# ROADWAY GEOTECHNICAL REPORT US 52 IOWA SECTION FROM STATION 1546+00 to STATION 1560+65 IDOT PROJECT P-92-001-11 SECTION 104B-2, CONTRACT No. 64G59 JACKSON COUNTY, IOWA

# for

#### **PARSONS**

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# ROADWAY GEOTECHNICAL REPORT US ROUTE 52 IOWA SECTION FROM STATION 1546+00 to STATION 1560+65 IDOT PROJECT P-92-001-11 SECTION 104B-2, CONTRACT No. 64G59 JACKSON COUNTY, IOWA FOR PARSONS

#### 1.0 INTRODUCTION

This roadway geotechnical report presents the results of Wang Engineering, Inc. (Wang) subsurface investigation, laboratory testing, groundwater conditions, and geotechnical evaluations performed to support the design and construction of a section of US Route 52 (US 52) in Jackson County, Iowa. A *Site Location Map* is presented as Exhibit 1. The purpose of our geotechnical work was to evaluate the subsurface soil and groundwater conditions within this project area that would form a basis for design and construction recommendations and provide a report summarizing the results of our studies, conclusions, and recommendations.

#### 1.1 Project Description

The US 52/IL 64/IL 84 project includes the replacement of the existing US 52 Bridge over the Mississippi River and reconstruction of the IL 84 from approximately Randolph Street to approximately 1290 feet north of US 52 Bridge. The new bridge will be wider and will be on a new alignment to the south of the existing location to eliminate the need for an excessive detour route. The IL 84 work includes widening of shoulders, which requires, on the east side of the roadway, deep cut into the bluff and retaining wall, upgrading existing guardrail, culvert replacement, and roadway resurfacing. The existing bridge will remain in service until the new bridge is open to the traffic. A connection will be made from the old Iowa's causeway to the new Iowa approach.

According to plans and cross sections provided by Parsons Transportation Group (Parsons), Wang understands that approximately 1500 feet of US 52 Iowa section roadway will be reconstructed just west of the US 52 Bridge over the Mississippi River between Station 1546+00 and Station 1560+65. The improvement consists of realigning, widening, and full reconstruction of this section of the roadway to accommodate the construction of the new US 52 Bridge over



Mississippi River. The reconstructed roadway will have 12-foot wide traffic lane, 8-foot wide shoulder and guardrail in each direction. The embankments on both the sides are proposed to be with a maximum slope of 1(V): 2(H).

#### 1.2 Other Reports

#### **Geotechnical Reports**

Structure Geotechnical Reports (SGR) for the US 52 Bridge over Mississippi River and IL 84 retaining wall were completed by Wang. Wang also prepared a separate Roadway Geotechnical Report (RGR) for IL 84 roadway.

#### **Iowa DOT Reports**

Iowa Department of Transportation District 6 prepared three reports dated November 14, 2012, January 14, 2014 and March 21, 2014 for US 52 Causeway related to the original construction and existing condition of the roadway pavement and embankment.

#### 2.0 GEOLOGIC SETTING

The project area is located just north of Savanna, Carroll County, Illinois, and two miles north of Sabula, Jackson County, Iowa. On the USGS *Savanna Quadrangle 7.5 Minute Series* map, the proposed roadway improvement in the NE¼ of Section 8 of Tier 84 N, Range 7 E of the Fifth Principal Meridian.

The following review of published geologic data, with emphasis on factors that might influence the design and construction of the proposed engineering works, is meant to place the project area within a geological framework and, thus, to confirm the dependability and consistency of the present subsurface investigation results. For the study of the regional geologic framework, Wang considered northwestern Illinois and northeastern Iowa area in general and Carroll and Jackson Counties in particular. Exhibit 3 illustrates the *Site and Regional Geology*.

#### 2.1 Physiography

Northwestern Carroll County and a slice of northeastern Jackson County are part of the Wisconsin Driftless Section, which lacks a cover of tills and erratics. It is a low plateau area dissected by the



outwash-filled valley of the upper Mississippi River (Leighton et al. 1948). On the Iowa side, west of the main river channel, forested, marshy lowlands make up a more than 1.5-mile wide floodplain having elevations lower than 600 feet. At the time of our investigation, the Mississippi River water elevation measured 582.5 feet.

#### 2.2 Pedological Features

In Jackson County, Iowa the soil types were surveyed by the USDA (2013). Summaries of the USDA soil types along the project area, including their relevant geotechnical index properties and suitability as subgrade and road fill are shown in the *Site Pedological Map and Table* (Exhibits 2-1 and 2-2). The soil information provided by USDA is meant to be used as a general reference in the absence of a site-specific investigation. In this instance, our findings regarding soil features affecting suitability for highway and street construction are not necessarily consistent with the information presented in Exhibit 2. Though a useful guide to general surficial soil conditions within the county, the USDA ratings for soil frost susceptibility and suitability as road subgrade seem overly critical and are probably based on different criteria than those applied in roadway engineering works.

#### 2.3 Surficial Cover

The surficial cover is made up of stratified sand, silt, and clay of the Cahokia Alluvium, which makes up the modern floodplain and channel deposits, and sand and gravel outwash deposits of the Henry Formation that probably fills most of the valley (Grimley 1997). Along the new roadway alignment the surficial cover thickness is more than 120 feet.

#### 2.4 Bedrock

The project lies on the southwestern flank of the gently sloping Wisconsin Arch. The bedrock outcrops along the Illinois side of the Mississippi River known as the Mississippi Palisades. Westward, across the river valley, the bedrock surface elevation becomes gradually deeper; on the Iowa side of the main river channel bedrock lies at approximately 120 feet below the water surface. The general lithological profile includes Ordovician shale and dolostones of the Glena Group and Platteville Formation (Witzke et al. 1998, Frankie 2001).

The existing and proposed US 52 roadway and bridge alignments are located within the up to 4,000-foot wide, east-west trending Plum River Fault Zone. The fault is considered inactive, but



the near vertical fault lines with a strike-slip component of movement have resulted in a structure with horsts and grabens and zones of breccia with rotated blocks. The Silurian dolostones that crop out in the Mississippi Palisades are part of the down-through fault block, whereas the upthrown block south of Savanna is made up of Ordovician rocks. Just west of the existing bridge, the vertical displacement along the fault plane likely measures 100 to 150 feet (Kolata and Buschbach 1976).

Our subsurface investigation results fit into the local geologic context. The borings drilled in the project area revealed the native sediments consists of gravelly sand, sand, and silt (Cahokia Alluvium and the Henry Formation). Along the Iowa side of the proposed US 52, borings drilled for the proposed bridge over Mississippi River encountered the bedrock at depths ranging from 58.0 to 132.0 feet below ground surface (bgs), that is, at elevations of 524.2 to 455.0 feet.

#### 2.5 Climatological Data

The main subsurface investigation was performed from October to November 2013. To assess the possible effects of temperature and precipitation on water table data and soil moisture, the climatic conditions for the investigation period and three months prior to the start of the investigation are summarized graphically in Figures 1 and 2. The precipitation and temperature data for the investigation period are compared against thirty-year monthly data (1981 to 2010) in box-and-whiskers format to illustrate deviations from "normal" climate conditions during the current investigation. Local climatologic data were obtained from the Mount Carroll, Illinois Station (NCDC 2013).



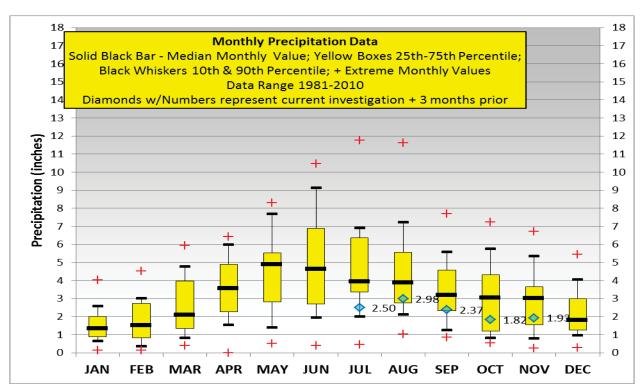


Figure 1: Monthly Precipitation Data

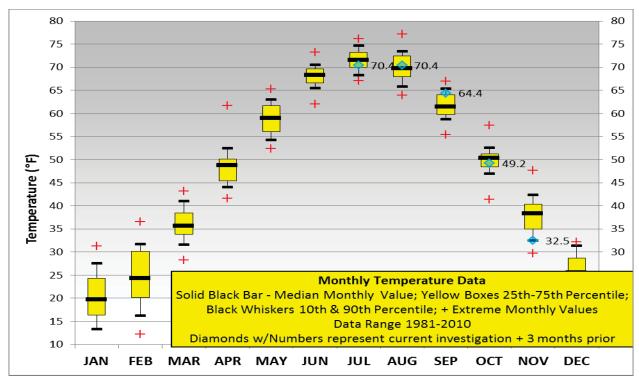


Figure 2: Monthly Temperature Data



No significant precipitation and temperature deviations from the historical 30-year climate data were recorded during our soil investigation period and the three month before it. However, groundwater level may be influenced by seasonal water level fluctuations.

#### 3.0 METHODS OF INVESTIGATION

The following sections outline the subsurface and laboratory investigations.

#### 3.1 Subsurface Exploration

To characterize the subgrade and groundwater conditions along the investigated US 52 alignment, we utilized a combination of SGB (subgrade) and BSB (bridge) borings. Wang drilled 12 subgrade borings designated as 52-SGB-01 through 52-SGB-12 and also considered bridge boring BSB-24.

The borings were advanced to depths ranging from 29.5 to 120.0 feet bgs. The boring locations were marked in the field by Wang based on approved plans. The as-drilled northing, easting, and elevation were acquired with a mapping-grade GPS unit. Stations and offsets were determined from design drawings provided by Parsons. Boring location data are included in the *Boring Logs* (Appendix A) and in the *Soil Boring Locations Plans and Soil Profiles* (Appendix E).

The boring were completed using truck-mounted drilling rigs. The drilling rigs were equipped with hollow stem augers to advance and maintain an open borehole. Soil sampling was performed according to AASHTO T 206, "Penetration Test and Split Barrel Sampling of Soils." The soil was continuously sampled to 10 feet, at 2.5 feet intervals 10 to 30 feet, and at 5 feet thereafter in the subgrade/embankment borings, whereas soil was sampled at 2.5 feet intervals to 30 feet and at 5 feet thereafter in the structure boring. Samples collected from each sampling interval were placed in sealed jars. A total of seven Shelby tube samples were obtained from selected locations shown in the Soil Boring Locations Plans and Soil Profiles (Appendix E).

Field boring logs, prepared and maintained by Wang soil inspectors and geologists, included lithological descriptions, visual-manual classifications, Rimac and pocket penetrometer unconfined compressive strength test results. Results of Standard Penetration Tests (SPT) are recorded on the boring logs as blows per 6 inches of penetration.



Groundwater levels were measured while drilling and at completion of each boring. For safety considerations each boreholes was backfilled upon completion with soil cuttings and/or bentonite chips and, where necessary, the pavement surface was restored to its original condition.

#### 3.2 Laboratory Testing

The laboratory testing program included water content determination (AASHTO T 265) on all samples and particle size (T 88), and Atterberg limits (T 89 and T 90) tests on selected soil samples. Tested samples were classified according to the IDH and AASHTO classification systems. Field visual-manual classifications were also verified in the laboratory. The results of the laboratory testing program are shown in the attached *Boring Logs* (Appendix A), IDOT form BMPR 508A (Appendix B), and *Laboratory Test Results* (Appendix C).

#### 4.0 RESULTS OF SUBSURFACE INVESTIGATIONS

Detailed descriptions of the lithological units encountered by the borings are presented in the attached *Boring Logs* (Appendix A) and in the *Soil Boring Locations Plans and Soil Profiles* (Appendix D). Please note that the strata contact lines shown on logs and profiles represent approximate boundaries between soil types; the actual transition between soil types might be gradual in horizontal and vertical directions.

Beneath surface, the general lithological succession includes 1) man-made ground (fill) and 2) sand and gravelly sand.

#### 4.1 Surface Characterization

Existing US 52 roadway is flanked by riprap covered slopes and discontinuous vegetation. The proposed roadway alignment will be established over both existing riprap slopes and pavement and water covered land.

Borings 52-SGB-04, 52-SGB-07, 52-SGB-09, 52-SGB-11, and 52-SGB-12 were performed at the water covered base of the existing roadway embankment. At the time of our investigation, the depth of water measured 4 to 7 feet.



Borings 52-SGB-01, 52-SGB-02, 52-SGB-03, 52-SGB-05, 52-SGB-06, 52-SGB-08, and 52-SGB-10 were performed through the existing roadway and show the pavement structures consist of 4- to 7-inch thick asphalt over aggregate base. The aggregate base consists of gravelly sandy loam and its thickness ranges from 8 to more than 29 inches.

#### 4.2 Soil Conditions

The following sections present the soils conditions encountered during our subsurface investigation along the proposed roadway alignment. Beneath surface, in descending order, the borings encountered 1) man-made ground (fill); 2) very loose to loose sand to sandy loam; 3) very soft to stiff silty clay loam to silty loam; 4) very loose to medium dense sand to gravelly sand.

#### (1) Man-made ground (fill)

Borings drilled through the roadway embankment encountered up to 12-foot thick granular and cohesive fill. The fill, likely made up of local borrow materials, closely resembles the surrounding natural soils. The granular fill consists of loose to very dense, black and brown gravelly sand, sand, and loam and has SPT N-values of 3 to more than 50 blows/foot and moisture content (MC) values of 1 to 17%. The cohesive fill is made up of medium stiff to very stiff, brown silty clay loam to clay loam and has unconfined compressive strength (Q<sub>u</sub>) values of 0.5 to 2.0 tsf and MC values of 9 to 33%.

#### (2) Very loose to loose sandy loam to sand

Below the fill, borings advanced through alternating layers of sands, silts, and clays. Up to 7.5-foot thick very loose to loose sandy loam to sand with N-values of 1 to 4 blows/foot and MC values or 4 to 25% was encountered. Within the sand layer, lenses of soft silty loam with Qu values of 0.4 to 0.5 tsf and MC values of 31 to 33% are present. According to the AASHTO soil classification, this soil belongs to A-4 and A-2-4 groups. Boring 52-SGB-12, drilled at the toe of the existing embankment, encountered 5 feet of soft organic clay with Qu values of 0.25 tsf and MC values of 52 to 57%.

