Order No. 39 – 600 South Main Street
East Peoria, IL 61611



HA-7: Gray Concrete



HA-8: White Interior Window Caulk



HA-9: White Exterior Window Caulk



HA-10: Black Roof Coping Tile Sealant



HA-11: Black Built-Up Tar/Tar Paper Roof



HA-12: Black Tar on Flashing

FAP Routes 669 & 673 (IL 116/IL 8 & US 24)
Project NHPP-7VBW(880)
Section 13[(HVB,HB-3)]BR-1;R
Tazewell County
Contract No. 68C55

Arcadis U.S., Inc.
225 West Wacker Drive
Suite 2015
Chicago, Illinois 60606
Phone: 312 917 1000
www.arcadis.com

Arcadis. Improving quality of life.

STORM WATER POLLUTION PREVENTION PLAN (BDE 2342)



Storm Water Pollution Prevention Plan

| Route | Marked Route | Section Number |
|----------------|--------------|-----------------------|
| FAP 669 | IL 8/IL 116 | 13[(HVB,HB-3)] BR-1;R |
| Project Number | County | Contract Number |
| NHPP-7VBW(880) | Tazewell | 68C55 |

This plan has been prepared to comply with the provisions of the National Pollutant Discharge Elimination System (NPDES) Permit No. ILR10 (Permit ILR10), issued by the Illinois Environmental Protection Agency (IEPA) for storm water discharges from construction site activities.

I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gathered and evaluated the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.

Permittee Signature & Date

 05/09/25

SWPPP Notes

Preparing BDE 2342 (Storm Water Pollution Prevention Plan)

Guidance on preparing each section of BDE 2342 (Storm Water Pollution Prevention Plan) is found in Chapter 41 of the IDOT Bureau of Design and Environment (BDE) Manual, please consult this chapter during SWPPP preparation. Please note that the Illinois Environmental Protection Agency (IEPA) has 30 days to review the Notice of Intent (NOI) prior to project approval and any deficiencies can result in construction delays.

The Notice of Intent contains the following documents:

- BDE 2342 (Storm Water Pollution Prevention Plan)
- BDE 2342 A (Contractor Certification Statement)
- Erosion and Sediment Control Plan (See Section 63-4.09 of the BDE Manual)

Non-applicable information

If any section of the SWPPP is not applicable put "N/A" in box rather than leaving blank.

National Pollutant Discharge Elimination System (NPDES) Compliance

Description of Work: This work shall consist of those efforts necessary for compliance with the requirements of the Clean Water Act, Section 402 (NPDES), and the Illinois Environment Protection Act. This provision also provides the background information needed to comply with ILR10 and ILR40 permits for this project.

NPDES COMPLIANCE REQUIREMENTS

Part I: Site Description

1. Describe the project location; include latitude and longitude, section, town, and range.

The work on this project is located on Cedar Street (IL 8 / IL 116) and Main Street (IL 29 / US 24) in Tazewell County.

The project begins at a point on Cedar Street, approximately 0.3-miles west of the centerline of Main Street, and extends east over the TZPR Railroad and Main Street. The project extends south of Cedar Street along Main Street approximately 0.4-miles, and to the north along Main Street approximately 0.2-miles.

Lat: 40°39'50"N -- Long: 89°35'00"W -- S32, T26N, R4W

2. Describe the nature of the construction activity or demolition work.

New construction of SN 090-0182 (Cedar Street over TZPR), SN 090-0183 (Cedar Street over Main Street), SN 090-8517 (Retaining Wall A), SN 090-8520 (Retaining Wall D), and SN 090-8518 (Retaining Wall E).

Roadway improvements include resurfacing, reconstruction, and widening along Cedar Street (IL-8/IL-116), Main Street, Silver Street, Johnson Street, and Pekin Avenue.

Additional work to be performed under this contract shall include, but not be limited to removal of existing pavement, shoulder, sidewalk, curb and gutter, and other roadway appurtenances, earthwork and embankment, erosion control measures, drainage items, subbase improvements, hot-mix asphalt and concrete roadway paving, roadway lighting, sidewalk / driveway ADA curb ramps, pavement marking, signing and landscape restoration as well as all incidental and collateral work necessary to complete the project as shown on the plans and as described herein.

3. Describe the intended sequence of major activities which disturb soils for major portions of the site (e.g. clearing, grubbing, excavation, grading, on-site or off-site stockpiling of soils, on-site or off-site storage of materials).

The project will be completed in 6 Stages. The following stages involve soil disturbing activities: Stage 1 (Main Street) - Install temporary medians, relocate utilities, demolish houses and clear lots, install drainage and temporary pavement, and begin TZPR bridge work. Stage 2 (Main Street / Johnson / Pekin) - Begin pavement, and begin Wall D. Stage 3 (Main Street / Silver Street) - Continue pavement and bridges, and begin Walls A and E. Stage 4 (Main Street) - Continue pavement, bridges, and walls. Stage 5 (Cedar Street) - Install pavement. Stage 6 (Entire Project) - Final Cleaning. This project includes installation, maintenance and removal of erosion and sediment control measures, and will be permanently stabilized with sodding and Class 3 and 5A seeding.

4. The total area of the construction site is estimated to be 30 acres.

5. The total area of the site estimated to be disturbed by excavation, grading or other activities is 26 acres.

6. Determine an estimate of the runoff coefficient of the site after construction activities are completed.

Proposed C=0.43, Existing=0.41

7. Provide the existing information describing the potential erosivity of the soil at discharge locations at the project site.

Erosion factor (K) value of 0.28 is present at the steeper bluff areas of the site. Urban Land soils present throughout most of the project have assumed high erosivity potential.

8. Erosion and Sediment Control Plan (Graphic Plan) is included in the contract. ☒ Yes ☐ No

9. List all soils found within project boundaries; include map until name, slope information, and erosivity.

Based on USDA Natural Resources Conservation Service Web Soil Survey, the project area includes Urban land through most of the project. A small area of Disco Sandy Loams are present near the northwestern limits of Cedar Street, and the uphill area along the southeast side of Main Street is Senachine-Hennepin Loams (35% to 60% slopes; K=0.28).

The project area is predominantly paved or grassed areas in existing and proposed conditions. Steep slopes

along Ramp B are remaining undisturbed adjacent to Prospect Avenue / Hilltop Court.

Disturbed steeper roadway slopes along Cedar Street, Cedar Street Ramps, and Main Street are proposed to be stabilized with erosion control blanket. Steep slopes under proposed bridges are proposed to be covered with concrete slopewalls. Proposed storm sewer discharges are proposed to be protected with rip rap.

10. List of all MS4 permittees in the area of this project

Illinois Department of Transportation, City of East Peoria, and Tazewell County

Note: For sites discharging to an MS4, a separate map identifying the location of the construction site and the location where the MS4 discharges to surface water must be included.

Part II: Waters of the US

1. List the nearest named receiving water(s) and ultimate receiving waters.

The receiving water is an Unnamed Tributary to the Illinois River. Runoff is received by IDOT and City of East Peoria storm sewer systems, which discharge to the Unnamed Tributary. The Unnamed Tributary drains to a pump station within the East Peoria Levee System that discharges into the Illinois River.

2. Are wetlands present in the project area? ☐ Yes ☒ No

If yes, describe the areal extent of the wetland acreage at the site.

N/A

3. Natural buffers:

For any storm water discharges from construction activities within 50 feet of a Waters of the United States, except for activities for water-dependent structures authorized by a Section 404 permit, the following shall apply:

(i) A 50-foot undisturbed natural buffer between the construction activity and the Waters of the United States has been provided

☒ Yes ☐ No; and/or

(ii) Additional erosion and sediment controls within that area has been provided

☒ Yes ☐ No; and Describe: _____

Part III. Water Quality

1. Water Quality Standards

As determined by the Illinois Pollution Control Board, Illinois waters have defined numeric limits of pollutants under the umbrella term "Water Quality Standards." In the following table are commonly used chemicals/practices used on a construction site. These chemicals if spilled into a waterway, could potentially contribute to a violation of a Water Quality Standard. If other chemicals that could contribute a violation of a Water Quality Standard, add as needed.

☒ Fertilizer (check as appropriate)

☒ Nitrogen

☒ Phosphorus, and/or

☐ Potassium

☒ Herbicide

☒ Petroleum (gas, diesel, oil, kerosene, hydraulic oil / fluids)

☒ Waste water for concrete washout station

☒ Coal tar Pitch Emulsion

☐ Other (Specify) _____

☐ Other (Specify) _____

Table 1: Common chemicals/potential pollutants used during construction

If no boxes are checked in Table 1 above, check the following box:

☐ There are no chemicals on site that will exceed a Water Quality Standards if spilled.

If any boxes are checked in Table 1 above, check the following box:

FAP Routes 669 & 673 (IL 116/IL 8 & US 24)
 Project NHPP-7VBW(880)
 Section 13[(HVB,HB-3)]BR-1;R
 Tazewell County
 Contract No. 68C55

There are chemicals on site that if spilled could potentially cause an exceedance of a Water Quality Standard. The Department shall implement Pollution Prevention/Good Housekeeping Practices as described in the Department's ILR40 Discharge for Small
☒ Municipal Separate Storm Sewer Systems (MS4) reiterated below and Part VIII. Unexpected Regulated Substances/Chemical Spill Procedures:

Pollution Prevention:

The Department will design, and the contractor shall, install, implement, and maintain effective pollution prevention measures to minimize the discharge of pollutants from construction activities. At a minimum, such measures must be designed, installed, implemented and maintained to:

- (a) Minimize the discharge of pollutants from equipment and vehicle washing, wheel wash water, and other wash waters. Wash waters must be treated in a sediment basin or alternative control that provides equivalent or better treatment prior to discharge.
- (b) Minimize the exposure of building materials, building products, construction wastes, trash, landscape materials, fertilizers, pesticides, herbicides, chemical storage tanks, deicing material storage facilities and temporary stockpiles, detergents, sanitary waste, and other materials present on the site exposed to precipitation and to storm water.
- (c) Minimize the discharge of pollutants from spills, leaks and vehicle and equipment maintenance and repair activities and implement chemical spill and leak prevention and response procedures;
- (d) Minimize the exposure of fuel, oil, hydraulic fluids, other petroleum products, and other chemicals by storing in covered areas or containment areas. Any chemical container with a storage of 55 gallons or more must be stored a minimum of 50 feet from receiving waters, constructed or natural site drainage features, and storm drain inlets. If infeasible due to site constraints, store containers as far away as the site permits and document in your SWPPP the specific reasons why the 50-foot setback is infeasible and how the containers will be stored.
- (e) The contractor is to provide regular inspection of their construction activities and Best Management Practices (BMPs). Based on inspection findings, the contractor shall determine if repair, replacement, or maintenance measures are necessary in order to ensure the structural integrity, proper function, and treatment effectiveness of structural storm water BMPs. Necessary maintenance shall be completed as soon as conditions allow to prevent or reduce the discharge of pollutants to storm water or as ordered by the Engineer. The Engineer shall conduct inspections required in Section XI Inspections, and report to the contractor deficiencies noted. These Department conducted inspections do not relieve the contractor from their responsibility to inspect their operations and perform timely maintenance; and
- (f) In addition, all IDOT projects are screened for Regulated Substances as described in Section 27-3 of the BDE Manual and implemented via Section 669: Removal and Disposal of Regulated substances in the Standard Specifications for Road and Bridge Construction.

Approved alterations to the Department's provided SWPPP, including those necessary to protect Contractor Borrow, Use and Waste areas, shall be designed, installed, implemented and maintained by the Contractor in accordance with IDOT Standard Specifications Section 280.

2. 303(d) Impaired Waterways

Does the project area have any 303(d) impaired waterways with the following impairments?

- suspended solids
- turbidity, and or
- siltation

☐ Yes ☒ No

If yes, list the name(s) of the listed water body and the impairment(s)

| 303(d) waterbody | Impairments(s) |
|------------------|----------------|
| | |
| | |
| | |

In addition, It is paramount that the project does not increase the level of the impairment(s) described above. Discuss which BMPs will be implemented to reduce the risk of impairment increase

N/A

3. Total Maximum Daily Load (TMDL)

Does the project include any receiving waters with a TMDL for sediment, total suspended solids, turbidity or siltation? ☐ Yes ☐ No

If yes, List TMDL waterbodies below and describe associated TMDL

| TMDL waterbody | TMDL |
|----------------|------|
| | |
| | |
| | |

Provide a description of the erosion and sediment control strategy that will be incorporated into the site design that is consistent with the assumptions and requirements of the TMDL

N/A

If a specific numeric waste load allocation has been established that would apply to the project's discharges, provide a description of the necessary steps to meet that allocation

N/A

Part IV. Temporary Erosion and Sediment Controls

Stabilization efforts must be initiated within 1 working day of cessation of construction activity and completed within 14 days. Areas must be stabilized if they will not be disturbed for at least 14 calendar days. Exceptions to this time frame include:

- (i) Where the initiation of stabilization measures is precluded by snow cover, stabilization measures must be initiated as soon as practicable,
- (ii) On areas where construction activities have temporarily ceased and will resume after 14 days, a temporary stabilization method can be used (temporary stabilization techniques must be described), and
- (iii) Stabilization is not required for exit points at linear utility construction site that are used only episodically and for very short durations over the life of the project, provided other exit point controls are implemented to minimize sediment track-out.

