
**STRUCTURE GEOTECHNICAL REPORT
PEDESTRIAN BRIDGE AND TRAIL
OVER IL ROUTE 59
VILLAGE OF STREAMWOOD
COOK COUNTY, ILLINOIS**

**For
TranSystems
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11. Abstract			
<p>A new pedestrian bridge and trail (boardwalk) over IL Route 59 is being proposed. The proposed structures will have a total length of about 2,245 feet and it will be supported by 87 piers and two abutments. This report provides geotechnical recommendations for the design of the proposed bridge and boardwalk piers shown in the preliminary general plan drawing.</p> <p>Beneath the 4- to 36-inch thick topsoil, the general lithologic profile includes up to 10 feet of cohesive and granular fill or up to 10 feet of very soft to stiff silty clay to silty clay loam. Most of soil borings drilled west of IL 59 and a couple of borings drilled on the east of IL 59 revealed up to 35 feet of peat and organic soils. Deeper foundation soils include stiff to hard silty clay followed by medium dense to dense sand to sand and gravel to boring termination depths. Groundwater was at 0 to 30 feet below ground surface or at elevations of 746 to 774.0 feet. The sites classify in the Seismic Class D.</p> <p>The proposed piers can be supported by drilled shafts installed into stiff to hard clay or medium dense to dense sand to sandy gravel at elevations ranging from 709 to 759 feet. Geotechnical parameters for shafts analyses under axial and lateral loads are provided. Casing will be required within the areas where peat and soft soils are present. Hard drilling conditions due to the presence of possible cobbles are anticipated at depths ranging from 5 to 73 feet below ground surface or at elevations of 701 to 769 feet.</p>			
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**STRUCTURE GEOTECHNICAL REPORT
PEDESTRIAN BRIDGE AND TRAIL
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TRANSYSTEMS**

1.0 INTRODUCTION

This report presents the results of Wang Engineering, Inc.'s (Wang) subsurface investigation, laboratory testing, geotechnical evaluations, and recommendations for the proposed design and construction of a Pedestrian Bridge crossing over IL Route 59 (IL 59) and associated trail (boardwalk) in the Village of Streamwood, Cook County, Illinois. A *Site Location Map* is presented as Exhibit 1.

The proposed structures will be constructed to connect the existing trail over IL 59. This report presents our subsurface investigation, engineering analyses, and recommendations for the design and construction of the proposed bridge and trails.

1.1 Proposed Structure

Based on the draft General Plan and Elevation (GPE) drawings provided on August 10, 2021, prepared by TranSystems Corporation (TranSystems), Wang understands that the proposed bridge and trail will be a multi-span structure with 87 piers and two abutments. The bridge and trail will have a total length of about 2,244.67 feet, ranging from Station 100+13.33 to Station 122+58.00. Draft GPE drawings are included as Appendix C. The three main sections of the structures are as follows:

- **West Boardwalk Approach**

This section consists of 33 spans boardwalk starting from west abutment at Station 100+13.33 and continues from Pier #1 through Pier #33 at Station 106+68.85. The lengths of spans vary from 13'-10.9" to 20'-0". The boardwalk will be 15.25-foot wide and will be supported by drilled shafts.

- **Bridge Truss Spans**

The proposed bridge truss will have five spans. The structure will be 14'-0" clear wide and 627'-0" long starting from Pier #33 at Station 106+68.55 to Pier #38 at Station 112+95.85. The lengths of spans vary from 105'-0" to 207'-0" with the span over IL 59 being the longest span.

- **East Boardwalk Approach**

This section consists of 50 spans boardwalk starting from Pier #38 Station 112+95.85 through Pier #87 and east abutment at Station 122+58.00. Spans lengths range from 12'-2.6" to 20'-0". The boardwalk will be 15.25-foot wide and will be supported by drilled shafts.

The purpose of this investigation was to characterize the site soil and groundwater conditions, perform geotechnical analyses, and provide recommendations for the design and construction of the proposed bridge foundations and associated trails.

2.0 GEOLOGICAL SETTING

2.1 Physiography

Northwestern Cook County is part of the Wheaton Morainal Country within the Till Plains Physiographic Section (Leighton et al. 1948), which is characterized hummocky morainal uplands containing numerous kettles and man-made lakes (Stumpf 2007). The project site is situated at the limit between Valparaiso Morainic System and West Chicago Moraine, along an unnamed tributary to the Poplar Creek. This tributary flows through a hummocky topography that favored an irregular floodplain with low lands and mashes. Elevations across the proposed alignment range from 774 to 783 feet. Exhibit 2 illustrates the site geological map.

2.2 Site Pedological Features

After the Wisconsin glaciation, several types of soils developed through weathering of glacial sediments. In Cook County, the soil types were surveyed by the United States Department of Agriculture (2014). Summaries of the USDA soil types along the project area, including their relevant geotechnical index properties and suitability as subgrade and road fill, are shown in the Site Pedological Map and Data (Exhibit 2). The soil information provided by USDA is meant to be used as a general reference in the absence of a site-specific investigation. In this instance, our findings regarding soil features affecting suitability for proposed construction are mainly consistent with the information presented in Exhibit 3.

The pedological data show peat and organic soils along with fat clays make about 80% of the subsurface soil within the project limits. These soils are characterized by high shrink-swell potential, moderate to high organic matter content, high frost action, high ponding frequency, with slow to very slow infiltration rate.

2.3 Surficial Cover

The surficial cover is mainly the result of Wisconsin-age glacial activity (Hansel and Johnson, 1996). The glacial deposits were emplaced during pulsating advances and retreats of an icesheet lobe responsible for the formation of end moraines and associated low-relief till and lake plains. Kettle depressions and low-lying areas are filled with up to 15 feet of fine, sorted sediment of the Equality Formation, and peat and marl of the Grayslake Peat that veneers most of the project area. The project area is dominated by the Wadsworth Formation clayey diamicton and silty loamy diamicton and sorted sediments associated with the Haeger Member of the Lemont Formation. They are intercalated with lenses and layers of sorted sand and gravel outwash deposits of the Henry Formation, overlying thick deposits of clay loam diamicton of the Tiskilwa Formation. Multiple advances and retreats of the ice front account for the layers in the moraine (Hansel and Johnson 1996). The drift thickness along the project is about 125 feet (Leetaru et al. 2004).

2.4 Bedrock

In northwestern Cook County the surficial cover rests unconformably on top of Silurian-age dolostone. The top of the bedrock lies approximately 125 feet below the ground surface (bgs). The top of the bedrock lies at about 650 feet elevation (National Geodetic Vertical Datum) (Leetaru et al. 2004).

Our subsurface investigation results fit into the local geologic context. The borings drilled in the project area revealed that the native sediments consist of lacustrine clay to silty clay loam of the Equality Formation containing isolated areas of organic matter, peat and organic soil of the Grayslake Peat, silty clay to silty clay loam diamicton of the Wadsworth Formation, and sand and gravel outwash of the Henry Formation.

3.0 METHODS OF INVESTIGATION

The following sections outline the subsurface and laboratory investigations performed by Wang.

3.1 Field Investigation

The subsurface investigation consisted of four bridge borings, designated as TB-01 through TB-04, and 18 trail borings, designated as BWB-01 through BWB-15, BWB-02B, BWB-03B, and BWB-06-SH drilled by Wang from April 28 to May 18, 2021. Borings B-1 and B-2 drilled during the preliminary design stage in September 2020 were also considered to supplement our subsurface investigation. The borings were drilled from elevations of 773.6 to 780.8 feet to depths of 30 to 75 feet bgs. As-drilled northing and easting were surveyed by Wang and elevations, stations, and offsets were provided by TranSystems. Boring location data are presented in the *Boring Logs* (Appendix A) and the as-drilled boring locations are shown in the *Boring Location Plan* (Exhibit 3). The alignments, existing ground surface elevations, and the corresponding borings with termination depth ranges are summarized in Table 1.

Table 1: Subsurface Investigation Summary

Location	Reference Borings	Existing Surface Elevation (feet)	Boring Depth (feet)
West Boardwalk Approach	BWB-01 through BWB-06, BWB-02B, BWB-03B, and BWB-06-SH	773.6 to 774.4	30.0 to 60.0
Bridge	B-1, B-2, and TB-01 through TB-04	773.6 to 774.9	75.0
East Boardwalk Approach	BWB-07 through BWB-15	774.5 to 780.8	30.0

ATV-mounted drilling rig, equipped with hollow stem augers, was used to advance and maintain open boreholes. Mud rotary drilling techniques were used from below 10.0 feet bgs to advance Borings TB-01 through TB-04. Soil sampling was performed according to AASHTO T206, *"Penetration Test and Split Barrel Sampling of Soils."* The soil was sampled at 2.5-foot intervals to 30 feet bgs and at 5-foot intervals to the boring termination depths. Soil samples collected from each sampling interval were placed in sealed jars and transported to the laboratory for further examination and laboratory testing.

Field boring logs, prepared and maintained by a Wang field engineer, included lithological descriptions, visual-manual soil classifications, results of Rimac and pocket penetrometer unconfined compressive strength tests, and results of Standard Penetration Tests (SPT) recorded as blows per 6 inches of penetration.

Groundwater levels were measured while drilling and at completion of each boring. At each boring location, the borehole was backfilled upon completion with grouting and the surface was restored as much as possible to its original condition.

3.2 Laboratory Testing

The soil samples were tested in the laboratory for moisture content (AASHTO T265). Organic content (AASHTO T267), Atterberg limits (AASHTO T89 and T90), and particle size analysis (AASHTO T88) tests were performed on selected samples. Field visual descriptions of the soil samples were verified in the laboratory and index tested soils were classified according to the IDH Soil Classification System. The laboratory test results are shown in the *Boring Logs* (Appendix A) and in the *Laboratory Test Results* (Appendix B).

4.0 INVESTIGATION RESULTS

Detailed description of the soil condition encountered during the subsurface investigation is presented in the attached *Boring Logs* (Appendix A) and in the *Soil Profile* (Exhibit 4). Please note that strata contact lines represent approximate boundaries between soil types. The actual transition between soil types in the field may be gradual in horizontal and vertical directions.

4.1 Lithological Profile

The proposed improvements will be constructed through undeveloped area located south of Poplar Creek and both sides of IL 59. Topsoil thickness as encountered by our investigation range from 4 to 36 inches with an average of 9 inches. In descending order, the general lithologic succession encountered beneath the topsoil includes: 1) man-made ground (fill); 2) very soft to stiff silty clay to silty clay loam; 3) peat and organic soils; 4) stiff to hard silty clay; and 5) medium dense to dense sand to sand and gravel.

1) *Man-made ground (fill)*

Beneath the topsoil, Boring BWB-15 which is located at the east end of the trail, encountered 10.0 feet of fill. The fill consists of medium stiff to hard, gray to brown silty clay to silty clay loam and clay loam and 2.5 feet of medium dense, brown sand. The cohesive fill has unconfined compressive strength (Q_u) values of 0.5 to 5.1 tsf and moisture content values of 12 to 20%. The sand fill has a SPT N-value of 25 blows per foot and a moisture content value of 8%.

2) *Very soft to stiff silty clay to silty clay loam*

Beneath the topsoil, at depths of 0.3 to 3.0 feet bgs, the borings advanced through 2.3 to 10.0 feet of very soft to stiff silty clay to silty clay loam with some organic matter. This unit has Q_u values of less than 0.25 to 1.8 tsf and moisture content values of 13 to 28% with higher moisture content values of 31 to 95% indicating organic inclusions. Laboratory index testing on samples of this unit shows liquid limit (L_L) values of 34 and 63% and plastic limit (P_L) values of 18 to 24%.

3) *Peat and organic soils*

Beneath the topsoil or the soft silty clay to silty clay loam, at depths of 0.5 to 5.5 feet bgs (769 to 774 feet), all boring drilled on the west side of IL 59 and two borings on the east side of IL 59 augured through 2.5 to 35.0 feet of peat, organic soils, and soft silty clay. Borings BWB-04, BWB-06, and TB-01 revealed 2.5 and 10 feet of very loose sandy loam to sand interbedded within the organic soils. The peat is characterized by Q_u values of less than 0.08 to 0.5 tsf, SPT N-values of 0 to 6 blows per foot, and moisture content values of 181 to 686%. The organic silty clay is characterized by Q_u values of less than 0.08 to 0.33 tsf, SPT N-values of 0 to 1 blows per foot and moisture content values of 31 to 96%. The bottom soft silty clay to silty clay loam is characterized by 0.16 to 0.5 tsf and moisture content values of 21 to 35%. The interbedded sand has SPT-N values of 0 to 9 blows per foot and moisture content values of 18 to 23%. Laboratory index testing on samples of this unit shows L_L values of 28 to 73% and P_L values of 14 to 28%. The split spoon sampler advanced through this mostly under hammer weight.

4) *Stiff to hard silty clay*

At depths of 4.0 to 35.5 feet bgs (elevation 738.2 to 774.8 feet), the borings advanced through stiff to hard silty clay to silty clay loam with silt, sand, and gravel lenses. The soil has Q_u values of 1.1 to 6.5 tsf and moisture content values of 15 to 23%.

5) *Medium dense to dense sand to sand and gravel*

At depths of 3.0 to 28.0 feet bgs (elevation 746.1 to 771.4 feet), the borings advanced through medium dense to dense, saturated sand to sand and gravel. This soil unit has N-values of 6 to 54 blows per foot and moisture content values of 6 to 22%. Cobbles were encountered throughout this unit.

4.2 Groundwater Conditions

Groundwater was encountered while drilling and at the completion of drilling at elevations of 746 to 774 feet (0 to 30 feet bgs). We estimate the granular layers are water-bearing. In addition, during periods of precipitation, we anticipate ponding water to be common throughout the project area. It should be noted that groundwater levels might vary with seasonal rainfall patterns and long-term climate fluctuations or be influenced by local site conditions.

5.0 FOUNDATION ANALYSIS AND RECOMMENDATIONS

Geotechnical evaluations and recommendations for the bridge and trail are included in the following sections. The preliminary general plan drawing provided by TranSystems shows the proposed structure will have a total of 87 piers and two abutments. There are three main sections of the structure: west boardwalk approach (Piers 1 to 33), (Piers 33 to 38), and east boardwalk approach (Piers 38 to 87). As per information received from TranSystems, we understand the piers will be supported on drilled shafts. Recommendations for the design and construction of drilled shafts are discussed in the following sections.

5.1 Seismic Design Considerations

The seismic site class was determined in accordance with the IDOT All Geotechnical Manual Users (AGMU) 9.1 (2009) method of analysis. The soils within the top 100 feet have a weighted average N value of 19 blows per foot (AASHTO 2017; Method B controlling), and the results classify the site in the Seismic Site Class D.

The project location belongs to the Seismic Performance Zone 1. The seismic spectral acceleration parameters recommended for design in accordance with AASHTO LRFD Bridge Design Specifications (2017) are summarized in Table 2. According to the IDOT Bridge Manual (2012), liquefaction analysis is not required for sites in Seismic Performance Zone 1.

Table 2: Recommended Seismic Design Parameters

Spectral Acceleration Period (Sec)	Spectral Acceleration Coefficient ¹⁾	Site Factors	Design Spectrum for Site
	(% g)		Class D ²⁾ (% g)
0	PGA= 4.3	$F_{pga}= 1.6$	$A_s= 6.9$
0.2	$S_s= 9.4$	$F_a= 1.6$	$S_{Ds}= 15.0$
1	$S_1= 3.5$	$F_v= 2.4$	$S_{D1}= 8.4$

1) Spectral acceleration coefficients from AASHTO LRFD, 2020

2) Site Class D Spectrum to be included on plans; $A_s = PGA * F_{pga}$; $S_{Ds} = S_s * F_a$; $S_{D1} = S_1 * F_v$

5.2 Structure Foundations

According to the General Plan and Elevation (GPE) drawings prepared by TranSystems, Wang understands the proposed bridge piers will be supported on drilled shafts. The estimated maximum factored service and strength loads are summarized in Table 3.

Table 3: Preliminary Foundation Loads

Pier Type	Pier ID	Maximum Vertical Reaction (per Shaft)		Lateral Load (per shaft; kips)
		Service (kips)	Strength (kips)	
Boardwalk Bent	1 to 32, 39 to 87	29.0	42.7	2.7
Tulip Type Pier	33,34,37,38	111.0	132.0	5.3
“H” Type Pier	35, 36	90.6	111.2	4.5

According to the provided foundation plans, drilled shaft cap bottom elevations are shown at 6 feet below the existing grade elevation. Our subsurface investigation revealed up to 35 feet of peat and soft soil on most of the west side of IL 59 from Station 100+35 to Station 108+80 and some on the east side of IL 59 from Station 114+10 to Station 116+35 underlying the proposed shaft cap elevations.

The shafts capacities were calculated based on side resistance only for conservative approach with a shaft side resistance factor (ϕ_{qs}) of 0.55 for granular soil and 0.45 for cohesive soils (AASHTO 2017). The side resistance was estimated starting from the bottom of the very compressible soils (Layers 2 and 3). We have not included side resistance from these layers as the peat and organic soils tend to settle under their own weight and by organic matter breakdown. The possible settlement of these top layers will also generate downdrag loads on drilled shafts/piles. We consider these downdrag loads to be very small. We compensated the likelihood of downdrag loads by not considering the end resistance on our estimated shaft capacity and corresponding lengths. Since we have not considered the end resistance on the shaft length and capacity calculations, we recommend a minimum unconfined compressive strength of 1.5 tsf be achieved at the final shaft depth except for the locations where granular soil with properties described in the nearby soil boings is encountered at the designed shaft base elevations.

The factored resistances available, total shaft lengths, and estimated base elevations for 24-, 30-, and 36-inch diameter shafts are summarized below in Tables 4 through Table 24. The tables show approximate shaft lengths and capacities at the proposed maximum vertical reaction and at the boring termination depth.

Table 4: Estimated Resistances and Drilled Shafts Base Elevations for Piers 1 to 4

Location (Boring)	Estimated Shaft Cap Base Elevation (feet)	Shaft Diameter (inches)	Factored Resistance Available (kips)	Total Shaft Length (feet)	Estimated Shaft Base Elevation (feet)
W. Abutment, Piers 1 to 4 BWB-01	768	24	45	22	746
			51	24	744 ¹⁾
		30	45	17	751
			63	24	744 ¹⁾
		36	45	14	754
			76	24	744 ¹⁾

¹⁾ Boring termination depth

Table 5: Estimated Resistances and Drilled Shafts Base Elevations for Piers 5 to 9

Location (Boring)	Estimated Shaft Cap Base Elevation (feet)	Shaft Diameter (inches)	Factored Resistance Available (kips)	Total Shaft Length (feet)	Estimated Shaft Base Elevation (feet)	
Piers 5 to 9 BWB-02B	768	24	45	22	746	
			55	24	744 ¹⁾	
		30	45	18	750	
			70	24	744 ¹⁾	
			36	45	16	752
				83	24	744 ¹⁾

¹⁾ Boring termination depth

Table 6: Estimated Resistances and Drilled Shafts Base Elevations for Piers 10 to 15

Location (Boring)	Estimated Shaft Cap Base Elevation (feet)	Shaft Diameter (inches)	Factored Resistance Available (kips)	Total Shaft Length (feet)	Estimated Shaft Base Elevation (feet)	
Piers 10 to 15 BWB-03B	768	24	45	27.5	740.5	
			134	44	724 ¹⁾	
		30	45	26	742	
			168	44	724 ¹⁾	
			36	45	24.5	743.5
				200	44	724 ¹⁾

¹⁾ Boring termination depth

Table 7: Estimated Resistances and Drilled Shafts Base Elevations for Piers 16 to 20

Location (Boring)	Estimated Shaft Cap Base Elevation (feet)	Shaft Diameter (inches)	Factored Resistance Available (kips)	Total Shaft Length (feet)	Estimated Shaft Base Elevation (feet)
Piers 16 to 20 BWB-04	768	24	45	29	739
			132	44	724 ¹⁾
		30	45	27.5	740.5
			165	44	724 ¹⁾
		36	45	26.5	741.5
			198	44	724 ¹⁾

¹⁾ Boring termination depth

Table 8: Estimated Resistances and Drilled Shafts Base Elevations for Piers 21 to 25

Location (Boring)	Estimated Shaft Cap Base Elevation (feet)	Shaft Diameter (inches)	Factored Resistance Available (kips)	Total Shaft Length (feet)	Estimated Shaft Base Elevation (feet)
Piers 21 to 25 BWB-05	767.5	24	30	23.5	744 ¹⁾
			45	27	740.5 ²⁾
		30	38	23.5	744 ¹⁾
			45	25	742.5 ²⁾
		36	45	23.5	744 ¹⁾

¹⁾ Boring termination depth

²⁾ Soil data was extrapolated

Table 9: Estimated Resistances and Drilled Shafts Base Elevations for Piers 26 to 30

Location (Boring)	Estimated Shaft Cap Base Elevation (feet)	Shaft Diameter (inches)	Factored Resistance Available (kips)	Total Shaft Length (feet)	Estimated Shaft Base Elevation (feet)
Piers 26 to 30 BWB-06, BWB-06SH	768	24	45	46	722
			68	55	713 ¹⁾
			45	43	725
		30	85	55	713 ¹⁾
			45	41	727
			102	55	713 ¹⁾

¹⁾ Boring termination depth

Table 10: Estimated Resistances and Drilled Shafts Base Elevations for Piers 31 to 33

Location (Boring)	Estimated Shaft Cap Base Elevation (feet)	Shaft Diameter (inches)	Factored Resistance Available (kips)	Total Shaft Length (feet)	Estimated Shaft Base Elevation (feet)	
Piers 31 to 33 TB-01	768	24	45	29	739	
			140	59	709	
			280	69	699 ¹⁾	
		30	45	26	742	
			140	51	717	
			350	69	699 ¹⁾	
			36	45	26	743.5
				140	45	723
				420	69	699 ¹⁾

¹⁾ Boring termination depth

Table 11: Estimated Resistances and Drilled Shafts Base Elevations for Pier 34

Location (Boring)	Estimated Shaft Cap Base Elevation (feet)	Shaft Diameter (inches)	Factored Resistance Available (kips)	Total Shaft Length (feet)	Estimated Shaft Base Elevation (feet)
Pier 34 TB-02	768	24	140	48	720
			282	69	699 ¹⁾
		30	140	39	729
			353	69	699 ¹⁾
		36	140	35	733
			423	69	699 ¹⁾

¹⁾ Boring termination depth

Table 12: Estimated Resistances and Drilled Shafts Base Elevations for Pier 35

Location (Boring)	Estimated Shaft Cap Base Elevation (feet)	Shaft Diameter (inches)	Factored Resistance Available (kips)	Total Shaft Length (feet)	Estimated Shaft Base Elevation (feet)
Pier 35 B-01	768	24	120	45	723
			262	69	699 ¹⁾
		30	120	41	727
			328	69	699 ¹⁾
		36	120	65	732
			394	69	699 ¹⁾

¹⁾ Boring termination depth

Table 13: Estimated Resistances and Drilled Shafts Base Elevations for Pier 36

Location (Boring)	Estimated Shaft Cap Base Elevation (feet)	Shaft Diameter (inches)	Factored Resistance Available (kips)	Total Shaft Length (feet)	Estimated Shaft Base Elevation (feet)
Pier 36 B-02	769	24	120	36	733
			229 ¹⁾	70	699 ¹⁾
		30	120	27	742
			286	70	699 ¹⁾
		36	120	24	745
			343	70	699 ¹⁾

¹⁾ Boring termination depth

Table 14: Estimated Resistances and Drilled Shafts Base Elevations for Pier 37

Location (Boring)	Estimated Shaft Cap Base Elevation (feet)	Shaft Diameter (inches)	Factored Resistance Available (kips)	Total Shaft Length (feet)	Estimated Shaft Base Elevation (feet)
Pier 37 TB-03	769	24	140	33	736
			313	69	700 ¹⁾
		30	140	28	741
			391	69	700 ¹⁾
		36	140	25	744
			470	69	700 ¹⁾

¹⁾ Boring termination depth

Table 15: Estimated Resistances and Drilled Shafts Base Elevations for Piers 38 to 43

Location (Boring)	Estimated Shaft Cap Base Elevation (feet)	Shaft Diameter (inches)	Factored Resistance Available (kips)	Total Shaft Length (feet)	Estimated Shaft Base Elevation (feet)	
Piers 38 to 43 TB-04	768.5	24	45	16	752.5	
			140	38.5	730	
			329	69	699.5 ¹⁾	
		30	45	13.5	755	
			140	32	736.5	
			411	69	699.5 ¹⁾	
			36	45	12.5	756
				140	27.5	741
				493	69	699.5 ¹⁾

¹⁾ Boring termination depth

Table 16: Estimated Resistances and Drilled Shafts Base Elevations for Piers 44 to 50

Location (Boring)	Estimated Shaft Cap Base Elevation (feet)	Shaft Diameter (inches)	Factored Resistance Available (kips)	Total Shaft Length (feet)	Estimated Shaft Base Elevation (feet)
Piers 44 to 50 BWB-07	768.5	24	45	20.5	748
			54	24	744.5 ¹⁾
		30	45	18.5	751
			68	24	744.5 ¹⁾
		36	45	15.5	753
			82	24	744.5 ¹⁾

¹⁾ Boring termination depth

Table 17: Estimated Resistances and Drilled Shafts Base Elevations for Piers 51 to 55

Location (Boring)	Estimated Shaft Cap Base Elevation (feet)	Shaft Diameter (inches)	Factored Resistance Available (kips)	Total Shaft Length (feet)	Estimated Shaft Base Elevation (feet)
Piers 51 to 55 BWB-08	768.5	24	45	18.5	750
			66	24	744.5 ¹⁾
		30	45	16.5	752
			82	24	744.5 ¹⁾
		36	45	14.5	754
			99	24	744.5 ¹⁾

¹⁾ Boring termination depth

Table 18: Estimated Resistances and Drilled Shafts Base Elevations for Piers 56 to 59

Location (Boring)	Estimated Shaft Cap Base Elevation (feet)	Shaft Diameter (inches)	Factored Resistance Available (kips)	Total Shaft Length (feet)	Estimated Shaft Base Elevation (feet)
Piers 56 to 59 BWB-09	769.5	24	45	16.5	753
			76	24	745.5 ¹⁾
		30	45	14	755.5
			96	24	745.5 ¹⁾
		36	45	12.5	757
			115	24	745.5 ¹⁾

¹⁾ Boring termination depth

Table 19: Estimated Resistances and Drilled Shafts Base Elevations for Piers 60 to 65

Location (Boring)	Estimated Shaft Cap Base Elevation (feet)	Shaft Diameter (inches)	Factored Resistance Available (kips)	Total Shaft Length (feet)	Estimated Shaft Base Elevation (feet)
Piers 60 to 65 BWB-10	771	24	45	27.5	743.5 ²⁾
		30	45	23	748
			50	25	746 ¹⁾
			45	20	751
		36	60	25	746 ¹⁾

1) Boring termination depth

2) Soil data was extrapolated

Table 20: Estimated Resistances and Drilled Shafts Base Elevations for Piers 66 to 71

Location (Boring)	Estimated Shaft Cap Base Elevation (feet)	Shaft Diameter (inches)	Factored Resistance Available (kips)	Total Shaft Length (feet)	Estimated Shaft Base Elevation (feet)
Piers 66 to 71 BWB-11	770.5	24	45	14	756.5
			101	25	745.5 ¹⁾
		30	45	12	758.5
			126	25	745.5 ¹⁾
			45	11	759.5
36	151	25	745.5 ¹⁾		

1) Boring termination depth

Table 21: Estimated Resistances and Drilled Shafts Base Elevations for Piers 72 to 76

Location (Boring)	Estimated Shaft Cap Base Elevation (feet)	Shaft Diameter (inches)	Factored Resistance Available (kips)	Total Shaft Length (feet)	Estimated Shaft Base Elevation (feet)
Piers 72 to 76 BWB-12	770	24	45	18	752
			67	24	746 ¹⁾
		30	45	15	755
			84	24	746 ¹⁾
		36	45	13.5	756.5
			100	24	746 ¹⁾

¹⁾ Boring termination depth

Table 22: Estimated Resistances and Drilled Shafts Base Elevations for Piers 77 to 81

Location (Boring)	Estimated Shaft Cap Base Elevation (feet)	Shaft Diameter (inches)	Factored Resistance Available (kips)	Total Shaft Length (feet)	Estimated Shaft Base Elevation (feet)
Piers 77 to 81 BWB-13	770	24	45	18.5	751.5
			64	24	746 ¹⁾
		30	45	16	754
			80	24	746 ¹⁾
		36	45	14	756
			95	24	746 ¹⁾

¹⁾ Boring termination depth

Table 23: Estimated Resistances and Drilled Shafts Base Elevations for Piers 82 to 85

Location (Boring)	Estimated Shaft Cap Base Elevation (feet)	Shaft Diameter (inches)	Factored Resistance Available (kips)	Total Shaft Length (feet)	Estimated Shaft Base Elevation (feet)
Piers 82 to 85 BWB-14	770.5	24	45	16.5	754
			84	24.5	746 ¹⁾
		30	45	15	755.5
			105	24.5	746 ¹⁾
		36	45	13.5	757
			126	24.5	746 ¹⁾

¹⁾ Boring termination depth

Table 24: Estimated Resistances and Drilled Shafts Base Elevations for Piers 86 to 87

Location (Boring)	Estimated Shaft Cap Base Elevation (feet)	Shaft Diameter (inches)	Factored Resistance Available (kips)	Total Shaft Length (feet)	Estimated Shaft Base Elevation (feet)
Piers 86 to 87 East Abutment BWB-15	771	24	45	17	754
			56	20	751 ¹⁾
		30	45	15	756
			70	20	751 ¹⁾
		36	45	13	758
			84	20	751 ¹⁾

¹⁾ Boring termination depth

Lateral loads on the shafts should be analyzed for maximum moments and lateral deflections. Recommended soil parameters required for the lateral load analysis of shafts at Piers 1 through 87 are provided in Tables 25 through 45.

