### **Abbreviated Structural Geotechnical Report**

Proposed 12' x 3' Culvert Under I-80

**IDOT Job Number P-91-185-09** 

**Proposed SN: NA** 

**Existing SN: NA** 

Contract No.: 62R29

New Lenox, Will County, IL

### **Prepared for:**

EXP US Services Inc. 205 North Michigan Avenue, Suite 3600 Chicago, IL 60601-5924

### Prepared by:

Geo Services, Inc. 805 Amherst Court Suite 204 Naperville, Illinois 60565 (630) 305-9186

**JOB NO. 20012** 

April 04, 2023





### **Abbreviated Structure Geotechnical Report**

Original Report Date: 10/17/2022	Proposed SN: NA	Route:	IL-80
Revised Date: 04/04/2023	Existing SN: NA	Section:	
Geotechnical Engineer: Geo Services	Inc,	County:	Will
Structural Engineer: EXP US Service	s Inc	Contract:	62R29

**Indicate the proposed structure type, substructure types, and foundation locations (attach plan and elevation drawing):** The proposed structure is a 12' x 3' box culvert that goes under I-80, replacing the existing 36" pipe culvert. The preliminary plan and profile are showing the proposed culvert is about 153 feet. A 14'-6" culvert drop box is attached at the north and south end of the proposed culvert to transfer the water from the proposed drainage ditch to the proposed culvert. The preliminary design drawings are attached.

Discuss the existing boring data, existing plans foundation information, new subsurface exploration and need for any additional exploration to be provided with SGR Technical Memo (attach all data and subsurface profile plot): Three borings were drilled in December 2021, February and March 2022 for subsurface exploration. All borings had 12" of top soil and asphalt at the top of the borings. Boring CB-001 was drilled near the north end of the proposed culvert. Boring CB-001 is consists of stiff silty clay from 1 feet to 10 feet followed by very stiff clay loam to a termination of boring at 20 feet below the surface. Boring CB-002 was drilled close to the middle of the proposed culvert. CB-002 is consists of stiff to very stiff clay loam and a a pocket of silty clay from 1 feet to a termination of boring at 20 feet below the surface. Boring CB-003 was drilled close to the south end of the proposed culvert. CB-003 consists of very stiff silty clay and clay loam with a pocket of topsoil to a termination of boring at 20 feet below the surface.

Provide the location and maximum height of any new soil fill or magnitude of footing bearing pressure. Estimate the amount and time of the expected settlement. Indicate if further testing, analysis, and/or ground improvement/treatment is necessary: The proposed plan and profile are showing 1' of fill across the length of the culvert is expected. No settlement issues are anticipated.

Identify any new cuts or fill slope angles and heights. Estimate the factor of safety against slope failure. Indicate if further testing, analysis or ground improvement/treatment is necessary: The proposed plan and profile are showing 1' of fill across the length of the culvert is expected. No settlement concerns are anticipated.

Indicate at each substructure, the 100-year and 200-year total scour depths in the Hydraulics report, the non-granular scour depth reduction, the proposed ground surface, and the recommended foundation design scour elevations: No longer required for culverts as per ABD memo 14.2.

Determining the seismic soil site class, the seismic performance zone, the 0.2 and 1.0 second design spectral accelerations and indicate if that the soils are liquefiable: Not required for buried structures as per the Bridge Manual 2.3.10

Confirm feasibility of the proposed foundation or wall type and provide design parameters. Attach a pile design table indicating feasible pile types, various nominal required bearings, factored resistances available and corresponding estimated lengths at locations where piles will be used. Provide factored bearing resistance and unit sliding resistance at various elevations and confirm no ground improvement/treatment is necessary where spread footings are proposed. Estimated top of rock elevations as well as preliminary factored unit side and tip resistance values shall be indicated when drilled shafts are proposed: The proposed culvert should be designed based on factored bearing resistance of 5,000.0 psf, the boring logs show stiff to very stiff clay below the length of the structure.

The design drawings are showing that the 12'x 3' culvert will have a drop box on both side. It will serve as a drainage path to transfer water from the proposed drainage ditch (soth of the proposed culvert) to the (north of the proposed culvert) that is going under I-80.

Design parameters for lateral soil properties are provided in appendix F.

Calculate the estimated water surface elevation and determine the need for cofferdams (type 1 or 2), and seal coat: The construction site will be dewatered. Cofferdam will not be needed.

Assess the need for sheeting or soil retention or temporary construction slope and provide recommendation for other construction concerns: Per the structural engineering (GKE), temporary soil retention system (TSRS) will be required for staging of construction. The design of the TSRS is the responsibility of the contractor.

## APPENDIX A GENERAL NOTES

### **GENERAL NOTES**

### **CLASSIFICATION**

American Association of State Highway & Transportation Officials (AASHTO) System used for soil classification.