Additionally, a record must be kept with the SWPPP throughout construction of the dates when major grading activities occur, when construction activities temporarily or permanently cease on a portion of the site, and when stabilization measures are initiated.

At a minimum, controls must be coordinated, installed and maintained to:

1. Minimize the amount of soil exposed during construction activity.
2. Minimize the disturbance of steep slopes.
3. Maintain natural buffers around surface waters, direct storm water to vegetated areas to increase sediment removal and maximize storm water infiltration, unless infeasible.
4. Minimize soil compaction and, unless infeasible, preserve topsoil.

Note: For practices below, consult relevant design criteria in Chapter 41 of the BDE Manual and maintenance criteria in Erosion and Sediment Control Field Guide for Construction.

1. Erosion Control:

The following are erosion control practices which may be used on a project (place a check by each practice that will be utilized on the project, add additional practices as needed):

- | | |
|---|--|
| <input checked="" type="checkbox"/> Mulch | <input checked="" type="checkbox"/> Preservation of existing vegetation |
| <input checked="" type="checkbox"/> Erosion Control Blanket | <input checked="" type="checkbox"/> Temporary Turf Cover Mixture (Class 7) |
| <input type="checkbox"/> Turf Reinforcement Mat | <input checked="" type="checkbox"/> Permanent seeding (Class 1-6) |
| <input checked="" type="checkbox"/> Sodding | <input checked="" type="checkbox"/> Other (Specify) <u>Staged Construction</u> |
| <input type="checkbox"/> Geotextile fabric | <input type="checkbox"/> Other (Specify) _____ |
| | <input type="checkbox"/> Other (Specify) _____ |

2. Sediment Control:

The following sediment control devices will be implemented on this project:

FAP Routes 669 & 673 (IL 116/IL 8 & US 24)
 Project NHPP-7VBW(880)
 Section 13[(HVB,HB-3)]BR-1;R
 Tazewell County
 Contract No. 68C55

- | | |
|---|---|
| <input checked="" type="checkbox"/> Ditch Checks <input checked="" type="checkbox"/> Inlet and Pipe protection <input checked="" type="checkbox"/> Hay or Straw bales <input type="checkbox"/> Above grade inlet filters (fitted) <input type="checkbox"/> Above grade inlet filters (non-fitted) <input type="checkbox"/> Inlet filters | <input checked="" type="checkbox"/> Perimeter Erosion Barrier <input checked="" type="checkbox"/> Rolled Excelsior <input checked="" type="checkbox"/> Silt Filter Fence <input checked="" type="checkbox"/> Urethane foam/geotextiles <input type="checkbox"/> Other (Specify) _____ <input type="checkbox"/> Other (Specify) _____ <input type="checkbox"/> Other (Specify) _____ |
|---|---|

3. Structural Practices:

Provide below is a description of structural practices that will be implemented:

- | | |
|--|---|
| <input type="checkbox"/> Aggregate Ditch <input type="checkbox"/> Articulated Block Revetment Mat <input type="checkbox"/> Barrier (Permanent) <input type="checkbox"/> Concrete Revetment Mats <input type="checkbox"/> Dewatering Filtering <input checked="" type="checkbox"/> Gabions <input type="checkbox"/> In-Stream or Wetland Work <input type="checkbox"/> Level Spreaders <input checked="" type="checkbox"/> Paved Ditch <input type="checkbox"/> Permanent Check Dams <input type="checkbox"/> Precast Block Revetment Mat <input checked="" type="checkbox"/> Rock Outlet Protection | <input checked="" type="checkbox"/> Stabilized Construction Exits <input type="checkbox"/> Stabilized Trench Flow <input type="checkbox"/> Sediment Basin <input checked="" type="checkbox"/> Retaining Walls <input checked="" type="checkbox"/> Riprap <input checked="" type="checkbox"/> Storm Drain Inlet Protection <input checked="" type="checkbox"/> Slope Walls <input type="checkbox"/> Sediment Trap <input type="checkbox"/> Other (Specify) _____ <input type="checkbox"/> Other (Specify) _____ <input type="checkbox"/> Other (Specify) _____ <input type="checkbox"/> Other (Specify) _____ |
|--|---|

4. Polymer Flocculants

Design guidance for polymer flocculants is available in Chapter 41 of the BDE Manual. In addition, Polymer Flocculants may only be used by district Special Provision.

If polymer flocculants are used for this project, the following must be adhered to and described below:

- Identify the use of all polymer flocculants at the site.
- Dosage of treatment chemicals shall be identified along with any information from any Material Safety Data Sheet.
- Describe the location of all storage areas for chemicals.
- Include any information from the manufacturer's specifications.
- Treatment chemicals must be stored in areas where they will not be exposed to precipitation.
- The SWPPP must describe procedures for use of treatment chemicals and staff responsible for use/application of treatment chemicals must be trained on the established procedures.

N/A

Part V. Other Conditions

1. Dewatering

Will dewatering be required for this project? ☐ Yes ☒ No

If yes, the following applies:

- Dewatering discharges shall be routed through a sediment control (e.g., sediment trap or basin, pumped water filter bag) designed to minimize discharges with visual turbidity;
- The discharge shall not include visible floating solids or foam;
- The discharge must not cause the formation of a visible sheen on the water surface, or visible oily deposits on the bottom or shoreline of the receiving water. An oil-water separator or suitable filtration device shall be used to treat oil, grease, or other similar products if dewatering water is found to or expected to contain these materials;
- To the extent feasible, use well-vegetated (e.g., grassy or wooded), upland areas of the site to infiltrate dewatering water before discharge;
- You are prohibited from using receiving waters as part of the treatment area;
- To minimize dewatering-related erosion and related sediment discharges, use stable, erosion-resistant surfaces (e.g., well-vegetated grassy areas, clean filler stone, geotextile underlayment) to discharge from dewatering controls. Do not place dewatering controls, such as pumped water filter bags, on steep slopes (15% or greater in grade);
- Backwash water (water used to backwash/clean any filters used as part of storm water treatment) must be properly treated or hauled off-site for disposal;
- Dewatering treatment devices shall be properly maintained; and
- See Part XI (Inspections) for inspection requirement.

Part VI. Permanent (i.e., Post-Construction) Storm Water Management Controls

Provided below is a description of measures that may be installed during the construction process to control volume and therefore the amount pollutants in storm water runoff that can occur after construction operations have been completed.

Practices may include but are not limited to the following:

- Aggregate ditch checks;
- bioswales,
- detention pond(s),
- infiltration trench;
- retention pond(s),
- open vegetated swales and natural depressions,
- treatment train (sequential system which combine several practices).
- Velocity dissipation devices (See Structural Practices above)

Describe these practices below

1. Permanent seeding / sodding will be used on all areas that have been hydraulically determined to have flow velocities and shear stress below levels that would cause erosion and scour.
2. Gabions are proposed to stabilize steep slopes at the south project limits of Main Street.
3. Paved Ditch is proposed to stabilize concentrated flow at steep slopes.
4. Perimeter Erosion Barrier will be used to prevent sediment loss by sheet flow. This item should be placed as shown on the plans. Silt fence should only be used as Perimeter Erosion Barrier in areas where the work area is higher than the perimeter. The use of silt fence at the top of the slope/elevations higher than the work area should always be avoided. If necessary, temporary fence should be utilized in these locations (where top of slope/elevation is higher than the work area) in lieu of silt fence.
5. Retaining Walls are proposed to stabilize steep slopes at Wall A, D, and E.
6. Stone Riprap will be utilized at steep slopes and outlets of culverts and drainage structures with high exit velocities, or which discharge to steep slopes, to stabilize slopes and prevent erosion.
7. Slope Walls are proposed under bridges to stabilize steep slopes and cannot establish vegetation.
8. Erosion Control Blanket will be placed at 3:1 slopes as shown on the plans to stabilize steep slopes and prevent erosion of slopes.

Part VII. Additional Practices Incorporated From Local Ordinance(s)

In some instances, an additional practice from a local ordinance may be included in the project. If so, describe below (Note: the Department is not subject to local ordinances)

N/A

Part VIII. Unexpected Regulated Substances/Chemical Spill Procedures

When Unexpected Regulated Substances or chemical spills occur, Article 107.19 of the Standard Specifications for Road and Bridge Construction shall apply. In addition, it is the contractor's responsibility to notify the Engineer in the event of a chemical spill into a ditch or waterway, the Engineer will then notify appropriate IEPA and IEMA personnel for the appropriate cleanup procedures.

Part IX. Contractor Required Submittals

Prior to conducting any professional services at the site covered by this plan, the Contractor and each subcontractor responsible for compliance with the permit shall submit to the Resident Engineer a Contractor Certification Statement, BDE 2342A.

1. The Contractor shall provide a construction schedule containing an adequate level of detail to show major activities with implementation of pollution prevention BMPs, including the following items:

- Approximate duration of the project, including each stage of the project
- Rainy season, dry season, and winter shutdown dates
- Temporary stabilization measures to be employed by contract phases
- Mobilization time-frame
- Mass clearing and grubbing/roadside clearing dates
- Deployment of Erosion Control Practices
- Deployment of Sediment Control Practices (including stabilized construction entrances and exits to be used and how they will be maintained)
- Deployment of Construction Site Management Practices (including concrete washout facilities, chemical storage, refueling locations, etc.)
- Paving, saw-cutting, and any other pavement related operations
- Major planned stockpiling operation
- Time frame for other significant long-term operations or activities that may plan non-storm water discharges as dewatering, grinding, etc.
- Permanent stabilization activities for each area of the project

2. During the pre-construction meeting, the Contractor and each subcontractor shall provide, as an attachment to their signed Contractor Certification Statement, a discussion of how they will comply with the requirements of the permit in regard to the following items and provide a graphical representation showing location and type of BMPs to be used when applicable:

- Temporary Ditch Checks - Identify what type and the source of Temporary Ditch Checks that will be installed as part of the project. The installation details will then be included with the SWPPP.
- Vehicle Entrances and Exits - Identify type and location of stabilized construction entrances and exits to be used and how they will be maintained.
- Material Delivery, Storage and Use- Discuss where and how materials including chemicals, concrete curing compounds, petroleum products, etc. will be stored for this project. Specifically, any chemical stored in a 55 gallon drum provided by the contractor.
- Stockpile Management - Identify the location of both on-site and off-site stockpiles. Discuss what BMPs will be used to prevent pollution of storm water from stockpiles.

- Waste Disposal - Discuss methods of waste disposal that will be used for this project.
- Spill Prevention and Control - Discuss steps that will be taken in the event of a material spill.
- Concrete Residuals and Washout Wastes - Discuss the location and type of concrete washout facilities to be used on this project and how they will be signed and maintained.
- Litter Management - Discuss how litter will be maintained for this project (education of employees, number of dumpsters, frequency of dumpster pick-up, etc.).
- Vehicle and Equipment Fueling - Identify equipment fueling locations for this project and what BMPs will be used to ensure containment and spill prevention.
- Vehicle and Equipment Cleaning and Maintenance - Identify where equipment cleaning and maintenance locations for this project and what BMPs will be used to ensure containment and spill prevention.
- Dewatering Activities - Identify the controls which will be used during dewatering operations to ensure sediments will not leave the construction site.

Additional measures indicated in the plan

N/A

Part X. Maintenance

It will be the Contractor's responsibility to attain maintenance guidelines for any manufactured BMPs which are to be installed and maintained per manufacture's specifications. However, when requested by the Contractor, the Resident Engineer will provide general maintenance guides (e.g., IDOT Erosion and Sediment Control Field Guide) to the Contractor for the practices associated with this project. Any damage or undermining shall be repaired immediately.

For Inlet Protection: Where there is evidence of sediment accumulation adjacent to the inlet protection measure, the deposited sediment must be removed by the following business day.

Below, describe procedures to maintain in good and effective operating conditions

All installation and maintenance guidelines will be followed per the manufacturer's specifications, and IDOT specifications.

