

FILE NAME =	DESIGNED - GRR	REVISED -
...\\D160W75-sh1-155-pr1light-NB-14.dgn	DRAWN - JLW	REVISED -
USER NAME = jworthington	CHECKED - GHT	REVISED -
PLOT DATE = 6/17/2015	DATE - 6/12/2015	REVISED -



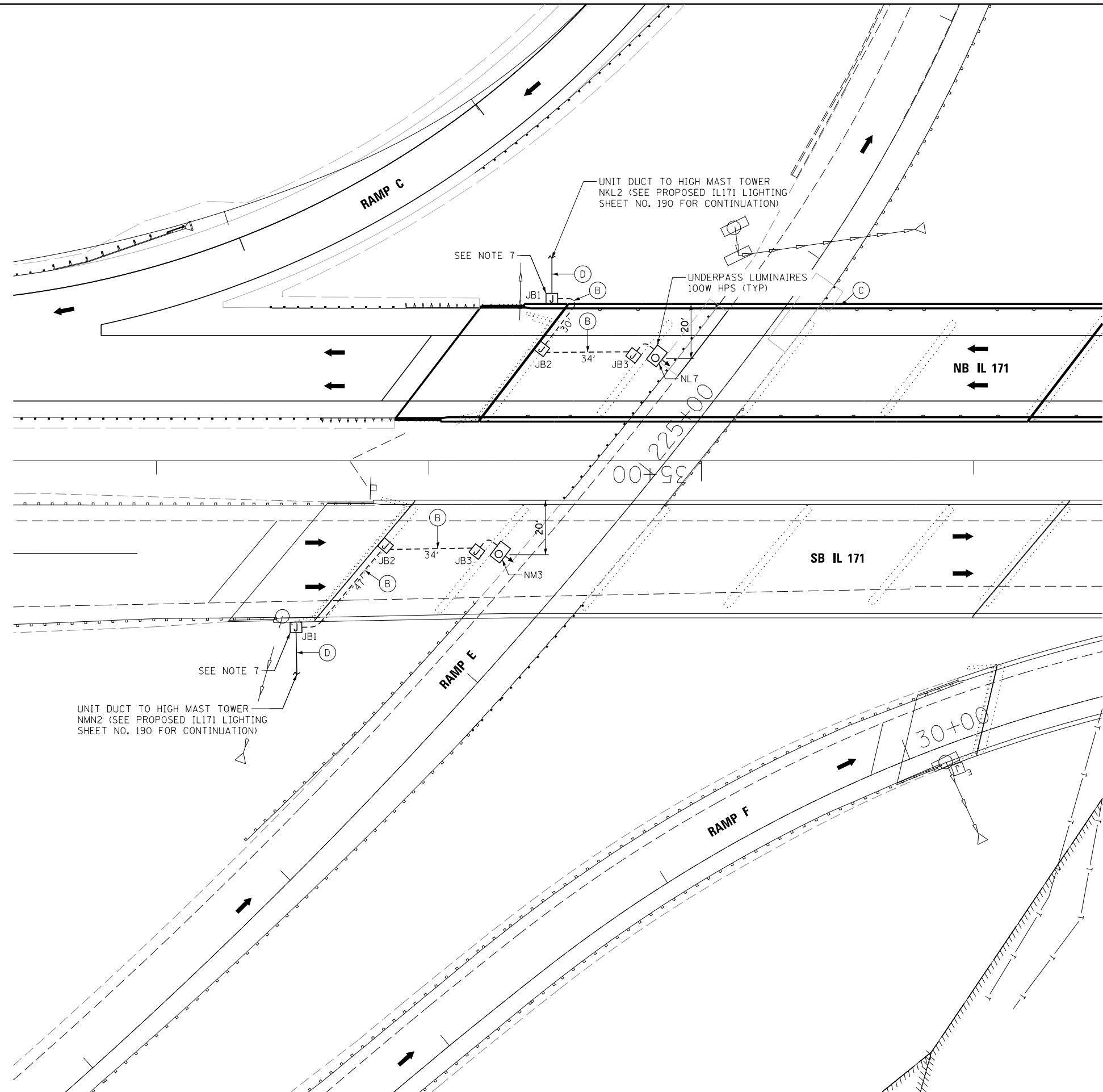
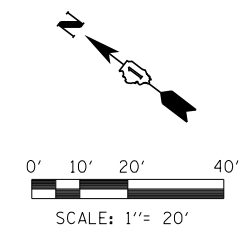
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**PROPOSED LIGHTING PLAN
I-55**