Table 25: Recommended Soil Parameters for Lateral Load Analysis
 for West abutment, Piers 1 to 4, Ref. Boring BWB-01

Elevation (feet) Soil Type	Unit Weight, γ (pcf)	Undrained Shear Strength, c_u (psf)	Estimated Friction Angle, Φ ($^\circ$)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ϵ_{50} (%)
Stiff Peat EL 774.4 to 771.4	100	1000	0	500	0.7
M Dense SA Loam EL 771.4 to 768.4	115	0	30	90	--
M Dense SA Loam EL 768.4 ⁽¹⁾ to 766.4	53 ⁽²⁾	0	30	60	--
V Stiff SI Clay EL 766.4 to 763.9	58 ⁽²⁾	2000	0	1000	0.5
Loose to M Dense SA Loam EL 763.9 to 758.9	53 ⁽²⁾	0	30	60	--
M Stiff to V Stiff SI Clay EL 758.9 to 751.4	58 ⁽²⁾	1500	0	500	0.7
M Dense Silt EL 751.4 to 748.9	53 ⁽²⁾	0	29	60	--
Stiff to V Stiff SI Clay EL 748.9 to 744.4 ⁽³⁾	58 ⁽²⁾	1900	0	500	0.7

⁽¹⁾Water table elevation; ⁽²⁾Submerged unit weight; ⁽³⁾Boring termination depth

Table 26: Recommended Soil Parameters for Lateral Load Analysis
 for Piers 5 to 9, Ref. Boring BWB-02-B

Elevation (feet) Soil Type	Unit Weight, γ (pcf)	Undrained Shear Strength, c_u (psf)	Estimated Friction Angle, Φ ($^\circ$)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ϵ_{50} (%)
V Soft Peat EL 774.3 to 770.3	70	100	0	30	2.0
V Soft Peat EL 770.3 to 767.8	10 ⁽²⁾	100	0	30	2.0

Elevation (feet) Soil Type	Unit Weight, γ (pcf)	Undrained Shear Strength, c_u (psf)	Estimated Friction Angle, Φ ($^\circ$)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ϵ_{50} (%)
Loose to M Dense Sand EL 767.8 ⁽¹⁾ to 763.8	53 ⁽²⁾	0	29	20	--
Loose to M Dense SA Gravel EL 763.8 to 756.3	53 ⁽²⁾	0	30	60	--
Stiff to V Stiff SI Clay EL 756.3 to 746.3	58 ⁽²⁾	2000	0	1000	0.5
Loose Silt EL 746.3 to 744.3 ⁽³⁾	48 ⁽²⁾	0	28	20	--

⁽¹⁾Water table elevation; ⁽²⁾Submerged unit weight; ⁽³⁾Boring termination depth

Table 27: Recommended Soil Parameters for Lateral Load Analysis
 for Piers 10 to 15, Ref. Boring BWB-03-B

Elevation (feet) Soil Type	Unit Weight, γ (pcf)	Undrained Shear Strength, c_u (psf)	Estimated Friction Angle, Φ ($^\circ$)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ϵ_{50} (%)
V Soft Peat EL 773.9 ⁽¹⁾ to 760.9	10 ⁽²⁾	100	0	30	2.0
V Soft Organic SI CL to SI CL Loam EL 760.9 to 750.9	30 ⁽²⁾	100	0	30	2.0
Soft SI CL Loam EL 750.9 to 748.4	48 ⁽²⁾	400	0	30	2.0
M Dense Sand EL 748.4 to 745.9	53 ⁽²⁾	0	30	60	--
M Dense SA Gravel EL 745.9 to 742.2	53 ⁽²⁾	0	31	60	--
M Dense to Dense SAND EL 742.2 to 727.2	58 ⁽²⁾	0	32	125	--
V Stiff SI Clay EL 727.2 to 723.9 ⁽³⁾	58 ⁽²⁾	2000	0	1000	0.5

⁽¹⁾Water table elevation; ⁽²⁾Submerged unit weight; ⁽³⁾Boring termination depth

Table 28: Recommended Soil Parameters for Lateral Load Analysis
 for Piers 16 to 20, Ref. Boring BWB-04

Elevation (feet) Soil Type	Unit Weight, γ (pcf)	Undrained Shear Strength, c_u (psf)	Estimated Friction Angle, Φ ($^\circ$)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ϵ_{50} (%)
V Soft Peat EL 774.1 to 772.1	70	100	0	30	2.0
V Soft Peat EL 772.1 ⁽¹⁾ to 761.1	10 ⁽²⁾	100	0	30	2.0
Loose Sand EL 761.1 to 758.6	48 ⁽²⁾	0	27	20	--
V Soft Organic SI CL to SI CL Loam EL 758.6 to 746.1	30 ⁽²⁾	100	0	30	2.0
M Dense to Dense SA to SA Loam EL 746.1 to 727.4	58 ⁽²⁾	0	32	60	--
V Stiff SI Clay EL 727.4 to 724.1 ⁽³⁾	58 ⁽²⁾	2200	0	1000	0.5

⁽¹⁾Water table elevation; ⁽²⁾Submerged unit weight; ⁽³⁾Boring termination depth

Table 29: Recommended Soil Parameters for Lateral Load Analysis
 for Piers 21 to 25, Ref. Boring BWB-05

Elevation (feet) Soil Type	Unit Weight, γ (pcf)	Undrained Shear Strength, c_u (psf)	Estimated Friction Angle, Φ ($^\circ$)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ϵ_{50} (%)
V Soft Organic SI Clay EL 773.8 to 771.8	90	100	0	30	2.0
V Soft Organic SI Clay EL 771.8 ⁽¹⁾ to 770.8	30 ⁽²⁾	100	0	30	2.0
V Soft Peat EL 770.8 to 760.8	10 ⁽²⁾	100	0	30	2.0
V Soft to M Stiff Organic SI CL to SI CL Loam EL 760.8 to 750.8	30 ⁽²⁾	100	0	30	2.0

Elevation (feet) Soil Type	Unit Weight, γ (pcf)	Undrained Shear Strength, c_u (psf)	Estimated Friction Angle, Φ ($^\circ$)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ϵ_{50} (%)
Stiff to Hard SI Clay EL 750.8 to 743.8 ⁽³⁾	58 ⁽²⁾	2800	0	1000	0.5

⁽¹⁾Water table elevation; ⁽²⁾Submerged unit weight; ⁽³⁾Boring termination depth

Table 30: Recommended Soil Parameters for Lateral Load Analysis
 for Piers 26 to 30, Ref. Borings BWB-06 & BWB-06-SH

Elevation (feet) Soil Type	Unit Weight, γ (pcf)	Undrained Shear Strength, c_u (psf)	Estimated Friction Angle, Φ ($^\circ$)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ϵ_{50} (%)
V Soft to M Stiff Peat EL 773.7 to 758.2	70	100	0	30	2.0
V Soft Organic SI Clay EL 758.2 to 755.7	90	100	0	30	2.0
V Loose SA Loam to Sand EL 755.7 ⁽¹⁾ to 745.7	48 ⁽²⁾	0	27	20	--
V Soft Organic SI CL to SI CL Loam EL 745.7 to 738.2	30 ⁽²⁾	100	0	30	2.0
Stiff SI Clay EL 738.2 to 713.6 ⁽³⁾	58 ⁽²⁾	2000	0	1000	0.5

⁽¹⁾Water table elevation; ⁽²⁾Submerged unit weight; ⁽³⁾Boring termination depth

Table 31: Recommended Soil Parameters for Lateral Load Analysis
 for Piers 31 to 33, Ref. Boring TB-01

Elevation (feet) Soil Type	Unit Weight, γ (pcf)	Undrained Shear Strength, c_u (psf)	Estimated Friction Angle, Φ ($^\circ$)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ϵ_{50} (%)
M Stiff SI Clay EL 774.0 to 771.0	115	500	0	100	1.0

Elevation (feet) Soil Type	Unit Weight, γ (pcf)	Undrained Shear Strength, c_u (psf)	Estimated Friction Angle, Φ ($^\circ$)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ϵ_{50} (%)
V Soft to M Stiff Peat EL 771.0 to 761.00	70	180	0	30	2.0
V Loose SA Loam EL 761.0 ⁽¹⁾ to 758.5	30 ⁽²⁾	0	27	20	--
V Soft to M Stiff SI CL Loam EL 758.5 to 753.5	48 ⁽²⁾	230	0	30	2.0
Stiff to V Stiff SI Clay EL 753.5 to 716.5	58 ⁽²⁾	2000	0	1000	0.5
V Dense Sand EL 716.5 to 712.3	63 ⁽²⁾	0	33	125	--
Hard SI CL Loam EL 712.3 to 707.3	63 ⁽²⁾	4000	0	2000	0.4
M Dense Sand EL 707.3 to 702.3	53 ⁽²⁾	0	31	60	--
Dense SA Gravel EL 707.2 to 699.0 ⁽³⁾	58 ⁽²⁾	0	32	125	--

⁽¹⁾Water table elevation; ⁽²⁾Submerged unit weight; ⁽³⁾Boring termination depth

Table 32: Recommended Soil Parameters for Lateral Load Analysis
 for Pier 34. Boring TB-02

Elevation (feet) Soil Type	Unit Weight, γ (pcf)	Undrained Shear Strength, c_u (psf)	Estimated Friction Angle, Φ ($^\circ$)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ϵ_{50} (%)
M Stiff SI Clay EL 773.6 to 770.6	120	1000	0	500	0.7
V Soft Peat EL 770.6 to 757.6	70	100	0	30	2.0
V Soft to M Stiff SI CL Loam EL 757.6 ⁽¹⁾ to 753.1	30 ⁽²⁾	300	0	30	2.0
Stiff to Hard SI Clay EL 753.1 to 731.9	58 ⁽²⁾	3000	0	1000	0.5

Elevation (feet) Soil Type	Unit Weight, γ (pcf)	Undrained Shear Strength, c_u (psf)	Estimated Friction Angle, Φ ($^\circ$)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ϵ_{50} (%)
M Dense GR Sand EL 731.9 to 724.4	53 ⁽²⁾	0	30	60	--
V Stiff SI Clay EL 724.4 to 716.0	58 ⁽²⁾	2400	0	1000	0.5
Soft SI Clay EL 716.0 to 711.9	48 ⁽²⁾	400	0	30	2.0
M Dense to Dense SA GR to Gravel EL 711.9 to 698.6 ⁽³⁾	58 ⁽²⁾	0	32	125	--

⁽¹⁾Water table elevation; ⁽²⁾Submerged unit weight; ⁽³⁾Boring termination depth

Table 33: Recommended Soil Parameters for Lateral Load Analysis
 for Pier 35, Ref. Boring B-1

Elevation (feet) Soil Type	Unit Weight, γ (pcf)	Undrained Shear Strength, c_u (psf)	Estimated Friction Angle, Φ ($^\circ$)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ϵ_{50} (%)
V Stiff SI CL Loam Topsoil EL 774.6 to 771.6	120	3500	0	1000	0.5
V Soft to M Stiff Peat, CL to SI CL Loam EL 771.6 to 754.1	110	350	0	30	2.0
SA Gravel EL 754.1 ⁽¹⁾ to 753.3	53 ⁽²⁾	0	28	20	--
Stiff SI CL to SI CL Loam EL 753.3 to 746.6	58 ⁽²⁾	1600	0	500	0.7
Stiff to Hard SI CL to SI CL Loam EL 746.6 to 727.9	58 ⁽²⁾	2900	0	1000	0.5
Medium Dense M Sand EL 727.9 to 720.6	53 ⁽²⁾	0	32	60	--
V Stiff SI CL EL 720.6 to 712.9	58 ⁽²⁾	2100	0	1000	0.5

Elevation (feet) Soil Type	Unit Weight, γ (pcf)	Undrained Shear Strength, c_u (psf)	Estimated Friction Angle, Φ ($^\circ$)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ϵ_{50} (%)
M Dense SA to SA Loam EL 712.9 to 699.6 ⁽³⁾	53 ⁽²⁾	0	32	60	--

⁽¹⁾Water table elevation; ⁽²⁾Submerged unit weight; ⁽³⁾Boring termination depth

Table 34: Recommended Soil Parameters for Lateral Load Analysis
 for Pier 36, Ref. Boring B-2

Elevation (feet) Soil Type	Unit Weight, γ (pcf)	Undrained Shear Strength, c_u (psf)	Estimated Friction Angle, Φ ($^\circ$)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ϵ_{50} (%)
Soft to Stiff SI Clay EL 774.4 to 770.4	115	900	0	100	1.0
Loose Silt EL 770.4 to 768.9	110	0	28	20	--
Stiff to Hard SI Clay EL 768.9 to 747.7	120	2500	0	1000	0.5
M Dense SA Gravel EL 747.7 ⁽¹⁾ to 740.4	53 ⁽²⁾	0	32	60	--
Stiff to V Stiff SI Clay Loam EL 740.4 to 735.1	58 ⁽²⁾	1900	0	500	0.7
Medium Dense to Dense Sand EL 735.1 to 730.4	58 ⁽²⁾	0	32	60	--
Stiff SI Clay EL 730.4 to 722.6	58 ⁽²⁾	1400	0	500	0.7
M Dense SI Loam EL 722.6 to 717.6	53 ⁽²⁾	0	32	60	--
Stiff SI Clay EL 717.6 to 705.6	58 ⁽²⁾	1300	0	500	0.7
M Dense SI Loam to SA Gravel EL 705.6 to 699.4 ⁽³⁾	53 ⁽²⁾	0	32	60	--

⁽¹⁾Water table elevation; ⁽²⁾Submerged unit weight; ⁽³⁾Boring termination depth

Table 35: Recommended Soil Parameters for Lateral Load Analysis
 for Pier 37, Ref. Boring TB-03

Elevation (feet) Soil Type	Unit Weight, γ (pcf)	Undrained Shear Strength, c_u (psf)	Estimated Friction Angle, Φ ($^\circ$)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ϵ_{50} (%)
M Stiff SI Clay EL 774.9 to 771.9	120	1000	0	500	0.7
Stiff SI Clay EL 771.9 to 768.9	120	3800	0	1000	0.5
Stiff to Hard SI Clay EL 768.9 ⁽¹⁾ to 749.4	58 ⁽²⁾	3000	0	1000	0.5
M Dense SA Grave to Gravel EL 749.4 to 738.2	53 ⁽²⁾	0	30	60	--
Stiff SI CL Loam EL 738.2 to 733.2	58 ⁽²⁾	1000	0	500	0.7
M Dense to Dense Sand to SA Gravel EL 733.2 to 723.2	58 ⁽²⁾	0	31	60	--
M Dense SI to SI Loam EL 723.2 to 708.2	53 ⁽²⁾	0	30	60	--
V Stiff SI Clay EL 708.2 to 703.2	58 ⁽²⁾	3200	0	1000	0.5
M Dense SA Gravel EL 703.2 to 699.9 ⁽³⁾	53 ⁽²⁾	0	30	60	--

⁽¹⁾Water table elevation; ⁽²⁾Submerged unit weight; ⁽³⁾Boring termination depth

Table 36: Recommended Soil Parameters for Lateral Load Analysis
 for Piers 38 to 43, Ref. Boring TB-04

Elevation (feet) Soil Type	Unit Weight, γ (pcf)	Undrained Shear Strength, c_u (psf)	Estimated Friction Angle, Φ ($^\circ$)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ϵ_{50} (%)
Soft SI Clay EL 774.6 to 773.1	110	330	0	30	2.0
V Soft to Soft SI Clay EL 773.1 ⁽¹⁾ to 769.1	30 ⁽²⁾	200	0	30	2.0

Elevation (feet) Soil Type	Unit Weight, γ (pcf)	Undrained Shear Strength, c_u (psf)	Estimated Friction Angle, Φ ($^\circ$)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ϵ_{50} (%)
M Dense SA Loam EL 769.1 to 766.6	53 ⁽²⁾	0	30	60	--
V Stiff to Hard SI Clay EL 766.6 to 756.6	58 ⁽²⁾	3400	0	1000	0.5
Stiff to V Stiff SI Clay EL 756.6 to 737.9	58 ⁽²⁾	2100	0	1000	0.5
M Dense SA Gravel EL 737.9 to 732.9	53 ⁽²⁾	0	30	60	--
V Stiff SI Clay EL 732.9 to 727.9	58 ⁽²⁾	2600	0	1000	0.5
V Dense Sand EL 727.9 to 722.9	63 ⁽²⁾	0	33	125	--
M Dense to Dense SA Gravel to Silty Loam EL 722.9 to 707.9	58 ⁽²⁾	0	31	60	--
M Stiff to Stiff SI Clay EL 707.9 to 699.6 ⁽³⁾	58 ⁽²⁾	1000	0	500	0.7

⁽¹⁾Water table elevation; ⁽²⁾Submerged unit weight; ⁽³⁾Boring termination depth

Table 37: Recommended Soil Parameters for Lateral Load Analysis
 for Piers 44 to 50, Ref. Boring BWB-07

Elevation (feet) Soil Type	Unit Weight, γ (pcf)	Undrained Shear Strength, c_u (psf)	Estimated Friction Angle, Φ ($^\circ$)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ϵ_{50} (%)
V Soft to Soft Organic SI CL to SI CL Loam EL 774.5 to 771.5	90	250	0	30	2.0
V Soft Organic SI Clay EL 771.5 ⁽¹⁾ to 764.0	10 ⁽²⁾	100	0	30	2.0
M Stiff to V Stiff SI CL to SI CL Loam EL 764.0 to 751.5	58 ⁽²⁾	1700	0	500	0.7

Elevation (feet) Soil Type	Unit Weight, γ (pcf)	Undrained Shear Strength, c_u (psf)	Estimated Friction Angle, Φ ($^\circ$)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ϵ_{50} (%)
V Dense SA Gravel EL 751.5 to 749.0	63 ⁽²⁾	0	33	125	--
Stiff to V Stiff SI Clay EL 749.0 to 744.5 ⁽³⁾	58 ⁽²⁾	1700	0	500	0.7

⁽¹⁾Water table elevation; ⁽²⁾Submerged unit weight; ⁽³⁾Boring termination depth

Table 38: Recommended Soil Parameters for Lateral Load Analysis
 for Piers 51 to 55, Ref. Boring BWB-08

Elevation (feet) Soil Type	Unit Weight, γ (pcf)	Undrained Shear Strength, c_u (psf)	Estimated Friction Angle, Φ ($^\circ$)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ϵ_{50} (%)
Soft to M Stiff SI CL Loam EL 774.6 to 769.1	115	500	0	100	1.0
V Soft Peat EL 769.1 to 761.6	90	100	0	30	2.0
M Stiff to Stiff SI CL to SI CL Loam EL 761.6 to 724.1	115	900	0	100	1.0
Stiff to Hard SI CL to SI CL Loam EL 724.1 to 744.6 ⁽¹⁾	120	3700	0	1000	0.5

⁽¹⁾Boring termination depth

Table 39: Recommended Soil Parameters for Lateral Load Analysis
 for Piers 56 to 59, Ref. Boring BWB-09

Elevation (feet) Soil Type	Unit Weight, γ (pcf)	Undrained Shear Strength, c_u (psf)	Estimated Friction Angle, Φ ($^\circ$)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ϵ_{50} (%)
M Stiff to V Stiff SI CL to SI CL Loam EL 775.6 to 765.1	120	2200	0	1000	0.5
Stiff to V Stiff SI CL to SI CL Loam EL 765.1 to 752.6	120	2400	0	1000	0.5
V Stiff to Hard SI CL to SI CL Loam EL 752.6 to 745.6 ⁽¹⁾	120	3900	0	1000	0.5

⁽¹⁾Boring termination depth

Table 40: Recommended Soil Parameters for Lateral Load Analysis
 for Piers 60 to 65, Ref. Boring BWB-10

Elevation (feet) Soil Type	Unit Weight, γ (pcf)	Undrained Shear Strength, c_u (psf)	Estimated Friction Angle, Φ ($^\circ$)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ϵ_{50} (%)
M Stiff to V Stiff SI CL to SI CL Loam EL 776.1 to 763.1	120	2300	0	1000	0.5
M Stiff GR Clay Loam EL 763.1 ⁽¹⁾ to 760.6	53 ⁽²⁾	800	0	100	1.0
Soft to Stiff SI CL to SI CL Loam EL 760.6 to 746.1 ⁽³⁾	58 ⁽²⁾	1300	0	500	0.7

⁽¹⁾Water table elevation; ⁽²⁾Submerged unit weight; ⁽³⁾Boring termination depth

Table 41: Recommended Soil Parameters for Lateral Load Analysis
 for Piers 66 to 71, Ref. Boring BWB-11

Elevation (feet) Soil Type	Unit Weight, γ (pcf)	Undrained Shear Strength, c_u (psf)	Estimated Friction Angle, Φ ($^\circ$)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ϵ_{50} (%)
Stiff SI Clay EL 775.4 to 772.4	120	1800	0	500	0.7
Soft SI Clay EL 772.4 to 769.9	110	300	0	30	2.0
Stiff SI CL to SI CL Loam EL 769.9 to 759.4	120	1200	0	500	0.7
V Stiff SI CL to SI CL Loam EL 759.4 to 745.4 ⁽¹⁾	120	3300	0	1000	0.5

⁽¹⁾Boring termination depth

Table 42: Recommended Soil Parameters for Lateral Load Analysis
 for Piers 72 to 76, Ref. Boring BWB-12

Elevation (feet) Soil Type	Unit Weight, γ (pcf)	Undrained Shear Strength, c_u (psf)	Estimated Friction Angle, Φ ($^\circ$)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ϵ_{50} (%)
V Stiff SI Clay EL 775.9 to 772.9	120	1200	0	500	0.7
V Soft SI Clay EL 772.9 to 770.4	110	200	0	30	2.0
Loose SI Loam EL 770.4 to 767.9	110	0	27	20	--
Soft SI Clay EL 767.9 to 765.4	110	490	0	30	2.0
Stiff to Hard SI Clay EL 765.4 to 745.9 ⁽¹⁾	120	2300	0	1000	0.5

⁽¹⁾Boring termination depth

Table 43: Recommended Soil Parameters for Lateral Load Analysis
 for Piers 77 to 81, Ref. Boring BWB-13

Elevation (feet) Soil Type	Unit Weight, γ (pcf)	Undrained Shear Strength, c_u (psf)	Estimated Friction Angle, Φ ($^\circ$)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ϵ_{50} (%)
Soft to M Stiff SI Clay EL 776.1 to 770.6	115	600	0	100	1.0
Soft to M Stiff SI Clay to SI CL Loam EL 770.6 to 763.1	110	490	0	30	2.0
Stiff to V Stiff SI CL to SI CL Loam EL 763.1 to 746.1	110	2400	0	1000	0.5

⁽¹⁾Boring termination depth

Table 44: Recommended Soil Parameters for Lateral Load Analysis
 for Piers 82 to 85, Ref. Boring BWB-14

Elevation (feet) Soil Type	Unit Weight, γ (pcf)	Undrained Shear Strength, c_u (psf)	Estimated Friction Angle, Φ ($^\circ$)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ϵ_{50} (%)
Stiff SI Clay EL 776.2 to 773.2	120	1000	0	500	0.7
Soft to Soft SI Clay to SI CL Loam EL 773.2 to 763.2	90	150	0	30	2.0
V Stiff to Hard SI CL to SI CL Loam EL 763.2 to 746.2 ⁽¹⁾	120	3200	0	1000	0.5

⁽¹⁾Boring termination depth

Table 45: Recommended Soil Parameters for Lateral Load Analysis
 for Piers 86 to 87 and East Abutment, Ref. Boring BWB-15

Elevation (feet) Soil Type	Unit Weight, γ (pcf)	Undrained Shear Strength, c_u (psf)	Estimated Friction Angle, Φ ($^\circ$)	Estimated Lateral Soil Modulus Parameter, k (pci)	Estimated Soil Strain Parameter, ϵ_{50} (%)
M Stiff SI Clay Fill EL 780.8 to 777.8	115	500	0	100	1.0
Hard SI Clay Fill EL 777.8 to 775.3	125	5000	0	2000	0.4
V Stiff CL Loam Fill EL 775.3 to 772.8	120	2500	0	1000	0.5
M Dense SA Loam Fill EL 772.8 to 770.3	115	0	30	90	--
Stiff to Hard SI Clay to SI CL Loam EL 770.3 to 762.3	120	4900	0	2000	0.4
Stiff to V Stiff SI CL to SI CL Loam EL 762.3 ⁽¹⁾ to 750.8 ⁽³⁾	53 ⁽²⁾	2500	0	1000	0.5

⁽¹⁾Water table elevation; ⁽²⁾Submerged unit weight; ⁽³⁾Boring termination depth

6.0 CONSTRUCTION CONSIDERATIONS

6.1 Site Preparation

Vegetation, surface topsoil, and debris should be cleared and stripped where the structure will be placed. If unstable or unsuitable materials are exposed during excavation, they should be removed and replaced with compacted fill as described in Section 6.3.

6.2 Excavation, Dewatering, and Utilities

Excavations should be performed in accordance with local, state, and federal regulations. The potential effect of ground movements upon nearby utilities should be considered during construction. In general, the groundwater level is within 20 feet bgs. Water that does accumulate in open excavations by seepage or runoff should be immediately removed by sump pump.

The Contractor should ensure proper surface grading to prevent the pooling of water and runoff into open excavations. Water that does accumulate into open excavations by seepage or runoff should be immediately removed by sump pump. Any soils allowed to soften under standing water should be removed and replaced with compacted fill as described in Section 6.4.

6.3 Filling and Backfilling

Fill material used to attain final design elevations should be as per IDOT Standard Specifications. The fill material should be free of organic matter and debris and should be placed in lifts and compacted according to the IDOT Section 205, *Embankment* (IDOT 2016). For new fill to be placed on existing slopes, we recommend benching the slopes.

Backfill materials for the abutments must be pre-approved by the Resident Engineer. To backfill the abutments, we recommend porous granular material conforming to the requirements specified in the IDOT Special Provision No.76, *Granular Backfill for Structures*. Backfill material should be placed and compacted in accordance with the Special Provision. For new fill to be placed on existing slopes, we recommend benching the slopes according to IDOT embankment construction details.

6.4 Earthwork Operations

The required earthwork can be accomplished with conventional construction equipment. Moisture and traffic will cause deterioration of exposed subgrade soils. Precautions should be taken by the Contractor to prevent water erosion of the exposed subgrade. A compacted subgrade will minimize water runoff erosion.

Earth moving operations should be scheduled to not coincide with excessive cold or wet weather (early spring, late fall or winter). Any soil allowed to freeze or soften due to the standing water should be removed. Wet weather can cause problems with subgrade compaction.

It is recommended that an experienced geotechnical engineer be retained to inspect the exposed subgrade, monitor earthwork operations, and provide material inspection services during the construction phase of this project.

6.5 Drilled Shafts

The installation of drilled shafts through and on top of the peat material may present challenges. This will be encountered especially for the construction on the west of IL 59. The Contractor should be prepared to install casing to the bottom of peat and soft soils. The approximate bottom of peat and soft soil for each pier is summarized below in Table 46. The shafts should be constructed in accordance with FHWA Publication NHI-10-016, *Drilled Shafts: Construction Procedures and LRFD Design Methods* (Brown et al. 2010).

Table 45: Approximate Bottom Elevation of Peat and Soft Soils

Pier Number	Boring Reference	Approximate Bottom Elevation of Peat and Soft Soil (feet)
1 to 4	BWB-01	771
5 to 9	BWB-02B	767
10 to 15	BWB-03B	748
16 to 20	BWB-04	746
21 to 25	BWB-05	750
26 to 30	BWB-06	738
31 to 33	TB-01	758
34	TB-02	753
35	B-01	754
36 to 37	B-02 and TB-03	NA
38 to 43	TB-04	769
44 to 50	BWB-07	764
51 to 55	BWB-08	761
56 to 83	BWB-09 to BWB-15	NA

Based on the proposed loads for different pier types, the 2-foot diameter drilled shafts will be established at elevations of about 730 to 758 feet. Based on soil and groundwater conditions encountered, temporary or permanent casings, slurry or other method approved by engineer would be required to prevent caving of granular layers and possible excess water flow.

For drilled constructed for Piers 50 to 87, the drilled shafts may be constructed using the dry method. However, the Contractor should be prepared for minor dewatering efforts during

construction. Further groundwater study is recommended. We do not anticipate a need for temporary or permanent casing. Concrete should be placed in dry holes. Any excess water in the excavation (if any) should be removed prior to concrete pouring.

Rig chatter and hard drilling conditions due to possible cobbles were observed in Borings B-01, B-02, TB-01 through TB-03, and BWB-07. The Contractor should be aware of this condition prior to casing installations and drilled shaft constructions.

Thick vegetation covers most of the area of proposed improvements. Moreover, we expect most of the drilled shafts west of the IL59 and some drilled shaft adjacent to IL 59 on the east side will be installed within persistent wet areas and extremely soft upper soils. A significant area of the project site is also subjected to frequent water ponding and flooding as shown in Exhibit 3-1. The contractor should be informed and be aware of these difficult site access conditions and the need to plan to establish a stable working platform. This special site access with the construction equipment should be included in the contractor's means and methods of constructions.

7.0 QUALIFICATIONS

The analysis and recommendations submitted in this report are based upon the data obtained from the borings drilled at the locations shown on the boring logs and in Exhibit 3. This report does not reflect any variations that may occur between the borings or elsewhere on the site, variations whose nature and extent may not become evident until the course of construction. In the event that any changes in the design and/or location of the new foundation are planned, we should be timely informed so that our recommendations can be adjusted accordingly.

It has been a pleasure to assist TranSystems and the Village of Streamwood on this project. Please call if there are any questions, or if we can be of further service.

Respectfully Submitted,

WANG ENGINEERING, INC.

Andri Kurnia, P.E.
Project Manager

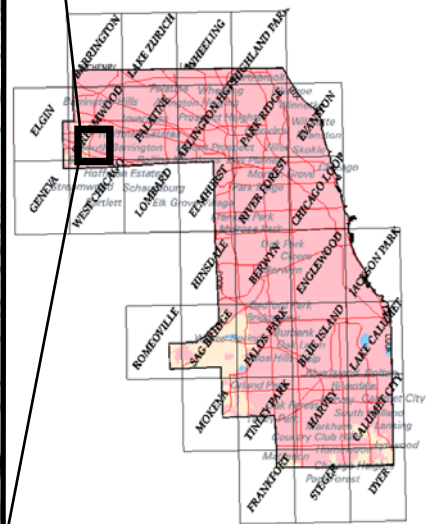
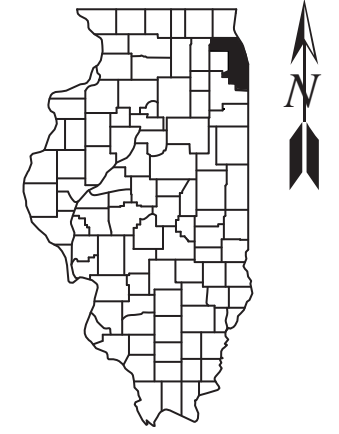
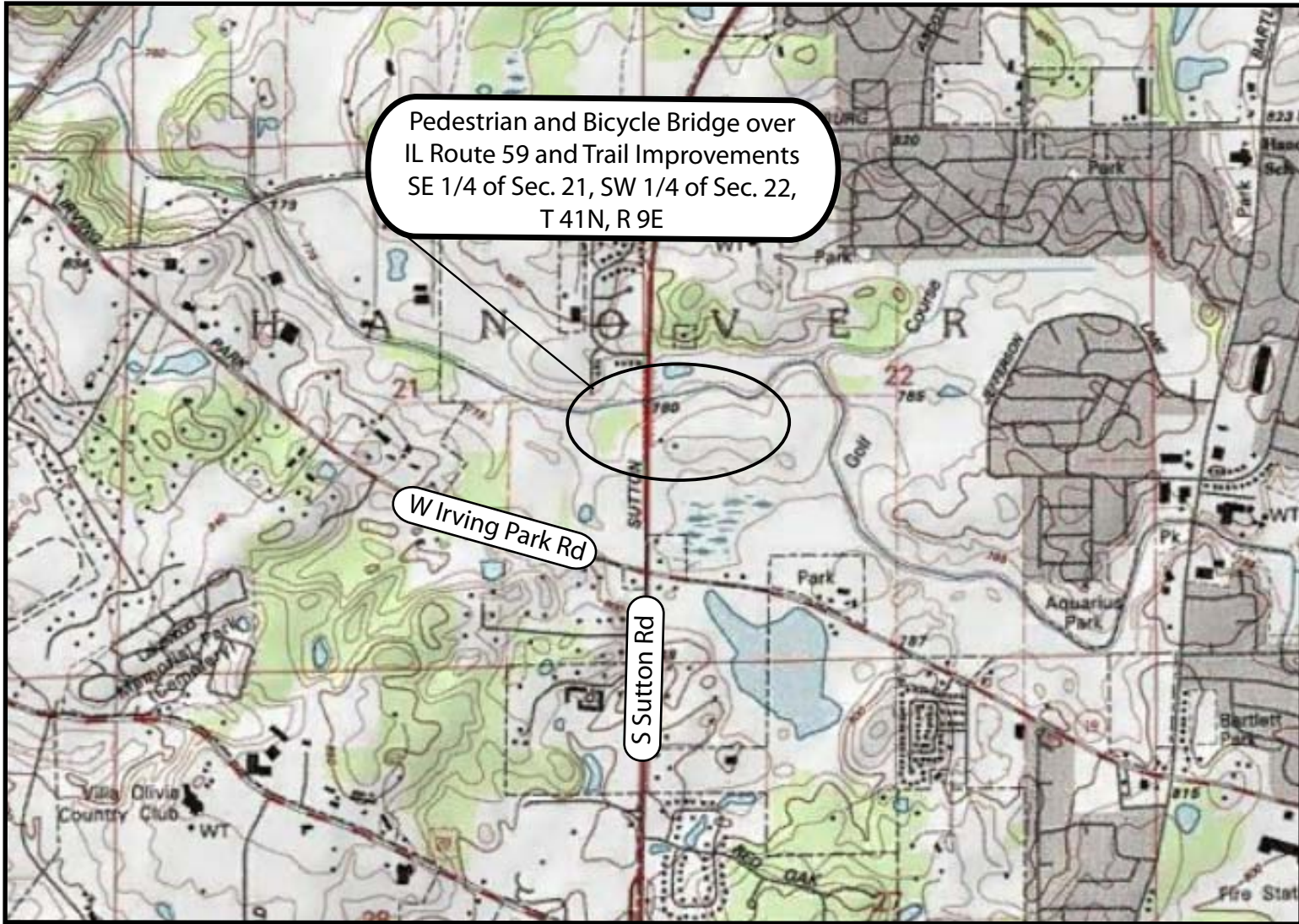
Nesam S. Balakumaran, P.Eng.
Project Geotechnical Engineer

Corina T. Farez, P.E., P.G.
QA/QC Reviewer

REFERENCES

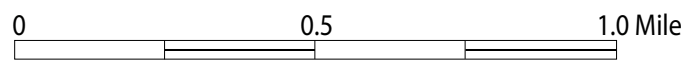
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EXHIBITS



Cook County

Scale

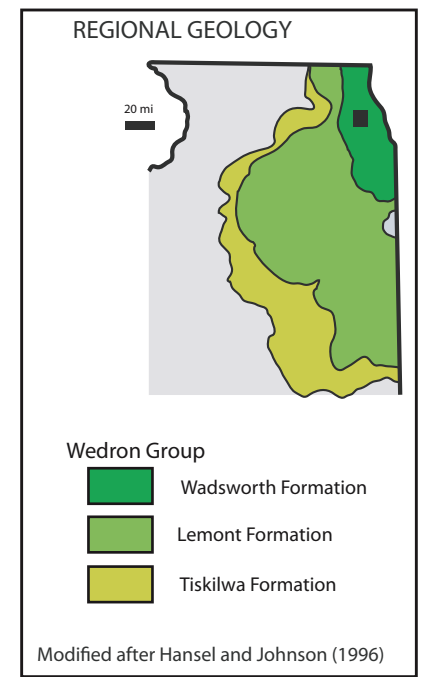
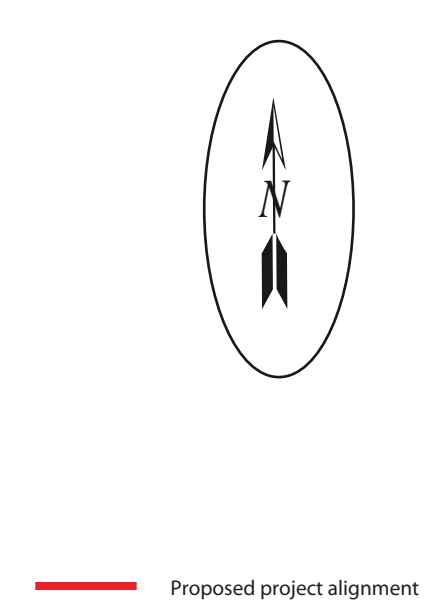
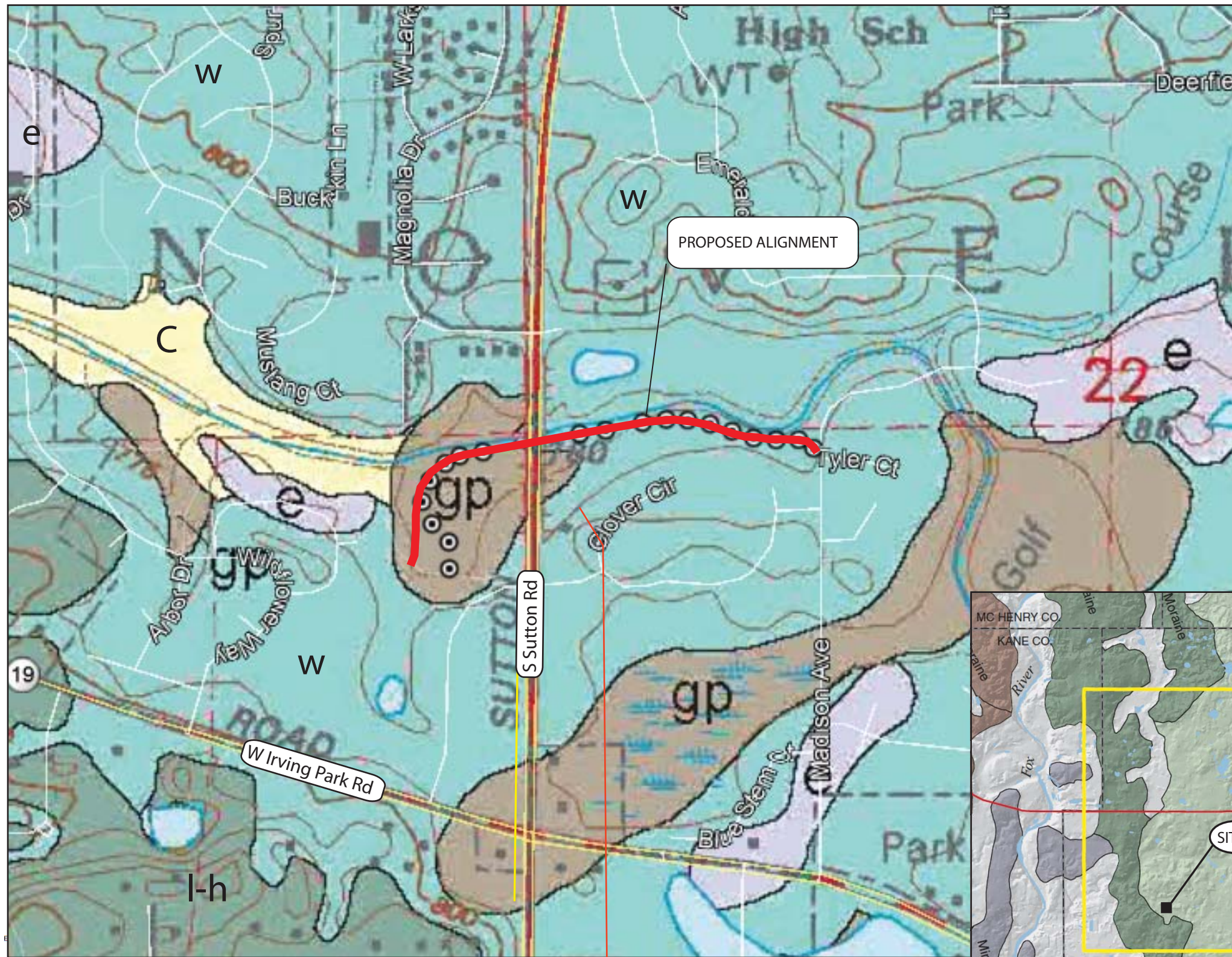