1. Erosion Control Blanket / Mulching will be applied to all proposed grassed areas immediately following the finished grading.
2. Permanent Seeding Classes 3 and 5A are proposed for application to all proposed grass areas.
3. Preservation of Mature Seeding (or vegetation) will be used throughout the project duration. The Contractor shall take whatever precautions are necessary to limit the amount of vegetation removed by construction operations, protect vegetation outside the limits of construction from damage, and remove only vegetation necessary for completion of the project.
4. Protection of Trees is specifically proposed for trees along the right-of-way at commercial properties, and generally those that are outside of clearing limits.
5. Sodding is proposed to expedite stabilization within the northern commercial properties.
6. Temporary Erosion Control Seeding will be used as a temporary erosion method when permanent seeding cannot be accomplished to minimize the amount of exposed erodible/bare surface areas.
7. Temporary Mulching will be used as a temporary erosion control method to prevent sheet erosion of areas that are to be altered during later construction phases.
8. Staged Construction will minimize the duration of exposed soils. Where possible, work will be completed leaving a stabilized surface or temporary stabilization before work is transitioned to subsequent stages.
9. Storm Drain Inlet Protection will control the loss of sediment from the project site, and is to be placed at all existing inlets within the construction limits and all new inlets constructed as well.
10. Stabilized Construction Exits will be placed at all points of construction ingress/egress where sediment can be deposited onto roadways via construction equipment/vehicles. Adjacent streets shall be swept regularly to remove sediment tracked onto the roadway from construction activities. All work associated with installation and maintenance of Stabilized Concrete Exits / Concrete Washouts are included in the contract.
11. Temporary Ditch Check will be used to reduce the velocity of water as concentrated flow to prevent erosion or scour of swales and ditches. These should be placed as shown on the plans.

Part XI. Inspections

Qualified personnel shall inspect disturbed areas of the construction site that have not been finally stabilized, structural control measures, and locations where vehicles enter or exit the site at least once every seven calendar days and within 24 hours of the end of a storm or by the end of the following business or workday that is 0.50 inches or greater or equivalent snowmelt (except as allowed for Frozen Conditions).

In addition, all areas where storm water typically flows within the site should be inspected periodically to check for evidence of pollutants entering the drainage system, as well as all locations where stabilization measures have been implemented to ensure they are operating correctly.

Inspections shall be documented on the form BC 2259 (Storm Water Pollution Prevention Plan Erosion Control Inspection Report).

The Erosion and Sediment Control Field Guide for Construction Inspection shall be consulted as needed.

Dewatering

For site(s) discharging dewatering water, an inspection during the discharge shall be done once per day on which the discharge occurs and record the following in a report within 24 hours of completing the inspection:

- The inspection date;
- Names and titles of personnel performing the inspection;
- Approximate times that the dewatering discharge began and ended on the day of inspection;
- Estimates of the rate (in gallons per day) of discharge on the day of inspection;
- Whether or not any of the following indications of pollutant discharge were observed at the point of discharge: a sediment plume, suspended solids, unusual color, presence of odor, decreased clarity, or presence of foam; and/or a visible sheen on the water surface or visible oily deposits on the bottom or shoreline of the receiving water.

Frozen Conditions

Inspections may be reduced to once per month when all construction activities have ceased due to frozen conditions. Weekly inspections will recommence when construction activities resume, either temporarily or continuously, or if there is 0.5" or greater rain event, or a discharge due to snowmelt occurs.

Flooding or unsafe conditions

Areas that are inaccessible during required inspections due to flooding or other unsafe conditions must be inspected within 72 hours of becoming accessible.

Part XII. Incidence of Noncompliance (ION)

The Department shall notify the appropriate Agency Field Operations Section office by email as described on the IEPA ION form, within 24 hours of any incidence of noncompliance for any violation of the storm water pollution prevention plan observed during any inspection conducted, or for violations of any condition of this permit.

The Department shall complete and submit within 5 days an "Incidence of Noncompliance" (ION) report for any violation of the storm water pollution prevention plan observed during any inspection conducted, or for violations of any condition of this permit. Submission shall be on forms provided by the IEPA and include specific information on the cause of noncompliance, actions which were taken to prevent any further causes of noncompliance, and a statement detailing any environmental impact which may have resulted from the noncompliance. Corrective actions must be undertaken immediately to address the identified non-compliance issue(s).

Illinois EPA
2520 W. Iles Ave./P.O. Box 19276
Springfield, IL 62794-9276

Please note that if these are delivered via FedEx or UPS, these carriers cannot deliver to our P.O. Box and this number must be excluded from the mailing address.

Part XIII. Corrective Actions

Corrective actions must be taken when:

- A storm water control needs repair or replacement;
- A storm water control necessary to comply with the requirements of this permit was never installed, or was installed incorrectly;
- Discharges are causing an exceedance of applicable water quality standards; or
- A prohibited discharge has occurred.

Corrective Actions must be completed as soon as possible and documented within 7 days in an Inspection Report or report of noncompliance. If it is infeasible to complete the installation or repair within 7 calendar days, it must be documented in the records why it is infeasible to complete the installation or repair within the 7 day time-frame and document the schedule for installing the storm water control(s) and making it operational as soon as feasible after the 7-day time-frame.. In the event that maintenance is required for the same storm water control at the same location three or more times, the control must be repaired in a manner that prevents continued failure to the extent feasible, and it must be documented the condition and how it was repaired in the records. Alternatively, it must be documented why the specific re-occurrence of this same issue must continue to be addressed as a routine maintenance fix.

Part XIV. Retention of Records

The Department must retain copies of the SWPPP and all reports and notices required by this permit, records of all data used to complete the NOI to be covered by this permit, and the Agency Notice of Permit Coverage letter for at least three years from the date that the permit coverage expires or is terminated. the permittee must retain a copy of the SWPPP and any revisions to the SWPPP required by this permit at the construction site from the date of project initiation to the date of final stabilization. Any manuals or other documents referenced in the SWPPP must also be retained at the construction site.

Part XV. Failure to Comply

Failure to comply with any provisions of this Storm Water Pollution Prevention Plan will result in the implementation of a National Pollutant Discharge Elimination System/Erosion and Sediment Control Deficiency Deduction against the Contractor and/or penalties under the Permit ILR10 which could be passed on to the contractor (See Article 105.03 Conformity with Contract)

Part XVI. Keeping the SWPPP ("plan") Current

IDOT shall amend the plan whenever there is a change in design, construction, operation, or maintenance, which has a significant effect on the potential for the discharge of pollutants to Waters of the United States and which has not otherwise been addressed in the plan or if the plan proves to be ineffective in eliminating or significantly minimizing sediment and/or pollutants identified under paragraph Part II. Water Quality or in otherwise achieving the general objectives of controlling pollutants in storm water discharges associated with construction site activity.

In addition, the plan shall be amended to identify any new contractor and/or subcontractor that will implement a measure of the plan. Amendments to the plan may be reviewed by the IEPA the same manner as the SWPPP and Erosion and Sediment Control Plan (ESCP) submitted as part of the Notice of Intent (NOI). The SWPPP and site map must be modified within 7 days for any changes to construction plans, storm water controls or other activities at the site that are no longer accurately reflected in the SWPPP.

In addition, the NOI shall be modified using the CDX system for any substantial modifications to the project such as:

- address changes
- new contractors
- area coverage
- additional discharges to Waters of the United States, or
- other substantial modifications (e.g. addition of dewatering activities).

The notice of intent shall be modified within 30 days of the modification to the project.

Part XVII: Notifications

In addition to the NOI submitted to IEPA, all MS4 permittees identified in Part I. Site Description shall receive a copy of the NOI.

Part XVIII. Notice of Termination

Where a site has completed final stabilization and all storm water discharges from construction activities that are authorized by this permit are eliminated, the permittee must submit a completed Notice of Termination (NOT) that is signed in accordance with ILR10 permit.

Method of Measurement: NPDES Compliance shall not be measured for payment separately. Measurement for payment for Temporary Erosion and Sediment Control shall be in accordance with Section 280 or as otherwise provided in the contract. Permanent BMPs necessary to comply with this provision shall be measured for payment in accordance with their respective provisions in the contract.

Basis of Payment: NPDES Compliance shall not be paid for separately. Payment for Temporary Erosion and Sediment Control shall be in accordance with Section 280 or as otherwise provided in the contract. Permanent BMPs necessary to comply with this provision shall be paid for in accordance with their respective payment provisions in the contract.

CONTRACTOR'S CERTIFICATION STATEMENT (BDE 2342A)



Contractor Certification Statement

Prior to conducting any professional services at the site covered by this contract, the Contractor and every subcontractor must complete and return to the Resident Engineer the following certification. A separate certification must be submitted by each firm. Attach to this certification all items required by Part IX. Contractor Required Submittals of the Storm Water Pollution Prevention Plan (SWPPP) which will be handled by the Contractor/subcontractor completing this form.

| | | |
|----------------|--------------|-----------------------|
| Route | Marked Route | Section Number |
| FAP 669 | IL 8/IL 116 | 13[(HVB,HB-3)] BR-1;R |
| Project Number | County | Contract Number |
| NHPP-7VBW(880) | Tazewell | 68C55 |

This certification statement is a part of SWPPP for the project described above, in accordance with the General NPDES Permit No. ILR10 issued by the Illinois Environmental Protection Agency.

I certify under penalty of law that I understand the terms of the Permit No. ILR 10 that authorizes the storm water discharges associated with industrial activity from the construction site identified as part of this certification.

Additionally, I have read and understand all of the information and requirements stated in SWPPP for the above mentioned project; I have received copies of all appropriate maintenance procedures; and, I have provided all documentation required to be in compliance with the Permit ILR10 and SWPPP and will provide timely updates to these documents as necessary.

- ☐ Contractor
☐ Sub-Contractor

| | | | |
|---|-------|-------|----------|
| Signature | Date | | |
| | | | |
| Print Name | Title | | |
| | | | |
| Name of Firm | Phone | | |
| | | | |
| Street Address | City | State | Zip Code |
| | | | |
| Items which this Contractor/subcontractor will be responsible for as required in Section II.G. of SWPPP | | | |
| | | | |

REQUIRED CONTRACT PROVISIONS FEDERAL-AID CONSTRUCTION CONTRACTS

- I. General
- II. Nondiscrimination
- III. Non-segregated Facilities
- IV. Davis-Bacon and Related Act Provisions
- V. Contract Work Hours and Safety Standards Act Provisions
- VI. Subletting or Assigning the Contract
- VII. Safety: Accident Prevention
- VIII. False Statements Concerning Highway Projects
- IX. Implementation of Clean Air Act and Federal Water Pollution Control Act
- X. Certification Regarding Debarment, Suspension, Ineligibility and Voluntary Exclusion
- XI. Certification Regarding Use of Contract Funds for Lobbying
- XII. Use of United States-Flag Vessels:

ATTACHMENTS

A. Employment and Materials Preference for Appalachian Development Highway System or Appalachian Local Access Road Contracts (included in Appalachian contracts only)

I. GENERAL

1. Form FHWA-1273 must be physically incorporated in each construction contract funded under title 23, United States Code, as required in 23 CFR 633.102(b) (excluding emergency contracts solely intended for debris removal). The contractor (or subcontractor) must insert this form in each subcontract and further require its inclusion in all lower tier subcontracts (excluding purchase orders, rental agreements and other agreements for supplies or services). 23 CFR 633.102(e).

The applicable requirements of Form FHWA-1273 are incorporated by reference for work done under any purchase order, rental agreement or agreement for other services. The prime contractor shall be responsible for compliance by any subcontractor, lower-tier subcontractor or service provider. 23 CFR 633.102(e).

Form FHWA-1273 must be included in all Federal-aid design-build contracts, in all subcontracts and in lower tier subcontracts (excluding subcontracts for design services, purchase orders, rental agreements and other agreements for supplies or services) in accordance with 23 CFR 633.102. The design-builder shall be responsible for compliance by any subcontractor, lower-tier subcontractor or service provider.

Contracting agencies may reference Form FHWA-1273 in solicitation-for-bids or request-for-proposals documents, however, the Form FHWA-1273 must be physically incorporated (not referenced) in all contracts, subcontracts and lower-tier subcontracts (excluding purchase orders, rental agreements and other agreements for supplies or services related to a construction contract). 23 CFR 633.102(b).

2. Subject to the applicability criteria noted in the following sections, these contract provisions shall apply to all work

performed on the contract by the contractor's own organization and with the assistance of workers under the contractor's immediate superintendence and to all work performed on the contract by piecework, station work, or by subcontract. 23 CFR 633.102(d).

3. A breach of any of the stipulations contained in these Required Contract Provisions may be sufficient grounds for withholding of progress payments, withholding of final payment, termination of the contract, suspension / debarment or any other action determined to be appropriate by the contracting agency and FHWA.

4. Selection of Labor: During the performance of this contract, the contractor shall not use convict labor for any purpose within the limits of a construction project on a Federal-aid highway unless it is labor performed by convicts who are on parole, supervised release, or probation. 23 U.S.C. 114(b). The term Federal-aid highway does not include roadways functionally classified as local roads or rural minor collectors. 23 U.S.C. 101(a).