#### (3) Very soft to stiff silty clay loam to silty loam

At elevations ranging from 583.3 to 592.9 feet, borings encountered up to 16.3-foot thick of very soft to stiff silty clay loam to silty loam with Qu values of 0.16 to 1.15 tsf with an



average of 0.6 tsf and MC values of 20 to 48% with an average of 29%. Liquid limit ( $L_L$ ) values range from 33 to 42%, and plastic limit ( $P_L$ ) values range from 19 to 21%. According to the AASHTO soil classification, the soils belong to the A-4, A-6 and A-7-6 groups.

#### (4) Very loose to medium dense sand to gravelly sand

At elevations ranging from 570.5 to 579.7 feet, borings advanced in more than 95-foot thick very loose to medium dense sand to gravelly sand with N-values of 1 to 20 blows/foot. According to the AASHTO soil classification, the soils belong to the A-3 and A-1-b groups.

#### 4.3 Groundwater Conditions

During drilling, groundwater was encountered between 0.0 and 23.0 feet bgs. At the drilling completion, water level as measured at 0.0 to 22.0 feet bgs. Mississippi River water level at the time of drilling was recorded at 582.45 feet elevation.

#### 5.0 Existing Causeway Construction and Condition

There are no plans available for the original construction of the causeway. As per Iowa DOT reports, it appears that the causeway was this area was a land with trees when it was constructed in 1932. The existing dams were not in place on the Mississippi River so the area was not flooded as it is today.. After removal of trees, soil (basically sand) from each side of the causeway was excavated and piled along the centerline of the causeway. After the soil was piled, bull dozers levelled off the top of the piled soil creating driving surface and side slopes. It appears that this created embankments in different conditions within the central portion and outside the central portion on each side of the roadway. No other information is available such as placement in layers, compaction and period of time soil pile was left in place before levelling off by bulldozer.

Iowa DOT personnel performed field inspection to determine condition of the causeway in November 2012, and in January and March 2014. Longitudinal cracks were observed along most of the roadway. These cracks appear to be at the third points of the original soil pile the way embankment was graded. Iowa DOT reports describe condition of side slopes and pavement condition. Iowa DOT found the integrity of the causeway to be in good condition.



#### 6.0 ANALYSES AND RECOMMENDATIONS

Based on the project information provided by Parsons and the subsurface soil conditions revealed during investigations, recommendations are presented in the following sections for the design and construction of the proposed improvements. The results of the field and laboratory data were utilized to evaluate the engineering properties of the soils encountered during site investigation.

US 52 Iowa section roadway will be reconstructed from Station 1546+00 to Station 1560+65 and supported by adding new fill to the existing embankment, creating 1:2 (V:H) slopes. Most of the widening will be on the south side of the existing embankment.

We considered field observations and conclusions mentioned in Iowa reports in our analysis and recommendations. We agree that the existing subgrade is in varying soil conditions with low bearing capacity and proper surface drainage should be provided along with proper side slope treatment to deter undermining of sandy subsurface soil. We also agree that after reconstruction of roadway, inspection and maintenance such as pavement cracks sealing on a regular basis should be performed.

#### **6.1 Existing Pavement Stripping**

Prior to embankment construction, exiting pavement should be stripped. Record plans for this location indicate that the existing pavement was constructed using HMA materials. No pavement cores were performed. However, based on borings measurements, for quantity estimating purposes, the average pavement thickness to be stripped is seven (7) inches. The actual depth of stripping should be determined in the field during construction. The roadway was last paved in 1994 as per Iowa report dated November 14, 2012.

#### **6.2** Subgrade Preparation and Treatment

All vegetation, pavement, debris and riprap should be cleared and stripped from the existing roadway construction areas and where embankment fills are to be placed. After stripping of pavement, to provide stable subgrade and to increase bearing capacity two options can be considered. Following stripping of exiting pavement and excavating to the proposed subgrade, 2 feet of existing embankment material can be removed and replaced with compacted fill as per Iowa DOT embankment specifications requirements. Another option is to place Geogrid after stripping



existing pavement and excavating to the proposed subgrade. We recommend providing Geogrid considering required earthwork, construction and cost. We recommend TriAx TX5 Geogrid manufactured by Tensar or equivalent. With the inclusion of the Geogrid at the proposed subgrade level, it will provide uniformity in subgrade strength. The addition of the Geogrid will also improve the performance of the pavement section. The exposed subgrade should be proofrolled and observed under construction equipment. Proofrolling aids in providing a firm base for compaction of new fill and identifying soft or unstable soil conditions that may exist at or near the exposed subgrade level. It is recommended that the proofrolling be accomplished with a fully-loaded, 25-ton dump truck or other equipment providing an equivalent loading such as heavy vibratory roller. A minimum of 4 passes should be made over the entire subgrade areas in both the direction. In confined areas and in areas where a dump truck or a roller cannot be used effectively, such as at edges of the excavation bottom, proofrolling/compaction may be performed with hand-operated heavy vibratory equipment with enough number of passes so that unstable subgrade can be detected. The exposed subgrade should be observed for the amount of deflection or rutting.

Remedial work for unstable subgrade should consist of disking, aerating, and recompacting exposed subgrade soils. Solutions to a persistent pumping problem may include removal of unsuitable soils and replacement with granular fill, use of a subgrade stabilization material or a combination thereof. Removal and replacements should extend to at least one foot beyond outside edge of new shoulders. Subgrade stabilization material needed should consist of Geogrid mentioned previously.

The stability of prepared roadway subgrade will depend upon such factors as surface drainage provided by the contractor as well as prevailing temperature and precipitation experienced during construction. The amount of construction traffic and subgrade disturbance created by heavy vehicles will also have an influence on subgrade stability. The contractor should try to make full use of ditches in order to maintain positive drainage for subgrade areas. Temporary drainage ditches or pumping from depressed areas should be provided as needed during construction in order to prevent ponded water from affecting the stability of the roadway.



#### 6.3 Subgrade Support Rating

Based on the soil types and conditions encountered in boring, we recommend an estimated California Bearing Ratio (CBR) value of 2 and Resilient Modulus of Subgrade ( $M_{RSG}$ ) value of 3,000 psi for the pavement design purposes.

#### 6.4 Pavement Design

Wang understand that the new pavement structure based on 20-year pavement design will be as follow for the traffic lanes:

- 1.5-inch thick Surface Hot Mix Asphalt (HMA)
- 1.5-inch thick Intermediate HMA
- 6.5-inch thick Base HMA
- 12-inch thick Iowa Modified Subbase

For HMA shoulders, a thickness of 8 inches will be used on 12-inch thick aggregate subbase.

#### 6.5 Drainage Considerations

The proposed subgrade and pavement should have proper surface grading to remove water accumulations and prevent the pooling of water. Since the surface drainage will not be provided by an enclosed system consisting of curb and gutter, we do not see any need for transverse or longitudinal underdrain. We recommend the granular subbase be daylighted to the outside of the embankment to provide proper drainage. If the subbase is not daylighted, a combination of longitudinal and transverse underdrains should be considered within the subgrade.

#### 6.6 Embankment Material and Stability

The material to be used for the roadway embankment construction should be in accordance with Sections 2102 and 2107 the Iowa DOT Standard Specifications. A shrinkage factor of 15% should be used to measure the borrowed and furnished excavation quantities.

The computer program, SLIDE Version 5.0, was used to calculate the factors of safety against global slope stability. The Simplified Bishop Method was used for slope stability analyses. Details of stability analysis with the critical failure surface and results are shown in Appendix D.



Slope stability analyses were performed for the embankment side slopes at sections where soil borings were performed at the top and at the toe of existing embankment. These sections are at Stations 1554+50, 11557+50 and 1559+50. The estimated soil parameters considered and results of stability analysis are shown in appendix D. The calculated minimum factor of safety (FOS) is greater than minimum required by Illinois DOT and Iowa DOT. The minimum FOS required by Illinois DOT is 1.5 and by Iowa DOT is 1.3 for embankment slopes.

The slopes will require permanent protection to prevent erosion due to precipitation and storm water runoff. A layer of riprap designed similar to end slope protection at the bridge west abutment can be used from the paved shoulder to the bottom of the embankment at all locations.

#### 6.7 Embankment Settlement

Settlement of the embankment will occur due to consolidation of the foundation soils and within the new embankment mass. The immediate (elastic) settlement is expected to occur from the underlying granular soil layers.

We performed settlement evaluations for the new embankment at Station 1560+00 where embankment thickness is estimated to be approximately 20 feet. A computer program FoSSA v2.0 (Foundation Stress and Settlement Analysis) was used for assessing stresses and settlements under embankment. Soil parameters required for elastic settlement evaluation were estimated from the borings and other published data.

We estimate initial settlement on the order of 3.2 inches after placement of riprap to one foot above normal river level (normal river elevation 582.3 feet). We estimate additional settlement on the order of 2.8 inches after constructing embankment from top of riprap to the proposed subgrade level. Most of the elastic settlement in the granular soils is expected to be occurring at the same rate as construction. Settlement from within the cohesive embankment is expected to be on the order of 1.5 inches.



#### 7.0 CONSTRUCTION CONSIDERATIONS

#### 7.1 Excavation

Temporary excavations should have a slope as required to provide a stable side slopes. Excavation slopes should be sloped at no greater than 1:2 (V:H). All temporary cut excavation should be analyzed individually for excavation of more than 4 feet deep or slope steeper than 1:2 (V:H). Excavations should be performed in accordance with local, state, and federal regulations. The potential effect of ground movements upon open roadway and utilities should also be taken into consideration.

#### 7.2 Reuse of Excavated Materials

Excavated riprap materials free of miscellaneous debris can be reused below river level. Reuse of existing riprap should be approved by the Engineer. Excavated soil material from IL 84 retaining wall is expected to be suitable for reuse as embankment fill.

#### 7.3 Dewatering

The near surface granular soils may exhibit perched groundwater conditions. These layers may be intercepted during cut slope excavation; however, the water flow will likely be temporary and could be controlled and diverted away from the proposed subgrade area or pumped out. Seepage water that does accumulate in open excavations above groundwater level can be removed using the sump pump method. Surface runoff and ditches should be directed away from the excavation to facilitate dewatering operations.

#### 7.4 Embankment Construction

Embankment should be constructed as early as possible in the project construction period in order to allow the embankments to adjust or settle under its own weight as much as possible prior to pavement construction. We recommend delaying pavement construction for at least two weeks after completion of embankment to the proposed subgrade level.

Most of the roadway widening will be over the exiting embankment slope and river bed. We recommend removing exiting debris, vegetation and riprap from the existing embankment without destabilizing the slopes before placement of new fill. Riprap should be placed from the bottom of river bed to one foot above normal river level. The exiting salvaged clean riprap can



be reused. A 12-inch thick capping layer of aggregate meeting the requirements for Gradation No. 4 of Iowa DOT Standard Specification should be placed on top of riprap. On top of capping layer a geotextile fabric meeting the requirements of Article 4196.01.B.5 should be placed. Embankment soil material can be place after placement of fabric. Embankment should be constructed in accordance with Section 2107 *Embankment* of Iowa Standard Specifications.

The new fill should be benched into the side slopes of the existing embankment to provide interlocking between the old and new fill. During benching, any water seeped from the existing embankment should be removed or placement of new fill should be postponed until the area is dry enough. We recommend benching the slopes according to Article 2107.01.C.2 and placement according to Article 2127.03.B of Iowa DOT Standard Specifications.

Maintenance of the slope during the construction will be required for localized areas of cut slopes where erosion prone soils (silt and sand) are encountered. These soils will develop minor sloughing however; major sloughing may develop if these soils are saturated with perched groundwater. These conditions should be observed during construction and corrective measure should be taken. Heavy construction equipment and material should not be placed near the top of the slope, unless the stability of slope is acceptable.

We recommend showing the following or similar note on the plan.

"Proper construction including quality control and inspection during construction is vital for the overall stability of the embankment even for a short time period. The failure of widened embankment may result due to sub-standard compaction of fill, inadequate benching into the existing embankment and no proper drainage control. Earthwork procedures should include provisions to minimize soil disturbance and exposure. Earth moving operations should be scheduled not to coincide with excessive cold or wet weather (early spring, late fall or winter). Wet sand exposed to cold weather should be protected from freezing. Any soil allowed to freeze or soften due to the standing water should be removed from the subgrade. Wet weather can cause problems with subgrade compaction."

#### 7.5 Earthwork Operations

The required earthwork can be accomplished with conventional construction equipment. Precautions should be taken by the Contractor to prevent water erosion of the exposed subgrade. A compacted subgrade will minimize water runoff erosion. Silt and fine sands are sensitive to



mechanical disturbance such as traffic and construction crew and will cause deterioration of exposed subgrade soils.

#### 7.6 Construction Monitoring

There is no need for a special construction monitoring for the earthwork except normally required by the Iowa DOT Standard Specifications, Special Provisions and Contract Plans.

During construction, an experienced geotechnical engineer or soil technician should be retained to perform the following tasks:

- Monitor earthwork operations;
- Evaluate the suitability of the soils for subgrade support and embankment construction;
- Observe excavation including cut slopes;
- Check soil materials and compaction for project specifications compliance;
- Monitor locations and depths of undercuts; and
- Advise the IDOT/Iowa DOT/Resident Engineer of any conditions not apparent during the subsurface exploration.

US 52 Iowa Section, US 52 over Mississippi River Jackson County, Iowa Roadway Geotechnical Report August 20, 2014



#### 8.0 QUALIFICATIONS

The analysis and recommendations submitted in this report are based upon the data obtained from borings by Wang during previous and current investigations. This report does not reflect any variations that may occur between the borings or elsewhere on the site, variations whose nature and extent may not become evident until the course of construction. In the event that any changes in the design are planned, we should be timely informed so that changes can be reviewed, modified, and approved in writing by the geotechnical engineer.

It has been a pleasure to assist Parsons Transportation Group and Illinois Department of Transportation on this project. Please call if there are any questions, or if we can be of further service.

Respectfully Submitted,

WANG ENGINEERING, INC.