SITE LOCATION MAP: PEDESTRIAN AND BICYCLE BRIDGE OVER IL ROUTE 59 AND TRAIL IMPROVEMENTS, COOK COUNTY, ILLINOIS

SCALE: GRAPHICAL	EXHIBIT 1	DRAWN BY: J. Bensen CHECKED BY: A. Kurnia
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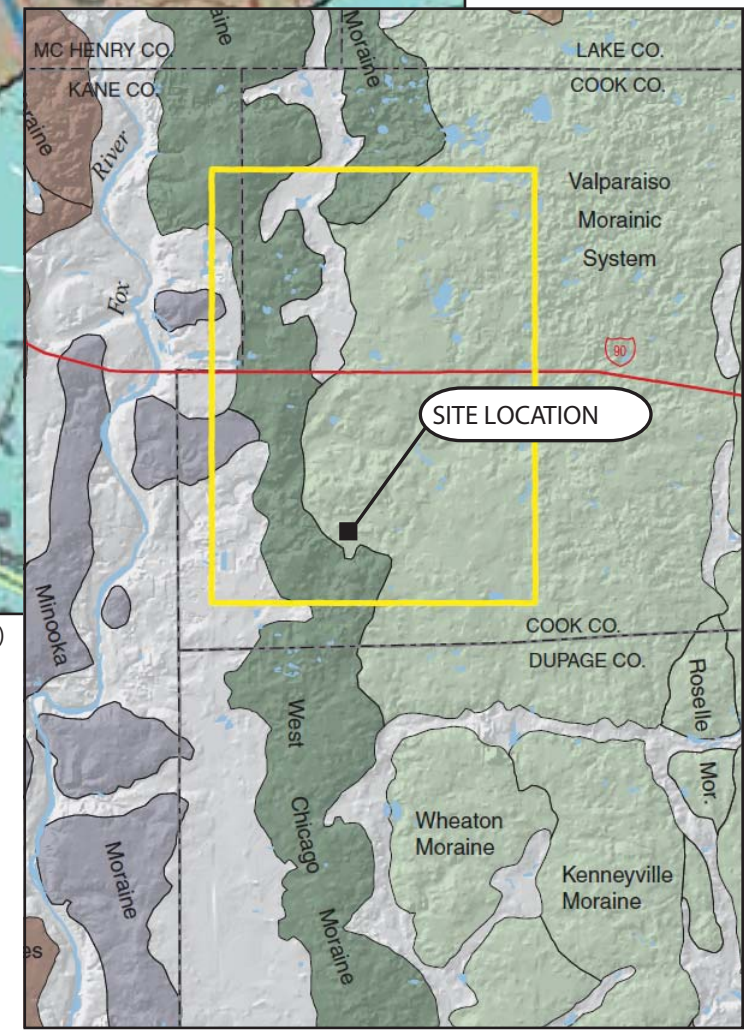
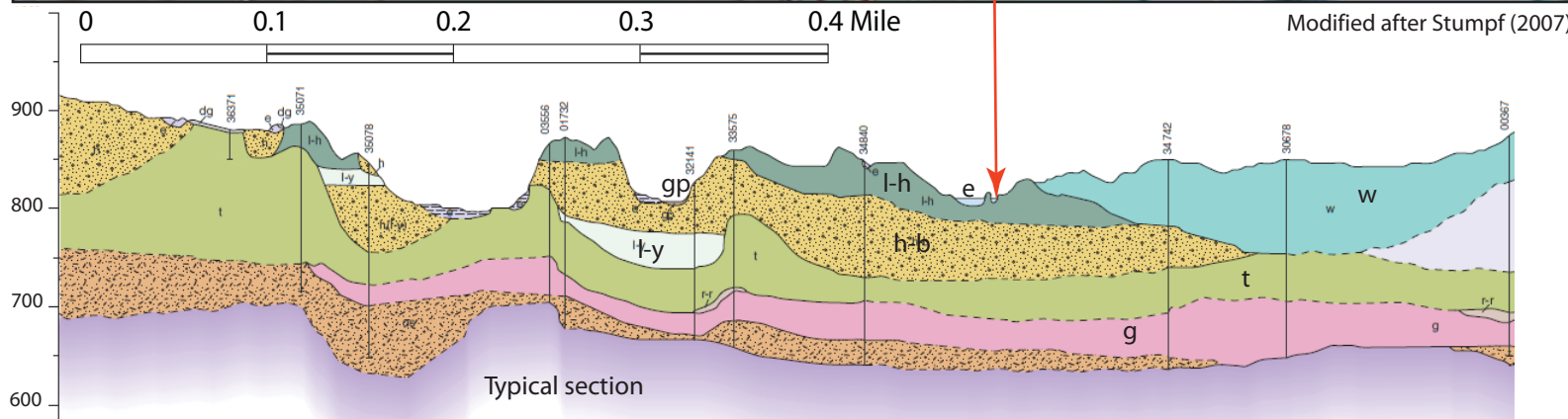
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7901-12-02



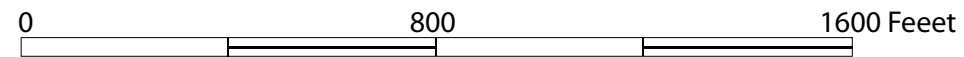
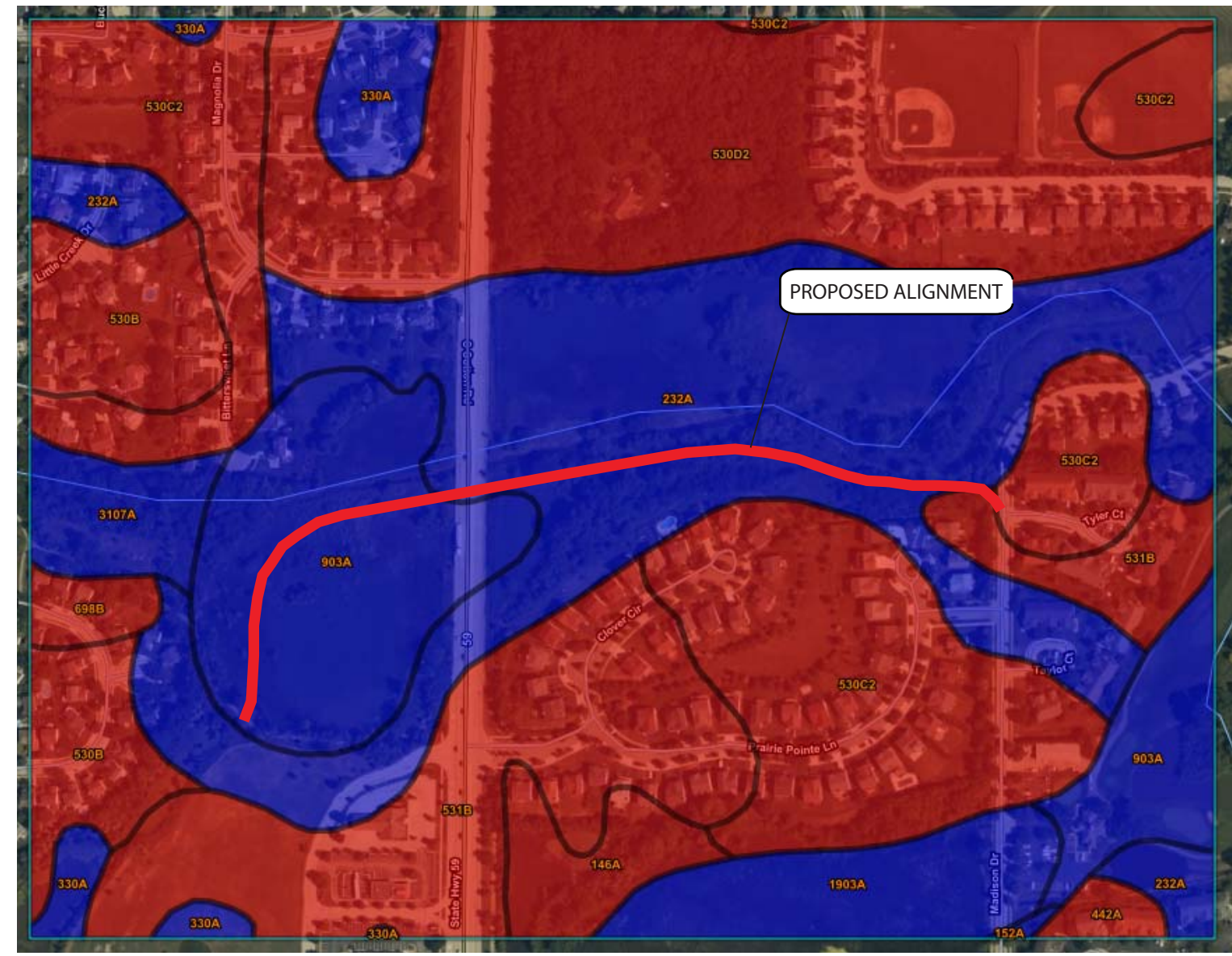
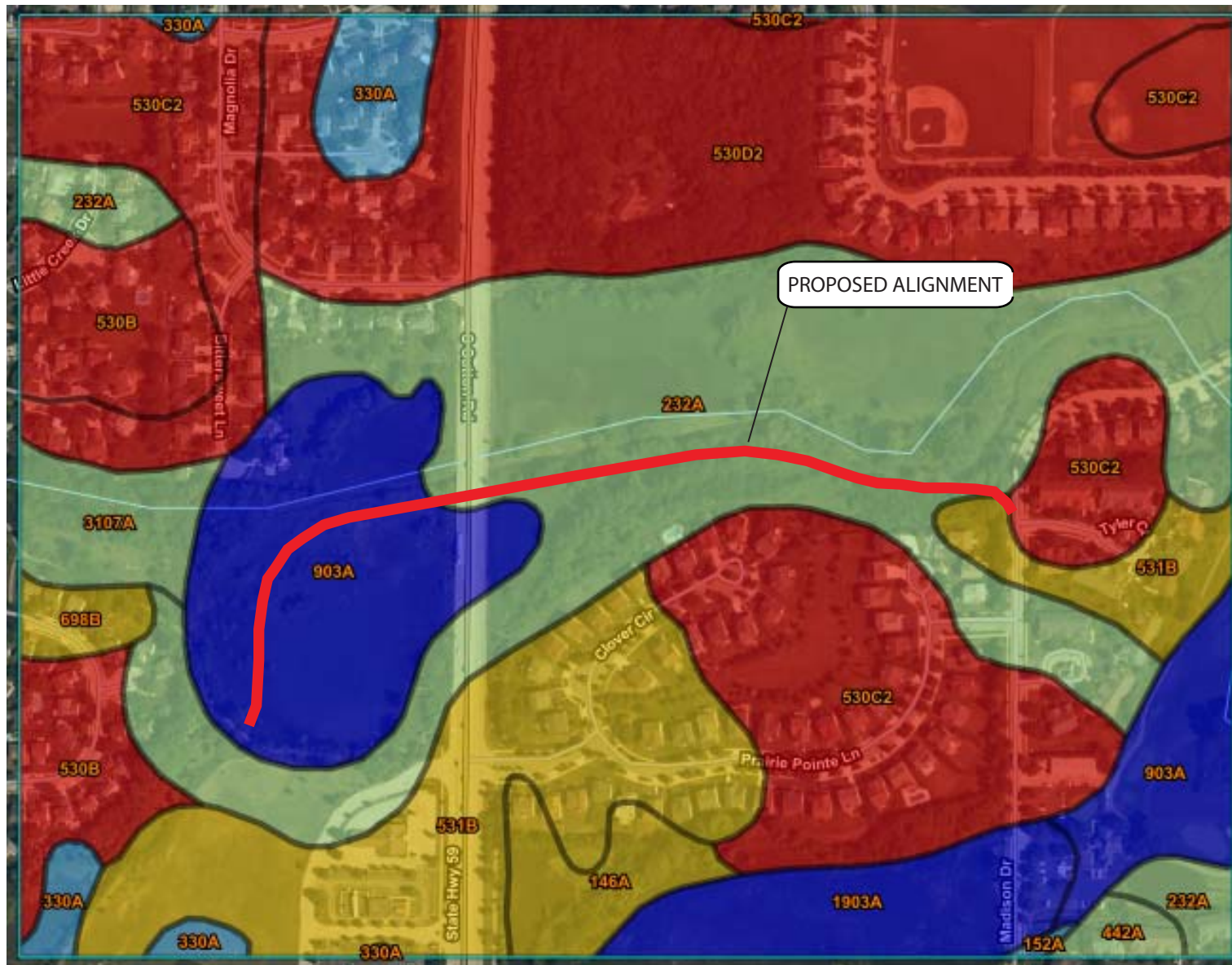
LEGEND
QUATERNARY DEPOSITS

- HUDSON EPISODE (~12,500 years before present [B.P.] to today)*
- C** **Cahokia Formation**
Silt and clay with occasional sand lenses; postglacial (modern) alluvial deposits present on active floodplains, natural levees
 - gp** **Grayslake Peat**
Decomposed wetland vegetation and sediment; peat and muck, may contain interbeds of silt, clay, and some fine sand
- WISCONSIN EPISODE (~25,000-12,500 years B.P.)*
- e** **Equality Formation**
Lake deposits in kettles and valleys; silt, clay, and fine sand; layered to massive
 - w** **Wadsworth Formation**
Diamicton; subglacial and ice-marginal sediment; gray to yellowish brown silty clay to silty clay loam, containing silt and sand inclusions and sand and gravel lenses
 - l-h** **Lemont Formation, Haeger Member**
Diamicton; subglacial and ice-marginal sediment; yellowish brown sandy loam to loam; dolomite rich; contains lenses or beds of sand and gravel
- Cross section only*
- h-b** **Henry Formation, Beverly Tongue**
 - l-y** **Lemont Formation, Yorkville Member**
 - t** **Tiskilwa Formation**
 - g** **Glasford Formation**



Modified after Stumpf (2007)

SITE GEOLOGICAL MAP: PEDESTRIAN AND BICYCLE BRIDGE OVER IL ROUTE 59 AND TRAIL IMPROVEMENTS, COOK COUNTY, ILLINOIS		
SCALE: GRAPHICAL	EXHIBIT 2	DRAWN BY: J. Bensen CHECKED BY: A. Kurnia
Wang Engineering		1145 N. Main Street Lombard, IL 60148 www.wangeng.com
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Organic Matter rating (%)

- ≤ 0.58 %
- > 0.58 and ≤ 1.02
- > 1.02 and ≤ 1.52
- > 1.52 and ≤ 2.39
- > 2.39 and ≤ 41.14

Ponding Frequency Class

- None
- Frequent

Frost Action rating

- Moderate
- High

Data Source Information
 Soil Survey Area: Cook County, Illinois
 Survey Area Data: Version 14, May 29, 2020

PEDOLOGY MAP AND DATA: PEDESTRIAN AND BICYCLE BRIDGE OVER IL ROUTE 59 AND TRAIL IMPROVEMENTS, COOK COUNTY, ILLINOIS		
SCALE: GRAPHICAL	EXHIBIT 3-1	DRAWN BY: J. Bensen CHECKED BY: A. Kurnia
		1145 N. Main Street Lombard, IL 60148 www.wangeng.com
FOR TRANSYSTEMS		7901-12-02

Map unit symbol and soil name	Depth <i>In</i>	USDA texture	Classification		Pct Fragments		Sand %	Silt %	Clay %	Moist bulk density <i>g/cc</i>	Saturated hydraulic conductivity <i>micro m/sec</i>	Organic matter %	Erosion factors			Liquid limit <i>L-R-H</i>	Plasticity index <i>L-R-H</i>	Hydrologic group
			Unified	AASHTO	>10	3-10							Kw	Kf	T			
					<i>inches</i> <i>L-R-H</i>	<i>inches</i> <i>L-R-H</i>												
146A—Elliott silt loam , 0 to 2 percent slopes																		
Elliott	0-6	Silt loam	CL, ML	A-6, A-7-6	0-0-0	0-0-0	2-10-15	58-65-76	22-25-27	1.30-1.40-1.45	4.23-9.17-14.11	3.0-4.3-5.0	0.32	0.32	4	38-44-47	15-17-18	C/D
	6-11	Silty clay loam	MH, CL, ML	A-7-6	0-0-0	0-0-0	2-8-15	50-62-71	27-30-35	1.25-1.35-1.45	4.23-9.17-14.11	2.5-3.3-4.0	0.28	0.28		41-46-53	18-21-24	
	11-16	Silty clay, silty clay loam	CL, CH	A-7-6	0-0-0	0-0-0	2-7-15	40-51-61	37-42-49	1.35-1.45-1.55	1.41-2.82-4.23	0.5-1.0-1.6	0.32	0.32		46-52-60	26-30-35	
	16-41	Silty clay loam, silty clay	CH, CL	A-7-6, A-6	0-0-0	0-0-1	2-10-20	40-55-65	27-35-45	1.45-1.55-1.75	0.42-1.41-4.23	0.1-0.4-0.8	0.37	0.37		34-43-55	17-24-32	
	41-60	Silty clay loam	CL	A-7-6, A-6	0-0-0	0-0-2	3-10-20	42-60-70	27-30-38	1.65-1.75-1.85	0.42-0.92-1.41	0.0-0.2-0.5	0.49	0.49		34-38-46	16-19-26	
152A—Drummer silty clay loam , 0 to 2 percent slopes																		
Drummer, drained	0-14	Silty clay loam	ML, MH, CL	A-7-6, A-7-5	0-0-0	0-0-0	0-8-15	50-61-73	27-31-35	1.20-1.30-1.42	4.23-9.17-14.11	3.5-5.5-7.0	0.24	0.24	5	44-51-58	18-21-24	B/D
	14-41	Silty clay loam, silt loam	CL	A-6, A-7-6	0-0-0	0-0-0	0-8-15	50-61-78	22-31-35	1.20-1.33-1.50	4.23-9.17-14.11	0.5-1.2-2.7	0.37	0.37		33-43-50	15-22-25	
	41-47	Loam, sandy loam, clay loam, silt loam	CL, SC	A-7-6, A-6, A-4	0-0-0	0-0-1	15-35-55	12-41-70	15-24-33	1.30-1.43-1.59	4.23-9.17-14.11	0.2-0.3-0.5	0.37	0.37		25-35-44	9-16-23	
	47-60	Stratified sandy loam to clay loam	CL, CL-ML, SC, SC-SM	A-4, A-6, A-7-6	0-0-0	0-1-4	20-48-65	3-31-53	12-21-32	1.45-1.55-1.65	4.23-11.64-14.11	0.0-0.1-0.4	0.32	0.32		22-31-42	6-14-23	
232A—Ashkum silty clay loam , 0 to 2 percent slopes																		
Ashkum, drained	0-12	Silty clay loam	CH, MH	A-7-6, A-7-5	0-0-0	0-0-0	1-8-15	45-55-64	35-37-40	1.20-1.35-1.45	1.41-2.82-4.23	3.0-5.0-8.0	0.20	0.20	5	51-58-67	25-26-28	C/D
	12-29	Silty clay loam, silty clay	CL, CH	A-7-6	0-0-0	0-0-0	2-8-15	43-51-63	35-41-42	1.30-1.40-1.50	1.41-2.82-4.23	0.5-1.3-2.5	0.32	0.32		46-54-58	25-30-30	
	29-54	Silty clay loam, silty clay	CL, CH	A-6, A-7-6	0-0-0	0-0-1	5-9-20	40-58-65	30-33-42	1.50-1.60-1.70	1.41-2.82-4.23	0.1-0.3-1.0	0.43	0.43		39-43-53	21-23-30	
	54-60	Silty clay loam	CL	A-6, A-7-6	0-0-0	0-0-1	5-9-20	45-61-68	27-30-35	1.55-1.65-1.75	1.41-2.82-4.23	0.0-0.3-1.0	0.43	0.43		37-41-47	19-21-25	
330A—Peotone silty clay loam , 0 to 2 percent slopes																		
Peotone, drained	0-7	Silty clay loam	MH	A-7-6, A-7-5	0-0-0	0-0-0	1-5-10	50-60-67	32-35-40	1.20-1.30-1.40	1.41-2.82-4.23	4.5-6.2-7.5	0.24	0.24	5	50-57-65	22-25-28	C/D
	7-27	Silty clay loam, silty clay	CL, MH, CH	A-7-6, A-7-5	0-0-0	0-0-0	1-5-10	45-56-64	35-39-45	1.30-1.40-1.55	1.41-2.82-4.23	1.5-3.2-6.0	0.28	0.28		48-56-68	25-28-32	
	27-50	Silty clay loam, silty clay	CL, CH	A-7-6	0-0-0	0-0-1	1-6-12	43-53-66	33-41-45	1.35-1.45-1.60	1.41-2.82-4.23	0.5-1.3-2.7	0.32	0.32		43-54-61	23-29-33	
	50-60	Silty clay loam, silt loam	CH, CL	A-6, A-7-6	0-0-0	0-0-3	1-11-20	40-55-74	25-34-40	1.40-1.53-1.65	1.41-2.82-4.23	0.0-0.5-1.2	0.37	0.37		34-44-52	17-24-29	
442A—Mundelein silt loam , 0 to 2 percent slopes																		
Mundelein	0-17	Silt loam	CL, ML	A-6, A-7-6	0-0-0	0-0-0	0-9-15	58-68-80	20-24-27	1.15-1.23-1.30	4.23-9.17-14.11	3.0-4.0-5.0	0.28	0.28	5	35-41-47	13-16-18	B/D
	17-31	Silty clay loam, silt loam	CL	A-6, A-7-6	0-0-0	0-0-0	0-9-15	50-61-75	25-30-35	1.20-1.33-1.45	4.23-9.17-14.11	0.5-1.3-2.0	0.37	0.37		36-43-49	17-21-25	
	31-42	Sandy loam, silt loam, clay loam, loam	CL, SC	A-4, A-6	0-0-0	0-0-0	10-25-60	10-52-75	15-23-30	1.40-1.48-1.55	4.23-9.17-14.11	0.2-0.3-0.5	0.43	0.43		25-33-40	9-15-21	
	42-60	Stratified sandy loam to silt loam	CL, CL-ML, SC-SM, SC	A-2-4, A-4, A-6, A-2-6	0-0-0	0-0-0	10-40-75	5-45-80	5-15-25	1.50-1.60-1.70	4.23-23.29-42.34	0.0-0.1-0.2	0.43	0.43		16-26-35	2-10-17	
530B—Ozaukee silt loam , 2 to 4 percent slopes																		
Ozaukee	0-4	Silt loam	CL, ML	A-4, A-7-6, A-6	0-0-0	0-0-1	7-14-23	52-67-76	15-19-27	1.30-1.40-1.50	4.23-9.17-14.11	1.2-2.0-3.0	0.43	0.43	3	28-33-43	9-12-18	C
	4-10	Silt loam	CL	A-6	0-0-0	0-0-1	5-10-18	57-69-76	17-21-27	1.35-1.45-1.55	4.23-9.17-14.11	0.3-0.6-1.0	0.55	0.55		27-32-38	11-15-19	
	10-21	Clay, silty clay loam, silty clay	CL	A-7-6, A-6	0-0-1	0-1-4	5-11-18	34-48-58	35-41-50	1.45-1.55-1.65	0.42-2.33-4.23	0.2-0.5-0.9	0.32	0.32		31-38-48	15-19-25	
	21-39	Silty clay loam, silty clay	CL	A-6	0-1-2	0-1-5	5-12-20	40-52-64	29-36-42	1.55-1.65-1.70	0.42-0.92-1.41	0.1-0.3-0.6	0.37	0.37		24-31-37	11-15-19	
	39-60	Silty clay loam, clay loam	CL	A-4, A-6	0-1-2	0-2-7	7-14-23	50-55-64	27-31-35	1.60-1.70-1.85	0.42-0.75-1.41	0.0-0.2-0.5	0.43	0.43		21-26-30	9-12-14	
530C2—Ozaukee silt loam , 4 to 6 percent slopes, eroded																		
Ozaukee, eroded	0-7	Silt loam	CL	A-6, A-7-6	0-0-0	0-0-1	5-12-22	53-66-75	18-22-27	1.30-1.43-1.55	4.23-9.17-14.11	1.0-1.7-2.5	0.43	0.43	3	30-35-42	12-14-18	C
	7-26	Silty clay loam, clay, silty clay	CL	A-7-6, A-6	0-0-1	0-1-4	5-11-18	34-48-58	35-41-50	1.45-1.55-1.65	0.42-2.33-4.23	0.2-0.5-0.9	0.32	0.32		31-38-48	15-19-25	
	26-37	Silty clay loam, silty clay	CL	A-6	0-1-2	0-1-5	5-12-20	40-52-64	29-36-42	1.55-1.65-1.70	0.42-0.92-1.41	0.1-0.3-0.6	0.37	0.37		24-31-37	11-15-19	
	37-60	Silty clay loam, clay loam	CL	A-6, A-4	0-1-2	0-2-7	7-14-23	50-55-64	27-31-35	1.60-1.70-1.85	0.42-0.75-1.41	0.0-0.2-0.5	0.43	0.43		21-26-30	9-12-14	

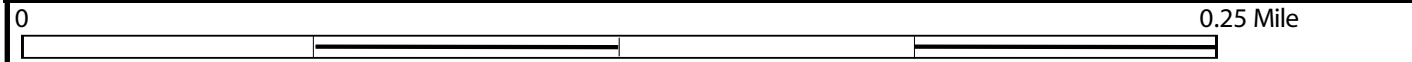
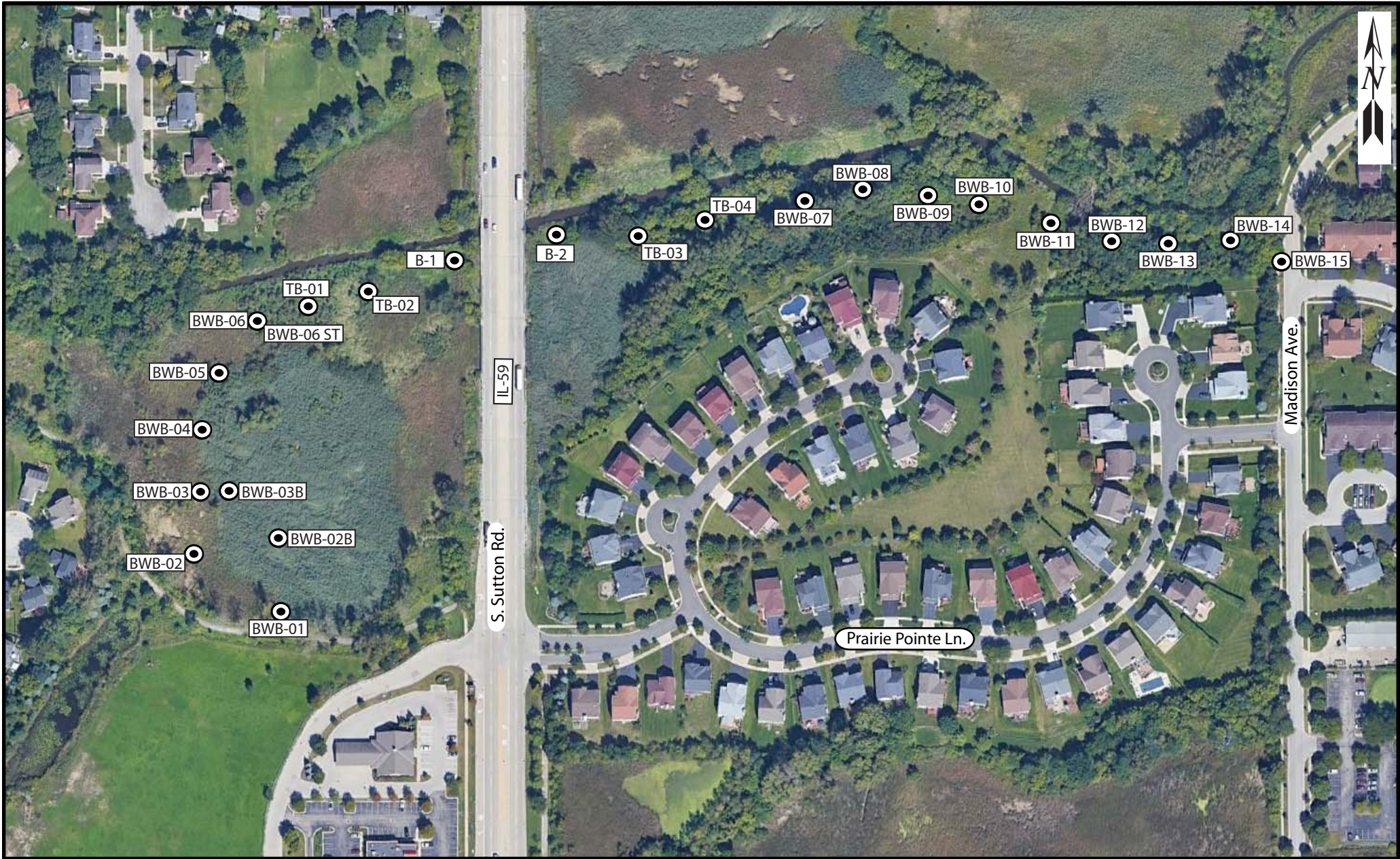
Data Source Information
Soil Survey Area: Cook County, Illinois
Survey Area Data: Version 14, May 29, 2020

PEDOLOGY MAP AND DATA: PEDESTRIAN AND BICYCLE BRIDGE OVER IL ROUTE 59 AND TRAIL IMPROVEMENTS, COOK COUNTY, ILLINOIS		
SCALE: GRAPHICAL	EXHIBIT 3-2	DRAWN BY: J. Bensen CHECKED BY: A. Kurnia
		1145 N. Main Street Lombard, IL 60148 www.wangeng.com
		FOR TRANSYSTEMS