SCALE: 1"=50' SHEET 31 OF 61 SHEETS STA. TO STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372/373	2013-037B-R	COOK	787	201
CONTRACT NO. 60W75				

ILLINOIS FED. AID PROJECT



LEGEND

- (A) CONDUIT ATTACHED TO STRUCTURE, 1" DIA. PVCC RGC WITH 3-1/2 NO. 10 AND 1/2 NO. 10 GND.
 - (B) CONDUIT ATTACHED TO STRUCTURE, 1" DIA. PVCC RGC WITH 2-1/2 NO. 10 AND 1/2 NO. 10 GND.
 - (C) CONDUIT EMBEDDED IN STRUCTURE, 2" DIA. PVC WITH 3-1/2 NO. 2 AND 1/2 NO. 4 GND.
 - (D) CONDUIT ATTACHED TO STRUCTURE, 2 1/2 " DIA. PVCC RGC WITH UNIT DUCT AS NOTED ON PLANS (FOR TRANSITION TO UNDERGROUND) - 10'
- JB1 JUNCTION BOX, STAINLESS STEEL, 18"x12"x6", ATTACHED TO PARAPET
- JB2 JUNCTION BOX, STAINLESS STEEL, 12"x12"x6", ATTACHED TO ABUTMENT
- JB3 JUNCTION BOX, STAINLESS STEEL, 6"x6"x4", ATTACHED TO TOP OF PIER CAP

NOTES:

1. UNDERPASS LUMINAIRES SHALL BE MOUNTED TO THE FACE OF PIERS AT A MOUNTING HEIGHT OF APPROXIMATELY 15'.
2. THE DISTANCES SHOWN BETWEEN THE UNDERPASS LUMINAIRE IS MEASURED FROM CENTER OF LUMINAIRES.
3. THE CABLE SPLICES AT THE JUNCTION BOX SHALL BE DONE ACCORDING TO THE SPLICING DETAIL SHOWN ON IDOT-D1 STANDARD DETAIL DRAWING NO. BE-702 PROVIDE DOUBLE POLE FUSE HOLDERS WITH 5 AMP FUSE AND SOLID NEUTRAL.
4. UNDERPASS LUMINAIRES SHALL BE INSTALLED AS SHOWN ON IDOT-D1 STANDARDS DRAWING NO. BE-902. SEE THIS DETAIL FOR FURTHER INFORMATION ON CONDUIT ROUTING AND INSTALLATION.
5. AT ALL LOCATIONS WHERE EXPANSION/DEFLECTION CAN OCCUR IN CONDUIT ATTACHED TO STRUCTURE AND FOR CONNECTION TO LUMINAIRES THE CONTRACTOR SHALL UTILIZE LIQUID TIGHT FLEXIBLE NONMETALLIC CONDUIT.
6. SEE LIGHTING PLAN SHEET FOR CONDUIT EMBEDDED IN PARAPET WALL AND ASSOCIATED CABLING.
7. CONTRACTOR SHALL PROVIDE AND INSTALL TWO POLE WEATHERPROOF FUSE HOLDERS INSIDE THE FIRST UNDERPASS LIGHTING JUNCTION BOX. PROVIDE 10A FUSE AND SOLID NEUTRAL FOR EACH FUSE HOLDER.