Mohammed (Mike) Kothawala, P.E., D.GE Sr. Project Manager/Sr. Geotechnical Engineer

Jerry W.H. Wang, Ph.D., P.E. Principal

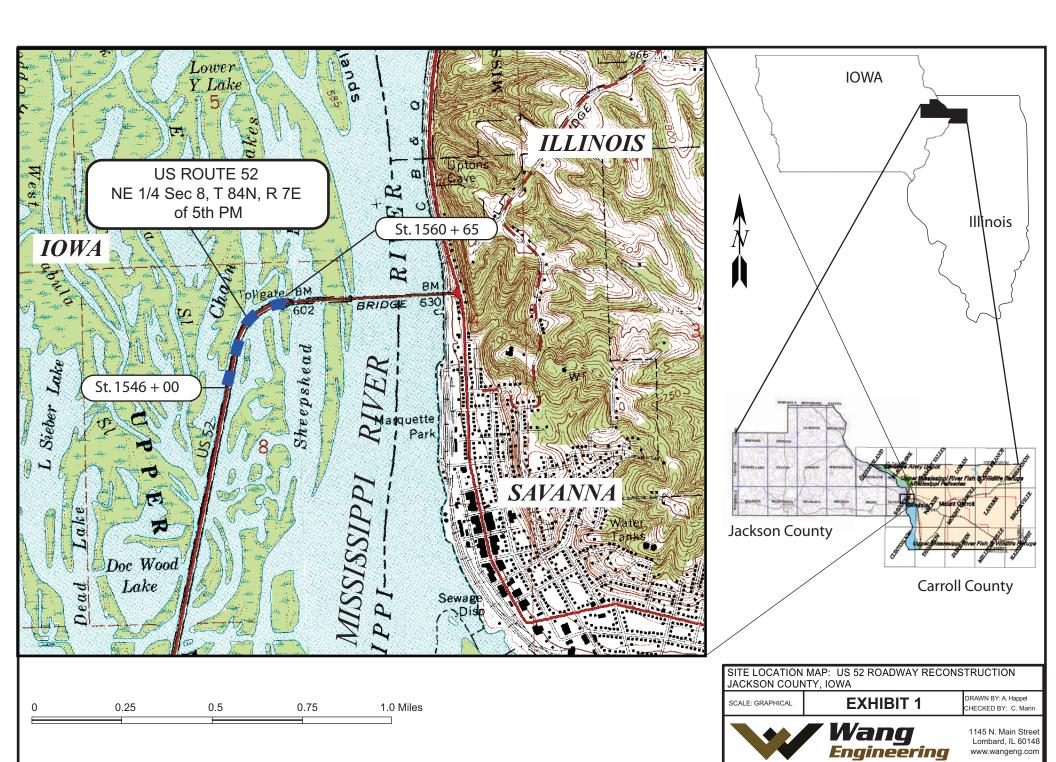


#### REFERENCES

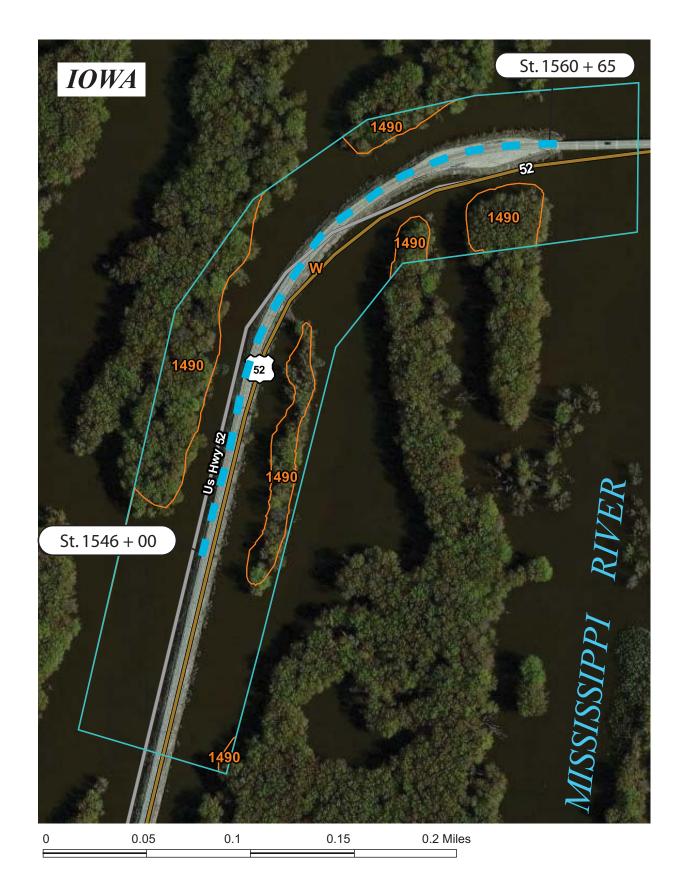
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# **EXHIBITS**



FOR PARSONS 342-06-01



# *ILLINOIS*



Map unit symbol and soil name	Depth	USDA texture	Classification  AASHTO	Sand	Silt	Clay	Moist bulk density	Saturated hydraulic conductivity	limit	Plasticity index	Organic matter
	In			%	%	%	g/cc	micro m/sec	%		%
1490—Caneek silt loam, channeled, 0 to 2											
Caneek, frequently	0-28	Silt loam	A-4	-15-	-64-	18-21- 24	1.20-1.30	1.00-10.00	25-35	5-10	0.7-1.7
flooded, channeled	28-60	Silty clay loam, silt loam	A-6, A-7	-10-	-68-	18-23- 28	1.25-1.40	1.00-10.00	35-45	10-20	0.0-0.5

Map symbol and	Potential as a source of roadfill	Potential as a source of topsoil
soil name	Rating class and limiting features	Rating class and limiting features
1490— Caneek silt loa	nm, channeled, 0 to 2 percent sk	opes
Caneek, frequently flooded, channeled	Poor, Wetness, Low strength, Dusty, Shrink-swell	Poor, Wetness

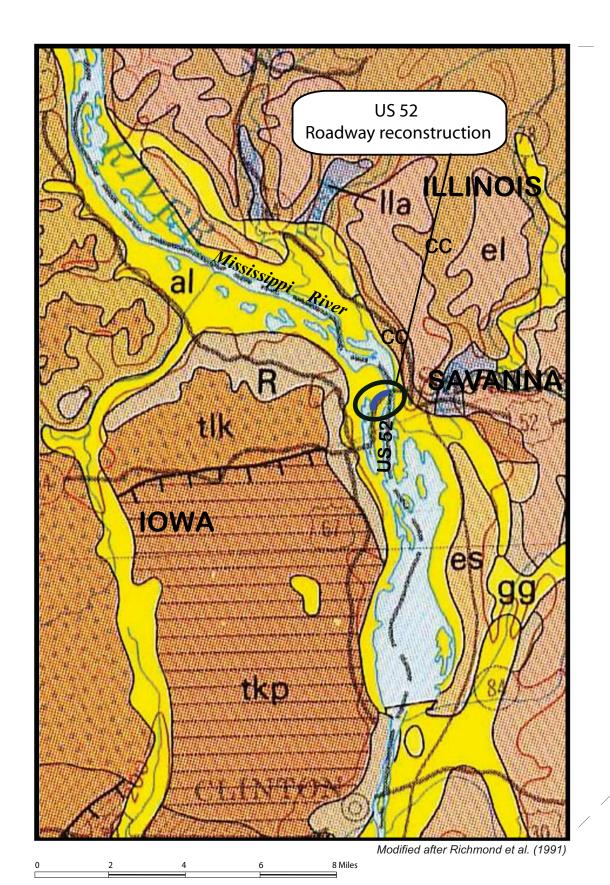


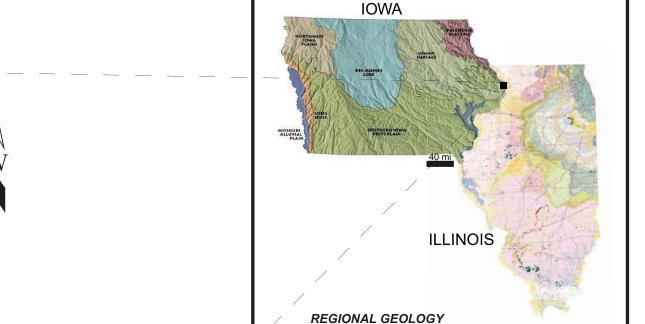


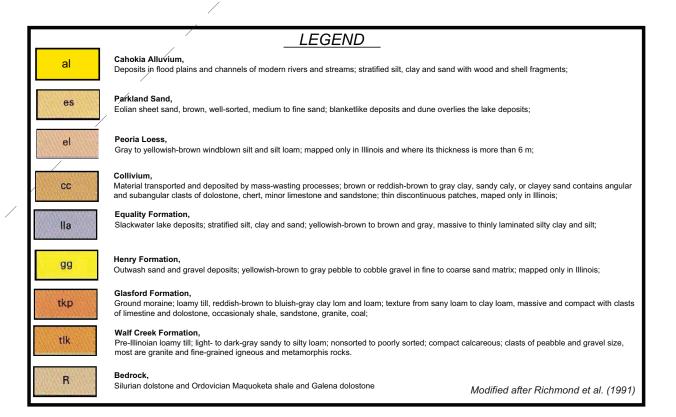
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FOR PARSONS

342-06-01











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342-06-01

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# **APPENDIX A**



### **LEGEND FOR BORING LOG**

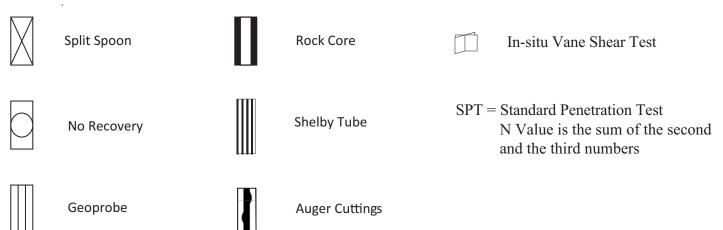
Relative Density of Non- Cohesive Soils										
N-Blows/ 12 inches	Relative Density Term									
0-3	Very Loose									
4-9	Loose									
10-29	Medium Dense									
30-49	Dense									
50-80+	Very Dense									

Consistency of Cohesive Soils											
Unconfined Compressive Strength Qu, tsf	Consistency Term										
<0.25	Very Soft										
0.25-0.49	Soft										
0.50-0.99	Medium Stiff										
1.00-1.99	Stiff										
2.00-3.99	Very Stiff										
>4.00	Hard										

Propo	rtional T	erms						
Trace	1-9	Pe						
Little	10-19	ercent Dry Weigh						
Some	20-34	gh spat						
And	35-50	of of						
Gradatio	on Termi	inology						
Boulders	>200mm							
Cobbles	200mm to 75mm							
Gravel	75mm	to 2mm						
Sand	2-0m	ım to						
Sanu	0.07	4mm						
Silt	0.074							
Clay	<0.00	)2mm						

SS	= Split Spoon
ST	= Shelby Tube
SPT	= Standard Penetration Test
Qu	= Unconfined Compressive Strength
	P = Pocket Penetrometer
	S = Shear failure of sample, Rimac test
	B = Bulge failure of sample, Rimac test
SSA	= Solid Stem Augers,
HSA	= Hollow Stem Augers,

# **Sample Type Symbols**





# **BORING LOG 52-SGB-01**

WEI Job No.: 342-06-01

Client PARSONS

Project US 52 / IL 64 Over the Mississippi River
Location Carroll County, IL and Jackson County, IA

Datum: NAVD 88 Elevation: 597.40 ft North: 1979606.33 ft East: 2295584.75 ft Station: 1546+94.20 Offset: 5.61 LT

Profile	SOIL AND ROCK do DESCRIPTION	Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCI	0	Sample Type	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	
	Black and brown SILTY LOAM, trace gravelFILL 596.87-inch thick, ASPHALTPAVEMENTPAVEMENTPAVEMENTPAVEMENTPAVEMENTPAVEMENTPAVEMENTPAVEMENTPAVEMENTPAVEMENT		1	6 5 5 5	1.48 B	19				- - - - -		12 1 2 4	NP	17	
	gravel 592.9FILL Loose, brown, medium SAND		2	3 4 4 4	NP	5		567.4		30		13 2 4 6	NP	22	
	Soft, gray SILTY CLAY, trace gravelFILL		3	1 1 2 1	0.25 P	25		Во	ring terminated at 30.00	• ft _ _ _ _					
			4	1 1 1 3	0.41 B	28				- - -					
	Very loose, brown, medium 10 SAND		5	1 1 2	NP	7				35 <u> </u>					
	Soft to medium stiff, gray SILTY CLAY LOAM to SILTY LOAM, trace gravel		6	2 2 2	0.25 P	26				- - - -					
	15	- <b>I</b>	7	1 1 1	0.79 B	38				40					
		¥	8	1 2 2	0.51 B	27				- - -					
	L <sub>L</sub> (%)=34, P <sub>L</sub> (%)=20 %Gravel=0.1 %Sand=17.320 %Silt=61.1		9	P U S H	0.37 S	28				- - 45_ -					
	%Clay=21.5 A-6 (11)/ Very loose to medium dense, gray, medium SAND		10	0 1 1	NP	25				- - -					
Beg Drill Drill	25	5	11	1 2 3	NP	15				50					
	GENERAL	NOT	ĖS				_		WATE	R LEVE	L DA	TA			
Beg	Begin Drilling 11-12-2013 Complete Drilling 11-12-2013								While Drilling	<u> </u>		1.00 ft			
Drilling Contractor Wang Testing Services Drill Rig D-50 TMR								At Completion of Drilling	¥	1	7.00 ft				
Driller R&N Logger A. Tomaras Checked by C. Marin  Drilling Method 3.25" HSA, boring backfilled upon completion									Time After Drilling NA						
Urill	ing Method 3.25" HSA, boring ba	CKTIII	ed L	ipon .	com	pieti	on		Depth to Water  The stratification lines represent the party of the pa	NA esent the appropriate report to the contract of the contrac	roximat	e bounda	ary		
		between soil types; the actu	<u>ual transition r</u>	nay be	<u>gradual.</u>										



# **BORING LOG 52-SGB-02**

WEI Job No.: 342-06-01

Client PARSONS

Project US 52 / IL 64 Over the Mississippi River
Location Carroll County, IL and Jackson County, IA

Datum: NAVD 88 Elevation: 597.26 ft North: 1979895.00 ft East: 2295653.49 ft Station: 1549+90.80 Offset: 5.28 LT