II. NONDISCRIMINATION (23 CFR 230.107(a); 23 CFR Part 230, Subpart A, Appendix A; EO 11246)

The provisions of this section related to 23 CFR Part 230, Subpart A, Appendix A are applicable to all Federal-aid construction contracts and to all related construction subcontracts of \$10,000 or more. The provisions of 23 CFR Part 230 are not applicable to material supply, engineering, or architectural service contracts.

In addition, the contractor and all subcontractors must comply with the following policies: Executive Order 11246, 41 CFR Part 60, 29 CFR Parts 1625-1627, 23 U.S.C. 140, Section 504 of the Rehabilitation Act of 1973, as amended (29 U.S.C. 794), Title VI of the Civil Rights Act of 1964, as amended (42 U.S.C. 2000d et seq.), and related regulations including 49 CFR Parts 21, 26, and 27; and 23 CFR Parts 200, 230, and 633.

The contractor and all subcontractors must comply with: the requirements of the Equal Opportunity Clause in 41 CFR 60-1.4(b) and, for all construction contracts exceeding \$10,000, the Standard Federal Equal Employment Opportunity Construction Contract Specifications in 41 CFR 60-4.3.

Note: The U.S. Department of Labor has exclusive authority to determine compliance with Executive Order 11246 and the policies of the Secretary of Labor including 41 CFR Part 60, and 29 CFR Parts 1625-1627. The contracting agency and the FHWA have the authority and the responsibility to ensure compliance with 23 U.S.C. 140, Section 504 of the Rehabilitation Act of 1973, as amended (29 U.S.C. 794), and Title VI of the Civil Rights Act of 1964, as amended (42 U.S.C. 2000d et seq.), and related regulations including 49 CFR Parts 21, 26, and 27; and 23 CFR Parts 200, 230, and 633.

The following provision is adopted from 23 CFR Part 230, Subpart A, Appendix A, with appropriate revisions to conform to the U.S. Department of Labor (US DOL) and FHWA requirements.

1. Equal Employment Opportunity: Equal Employment Opportunity (EEO) requirements not to discriminate and to take affirmative action to assure equal opportunity as set forth under laws, executive orders, rules, regulations (see 28 CFR Part 35, 29 CFR Part 1630, 29 CFR Parts 1625-1627, 41 CFR Part 60 and 49 CFR Part 27) and orders of the Secretary of Labor as modified by the provisions prescribed herein, and imposed pursuant to 23 U.S.C. 140, shall constitute the EEO and specific affirmative action standards for the contractor's project activities under this contract. The provisions of the Americans with Disabilities Act of 1990 (42 U.S.C. 12101 et seq.) set forth under 28 CFR Part 35 and 29 CFR Part 1630 are incorporated by reference in this contract. In the execution of this contract, the contractor agrees to comply with the following minimum specific requirement activities of EEO:

a. The contractor will work with the contracting agency and the Federal Government to ensure that it has made every good faith effort to provide equal opportunity with respect to all of its terms and conditions of employment and in their review of activities under the contract. 23 CFR 230.409 (g)(4) & (5).

b. The contractor will accept as its operating policy the following statement:

"It is the policy of this Company to assure that applicants are employed, and that employees are treated during employment, without regard to their race, religion, sex, sexual orientation, gender identity, color, national origin, age or disability. Such action shall include: employment, upgrading, demotion, or transfer; recruitment or recruitment advertising; layoff or termination; rates of pay or other forms of compensation; and selection for training, including apprenticeship, pre-apprenticeship, and/or on-the-job training."

2. EEO Officer: The contractor will designate and make known to the contracting officers an EEO Officer who will have the responsibility for and must be capable of effectively administering and promoting an active EEO program and who must be assigned adequate authority and responsibility to do so.

3. Dissemination of Policy: All members of the contractor's staff who are authorized to hire, supervise, promote, and discharge employees, or who recommend such action or are substantially involved in such action, will be made fully cognizant of and will implement the contractor's EEO policy and contractual responsibilities to provide EEO in each grade and classification of employment. To ensure that the above agreement will be met, the following actions will be taken as a minimum:

a. Periodic meetings of supervisory and personnel office employees will be conducted before the start of work and then not less often than once every six months, at which time the contractor's EEO policy and its implementation will be reviewed and explained. The meetings will be conducted by the EEO Officer or other knowledgeable company official.

b. All new supervisory or personnel office employees will be given a thorough indoctrination by the EEO Officer, covering all major aspects of the contractor's EEO obligations within thirty days following their reporting for duty with the contractor.

c. All personnel who are engaged in direct recruitment for the project will be instructed by the EEO Officer in the contractor's procedures for locating and hiring minorities and women.

d. Notices and posters setting forth the contractor's EEO policy will be placed in areas readily accessible to employees, applicants for employment and potential employees.

e. The contractor's EEO policy and the procedures to implement such policy will be brought to the attention of employees by means of meetings, employee handbooks, or other appropriate means.

4. Recruitment: When advertising for employees, the contractor will include in all advertisements for employees the notation: "An Equal Opportunity Employer." All such advertisements will be placed in publications having a large circulation among minorities and women in the area from which the project work force would normally be derived.

a. The contractor will, unless precluded by a valid bargaining agreement, conduct systematic and direct recruitment through public and private employee referral sources likely to yield qualified minorities and women. To meet this requirement, the contractor will identify sources of potential minority group employees and establish with such identified sources procedures whereby minority and women applicants may be referred to the contractor for employment consideration.

b. In the event the contractor has a valid bargaining agreement providing for exclusive hiring hall referrals, the contractor is expected to observe the provisions of that agreement to the extent that the system meets the contractor's compliance with EEO contract provisions. Where implementation of such an agreement has the effect of discriminating against minorities or women, or obligates the contractor to do the same, such implementation violates Federal nondiscrimination provisions.

c. The contractor will encourage its present employees to refer minorities and women as applicants for employment. Information and procedures with regard to referring such applicants will be discussed with employees.

5. Personnel Actions: Wages, working conditions, and employee benefits shall be established and administered, and personnel actions of every type, including hiring, upgrading, promotion, transfer, demotion, layoff, and termination, shall be taken without regard to race, color, religion, sex, sexual orientation, gender identity, national origin, age or disability. The following procedures shall be followed:

a. The contractor will conduct periodic inspections of project sites to ensure that working conditions and employee facilities do not indicate discriminatory treatment of project site personnel.

b. The contractor will periodically evaluate the spread of wages paid within each classification to determine any evidence of discriminatory wage practices.

c. The contractor will periodically review selected personnel actions in depth to determine whether there is evidence of discrimination. Where evidence is found, the contractor will promptly take corrective action. If the review indicates that the discrimination may extend beyond the actions reviewed, such corrective action shall include all affected persons.

d. The contractor will promptly investigate all complaints of alleged discrimination made to the contractor in connection with its obligations under this contract, will attempt to resolve such complaints, and will take appropriate corrective action

within a reasonable time. If the investigation indicates that the discrimination may affect persons other than the complainant, such corrective action shall include such other persons. Upon completion of each investigation, the contractor will inform every complainant of all of their avenues of appeal.

6. Training and Promotion:

a. The contractor will assist in locating, qualifying, and increasing the skills of minorities and women who are applicants for employment or current employees. Such efforts should be aimed at developing full journey level status employees in the type of trade or job classification involved.

b. Consistent with the contractor's work force requirements and as permissible under Federal and State regulations, the contractor shall make full use of training programs (i.e., apprenticeship and on-the-job training programs for the geographical area of contract performance). In the event a special provision for training is provided under this contract, this subparagraph will be superseded as indicated in the special provision. The contracting agency may reserve training positions for persons who receive welfare assistance in accordance with 23 U.S.C. 140(a).

c. The contractor will advise employees and applicants for employment of available training programs and entrance requirements for each.

d. The contractor will periodically review the training and promotion potential of employees who are minorities and women and will encourage eligible employees to apply for such training and promotion.

7. Unions: If the contractor relies in whole or in part upon unions as a source of employees, the contractor will use good faith efforts to obtain the cooperation of such unions to increase opportunities for minorities and women. 23 CFR 230.409. Actions by the contractor, either directly or through a contractor's association acting as agent, will include the procedures set forth below:

a. The contractor will use good faith efforts to develop, in cooperation with the unions, joint training programs aimed toward qualifying more minorities and women for membership in the unions and increasing the skills of minorities and women so that they may qualify for higher paying employment.

b. The contractor will use good faith efforts to incorporate an EEO clause into each union agreement to the end that such union will be contractually bound to refer applicants without regard to their race, color, religion, sex, sexual orientation, gender identity, national origin, age, or disability.

c. The contractor is to obtain information as to the referral practices and policies of the labor union except that to the extent such information is within the exclusive possession of the labor union and such labor union refuses to furnish such information to the contractor, the contractor shall so certify to the contracting agency and shall set forth what efforts have been made to obtain such information.

d. In the event the union is unable to provide the contractor with a reasonable flow of referrals within the time limit set forth in the collective bargaining agreement, the contractor will, through independent recruitment efforts, fill the employment vacancies without regard to race, color, religion, sex, sexual orientation, gender identity, national origin, age, or disability; making full efforts to obtain qualified and/or qualifiable minorities and women. The failure of a union to provide

sufficient referrals (even though it is obligated to provide exclusive referrals under the terms of a collective bargaining agreement) does not relieve the contractor from the requirements of this paragraph. In the event the union referral practice prevents the contractor from meeting the obligations pursuant to Executive Order 11246, as amended, and these special provisions, such contractor shall immediately notify the contracting agency.

8. Reasonable Accommodation for Applicants /

Employees with Disabilities: The contractor must be familiar with the requirements for and comply with the Americans with Disabilities Act and all rules and regulations established thereunder. Employers must provide reasonable accommodation in all employment situations unless to do so would cause an undue hardship.

9. Selection of Subcontractors, Procurement of Materials

and Leasing of Equipment: The contractor shall not discriminate on the grounds of race, color, religion, sex, sexual orientation, gender identity, national origin, age, or disability in the selection and retention of subcontractors, including procurement of materials and leases of equipment. The contractor shall take all necessary and reasonable steps to ensure nondiscrimination in the administration of this contract.

a. The contractor shall notify all potential subcontractors, suppliers, and lessors of their EEO obligations under this contract.

b. The contractor will use good faith efforts to ensure subcontractor compliance with their EEO obligations.

10. Assurances Required:

a. The requirements of 49 CFR Part 26 and the State DOT's FHWA-approved Disadvantaged Business Enterprise (DBE) program are incorporated by reference.

b. The contractor, subrecipient or subcontractor shall not discriminate on the basis of race, color, national origin, or sex in the performance of this contract. The contractor shall carry out applicable requirements of 49 CFR part 26 in the award and administration of DOT-assisted contracts. Failure by the contractor to carry out these requirements is a material breach of this contract, which may result in the termination of this contract or such other remedy as the recipient deems appropriate, which may include, but is not limited to:

- (1) Withholding monthly progress payments;
- (2) Assessing sanctions;
- (3) Liquidated damages; and/or
- (4) Disqualifying the contractor from future bidding as non-responsible.

c. The Title VI and nondiscrimination provisions of U.S. DOT Order 1050.2A at Appendixes A and E are incorporated by reference. 49 CFR Part 21.

11. Records and Reports: The contractor shall keep such records as necessary to document compliance with the EEO requirements. Such records shall be retained for a period of three years following the date of the final payment to the contractor for all contract work and shall be available at reasonable times and places for inspection by authorized representatives of the contracting agency and the FHWA.

a. The records kept by the contractor shall document the following:

(1) The number and work hours of minority and non-minority group members and women employed in each work classification on the project;

(2) The progress and efforts being made in cooperation with unions, when applicable, to increase employment opportunities for minorities and women; and

(3) The progress and efforts being made in locating, hiring, training, qualifying, and upgrading minorities and women.

b. The contractors and subcontractors will submit an annual report to the contracting agency each July for the duration of the project indicating the number of minority, women, and non-minority group employees currently engaged in each work classification required by the contract work. This information is to be reported on [Form FHWA-1391](#). The staffing data should represent the project work force on board in all or any part of the last payroll period preceding the end of July. If on-the-job training is being required by special provision, the contractor will be required to collect and report training data. The employment data should reflect the work force on board during all or any part of the last payroll period preceding the end of July.

III. NONSEGREGATED FACILITIES

This provision is applicable to all Federal-aid construction contracts and to all related construction subcontracts of more than \$10,000. 41 CFR 60-1.5.