$\sim$ 1		1	O - 11 -
Cor	nesion	1229	Solis
001	1001011		

Relative	No. of Blows	<u>TERMINOLOGY</u>
<u>Density</u>	per foot N	
<del></del>		<b>Streaks</b> are considered to be paper thick.
Very Loose	0 to 4	Lenses are considered to be less than 2
Loose	4 to 10	inches thick. Layers are considered to
Medium Dense	10 to 30	be less than 6 inches thick. Stratum are
Dense	30 to 50	considered to be greater than 6 inches thick.
Very Dense	Over 50	•

### Cohesive Soils

Very Stiff

Hard

Consistency	Unconfined Compressive Strength - qu (tsf)
Very Soft Soft	Less than 0.25 0.25 - 0.5
Medium Stiff	0.5 - 1.0
Stiff	10 - 20

2.0 - 4.0

Over 4.0

### DRILLING AND SAMPLING SYMBOLS

SS:	Split Spoon 1-3/8" I.D., 2" O.D.	HS:	Housel Sampler
ST:	Shelby Tube 2" O.D., except where noted	WS:	Wash Sample
AS:	Auger Sample	FT:	Fish Tail
DB:	Diamond Bit - NX: BX: AX	RB:	Rock Bit
CB:	Carboloy Bit - NX: BX: AX	WO:	Wash Out
OS:	Osterberg Sampler		

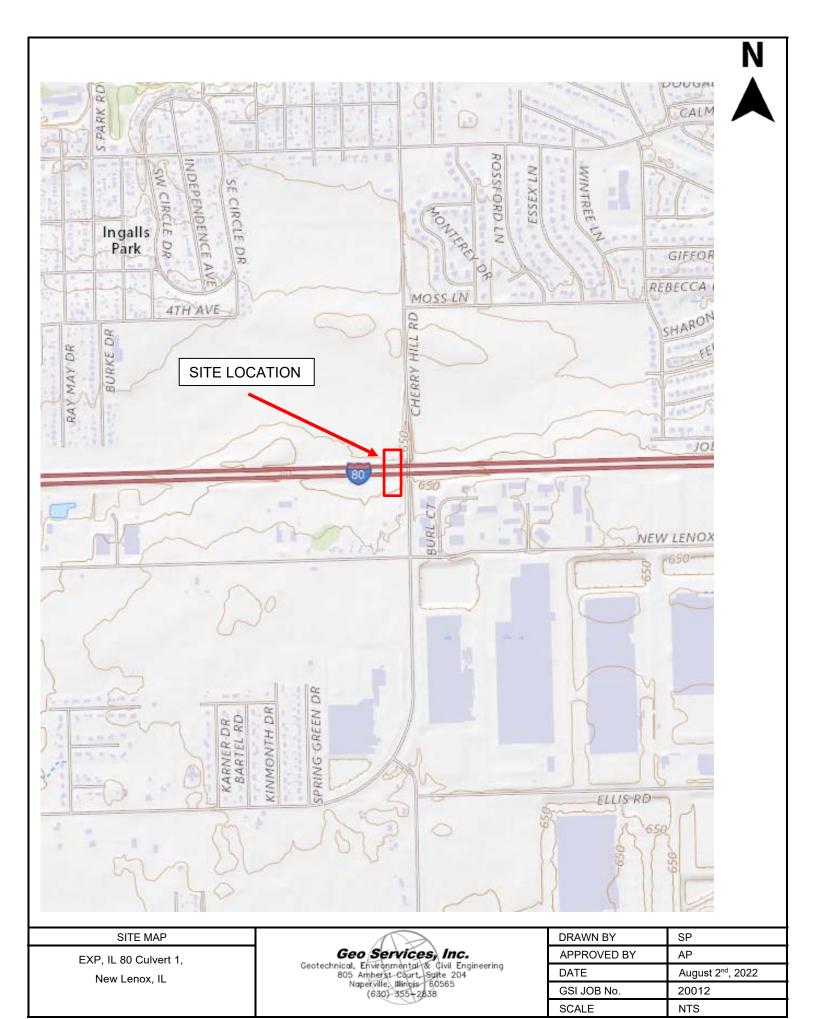
Standard "N" Penetration: Blows per foot of a 140 lb. hammer falling 30" on a 2" O.D. Split Spoon