		- I									-				
Profile	SOIL AND ROCK (t) DESCRIPTION	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
11000 0000	596.77-inch thick, ASPHALTPAVEMENT  595.5Black and brown GRAVELLY 594\9SANDY LOAM								ose to medium dense, graedium SAND, trace gravel	y, _		11	3 2	NP	21
	Stiff, brown SILTY CLAYFILL/		1	8 13 9 6	NP	1				- - -	/ \		3		
	Stiff, brown and gray SILTY CLAY LOAM, trace gravel		2	2 2 3 3	1.23 B	26	2			30		12	1 3 6	NP	16
	FILL 589.0		3	2 3 3 4	1.50 P	33				- - -					
	Very loose to loose, brown, medium SAND, trace gravelFILL		4	3 2 2 3	NP	11		562.3	ring terminated at 35.00 fl	- - 35		13	6 5 6	NP	16
			5	3 2 1	NP	4		BU	ning terminated at 33.00 II	- - - -					
	583.3 Soft, gray SILTY CLAY LOAM, trace organic matter		6	2 1 1	NP	16				- - - 40_					
	L <sub>L</sub> (%)=42, P <sub>L</sub> (%)=21 %Gravel=0.0 %Sand=6.0 %Silt=70.6		7	P U S H	0.77 B	31				- - - - -					
	%Clay=23.3 A-7-6 (21) 20_		8	0 1 2	0.25 P	24				- - - 45_					
11 //10/14	Very loose, SANDY LOAM, trace organic matter		9	1 0 1	NP	23				- - - -					
WANGENGING 34Z0601.GFD WANGENG.GDI 7/10/14  Du Du Du Du	25_		10	0 1 2	NP	23				- - 50_					
5	GENERAL NOTES								WATER	LEVE	L D	ΑT	Α		
Be	Begin Drilling 11-12-2013 Complete Drilling 11-12-2013								While Drilling	<u> </u>			00 ft		
Drilling Contractor Wang Testing Services Drill Rig D-50 TMR								At Completion of Drilling	<u> </u>		22.0	00 ft			
Driller R&N Logger A. Tomaras Checked by C. Marin								Time After Drilling	NA						
Dr	illing Method 3.25" HSA, boring back	fille	d u	pon	com	pletio	on		Depth to Water  The stratification lines repres	NA ent the app	roxim	ate b	oundan	/	
<b> </b>		between soil types; the actual	transition i	may b	e gra	idual.	,								



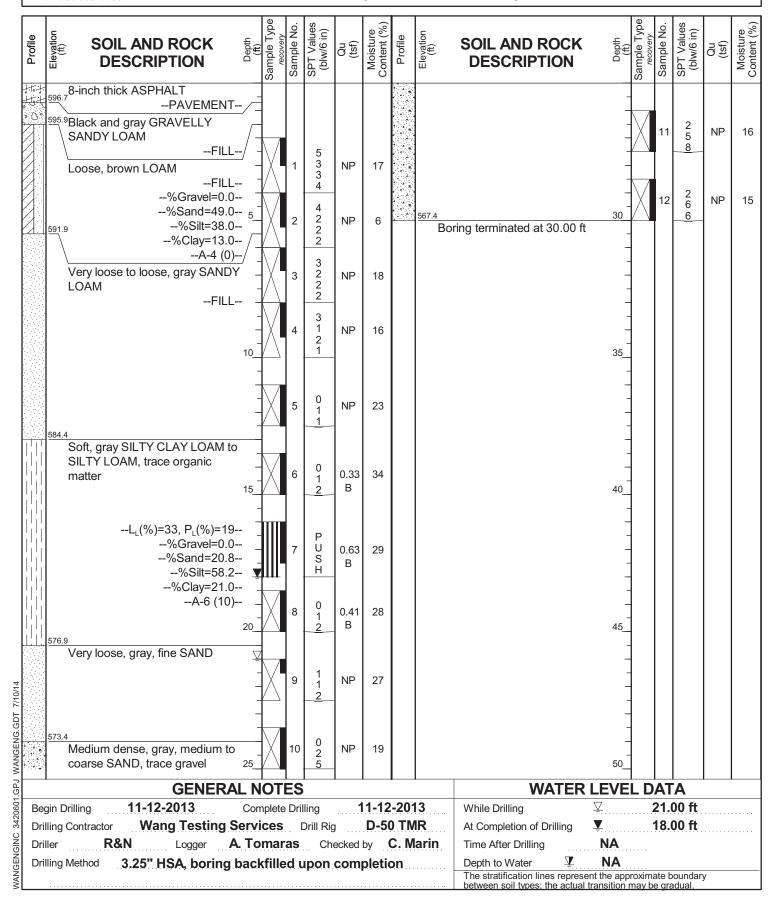
# **BORING LOG 52-SGB-03**

WEI Job No.: 342-06-01

Client PARSONS

Project US 52 / IL 64 Over the Mississippi River
Location Carroll County, IL and Jackson County, IA

Datum: NAVD 88 Elevation: 597.40 ft North: 1980117.59 ft East: 2295766.60 ft Station: 1552+41.46 Offset: 5.56 RT





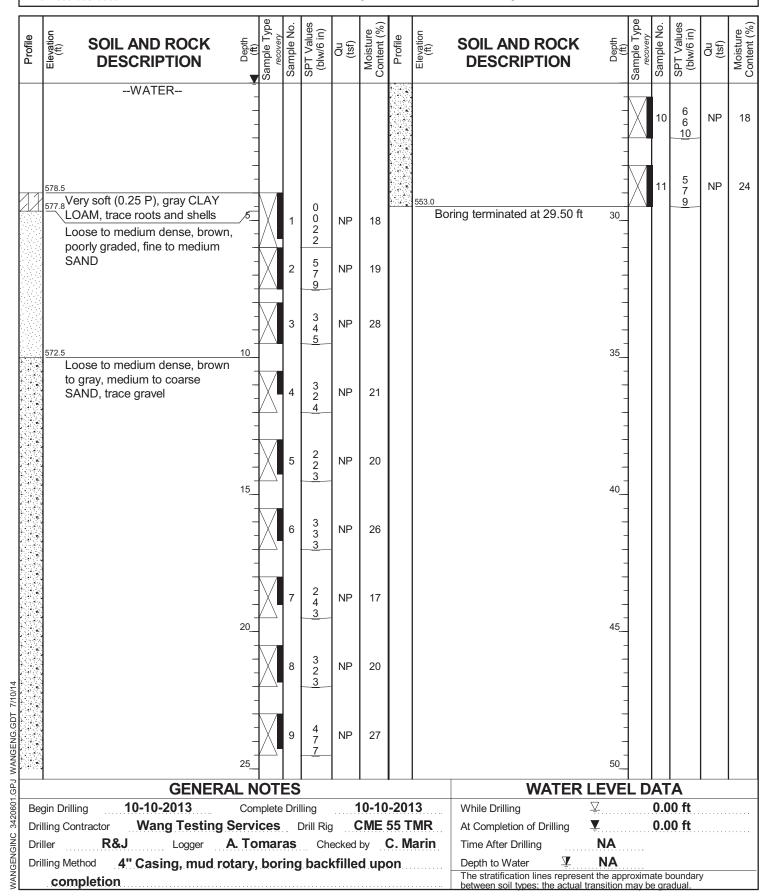
# **BORING LOG 52-SGB-04**

WEI Job No.: 342-06-01

Client PARSONS

Project US 52 / IL 64 Over the Mississippi River
Location Carroll County, IL and Jackson County, IA

Datum: NAVD 88 Elevation: 582.45 ft North: 1980089.00 ft East: 2295819.66 ft Station: 1552+48.06 Offset: 65.50 RT





# **BORING LOG 52-SGB-05**

WEI Job No.: 342-06-01

Client PARSONS

Project US 52 / IL 64 Over the Mississippi River
Location Carroll County, IL and Jackson County, IA

Datum: NAVD 88 Elevation: 597.83 ft North: 1980208.96 ft East: 2295820.81 ft Station: 1553+46.72 Offset: 8.15 LT

0	E COU AND DOOK	Type	S.	lues in)		lre (%)	Φ	-G	COUL AND DOOK	ــــــــــــــــــــــــــــــــــــــ	Type	No.	lues in)		ure (%)
Profile	SOIL AND ROCK DESCRIPTION	(ft) Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
. O.	597.45-inch thick ASPHALT PAVEMENT	7								-					
, O (	Brown GRAVELLY SANDY 595.8LOAM									_		11	1	NP	17
	FILL/	1	1	7 6 5	1.56	13				-	/ \		_4		
	littile gravel	A		5 4	В					-		12	2	NP	17
	Very loose, brown and gray 5 SANDY LOAMFILL	5-	2	3 1 2 2	NP	20				30 <u> </u>		12	4 4	INF	17
			3	2 1 1	NP	26				- - -					
			4	1 1	NP	23				-			3		
	10			1				562.8	wing towningted at 25 00 ft	35 35	Ă,	13	3 7 11	NP	16
	Medium stiff, gray SILTY CLAY LOAM							ВО	ring terminated at 35.00 ft	-					
	FILL		5	1 1 2	0.66 B	26				- - -					
	Soft to stiff, gray SILTY CLAY LOAM, trace to little organic	┧,,,,,								-					
	matterL <sub>L</sub> (%)=39, P <sub>L</sub> (%)=2015%Gravel=0.0	- 	6	P U S H	0.81 B	29				40 <u> </u>					
	%Sand=13.3 %Silt=60.4			1						_					
	%Clay=26.3 A-6 (17)		7	1 2	0.66 B	31				_					
				0						-					
	20		8	2 3	1.15 B	30				45 <u> </u>					
		+								-					
7/10/14			9	1 2 2	0.49 B	30				-					
G.GDT	Loose to medium dense, gray, madium SAND, trace gravel	¥								-					
WANGENGINC 3420601.GPJ WANGENG.GDT 7/10/14	Thadidin GAND, trace graver	<u> </u>	10	0 2 4	NP	23				50					
,GPJ v	GENERAL	WATER I	EVE	L D	AT	Α									
Be	gin Drilling 11-13-2013 Co		<u> </u>			00 ft									
β Dr S Dr	Drilling Contractor Wang Testing Services Drill Rig D-50 TMR  Driller R&N Logger A. Tomaras Checked by C. Marin								At Completion of Drilling Time After Drilling	<b>▼</b> NA	2	20.0	00 ft		
Dr GENG	illing Method 3.25" HSA, boring back	Depth to Water	NA												
XAN NAN	·	The stratification lines represer between soil types; the actual tr	t the appl ansition r	roxima nay be	ate b	oundar dual.	/								



# **BORING LOG 52-SGB-06**

WEI Job No.: 342-06-01

**PARSONS** Client

Project US 52 / IL 64 Over the Mississippi River Carroll County, IL and Jackson County, IA Location

Datum: NAVD 88 Elevation: 596.81 ft North: 1980270.12 ft East: 2295897.22 ft Station: 1554+43.77 Offset: 1.28 RT

between soil types; the actual transition may be gradual

SPT Values (blw/6 in) SPT Values (blw/6 in) Moisture Content (%) Sample Type Sample No Sample No Moisture Content (% Elevation (ft) Elevation (ft) Profile Profile **SOIL AND ROCK** SOIL AND ROCK Qu (tsf) Sample <sup>-</sup> Qu (tsf) **DESCRIPTION** DESCRIPTION 596.54-inch thick ASPHALT --PAVEMENT-Very loose to medium dense, gray, medium SAND 595.1Brown GRAVELLY SANDY NP 21 0 LOAM 5 6 6 2.00 13 Stiff to very stiff, brown and gray CLAY LOAM, little gravel --FILL--NΡ 12 19 2 2 3 Loose, brown SAND 1.00 25 --FILL--<u>591</u>.3 Soft to medium stiff, gray SILTY LOAM, trace organic matter --FILL--0.41 31 2 --%Gravel=0.0----%Sand=26.4----%Silt=59.2--U 0.50 33 10 6 NΡ 13 16 --%Clay=14.4--S --A-4 (0)--10 Boring terminated at 35.00 ft 586.3 Very loose, gray, medium SAND 5 NP 25 0 Very soft to medium stiff, gray SILTY CLAY LOAM, trace to little 6 0.49 21 organic matter 0.82 35 В 0.90 30 2 0.16 29 3420601.GPJ WANGENG.GDT 7/10/14 В 0.90 48 **WATER LEVEL DATA GENERAL NOTES** 11.00 ft 11-13-2013 11-13-2013 Begin Drilling Complete Drilling While Drilling 22.00 ft Wang Testing Services Drill Rig **D-50 TMR Drilling Contractor** At Completion of Drilling WANGENGINC Logger A. Tomaras Checked by **C. Marin** Time After Drilling NA Driller **Drilling Method** 3.25" HSA, boring backfilled upon completion Depth to Water The stratification lines represent the approximate boundary



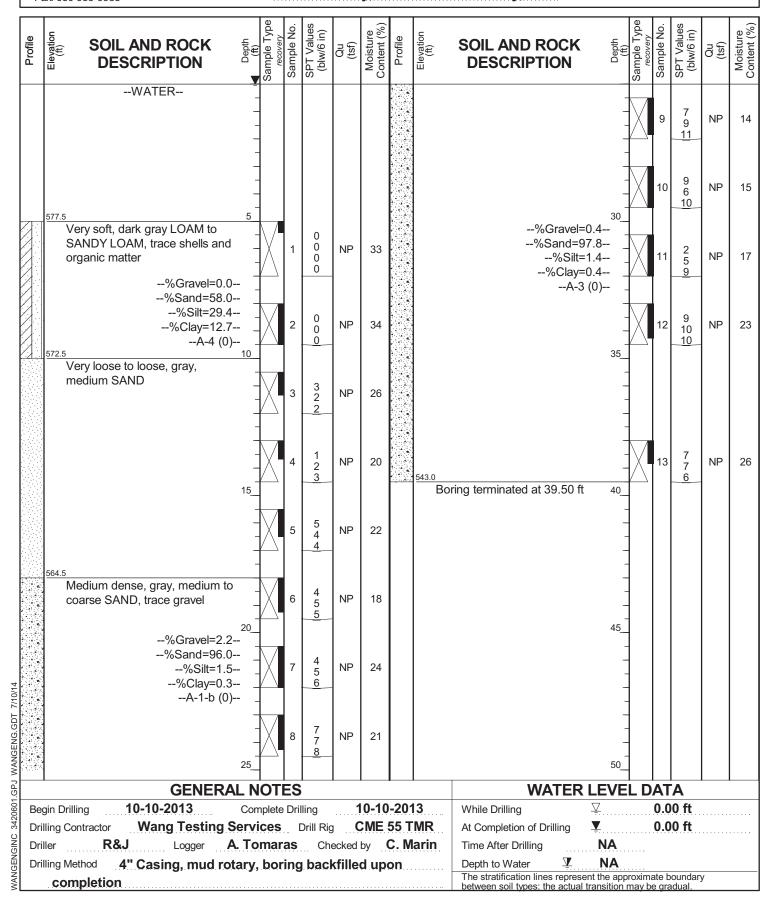
# **BORING LOG 52-SGB-07**

WEI Job No.: 342-06-01

Client PARSONS

Project US 52 / IL 64 Over the Mississippi River
Location Carroll County, IL and Jackson County, IA

Datum: NAVD 88 Elevation: 582.45 ft North: 1980226.38 ft East: 2295936.49 ft Station: 1554+45.35 Offset: 60.04 RT





# **BORING LOG 52-SGB-08**

WEI Job No.: 342-06-01

Client PARSONS

Project US 52 / IL 64 Over the Mississippi River
Location Carroll County, IL and Jackson County, IA

Datum: NAVD 88 Elevation: 597.66 ft North: 1980430.99 ft East: 2296160.14 ft Station: 1557+51.28 Offset: 13.66 LT

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.