FILE NAME =	DESIGNED - GRR	REVISED -
... \D160W75-sh-t-IL171-underpass-RampE.dgn	DRAWN - JLW	REVISED -
USER NAME = jworthington	CHECKED - GHT	REVISED -
PLOT DATE = 8/31/2015	DATE - 6/12/2015	REVISED -



STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

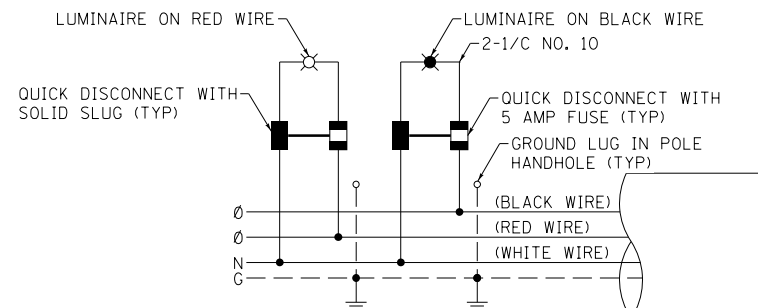
**UNDERPASS LIGHTING PLAN
RAMP E UNDER IL171**

SCALE: 1"=20' SHEET 32 OF 61 SHEETS STA. TO STA.

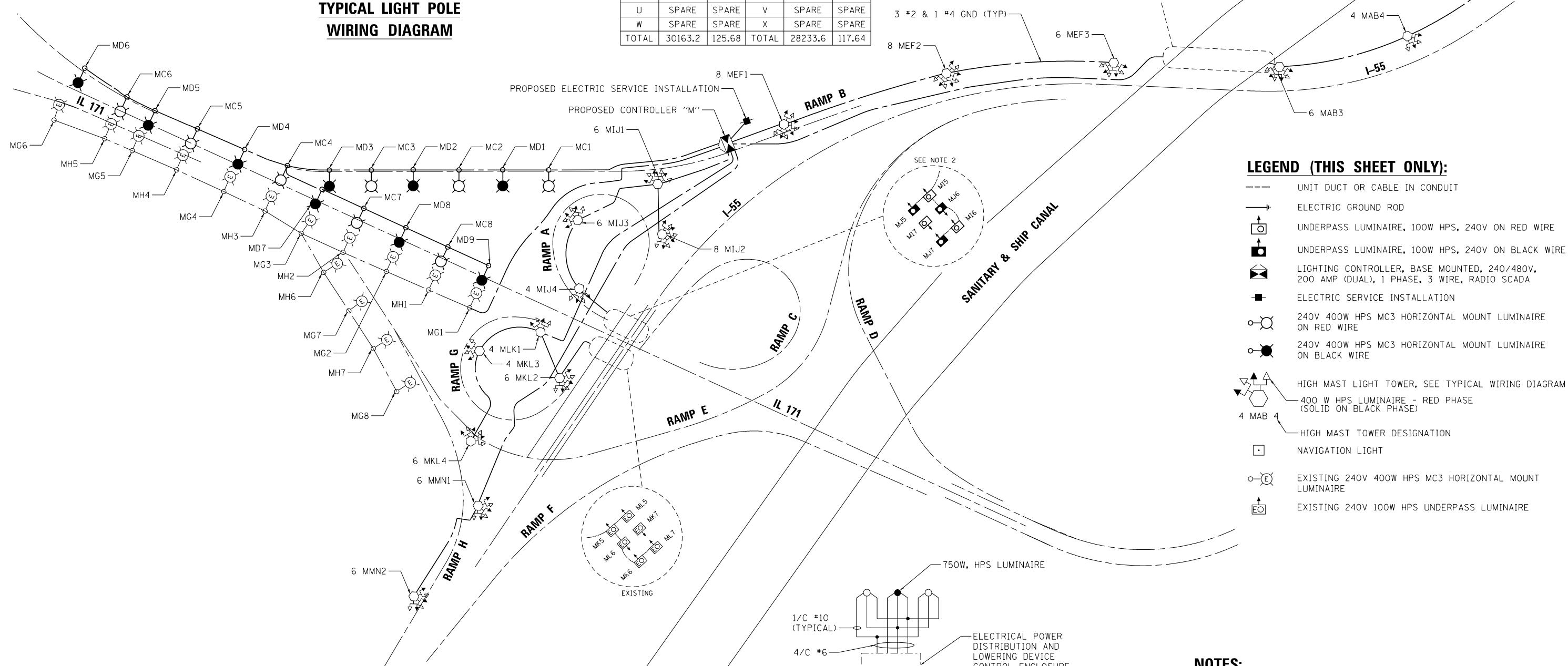
F.A.P. R/E.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372/373	2013-037B-R	COOK	787	202
CONTRACT NO. 60W75			ILLINOIS FED. AID PROJECT	