As prescribed by 41 CFR 60-1.8, the contractor must ensure that facilities provided for employees are provided in such a manner that segregation on the basis of race, color, religion, sex, sexual orientation, gender identity, or national origin cannot result. The contractor may neither require such segregated use by written or oral policies nor tolerate such use by employee custom. The contractor's obligation extends further to ensure that its employees are not assigned to perform their services at any location under the contractor's control where the facilities are segregated. The term "facilities" includes waiting rooms, work areas, restaurants and other eating areas, time clocks, restrooms, washrooms, locker rooms and other storage or dressing areas, parking lots, drinking fountains, recreation or entertainment areas, transportation, and housing provided for employees. The contractor shall provide separate or single-user restrooms and necessary dressing or sleeping areas to assure privacy between sexes.

IV. DAVIS-BACON AND RELATED ACT PROVISIONS

This section is applicable to all Federal-aid construction projects exceeding \$2,000 and to all related subcontracts and lower-tier subcontracts (regardless of subcontract size), in accordance with 29 CFR 5.5. The requirements apply to all projects located within the right-of-way of a roadway that is functionally classified as Federal-aid highway. 23 U.S.C. 113. This excludes roadways functionally classified as local roads or rural minor collectors, which are exempt. 23 U.S.C. 101. Where applicable law requires that projects be treated as a project on a Federal-aid highway, the provisions of this subpart will apply regardless of the location of the project. Examples include: Surface Transportation Block Grant Program projects funded under 23 U.S.C. 133 [excluding recreational trails projects], the Nationally Significant Freight and Highway

Projects funded under 23 U.S.C. 117, and National Highway Freight Program projects funded under 23 U.S.C. 167.

The following provisions are from the U.S. Department of Labor regulations in 29 CFR 5.5 "Contract provisions and related matters" with minor revisions to conform to the FHWA- 1273 format and FHWA program requirements.

1. Minimum wages (29 CFR 5.5)

a. *Wage rates and fringe benefits.* All laborers and mechanics employed or working upon the site of the work (or otherwise working in construction or development of the project under a development statute), will be paid unconditionally and not less often than once a week, and without subsequent deduction or rebate on any account (except such payroll deductions as are permitted by regulations issued by the Secretary of Labor under the Copeland Act ([29 CFR part 3](#))), the full amount of basic hourly wages and bona fide fringe benefits (or cash equivalents thereof) due at time of payment computed at rates not less than those contained in the wage determination of the Secretary of Labor which is attached hereto and made a part hereof, regardless of any contractual relationship which may be alleged to exist between the contractor and such laborers and mechanics. As provided in paragraphs (d) and (e) of 29 CFR 5.5, the appropriate wage determinations are effective by operation of law even if they have not been attached to the contract. Contributions made or costs reasonably anticipated for bona fide fringe benefits under the Davis-Bacon Act ([40 U.S.C. 3141\(2\)\(B\)](#)) on behalf of laborers or mechanics are considered wages paid to such laborers or mechanics, subject to the provisions of paragraph 1.e. of this section; also, regular contributions made or costs incurred for more than a weekly period (but not less often than quarterly) under plans, funds, or programs which cover the particular weekly period, are deemed to be constructively made or incurred during such weekly period. Such laborers and mechanics must be paid the appropriate wage rate and fringe benefits on the wage determination for the classification(s) of work actually performed, without regard to skill, except as provided in paragraph 4. of this section. Laborers or mechanics performing work in more than one classification may be compensated at the rate specified for each classification for the time actually worked therein: *Provided*, That the employer's payroll records accurately set forth the time spent in each classification in which work is performed. The wage determination (including any additional classifications and wage rates conformed under paragraph 1.c. of this section) and the Davis-Bacon poster (WH-1321) must be posted at all times by the contractor and its subcontractors at the site of the work in a prominent and accessible place where it can be easily seen by the workers.

b. *Frequently recurring classifications.* (1) In addition to wage and fringe benefit rates that have been determined to be prevailing under the procedures set forth in [29 CFR part 1](#), a wage determination may contain, pursuant to § 1.3(f), wage and fringe benefit rates for classifications of laborers and mechanics for which conformance requests are regularly submitted pursuant to paragraph 1.c. of this section, provided that:

(i) The work performed by the classification is not performed by a classification in the wage determination for which a prevailing wage rate has been determined;

(ii) The classification is used in the area by the construction industry; and

(iii) The wage rate for the classification bears a reasonable relationship to the prevailing wage rates contained in the wage determination.

(2) The Administrator will establish wage rates for such classifications in accordance with paragraph 1.c.(1)(iii) of this section. Work performed in such a classification must be paid at no less than the wage and fringe benefit rate listed on the wage determination for such classification.

c. *Conformance.* (1) The contracting officer must require that any class of laborers or mechanics, including helpers, which is not listed in the wage determination and which is to be employed under the contract be classified in conformance with the wage determination. Conformance of an additional classification and wage rate and fringe benefits is appropriate only when the following criteria have been met:

(i) The work to be performed by the classification requested is not performed by a classification in the wage determination; and

(ii) The classification is used in the area by the construction industry; and

(iii) The proposed wage rate, including any bona fide fringe benefits, bears a reasonable relationship to the wage rates contained in the wage determination.

(2) The conformance process may not be used to split, subdivide, or otherwise avoid application of classifications listed in the wage determination.

(3) If the contractor and the laborers and mechanics to be employed in the classification (if known), or their representatives, and the contracting officer agree on the classification and wage rate (including the amount designated for fringe benefits where appropriate), a report of the action taken will be sent by the contracting officer by email to DBAconformance@dol.gov. The Administrator, or an authorized representative, will approve, modify, or disapprove every additional classification action within 30 days of receipt and so advise the contracting officer or will notify the contracting officer within the 30-day period that additional time is necessary.

(4) In the event the contractor, the laborers or mechanics to be employed in the classification or their representatives, and the contracting officer do not agree on the proposed classification and wage rate (including the amount designated for fringe benefits, where appropriate), the contracting officer will, by email to DBAconformance@dol.gov, refer the questions, including the views of all interested parties and the recommendation of the contracting officer, to the Administrator for determination. The Administrator, or an authorized representative, will issue a determination within 30 days of receipt and so advise the contracting officer or will notify the contracting officer within the 30-day period that additional time is necessary.

(5) The contracting officer must promptly notify the contractor of the action taken by the Wage and Hour Division

under paragraphs 1.c.(3) and (4) of this section. The contractor must furnish a written copy of such determination to each affected worker or it must be posted as a part of the wage determination. The wage rate (including fringe benefits where appropriate) determined pursuant to paragraph 1.c.(3) or (4) of this section must be paid to all workers performing work in the classification under this contract from the first day on which work is performed in the classification.

d. *Fringe benefits not expressed as an hourly rate.*

Whenever the minimum wage rate prescribed in the contract for a class of laborers or mechanics includes a fringe benefit which is not expressed as an hourly rate, the contractor may either pay the benefit as stated in the wage determination or may pay another bona fide fringe benefit or an hourly cash equivalent thereof.

e. *Unfunded plans.* If the contractor does not make payments to a trustee or other third person, the contractor may consider as part of the wages of any laborer or mechanic the amount of any costs reasonably anticipated in providing bona fide fringe benefits under a plan or program, *Provided*, That the Secretary of Labor has found, upon the written request of the contractor, in accordance with the criteria set forth in § 5.28, that the applicable standards of the Davis-Bacon Act have been met. The Secretary of Labor may require the contractor to set aside in a separate account assets for the meeting of obligations under the plan or program.

f. *Interest.* In the event of a failure to pay all or part of the wages required by the contract, the contractor will be required to pay interest on any underpayment of wages.

2. Withholding (29 CFR 5.5)

a. *Withholding requirements.* The contracting agency may, upon its own action, or must, upon written request of an authorized representative of the Department of Labor, withhold or cause to be withheld from the contractor so much of the accrued payments or advances as may be considered necessary to satisfy the liabilities of the prime contractor or any subcontractor for the full amount of wages and monetary relief, including interest, required by the clauses set forth in this section for violations of this contract, or to satisfy any such liabilities required by any other Federal contract, or federally assisted contract subject to Davis-Bacon labor standards, that is held by the same prime contractor (as defined in § 5.2). The necessary funds may be withheld from the contractor under this contract, any other Federal contract with the same prime contractor, or any other federally assisted contract that is subject to Davis-Bacon labor standards requirements and is held by the same prime contractor, regardless of whether the other contract was awarded or assisted by the same agency, and such funds may be used to satisfy the contractor liability for which the funds were withheld. In the event of a contractor's failure to pay any laborer or mechanic, including any apprentice or helper working on the site of the work all or part of the wages required by the contract, or upon the contractor's failure to submit the required records as discussed in paragraph 3.d. of this section, the contracting agency may on its own initiative and after written notice to the contractor, take such action as may be necessary to cause the suspension of any further payment, advance, or guarantee of funds until such violations have ceased.

b. *Priority to withheld funds.* The Department has priority to funds withheld or to be withheld in accordance with paragraph

2.a. of this section or Section V, paragraph 3.a., or both, over claims to those funds by:

- (1) A contractor's surety(ies), including without limitation performance bond sureties and payment bond sureties;
- (2) A contracting agency for its procurement costs;
- (3) A trustee(s) (either a court-appointed trustee or a U.S. trustee, or both) in bankruptcy of a contractor, or a contractor's bankruptcy estate;
- (4) A contractor's assignee(s);
- (5) A contractor's successor(s); or
- (6) A claim asserted under the Prompt Payment Act, [31 U.S.C. 3901–3907](#).

3. Records and certified payrolls (29 CFR 5.5)

a. Basic record requirements (1) Length of record retention. All regular payrolls and other basic records must be maintained by the contractor and any subcontractor during the course of the work and preserved for all laborers and mechanics working at the site of the work (or otherwise working in construction or development of the project under a development statute) for a period of at least 3 years after all the work on the prime contract is completed.

(2) Information required. Such records must contain the name; Social Security number; last known address, telephone number, and email address of each such worker; each worker's correct classification(s) of work actually performed; hourly rates of wages paid (including rates of contributions or costs anticipated for bona fide fringe benefits or cash equivalents thereof of the types described in [40 U.S.C. 3141\(2\)\(B\)](#) of the Davis-Bacon Act); daily and weekly number of hours actually worked in total and on each covered contract; deductions made; and actual wages paid.

(3) Additional records relating to fringe benefits. Whenever the Secretary of Labor has found under paragraph 1.e. of this section that the wages of any laborer or mechanic include the amount of any costs reasonably anticipated in providing benefits under a plan or program described in [40 U.S.C. 3141\(2\)\(B\)](#) of the Davis-Bacon Act, the contractor must maintain records which show that the commitment to provide such benefits is enforceable, that the plan or program is financially responsible, and that the plan or program has been communicated in writing to the laborers or mechanics affected, and records which show the costs anticipated or the actual cost incurred in providing such benefits.

(4) Additional records relating to apprenticeship. Contractors with apprentices working under approved programs must maintain written evidence of the registration of apprenticeship programs, the registration of the apprentices, and the ratios and wage rates prescribed in the applicable programs.

b. Certified payroll requirements (1) Frequency and method of submission. The contractor or subcontractor must submit weekly, for each week in which any DBA- or Related Acts-covered work is performed, certified payrolls to the contracting

agency. The prime contractor is responsible for the submission of all certified payrolls by all subcontractors. A contracting agency or prime contractor may permit or require contractors to submit certified payrolls through an electronic system, as long as the electronic system requires a legally valid electronic signature; the system allows the contractor, the contracting agency, and the Department of Labor to access the certified payrolls upon request for at least 3 years after the work on the prime contract has been completed; and the contracting agency or prime contractor permits other methods of submission in situations where the contractor is unable or limited in its ability to use or access the electronic system.

(2) Information required. The certified payrolls submitted must set out accurately and completely all of the information required to be maintained under paragraph 3.a.(2) of this section, except that full Social Security numbers and last known addresses, telephone numbers, and email addresses must not be included on weekly transmittals. Instead, the certified payrolls need only include an individually identifying number for each worker (e.g., the last four digits of the worker's Social Security number). The required weekly certified payroll information may be submitted using Optional Form WH-347 or in any other format desired. Optional Form WH-347 is available for this purpose from the Wage and Hour Division website at <https://www.dol.gov/sites/dolgov/files/WHDL/legacy/files/wh347.pdf> or its successor website. It is not a violation of this section for a prime contractor to require a subcontractor to provide full Social Security numbers and last known addresses, telephone numbers, and email addresses to the prime contractor for its own records, without weekly submission by the subcontractor to the contracting agency.