SPT Values (blw/6 in) SPT Values (blw/6 in) Moisture Content (%) Sample Type Sample No Sample No Moisture Content (% Elevation (ft) Elevation (ft) Profile Profile **SOIL AND ROCK** SOIL AND ROCK Qu (tsf) Sample <sup>-</sup> Qu (tsf) **DESCRIPTION DESCRIPTION** 597.25-inch thick, ASPHALT --PAVEMENT-**Brown GRAVELLY SANDY** NP 5 12 NP 17 5 LOAM 3 --FILL --Medium stiff, brown CLAY 0.50 21 LOAM, trace gravel Ρ --%Gravel=0.1--3 5 --FILL--NΡ --%Sand=97.2--13 20 Very loose, brown SAND --%Silt=2.3--30 NP 10 --FILL----%Clay=0.4--2 <sub>591.4</sub>Medium stiff (0.75P), brown --A-3 (0)--CLAY LOAM --FILL--2 NP 6 Very loose, brown SAND --FILL--2 2 NP 6 4 5 NΡ 22 --%Gravel=0.1--10 --%Sand=88.5----%Silt=9.1--U NP 14 --%Clay=2.3--S --A-2-4 (0)--Soft to medium stiff, gray SILTY CLAY, trace organics --Silt seams--5 7 15 NP 20 0.66 26 В Boring terminated at 40.00 ft 0.41 28 В Loose to medium dense, brown, medium SAND, trace gravel NP 21 2 20 10 NP 22 2 3420601.GPJ WANGENG.GDT 7/10/14 NP 21 WATER LEVEL DATA **GENERAL NOTES** 18.50 ft 11-14-2013 11-14-2013 Begin Drilling Complete Drilling While Drilling 8.00 ft Wang Testing Services Drill Rig **D-50 TMR Drilling Contractor** At Completion of Drilling Logger A. Tomaras Checked by **C. Marin** Time After Drilling NA Driller **Drilling Method** 3.25" HSA, boring backfilled upon completion Depth to Water



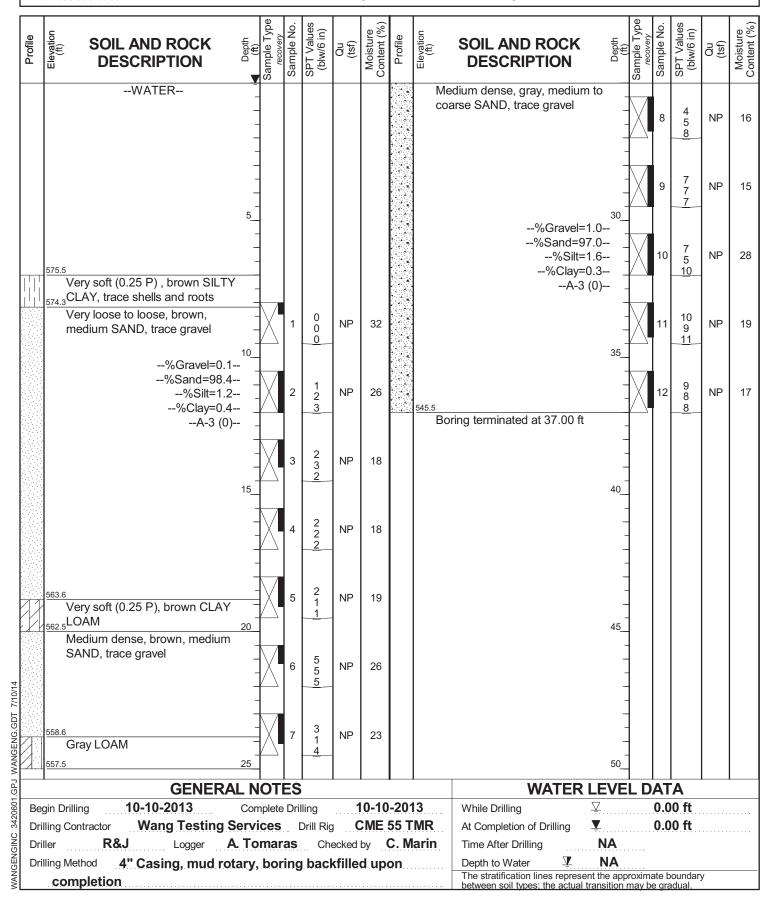
### **BORING LOG 52-SGB-09**

WEI Job No.: 342-06-01

Client PARSONS

Project US 52 / IL 64 Over the Mississippi River
Location Carroll County, IL and Jackson County, IA

Datum: NAVD 88 Elevation: 582.45 ft North: 1980369.08 ft East: 2296188.70 ft Station: 1557+61.19 Offset: 53.83 RT





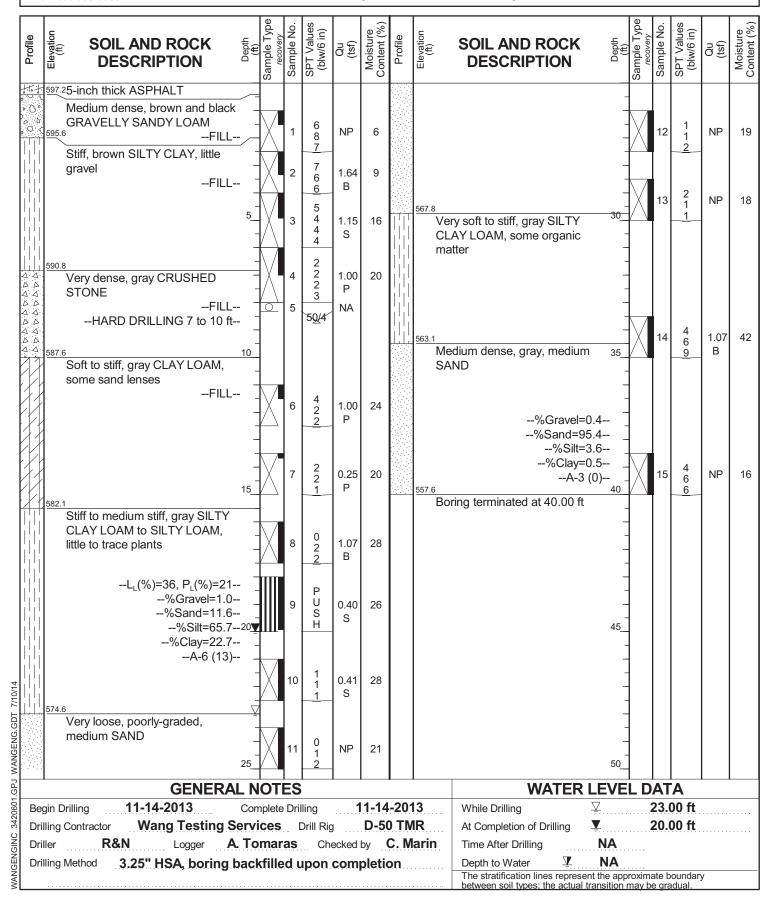
## **BORING LOG 52-SGB-10**

WEI Job No.: 342-06-01

Client PARSONS

Project US 52 / IL 64 Over the Mississippi River
Location Carroll County, IL and Jackson County, IA

Datum: NAVD 88 Elevation: 597.58 ft North: 1980462.75 ft East: 2296357.35 ft Station: 1559+48.16 Offset: 12.52 LT





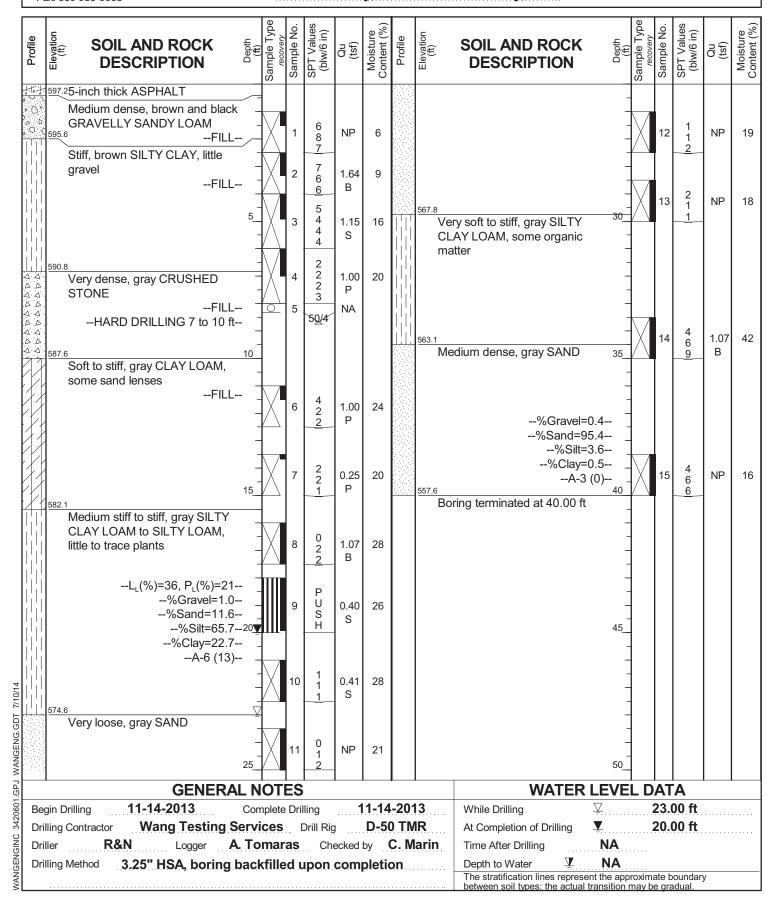
## **BORING LOG 52-SGB-10**

WEI Job No.: 342-06-01

Client PARSONS

Project US 52 / IL 64 Over the Mississippi River
Location Carroll County, IL and Jackson County, IA

Datum: NAVD 88 Elevation: 597.58 ft North: 1980462.75 ft East: 2296357.35 ft Station: 1559+48.16 Offset: 12.52 LT





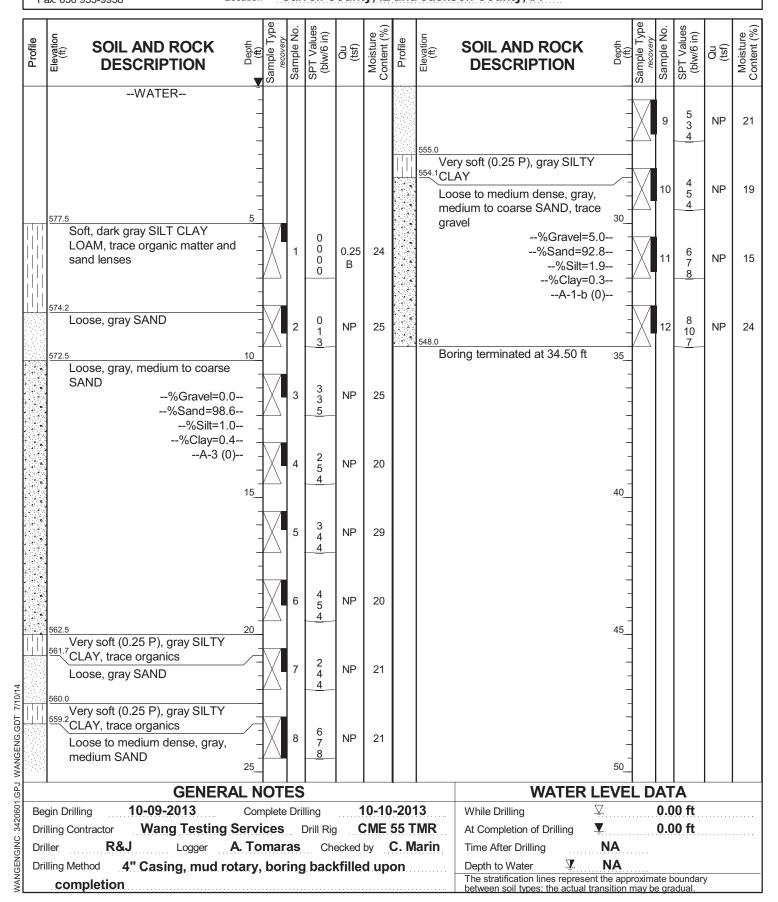
## **BORING LOG 52-SGB-11**

WEI Job No.: 342-06-01

Client PARSONS

Project US 52 / IL 64 Over the Mississippi River
Location Carroll County, IL and Jackson County, IA

Datum: NAVD 88 Elevation: 582.45 ft North: 1980395.05 ft East: 2296369.01 ft Station: 1559+58.25 Offset: 55.48 RT





## **BORING LOG BSB-24**

WEI Job No.: 342-06-01

Client PARSONS

Project US 52 / IL 64 Over the Mississippi River
Location Carroll County, IL and Jackson County, IA