Map unit symbol and soil name	Depth <i>In</i>	USDA texture	Classification		Pct Fragments		Sand %	Silt %	Clay %	Moist bulk density <i>g/cc</i>	Saturated hydraulic conductivity <i>micro m/sec</i>	Organic matter %	Erosion factors			Liquid limit <i>L-R-H</i>	Plasticity index <i>L-R-H</i>	Hydrologic group
			Unified	AASHTO	>10 inches <i>L-R-H</i>	3-10 inches <i>L-R-H</i>							Kw	Kf	T			
530D2—Ozaukee silt loam, 6 to 12 percent slopes, eroded																		
Ozaukee, eroded	0-7	Silt loam	CL	A-6, A-7-6	0-0-0	0-0-1	7-14-23	52-65-73	18-21-27	1.30-1.45-1.55	4.23-9.17-14.11	1.0-1.7-2.5	0.43	0.43	3	30-35-42	12-15-19	C
	7-11	Silty clay loam, silt loam	CL	A-6, A-7-6	0-0-0	0-0-1	5-10-18	50-59-69	24-31-34	1.40-1.50-1.60	4.23-9.17-14.11	0.3-0.6-1.0	0.43	0.43		34-41-45	16-21-24	
	11-27	Silty clay, silty clay loam, clay	CL, CH	A-7-6, A-6	0-0-1	0-1-4	5-11-18	34-48-58	35-41-50	1.45-1.55-1.65	0.42-2.33-4.23	0.2-0.5-0.9	0.32	0.32		30-38-52	15-19-26	
	27-32	Silty clay loam, silty clay	CL	A-6	0-1-2	0-1-5	5-12-20	40-52-64	29-36-42	1.55-1.65-1.70	0.42-0.92-1.41	0.1-0.3-0.6	0.37	0.37		24-31-37	11-15-19	
	32-60	Silty clay loam, clay loam	CL	A-6, A-4	0-1-2	0-2-7	7-14-23	50-55-64	27-31-35	1.65-1.75-1.85	0.42-0.75-1.41	0.0-0.2-0.5	0.43	0.43		21-26-31	9-12-15	
531B—Markham silt loam, 2 to 4 percent slopes																		
Markham	0-8	Silt loam	ML, CL	A-7-6, A-6	0-0-0	0-0-1	5-10-15	58-66-75	20-24-27	1.30-1.40-1.50	4.23-9.17-14.11	2.0-3.0-4.0	0.37	0.37	3	34-41-46	13-16-19	C
	8-21	Clay, silty clay, silty clay loam	CH, CL	A-7-6	0-0-1	0-1-4	5-12-20	30-49-60	35-39-50	1.40-1.50-1.60	0.42-2.33-4.23	0.2-0.6-1.0	0.37	0.37		43-48-60	25-28-36	
	21-32	Silty clay loam, silty clay	CH, CL	A-6, A-7-6	0-1-1	0-3-4	5-12-20	40-52-65	30-36-45	1.55-1.65-1.75	0.42-0.92-1.41	0.1-0.3-0.5	0.37	0.37		37-44-53	19-25-32	
	32-60	Clay loam, silty clay loam	CL	A-7-6, A-6	0-1-1	0-2-4	5-15-25	40-53-68	27-32-38	1.65-1.75-1.85	0.42-0.92-1.41	0.0-0.3-0.5	0.43	0.43		34-40-47	17-22-27	
696C2—Zurich silt loam, 4 to 6 percent slopes, eroded																		
Zurich, eroded	0-10	Silt loam	CL, ML	A-7-6, A-4, A-6	0-0-0	0-0-0	0-9-15	58-68-85	15-23-27	1.25-1.35-1.45	4.23-9.17-14.11	1.0-1.5-2.0	0.43	0.43	5	27-35-41	9-15-19	C
	10-27	Silty clay loam, silt loam	CL	A-6, A-7-6	0-0-0	0-0-0	0-9-15	50-61-75	25-30-35	1.20-1.33-1.45	4.23-9.17-14.11	0.2-0.6-1.0	0.43	0.43		35-41-47	17-21-25	
	27-40	Silt loam, sandy loam, loam	CL, SC, CL-ML, SC-SM	A-6, A-4	0-0-0	0-0-0	10-25-60	13-53-75	10-22-27	1.45-1.55-1.65	4.23-9.17-14.11	0.2-0.3-0.5	0.43	0.43		21-32-38	6-15-19	
	40-60	Stratified loamy sand to silt loam	CL, SC- SM, SC, CL- ML	A-2-4, A-4, A-6, A-2-6	0-0-1	0-1-4	10-45-85	0-37-80	5-18-25	1.50-1.63-1.75	4.23-23.29-42.34	0.0-0.3-0.5	0.32	0.32		16-28-36	2-12-17	
698B—Grays silt loam, 2 to 4 percent slopes																		
Grays	0-8	Silt loam	CL, ML	A-7-6, A-4, A-6	0-0-0	0-0-0	0-9-15	58-70-85	15-21-27	1.15-1.23-1.30	4.23-9.17-14.11	2.0-3.0-4.0	0.37	0.37	5	29-37-45	9-14-18	C
	8-11	Silt loam	CL	A-4, A-6	0-0-0	0-0-0	0-9-15	58-71-85	15-20-27	1.20-1.28-1.35	4.23-9.17-14.11	0.5-0.8-1.0	0.49	0.49		26-31-39	9-13-19	
	11-34	Silty clay loam, silt loam	CL	A-6, A-7-6	0-0-0	0-0-0	0-9-15	50-61-75	25-30-35	1.25-1.35-1.45	4.23-9.17-14.11	0.2-0.6-1.0	0.43	0.43		35-41-47	17-21-25	
	34-42	Silt loam, sandy loam, loam	CL, SC, CL-ML, SC-SM	A-6, A-4	0-0-0	0-0-0	15-42-60	15-42-75	8-17-25	1.40-1.50-1.60	4.23-9.17-14.11	0.2-0.3-0.5	0.37	0.37		19-27-36	4-11-17	
	42-60	Stratified loamy sand to silt loam	CL, SC- SM, SC, CL- ML	A-2-4, A-4, A-6, A-2-6	0-0-1	0-1-4	15-45-85	0-43-80	5-13-20	1.50-1.60-1.70	4.23-23.29-42.34	0.0-0.1-0.2	0.43	0.43		16-23-31	2-8-13	
903A—Muskego and Houghton mucks, 0 to 2 percent slopes																		
Muskego	0-5	Muck	PT	A-8	—	—	—	—	—	0.10-0.16-0.21	4.23-23.29-42.34	60.0-75.0-90.0			1	0-0-0	NP	C/D
	5-36	Muck	PT	A-8	—	—	—	—	—	0.10-0.16-0.21	4.23-23.29-42.34	60.0-75.0-90.0				0-0-0	NP	
	36-80	Coprogenous silt loam, coprogenous silty clay loam, coprogenous earth	MH, OH, OL	A-7-5	0-0-0	0-0-0	4-14-25	40-60-78	18-26-35	0.30-0.70-1.10	0.42-0.92-1.41	6.0-13.0-20.0	0.37	0.37		42-64-86	12-17-23	
Houghton	0-19	Muck	PT	A-8	—	—	—	—	—	0.20-0.28-0.35	1.41-21.88-42.34	70.0-84.5-99.0			2	0-0-0	NP	A/D
	19-60	Muck	PT	A-8	—	—	—	—	—	0.15-0.20-0.25	1.41-21.88-42.34	70.0-84.5-99.0				0-0-0	NP	
1903A—Muskego and Houghton mucks, undrained, 0 to 2 percent slopes																		
Muskego	0-5	Muck	PT	A-8	—	—	—	—	—	0.10-0.16-0.21	4.23-23.29-42.34	60.0-75.0-90.0			1	0-0-0	NP	C/D
	5-27	Muck	PT	A-8	—	—	—	—	—	0.10-0.16-0.21	4.23-23.29-42.34	60.0-75.0-90.0				0-0-0	NP	
	27-60	Coprogenous silt loam, coprogenous silty clay loam, coprogenous earth	OH, MH, OL	A-7-5	0-0-0	0-0-0	4-14-25	40-60-78	18-26-35	0.30-0.70-1.10	0.42-0.92-1.41	6.0-13.0-20.0	0.37	0.37		42-64-86	12-17-23	
Houghton	0-19	Muck	PT	A-8	—	—	—	—	—	0.20-0.28-0.35	1.41-21.88-42.34	70.0-84.5-99.0			2	0-0-0	NP	A/D
	19-60	Muck	PT	A-8	—	—	—	—	—	0.15-0.20-0.25	1.41-21.88-42.34	70.0-84.5-99.0				0-0-0	NP	
3107A—Saw mill silty clay loam, heavy till plain, 0 to 2 percent slopes, frequently flooded																		
Saw mill, heavy till plain, frequently flooded	0-8	Silty clay loam	CL, CH	A-7-6, A-6	0-0-0	0-0-0	2-7-15	50-62-71	27-31-35	1.43-1.44-1.46	4.23-9.17-14.11	1.0-3.0-4.0	0.28	0.28	5	39-47-53	19-22-25	B/D
	8-30	Silty clay loam	CL, CH	A-7-6, A-6	0-0-0	0-0-0	2-10-15	50-59-71	27-31-35	1.30-1.34-1.39	4.23-9.17-14.11	1.0-2.0-4.0	0.32	0.32		39-45-53	19-22-25	
	30-51	Silty clay loam, clay loam, loam, silt loam	CL	A-7-6, A-6	0-0-0	0-0-0	3-11-25	45-59-72	20-30-35	1.35-1.38-1.41	4.23-9.17-14.11	0.0-0.8-2.0	0.43	0.43		31-42-49	14-21-25	
	51-65	Clay loam, silty clay loam, silt loam, loam	CL	A-6, A-7-6	0-0-0	0-0-0	5-11-25	40-65-77	18-24-35	1.40-1.42-1.44	4.23-9.17-14.11	0.0-0.4-1.0	0.49	0.49		29-39-47	12-19-25	

Data Source Information
Soil Survey Area: Cook County, Illinois
Survey Area Data: Version 14, May 29, 2020

PEDOLOGY MAP AND DATA: PEDESTRIAN AND BICYCLE BRIDGE OVER IL ROUTE 59 AND TRAIL IMPROVEMENTS, COOK COUNTY, ILLINOIS		
SCALE: GRAPHICAL	EXHIBIT 3-3	DRAWN BY: J. Bensen CHECKED BY: A. Kurnia
		1145 N. Main Street Lombard, IL 60148 www.wangeng.com
FOR TRANSYSTEMS		7901-12-02



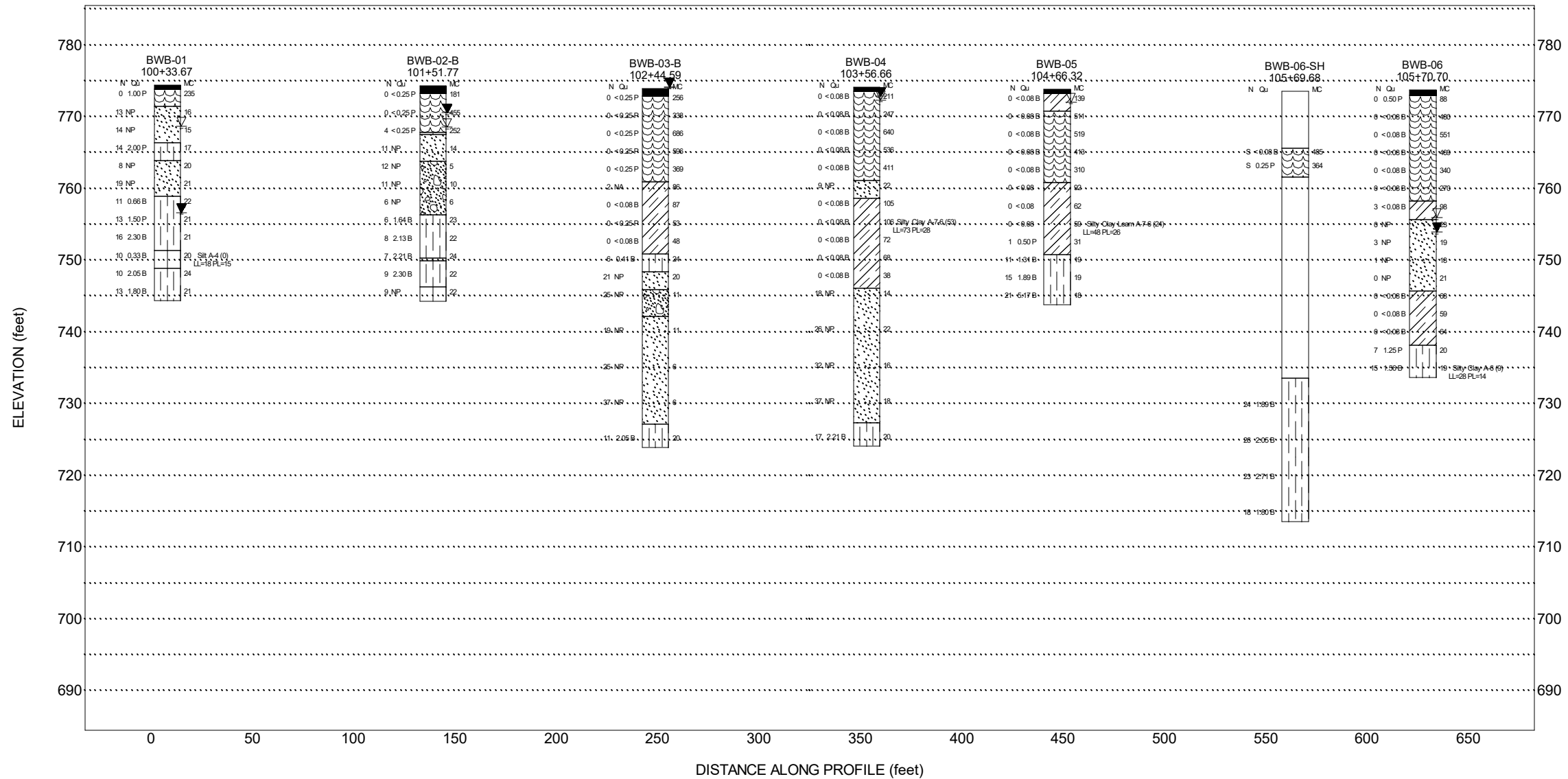
BORING LOCATION PLAN: PEDESTRIAN AND BICYCLE BRIDGE OVER IL ROUTE 59 AND TRAIL IMPROVEMENTS, COOK COUNTY, ILLINOIS

SCALE: GRAPHICAL	EXHIBIT 4	DRAWN BY: J. Bensen CHECKED BY: A. Kurmia
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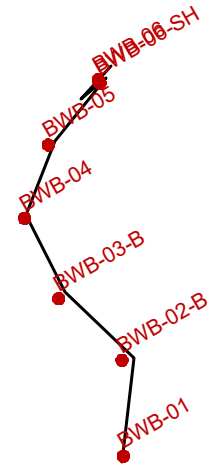
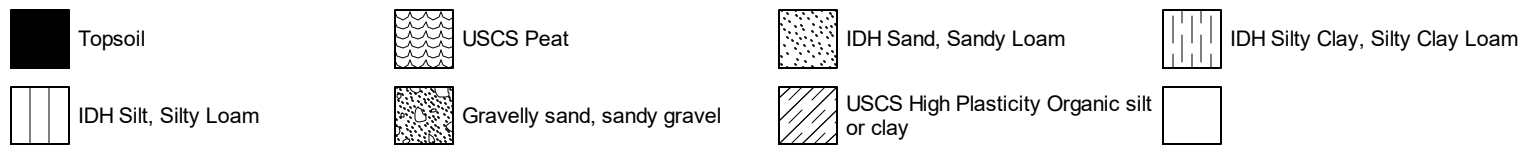
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Legend
 ● Boring Location

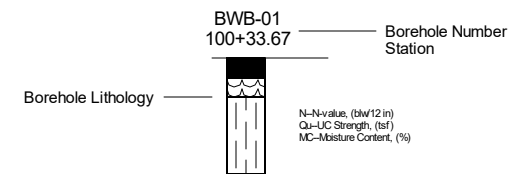


Lithology Graphics

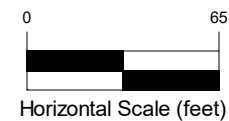


Site Map Scale 1 inch equals 240 feet

Explanation:



- ▽ Water Level Reading at time of drilling.
- ▼ Water Level Reading 24-hr after drilling or at end of drilling



Vertical Exaggeration: 3.5x

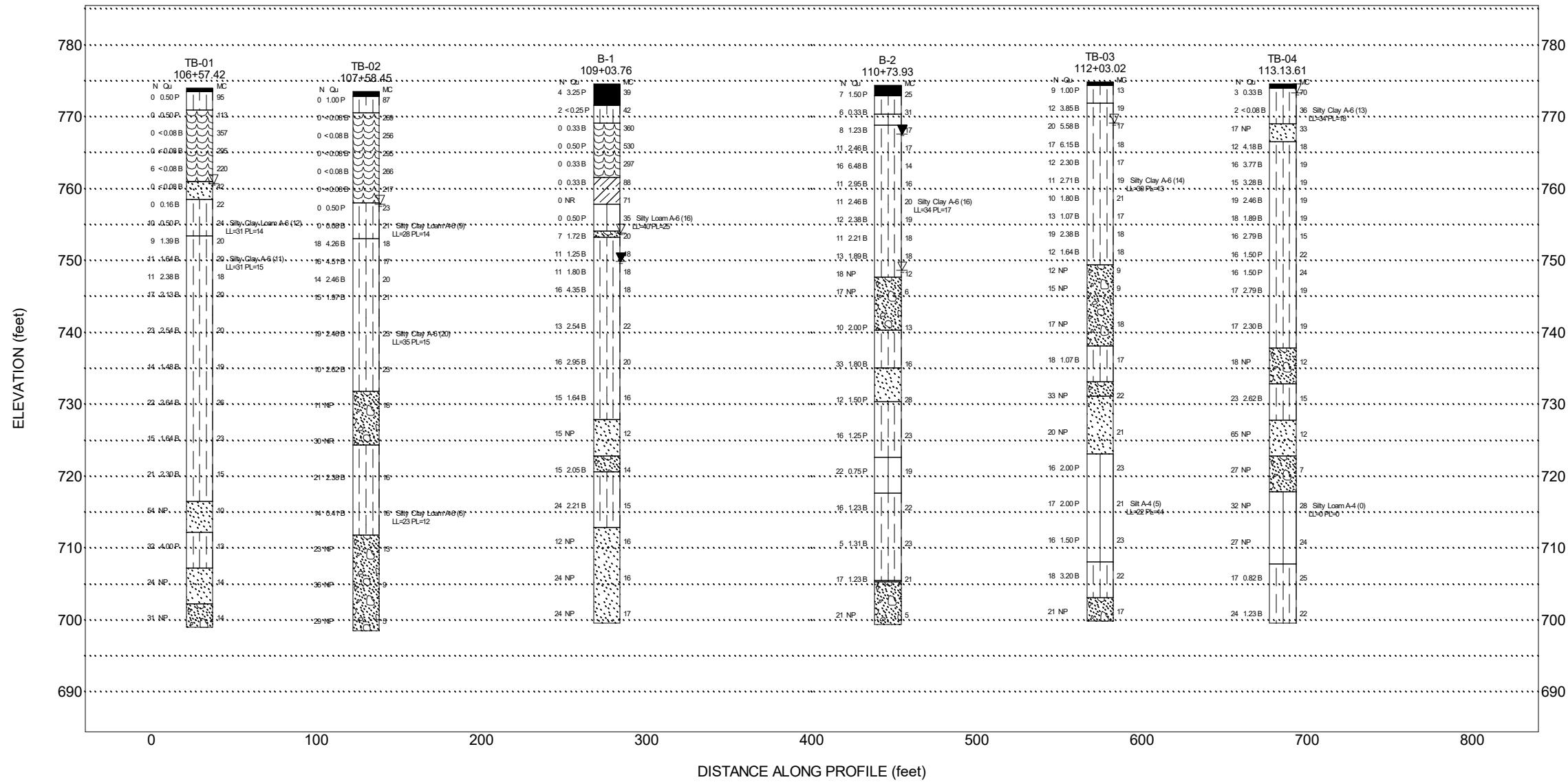
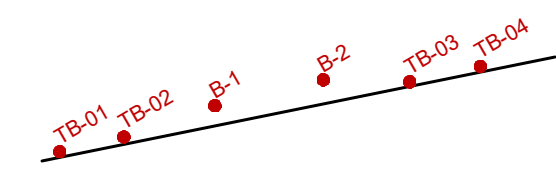
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Lombard, IL 60148

Soil Profile

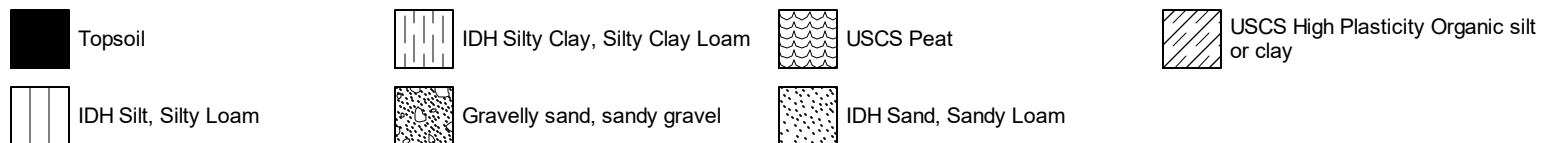


Pedestrian Bridge and Trail over IL 59
Streamwood, IL

JOB NUMBER	PLATE NUMBER
7901-12-02	EXHIBIT 5-1

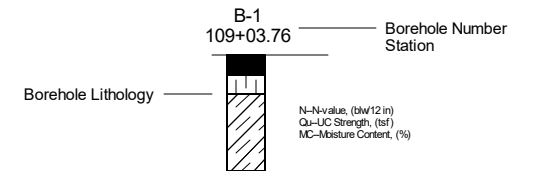


Lithology Graphics

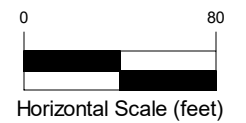


Site Map Scale 1 inch equals 295 feet

Explanation:



- Water Level Reading at time of drilling.
- Water Level Reading 24-hr after drilling or at end of drilling



Vertical Exaggeration: 4.5x

Wang Engineering
1145 N Main Street
Lombard, IL 60148

Soil Profile

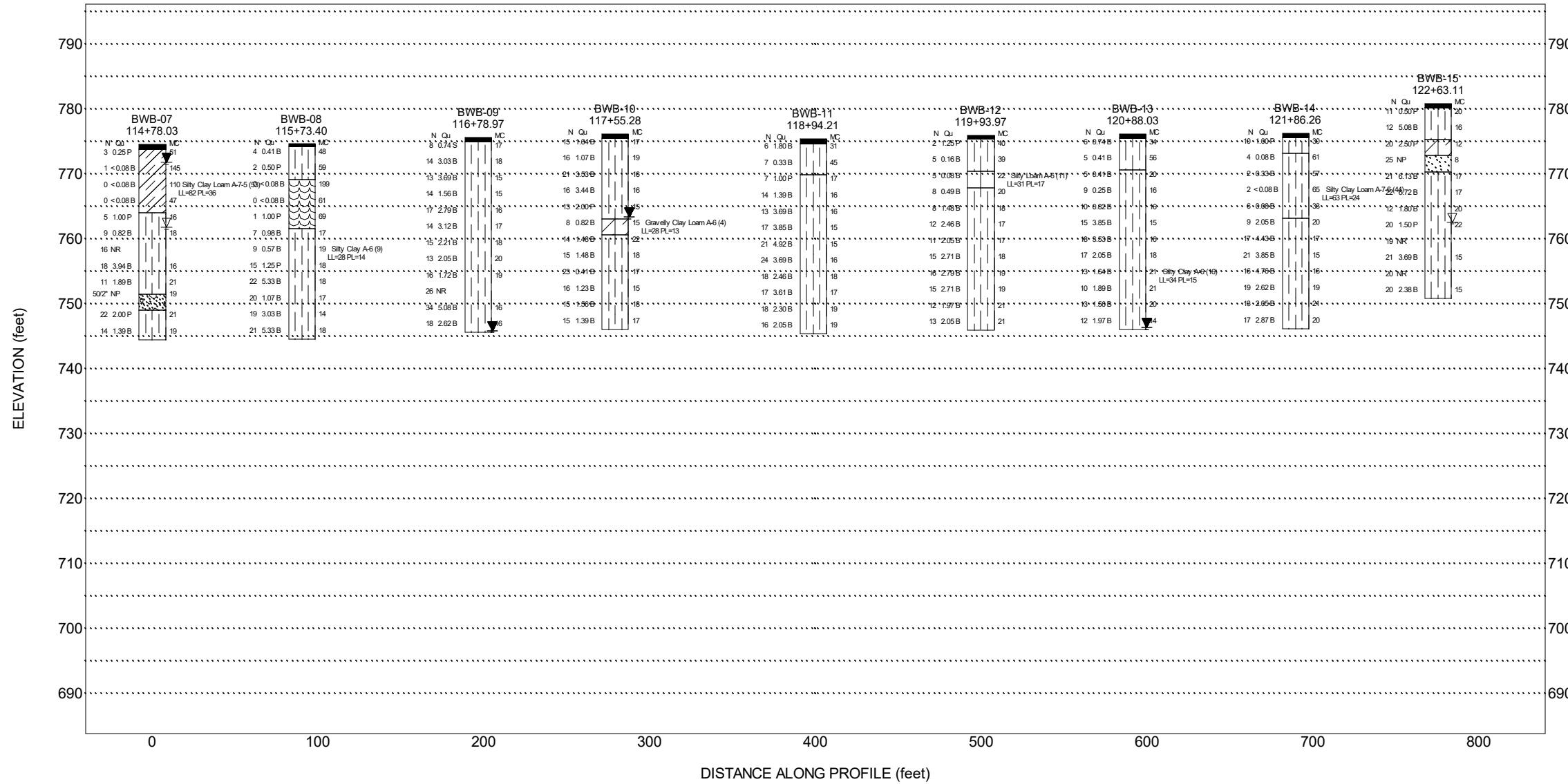


Pedestrian Bridge and Trail over IL 59
Streamwood, IL

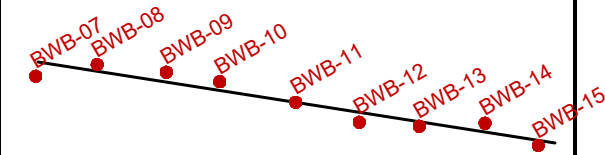
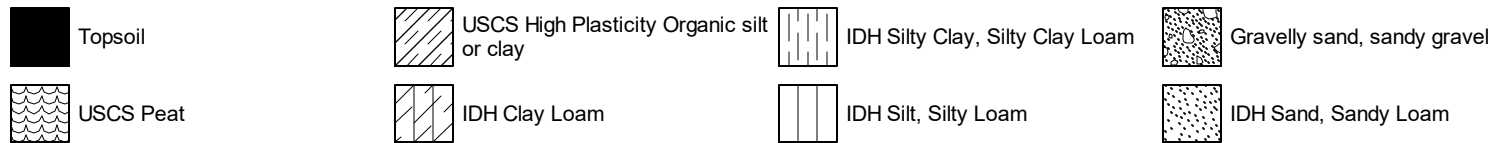
JOB NUMBER	PLATE NUMBER
7901-12-02	EXHIBIT 5-2



N

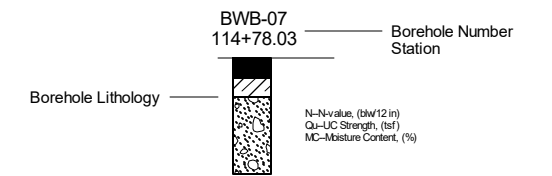


Lithology Graphics

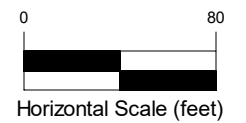


Site Map Scale 1 inch equals 295 feet

Explanation:



- Water Level Reading at time of drilling.
- Water Level Reading 24-hr after drilling or at end of drilling



Vertical Exaggeration: 4x

Wang Engineering
 1145 N Main Street
 Lombard, IL 60148

Soil Profile



Pedestrian Bridge and Trail over IL
 59
 Streamwood, IL

JOB NUMBER	PLATE NUMBER
7901-12-02	EXHIBIT 5-3

APPENDIX A



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 1145 N Main Street
 Lombard, IL 60148
 Telephone: 630 953-9928
 Fax: 630 953-9938

BORING LOG B-1

WEI Job No.: 7901-12-02

Client **TranSystems Corporation**
 Project **Pedestrian Bridge and Trail over IL 59**
 Location **Streamwood, IL**

Datum: NAVD 88
 Elevation: 774.64 ft
 North: 1951349.79 ft
 East: 1019593.51 ft
 Station: 109+03.76
 Offset: 17.00 RT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
		Very stiff, brown SILTY CLAY LOAM; damp --TOPSOIL--			1	3 2 2	3.25 P	39		754.1	--%Clay=19.4-- --RDR 2--						
	771.6	Very soft, gray SILTY CLAY LOAM, trace organic matter; wet --RDR 2--			2	1 1 1	< 0.25 P	42		753.3	Gray SANDY GRAVEL, some clay; saturated Stiff to hard, gray SILTY CLAY to SILTY CLAY LOAM, trace gravel; damp to moist			9	2 3 4	1.72 B	20
	769.1	Soft to medium stiff, brown PEAT; moist to wet --RDR 2--			3	0 0 0	0.33 B	360			--RDR 2--			10	3 4 7	1.25 B	18
		--Organic content= 54.1%--			4	0 0 0	0.50 P	530						11	3 5 6	1.80 B	18
					5	0 0 0	0.33 B	297			--water exited through auger flights from the surface of boring while drilling--			12	5 7 9	4.35 B	18
	761.6	Very loose, gray, organic SILTY LOAM; wet --RDR 2--			6	0 0 0	0.33 B	88						13	3 5 8	2.54 B	22
	757.9	Medium stiff, gray SILTY LOAM to SILTY CLAY LOAM, trace organic matter; wet --L _L (%)=40, P _L (%)=25-- --%Gravel=0.1-- --%Sand=3.5-- --%Silt=77.0--			7	0 0 0	NR	71									
					8	0 0 0	0.50 P	35						14	6 7 9	2.95 B	20

GENERAL NOTES

Begin Drilling **09-04-2020** Complete Drilling **09-04-2020**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&J** Logger **E. Yim** Checked by **C. Marin**
 Drilling Method **2.25" IDA HSA, autohammer, backfilled upon completion**

WATER LEVEL DATA

While Drilling ∇ **21.00 ft**
 At Completion of Drilling ∇ **25.00 ft**
 Time After Drilling **NA**
 Depth to Water ∇ **NA**

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.

WANGENGINC 79011202.GPJ WANGENG.GDT 8/26/21



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 Lombard, IL 60148
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 Fax: 630 953-9938

BORING LOG B-1

WEI Job No.: 7901-12-02

Client: **TranSystems Corporation**
 Project: **Pedestrian Bridge and Trail over IL 59**
 Location: **Streamwood, IL**

Datum: NAVD 88
 Elevation: 774.64 ft
 North: 1951349.79 ft
 East: 1019593.51 ft
 Station: 109+03.76
 Offset: 17.00 RT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	727.9	--slow hard drilling-- --rig chatter; possible cobbles-- Medium dense, gray, fine to medium SAND, some gravel; saturated								712.9	--hard drilling-- Medium dense, gray, medium SAND to SANDY LOAM; saturated						
		--RDR 4--	45		15	6 7 8	1.64 B	16			--RDR 2-3--	65		19	3 4 8	NP	16
			50		16	5 9 6	NP	12				70		20	11 11 13	NP	16
	722.9	Gray GRAVELLY SAND; saturated															
		--RDR 3--															
	720.6	Very stiff, gray SILTY CLAY, trace gravel; damp to moist															
		--RDR 2--	55		17	4 7 8	2.05 B	14		699.6	Boring terminated at 75.00 ft	75		21	7 9 15	NP	17
			60		18	8 10 14	2.21 B	15				80					

GENERAL NOTES

WATER LEVEL DATA

Begin Drilling **09-04-2020** Complete Drilling **09-04-2020**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&J** Logger **E. Yim** Checked by **C. Marin**
 Drilling Method **2.25" IDA HSA, autohammer, backfilled upon completion**

While Drilling ▽ **21.00 ft**
 At Completion of Drilling ▼ **25.00 ft**
 Time After Drilling **NA**
 Depth to Water ▼ **NA**

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.

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BORING LOG B-2

WEI Job No.: 7901-12-02

Client **TranSystems Corporation**
 Project **Pedestrian Bridge and Trail over IL 59**
 Location **Streamwood, IL**

Datum: NAVD 88
 Elevation: 774.39 ft
 North: 1951389.28 ft
 East: 1019759.30 ft
 Station: 110+73.93
 Offset: 26.31 LT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
		17-inch thick, black SILTY CLAY LOAM --TOPSOIL--									wet--						
	773.0	Soft to stiff, brown to gray SILTY CLAY; moist to wet --RDR 2--			1	7 4 3	1.50 P	25						9	4 5 6	2.21 B	18
	770.4	Loose, brown SILT; moist --rig chatter; possible cobbles--			2	2 2 4	0.33 B	31				25		10	4 5 8	1.89 B	18
	768.9	Stiff to hard, gray SILTY CLAY, trace gravel; damp to moist --RDR 2-3--			3	2 3 5	1.23 B	17						11	6 9 9	NP	12
					4	3 5 6	2.46 B	17						12	6 7 10	NP	6
					5	4 6 10	6.48 B	14									
		--rig chatter; possible cobbles--			6	3 5 6	2.95 B	16						13	9 6 4	2.00 P	13
		--L _L (%)=34, P _L (%)=17-- --%Gravel=1.9-- --%Sand=3.6-- --%Silt=54.7-- --%Clay=39.8--			7	4 4 7	2.46 B	20									
		--2-inch thick, silty clay loam lens; 20			8	5 5 7	2.38 B	19						14	6 16 17	1.80 B	16

GENERAL NOTES

Begin Drilling **09-02-2020** Complete Drilling **09-02-2020**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&J** Logger **E. Yim** Checked by **C. Marin**
 Drilling Method **2.25" IDA HSA, autohammer, backfilled upon completion**

WATER LEVEL DATA

While Drilling **26.00 ft**
 At Completion of Drilling **7.00 ft**
 Time After Drilling **NA**
 Depth to Water **NA**

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.

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BORING LOG B-2

WEI Job No.: 7901-12-02

Client **TranSystems Corporation**
 Project **Pedestrian Bridge and Trail over IL 59**
 Location **Streamwood, IL**

Datum: NAVD 88
 Elevation: 774.39 ft
 North: 1951389.28 ft
 East: 1019759.30 ft
 Station: 110+73.93
 Offset: 26.31 LT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
		saturated --RDR 2--									--water exited through auger flights from the surface of boring while drilling--						
	730.4	Stiff, gray SILTY CLAY; moist --RDR 2-- --laminated sand and silt seams; wet--	45		15	7 5 7	1.50 P	28			--laminated sand and silt seams; saturated--	65		19	3 2 3	1.31 B	23
		--laminated sand and silt seams; wet--	50		16	5 6 10	1.25 P	23		705.6 705.4 Gray SILTY LOAM, trace gravel; saturated Medium dense, gray SANDY GRAVEL; saturated --RDR 2--	70		20	7 8 9	1.23 B	21	
	722.6	Medium dense, gray SILTY LOAM; saturated --RDR 2-- --laminated silty clay and sand seams--	55		17	6 9 13	0.75 P	19			--heaving sand in augers; flushed augers with water--	75		21	11 11 10	NP	5
	717.6	Stiff, gray SILTY CLAY; moist to wet --RDR 2-- --laminated silty loam seams; saturated--	60		18	6 7 9	1.23 B	22		699.4	Boring terminated at 75.00 ft	80					

GENERAL NOTES

WATER LEVEL DATA

Begin Drilling **09-02-2020** Complete Drilling **09-02-2020**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&J** Logger **E. Yim** Checked by **C. Marin**
 Drilling Method **2.25" IDA HSA, autohammer, backfilled upon completion**

While Drilling ∇ **26.00 ft**
 At Completion of Drilling ∇ **7.00 ft**
 Time After Drilling **NA**
 Depth to Water ∇ **NA**

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.