LOAD TABLE

IDOT LIGHTING CONTROLLER "M" 240/480V					
RED PHASE	WATTS	AMPS	BLACK PHASE	WATTS	AMPS
A	3379.2	14.08	B	3369.6	14.04
C	4320	18.0	D	3360	14.0
E	5280	22.0	F	5280	22.0
G	3840	16.0	H	2880	12.0
I	6192	25.8	J	6192	25.8
K	4272	17.8	L	4272	17.8
M	2880	12.0	N	2880	12.0
O	SPARE	SPARE	P	SPARE	SPARE
Q	SPARE	SPARE	R	SPARE	SPARE
S	SPARE	SPARE	T	SPARE	SPARE
U	SPARE	SPARE	V	SPARE	SPARE
W	SPARE	SPARE	X	SPARE	SPARE
TOTAL	30163.2	125.68	TOTAL	28233.6	117.64

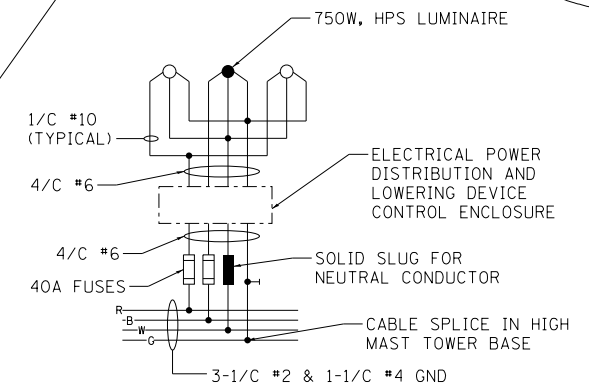


TYPICAL LIGHT POLE WIRING DIAGRAM



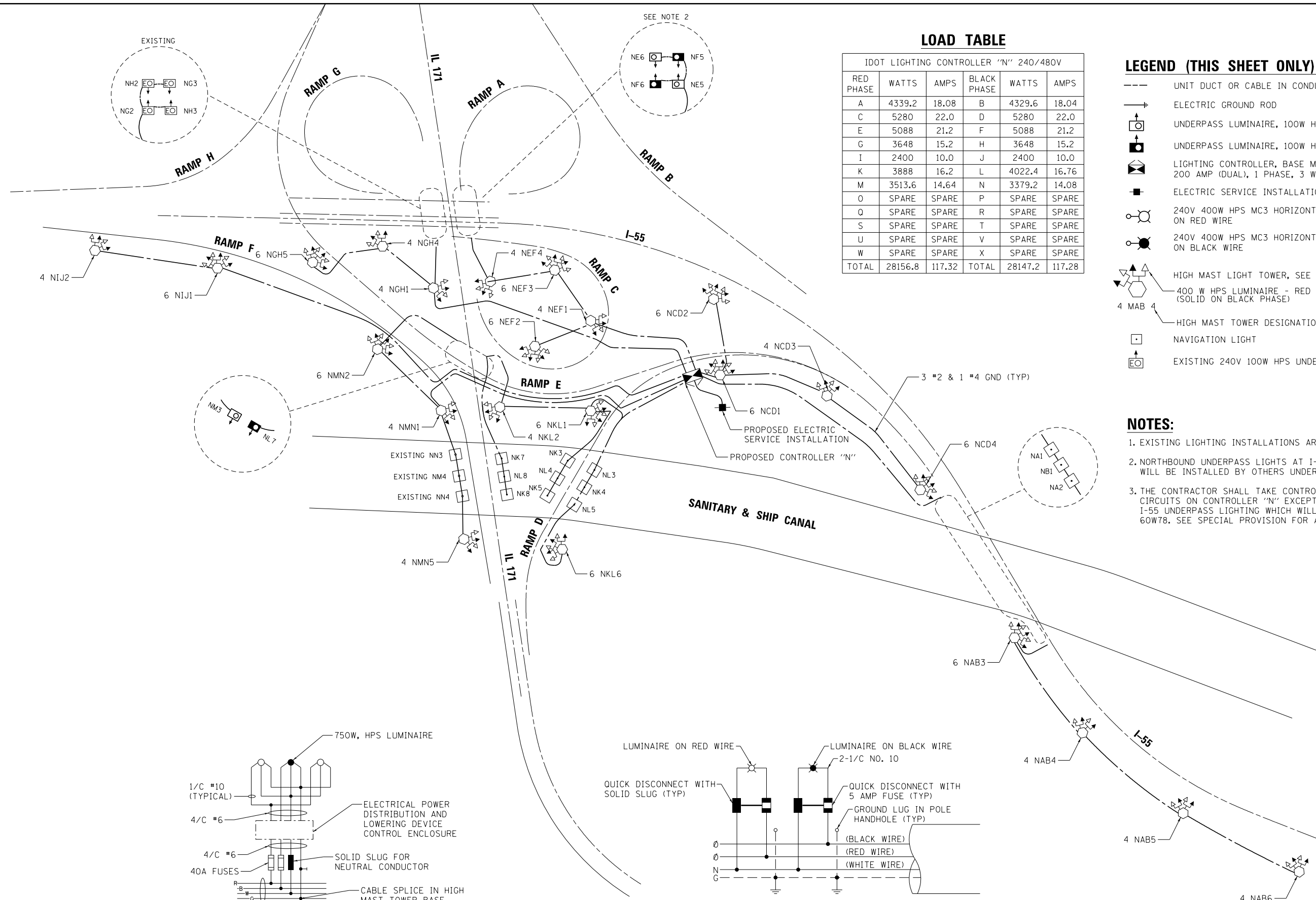
LEGEND (THIS SHEET ONLY):