Datum: NAVD 88 Elevation: 598.94 ft North: 1980482.42 ft East: 2296444.64 ft Station: 1560+58.13 Offset: 37.04 LT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft) Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)		ND ROCK RIPTION	Depth (#)	Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
11.1	598.64	.5-inch thick, ASPHALT	/=						me	edium SAND	, trace grave	I					
		PAVEMENT- Medium dense, brown LOAM, ttle gravel FILL-		1	9 9 4	NP	2					-		11	6 8 10	NP	23
	b	ery loose to medium dense, rown, medium to coarse SAND ace gravel FILL-		2	2 3 2	NR						96.3		12	4 6 7	NP	23
	590.9		-	3	2 2 1	NP	10				/\-						
22	590.2 V	Yery soft (<0.25P), brown CLAY OAMFILL- Yery loose to loose, brown, nedium to coarse SAND, trace	A	4	2 2 1	NP	3					35_		13	3 4 5	NP	22
	587.80 V	rganic matterFILL-  Yery soft to soft, brown and gray  ILTY LOAM, trace sand lenses	-/ I X	5	2 2 4	0.33 B	16										
	584.9	FILL- L <sub>L</sub> (%)=36, P <sub>L</sub> (%)=20- %Gravel=0.0- %Sand=16.5- %Silt=67.4-		6	P U S H	< 0.25 P	18					40_		14	2 3 2	NP	22
	B	%Clay=16.0- A-6 (13)- frown and gray SANDY LOAM fedium stiff, brown and gray EILTY CLAY, trace organic	-// 🖳	7	1 2 3	0.74 B	29				, gray, coars o some grave						
		natter	20	8	1 1 1	0.57 B	33					45_		15	6 6	NP	21
T 7/10/14	575.9	oose, gray, medium SAND		9	1 2 3	0.82 B	36				%Gravel	96.3	-				
WANGENGINC 3420601.GPJ WANGENG.GDT 7/10/14	575.2C	Medium stiff (0.5P), gray SILTY CLAY oose to medium sense, gray,	25	10	6 11 10	NP	20				%Silt %Clay A-			16	10 10 10	NP	19
GP.		GENERA	L NOT	ES	<u> </u>						WATER	R LEVE	L D	ΑT	Α		
690 Be	gin Dril	ling 11-11-2013	Complet	e Dri	lling	1	11-15	-201	13	While Drillin	ng	<u>Ā</u>		14.0	00 ft		
Dri	Drilling Contractor Wang Testing Service Drill Rig D-50 TMR						At Complete	ion of Drilling	<u>Ť</u>			IA					
ğ Dri	Driller R&N Logger A. Tomaras Checked by C. Marin						Time After	•	NA								
Dri	Drilling Method 3.25" HSA to 18', 4" Casing, mud rotary thereafter,							Depth to W	***	NA	orovi~	oto b	ounda	,			
<b>\$</b>	bor	ing backfilled upon compl	etion							between soil	ation lines repre types; the actua	al transition	may b	ate D e gra	oundar <u>ı</u> ıdual.	y	



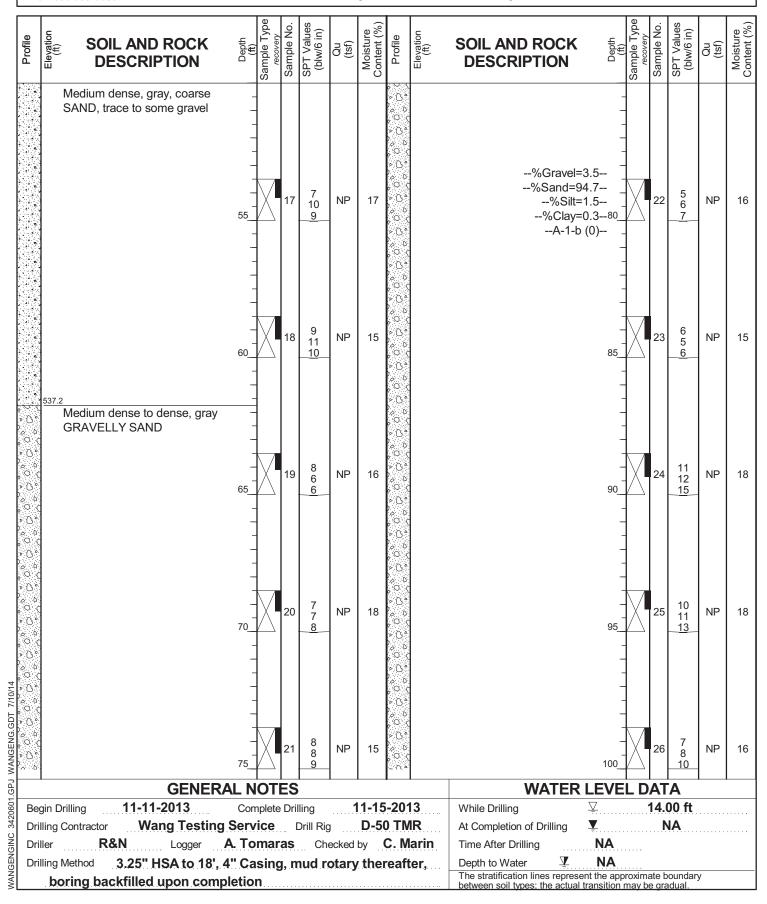
### **BORING LOG BSB-24**

WEI Job No.: 342-06-01

Client PARSONS

Project US 52 / IL 64 Over the Mississippi River
Location Carroll County, IL and Jackson County, IA

Datum: NAVD 88 Elevation: 598.94 ft North: 1980482.42 ft East: 2296444.64 ft Station: 1560+58.13 Offset: 37.04 LT





## **BORING LOG BSB-24**

WEI Job No.: 342-06-01

Client PARSONS

Project US 52 / IL 64 Over the Mississippi River
Location Carroll County, IL and Jackson County, IA

Datum: NAVD 88 Elevation: 598.94 ft North: 1980482.42 ft East: 2296444.64 ft Station: 1560+58.13 Offset: 37.04 LT

Profile	SOIL AND ROCK page DESCRIPTION	Sample Type	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND DESCRIF		Depth (ft)	Sample Type	Sample No.	(blw/6 in)	Moisture Content (%)
, O														
, O		2	1 12	NP	17									
	83	-	13											
, O		- - -												
, o	%Gravel=10.8 %Sand=86.2 %Sit=2.6-	28	3 10 12	NP	14									
	X X X X X X X X X X X X X X X X X X X		15	-										
, O,	6) .g	- - -												
		25	1 '	NP	16									
,0		-	8	-										
, O,	d	30	41	NP	15									
353,530	Boring terminated at 120.00 ft	+	<u>19</u>											
4														
WANGENGINC 3420601.GPJ WANGENG.GDT 7/10/14														
ENG.GD		1												
WANGE	125_													
1.GPJ	GENERAL NOTES							\	VATER	LEVE	L D	ΑΤΑ	1	-
Be	Begin Drilling 11-11-2013 Complete Drilling 11-15-2013						While Drilling		<u> </u>	1	4.00			
Ö Di	Drilling Contractor Wang Testing Service Drill Rig D-50 TMR  Logger A. Tomaras Checked by C. Marin						At Completion of Time After Drilli	_	<b>▼</b> NA		NΑ	١		
ID GENG	Driller R&N Logger A. Tomaras Checked by C. Marin  Drilling Method 3.25" HSA to 18', 4" Casing, mud rotary thereafter,					Depth to Water	Ā	NA						
WAN	boring backfilled upon completic	_		-				The stratification between soil type	lines repres s; the actua	ent the app I transition i	roxima	te bou gradu	ndary al.	



# **APPENDIX B**

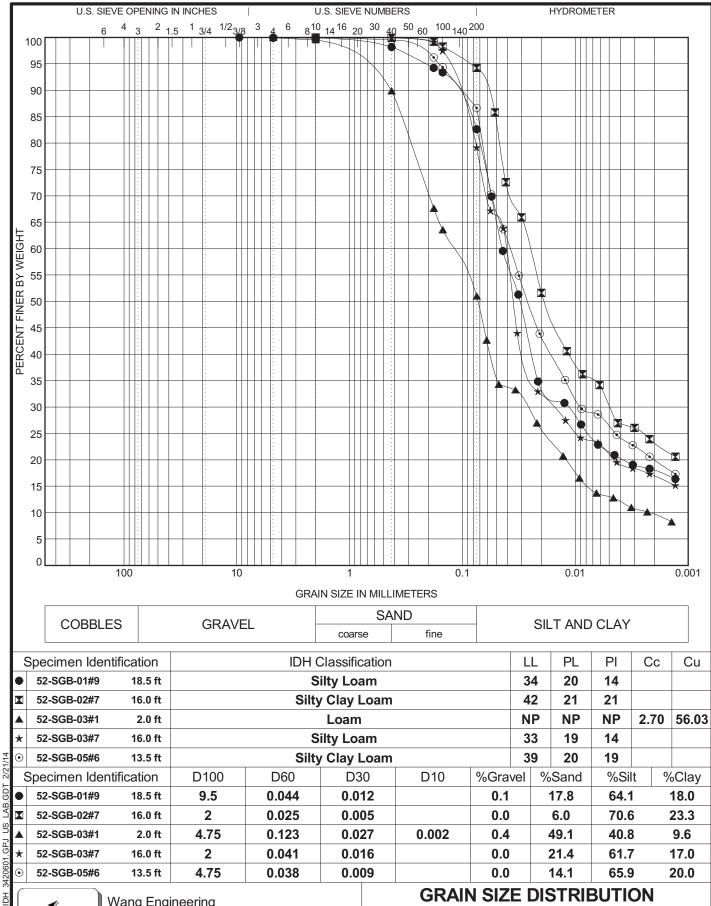
#### **SOIL TEST DATA**

| ROUTE | P-92-001-11 | US 52 | | COUNTY | COUNTY | COUNTY | US 25 (Sta. 1546+00 to 1560+65) | Jackson County, lowar

Lab. No.	52-SGB-03 No.1	
Station ft)	1552+41.46	
Offset (ft)	5.56 RT	
Depth (ft)	2	
AASHTO M 145		
Classification and Group Index	A-4 (0)	
Illinois Textural Classification	<b>.</b>	
(Illinois Method)	Loam	
GradationPassing 1" Sieve %		
" 3/4" Sieve %		
" 1/2" Sieve %		
" No.4 Sieve %	100.0	
" No.10 Sieve %	99.6	
" No.4 0 Sieve %	89.9	
" No.100 Sieve %	63.6	
" No.200 Sieve %	50.5	
Sand % (AASHTO T 88)	49.1	
Silt % (AASHTO T 88)	40.8	
Clay % (AASHTO T 88)	9.6	
Liquid limit % (AASHTO T 89)	0.0	
Plasticity index % (AASHTO T 90)	0.0	
IBR % (Illinois Method)		
Standard Dry Density % (AASHTO T 99)		
Optimum Moisture % (AASHTO T 99)		
Subgrade Support Rating	POOR	
Insitu Moisture % (AASHTO T 99)	17	