### WATER LEVEL MEASUREMENT SYMBOLS

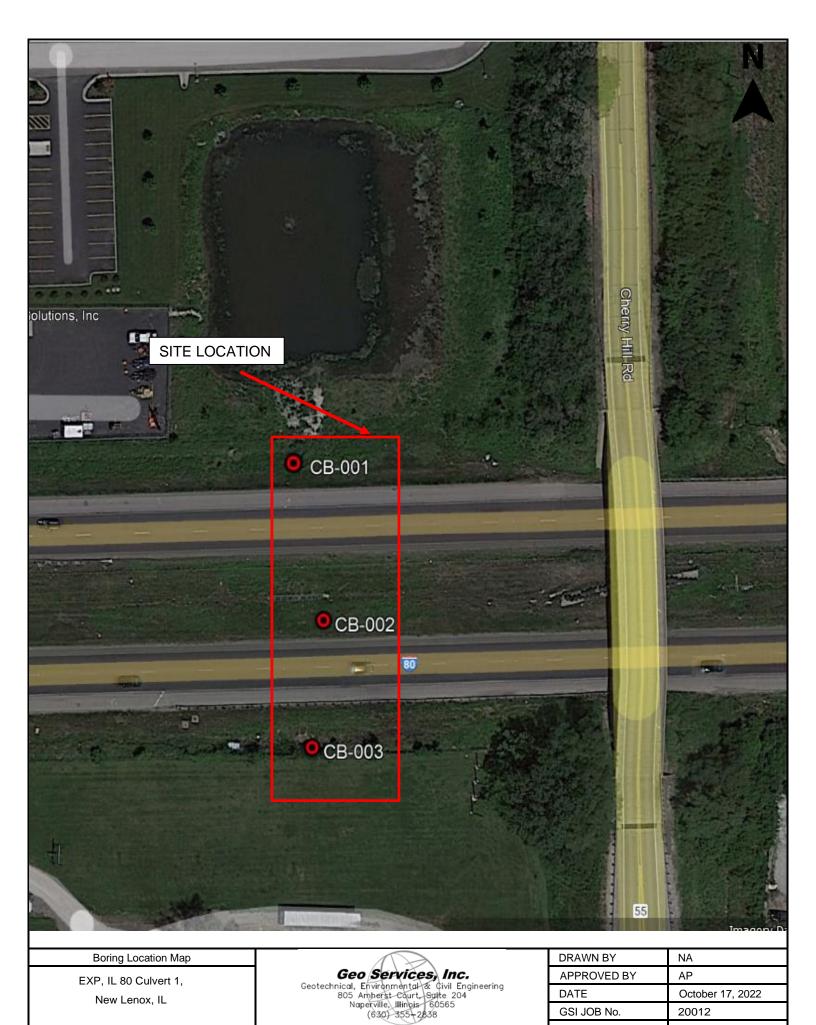
VVL:	vvater	WD:	while Drilling
WCI:	Wet Cave In	BCR:	Before Casing Removal
DCI:	Dry Cave In	ACR:	After Casing Removal
WS:	While sampling	AB:	After Boring

Water levels indicated on the boring logs are the levels measured in the boring at the times indicated. In pervious soils, the indicated elevations are considered reliable ground water levels. In impervious soils, the accurate determination of ground water elevations is not possible in even several days observation, and additional evidence on ground water elevations must be sought.

### APPENDIX B SITE LOCATION MAP



## APPENDIX C BORING LOCATION MAP



SCALE

NTS

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## APPENDIX D BORING LOGS



SOIL BORING 20012\_LOG.GPJ IL\_DOT.GDT 8/8/22

### **SOIL BORING LOG**

Page  $\underline{1}$  of  $\underline{1}$ 

**Date** 2/11/22

ROUTE	DE	SCR	IPTIO	N		I-80 Phase II	L	OGGI	ED BY		DJ
<b>SECTION</b> 13		I	LOCA	TION _	SW 1/	4, <b>SEC.</b> 13, <b>TWP</b> . T35N, <b>RNG</b> . R10 ing 1765541.6, <b>Easting</b> 1068838.	E, 3 <sup>rd</sup> <b>PI</b>	М,			
COUNTY Will DRI	LLING	ME	THOD						CME A	utoma	tic
STRUCT. NO.         -           Station         -           BORING NO.         CB-001           Station         863+69           Offset         69.6 ft Left	_ 	D E P T H	L O W	U C S Qu	M O I S T	Surface Water Elev. Stream Bed Elev.  Groundwater Elev.: First Encounter Upon Completion After Hrs.	ft▼	D E P T H	B L O W S (/6")	U C S Qu (tsf)	M O I S T
Ground Surface Elev. 644.20 12.0" TOPSOIL-black	_ π	(11)	(10 )	(tsf)	(70)	Aπer Hrs End Of Boring @ -20.0'. Boring	_ π	(11)	(10)	(tSI)	(%)
	643.20		2 2 3	1.00 P	53	backfilled with cuttings.					
becoming brown & gray @ -3.0'			2 3 4	1.40 B	27			-25			
	-	<u></u>	2 2 2	1.40 B	31						
6 CLAY LOAM-brown & gray-very	34.20	-10	2 2 3	1.00 P	36			-30			
stiff			ST	2.25 P	24						
becoming gray @ -13.0'		-15	4 5 7	2.50 P	21			-35			
			3 5 7	2.00 P	23						
6	324 20		3 4 6	2.00 P	23						



