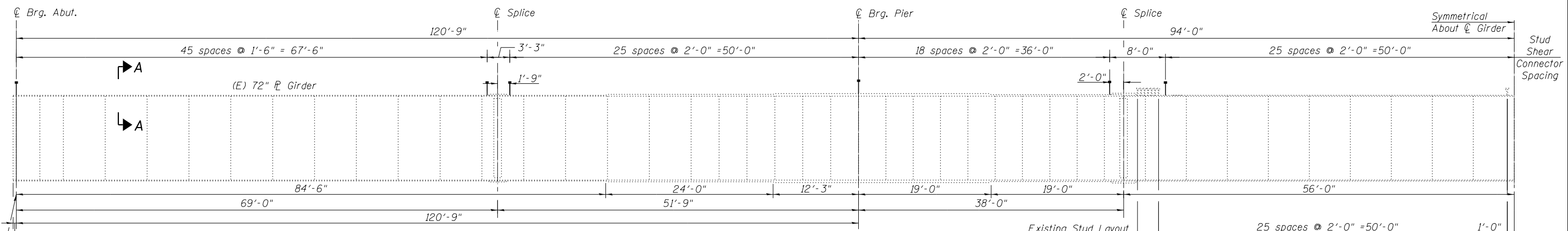


HALF FRAMING PLAN

I_s, S_s : Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total and Overload) due to non-composite dead loads (in^4 and in^3).
 $I_c(n), S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total and Overload) due to short-term composite live loads (in^4 and in^3).
 $I_c(3n), S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total and Overload) due to long-term composite (superimposed) dead loads (in^4 and in^3).
 $I_c(cr), S_c(cr)$: Composite moment of inertia and section modulus of the steel and longitudinal deck reinforcement, used for computing f_s (Total-Strength I and Service II) in cracked sections, due to both short-term composite live loads and long-term composite (superimposed) dead loads (in^4 and in^3).
 Z : Plastic Section Modulus of the steel section in non-composite areas (in^3).
 Q : Un-factored non-composite dead load (kips/ft.).
 M_Q : Un-factored moment due to non-composite dead load (kip-ft.).
 s_Q : Un-factored long-term composite (superimposed) dead load (kips/ft.).
 M_{sQ} : Un-factored moment due to long-term composite (superimposed) dead load (kip-ft.).
 M_L : Un-factored live load moment (kip-ft.).
 M_I : Un-factored moment due to impact (kip-ft.).
 M_a : Factored design moment (kip-ft.).
 $1.3 [M_Q + M_{sQ} + \frac{5}{3} (M_L + M_I)]$
 M_u : Compact composite moment capacity according to AASHTO LFD 10.50.1.1 or compact non-composite moment capacity according to AASHTO LFD 10.48.1 (kip-ft.).
 f_s (Overload): Sum of stresses as computed from the moments below (ksi).
 $M_Q + M_{sQ} + \frac{5}{3} (M_L + M_I)$
 f_s (Total): Sum of stresses as computed from the moments below on non-compact section (ksi).
 $1.3 [M_Q + M_{sQ} + \frac{5}{3} (M_L + M_I)]$
 VR : Maximum L + impact shear range within the composite portion of the span for stud shear connector design (kips).



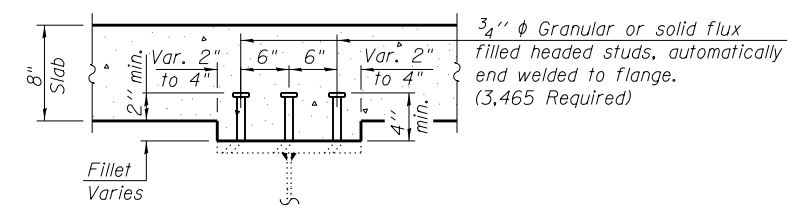
HALF GIRDER ELEVATION

MOMENT TABLE	INTERIOR GIRDERS		
	0.4 Sp.1/ 0.6 Sp.3	Pier 1 & 2	0.5 Sp. 2
I_s	(in^4) 45365	137690	63979
$I_c(n)$	(in^4) 104552	230825	165155
$I_c(3n)$	(in^4) 77603	179748	115276
$I_c(cr)$	(in^4) 150841		
S_s	(in^3) 1234	3600	2250
$S_c(n)$	(in^3) 1697	4152	3002
$S_c(3n)$	(in^3) 1542	3900	2749
$S_c(cr)$	(in^3) 3707		
Q	(k/ft) 1.120	1.222	1.120
M_Q	(k) 496	3266	1652
s_Q	(k/ft) 0.340	0.340	0.340
M_{sQ}	(k) 208	951	639
M_L	(k) 934	1656	1444
M_{IM}	(k) 190	301	231
$\frac{5}{3} [M_L + M_I]$	(k) 1874	3262	2792
M_a	(k) 3351	9722	6607
M_u	(k) 4821		7181
f_s Q non-comp	(ksi) 4.9	10.9	8.8
f_s Q (comp)	(ksi) 1.7	3.1	2.8
f_s $\frac{5}{3} [M_L + M_I]$	(ksi) 13.3	10.6	11.2
f_s (Overload)	(ksi) 19.9	24.6	22.8
f_s (Total)	(ksi) 32.0		
VR	(k) 58.2	75.2	59.9

REACTION TABLE	INTERIOR GIRDERS	
	Abut.	Pier
R_Q	(k) 47.5	250.5
R_L	(k) 41.4	94.2
R_I	(k) 8.4	10.9
R_{Total}	(k) 97.3	355.6

CONSTRUCTION SEQUENCE

The bearings shall be removed and replaced following the concrete deck removal and before the proposed deck is constructed.



SECTION A-A

*Compact section
 **Braced non-compact and partially braced section

BLANK, WESSELINK, COOK & ASSOCIATES

DECATUR, ILLINOIS ENGINEERS - CONSULTANTS DESIGN FIRM NO. 184000894

FILE NAME =	USER NAME =	DESIGNED PBB	REVISD -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	FRAMING PLAN STRUCTURE NO. 058-0049	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
		CHECKED MCB	REVISD -			710	(48X-B-2)BR & 148BR)BR	MACON	144	54	
		DRAWN MLO	REVISD -			CONTRACT NO. 74438					
		CHECKED PBB/MCB	REVISD -			ILLINOIS FED. AID PROJECT					