## **APPENDIX C**





Wang Engineering 1145 N Main Street Lombard, IL 60148

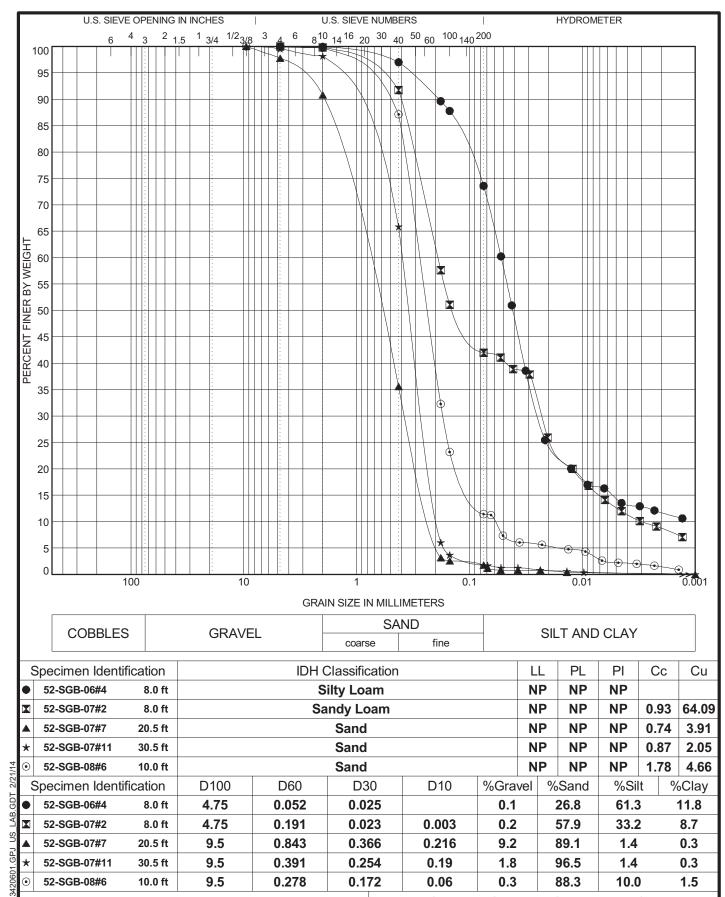
Telephone: 630 953-9928

Fax: 630 953-9938

#### **GRAIN SIZE DISTRIBUTION**

Project: US 52 / IL 64 Over the Mississippi River

Location: Carroll County, IL and Jackson County, IA





Wang Engineering 1145 N Main Street Lombard, IL 60148

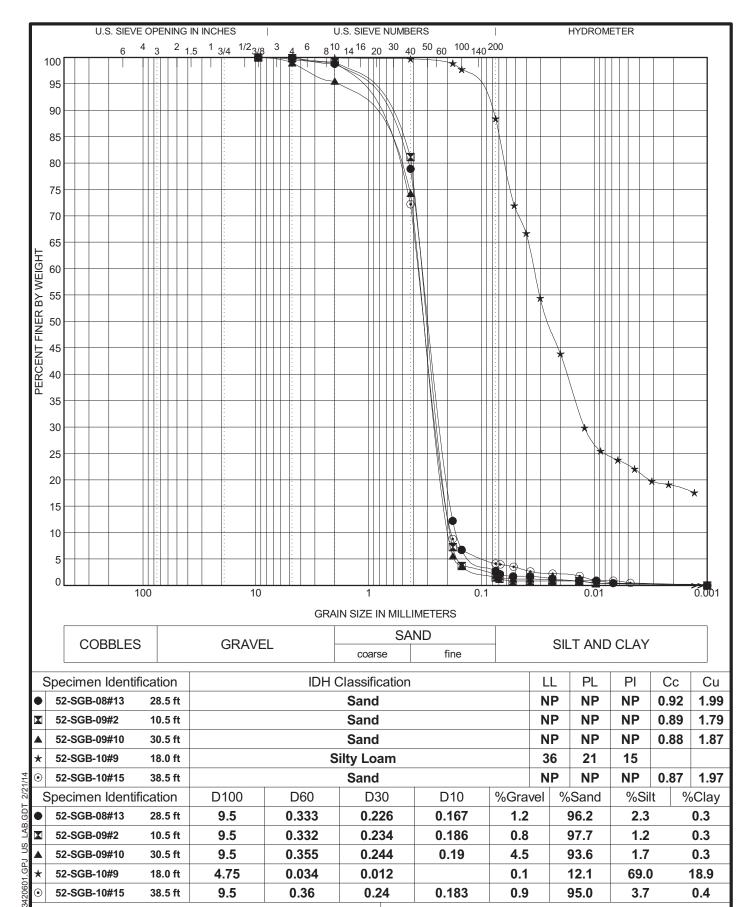
Telephone: 630 953-9928

Fax: 630 953-9938

#### **GRAIN SIZE DISTRIBUTION**

Project: US 52 / IL 64 Over the Mississippi River

Location: Carroll County, IL and Jackson County, IA





Wang Engineering 1145 N Main Street Lombard, IL 60148

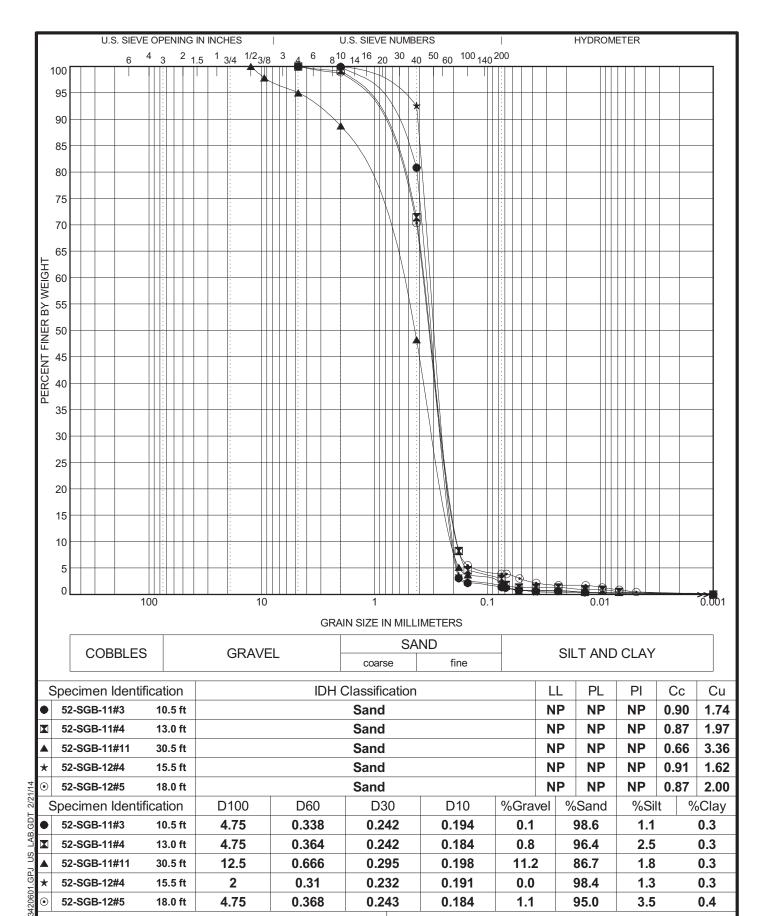
Telephone: 630 953-9928

Fax: 630 953-9938

#### **GRAIN SIZE DISTRIBUTION**

Project: US 52 / IL 64 Over the Mississippi River

Location: Carroll County, IL and Jackson County, IA





Wang Engineering 1145 N Main Street Lombard, IL 60148

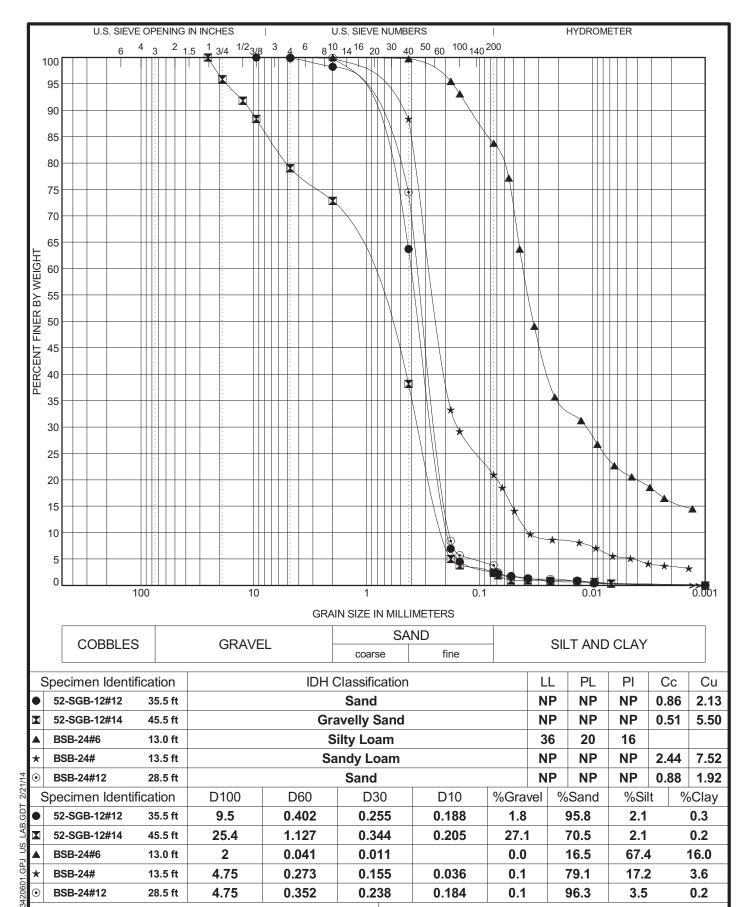
Telephone: 630 953-9928

Fax: 630 953-9938

#### **GRAIN SIZE DISTRIBUTION**

Project: US 52 / IL 64 Over the Mississippi River

Location: Carroll County, IL and Jackson County, IA





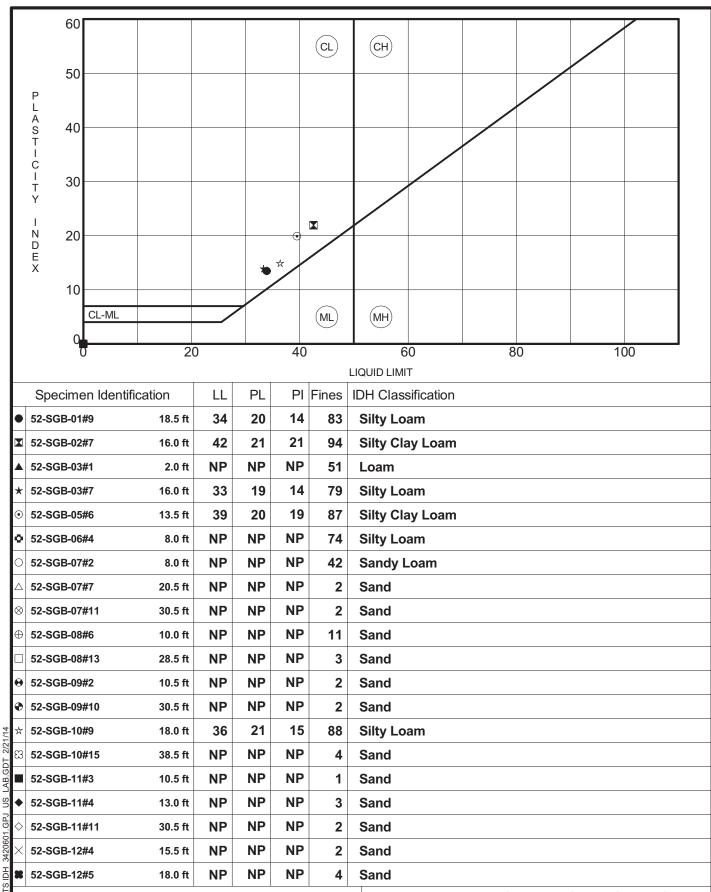
Wang Engineering 1145 N Main Street Lombard, IL 60148 Telephone: 630 953-9928

Telephone: 630 953-9928 Fax: 630 953-9938

#### **GRAIN SIZE DISTRIBUTION**

Project: US 52 / IL 64 Over the Mississippi River

Location: Carroll County, IL and Jackson County, IA



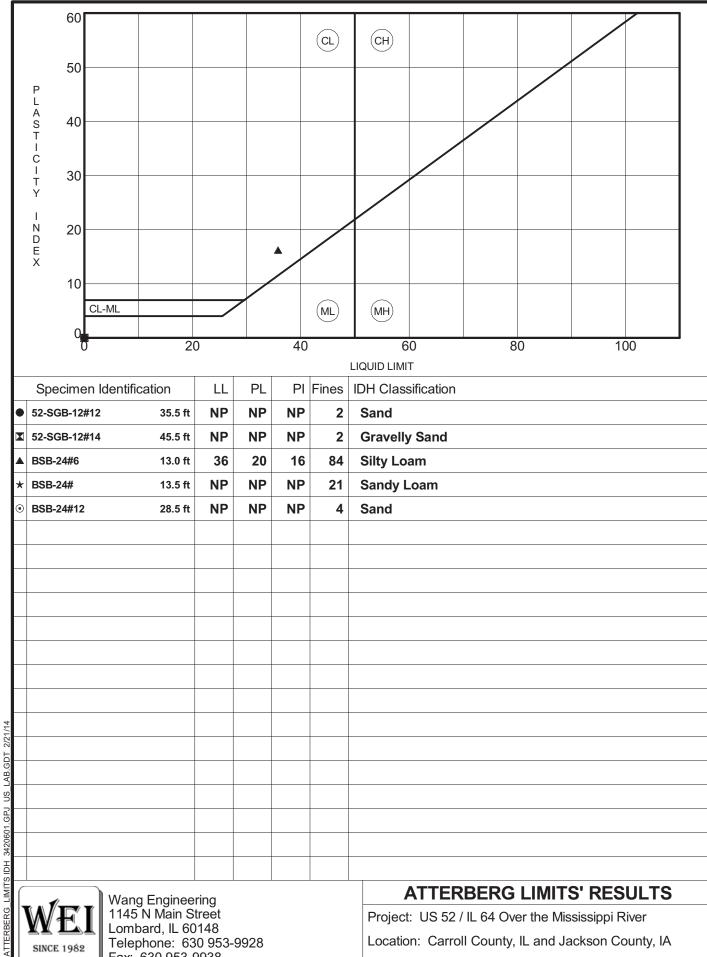


Telephone: 630 953-9928 Fax: 630 953-9938

#### ATTERBERG LIMITS' RESULTS

Project: US 52 / IL 64 Over the Mississippi River

Location: Carroll County, IL and Jackson County, IA



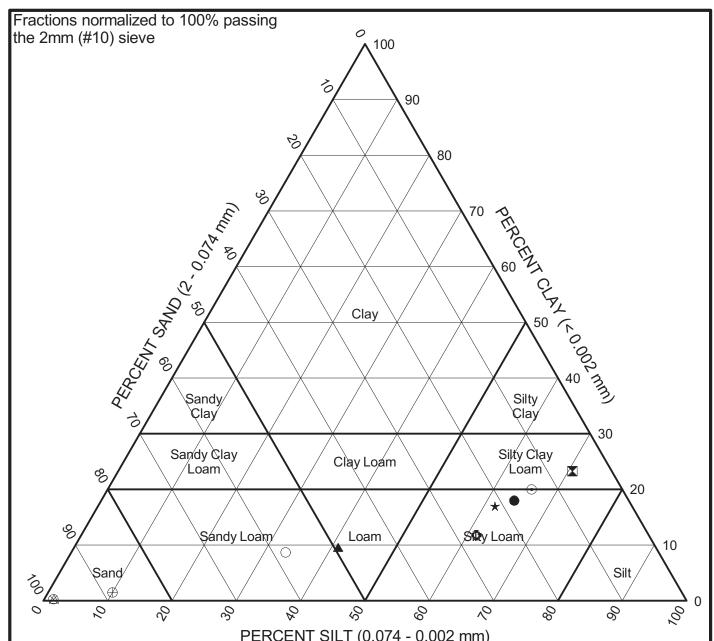
SINCE 1982

Lombard, IL 60148

Telephone: 630 953-9928

Fax: 630 953-9938 Number: 342-06-01

Location: Carroll County, IL and Jackson County, IA



	1 ENGENT GIET (0.074 0.002 mm)									
	Sample	Depth (ft)	Sand	Silt Clay		Silt Clay Classifica				
	Sample	Deptii (it)	(%)	(%)	(%)	IL DOT	AASHTO	ASTM		
•	52-SGB-01#9	18.5	17.8	64.2	18.0	Silty Loam	A-6 (11)	CL		
X!	52-SGB-02#7	7 16.0	6.0	70.6	23.3	Silty Clay Loam	A-7-6 (21)	CL		
<b>A</b> !	52-SGB-03#1	2.0	49.3	41.0	9.6	Loam	A-4 (0)	ML		
*	52-SGB-03#7	7 16.0	21.4	61.7	17.0	Silty Loam	A-6 (10)	CL		
<u>•</u>	52-SGB-05#6	3 13.5	14.1	65.9	20.0	Silty Clay Loam	A-6 (17)	CL		
Q:	52-SGB-06#4	8.0	26.8	61.4	11.8	Silty Loam	A-4 (0)	ML		
0:	52-SGB-07#2	2 8.0	58.0	33.3	8.7	Sandy Loam	A-4 (0)	SM		
Δ;	52-SGB-07#7	20.5	98.1	1.5	0.3	Sand	A-1-b (0)	SP		
⊗5	2-SGB-07#1	1 30.5	98.3	1.4	0.3	Sand	A-3 (0)	SP		
<b>⊕</b> :	52-SGB-08#6	10.0	88.6	10.0	1.5	Sand	A-2-4 (0)	SP-SM		