SOIL BORING 20012\_LOG.GPJ IL\_DOT.GDT 8/8/22

### **SOIL BORING LOG**

Page  $\underline{1}$  of  $\underline{1}$ 

**Date** 3/16/22

ROUTE	DESC	CRI	PTIO	N		I-80 Phase II		L0	OGG	ED BY	N	1M
SECTION 13		_ L	OCA	TION _	SE 1/4	l, SEC. 13, TWP. T35N, I ing 1765462.4, Easting	RNG. R10E,	3 <sup>rd</sup> PM	,			
COUNTY Will DRI	ILLING I	ME	THOD			low Stem Auger			(	CME A	utoma	tic
STRUCT. NO.         -           Station         -           BORING NO.         CB-002	_	D E P T :	B L O W	U C S	M O I S	Surface Water Elev Stream Bed Elev Groundwater Elev.:			D E P T	B L O W	U C S	M O I S
Station         863+84           Offset         31.2 ft Right		Н	S	Qu	Т	First Encounter Upon Completion	Dry Dry	_ ft _ ft	Н	S	Qu	Т
Ground Surface Elev. 644.10	ft (	(ft)	(/6")	(tsf)	(%)			ft	(ft)	(/6")	(tsf)	(%)
12.0" ASPHALT	643.10	_				End Of Boring @ -20.0 backfilled with cuttings	)'. Boring		_			
CLAY LOAM-dark brown & gray-stiff (Fill)			3	1.50	24							
	_		3	P P	27							
SILTY CLAY-dark gray to	641.10											
black-stiff	_		3 5	1.80	27							
		-5	7	В					-25			
CLAY LOAM-brown & gray-stiff to very stiff	638.60 —		3						_			
	_		2 3	1.00 P	25							
	_		2									
		-10	3 6	1.30 B	26				-30			
	_		2						_			
	_		4 7	1.60 B	22							
	_		-									
		-15	5 8 12	2.20 B	20				-35			
	_											
	_		7 8	3.50 P	20				_			
	_	_										
	- 624 10	-20	3 4 5	1.50 P	23				-40			



SOIL BORING 20012\_LOG.GPJ IL\_DOT.GDT 8/8/22

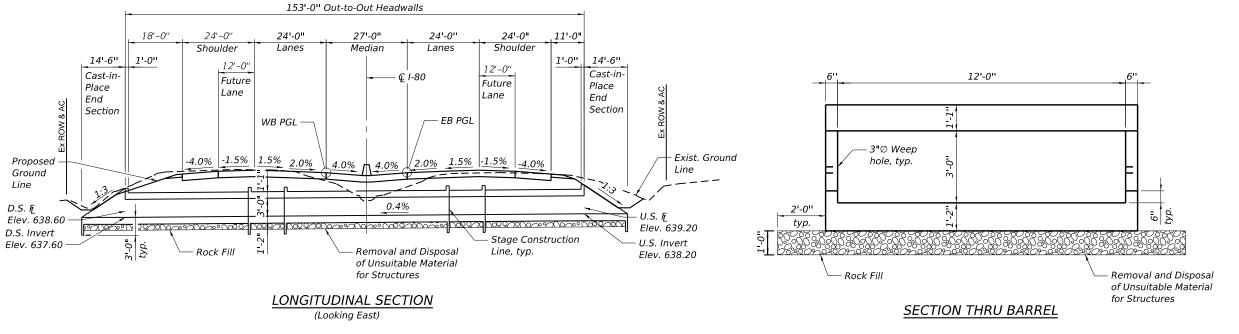
### **SOIL BORING LOG**

Page  $\underline{1}$  of  $\underline{1}$ 

**Date** 12/9/21

ROUTE	-	DE	SCR	IPTIOI	N		I-80 Phase II		L0	OGG	ED BY	<u>L</u>	.Р
SECTION	13		_	LOCA	TION _	SE 1/4	l, SEC. 13, TWP. T35N, I ing 1765408.2, Easting	RNG. R10E, 1068830.7	3 <sup>rd</sup> PM	,			
COUNTY	Vill D	RILLING	ME	THOD			low Stem Auger			(	CME A	utoma	tic
STRUCT. NO Station BORING NO Station Offset Ground Surface E	CB-003 863+56 63.5 ft Right		D E P T H	B L O W S	U C S Qu (tsf)	M O I S T	Surface Water Elev. Stream Bed Elev.  Groundwater Elev.: First Encounter Upon Completion After Hrs.	Drv	ft	D E P T H	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)
8.0" ASPHALT		643.33	l				End Of Boring @ -20.0	'. Boring					
CLAY LOAM-brown	n-very stiff (Fill)			5			backfilled with cuttings						
				3	3.90	19				_			
				5	В	'							
		641.00											
TOPSOIL-black			_	5						_			
				5	1.25	34							
			-5	5	В					-25			
CLAVI OAM brown	om. otiff	638.50	_							_			
CLAY LOAM-brown	i-very suii			4									
				6	2.20	25							
			_	5	В					_			
			_	3						_			
				5	2.20	26							
			-10	5	В					-30			
			_							_			
				4									
				6 8	3.10 B	22							
			_		В					_			
				5	0.00	00							
			-15	10 12	2.20 B	23				-35			
		628.50	-13							55			
SILTY CLAY-gray-v	ery stiff			]									
			_	8	3.00	25				_			
				7	3.00 P	23							
		626.00											
CLAY-gray-very stif	f												
				3	2.00	23							
		624 00	-20	5	P					<u>-40</u>			

### APPENDIX E CULVERT CROSS-SECTION



LOADING HL-93

Allow 50#/sq.ft. for future wearing surface.