- UNIT DUCT OR CABLE IN CONDUIT
- ⊥ ELECTRIC GROUND ROD
- ⬆ UNDERPASS LUMINAIRE, 100W HPS, 240V ON RED WIRE
- ⬆ UNDERPASS LUMINAIRE, 100W HPS, 240V ON BLACK WIRE
- ⬆ LIGHTING CONTROLLER, BASE MOUNTED, 240/480V, 200 AMP (DUAL), 1 PHASE, 3 WIRE, RADIO SCADA
- ⬆ ELECTRIC SERVICE INSTALLATION
- ⊙ 240V 400W HPS MC3 HORIZONTAL MOUNT LUMINAIRE ON RED WIRE
- ⊙ 240V 400W HPS MC3 HORIZONTAL MOUNT LUMINAIRE ON BLACK WIRE
- ⬆ HIGH MAST LIGHT TOWER, SEE TYPICAL WIRING DIAGRAM
- ⬆ 400 W HPS LUMINAIRE - RED PHASE (SOLID ON BLACK PHASE)
- ⬆ HIGH MAST TOWER DESIGNATION
- ⊠ NAVIGATION LIGHT
- ⊙ EXISTING 240V 400W HPS MC3 HORIZONTAL MOUNT LUMINAIRE
- ⊙ EXISTING 240V 100W HPS UNDERPASS LUMINAIRE



TYPICAL HIGH MAST TOWER WIRING DIAGRAM

- NOTES:**
- EXISTING LIGHTING INSTALLATIONS ARE "FOR INFORMATION ONLY".
 - NORTHBOUND UNDERPASS LIGHTS, SHOWN ARE PROPOSED AND WILL BE INSTALLED BY OTHERS UNDER CONTRACT 60W78.
 - THE CONTRACTOR SHALL TAKE CONTROL AND MAINTAIN ALL LIGHTING CIRCUITS ON CONTROLLER "M" EXCEPT FOR THE EXISTING AND PROPOSED I-55 UNDERPASS LIGHTING WHICH WILL BE MAINTAINED BY CONTRACT 60W78. SEE SPECIAL PROVISION FOR ADDITIONAL INFORMATION.