(3) Statement of Compliance. Each certified payroll submitted must be accompanied by a "Statement of Compliance," signed by the contractor or subcontractor, or the contractor's or subcontractor's agent who pays or supervises the payment of the persons working on the contract, and must certify the following:

(i) That the certified payroll for the payroll period contains the information required to be provided under paragraph 3.b. of this section, the appropriate information and basic records are being maintained under paragraph 3.a. of this section, and such information and records are correct and complete;

(ii) That each laborer or mechanic (including each helper and apprentice) working on the contract during the payroll period has been paid the full weekly wages earned, without rebate, either directly or indirectly, and that no deductions have been made either directly or indirectly from the full wages earned, other than permissible deductions as set forth in [29 CFR part 3](#); and

(iii) That each laborer or mechanic has been paid not less than the applicable wage rates and fringe benefits or cash equivalents for the classification(s) of work actually performed, as specified in the applicable wage determination incorporated into the contract.

(4) Use of Optional Form WH-347. The weekly submission of a properly executed certification set forth on the reverse side of Optional Form WH-347 will satisfy the requirement for submission of the "Statement of Compliance" required by paragraph 3.b.(3) of this section.

(5) *Signature.* The signature by the contractor, subcontractor, or the contractor's or subcontractor's agent must be an original handwritten signature or a legally valid electronic signature.

(6) *Falsification.* The falsification of any of the above certifications may subject the contractor or subcontractor to civil or criminal prosecution under [18 U.S.C. 1001](#) and [31 U.S.C. 3729](#).

(7) *Length of certified payroll retention.* The contractor or subcontractor must preserve all certified payrolls during the course of the work and for a period of 3 years after all the work on the prime contract is completed.

c. *Contracts, subcontracts, and related documents.* The contractor or subcontractor must maintain this contract or subcontract and related documents including, without limitation, bids, proposals, amendments, modifications, and extensions. The contractor or subcontractor must preserve these contracts, subcontracts, and related documents during the course of the work and for a period of 3 years after all the work on the prime contract is completed.

d. *Required disclosures and access (1) Required record disclosures and access to workers.* The contractor or subcontractor must make the records required under paragraphs 3.a. through 3.c. of this section, and any other documents that the contracting agency, the State DOT, the FHWA, or the Department of Labor deems necessary to determine compliance with the labor standards provisions of any of the applicable statutes referenced by § 5.1, available for inspection, copying, or transcription by authorized representatives of the contracting agency, the State DOT, the FHWA, or the Department of Labor, and must permit such representatives to interview workers during working hours on the job.

(2) *Sanctions for non-compliance with records and worker access requirements.* If the contractor or subcontractor fails to submit the required records or to make them available, or refuses to permit worker interviews during working hours on the job, the Federal agency may, after written notice to the contractor, sponsor, applicant, owner, or other entity, as the case may be, that maintains such records or that employs such workers, take such action as may be necessary to cause the suspension of any further payment, advance, or guarantee of funds. Furthermore, failure to submit the required records upon request or to make such records available, or to permit worker interviews during working hours on the job, may be grounds for debarment action pursuant to § 5.12. In addition, any contractor or other person that fails to submit the required records or make those records available to WHD within the time WHD requests that the records be produced will be precluded from introducing as evidence in an administrative proceeding under [29 CFR part 6](#) any of the required records that were not provided or made available to WHD. WHD will take into consideration a reasonable request from the contractor or person for an extension of the time for submission of records. WHD will determine the reasonableness of the request and may consider, among other things, the location of the records and the volume of production.

(3) *Required information disclosures.* Contractors and subcontractors must maintain the full Social Security number and last known address, telephone number, and email address

of each covered worker, and must provide them upon request to the contracting agency, the State DOT, the FHWA, the contractor, or the Wage and Hour Division of the Department of Labor for purposes of an investigation or other compliance action.

4. Apprentices and equal employment opportunity (29 CFR 5.5)

a. *Apprentices (1) Rate of pay.* Apprentices will be permitted to work at less than the predetermined rate for the work they perform when they are employed pursuant to and individually registered in a bona fide apprenticeship program registered with the U.S. Department of Labor, Employment and Training Administration, Office of Apprenticeship (OA), or with a State Apprenticeship Agency recognized by the OA. A person who is not individually registered in the program, but who has been certified by the OA or a State Apprenticeship Agency (where appropriate) to be eligible for probationary employment as an apprentice, will be permitted to work at less than the predetermined rate for the work they perform in the first 90 days of probationary employment as an apprentice in such a program. In the event the OA or a State Apprenticeship Agency recognized by the OA withdraws approval of an apprenticeship program, the contractor will no longer be permitted to use apprentices at less than the applicable predetermined rate for the work performed until an acceptable program is approved.

(2) *Fringe benefits.* Apprentices must be paid fringe benefits in accordance with the provisions of the apprenticeship program. If the apprenticeship program does not specify fringe benefits, apprentices must be paid the full amount of fringe benefits listed on the wage determination for the applicable classification. If the Administrator determines that a different practice prevails for the applicable apprentice classification, fringe benefits must be paid in accordance with that determination.

(3) *Apprenticeship ratio.* The allowable ratio of apprentices to journeymen on the job site in any craft classification must not be greater than the ratio permitted to the contractor as to the entire work force under the registered program or the ratio applicable to the locality of the project pursuant to paragraph 4.a.(4) of this section. Any worker listed on a payroll at an apprentice wage rate, who is not registered or otherwise employed as stated in paragraph 4.a.(1) of this section, must be paid not less than the applicable wage rate on the wage determination for the classification of work actually performed. In addition, any apprentice performing work on the job site in excess of the ratio permitted under this section must be paid not less than the applicable wage rate on the wage determination for the work actually performed.

(4) *Reciprocity of ratios and wage rates.* Where a contractor is performing construction on a project in a locality other than the locality in which its program is registered, the ratios and wage rates (expressed in percentages of the journeyworker's hourly rate) applicable within the locality in which the construction is being performed must be observed. If there is no applicable ratio or wage rate for the locality of the project, the ratio and wage rate specified in the contractor's registered program must be observed.

b. *Equal employment opportunity.* The use of apprentices and journeymen under this part must be in conformity with

the equal employment opportunity requirements of Executive Order 11246, as amended, and [29 CFR part 30](#).

c. Apprentices and Trainees (programs of the U.S. DOT).

Apprentices and trainees working under apprenticeship and skill training programs which have been certified by the Secretary of Transportation as promoting EEO in connection with Federal-aid highway construction programs are not subject to the requirements of paragraph 4 of this Section IV. 23 CFR 230.111(e)(2). The straight time hourly wage rates for apprentices and trainees under such programs will be established by the particular programs. The ratio of apprentices and trainees to journeyworkers shall not be greater than permitted by the terms of the particular program.

5. Compliance with Copeland Act requirements. The contractor shall comply with the requirements of 29 CFR part 3, which are incorporated by reference in this contract as provided in 29 CFR 5.5.

6. Subcontracts. The contractor or subcontractor must insert FHWA-1273 in any subcontracts, along with the applicable wage determination(s) and such other clauses or contract modifications as the contracting agency may by appropriate instructions require, and a clause requiring the subcontractors to include these clauses and wage determination(s) in any lower tier subcontracts. The prime contractor is responsible for the compliance by any subcontractor or lower tier subcontractor with all the contract clauses in this section. In the event of any violations of these clauses, the prime contractor and any subcontractor(s) responsible will be liable for any unpaid wages and monetary relief, including interest from the date of the underpayment or loss, due to any workers of lower-tier subcontractors, and may be subject to debarment, as appropriate. 29 CFR 5.5.

7. Contract termination: debarment. A breach of the contract clauses in 29 CFR 5.5 may be grounds for termination of the contract, and for debarment as a contractor and a subcontractor as provided in 29 CFR 5.12.

8. Compliance with Davis-Bacon and Related Act requirements. All rulings and interpretations of the Davis-Bacon and Related Acts contained in 29 CFR parts 1, 3, and 5 are herein incorporated by reference in this contract as provided in 29 CFR 5.5.

9. Disputes concerning labor standards. As provided in 29 CFR 5.5, disputes arising out of the labor standards provisions of this contract shall not be subject to the general disputes clause of this contract. Such disputes shall be resolved in accordance with the procedures of the Department of Labor set forth in 29 CFR parts 5, 6, and 7. Disputes within the meaning of this clause include disputes between the contractor (or any of its subcontractors) and the contracting agency, the U.S. Department of Labor, or the employees or their representatives.

10. Certification of eligibility. a. By entering into this contract, the contractor certifies that neither it nor any person or firm who has an interest in the contractor's firm is a person or firm ineligible to be awarded Government contracts by virtue of [40 U.S.C. 3144\(b\)](#) or § 5.12(a).

b. No part of this contract shall be subcontracted to any person or firm ineligible for award of a Government contract by virtue of [40 U.S.C. 3144\(b\)](#) or § 5.12(a).

c. The penalty for making false statements is prescribed in the U.S. Code, Title 18 Crimes and Criminal Procedure, [18 U.S.C. 1001](#).

11. Anti-retaliation. It is unlawful for any person to discharge, demote, intimidate, threaten, restrain, coerce, blacklist, harass, or in any other manner discriminate against, or to cause any person to discharge, demote, intimidate, threaten, restrain, coerce, blacklist, harass, or in any other manner discriminate against, any worker or job applicant for:

a. Notifying any contractor of any conduct which the worker reasonably believes constitutes a violation of the DBA, Related Acts, this part, or [29 CFR part 1](#) or [3](#);

b. Filing any complaint, initiating or causing to be initiated any proceeding, or otherwise asserting or seeking to assert on behalf of themselves or others any right or protection under the DBA, Related Acts, this part, or [29 CFR part 1](#) or [3](#);

c. Cooperating in any investigation or other compliance action, or testifying in any proceeding under the DBA, Related Acts, this part, or [29 CFR part 1](#) or [3](#); or

d. Informing any other person about their rights under the DBA, Related Acts, this part, or [29 CFR part 1](#) or [3](#).

V. CONTRACT WORK HOURS AND SAFETY STANDARDS ACT

Pursuant to 29 CFR 5.5(b), the following clauses apply to any Federal-aid construction contract in an amount in excess of \$100,000 and subject to the overtime provisions of the Contract Work Hours and Safety Standards Act. These clauses shall be inserted in addition to the clauses required by 29 CFR 5.5(a) or 29 CFR 4.6. As used in this paragraph, the terms laborers and mechanics include watchpersons and guards.

1. Overtime requirements. No contractor or subcontractor contracting for any part of the contract work which may require or involve the employment of laborers or mechanics shall require or permit any such laborer or mechanic in any workweek in which he or she is employed on such work to work in excess of forty hours in such workweek unless such laborer or mechanic receives compensation at a rate not less than one and one-half times the basic rate of pay for all hours worked in excess of forty hours in such workweek. 29 CFR 5.5.

2. Violation; liability for unpaid wages; liquidated damages. In the event of any violation of the clause set forth in paragraph 1. of this section the contractor and any subcontractor responsible therefor shall be liable for the unpaid wages and interest from the date of the underpayment. In addition, such contractor and subcontractor shall be liable to the United States (in the case of work done under contract for the District of Columbia or a territory, to such District or to such territory), for liquidated damages. Such liquidated damages shall be computed with respect to each individual laborer or

mechanic, including watchpersons and guards, employed in violation of the clause set forth in paragraph 1. of this section, in the sum currently provided in 29 CFR 5.5(b)(2)* for each calendar day on which such individual was required or permitted to work in excess of the standard workweek of forty hours without payment of the overtime wages required by the clause set forth in paragraph 1. of this section.

* \$31 as of January 15, 2023 (See 88 FR 88 FR 2210) as may be adjusted annually by the Department of Labor, pursuant to the Federal Civil Penalties Inflation Adjustment Act of 1990.

3. Withholding for unpaid wages and liquidated damages

a. *Withholding process.* The FHWA or the contracting agency may, upon its own action, or must, upon written request of an authorized representative of the Department of Labor, withhold or cause to be withheld from the contractor so much of the accrued payments or advances as may be considered necessary to satisfy the liabilities of the prime contractor or any subcontractor for any unpaid wages; monetary relief, including interest; and liquidated damages required by the clauses set forth in this section on this contract, any other Federal contract with the same prime contractor, or any other federally assisted contract subject to the Contract Work Hours and Safety Standards Act that is held by the same prime contractor (as defined in § 5.2). The necessary funds may be withheld from the contractor under this contract, any other Federal contract with the same prime contractor, or any other federally assisted contract that is subject to the Contract Work Hours and Safety Standards Act and is held by the same prime contractor, regardless of whether the other contract was awarded or assisted by the same agency, and such funds may be used to satisfy the contractor liability for which the funds were withheld.

b. *Priority to withheld funds.* The Department has priority to funds withheld or to be withheld in accordance with Section IV paragraph 2.a. or paragraph 3.a. of this section, or both, over claims to those funds by:

- (1) A contractor's surety(ies), including without limitation performance bond sureties and payment bond sureties;
- (2) A contracting agency for its repurchase costs;
- (3) A trustee(s) (either a court-appointed trustee or a U.S. trustee, or both) in bankruptcy of a contractor, or a contractor's bankruptcy estate;
- (4) A contractor's assignee(s);
- (5) A contractor's successor(s); or
- (6) A claim asserted under the Prompt Payment Act, [31 U.S.C. 3901](#)–3907.