### **DESIGN SPECIFICATIONS**

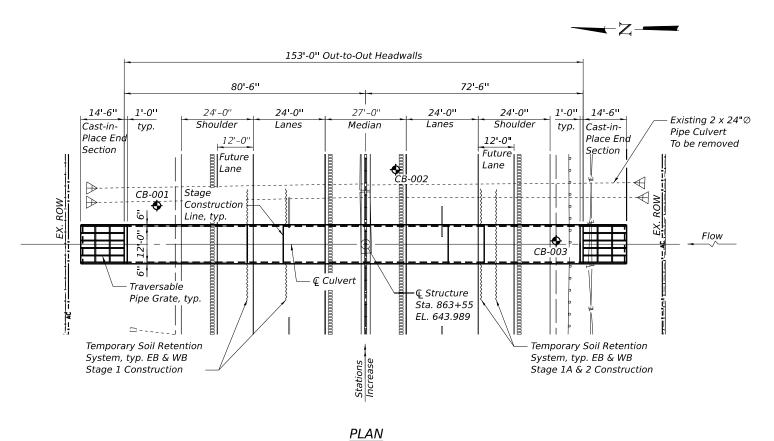
2020 AASHTO LRDF Bridge Design Specifications, 9th Edition

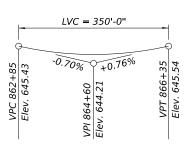
### **DESIGN STRESSES**

FIELD UNITS

fc = 3500 psi

fy = 60000 psi (Reinforcement)





### PROPOSED PROFILE GRADE

(Along I-80 EB & WB PGL)

GENERAL PLAN AND ELEVATION F.A.I ROUTE 80 WILL COUNTY **STATION 863+55** 

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### **GENERAL NOTES**

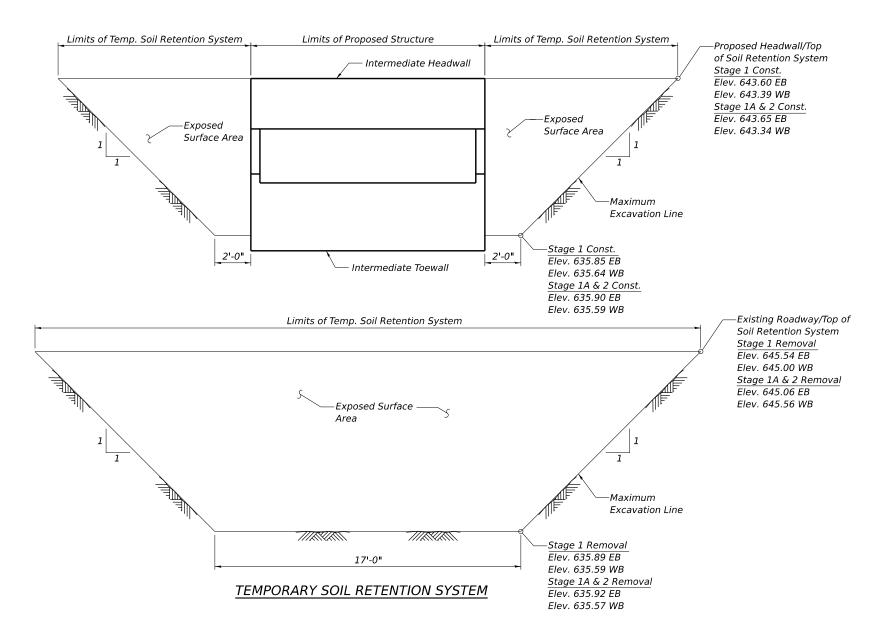
- 1. Reinforcement bars designated (E) shall be epoxy coated.
- Slopewall shall be reinforced with welded wire fabric, 6" x 6" W4.0 x W4.0, weighing 58 lbs. per 100 sq. ft.
- Protective coat shall not be applied to surfaces to which Waterproofing Membrane System is applied.
- Precast alternate is not allowed.
- See Civil Plans for additional information.
- Geocomposite Wall Drain shall be according to Section 591 of Standard Specification, except that concrete nails shall not be used in areas where it overlaps Membrane Waterproofing System for Buried Structures.

### **INDEX OF SHEETS**

- 1. General Plan
- 2. General Notes, Index of Sheets and Total Bill of Materials
- Stage Construction Details
- 4-5. Culvert Plan and Details
- Transversable Pipe Grate Details for End Section
- Bar Splicer Assembly and Mechanical Splicer Details
- Soil Boring Logs

### TOTAL BILL OF MATERIAL

ITEM	UNIT	TOTAL
Porous Granular Embankment	Cu. Yd.	275
Removal and Disposal of Unsuitable Material for Structures	Cu. Yd.	96
Reinforced Bars, Epoxy Coated	Pound	47,760
Bar Splicers	Each	144
Temporary Soil Retention System	Sq. Ft.	1380
Concrete Box Culverts	Cu. Yd.	180.9
Traversable Pipe Grate for Concrete End Section	Foot	54
Rock Fill	Cu. Yd.	96



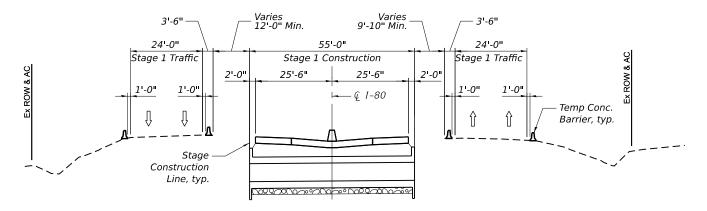
### NOTE:

A cantilevered sheet piling design does not appear feasible and additional members or other retention systems may be necessary. The Contractor shall submit a temporary soil retention system design including plan details and calculations for review and acceptance by the Engineer.