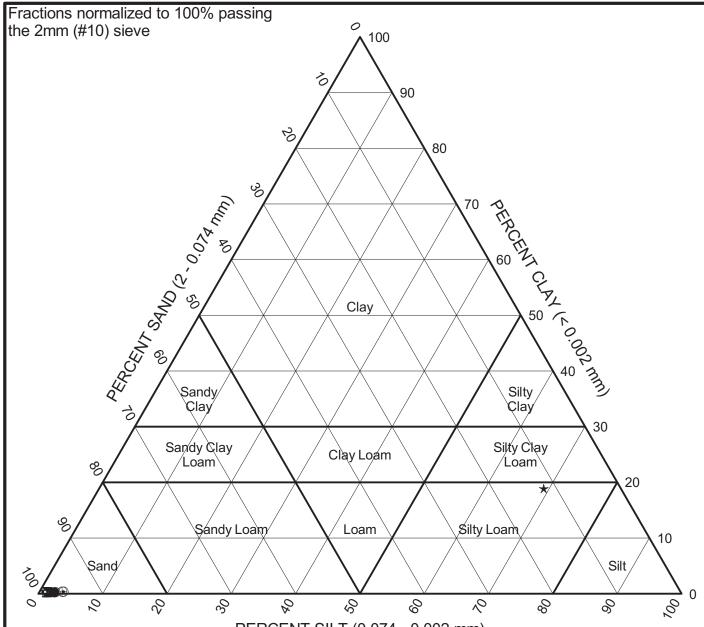


Fax: 630 953-9938

#### **IDH Textural Classification Chart**

Project: US 52 / IL 64 Over the Mississippi River

Location: Carroll County, IL and Jackson County, IA



PERCENT	SILT	(0.074 -	0.002	mm)
	OILI	(0.017	0.002	111111

					<u>,</u>	,				
	Sample	Depth (ft)	Sand	Silt	Clay	Classification				
	Sample	Deptil (it)	(%)	(%)	(%)	IL DOT	AASHTO	ASTM		
<b>©</b> 5	2-SGB-08#1	3 28.5	97.4	2.3	0.3	Sand	A-3 (0)	SP		
X!	2-SGB-09#2	2 10.5	98.5	1.2	0.3	Sand	A-3 (0)	SP		
<b>_</b> 5	2-SGB-09#1	0 30.5	98.0	1.8	0.3	Sand	A-3 (0)	SP		
*	52-SGB-10#9	18.0	12.1	69.1	18.9	Silty Loam	A-6 (13)	CL		
<b>⊙</b> 5	2-SGB-10#1	5 38.5	95.9	3.7	0.4	Sand	A-3 (0)	SP		
Q:	2-SGB-11#3	3 10.5	98.7	1.1	0.3	Sand	A-3 (0)	SP		
ः	2-SGB-11#4	13.0	97.2	2.5	0.3	Sand	A-3 (0)	SP		
∆5	2-SGB-11#1	1 30.5	97.6	2.0	0.3	Sand	A-1-b (0)	SP		
$\otimes$	52-SGB-12#4	15.5	98.4	1.3	0.3	Sand	A-3 (0)	SP		
⊕\$	52-SGB-12#5	18.0	96.1	3.5	0.4	Sand	A-3 (0)	SP		

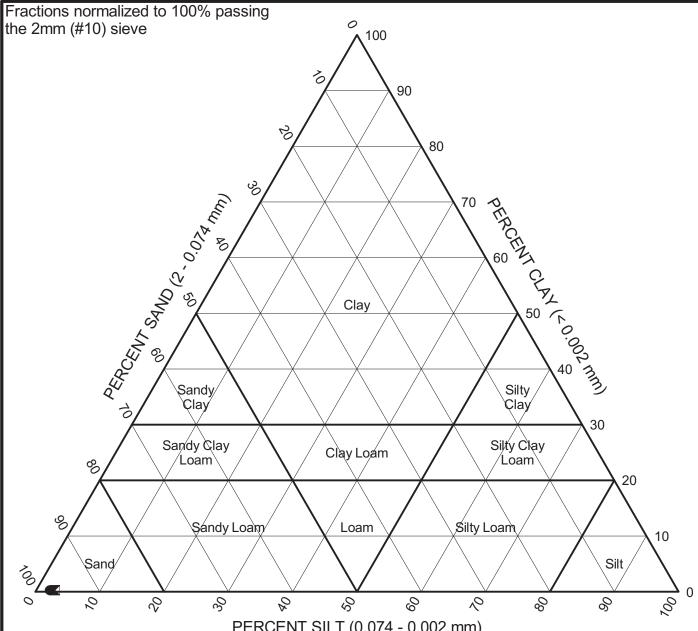


Fax: 630 953-9938

#### **IDH Textural Classification Chart**

Project: US 52 / IL 64 Over the Mississippi River

Location: Carroll County, IL and Jackson County, IA



<b>PERCENT</b>	SILT	(0 074 -	0.002	mm)
FLINGLINI	SILI	(U.U <i>14</i> -	0.002	1111111

	Sample Depth (ft)		nth (ft) Sand Sil		Clay	Classification				
	Sample	Deptii (it)	(%)	(%)	(%)	IL DOT	AASHTO	ASTM		
<b>●</b> 5	2-SGB-12#1	2 35.5	97.6	2.1	0.3	Sand	A-3 (0)	SP		
<b> ≤</b> 5	2-SGB-12#1	4 45.5	96.7	2.9	0.3	Gravelly Sand	A-1-b (0)	SP		



Fax: 630 953-9938

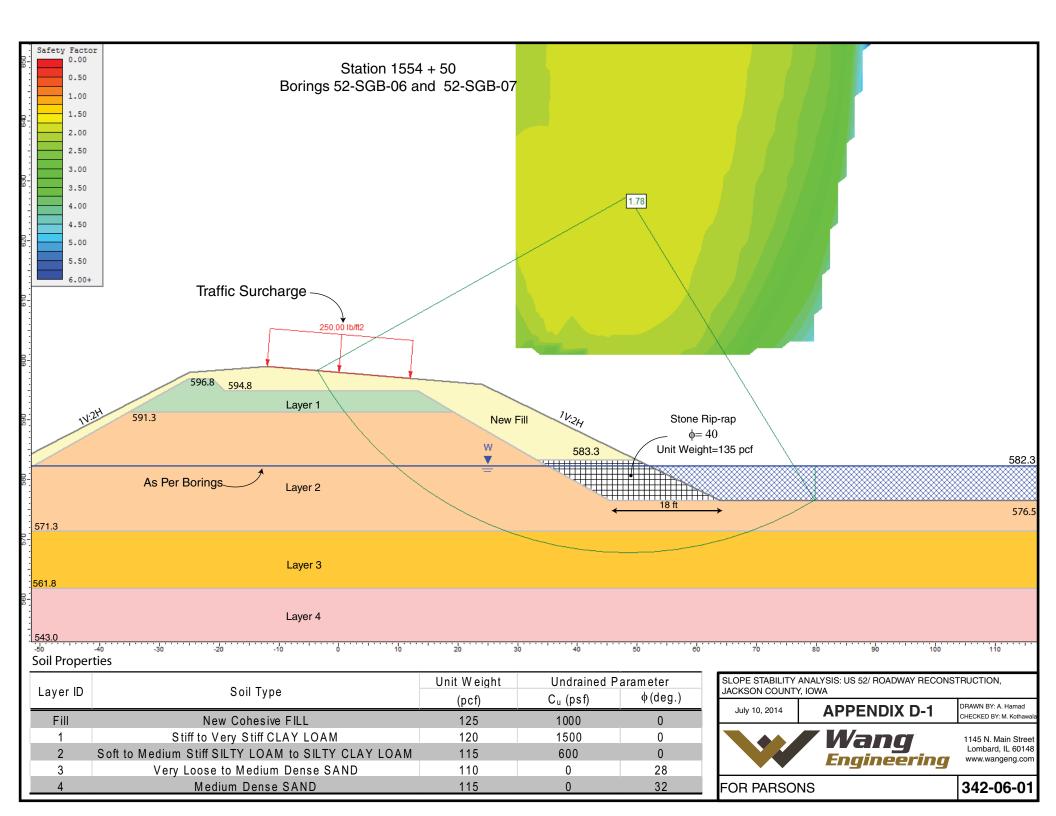
### **IDH Textural Classification Chart**

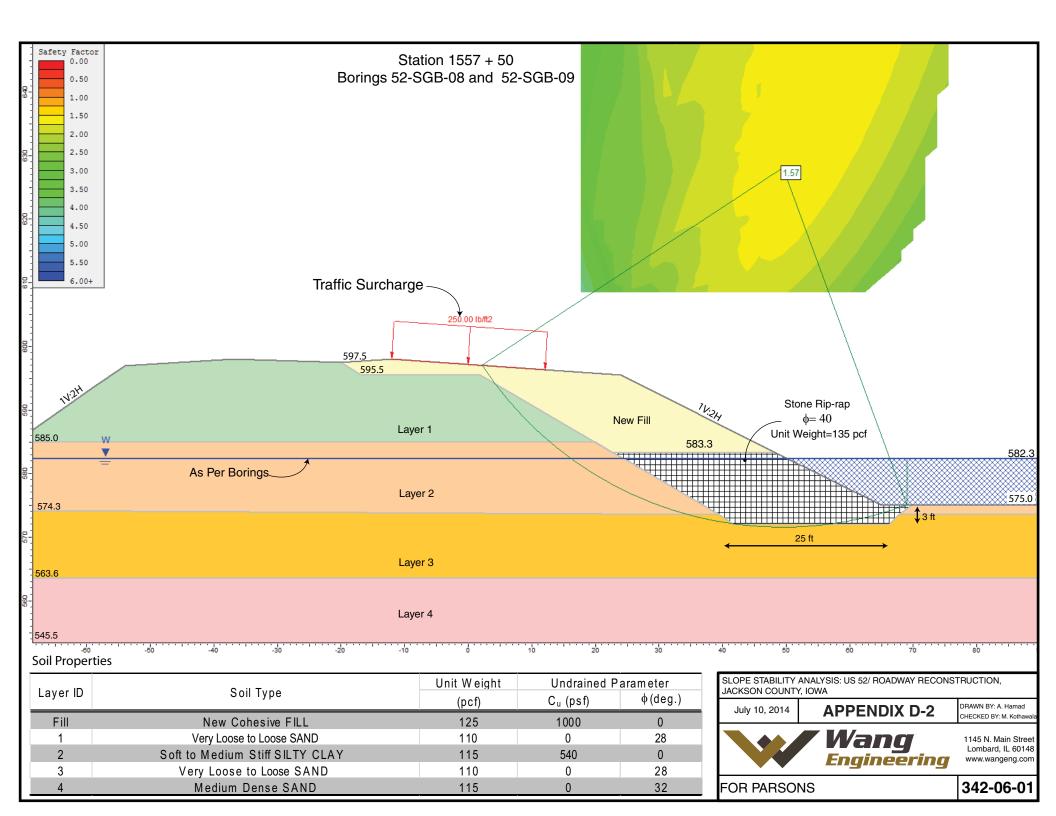
Project: US 52 / IL 64 Over the Mississippi River

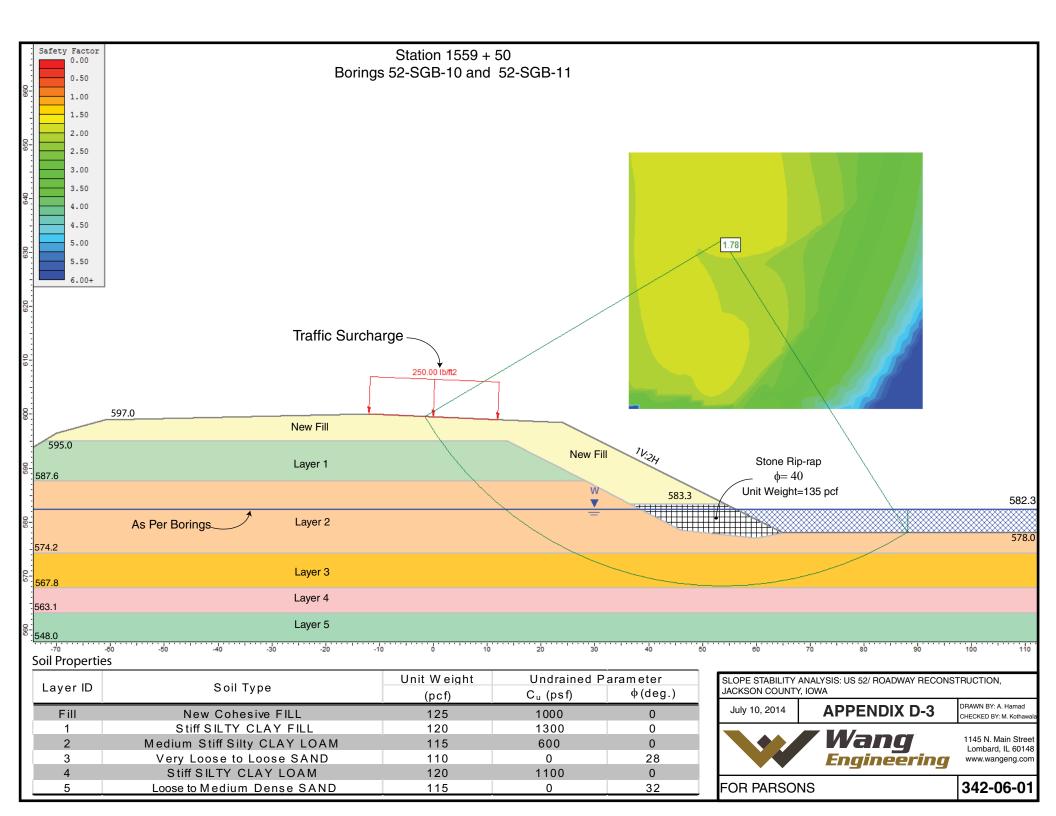
Location: Carroll County, IL and Jackson County, IA



# APPENDIX D









# **APPENDIX E**