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BORING LOG BWB-01

WEI Job No.: 7901-12-02

Client **TranSystems Corporation**
 Project **Pedestrian Bridge and Trail over IL 59**
 Location **Streamwood, IL**

Datum: NAVD 88
 Elevation: 774.40 ft
 North: 1950788.53 ft
 East: 1019305.04 ft
 Station: 100+33.67
 Offset: 38.78 RT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	773.8	7-inch thick, black SILTY CLAY LOAM --TOPSOIL-- Stiff, brown PEAT; moist --RDR 2--			1	0 0 0	1.00 P	235						9	5 6 10	2.30 B	21
	771.4	Medium dense, gray, SANDY LOAM, little gravel; moist --RDR 2--	5		2	4 6 7	NP	16		751.4	Medium dense, gray SILT; wet --RDR 2-- --L _L (%)=18, P _L (%)=15-- --%Gravel=0.0-- --%Sand=1.2--25 --%Silt=80.3-- --%Clay=18.5-- --A-4 (0)--			10	5 4 6	0.33 B	20
					3	6 5 9	NP	15		748.9	Stiff to very stiff, gray SILTY CLAY, trace gravel; moist --RDR 2--			11	4 4 6	2.05 B	24
	766.4	Very stiff, gray SILTY CLAY, trace gravel; moist --RDR 2--	10		4	5 6 8	2.00 P	17						12	5 6 7	1.80 B	21
	763.9	Loose to medium dense, gray SANDY LOAM; saturated --RDR 2--			5	3 4 4	NP	20		744.4	Boring terminated at 30.00 ft						
					6	5 6 13	NP	21									
	758.9	Medium stiff to very stiff, gray SILTY CLAY, trace gravel; moist to wet --RDR 2--	15		7	3 5 6	0.66 B	22									
					8	4 6 7	1.50 P	21									

GENERAL NOTES

WATER LEVEL DATA

Begin Drilling **05-07-2021** Complete Drilling **05-07-2021**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&A** Logger **M. Rojo** Checked by **J. Bensen**
 Drilling Method **2.25" ID HSA; boring completed upon completion**

While Drilling ∇ **6.00 ft**
 At Completion of Drilling ∇ **18.00 ft**
 Time After Drilling **NA**
 Depth to Water ∇ **NA**

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BORING LOG BWB-02

WEI Job No.: 7901-12-02

Client **TranSystems Corporation**
 Project **Pedestrian Bridge and Trail over IL 59**
 Location **Streamwood, IL**

Datum: NAVD 88
 Elevation: 774.00 ft
 North: 1950884.44 ft
 East: 1019167.42 ft
 Station: 101+29.58
 Offset: 98.84 LT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	773.5	6-inch thick, black CLAY LOAM --TOPSOIL-- Very soft, brown PEAT; saturated --RDR 1--			1	0 0 0	< 0.08 B	159		753.5	Medium stiff to stiff, gray SILTY CLAY, trace gravel; damp to moist --RDR 2--			9	3 6 10	1.56 B	22
			5		2	0 0 0	< 0.08 B	497				25		10	3 3 4	0.82 B	23
					3	0 0 0	< 0.08 B	486						11	3 5 7	1.23 B	22
			10		4	0 0 0	< 0.08 B	214						12	2 6 6	1.80 B	22
	763.5	Medium dense, brown GRAVEL; wet --RDR 2--			5	9 9 9	NP			744.0	Boring terminated at 30.00 ft						
	761.0	Medium dense, gray SANDY LOAM to coarse SAND, trace gravel; saturated --RDR 2--			6	8 10 7	NP	13									
					7	20 12 9	NP	19									
	756.0	Dense, gray SILTY LOAM; moist --RDR 2--			8	41 35 13	0.50 P	16									

GENERAL NOTES

Begin Drilling **05-05-2021** Complete Drilling **05-05-2021**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&A** Logger **M. Rojo** Checked by **J. Bensen**
 Drilling Method **2.25" ID HSA; boring completed upon completion**

WATER LEVEL DATA

While Drilling ∇ **2.00 ft**
 At Completion of Drilling ∇ **2.00 ft**
 Time After Drilling **NA**
 Depth to Water ∇ **NA**

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BORING LOG BWB-03

WEI Job No.: 7901-12-02

Client **TranSystems Corporation**
 Project **Pedestrian Bridge and Trail over IL 59**
 Location **Streamwood, IL**

Datum: NAVD 88
 Elevation: 774.10 ft
 North: 1950983.51 ft
 East: 1019179.23 ft
 Station: 102+74.63
 Offset: 37.93 LT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	773.5	7-inch thick, black SILTY CLAY LOAM --TOPSOIL-- Very soft, black, brown to gray PEAT; wet to saturated --RDR 2-1--			1	0 0 0	< 0.08 B	158		753.6	Medium dense to very dense, gray GRAVELLY SAND to SANDY GRAVEL; saturated --RDR 2--			9	7 7 5	NP	13
			5		2	0 0 0	< 0.08 B	514				25		10	32 31 23	NP	20
					3	0 0 0	< 0.08 B	652						11	38 15 10	NP	9
			10		4	0 0 0	< 0.08 B	456		746.1	Medium dense, gray, fine SAND; saturated --RDR 2--			12	6 5 5	NP	23
	763.6	Very soft gray, organic SILTY CLAY; wet --RDR 1--			5	0 0 0	< 0.08 B	88		744.1	Boring terminated at 30.00 ft						
			15		6	0 0 0	< 0.08 B	96									
	758.6	Medium dense, gray SANDY GRAVEL; saturated --RDR 2--			7	7 7 6	NP	13									
	756.1	Dense, gray, medium to coarse SAND; saturated --RDR 2--			8	7 19 25	NP	22									
			20									40					

GENERAL NOTES

WATER LEVEL DATA

Begin Drilling **05-10-2021** Complete Drilling **05-10-2021**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&A** Logger **M. Rojo** Checked by **J. Bensen**
 Drilling Method **2.25" ID HSA; boring completed upon completion**

While Drilling ∇ **1.50 ft**
 At Completion of Drilling ∇ **2.00 ft**
 Time After Drilling **NA**
 Depth to Water ∇ **NA**

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BORING LOG BWB-03-B

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WEI Job No.: 7901-12-02

Client **TranSystems Corporation**
 Project **Pedestrian Bridge and Trail over IL 59**
 Location **Streamwood, IL**

Datum: NAVD 88
 Elevation: 773.90 ft
 North: 1950984.20 ft
 East: 1019224.66 ft
 Station: 102+44.59
 Offset: 6.39 LT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	
	772.9	12-inch thick, black LOAM; saturated --TOPSOIL-- Very soft, black, brown to olive gray PEAT; saturated --RDR 1--			1	0 0 0	< 0.25 P	256						9	0 0 0	< 0.08 B	48	
			5		2	0 0 0	< 0.25 P	338		750.9	Soft, gray SILTY CLAY LOAM; moist --RDR 2--	25		10	3 3 3	0.41 B	24	
					3	0 0 0	< 0.25 P	686		748.4	Medium dense, gray, coarse SAND; saturated --RDR 2--			11	9 12 9	NP	20	
			10		4	0 0 0	< 0.25 P	596		745.9	Medium dense, gray SANDY GRAVEL; saturated --RDR 2--	30		12	12 14 11	NP	11	
					5	0 0 0	< 0.25 P	369		742.2	Medium dense to dense, gray, coarse SAND, little to some gravel; saturated --RDR 2--			13	28 12 7	NP	11	
	760.9	Very soft, gray to black, organic SILTY CLAY to SILTY CLAY LOAM; wet to saturated --RDR 1--	15		6	0 1 1	NA	86				35		14	9 12 13	NP	6	
					7	0 0 0	< 0.08 B	87										
			20		8	0 0 0	< 0.25 P	53				40						

GENERAL NOTES

WATER LEVEL DATA

Begin Drilling **05-18-2021** Complete Drilling **05-18-2021**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&R** Logger **I. Nenn** Checked by **J. Bensen**
 Drilling Method **2.25" ID HSA; boring completed upon completion**

While Drilling ∇ **0.00 ft**
 At Completion of Drilling ∇ **0.00 ft**
 Time After Drilling **NA**
 Depth to Water ∇ **NA**

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BORING LOG BWB-03-B

WEI Job No.: 7901-12-02

Client **TranSystems Corporation**
 Project **Pedestrian Bridge and Trail over IL 59**
 Location **Streamwood, IL**

Datum: NAVD 88
 Elevation: 773.90 ft
 North: 1950984.20 ft
 East: 1019224.66 ft
 Station: 102+44.59
 Offset: 6.39 LT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	727.2		45		15	12 16 21	NP	6									
	723.9	Very stiff, gray SILTY CLAY, trace gravel; damp --RDR 2--	50		16	15 4 7	2.05 B	20									
		Boring terminated at 50.00 ft															

GENERAL NOTES

WATER LEVEL DATA

Begin Drilling **05-18-2021** Complete Drilling **05-18-2021**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&R** Logger **I. Nenn** Checked by **J. Bensen**
 Drilling Method **2.25" ID HSA; boring completed upon completion**

While Drilling ∇ **0.00 ft**
 At Completion of Drilling ∇ **0.00 ft**
 Time After Drilling **NA**
 Depth to Water ∇ **NA**

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BORING LOG BWB-04

WEI Job No.: 7901-12-02

Client **TranSystems Corporation**
 Project **Pedestrian Bridge and Trail over IL 59**
 Location **Streamwood, IL**

Datum: NAVD 88
 Elevation: 774.10 ft
 North: 1951083.72 ft
 East: 1019182.30 ft
 Station: 103+56.66
 Offset: 10.77 RT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	773.5	7-inch thick, black SILTY CLAY --TOPSOIL-- Very soft, black to brown PEAT; wet to saturated --RDR 1--			1	0 0 0	< 0.08 B	211			--%Silt=68.8-- --%Clay=30.4-- --A-7-6 (53)--			9	0 0 0	< 0.08 B	72
			5		2	0 0 0	< 0.08 B	247				25		10	0 0 0	< 0.08 B	68
					3	0 0 0	< 0.08 B	640						11	0 0 0	< 0.08 B	38
			10		4	0 0 0	< 0.08 B	536		746.1	Medium dense to dense, gray, coarse SAND to SANDY LOAM; saturated --RDR 2--	30		12	7 8 10	NP	14
	761.1	Loose, gray, fine SAND; moist --RDR 2--			6	0 4 5	NP	22						13	13 13 13	NP	22
	758.6	Very soft, gray, organic SILTY CLAY to SILTY CLAY LOAM; moist to wet --RDR 2--			7	0 0 0	< 0.08 B	105									
		--L _L (%)=73, P _L (%)=28-- --%Gravel=0.0-- --%Sand=0.8--			8	0 0 0	< 0.08 B	106			--trace gravel--	40		14	25 16 16	NP	16

GENERAL NOTES

WATER LEVEL DATA

Begin Drilling **05-10-2021** Complete Drilling **05-11-2021**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&A** Logger **M. Rojo** Checked by **J. Bensen**
 Drilling Method **2.25" ID HSA; boring completed upon completion**

While Drilling ∇ **2.00 ft**
 At Completion of Drilling ∇ **1.50 ft**
 Time After Drilling **NA**
 Depth to Water ∇ **NA**

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BORING LOG BWB-04

WEI Job No.: 7901-12-02

Client **TranSystems Corporation**
 Project **Pedestrian Bridge and Trail over IL 59**
 Location **Streamwood, IL**

Datum: NAVD 88
 Elevation: 774.10 ft
 North: 1951083.72 ft
 East: 1019182.30 ft
 Station: 103+56.66
 Offset: 10.77 RT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	727.4	--trace gravel--	45		15	21 16 21	NP	18									
	724.1	Very stiff, gray SILTY CLAY, trace gravel; damp --RDR 2--	50		16	8 8 9	2.21 B	20									
		Boring terminated at 50.00 ft	55														
			60														

GENERAL NOTES

WATER LEVEL DATA

Begin Drilling **05-10-2021** Complete Drilling **05-11-2021**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&A** Logger **M. Rojo** Checked by **J. Bensen**
 Drilling Method **2.25" ID HSA; boring completed upon completion**

While Drilling ∇ **2.00 ft**
 At Completion of Drilling ∇ **1.50 ft**
 Time After Drilling **NA**
 Depth to Water ∇ **NA**

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BORING LOG BWB-05

WEI Job No.: 7901-12-02

Client **TranSystems Corporation**
 Project **Pedestrian Bridge and Trail over IL 59**
 Location **Streamwood, IL**

Datum: NAVD 88
 Elevation: 773.80 ft
 North: 1951174.80 ft
 East: 1019211.97 ft
 Station: 104+66.32
 Offset: 18.35 RT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	773.2	7-inch thick, brown CLAY LOAM --TOPSOIL-- Very soft, gray, organic SILTY CLAY; wet --RDR 1--			1	0 0 0	< 0.08 B	139			--%Silt=74.8-- --%Clay=21.5-- --A-7-6 (24)--			9	0 0 1	0.50 P	31
	770.8	Very soft, brown PEAT; saturated --RDR 1--			2	0 0 0	< 0.08 B	511		750.8	Stiff to hard, gray SILTY CLAY, trace gravel; damp to moist --RDR 2--			10	1 3 8	1.31 B	19
					3	0 0 0	< 0.08 B	519						11	3 7 8	1.89 B	19
					4	0 0 0	< 0.08 B	413						12	6 9 12	5.17 B	18
					5	0 0 0	< 0.08 B	310		743.8	Boring terminated at 30.00 ft						
	760.8	Very soft to medium stiff, gray, organic SILTY CLAY to SILTY CLAY LOAM; moist to wet --RDR 1--			6	0 0 0	< 0.08 B	92									
					7	0 0 0	< 0.08 B	62									
		--L _L (%)=48, P _L (%)=26-- --%Gravel=0.1-- --%Sand=3.6--			8	0 0 0	< 0.08 B	59									

GENERAL NOTES

WATER LEVEL DATA

Begin Drilling **05-05-2021** Complete Drilling **05-05-2021**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&A** Logger **M. Rojo** Checked by **J. Bensen**
 Drilling Method **2.25" ID HSA; boring completed upon completion**

While Drilling ∇ **2.00 ft**
 At Completion of Drilling ∇ **DRY**
 Time After Drilling **NA**
 Depth to Water ∇ **NA**

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.

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BORING LOG BWB-06

WEI Job No.: 7901-12-02

Client **TranSystems Corporation**
 Project **Pedestrian Bridge and Trail over IL 59**
 Location **Streamwood, IL**

Datum: NAVD 88
 Elevation: 773.70 ft
 North: 1951255.88 ft
 East: 1019273.96 ft
 Station: 105+70.70
 Offset: 1.33 LT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	
	773.0	9-inch thick, brown CLAY LOAM --TOPSOIL--																
		Very soft to medium stiff, brown PEAT; moist to wet --RDR 1--			1	0 0 0	0.50 P	88						9	2 1 2		NP	19
			5		2	0 0 0	< 0.08 B	480				25		10	1 0 1		NP	18
					3	0 0 0	< 0.08 B	551						11	0 0 0		NP	21
			10		4	0 0 0	< 0.08 B	469		745.7	Very soft, gray, organic SILTY CLAY to SILTY CLAY LOAM; wet --RDR 1--	30		12	0 0 0	< 0.08 B		68
					5	0 0 0	< 0.08 B	340						13	0 0 0	< 0.08 B		59
			15		6	0 0 0	< 0.08 B	270				35		14	0 0 0	< 0.08 B		64
	758.2	Very soft, gray, organic SILTY CLAY; wet --RDR 1--			7	3 1 2	< 0.08 B	98		738.2	Stiff, gray SILTY CLAY, trace gravel; damp to moist --RDR 2--			15	2 2 5	1.25 P		20
	755.7	Very loose, gray SANDY LOAM to fine SAND; saturated --RDR 2--			8	0 0 0	NP	23						16	4 6 9	1.56 B		19
			20							733.7		40						

GENERAL NOTES

Begin Drilling **05-05-2021** Complete Drilling **05-05-2021**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&A** Logger **M. Rojo** Checked by **J. Bensen**
 Drilling Method **2.25" ID HSA; boring completed upon completion**

WATER LEVEL DATA

While Drilling **18.00 ft**
 At Completion of Drilling **20.00 ft**
 Time After Drilling **NA**
 Depth to Water **NA**

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BORING LOG BWB-06

WEI Job No.: 7901-12-02

Client **TranSystems Corporation**
 Project **Pedestrian Bridge and Trail over IL 59**
 Location **Streamwood, IL**

Datum: NAVD 88
 Elevation: 773.70 ft
 North: 1951255.88 ft
 East: 1019273.96 ft
 Station: 105+70.70
 Offset: 1.33 LT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
		--%Silt=49.4-- --%Clay=28.3-- --A-6 (9)-- Boring terminated at 40.00 ft	45														
			50														
			55														
			60														

GENERAL NOTES

WATER LEVEL DATA

Begin Drilling **05-05-2021** Complete Drilling **05-05-2021**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&A** Logger **M. Rojo** Checked by **J. Bensen**
 Drilling Method **2.25" ID HSA; boring completed upon completion**

While Drilling ▽ **18.00 ft**
 At Completion of Drilling ▼ **20.00 ft**
 Time After Drilling **NA**
 Depth to Water ▼ **NA**

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BORING LOG BWB-06-SH

WEI Job No.: 7901-12-02

Client **TranSystems Corporation**
 Project **Pedestrian Bridge and Trail over IL 59**
 Location **Streamwood, IL**

Datum: NAVD 88
 Elevation: 773.60 ft
 North: 1951251.94 ft
 East: 1019275.80 ft
 Station: 105+69.68
 Offset: 2.97 RT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
		Blind drill to 8.0 feet	5									25					
	765.6	Very soft, brown PEAT; moist	10	Vertical Lines	1	PUSH	< 0.08	485				30					
	761.6			Vertical Lines	2	PUSH	0.25	364									
		Blind drill from 12.0 to 40.0 feet	15									35					
			20							733.6		40					

GENERAL NOTES

Begin Drilling **05-10-2021** Complete Drilling **05-10-2021**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&A** Logger **M. Rojo** Checked by **J. Bensen**
 Drilling Method **2.25" ID HSA; boring completed upon completion**

WATER LEVEL DATA

While Drilling ∇ **DRY**
 At Completion of Drilling \blacktriangledown **MUD**
 Time After Drilling **NA**
 Depth to Water ∇ **NA**

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



BORING LOG BWB-06-SH

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WEI Job No.: 7901-12-02

Client **TranSystems Corporation**
 Project **Pedestrian Bridge and Trail over IL 59**
 Location **Streamwood, IL**

Datum: NAVD 88
 Elevation: 773.60 ft
 North: 1951251.94 ft
 East: 1019275.80 ft
 Station: 105+69.68
 Offset: 2.97 RT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
		Stiff to very stiff, gray SILTY CLAY, trace gravel; damp to moist															
		--RDR 2--															
			45		17	7 10 14	1.89 B										
			50		18	8 11 15	2.05 B										
			55		19	5 10 13	2.71 B										
			60		20	6 9 9	1.80 B										
	713.6																

Boring terminated at 60.00 ft

GENERAL NOTES

WATER LEVEL DATA

Begin Drilling **05-10-2021** Complete Drilling **05-10-2021**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&A** Logger **M. Rojo** Checked by **J. Bensen**
 Drilling Method **2.25" ID HSA; boring completed upon completion**

While Drilling **DRY**
 At Completion of Drilling **MUD**
 Time After Drilling **NA**
 Depth to Water **NA**

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BORING LOG BWB-07

WEI Job No.: 7901-12-02

Client **TranSystems Corporation**
 Project **Pedestrian Bridge and Trail over IL 59**
 Location **Streamwood, IL**

Datum: NAVD 88
 Elevation: 774.50 ft
 North: 1951438.44 ft
 East: 1020161.06 ft
 Station: 114+78.03
 Offset: 3.09 LT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	773.8	8-inch thick, black CLAY LOAM --TOPSOIL-- Very soft to soft, black, brown to gray organic SILTY CLAY LOAM to SILTY CLAY; moist to wet --RDR 2--			1	1 1 2	0.25 P	51						9	3 4 7	1.89 B	21
			5		2	0 0 1	< 0.08 B	145		751.5	Very dense, gray SANDY GRAVEL, some cobble fragments; saturated --RDR 3-- --rig chatter; possible cobbles--25			10	50/2"	NP	19
		--L _L (%)=82, P _L (%)=36-- --%Gravel=0.0-- --%Sand=6.4-- --%Silt=71.2-- --%Clay=22.4-- --A-7-5 (53)--	10		3	0 0 0	< 0.08 B	110		749.0	Stiff to very stiff, gray SILTY CLAY, trace gravel; moist --RDR 2--			11	7 9 13	2.00 P	21
			10		4	0 0 0	< 0.08 B	47						12	4 6 8	1.39 B	19
	764.0	Medium stiff to very stiff, gray SILTY CLAY LOAM to SILTY CLAY, trace gravel; moist to wet --RDR 2--			5	1 2 3	1.00 P	16		744.5	Boring terminated at 30.00 ft						
			15		6	2 4 5	0.82 B	18									
					7	9 8 8	NR										
			20		8	6 8 10	3.94 B	16									

GENERAL NOTES

Begin Drilling **05-03-2021** Complete Drilling **05-03-2021**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&A** Logger **M. Rojo** Checked by **J. Bensen**
 Drilling Method **2.25" ID HSA; boring completed upon completion**

WATER LEVEL DATA

While Drilling ∇ **13.00 ft**
 At Completion of Drilling ∇ **3.00 ft**
 Time After Drilling **NA**
 Depth to Water ∇ **NA**

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BORING LOG BWB-08

WEI Job No.: 7901-12-02

Client **TranSystems Corporation**
 Project **Pedestrian Bridge and Trail over IL 59**
 Location **Streamwood, IL**

Datum: NAVD 88
 Elevation: 774.60 ft
 North: 1951456.35 ft
 East: 1020255.11 ft
 Station: 115+73.40
 Offset: 4.83 LT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	774.34	1-inch thick, black CLAY LOAM --TOPSOIL-- Soft to medium stiff, black to gray, SILTY CLAY LOAM, organic matter; moist --RDR 2--			1	1 1 3	0.41 B	48						9	5 10 12	5.33 B	18
			5		2	0 1 1	0.50 P	59				25		10 8 12	1.07 B	17	
	769.1	Very soft to stiff, brown to gray PEAT; wet --RDR 2--			3	0 0 0	< 0.08 B	199						11	8 7 12	3.03 B	14
			10		4	0 0 0	< 0.08 B	61				30		12	8 11 10	5.33 B	18
					5	0 0 1	1.00 P	69		744.6	Boring terminated at 30.00 ft						
	761.6	Medium stiff to hard, gray SILTY CLAY to SILTY CLAY LOAM, trace to little gravel; damp to wet --wood fragments-- --RDR 2--			6	1 3 4	0.98 B	17									
		--L _L (%)=28, P _L (%)=14-- --%Gravel=5.1-- --%Sand=15.5-- --%Silt=50.5-- --%Clay=29.0-- --A-6 (9)--			7	3 4 5	0.57 B	19									
			20		8	6 6 9	1.25 P	18				40					

GENERAL NOTES

Begin Drilling **05-04-2021** Complete Drilling **05-04-2021**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&A** Logger **M. Rojo** Checked by **J. Bensen**
 Drilling Method **2.25" ID HSA; boring completed upon completion**

WATER LEVEL DATA

While Drilling **DRY**
 At Completion of Drilling **DRY**
 Time After Drilling **NA**
 Depth to Water **NA**

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BORING LOG BWB-09

WEI Job No.: 7901-12-02

Client **TranSystems Corporation**
 Project **Pedestrian Bridge and Trail over IL 59**
 Location **Streamwood, IL**

Datum: NAVD 88
 Elevation: 775.60 ft
 North: 1951444.59 ft
 East: 1020360.93 ft
 Station: 116+78.97
 Offset: 3.22 LT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	775.0	7-inch thick, black CLAY LOAM --TOPSOIL-- Medium stiff to hard, brown to gray SILTY CLAY to SILTY CLAY LOAM, trace gravel; moist --RDR 2--			1	2 2 6	0.74 S	17						9	4 5 11	1.72 B	19
			5		2	3 5 9	3.03 B	18				25		10	10 12 14		NR
					3	4 6 7	3.69 B	15						11	5 8 26	5.08 B	16
			10		4	3 6 8	1.56 B	15		745.6		30		12	3 10 8	2.62 B	16
					5	6 7 10	2.79 B	16			Boring terminated at 30.00 ft						
		--cobble fragments--	15		6	4 7 7	3.12 B	17				35					
		--cobble fragments--			7	4 7 8	2.21 B	18									
			20		8	4 6 7	2.05 B	20				40					

GENERAL NOTES

Begin Drilling **04-29-2021** Complete Drilling **04-29-2021**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&A** Logger **M. Rojo** Checked by **J. Bensen**
 Drilling Method **2.25" ID HSA; boring completed upon completion**

WATER LEVEL DATA

While Drilling ∇ **30.00 ft**
 At Completion of Drilling ∇ **30.00 ft**
 Time After Drilling **NA**
 Depth to Water ∇ **NA**

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BORING LOG BWB-10

WEI Job No.: 7901-12-02

Client **TranSystems Corporation**
 Project **Pedestrian Bridge and Trail over IL 59**
 Location **Streamwood, IL**

Datum: NAVD 88
 Elevation: 776.10 ft
 North: 1951430.03 ft
 East: 1020442.25 ft
 Station: 117+55.28
 Offset: 24.30 LT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	775.5	7-inch thick, black SILTY CLAY LOAM --TOPSOIL-- Stiff to very stiff, brown to gray SILTY CLAY, trace gravel; damp to moist --RDR 2--			1	2 5 10	1.64 B	17						9	5 13 10	0.41 B	17
			5		2	5 6 10	1.07 B	19				25		10	5 6 10	1.23 B	15
					3	5 9 12	3.53 B	16						11	3 6 9	1.56 B	18
			10		4	5 6 10	3.44 B	16						12	4 6 9	1.39 B	17
					5	3 7 6	2.00 P	15		746.1	Boring terminated at 30.00 ft	30					
	763.1	Medium stiff, gray, gravelly CLAY LOAM; moist to wet --RDR 2-- --L _L (%)=28, P _L (%)=13-- --%Gravel=16.0-- --%Sand=32.0-- --%Silt=31.3-- --%Clay=20.7-- --A-6 (4)--			6	2 3 5	0.82 B	15									
	760.6	Soft to stiff, gray SILTY CLAY LOAM to SILTY CLAY, trace gravel; damp to moist --RDR 2--			7	4 6 8	1.48 B	22									
			20		8	5 7 8	1.48 B	18				40					

GENERAL NOTES

Begin Drilling **04-29-2021** Complete Drilling **04-29-2021**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&A** Logger **M. Rojo** Checked by **J. Bensen**
 Drilling Method **2.25" ID HSA; boring completed upon completion**

WATER LEVEL DATA

While Drilling ∇ **13.00 ft**
 At Completion of Drilling ∇ **13.00 ft**
 Time After Drilling **NA**
 Depth to Water ∇ **NA**

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BORING LOG BWB-11

WEI Job No.: 7901-12-02

Client **TranSystems Corporation**
 Project **Pedestrian Bridge and Trail over IL 59**
 Location **Streamwood, IL**

Datum: NAVD 88
 Elevation: 775.40 ft
 North: 1951398.61 ft
 East: 1020558.16 ft
 Station: 118+94.21
 Offset: 31.17 LT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	774.7	9-inch thick, black CLAY LOAM --TOPSOIL-- Soft to stiff, black to gray SILTY CLAY, trace organic matter; moist --RDR 2--			1	1 3 3	1.80 B	31						9	3 8 10	2.46 B	18
			5		2	2 2 5	0.33 B	45				25		10	4 6 11	3.61 B	17
	769.9	Stiff to hard, brown to gray SILTY CLAY LOAM to SILTY CLAY, trace gravel; damp to moist --RDR 2--			3	1 2 5	1.00 P	17						11	6 8 10	2.30 B	19
			10		4	5 6 8	1.39 B	16				30		12	4 8 8	2.05 B	19
					5	3 5 8	3.69 B	16		745.4	Boring terminated at 30.00 ft						
			15		6	3 8 9	3.85 B	15				35					
					7	7 9 12	4.92 B	15									
			20		8	8 13 11	3.69 B	16				40					

GENERAL NOTES

WATER LEVEL DATA

Begin Drilling **04-28-2021** Complete Drilling **04-28-2021**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&A** Logger **M. Rojo** Checked by **J. Bensen**
 Drilling Method **2.25" ID HSA; boring completed upon completion**

While Drilling **DRY**
 At Completion of Drilling **DRY**
 Time After Drilling **NA**
 Depth to Water **NA**

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BORING LOG BWB-12

WEI Job No.: 7901-12-02

Client **TranSystems Corporation**
 Project **Pedestrian Bridge and Trail over IL 59**
 Location **Streamwood, IL**

Datum: NAVD 88
 Elevation: 775.90 ft
 North: 1951368.16 ft
 East: 1020655.74 ft
 Station: 119+93.97
 Offset: 4.05 RT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	775.4	6-inch thick, black CLAY LOAM --TOPSOIL-- Very soft to stiff, black to gray SILTY CLAY, trace organic matter; moist --RDR 2--			1	0 0 2	1.25 P	40						9	5 7 9	2.79 B	19
			5		2	1 2 3	0.16 B	39				25		10	5 6 9	2.71 B	19
	770.4	Loose, gray SILTY LOAM; wet --RDR 2-- --L _L (%)=31, P _L (%)=17-- --%Gravel=4.6-- --%Sand=10.8-- --%Silt=67.3-- --%Clay=17.3-- --A-6 (11)--			3	1 2 3	0.08 B	22						11	4 4 8	1.97 B	21
	767.9	Soft to very stiff, brown to gray SILTY CLAY, trace gravel; damp to moist --RDR 2--			4	1 2 6	0.49 B	20						12	4 6 7	2.05 B	21
					5	5 3 5	1.48 B	18									
			15		6	3 5 7	2.46 B	17				35					
					7	4 4 7	2.05 B	17									
					8	5 6 9	2.71 B	18				40					
										745.9	Boring terminated at 30.00 ft						

GENERAL NOTES

WATER LEVEL DATA

Begin Drilling **04-28-2021** Complete Drilling **04-28-2021**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&A** Logger **M. Rojo** Checked by **J. Bensen**
 Drilling Method **2.25" ID HSA; boring completed upon completion**

While Drilling **DRY**
 At Completion of Drilling **DRY**
 Time After Drilling **NA**
 Depth to Water **NA**

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BORING LOG BWB-13

WEI Job No.: 7901-12-02

Client **TranSystems Corporation**
 Project **Pedestrian Bridge and Trail over IL 59**
 Location **Streamwood, IL**

Datum: NAVD 88
 Elevation: 776.10 ft
 North: 1951362.07 ft
 East: 1020747.35 ft
 Station: 120+88.03
 Offset: 13.68 RT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	775.5	7-inch thick, black CLAY LOAM --TOPSOIL-- Soft to medium stiff, gray SILTY CLAY, trace organic matter; damp to moist --RDR 2--			1	3 2 4	0.74 B	34						9	5 6 7	1.64 B	21
			5		2	0 1 4	0.41 B	56				25		10	3 4 6	1.89 B	21
	770.6	Soft to very stiff, brown to gray SILTY CLAY to SILTY CLAY LOAM, trace gravel; damp to moist --RDR 2--			3	0 2 3	0.41 B	20						11	5 6 7	1.56 B	20
			10		4	1 4 5	0.25 B	16			--moist to wet--	30		12	1 5 7	1.97 B	14
					5	5 4 6	0.82 B	16		746.1	Boring terminated at 30.00 ft						
			15		6	4 5 10	3.85 B	15				35					
					7	4 8 10	3.53 B	16									
			20		8	5 8 9	2.05 B	18				40					

GENERAL NOTES

Begin Drilling **04-28-2021** Complete Drilling **04-28-2021**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&A** Logger **M. Rojo** Checked by **J. Bensen**
 Drilling Method **2.25" ID HSA; boring completed upon completion**

WATER LEVEL DATA

While Drilling ∇ **30.00 ft**
 At Completion of Drilling ∇ **30.00 ft**
 Time After Drilling **NA**
 Depth to Water ∇ **NA**

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.