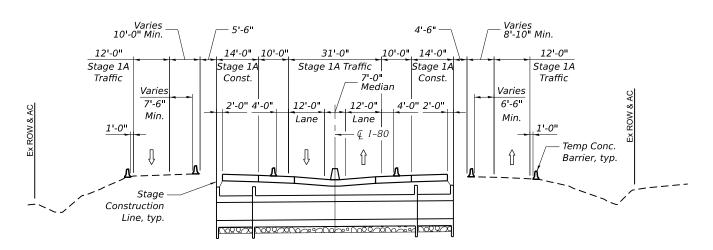
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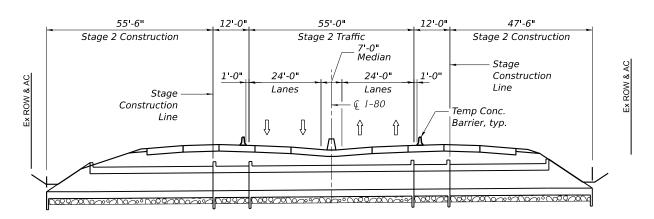
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 



STAGE 1 CONSTRUCTION



STAGE 1A CONSTRUCTION

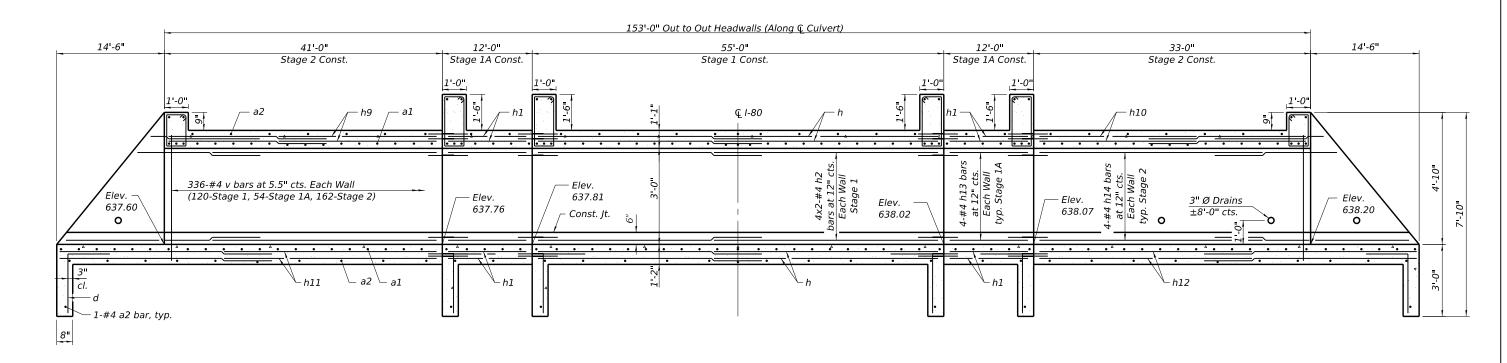


STAGE 2 CONSTRUCTION

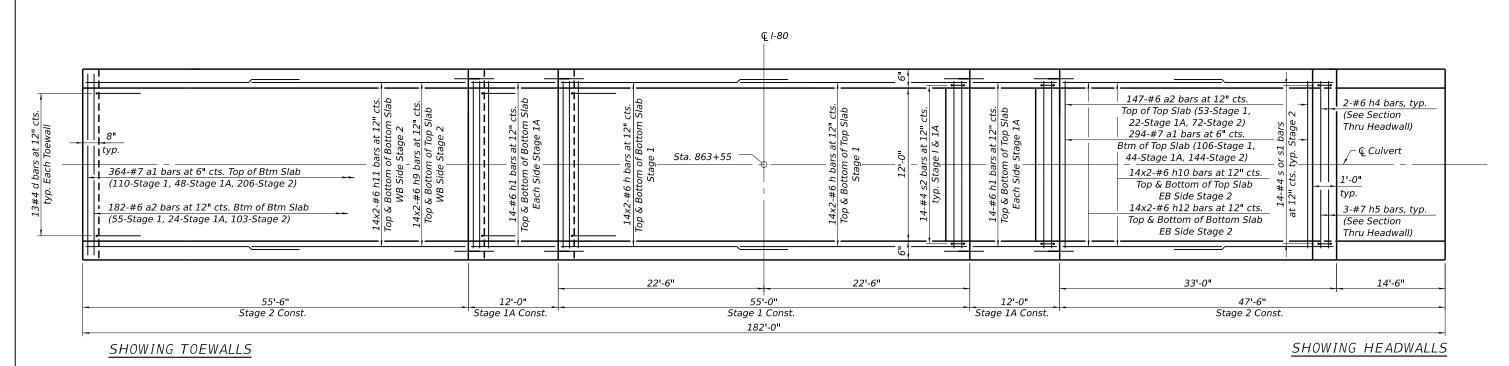
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

STAGE CONSTRUCTION DETAILS			TOTAL SHEETS	SHEET NO.	
BOX CULVERT AT 863+55	80	FAI 80 21 STRUCTURE 8 WILL 76		766	293
BOX COLVENT AT 605155			CONTRA	CT NO.	52R29
SHEET S-3 OF S-8 SHEETS		ILLINOIS FED. A	ID PROJECT		



### LONGITUDINAL SECTION



<u>PLAN</u>

A distance of half the length of the wingwall but not less than six feet of the barrel shall be poured monolithically with the wingwalls.