4. Subcontracts. The contractor or subcontractor must insert in any subcontracts the clauses set forth in paragraphs 1. through 5. of this section and a clause requiring the subcontractors to include these clauses in any lower tier subcontracts. The prime contractor is responsible for compliance by any subcontractor or lower tier subcontractor with the clauses set forth in paragraphs 1. through 5. In the

event of any violations of these clauses, the prime contractor and any subcontractor(s) responsible will be liable for any unpaid wages and monetary relief, including interest from the date of the underpayment or loss, due to any workers of lower-tier subcontractors, and associated liquidated damages and may be subject to debarment, as appropriate.

5. Anti-retaliation. It is unlawful for any person to discharge, demote, intimidate, threaten, restrain, coerce, blacklist, harass, or in any other manner discriminate against, or to cause any person to discharge, demote, intimidate, threaten, restrain, coerce, blacklist, harass, or in any other manner discriminate against, any worker or job applicant for:

a. Notifying any contractor of any conduct which the worker reasonably believes constitutes a violation of the Contract Work Hours and Safety Standards Act (CWHSSA) or its implementing regulations in this part;

b. Filing any complaint, initiating or causing to be initiated any proceeding, or otherwise asserting or seeking to assert on behalf of themselves or others any right or protection under CWHSSA or this part;

c. Cooperating in any investigation or other compliance action, or testifying in any proceeding under CWHSSA or this part; or

d. Informing any other person about their rights under CWHSSA or this part.

VI. SUBLETTING OR ASSIGNING THE CONTRACT

This provision is applicable to all Federal-aid construction contracts on the National Highway System pursuant to 23 CFR 635.116.

1. The contractor shall perform with its own organization contract work amounting to not less than 30 percent (or a greater percentage if specified elsewhere in the contract) of the total original contract price, excluding any specialty items designated by the contracting agency. Specialty items may be performed by subcontract and the amount of any such specialty items performed may be deducted from the total original contract price before computing the amount of work required to be performed by the contractor's own organization (23 CFR 635.116).

a. The term "perform work with its own organization" in paragraph 1 of Section VI refers to workers employed or leased by the prime contractor, and equipment owned or rented by the prime contractor, with or without operators. Such term does not include employees or equipment of a subcontractor or lower tier subcontractor, agents of the prime contractor, or any other assignees. The term may include payments for the costs of hiring leased employees from an employee leasing firm meeting all relevant Federal and State regulatory requirements. Leased employees may only be included in this term if the prime contractor meets all of the following conditions: (based on longstanding interpretation)

(1) the prime contractor maintains control over the supervision of the day-to-day activities of the leased employees;

(2) the prime contractor remains responsible for the quality of the work of the leased employees;

- (3) the prime contractor retains all power to accept or exclude individual employees from work on the project; and
- (4) the prime contractor remains ultimately responsible for the payment of predetermined minimum wages, the submission of payrolls, statements of compliance and all other Federal regulatory requirements.

b. "Specialty Items" shall be construed to be limited to work that requires highly specialized knowledge, abilities, or equipment not ordinarily available in the type of contracting organizations qualified and expected to bid or propose on the contract as a whole and in general are to be limited to minor components of the overall contract. 23 CFR 635.102.

2. Pursuant to 23 CFR 635.116(a), the contract amount upon which the requirements set forth in paragraph (1) of Section VI is computed includes the cost of material and manufactured products which are to be purchased or produced by the contractor under the contract provisions.

3. Pursuant to 23 CFR 635.116(c), the contractor shall furnish (a) a competent superintendent or supervisor who is employed by the firm, has full authority to direct performance of the work in accordance with the contract requirements, and is in charge of all construction operations (regardless of who performs the work) and (b) such other of its own organizational resources (supervision, management, and engineering services) as the contracting officer determines is necessary to assure the performance of the contract.

4. No portion of the contract shall be sublet, assigned or otherwise disposed of except with the written consent of the contracting officer, or authorized representative, and such consent when given shall not be construed to relieve the contractor of any responsibility for the fulfillment of the contract. Written consent will be given only after the contracting agency has assured that each subcontract is evidenced in writing and that it contains all pertinent provisions and requirements of the prime contract. (based on long-standing interpretation of 23 CFR 635.116).

5. The 30-percent self-performance requirement of paragraph (1) is not applicable to design-build contracts; however, contracting agencies may establish their own self-performance requirements. 23 CFR 635.116(d).

VII. SAFETY: ACCIDENT PREVENTION

This provision is applicable to all Federal-aid construction contracts and to all related subcontracts.

1. In the performance of this contract the contractor shall comply with all applicable Federal, State, and local laws governing safety, health, and sanitation (23 CFR Part 635). The contractor shall provide all safeguards, safety devices and protective equipment and take any other needed actions as it determines, or as the contracting officer may determine, to be reasonably necessary to protect the life and health of employees on the job and the safety of the public and to protect property in connection with the performance of the work covered by the contract. 23 CFR 635.108.

2. It is a condition of this contract, and shall be made a condition of each subcontract, which the contractor enters into pursuant to this contract, that the contractor and any subcontractor shall not permit any employee, in performance of the contract, to work in surroundings or under conditions which are unsanitary, hazardous or dangerous to his/her health or safety, as determined under construction safety and

health standards (29 CFR Part 1926) promulgated by the Secretary of Labor, in accordance with Section 107 of the Contract Work Hours and Safety Standards Act (40 U.S.C. 3704). 29 CFR 1926.10.

3. Pursuant to 29 CFR 1926.3, it is a condition of this contract that the Secretary of Labor or authorized representative thereof, shall have right of entry to any site of contract performance to inspect or investigate the matter of compliance with the construction safety and health standards and to carry out the duties of the Secretary under Section 107 of the Contract Work Hours and Safety Standards Act (40 U.S.C. 3704).

VIII. FALSE STATEMENTS CONCERNING HIGHWAY PROJECTS

This provision is applicable to all Federal-aid construction contracts and to all related subcontracts.

In order to assure high quality and durable construction in conformity with approved plans and specifications and a high degree of reliability on statements and representations made by engineers, contractors, suppliers, and workers on Federal-aid highway projects, it is essential that all persons concerned with the project perform their functions as carefully, thoroughly, and honestly as possible. Willful falsification, distortion, or misrepresentation with respect to any facts related to the project is a violation of Federal law. To prevent any misunderstanding regarding the seriousness of these and similar acts, Form FHWA-1022 shall be posted on each Federal-aid highway project (23 CFR Part 635) in one or more places where it is readily available to all persons concerned with the project:

18 U.S.C. 1020 reads as follows:

"Whoever, being an officer, agent, or employee of the United States, or of any State or Territory, or whoever, whether a person, association, firm, or corporation, knowingly makes any false statement, false representation, or false report as to the character, quality, quantity, or cost of the material used or to be used, or the quantity or quality of the work performed or to be performed, or the cost thereof in connection with the submission of plans, maps, specifications, contracts, or costs of construction on any highway or related project submitted for approval to the Secretary of Transportation; or

Whoever knowingly makes any false statement, false representation, false report or false claim with respect to the character, quality, quantity, or cost of any work performed or to be performed, or materials furnished or to be furnished, in connection with the construction of any highway or related project approved by the Secretary of Transportation; or

Whoever knowingly makes any false statement or false representation as to material fact in any statement, certificate, or report submitted pursuant to provisions of the Federal-aid Roads Act approved July 11, 1916, (39 Stat. 355), as amended and supplemented;

Shall be fined under this title or imprisoned not more than 5 years or both."

IX. IMPLEMENTATION OF CLEAN AIR ACT AND FEDERAL WATER POLLUTION CONTROL ACT (42 U.S.C. 7606; 2 CFR 200.88; EO 11738)

This provision is applicable to all Federal-aid construction contracts in excess of \$150,000 and to all related subcontracts. 48 CFR 2.101; 2 CFR 200.327.

By submission of this bid/proposal or the execution of this contract or subcontract, as appropriate, the bidder, proposer, Federal-aid construction contractor, subcontractor, supplier, or vendor agrees to comply with all applicable standards, orders or regulations issued pursuant to the Clean Air Act (42 U.S.C. 7401-7671q) and the Federal Water Pollution Control Act, as amended (33 U.S.C. 1251-1387). Violations must be reported to the Federal Highway Administration and the Regional Office of the Environmental Protection Agency. 2 CFR Part 200, Appendix II.

The contractor agrees to include or cause to be included the requirements of this Section in every subcontract, and further agrees to take such action as the contracting agency may direct as a means of enforcing such requirements. 2 CFR 200.327.

X. CERTIFICATION REGARDING DEBARMENT, SUSPENSION, INELIGIBILITY AND VOLUNTARY EXCLUSION

This provision is applicable to all Federal-aid construction contracts, design-build contracts, subcontracts, lower-tier subcontracts, purchase orders, lease agreements, consultant contracts or any other covered transaction requiring FHWA approval or that is estimated to cost \$25,000 or more – as defined in 2 CFR Parts 180 and 1200. 2 CFR 180.220 and 1200.220.

1. Instructions for Certification – First Tier Participants:

a. By signing and submitting this proposal, the prospective first tier participant is providing the certification set out below.

b. The inability of a person to provide the certification set out below will not necessarily result in denial of participation in this covered transaction. The prospective first tier participant shall submit an explanation of why it cannot provide the certification set out below. The certification or explanation will be considered in connection with the department or agency's determination whether to enter into this transaction. However, failure of the prospective first tier participant to furnish a certification or an explanation shall disqualify such a person from participation in this transaction. 2 CFR 180.320.

c. The certification in this clause is a material representation of fact upon which reliance was placed when the contracting agency determined to enter into this transaction. If it is later determined that the prospective participant knowingly rendered an erroneous certification, in addition to other remedies available to the Federal Government, the contracting agency may terminate this transaction for cause of default. 2 CFR 180.325.

d. The prospective first tier participant shall provide immediate written notice to the contracting agency to whom this proposal is submitted if any time the prospective first tier participant learns that its certification was erroneous when submitted or has become erroneous by reason of changed circumstances. 2 CFR 180.345 and 180.350.

e. The terms "covered transaction," "debarred," "suspended," "ineligible," "participant," "person," "principal," and "voluntarily excluded," as used in this clause, are defined in 2 CFR Parts 180, Subpart I, 180.900-180.1020, and 1200. "First Tier Covered Transactions" refers to any covered transaction between a recipient or subrecipient of Federal funds and a participant (such as the prime or general contract). "Lower Tier Covered Transactions" refers to any covered transaction under a First Tier Covered Transaction (such as subcontracts). "First Tier Participant" refers to the participant who has entered into a covered transaction with a recipient or subrecipient of Federal funds (such as the prime or general contractor). "Lower Tier Participant" refers any participant who has entered into a covered transaction with a First Tier Participant or other Lower Tier Participants (such as subcontractors and suppliers).

f. The prospective first tier participant agrees by submitting this proposal that, should the proposed covered transaction be entered into, it shall not knowingly enter into any lower tier covered transaction with a person who is debarred, suspended, declared ineligible, or voluntarily excluded from participation in this covered transaction, unless authorized by the department or agency entering into this transaction. 2 CFR 180.330.

g. The prospective first tier participant further agrees by submitting this proposal that it will include the clause titled "Certification Regarding Debarment, Suspension, Ineligibility and Voluntary Exclusion-Lower Tier Covered Transactions," provided by the department or contracting agency, entering into this covered transaction, without modification, in all lower tier covered transactions and in all solicitations for lower tier covered transactions exceeding the \$25,000 threshold. 2 CFR 180.220 and 180.300.

h. A participant in a covered transaction may rely upon a certification of a prospective participant in a lower tier covered transaction that is not debarred, suspended, ineligible, or voluntarily excluded from the covered transaction, unless it knows that the certification is erroneous. 2 CFR 180.300; 180.320, and 180.325. A participant is responsible for ensuring that its principals are not suspended, debarred, or otherwise ineligible to participate in covered transactions. 2 CFR 180.335. To verify the eligibility of its principals, as well as the eligibility of any lower tier prospective participants, each participant may, but is not required to, check the System for Award Management website (<https://www.sam.gov/>). 2 CFR 180.300, 180.320, and 180.325.

i. Nothing contained in the foregoing shall be construed to require the establishment of a system of records in order to render in good faith the certification required by this clause. The knowledge and information of the prospective participant is not required to exceed that which is normally possessed by a prudent person in the ordinary course of business dealings.

j. Except for transactions authorized under paragraph (f) of these instructions, if a participant in a covered transaction knowingly enters into a lower tier covered transaction with a person who is suspended, debarred, ineligible, or voluntarily excluded from participation in this transaction, in addition to other remedies available to the Federal Government, the department or agency may terminate this transaction for cause or default. 2 CFR 180.325.