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BORING LOG BWB-14

WEI Job No.: 7901-12-02

Client **TranSystems Corporation**
 Project **Pedestrian Bridge and Trail over IL 59**
 Location **Streamwood, IL**

Datum: NAVD 88
 Elevation: 776.20 ft
 North: 1951366.61 ft
 East: 1020847.58 ft
 Station: 121+86.26
 Offset: 11.82 LT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	
	775.6	7-inch thick, black CLAY LOAM --TOPSOIL-- Stiff, black SILTY CLAY, trace organic matter; moist --RDR 2--			1	4 4 6	1.00 P	30						9	4 5 11	4.76 B	16	
	773.2	Very soft to soft, gray, SILTY CLAY to SILTY CLAY LOAM, organic matter; moist to wet --RDR 2--	5		2	1 2 2	0.08 B	61				25		10	6 11 8	2.62 B	19	
					3	0 1 1	0.33 B	57						11	5 5 8	2.05 B	21	
		--L _L (%)=63, P _L (%)=24-- --%Gravel=0.0-- --%Sand=2.3-- --%Silt=70.7-- --%Clay=26.9-- --A-7-6 (44)--	10		4	0 1 1	< 0.08 B	65		746.2		30		12	4 8 9	2.87 B	20	
					5	1 2 4	0.08 B	33			Boring terminated at 30.00 ft							
	763.2	Very stiff to hard, brown to gray SILTY CLAY to SILTY CLAY LOAM, trace gravel; damp to moist --RDR 2--15			6	2 3 6	2.05 B	20										
					7	5 8 9	4.43 B	17										
					8	5 9 12	3.85 B	15										
			20									40						

GENERAL NOTES

WATER LEVEL DATA

Begin Drilling **04-28-2021** Complete Drilling **04-28-2021**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&A** Logger **M. Rojo** Checked by **J. Bensen**
 Drilling Method **2.25" ID HSA; boring completed upon completion**

While Drilling **DRY**
 At Completion of Drilling **DRY**
 Time After Drilling **NA**
 Depth to Water **NA**

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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BORING LOG BWB-15

WEI Job No.: 7901-12-02

Client **TranSystems Corporation**
 Project **Pedestrian Bridge and Trail over IL 59**
 Location **Streamwood, IL**

Datum: NAVD 88
 Elevation: 780.80 ft
 North: 1951332.60 ft
 East: 1020929.18 ft
 Station: 122+63.11
 Offset: 22.36 LT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	780.2	7-inch thick, brown SILTY CLAY LOAM --TOPSOIL-- Medium stiff to hard, gray to brown SILTY CLAY; damp to moist --FILL-- --RDR 2--			1	5 6 5	0.50 P	20									
			5		2	5 5 7	5.08 B	16				25		10	5 7 14	3.69 B	15
	775.3	Very stiff, brown CLAY LOAM, trace gravel; damp --FILL-- --RDR 2--			3	3 7 13	2.50 P	12							7 9 11		NR
	772.8	Medium dense, brown SANDY LOAM, trace gravel; damp --FILL-- --RDR 2--			4	4 10 15	NP	8							8 8 12	2.38 B	15
	770.3	Stiff to hard, gray SILTY CLAY to SILTY CLAY LOAM, trace gravel; damp to wet --RDR 2--			5	4 8 13	6.13 B	17		750.8	Boring terminated at 30.00 ft	30					
			15		6	6 9 13	6.72 B	17				35					
					7	3 4 8	1.80 B	20									
					8	19 13 7	1.50 P	22				40					

GENERAL NOTES

Begin Drilling **04-29-2021** Complete Drilling **04-29-2021**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&A** Logger **M. Rojo** Checked by **J. Bensen**
 Drilling Method **2.25" ID HSA; boring completed upon completion**

WATER LEVEL DATA

While Drilling ∇ **18.50 ft**
 At Completion of Drilling ∇ **DRY**
 Time After Drilling **NA**
 Depth to Water ∇ **NA**

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.

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BORING LOG TB-01

WEI Job No.: 7901-12-02

Client **TranSystems Corporation**
 Project **Pedestrian Bridge and Trail over IL 59**
 Location **Streamwood, IL**

Datum: NAVD 88
 Elevation: 774.00 ft
 North: 1951279.67 ft
 East: 1019356.23 ft
 Station: 106+57.42
 Offset: 9.56 RT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	773.6	5-inch thick, black SILTY CLAY LOAM --TOPSOIL-- Medium stiff, brown, SILTY CLAY, organic matter; wet --RDR 2--			1	0 0 0	0.50 P	95		753.5	--%Silt=53.8-- --%Clay=28.7-- --A-6 (12)-- Stiff to very stiff, gray SILTY CLAY, trace gravel; damp --RDR 2--			9	2 4 5	1.39 B	20
	771.0	Very soft to medium stiff, black, brown to gray PEAT; moist --RDR 1-2--			2	0 0 0	0.50 P	113			--L _L (%)=31, P _L (%)=15-- --%Gravel=3.2-- --%Sand=15.1-- --%Silt=52.6-- --A-6 (11)--			10	3 5 6	1.64 B	20
					3	0 0 0	< 0.08 B	357						11	2 5 6	2.38 B	18
					4	0 0 0	< 0.08 B	295						12	5 8 9	2.13 B	20
					5	0 3 3	< 0.08 B	220									
	761.0	Very loose, gray SANDY LOAM; wet --RDR 2--			6	0 0 0	< 0.08 B	42						13	6 9 14	2.54 B	20
	758.5	Very soft to medium stiff, gray SILTY CLAY LOAM; damp to moist --RDR 2--			7	0 0 0	0.16 B	22									
		--L _L (%)=31, P _L (%)=14-- --%Gravel=3.8-- --%Sand=13.7--			8	3 5 5	0.50 P	24						14	5 6 8	1.48 B	19

GENERAL NOTES

WATER LEVEL DATA

Begin Drilling **05-06-2021** Complete Drilling **05-06-2021**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&A** Logger **M. Rojo** Checked by **J. Bensen**
 Drilling Method **2.25" ID HSA to 25', mud rotary thereafter; boring backfilled upon completion**

While Drilling ∇ **13.50 ft**
 At Completion of Drilling ∇ **MUD**
 Time After Drilling **NA**
 Depth to Water ∇ **NA**

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.

WANGENGINC 79011202.GPJ WANGENG.GDT 8/26/21



BORING LOG TB-02

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WEI Job No.: 7901-12-02

Client **TranSystems Corporation**
 Project **Pedestrian Bridge and Trail over IL 59**
 Location **Streamwood, IL**

Datum: NAVD 88
 Elevation: 773.60 ft
 North: 1951301.99 ft
 East: 1019454.49 ft
 Station: 107+58.45
 Offset: 5.27 RT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	772.9	8-inch thick, brown SILTY CLAY LOAM --TOPSOIL-- Stiff, brown, SILTY CLAY, organic matter; moist --RDR 2--			1	0 0 0	1.00 P	87		753.1	--%Silt=51.6-- --%Clay=26.0-- --A-6 (9)-- Stiff to hard, gray SILTY CLAY, trace gravel; damp to moist --RDR 2--			9	5 8 10	4.26 B	18
	770.6	Very soft, brown PEAT; moist --RDR 1--	5		2	0 0 0	< 0.08 B	269				25		10	4 6 10	4.51 B	17
					3	0 0 0	< 0.08 B	256						11	4 6 8	2.46 B	20
			10		4	0 0 0	< 0.08 B	295				30		12	4 6 9	1.97 B	21
					5	0 0 0	< 0.08 B	266									
			15		6	0 0 0	< 0.08 B	217			--L _L (%)=35, P _L (%)=15-- --%Gravel=0.3-- --%Sand=0.8-- --%Silt=54.7-- --%Clay=44.2-- --A-6 (20)--			13	6 9 10	2.46 B	23
	758.1	Very soft to medium stiff, brown SILTY CLAY LOAM, trace gravel; moist --RDR 1--			7	0 0 0	0.50 P	23									
		--L _L (%)=28, P _L (%)=14-- --%Gravel=5.8-- --%Sand=16.5--	20		8	0 0 0	0.08 B	21				40		14	4 4 6	2.62 B	23

GENERAL NOTES

WATER LEVEL DATA

Begin Drilling **05-07-2021** Complete Drilling **05-07-2021**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&A** Logger **M. Rojo** Checked by **J. Bensen**
 Drilling Method **2.25" ID HSA to 30', mud rotary thereafter; boring backfilled upon completion**

While Drilling ∇ **16.00 ft**
 At Completion of Drilling ∇ **MUD**
 Time After Drilling **NA**
 Depth to Water ∇ **NA**

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.

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BORING LOG TB-02

WEI Job No.: 7901-12-02

Client **TranSystems Corporation**
 Project **Pedestrian Bridge and Trail over IL 59**
 Location **Streamwood, IL**

Datum: NAVD 88
 Elevation: 773.60 ft
 North: 1951301.99 ft
 East: 1019454.49 ft
 Station: 107+58.45
 Offset: 5.27 RT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	731.9	--rig chatter; possible cobbles--								711.9	--%Silt=58.3-- --%Clay=21.8-- --A-6 (6)-- --rig chatter; possible cobbles--						
		Medium dense, gray GRAVELLY SAND; saturated --RDR 2-3--	45		15	2 5 6	NP	18			Medium dense to dense, gray SANDY GRAVEL to GRAVEL; saturated --RDR 3-4--	65		19	10 13 10	NP	13
	724.4	Soft to very stiff, gray SILTY CLAY LOAM, trace gravel; damp to moist --RDR 2--	50		16	10 13 17	NR					70		20	18 13 23	NP	9
			55		17	6 10 11	2.38 B	16				75		21	13 14 15	NP	5
			60		18	8 9 5	0.41 B	16				80					
		--L _L (%)=23, P _L (%)=12-- --%Gravel=8.0-- --%Sand=11.8--								698.6	Boring terminated at 75.00 ft						

GENERAL NOTES

Begin Drilling **05-07-2021** Complete Drilling **05-07-2021**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&A** Logger **M. Rojo** Checked by **J. Bensen**
 Drilling Method **2.25" ID HSA to 30', mud rotary thereafter; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling ∇ **16.00 ft**
 At Completion of Drilling ∇ **MUD**
 Time After Drilling **NA**
 Depth to Water ∇ **NA**

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



BORING LOG TB-03

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WEI Job No.: 7901-12-02

Client **TranSystems Corporation**
 Project **Pedestrian Bridge and Trail over IL 59**
 Location **Streamwood, IL**

Datum: NAVD 88
 Elevation: 774.90 ft
 North: 1951386.20 ft
 East: 1019891.04 ft
 Station: 112+03.02
 Offset: 0.20 RT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	774.4	6-inch thick, black CLAY LOAM --TOPSOIL--															
		Stiff, gray SILTY CLAY LOAM, little gravel; damp			1	2 4 5	1.00 P	13						9	6 9 10	2.38 B	18
	771.9	Stiff to hard, gray SILTY CLAY, trace gravel; damp to wet			2	3 5 7	3.85 B	19						10	3 6 6	1.64 B	18
		--wet spoon recovery--			3	8 9 11	5.58 B	17		749.4	Medium dense, gray SANDY GRAVEL to GRAVEL; wet			11	4 4 8	NP	9
					4	4 7 10	6.15 B	18			--rig chatter; possible cobbles--			12	9 6 9	NP	9
					5	3 6 6	2.30 B	17						13	7 8 9	NP	18
		--L _L (%)=30, P _L (%)=13-- --%Gravel=2.4-- --%Sand=4.4-- --%Silt=56.7-- --%Clay=36.5-- --A-6 (14)--			6	2 5 6	2.71 B	19						14	4 7 11	1.07 B	17
					7	2 4 6	1.80 B	21		738.2	Stiff, gray SILTY CLAY LOAM, trace gravel; damp to moist						
					8	3 5 8	1.07 B	17			--RDR 2-3--						

GENERAL NOTES

Begin Drilling **05-04-2021** Complete Drilling **05-04-2021**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&A** Logger **M. Rojo** Checked by **J. Bensen**
 Drilling Method **2.25" ID HSA to 10', mud rotary thereafter; boring**
backfilled upon completion

WATER LEVEL DATA

While Drilling ∇ **6.00 ft**
 At Completion of Drilling ∇ **MUD**
 Time After Drilling **NA**
 Depth to Water ∇ **NA**

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.

WANGENGINC 79011202.GPJ WANGENG.GDT 8/26/21



BORING LOG TB-04

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WEI Job No.: 7901-12-02

Client **TranSystems Corporation**
 Project **Pedestrian Bridge and Trail over IL 59**
 Location **Streamwood, IL**

Datum: NAVD 88
 Elevation: 774.60 ft
 North: 1951409.89 ft
 East: 1019999.14 ft
 Station: 113.13.61
 Offset: 3.86 LT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	774.0	7-inch thick, black CLAY LOAM --TOPSOIL-- Very soft to soft, black to gray, SILTY CLAY, organic matter; wet --RDR 2--			1	1 1 2	0.33 B	70			--rig chatter, 20.0 to 23.0 feet-- --possible cobbles--			9	7 6 10	2.79 B	15
		--L _L (%)=34, P _L (%)=18-- --%Gravel=3.2-- --%Sand=9.7-- --%Silt=50.6-- --%Clay=36.4-- --A-6 (13)--			2	0 1 1	< 0.08 B	36				25		10	3 8 8	1.50 P	22
	769.1	Medium dense, gray SANDY LOAM, trace gravel and cobble fragments; wet --RDR 2--			3	6 9 8	NP	33						11	4 6 10	1.50 P	24
	766.6	Stiff to hard, gray SILTY CLAY, trace to little gravel; damp to moist --RDR 2-4--			4	6 6 6	4.18 B	18				30		12	5 8 9	2.79 B	19
					5	4 7 9	3.77 B	19									
					6	4 8 7	3.28 B	19				35		13	5 8 9	2.30 B	19
					7	5 8 11	2.46 B	19			--rig chatter; possible cobbles--						
					8	4 8 10	1.89 B	19		737.9	Medium dense, gray SANDY GRAVEL; wet --RDR 3-4--			14	5 8 10	NP	12

GENERAL NOTES

Begin Drilling **05-03-2021** Complete Drilling **05-03-2021**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&A** Logger **M. Rojo** Checked by **J. Bensen**
 Drilling Method **2.25" ID HSA to 10', mud rotary thereafter; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling ∇ **1.50 ft**
 At Completion of Drilling ∇ **MUD**
 Time After Drilling **NA**
 Depth to Water ∇ **NA**

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.

WANGENGINC 79011202.GPJ WANGENG.GDT 8/26/21



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WEI Job No.: 7901-12-02

Client **TranSystems Corporation**
 Project **Pedestrian Bridge and Trail over IL 59**
 Location **Streamwood, IL**

Datum: NAVD 88
 Elevation: 774.60 ft
 North: 1951409.89 ft
 East: 1019999.14 ft
 Station: 113.13.61
 Offset: 3.86 LT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	732.9										--%Clay=8.8-- --A-4 (0)--						
		Very stiff, gray SILTY CLAY LOAM, trace gravel; damp --RDR 3--	45		15	7 11 12	2.62 B	15				65		19	10 19 8	NP	24
	727.9	Very dense, gray, fine SAND, some gravel; saturated --RDR 3--	50		16	13 26 39	NP	12			Medium stiff to stiff, gray SILTY CLAY, trace gravel; moist --RDR 2--	70		20	5 8 9	0.82 B	25
	722.9	Medium dense, gray SANDY GRAVEL; wet --RDR 2--	55		17	8 10 17	NP	7				75		21	4 10 14	1.23 B	22
	717.9	Medium dense to dense, gray SILTY LOAM, trace gravel; wet --RDR 2-- --%Gravel=3.3-- --%Sand=29.1-- --%Silt=58.8--	60		18	6 16 16	NP	28			Boring terminated at 75.00 ft	80					

GENERAL NOTES

WATER LEVEL DATA

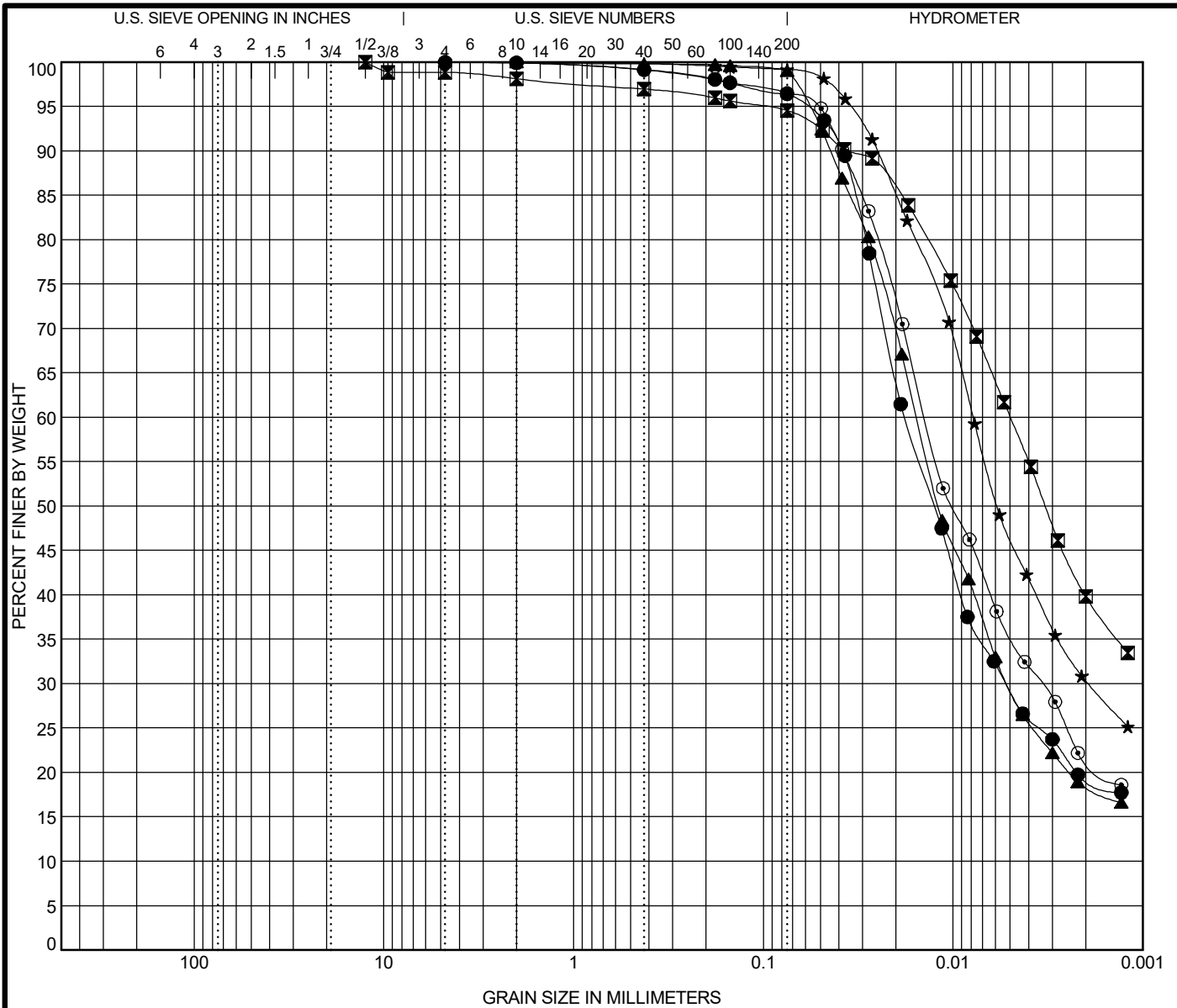
Begin Drilling **05-03-2021** Complete Drilling **05-03-2021**
 Drilling Contractor **Wang Testing Services** Drill Rig **17D50A [87%]**
 Driller **K&A** Logger **M. Rojo** Checked by **J. Bensen**
 Drilling Method **2.25" ID HSA to 10', mud rotary thereafter; boring backfilled upon completion**

While Drilling ∇ **1.50 ft**
 At Completion of Drilling ∇ **MUD**
 Time After Drilling **NA**
 Depth to Water ∇ **NA**

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.

WANGENGINC 79011202.GPJ WANGENG.GDT 8/26/21

APPENDIX B



COBBLES	GRAVEL	SAND		SILT AND CLAY
		coarse	fine	

Specimen Identification		IDH Classification				LL	PL	PI	Cc	Cu
●	B-1#8 18.5 ft	Silty Loam				40	25	15		
☒	B-2#7 16.0 ft	Silty Clay				34	17	17		
▲	BWB-01#10 23.5 ft	Silt				18	15	3		
★	BWB-04#8 18.5 ft	Silty Clay				73	28	45		
⊙	BWB-05#8 18.5 ft	Silty Clay Loam				48	26	22		
Specimen Identification		D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay	
●	B-1#8 18.5 ft	4.75	0.018	0.005		0.1	3.5	77.0	19.4	
☒	B-2#7 16.0 ft	12.5	0.005			1.9	3.6	54.7	39.8	
▲	BWB-01#10 23.5 ft	2	0.015	0.005		0.0	1.2	80.3	18.5	
★	BWB-04#8 18.5 ft	2	0.008	0.002		0.0	0.8	68.8	30.4	
⊙	BWB-05#8 18.5 ft	4.75	0.014	0.003		0.1	3.6	74.8	21.5	

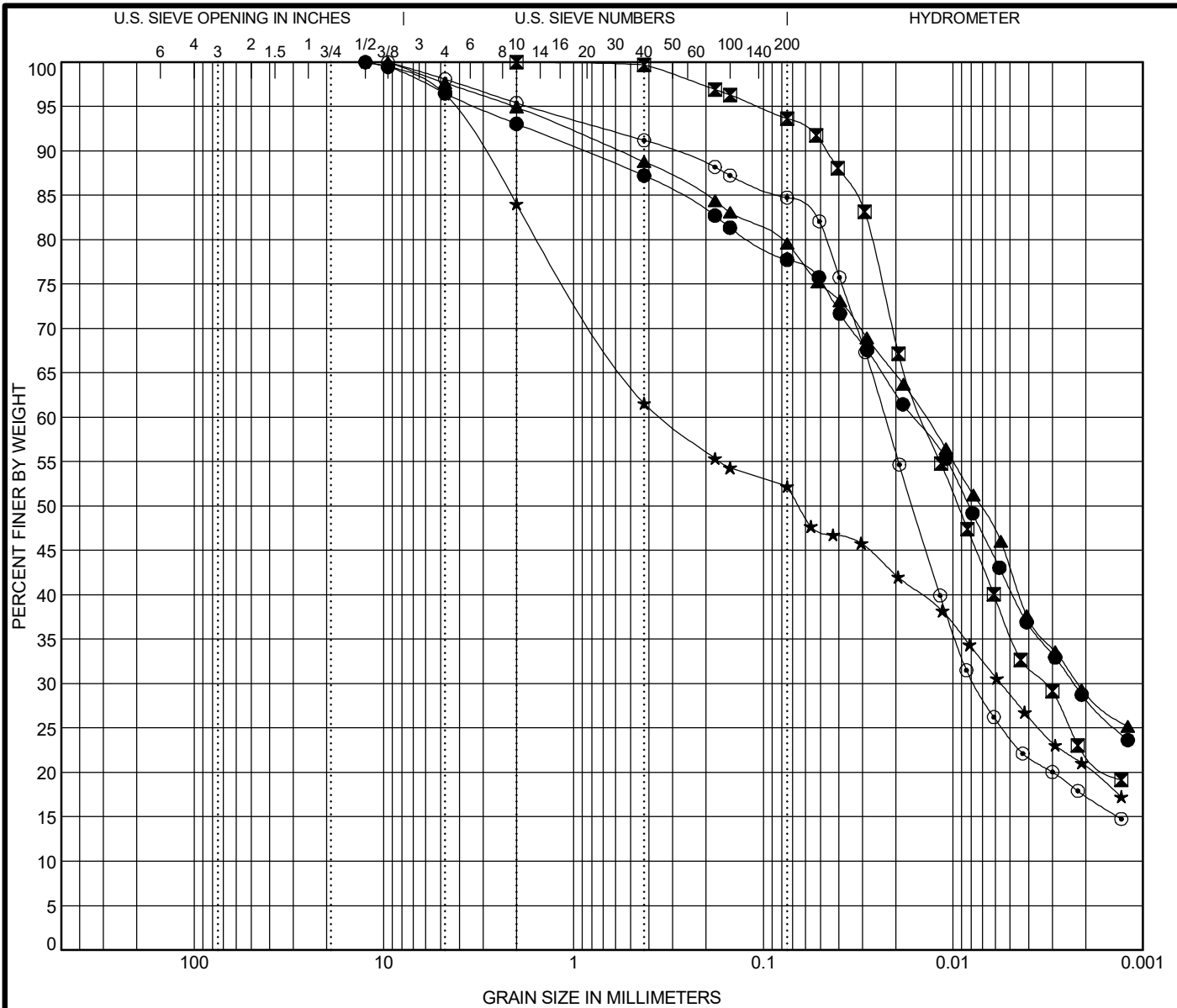
WEI GRAIN SIZE IDH 79011202.GPJ US LAB.GDT 8/26/21



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GRAIN SIZE DISTRIBUTION

Project: Pedestrian Bridge and Trail over IL 59
 Location: Streamwood, IL
 Number: 7901-12-02



COBBLES	GRAVEL	SAND		SILT AND CLAY
		coarse	fine	

Specimen Identification		IDH Classification					LL	PL	PI	Cc	Cu
●	BWB-06#16 38.5 ft	Silty Clay					28	14	14		
☒	BWB-07#3 6.0 ft	Silty Clay Loam					82	36	46		
▲	BWB-08#7 16.0 ft	Silty Clay					28	14	14		
★	BWB-10#6 13.5 ft	Gravelly Clay Loam					28	13	15		
◎	BWB-12#3 6.0 ft	Silty Loam					31	17	14		
Specimen Identification		D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay		
●	BWB-06#16 38.5 ft	12.5	0.016	0.002		7.0	15.4	49.4	28.3		
☒	BWB-07#3 6.0 ft	2	0.014	0.003		0.0	6.4	71.2	22.4		
▲	BWB-08#7 16.0 ft	9.5	0.014	0.002		5.1	15.5	50.5	29.0		
★	BWB-10#6 13.5 ft	12.5	0.342	0.006		16.0	32.0	31.3	20.7		
◎	BWB-12#3 6.0 ft	9.5	0.023	0.008		4.6	10.8	67.3	17.3		

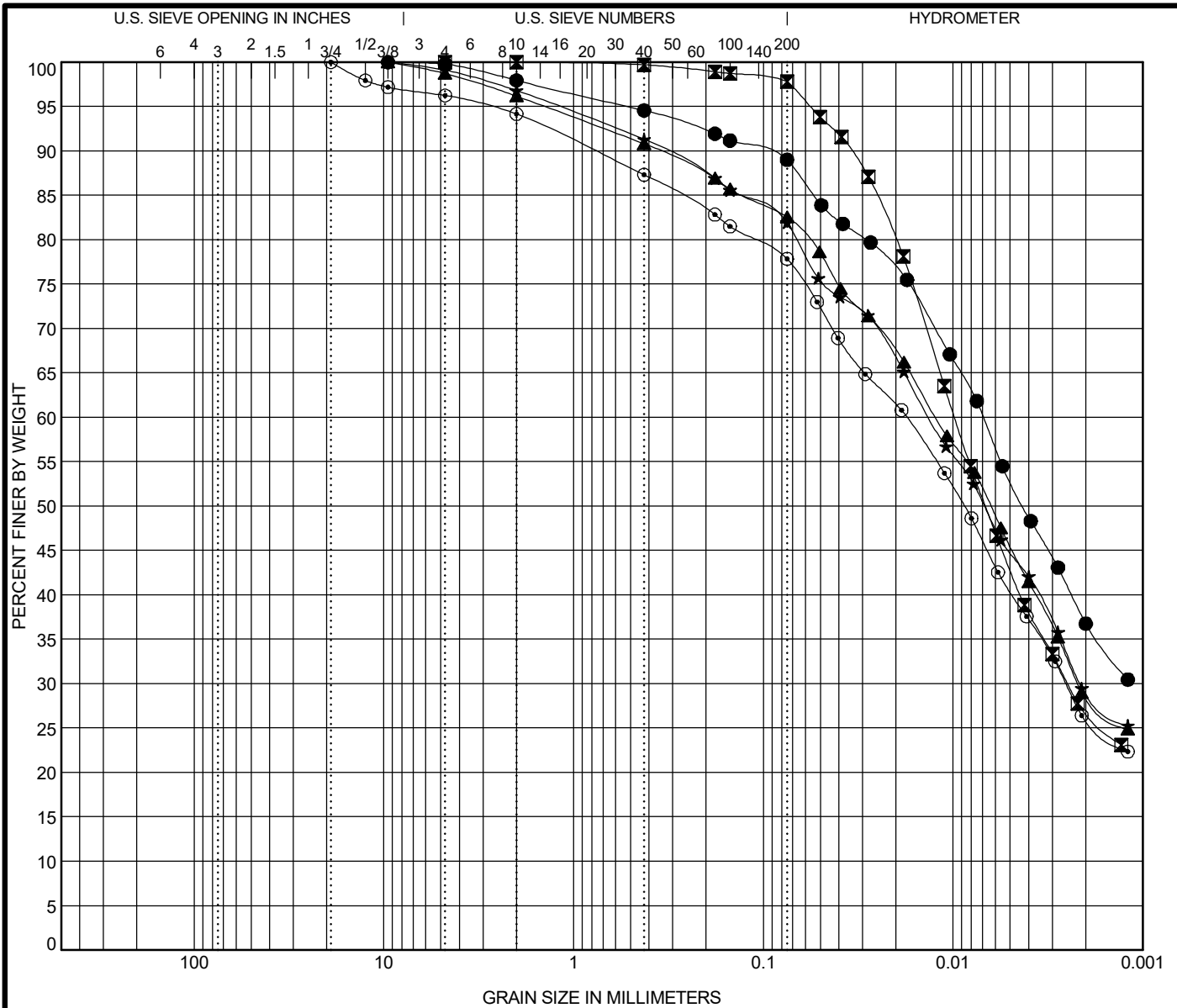
WEI GRAIN SIZE IDH 79011202.GPJ US LAB.GDT 8/26/21



Wang Engineering
 1145 N Main Street
 Lombard, IL 60148
 Telephone: 630 953-9928
 Fax: 630 953-9938

GRAIN SIZE DISTRIBUTION

Project: Pedestrian Bridge and Trail over IL 59
 Location: Streamwood, IL
 Number: 7901-12-02



COBBLES	GRAVEL	SAND		SILT AND CLAY
		coarse	fine	

Specimen Identification		IDH Classification				LL	PL	PI	Cc	Cu
●	BWB-13#9 21.0 ft	Silty Clay				34	15	19		
☒	BWB-14#4 8.5 ft	Silty Clay Loam				63	24	39		
▲	TB-01#8 18.5 ft	Silty Clay Loam				31	14	17		
★	TB-01#10 23.5 ft	Silty Clay				31	15	16		
◎	TB-02#8 18.5 ft	Silty Clay Loam				28	14	14		
Specimen Identification		D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay	
●	BWB-13#9 21.0 ft	9.5	0.007			2.1	9.1	52.1	36.7	
☒	BWB-14#4 8.5 ft	4.75	0.01	0.002		0.0	2.3	70.7	26.9	
▲	TB-01#8 18.5 ft	9.5	0.012	0.002		3.8	13.7	53.8	28.7	
★	TB-01#10 23.5 ft	9.5	0.013	0.002		3.2	15.1	52.6	29.1	
◎	TB-02#8 18.5 ft	19	0.018	0.003		5.8	16.5	51.6	26.0	

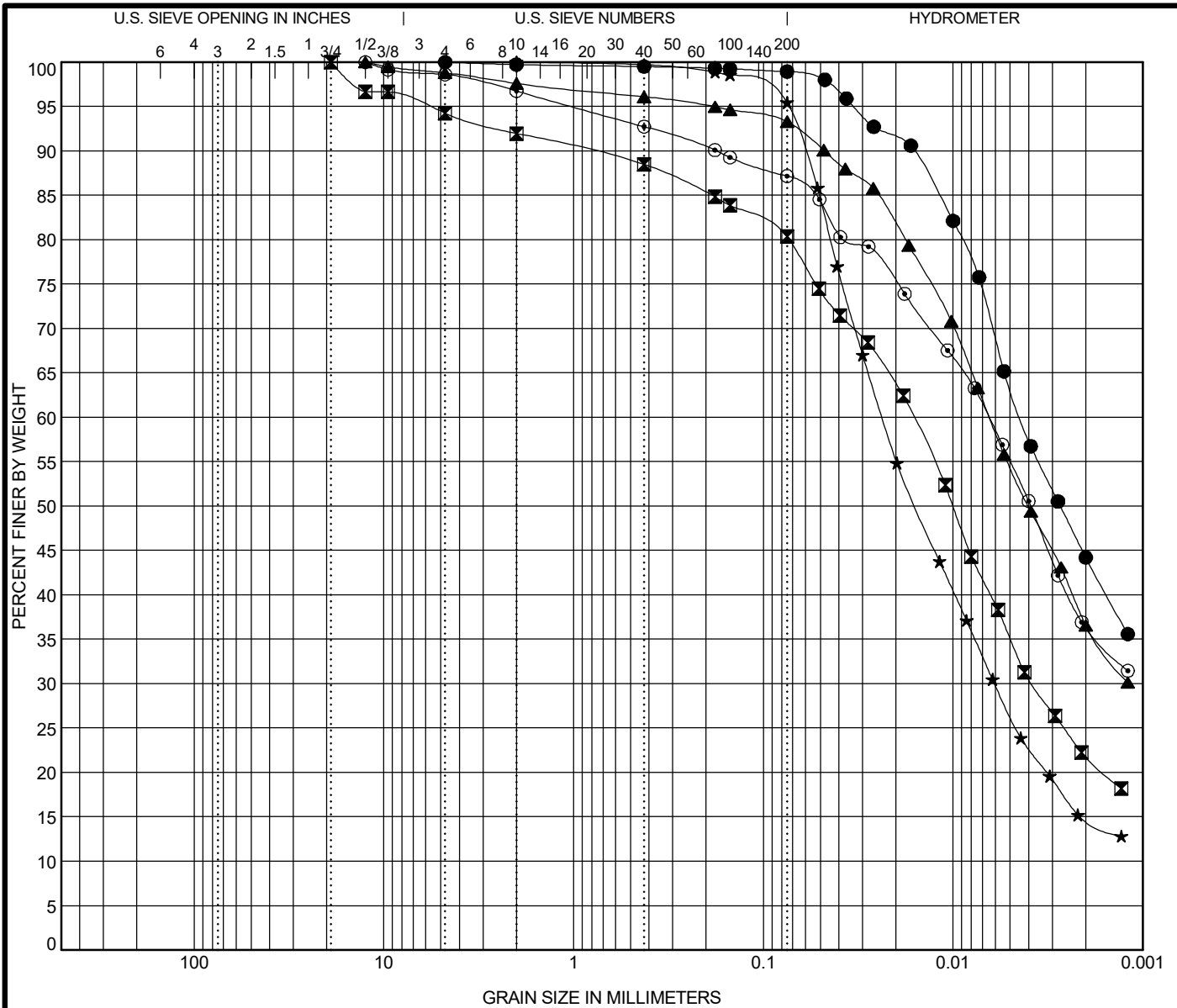
WEI GRAIN SIZE IDH 79011202.GPJ US LAB.GDT 8/26/21