Bars indicated thus 12 x 4-#5 etc. indicates 12 lines of bars with 4 lengths per line. See Culvert Details (2 of 2) for End Section Reinforcement, Details, and Bill of Materials.

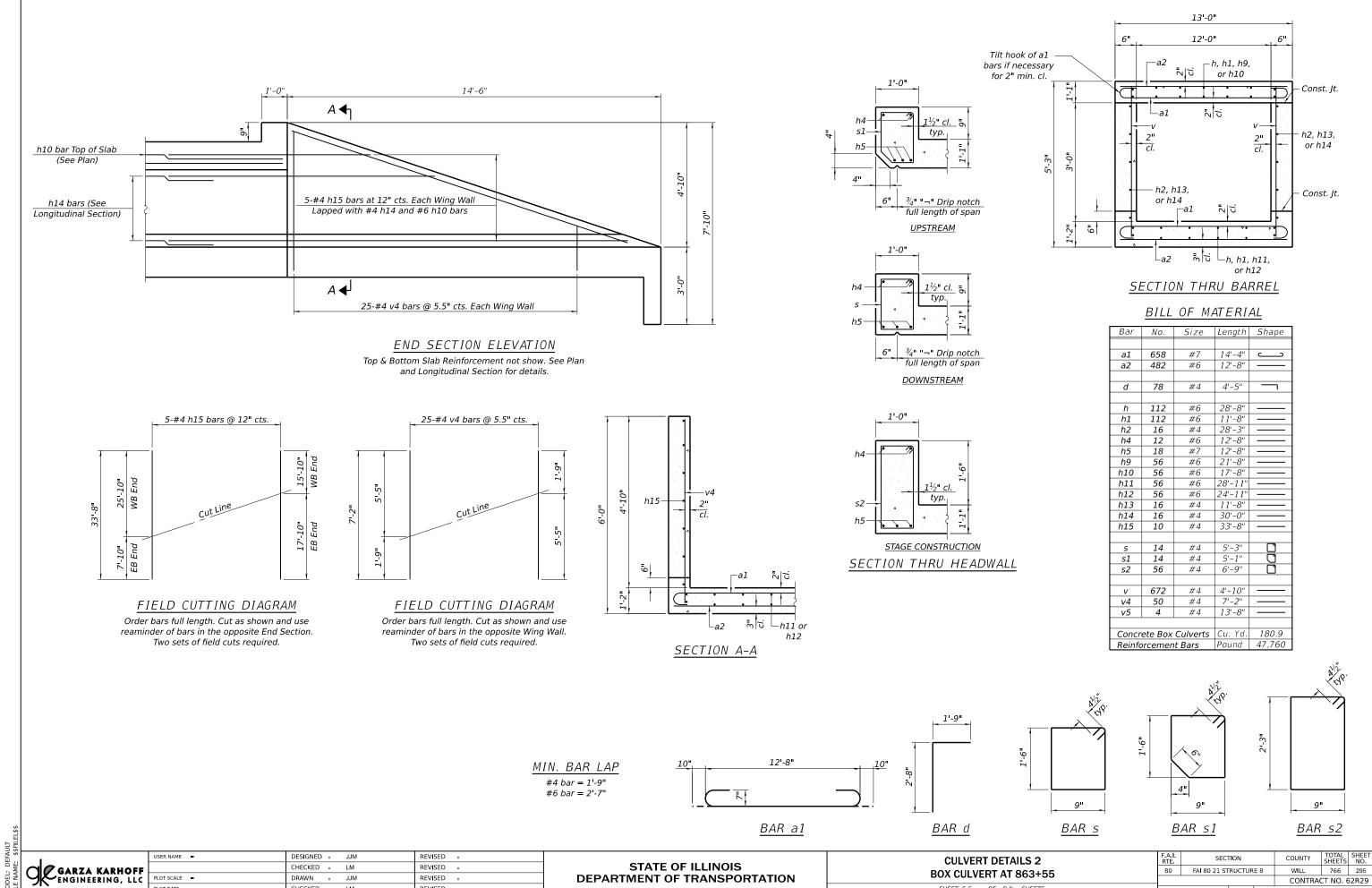
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**STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION** 

**CULVERT DETAILS 1** 80 **BOX CULVERT AT 863+55** SHEET S-4 OF S-8 SHEETS

TOTAL SHEET SHEETS NO. SECTION FAI 80 21 STRUCTURE 8 WILL 766 294 CONTRACT NO. 62R29



SHEET S-5 OF S-8 SHEETS

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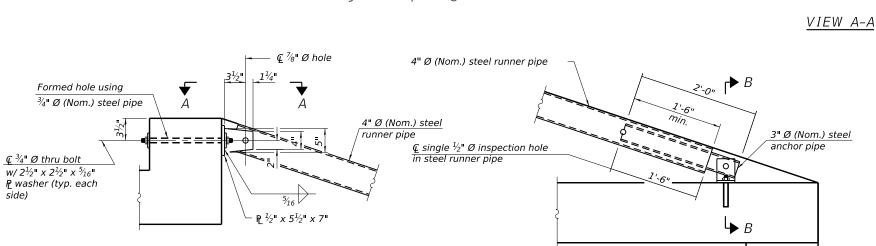
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# See Detail A