2. Certification Regarding Debarment, Suspension, Ineligibility and Voluntary Exclusion – First Tier Participants:

- a. The prospective first tier participant certifies to the best of its knowledge and belief, that it and its principals:

(1) Are not presently debarred, suspended, proposed for debarment, declared ineligible, or voluntarily excluded from participating in covered transactions by any Federal department or agency, 2 CFR 180.335;.

(2) Have not within a three-year period preceding this proposal been convicted of or had a civil judgment rendered against them for commission of fraud or a criminal offense in connection with obtaining, attempting to obtain, or performing a public (Federal, State, or local) transaction or contract under a public transaction; violation of Federal or State antitrust statutes or commission of embezzlement, theft, forgery, bribery, falsification or destruction of records, making false statements, or receiving stolen property, 2 CFR 180.800;

(3) Are not presently indicted for or otherwise criminally or civilly charged by a governmental entity (Federal, State or local) with commission of any of the offenses enumerated in paragraph (a)(2) of this certification, 2 CFR 180.700 and 180.800; and

(4) Have not within a three-year period preceding this application/proposal had one or more public transactions (Federal, State or local) terminated for cause or default. 2 CFR 180.335(d).

(5) Are not a corporation that has been convicted of a felony violation under any Federal law within the two-year period preceding this proposal (USDOT Order 4200.6 implementing appropriations act requirements); and

(6) Are not a corporation with any unpaid Federal tax liability that has been assessed, for which all judicial and administrative remedies have been exhausted, or have lapsed, and that is not being paid in a timely manner pursuant to an agreement with the authority responsible for collecting the tax liability (USDOT Order 4200.6 implementing appropriations act requirements).

- b. Where the prospective participant is unable to certify to any of the statements in this certification, such prospective participant should attach an explanation to this proposal. 2 CFR 180.335 and 180.340.

* * * * *

3. Instructions for Certification - Lower Tier Participants:

(Applicable to all subcontracts, purchase orders, and other lower tier transactions requiring prior FHWA approval or estimated to cost \$25,000 or more - 2 CFR Parts 180 and 1200). 2 CFR 180.220 and 1200.220.

- a. By signing and submitting this proposal, the prospective lower tier participant is providing the certification set out below.

b. The certification in this clause is a material representation of fact upon which reliance was placed when this transaction was entered into. If it is later determined that the prospective lower tier participant knowingly rendered an erroneous certification, in addition to other remedies available to the Federal Government, the department, or agency with which

this transaction originated may pursue available remedies, including suspension and/or debarment.

- c. The prospective lower tier participant shall provide immediate written notice to the person to which this proposal is submitted if at any time the prospective lower tier participant learns that its certification was erroneous by reason of changed circumstances. 2 CFR 180.365.

d. The terms "covered transaction," "debarred," "suspended," "ineligible," "participant," "person," "principal," and "voluntarily excluded," as used in this clause, are defined in 2 CFR Parts 180, Subpart I, 180.900 – 180.1020, and 1200. You may contact the person to which this proposal is submitted for assistance in obtaining a copy of those regulations. "First Tier Covered Transactions" refers to any covered transaction between a recipient or subrecipient of Federal funds and a participant (such as the prime or general contractor). "Lower Tier Covered Transactions" refers to any covered transaction under a First Tier Covered Transaction (such as subcontracts). "First Tier Participant" refers to the participant who has entered into a covered transaction with a recipient or subrecipient of Federal funds (such as the prime or general contractor). "Lower Tier Participant" refers any participant who has entered into a covered transaction with a First Tier Participant or other Lower Tier Participants (such as subcontractors and suppliers).

- e. The prospective lower tier participant agrees by submitting this proposal that, should the proposed covered transaction be entered into, it shall not knowingly enter into any lower tier covered transaction with a person who is debarred, suspended, declared ineligible, or voluntarily excluded from participation in this covered transaction, unless authorized by the department or agency with which this transaction originated. 2 CFR 1200.220 and 1200.332.

f. The prospective lower tier participant further agrees by submitting this proposal that it will include this clause titled "Certification Regarding Debarment, Suspension, Ineligibility and Voluntary Exclusion-Lower Tier Covered Transaction," without modification, in all lower tier covered transactions and in all solicitations for lower tier covered transactions exceeding the \$25,000 threshold. 2 CFR 180.220 and 1200.220.

g. A participant in a covered transaction may rely upon a certification of a prospective participant in a lower tier covered transaction that is not debarred, suspended, ineligible, or voluntarily excluded from the covered transaction, unless it knows that the certification is erroneous. A participant is responsible for ensuring that its principals are not suspended, debarred, or otherwise ineligible to participate in covered transactions. To verify the eligibility of its principals, as well as the eligibility of any lower tier prospective participants, each participant may, but is not required to, check the System for Award Management website (<https://www.sam.gov>), which is compiled by the General Services Administration. 2 CFR 180.300, 180.320, 180.330, and 180.335.

h. Nothing contained in the foregoing shall be construed to require establishment of a system of records in order to render in good faith the certification required by this clause. The knowledge and information of participant is not required to exceed that which is normally possessed by a prudent person in the ordinary course of business dealings.

- i. Except for transactions authorized under paragraph e of these instructions, if a participant in a covered transaction knowingly enters into a lower tier covered transaction with a person who is suspended, debarred, ineligible, or voluntarily

excluded from participation in this transaction, in addition to other remedies available to the Federal Government, the department or agency with which this transaction originated may pursue available remedies, including suspension and/or debarment. 2 CFR 180.325.

* * * * *

4. Certification Regarding Debarment, Suspension, Ineligibility and Voluntary Exclusion--Lower Tier Participants:

a. The prospective lower tier participant certifies, by submission of this proposal, that neither it nor its principals:

(1) is presently debarred, suspended, proposed for debarment, declared ineligible, or voluntarily excluded from participating in covered transactions by any Federal department or agency, 2 CFR 180.355;

(2) is a corporation that has been convicted of a felony violation under any Federal law within the two-year period preceding this proposal (USDOT Order 4200.6 implementing appropriations act requirements); and

(3) is a corporation with any unpaid Federal tax liability that has been assessed, for which all judicial and administrative remedies have been exhausted, or have lapsed, and that is not being paid in a timely manner pursuant to an agreement with the authority responsible for collecting the tax liability. (USDOT Order 4200.6 implementing appropriations act requirements)

b. Where the prospective lower tier participant is unable to certify to any of the statements in this certification, such prospective participant should attach an explanation to this proposal.

* * * * *

XI. CERTIFICATION REGARDING USE OF CONTRACT FUNDS FOR LOBBYING

This provision is applicable to all Federal-aid construction contracts and to all related subcontracts which exceed \$100,000. 49 CFR Part 20, App. A.

1. The prospective participant certifies, by signing and submitting this bid or proposal, to the best of his or her knowledge and belief, that:

a. No Federal appropriated funds have been paid or will be paid, by or on behalf of the undersigned, to any person for influencing or attempting to influence an officer or employee of any Federal agency, a Member of Congress, an officer or employee of Congress, or an employee of a Member of Congress in connection with the awarding of any Federal contract, the making of any Federal grant, the making of any Federal loan, the entering into of any cooperative agreement, and the extension, continuation, renewal, amendment, or modification of any Federal contract, grant, loan, or cooperative agreement.

b. If any funds other than Federal appropriated funds have been paid or will be paid to any person for influencing or attempting to influence an officer or employee of any Federal agency, a Member of Congress, an officer or employee of Congress, or an employee of a Member of Congress in connection with this Federal contract, grant, loan, or

cooperative agreement, the undersigned shall complete and submit Standard Form-LLL, "Disclosure Form to Report Lobbying," in accordance with its instructions.

2. This certification is a material representation of fact upon which reliance was placed when this transaction was made or entered into. Submission of this certification is a prerequisite for making or entering into this transaction imposed by 31 U.S.C. 1352. Any person who fails to file the required certification shall be subject to a civil penalty of not less than \$10,000 and not more than \$100,000 for each such failure.

3. The prospective participant also agrees by submitting its bid or proposal that the participant shall require that the language of this certification be included in all lower tier subcontracts, which exceed \$100,000 and that all such recipients shall certify and disclose accordingly.

XII. USE OF UNITED STATES-FLAG VESSELS:

This provision is applicable to all Federal-aid construction contracts, design-build contracts, subcontracts, lower-tier subcontracts, purchase orders, lease agreements, or any other covered transaction. 46 CFR Part 381.

This requirement applies to material or equipment that is acquired for a specific Federal-aid highway project. 46 CFR 381.7. It is not applicable to goods or materials that come into inventories independent of an FHWA funded-contract.

When oceanic shipments (or shipments across the Great Lakes) are necessary for materials or equipment acquired for a specific Federal-aid construction project, the bidder, proposer, contractor, subcontractor, or vendor agrees:

1. To utilize privately owned United States-flag commercial vessels to ship at least 50 percent of the gross tonnage (computed separately for dry bulk carriers, dry cargo liners, and tankers) involved, whenever shipping any equipment, material, or commodities pursuant to this contract, to the extent such vessels are available at fair and reasonable rates for United States-flag commercial vessels. 46 CFR 381.7.

2. To furnish within 20 days following the date of loading for shipments originating within the United States or within 30 working days following the date of loading for shipments originating outside the United States, a legible copy of a rated, 'on-board' commercial ocean bill-of-lading in English for each shipment of cargo described in paragraph (b)(1) of this section to both the Contracting Officer (through the prime contractor in the case of subcontractor bills-of-lading) and to the Office of Cargo and Commercial Sealift (MAR-620), Maritime Administration, Washington, DC 20590. (MARAD requires copies of the ocean carrier's (master) bills of lading, certified onboard, dated, with rates and charges. These bills of lading may contain business sensitive information and therefore may be submitted directly to MARAD by the Ocean Transportation Intermediary on behalf of the contractor). 46 CFR 381.7.

**ATTACHMENT A - EMPLOYMENT AND MATERIALS
PREFERENCE FOR APPALACHIAN DEVELOPMENT HIGHWAY
SYSTEM OR APPALACHIAN LOCAL ACCESS**

ROAD CONTRACTS (23 CFR 633, Subpart B, Appendix B)
This provision is applicable to all Federal-aid projects funded under the Appalachian Regional Development Act of 1965.

1. During the performance of this contract, the contractor undertaking to do work which is, or reasonably may be, done as on-site work, shall give preference to qualified persons who regularly reside in the labor area as designated by the DOL wherein the contract work is situated, or the subregion, or the Appalachian counties of the State wherein the contract work is situated, except:

a. To the extent that qualified persons regularly residing in the area are not available.

b. For the reasonable needs of the contractor to employ supervisory or specially experienced personnel necessary to assure an efficient execution of the contract work.

c. For the obligation of the contractor to offer employment to present or former employees as the result of a lawful collective bargaining contract, provided that the number of nonresident persons employed under this subparagraph (1c) shall not exceed 20 percent of the total number of employees employed by the contractor on the contract work, except as provided in subparagraph (4) below.

2. The contractor shall place a job order with the State Employment Service indicating (a) the classifications of the laborers, mechanics and other employees required to perform the contract work, (b) the number of employees required in each classification, (c) the date on which the participant estimates such employees will be required, and (d) any other pertinent information required by the State Employment Service to complete the job order form. The job order may be placed with the State Employment Service in writing or by telephone. If during the course of the contract work, the information submitted by the contractor in the original job order is substantially modified, the participant shall promptly notify the State Employment Service.

3. The contractor shall give full consideration to all qualified job applicants referred to him by the State Employment Service. The contractor is not required to grant employment to any job applicants who, in his opinion, are not qualified to perform the classification of work required.

4. If, within one week following the placing of a job order by the contractor with the State Employment Service, the State Employment Service is unable to refer any qualified job applicants to the contractor, or less than the number requested, the State Employment Service will forward a certificate to the contractor indicating the unavailability of applicants. Such certificate shall be made a part of the contractor's permanent project records. Upon receipt of this certificate, the contractor may employ persons who do not normally reside in the labor area to fill positions covered by the certificate, notwithstanding the provisions of subparagraph (1c) above.

5. The provisions of 23 CFR 633.207(e) allow the contracting agency to provide a contractual preference for the use of mineral resource materials native to the Appalachian region.

6. The contractor shall include the provisions of Sections 1 through 4 of this Attachment A in every subcontract for work which is, or reasonably may be, done as on-site work.