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GRAIN SIZE DISTRIBUTION

Project: Pedestrian Bridge and Trail over IL 59
 Location: Streamwood, IL
 Number: 7901-12-02



COBBLES	GRAVEL	SAND		SILT AND CLAY
		coarse	fine	

Specimen Identification		IDH Classification				LL	PL	PI	Cc	Cu
●	TB-02#13 33.5 ft	Silty Clay				35	15	20		
☒	TB-02#18 58.5 ft	Silty Clay Loam				23	12	11		
▲	TB-03#6 13.5 ft	Silty Clay				30	13	17		
★	TB-03#18 58.5 ft	Silt				22	14	8		
◎	TB-04#2 3.5 ft	Silty Clay				34	18	16		
Specimen Identification		D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay	
●	TB-02#13 33.5 ft	4.75	0.004			0.3	0.8	54.7	44.2	
☒	TB-02#18 58.5 ft	19	0.016	0.004		8.0	11.8	58.3	21.8	
▲	TB-03#6 13.5 ft	12.5	0.006			2.4	4.4	56.7	36.5	
★	TB-03#18 58.5 ft	2	0.024	0.006		0.0	4.9	80.3	14.8	
◎	TB-04#2 3.5 ft	12.5	0.006			3.2	9.7	50.6	36.4	

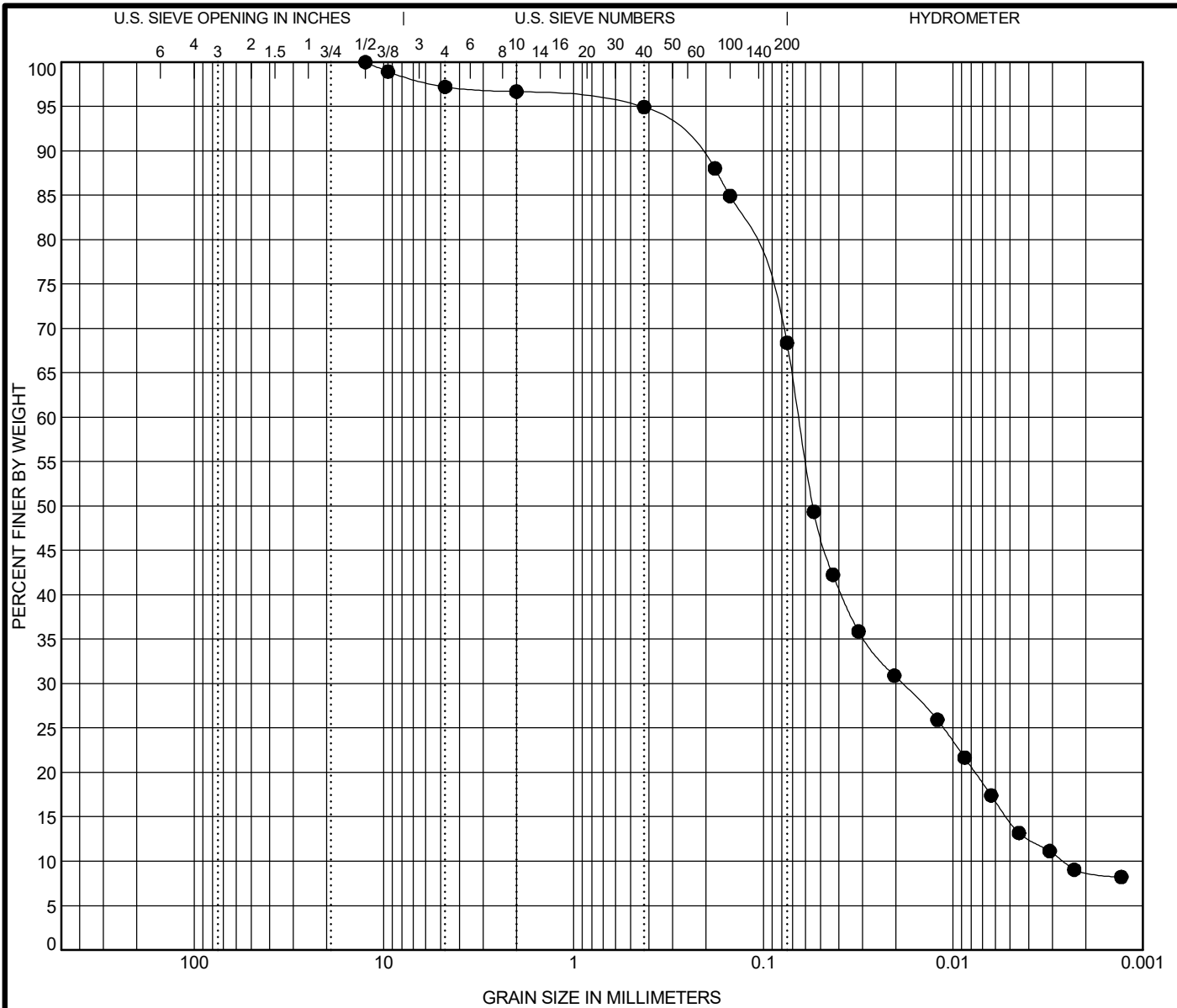
WEI GRAIN SIZE IDH 79011202.GPJ US LAB.GDT 8/26/21



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GRAIN SIZE DISTRIBUTION

Project: Pedestrian Bridge and Trail over IL 59
 Location: Streamwood, IL
 Number: 7901-12-02



COBBLES	GRAVEL	SAND		SILT AND CLAY
		coarse	fine	

Specimen Identification	IDH Classification	LL	PL	PI	Cc	Cu
● TB-04#18 58.5 ft	Silty Loam	NP	NP	NP	2.01	24.64

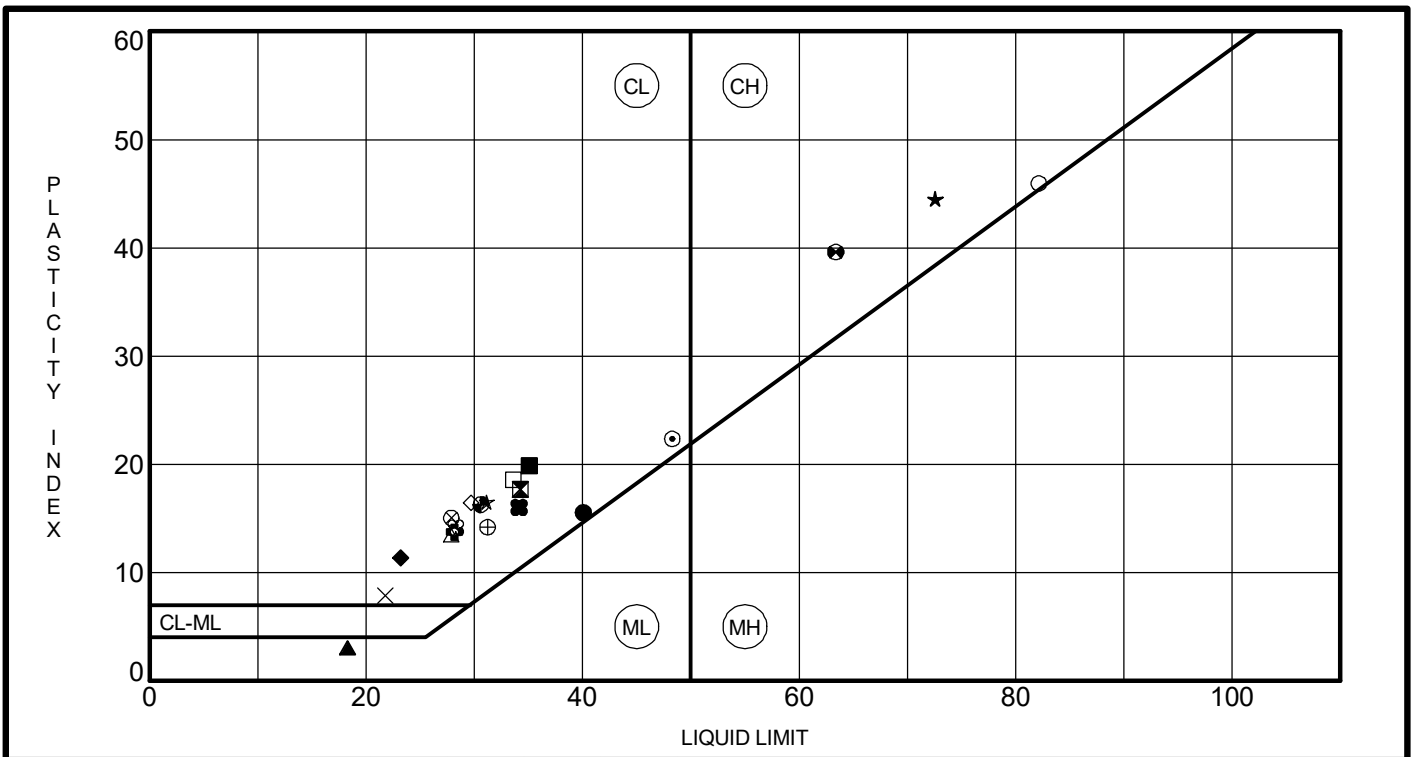
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
● TB-04#18 58.5 ft	12.5	0.065	0.019	0.003	3.3	29.1	58.8	8.8



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GRAIN SIZE DISTRIBUTION
 Project: Pedestrian Bridge and Trail over IL 59
 Location: Streamwood, IL
 Number: 7901-12-02

WEI GRAIN SIZE IDH 79011202.GPJ US LAB.GDT 8/26/21



Specimen Identification	LL	PL	PI	Fines	IDH Classification	
● B-1#8	18.5 ft	40	25	15	96	Silty Loam
⊗ B-2#7	16.0 ft	34	17	17	95	Silty Clay
▲ BWB-01#10	23.5 ft	18	15	3	99	Silt
★ BWB-04#8	18.5 ft	73	28	45	99	Silty Clay
⊙ BWB-05#8	18.5 ft	48	26	22	96	Silty Clay Loam
⊕ BWB-06#16	38.5 ft	28	14	14	78	Silty Clay
○ BWB-07#3	6.0 ft	82	36	46	94	Silty Clay Loam
△ BWB-08#7	16.0 ft	28	14	14	80	Silty Clay
⊗ BWB-10#6	13.5 ft	28	13	15	52	Gravelly Clay Loam
⊕ BWB-12#3	6.0 ft	31	17	14	85	Silty Loam
□ BWB-13#9	21.0 ft	34	15	19	89	Silty Clay
⊕ BWB-14#4	8.5 ft	63	24	39	98	Silty Clay Loam
⊕ TB-01#8	18.5 ft	31	14	17	83	Silty Clay Loam
☆ TB-01#10	23.5 ft	31	15	16	82	Silty Clay
⊗ TB-02#8	18.5 ft	28	14	14	78	Silty Clay Loam
■ TB-02#13	33.5 ft	35	15	20	99	Silty Clay
◆ TB-02#18	58.5 ft	23	12	11	80	Silty Clay Loam
◇ TB-03#6	13.5 ft	30	13	17	93	Silty Clay
× TB-03#18	58.5 ft	22	14	8	95	Silt
■ TB-04#2	3.5 ft	34	18	16	87	Silty Clay

WEI ATTERBERG LIMITS IDH 79011202.GPJ US LAB.GDT 8/26/21



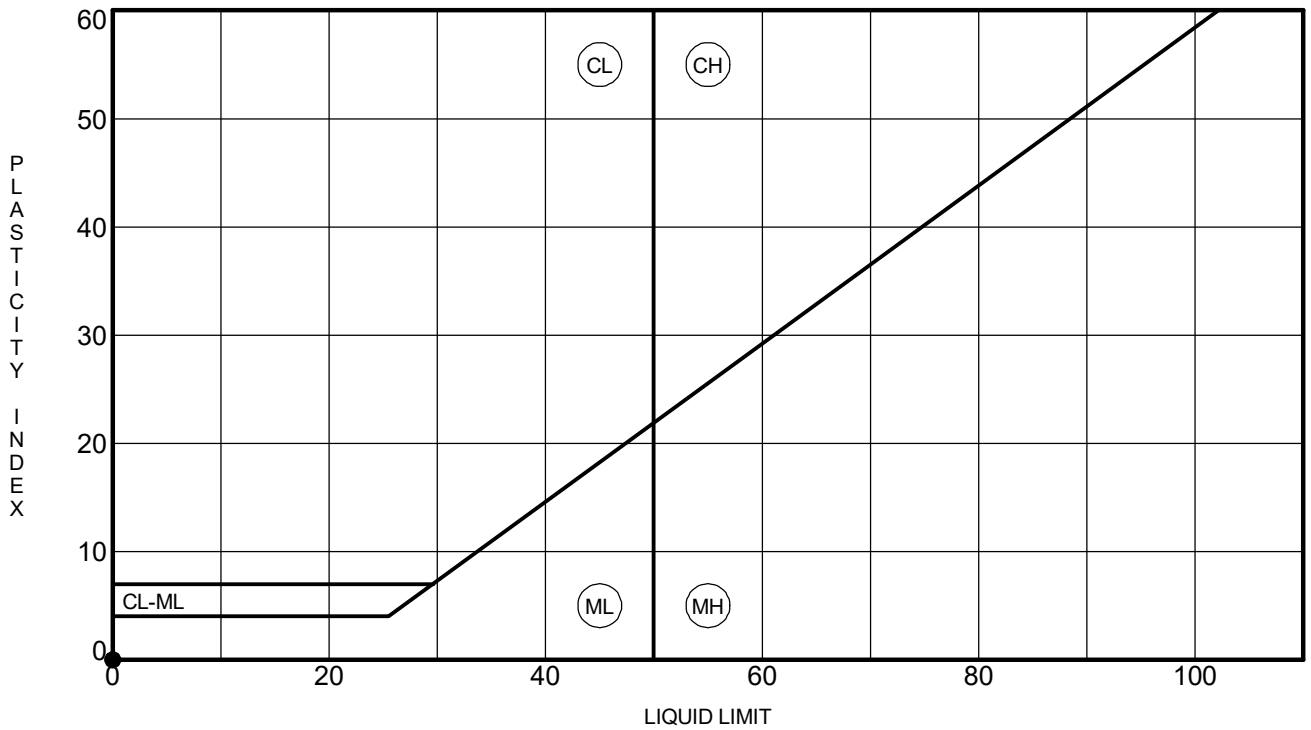
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ATTERBERG LIMITS' RESULTS

Project: Pedestrian Bridge and Trail over IL 59

Location: Streamwood, IL

Number: 7901-12-02



Specimen Identification	LL	PL	PI	Fines	IDH Classification
● TB-04#18 58.5 ft	NP	NP	NP	68	Silty Loam

WEI ATTERBERG LIMITS IDH 79011202.GPJ US LAB.GDT 8/26/21



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ATTERBERG LIMITS' RESULTS
 Project: Pedestrian Bridge and Trail over IL 59
 Location: Streamwood, IL
 Number: 7901-12-02

ORGANIC CONTENT in SOILS by LOSS on IGNITION
ASTM D 2974, Method C

Client: TranSystems
Project: IL 59 Overpass
WEI Job: 7901 12 02
Type/Condition: SS
Testing Furnace Temp °C.: 440

Analyst Name: L. Varzaru
Date Received: 5/3-5/10/2021
Date Tested: 5/24/2021

Sample No./ Depth	BWB-06 S#3 (6-7.5 ft.)	BWB-06 S#13 (31-32.5 ft.)	BWB-07 S#3 (6-7.5 ft.)		
Sample Description	Peat	Silty Clay	Silty Clay Loam		
wet soil + tare	80.46	107.16	88.01		
Dry Soil + Tare	48.03	84.06	64.11		
Tare Mass	42.09	43.24	42.42		
w (%)	546	57	110		
Dry Soil + Tare	48.03	84.06	64.11		
Ash+ Tare	44.72	81.92	60.11		
Tare Mass	42.09	43.24	42.42		
Ash Content (%)	44	95	82		
Organic Content (%)	55.7	5.2	18.4		

Prepared By: _____

Revised By: _____

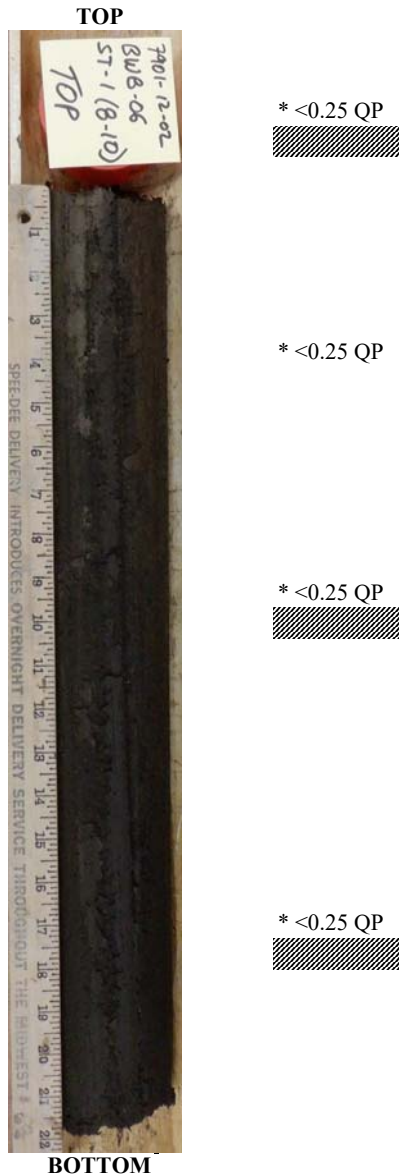
SHELBY TUBE DESCRIPTION

WEI JOB # 7901-12-02
PROJECT: IL-59 Overpass II
CLIENT: Transystems


ANALYST: M. Ciapas
EXTRUSION DATE: 6/25/2021

BORING ID: BWB-06
SAMPLE ID: ST-1
SAMPLE INTERVAL: 8-10ft


Length of Recovery 22"




Soil Description
Very Soft Brown PEAT -wet-

* <0.25 QP



* <0.25 QP

* <0.25 QP


* <0.25 QP


TOP	Tare Mass	42.21
	Wet + tare	102.3
	Dry + tare	53.3
	Moisture Content	442%
MIDDLE	Tare Mass	11.1
	Wet + tare	46.1
	Dry + tare	16.06
	Moisture Content	606%
BOTTOM	Tare Mass	41.41
	Wet + tare	108.04
	Dry + tare	54.52
	Moisture Content	408%

* Penetrometer Reading

 Moisture content taken in Hatched area

Unconfined Completed Y / N

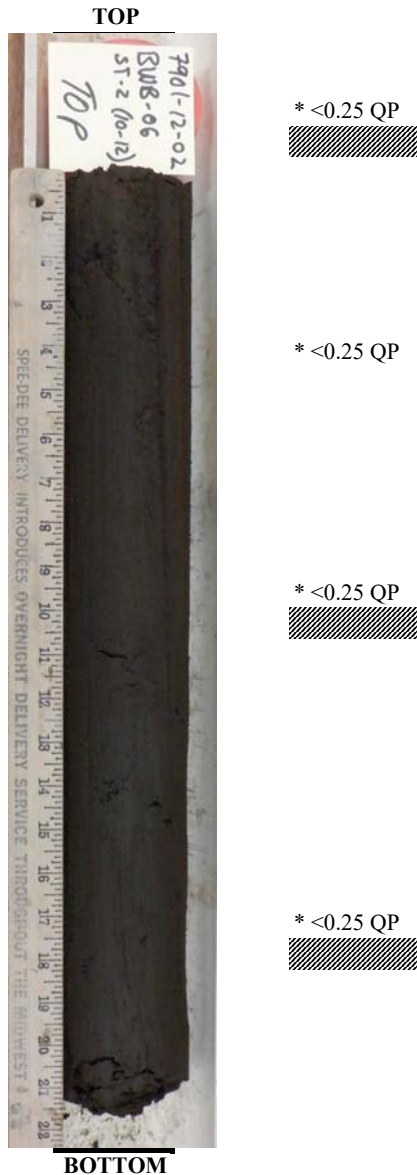
SHELBY TUBE DESCRIPTION

WEI JOB # 7901-12-02
PROJECT: IL-59 Overpass II
CLIENT: Transystems


ANALYST: M. Ciapas
EXTRUSION DATE: 6/25/2021

BORING ID: BWB-06
SAMPLE ID: ST-2
SAMPLE INTERVAL: 10-12ft


Length of Recovery 21.5"




Soil Description
Very Soft Brown PEAT -wet-


* <0.25 QP


* <0.25 QP

* <0.25 QP


* <0.25 QP


	Tare Mass	<u>43.52</u>
TOP	Wet + tare	<u>119.93</u>
	Dry + tare	<u>63.97</u>
	Moisture Content	274%
	Tare Mass	<u>11.1</u>
MIDDLE	Wet + tare	<u>48.01</u>
	Dry + tare	<u>18.93</u>
	Moisture Content	371%
	Tare Mass	<u>41.94</u>
BOTTOM	Wet + tare	<u>101.67</u>
	Dry + tare	<u>52.84</u>
	Moisture Content	448%

* Penetrometer Reading
 Moisture content taken in Hatched area

Unconfined Completed Y / N

APPENDIX C

Benchmark:

Existing Structure: None. IL Rte. 59 (Sutton Rd.) traffic to be maintained utilizing Stage Construction

No Salvage

SEISMIC DATA

Seismic Performance Zone (SPZ) =
 Design Spectral Acceleration at 1.0 sec. (SD1) =
 Design Spectral Acceleration at 0.2 sec. (SDS) =
 Soil Site Class =

DESIGN SPECIFICATIONS

2014 AASHTO LRFD Bridge Design Specifications, 7th Edition with 2015 Interims

DESIGN STRESSES

FIELD UNITS

$f'_c = 3,500$ psi
 $f_y = 60,000$ psi (Reinforcement)
 $f_y = 50,000$ psi (M270 Grade 50)

LOADING

Pedestrian Live Load = 90 psf
 Vehicle Live Load = H10

CURVE DATA

P.I. Sta. =
 $\Delta =$
 $D =$
 $R =$
 $T =$
 $L =$
 $E =$
 $e =$
 T.R. =
 S.E. Run =
 P.C. Sta. =
 P.T. Sta. =

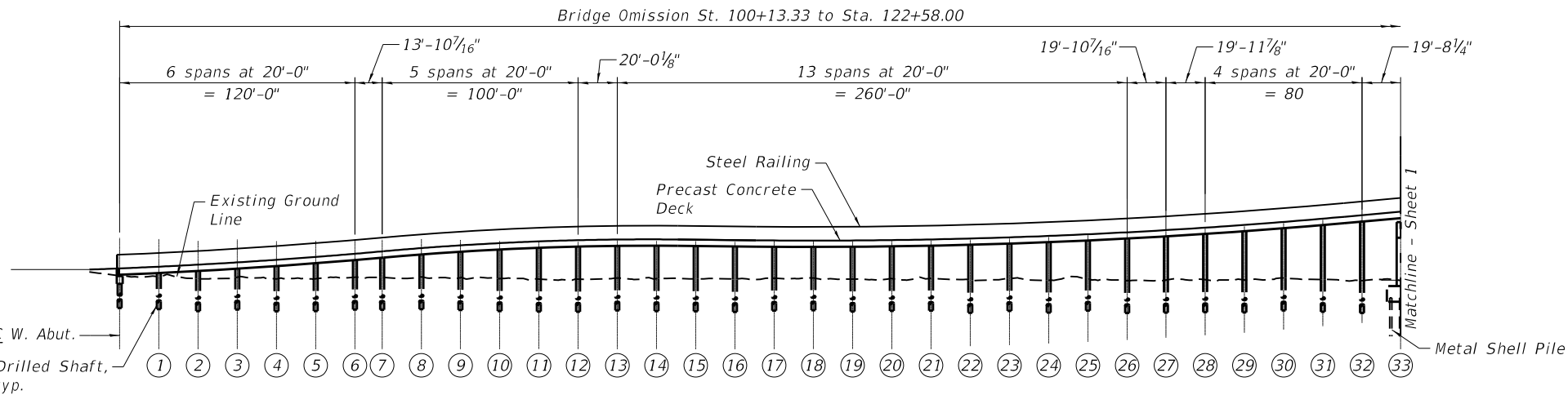
HIGHWAY CLASSIFICATION

Rte. - Rte.
 Functional Class:
 ADT: (20); (20)
 ADTT: (20); (20)
 DHV:
 Design Speed: m.p.h.
 Posted Speed: m.p.h.
 -Way Traffic
 Directional Distribution:

WATERWAY INFORMATION

Drainage Area = 5.90 Sq. Milesow Grade Elev. 779.34 @ Sta. -

Flood	Freq. Yr.	Q C.F.S.	Opening Ft ²		Nat. H.W.E.	Head - Ft.		Headwater El.	
			Exist.	Prop.		Exist.	Prop.	Exist.	Prop.
	10	10							
Design	30	441							
Base	100	673							
Overtopping	200	775							
Max. Calc.	500	1082							

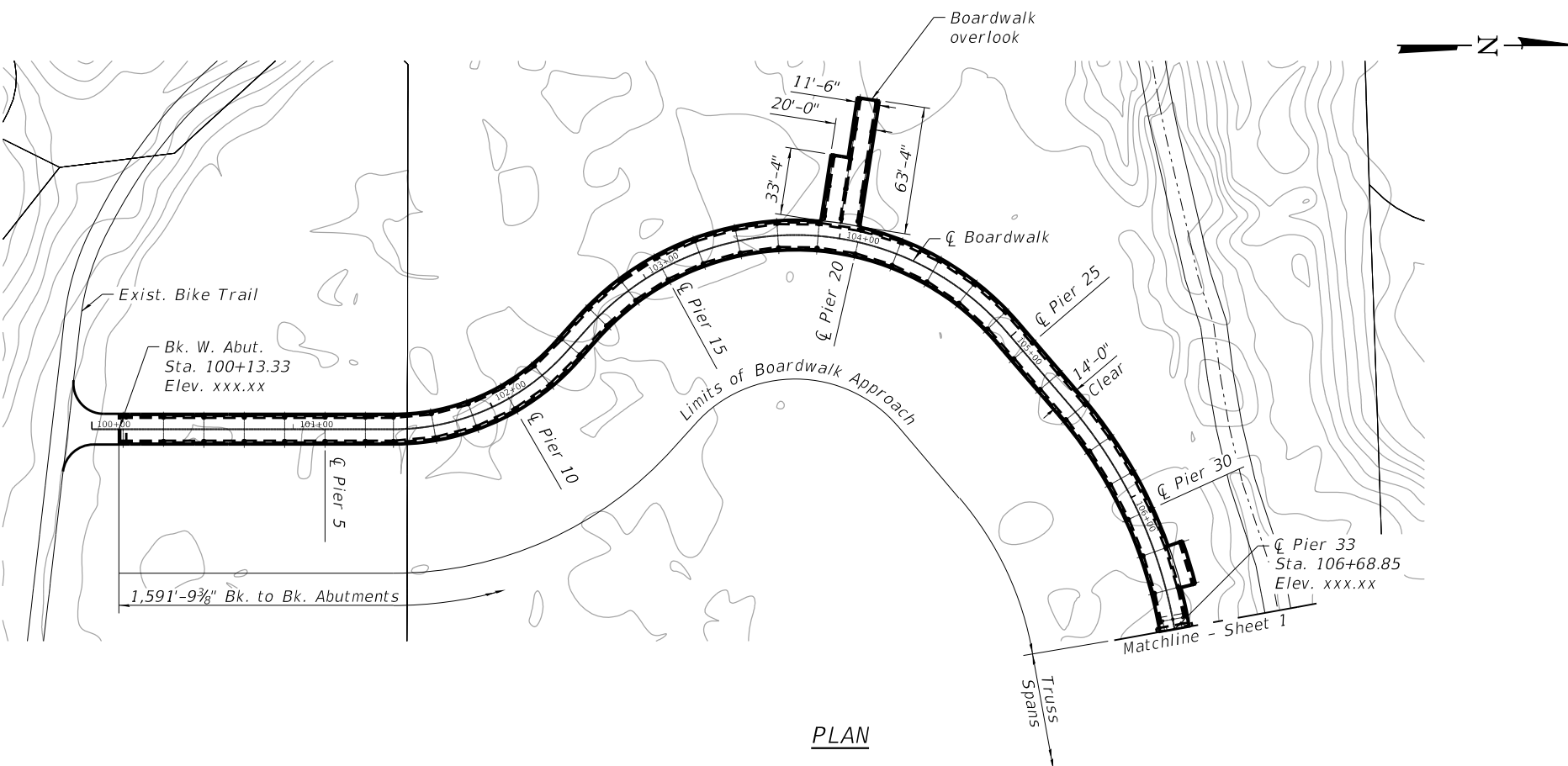


ELEVATION

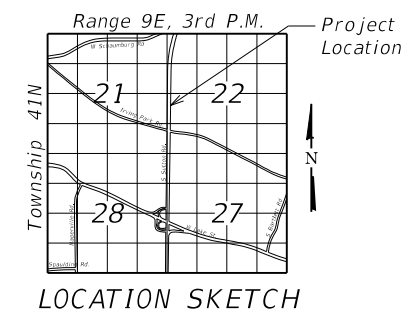
All dimensions along path baseline
 Scale shown is 1H:2V

DESIGN SCOUR ELEVATION TABLE

Event / Limit State	Design Scour Elevations (ft.)				
	W. Abut.	Pier -	Pier -	E. Abut.	Item 113
Q100					
Q200					
Design					
Check					

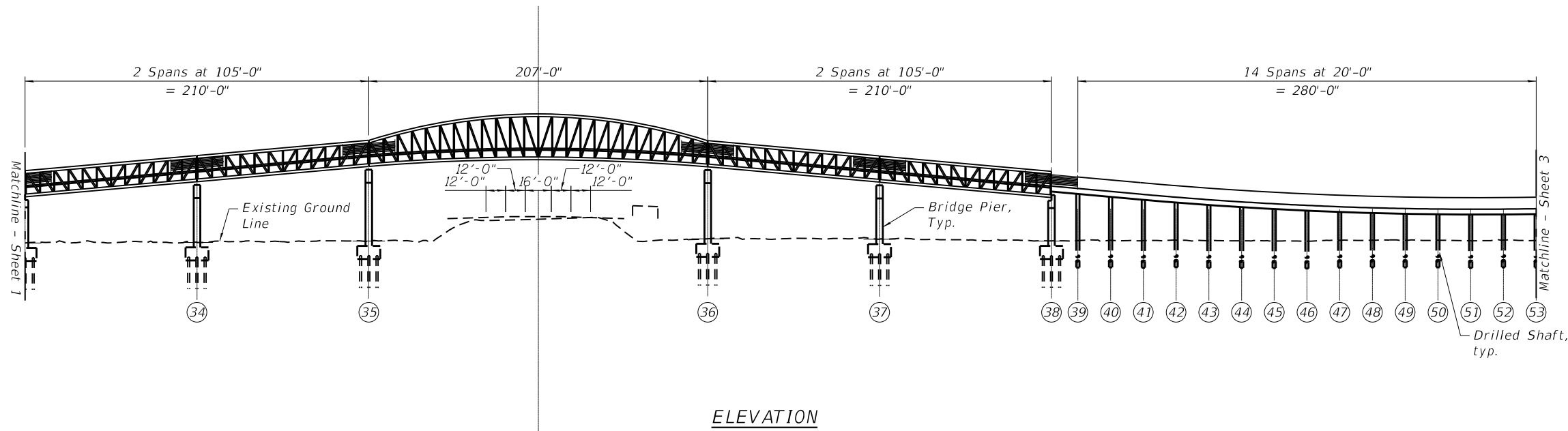


PLAN

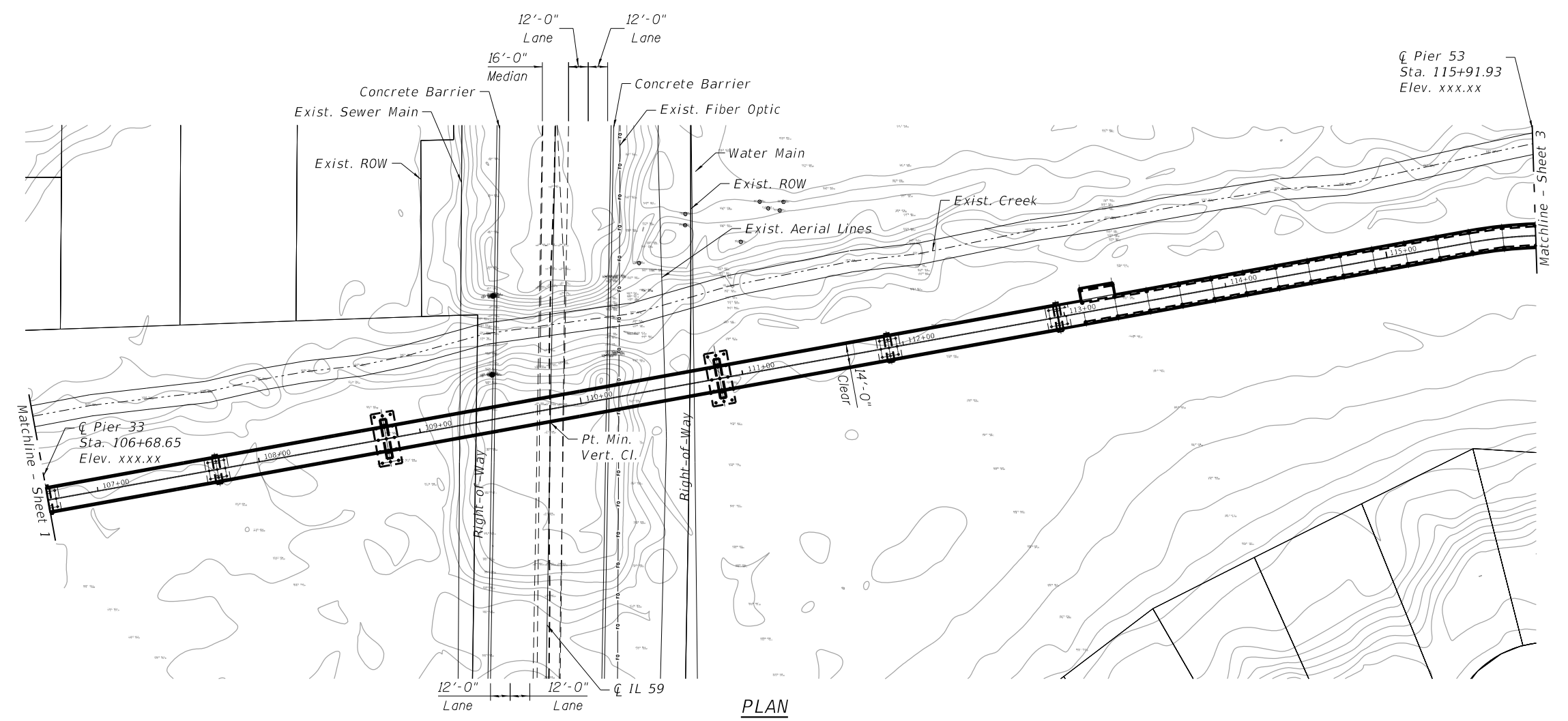


GENERAL PLAN & ELEVATION (1 OF 3)
TRAIL OVER ILLINOIS ROUTE 59
SECTION XX-XXXX-XX-XX
COOK COUNTY
STATION 100+00.00
STRUCTURE NUMBER XXX-XXXX