## Steel pipes at 2'-6" cts.

PLAN VIEW Number and Length of Main Pipes is 4 @ 13'-5"

LONGITUDINAL SECTION

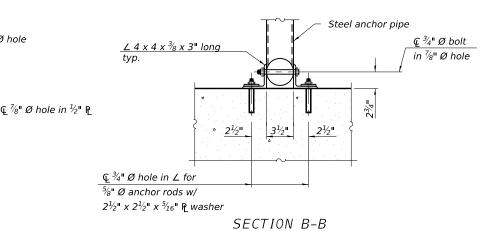


See Detail B

### GENERAL NOTES

The minimum edge distance from the center of a hole to the free edge of a structural shape or plate shall be  $1\frac{1}{2}$ " unless noted otherwise.

The Contractor may install the thru bolts using drilling and grouting in lieu of providing a formed hole using steel pipe. Installation shall be in accordance with Article 509.06 using a method that results in the annulus surrounding the bolt being completed filled with adhesive. The method of drilling shall not result in spalled concrete at the exit face. Epoxy grouted thru bolts shall be snug tightened followed by an additional  $\frac{1}{3}$  turn on the interior nut at final installation. Cost included with Traversable Pipe Grate.



### BILL OF MATERIAL

ITEM	UNIT	TOTAL
Traversable Pipe Grate for Concrete End Section	Foot	54

GARZA KARHOFF ENGINEERING, LLC

DETAIL A

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STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

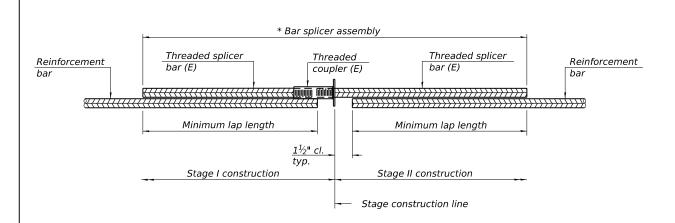
DETAIL B

TRAVERSABLE PIPE GRATE DETAILS FOR END SECTION **BOX CULVERT AT 863+55** SHEET S-6 OF S-8 SHEETS

-  $\mathbb{Q}^{3}_{4}$ " Ø bolt in  $^{7}_{8}$ " Ø hole

₽ ½", typ.

TOTAL SHEET NO. SECTION 80 FAI 80 21 STRUCTURE 8 WILL 766 296 CONTRACT NO. 62R29



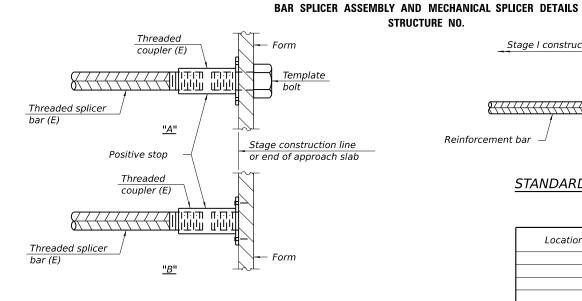
### STANDARD BAR SPLICER ASSEMBLY PLAN

(All components shall be provided from one supplier)

Threaded splicer bar length = min. lap length +  $1\frac{1}{2}$ " + thread length

\* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

I a aabian	Bar	No. assemblies	Minimum
Location	size	required	lap length
Barrel Stage 1	#6	56	2'-7"
Barrel Stage 1	#4	16	1'-9"
Barrel Stage 1A	#6	56	2'-7"
Barrel Stage 1A	#4	16	1'-9"



### INSTALLATION AND SETTING METHODS

"A": Set bar splicer assembly by means of a template bolt.

"B": Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E): Indicates epoxy coating.

### Stage line if applicable Stage I construction Stage II construction Mechanical splicer (E) Reinforcement bar – Reinforcement bar STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required

Notes:

Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.

All reinforcement shall be lapped and tied to the splicer bars. Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications. See approved list of bar splicer assemblies and mechanical splicers for alternatives.

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STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**  BAR SPLICER ASSEMBLY AND MECHANICAL SPLICER DETAILS **BOX CULVERT AT 863+55** SHEET S-7 OF S-8 SHEETS

SECTION COUNTY FAI 80 21 STRUCTURE 8 80 WILL 766 297 CONTRACT NO. 62R29

## **APPENDIX F** Long-term Geotechnical Parameters for Design Sheet Pile/Soldier Pile Walls

**Table 1- Long-term Geotechnical Parameters for Design** 

Material Description	Depth Below Ground Surface (ft)	Soil Unit Weight γ (pcf)	Active Earth Pressure Coeff (K <sub>a)</sub>	Passive Earth Pressur e Coeff (K <sub>P</sub> )	Undrained Cohesion "c" (psf)	Lateral Modulus of Subgrade Reaction (pci)	Strain	Long term (drained) Cohesion "c" (pcf)	Long term (drained) Fraction Angle (°)
Stiff to Very Stiff Silty Clay & Clay Loam	1 to 10	120	0.361	2.770	1700	555	0.0068	0	28
Very stiff Clay	10 to 20	120	0.361	2.770	2300	770	0.0059	0	28