FILE NAME =	USER NAME =	DESIGNED -	REVISD -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
		CHECKED -	REVISD -						
		DRAWN -	REVISD -		CONTRACT NO.				
		CHECKED -	REVISD -		ILLINOIS FED. AID PROJECT				

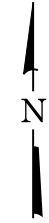


ELEVATION



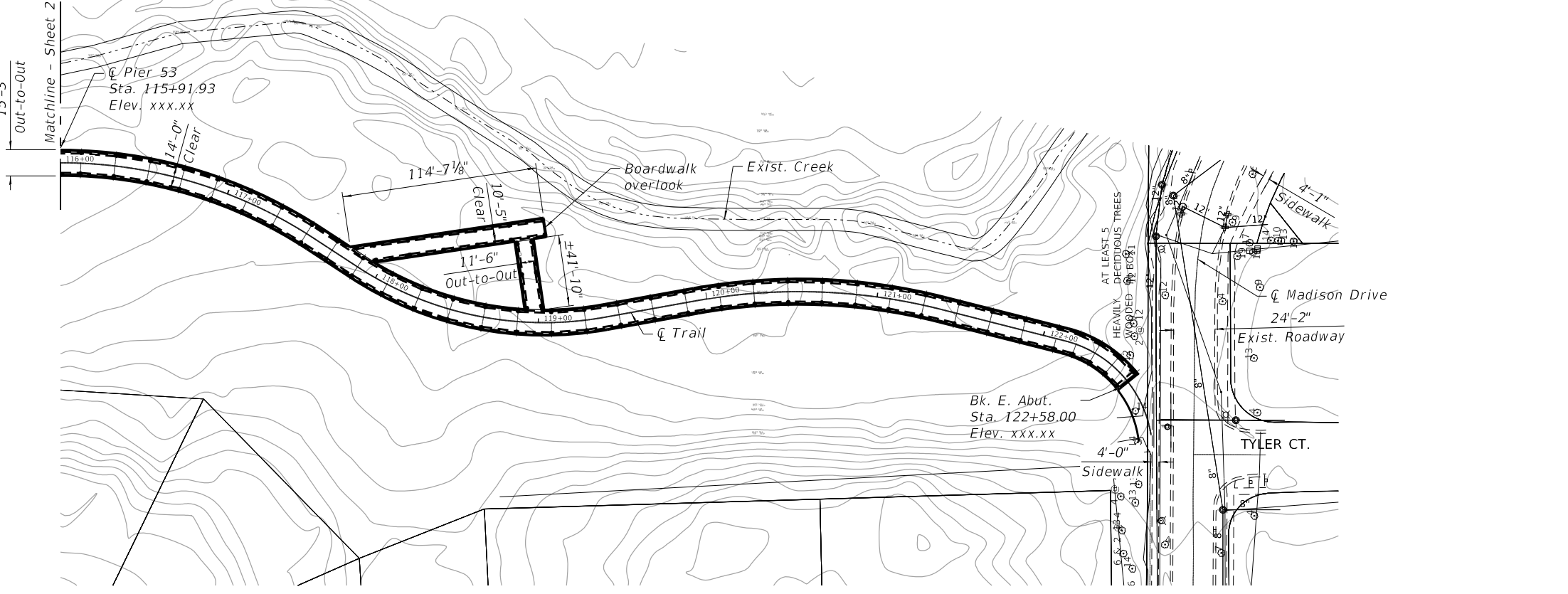
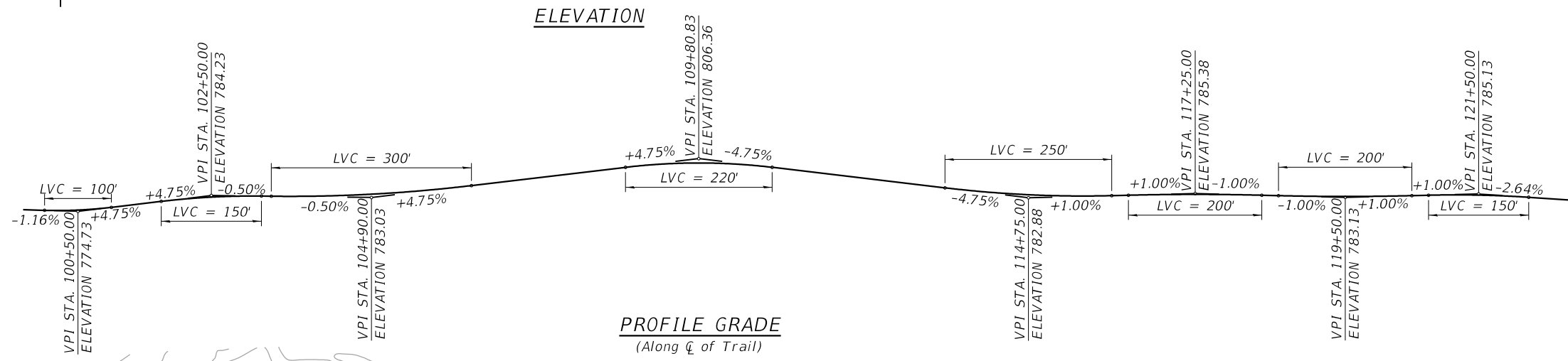
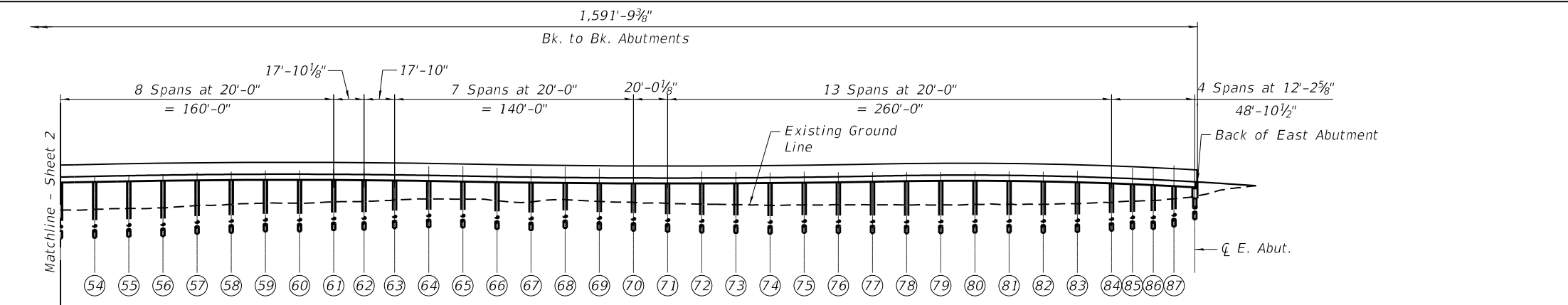
PLAN

Pier No.	Station	Design Elevations (ft.)		
		Deck Elev. @ C Path	Existing Ground	Design Scour
Bk. W. Abut.	100+13.33			
C W. Abut.	100+15.42	773.58	774.39	
Pier 1	100+35.42	773.64	774.33	
Pier 2	100+55.42	773.95	773.99	
Pier 3	100+75.42	774.49	774.14	
Pier 4	100+95.42	775.27	774.11	
Pier 5	101+15.42	776.21	774.01	
Pier 6	101+35.42	777.16	774.06	
Pier 7	101+49.29	777.82	774.04	
Pier 8	101+69.29	778.77	774.27	
Pier 9	101+89.29	779.69	774.09	
Pier 10	102+09.29	780.47	773.97	
Pier 11	102+29.29	781.11	773.99	
Pier 12	102+49.29	781.61	773.94	
Pier 13	102+69.30	781.97	774.00	
Pier 14	102+89.30	782.19	774.04	
Pier 15	103+09.30	782.27	774.12	
Pier 16	103+29.30	782.21	773.97	
Pier 17	103+49.30	782.12	774.02	
Pier 18	103+69.30	782.08	773.89	
Pier 19	103+89.30	782.12	773.72	
Pier 20	104+09.30	782.23	773.73	
Pier 21	104+29.30	782.41	773.78	
Pier 22	104+49.30	782.65	773.55	
Pier 23	104+69.30	782.97	773.55	
Pier 24	104+89.30	783.36	774.03	
Pier 25	105+09.30	783.82	773.66	
Pier 26	105+29.30	784.34	774.02	
Pier 27	105+49.17	784.94	774.14	
Pier 28	105+69.16	785.60	773.67	
Pier 29	105+89.16	786.34	773.72	
Pier 30	106+09.16	787.15	774.14	
Pier 31	106+29.16	788.03	773.69	
Pier 32	106+49.16	788.97	773.92	
Pier 33	106+68.85	789.90	773.86	
Pier 34	107+73.85	793.01	774.06	
Pier 35	108+78.85	798.00	774.28	
Pier 36	110+85.85	797.87	774.96	
Pier 37	111+90.85	792.88	774.98	
Pier 38	112+95.85	787.90	774.44	
Pier 39	113+11.93	789.01	774.52	
Pier 40	113+31.93	788.06	774.37	
Pier 41	113+51.93	787.10	774.36	
Pier 42	113+71.93	786.21	774.35	
Pier 43	113+91.93	785.40	774.34	
Pier 44	114+11.93	784.69	774.34	
Pier 45	114+31.93	784.07	774.20	
Pier 46	114+51.93	783.55	774.35	
Pier 47	114+71.93	783.11	774.44	
Pier 48	114+91.93	782.77	774.57	
Pier 49	115+11.93	782.52	774.33	
Pier 50	115+31.93	782.36	774.33	
Pier 51	115+51.93	782.29	774.30	
Pier 52	115+71.93	782.31	774.49	
Pier 53	115+91.93	782.43	774.51	



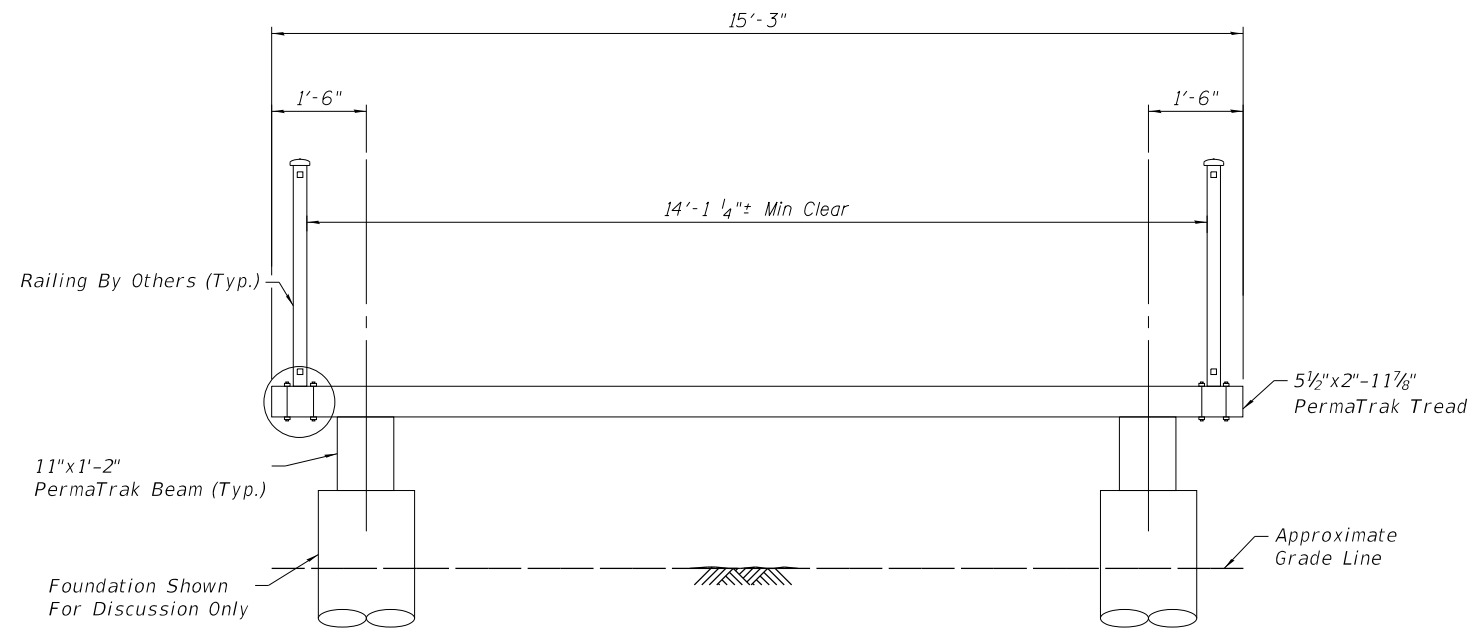
GENERAL PLAN & ELEVATION (2 OF 3)
 TRAIL OVER ILLINOIS ROUTE 59
 SECTION XX-XXXX-XX-XX
 COOK COUNTY
 STATION 100+00.00
 STRUCTURE NUMBER XXX-XXXX

FILE NAME =	USER NAME =	DESIGNED -	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
		CHECKED -	REVISED -						
		PLOT SCALE =	REVISED -			CONTRACT NO.			
		PLOT DATE =	REVISED -			ILLINOIS FED. AID PROJECT			

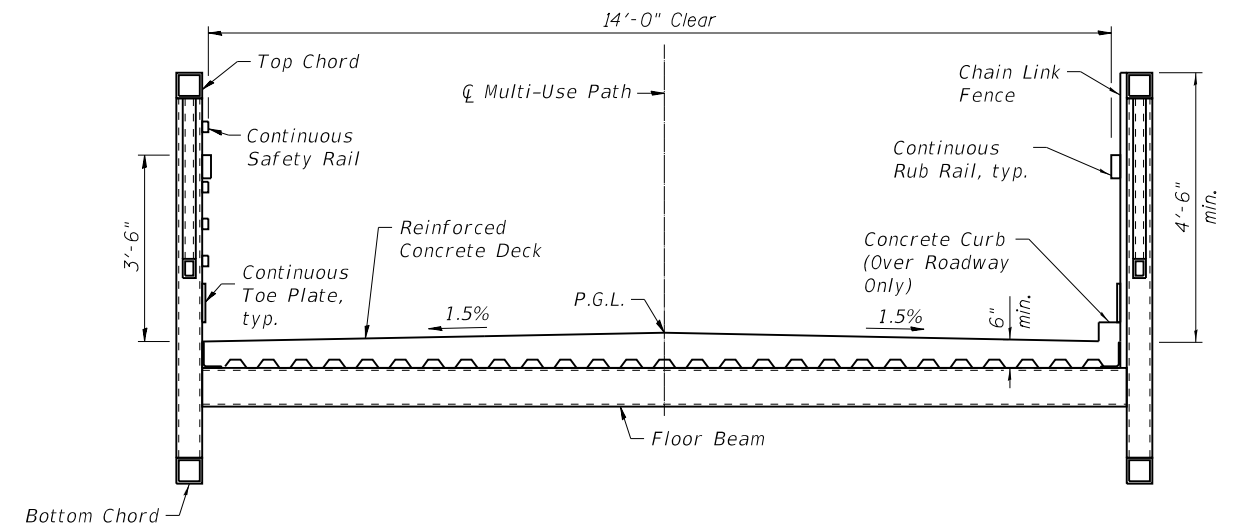


Pier No.	Station	Design Elevations (ft.)		
		Deck Elev. @ ζ Path	Existing Ground	Design Scour
Pier 54	116+11.93	782.62	774.48	
Pier 55	116+31.93	782.82	774.71	
Pier 56	116+51.93	782.99	774.93	
Pier 57	116+71.93	783.11	775.43	
Pier 58	116+91.93	783.20	775.99	
Pier 59	117+11.93	783.25	776.21	
Pier 60	117+31.93	783.25	776.21	
Pier 61	117+51.93	783.22	776.44	
Pier 62	117+69.77	783.15	776.76	
Pier 63	117+87.60	783.06	776.93	
Pier 64	118+07.60	782.91	777.34	
Pier 65	118+27.60	782.73	777.29	
Pier 66	118+47.60	782.53	776.90	
Pier 67	118+67.60	782.34	776.49	
Pier 68	118+87.60	782.20	777.07	
Pier 69	119+07.60	782.09	776.61	
Pier 70	119+27.60	782.03	776.56	
Pier 71	119+47.61	782.01	776.15	
Pier 72	119+67.61	782.02	775.94	
Pier 73	119+87.61	782.08	775.85	
Pier 74	120+07.61	782.17	775.90	
Pier 75	120+27.61	782.31	775.95	
Pier 76	120+47.61	782.48	775.95	
Pier 77	120+67.61	782.68	776.06	
Pier 78	120+87.61	782.86	776.00	
Pier 79	121+07.61	782.95	775.92	
Pier 80	121+27.61	782.95	775.96	
Pier 81	121+47.61	782.84	776.01	
Pier 82	121+67.61	782.64	776.06	
Pier 83	121+87.61	782.34	776.28	
Pier 84	122+07.61	781.95	776.58	
Pier 85	122+19.83	781.66	776.69	
Pier 86	122+32.05	781.34	776.84	
Pier 87	122+44.28	781.02	777.32	
ζ E. Abut.	122+56.50	780.69	778.08	
Bk. E. Abut.	122+58.00			

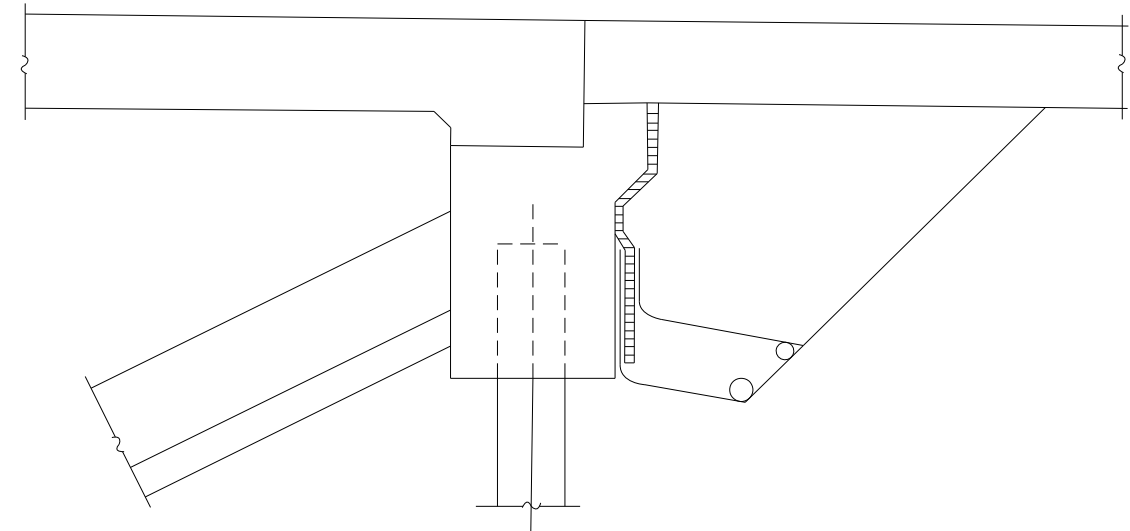
GENERAL PLAN & ELEVATION (3 OF 3)
 TRAIL OVER ILLINOIS ROUTE 59
 SECTION XX-XXXX-XX-XX
 COOK COUNTY
 STATION XXX+XX.XX
 STRUCTURE NUMBER XXX-XXXX



BOARDWALK CROSS SECTION



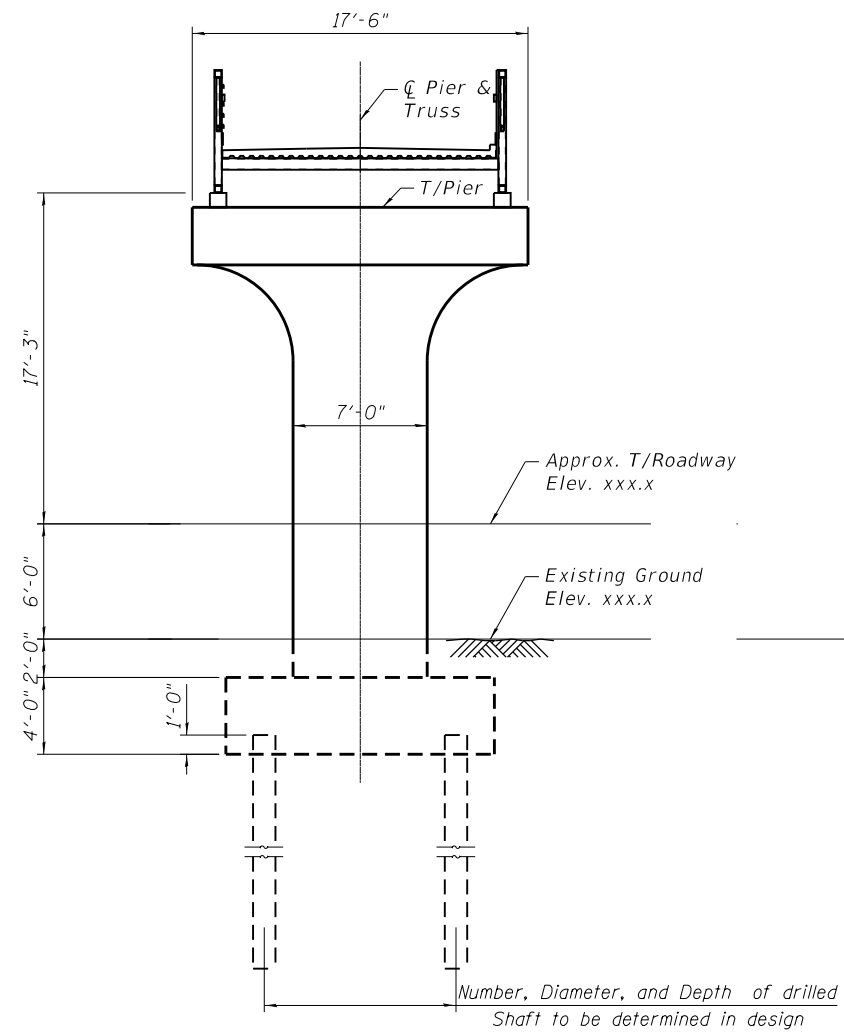
TRUSS CROSS SECTION



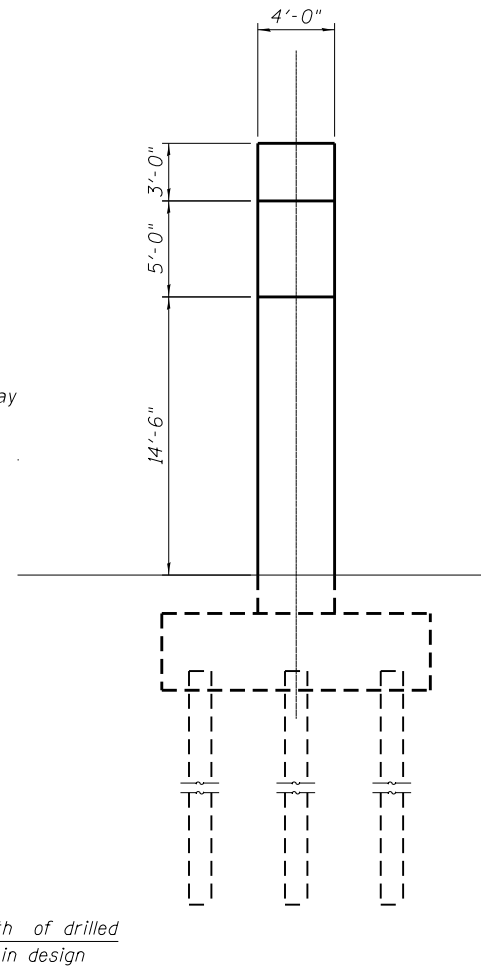
TRUSS PIER DETAIL

DETAILS (1 OF 2)
TRAIL OVER ILLINOIS ROUTE 59
SECTION XX-XXXX-XX-XX
COOK COUNTY
STATION XXX+XX.XX
STRUCTURE NUMBER XXX-XXXX

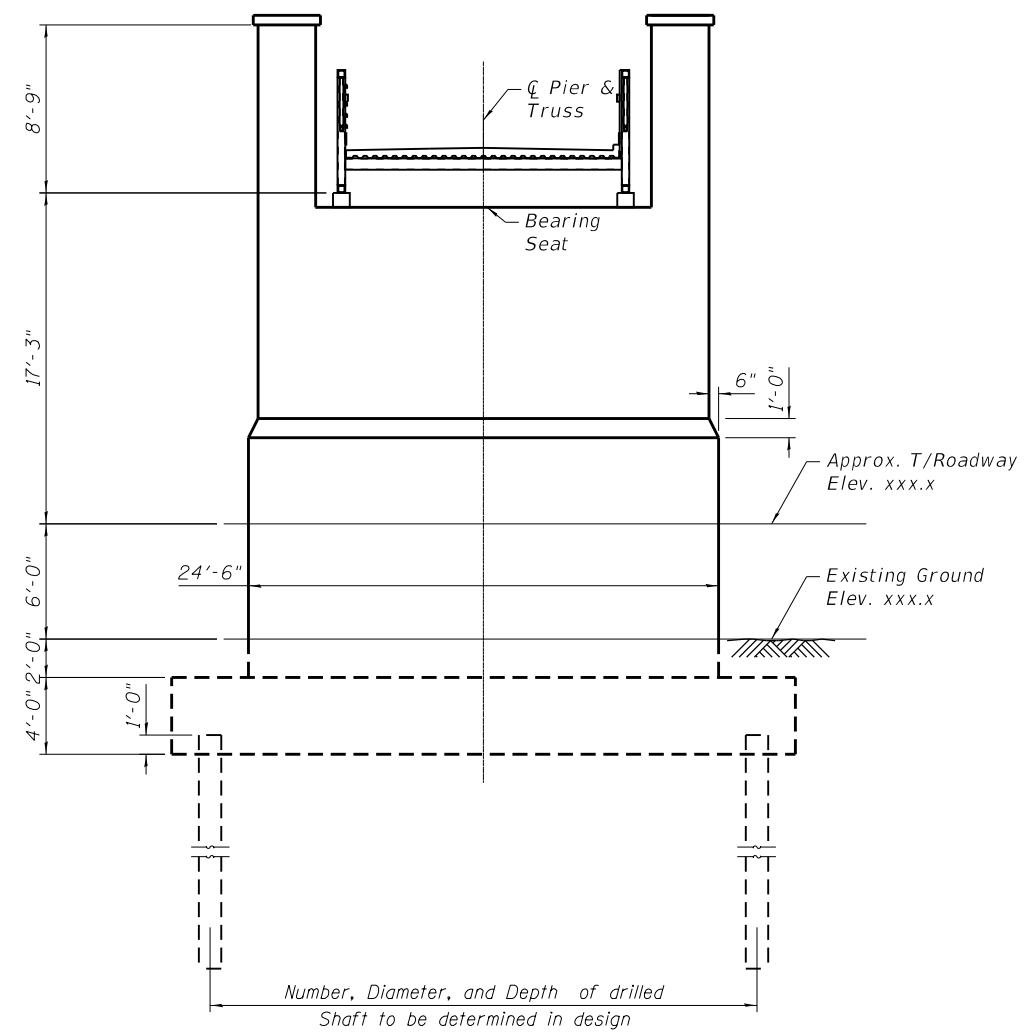
FILE NAME =	USER NAME =	DESIGNED -	REVISÉ -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	F.A. RTÉ.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
		CHECKED -	REVISÉ -						
		DRAWN -	REVISÉ -		CONTRACT NO.				
		CHECKED -	REVISÉ -		ILLINOIS FED. AID PROJECT				



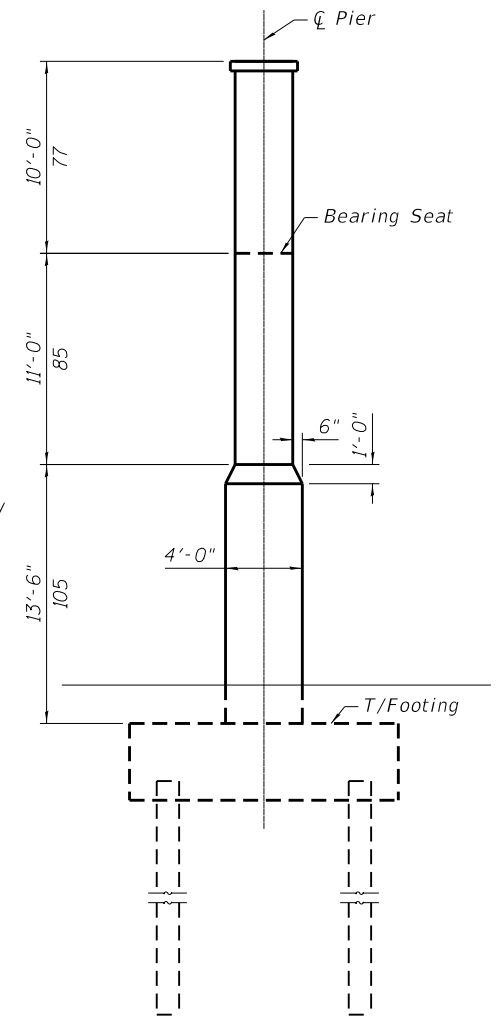
TRUSS PIER FRONT ELEVATION
(Pier No. 33 through Pier No. 38)



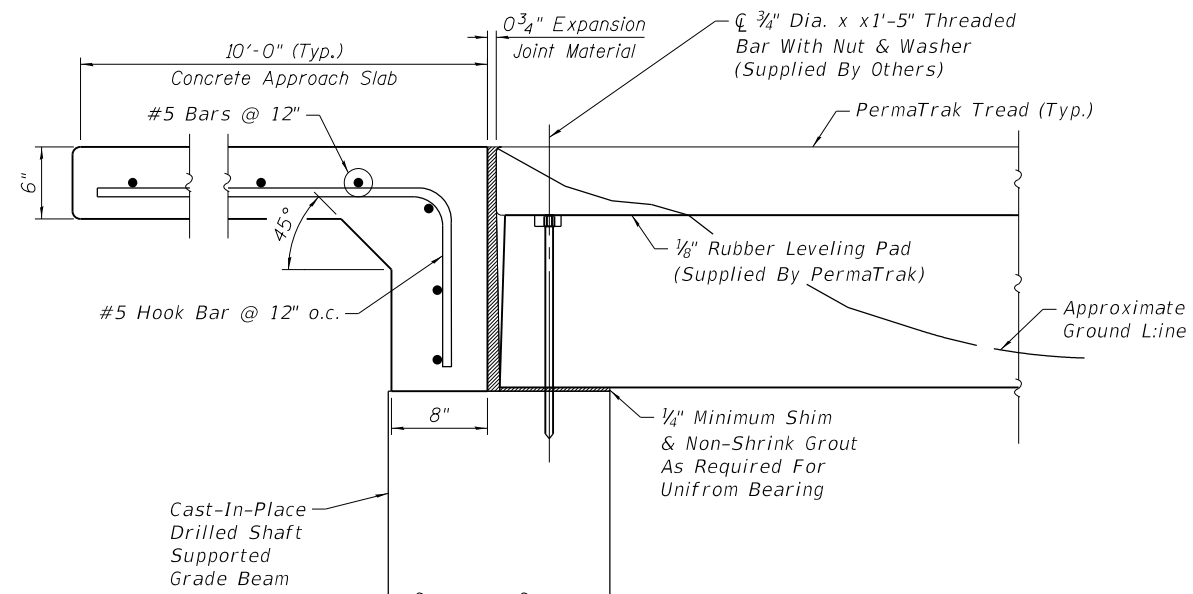
TRUSS PIER SIDE ELEVATION



BOARDWALK PIER FRONT ELEVATION
(Pier No. 1 through Pier No. 32,
Pier No. 39 through Pier No. 87)



BOARDWALK PIER SIDE ELEVATION



SECTION THROUGH ABUTMENT
Railing not shown for clarity

DETAILS (2 OF 2)
TRAIL OVER ILLINOIS ROUTE 59
SECTION XX-XXXX-XX-XX
COOK COUNTY
STATION XXX+XX.XX
STRUCTURE NUMBER XXX-XXXX

FILE NAME =	USER NAME =	DESIGNED -	REVISÉ -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
		CHECKED -	REVISÉ -						
		DRAWN -	REVISÉ -		CONTRACT NO.				
		CHECKED -	REVISÉ -		ILLINOIS FED. AID PROJECT				