LOAD TABLE

IDOT LIGHTING CONTROLLER "N" 240/480V					
RED PHASE	WATTS	AMPS	BLACK PHASE	WATTS	AMPS
A	4339.2	18.08	B	4329.6	18.04
C	5280	22.0	D	5280	22.0
E	5088	21.2	F	5088	21.2
G	3648	15.2	H	3648	15.2
I	2400	10.0	J	2400	10.0
K	3888	16.2	L	4022.4	16.76
M	3513.6	14.64	N	3379.2	14.08
O	SPARE	SPARE	P	SPARE	SPARE
Q	SPARE	SPARE	R	SPARE	SPARE
S	SPARE	SPARE	T	SPARE	SPARE
U	SPARE	SPARE	V	SPARE	SPARE
W	SPARE	SPARE	X	SPARE	SPARE
TOTAL	28156.8	117.32	TOTAL	28147.2	117.28

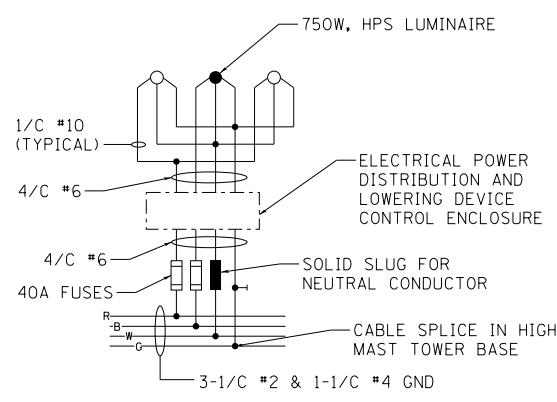
LEGEND (THIS SHEET ONLY):

- UNIT DUCT OR CABLE IN CONDUIT
- ⊥ ELECTRIC GROUND ROD
- ⊕ UNDERPASS LUMINAIRE, 100W HPS, 240V ON RED WIRE
- ⊕ UNDERPASS LUMINAIRE, 100W HPS, 240V ON BLACK WIRE
- ⊕ LIGHTING CONTROLLER, BASE MOUNTED, 240/480V, 200 AMP (DUAL), 1 PHASE, 3 WIRE, RADIO SCADA
- ⊕ ELECTRIC SERVICE INSTALLATION
- ⊕ 240V 400W HPS MC3 HORIZONTAL MOUNT LUMINAIRE ON RED WIRE
- ⊕ 240V 400W HPS MC3 HORIZONTAL MOUNT LUMINAIRE ON BLACK WIRE
- ⊕ HIGH MAST LIGHT TOWER, SEE TYPICAL WIRING DIAGRAM
- ⊕ 400 W HPS LUMINAIRE - RED PHASE (SOLID ON BLACK PHASE)
- 4 MAB 4 HIGH MAST TOWER DESIGNATION
- ⊕ NAVIGATION LIGHT
- ⊕ EXISTING 240V 100W HPS UNDERPASS LUMINAIRE

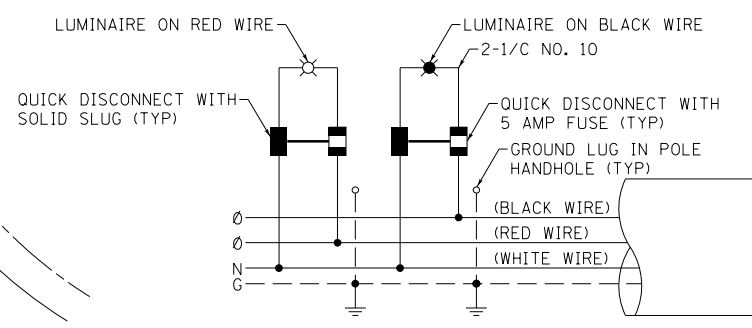
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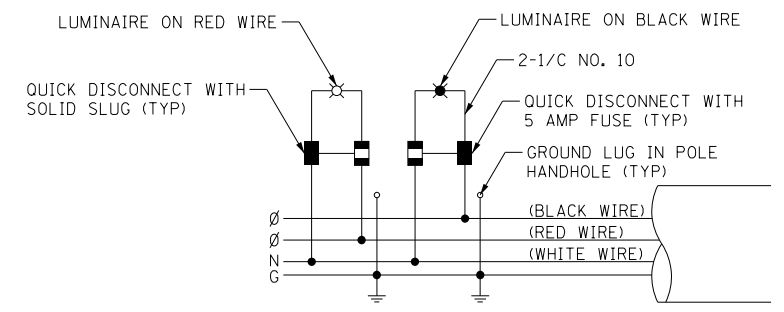
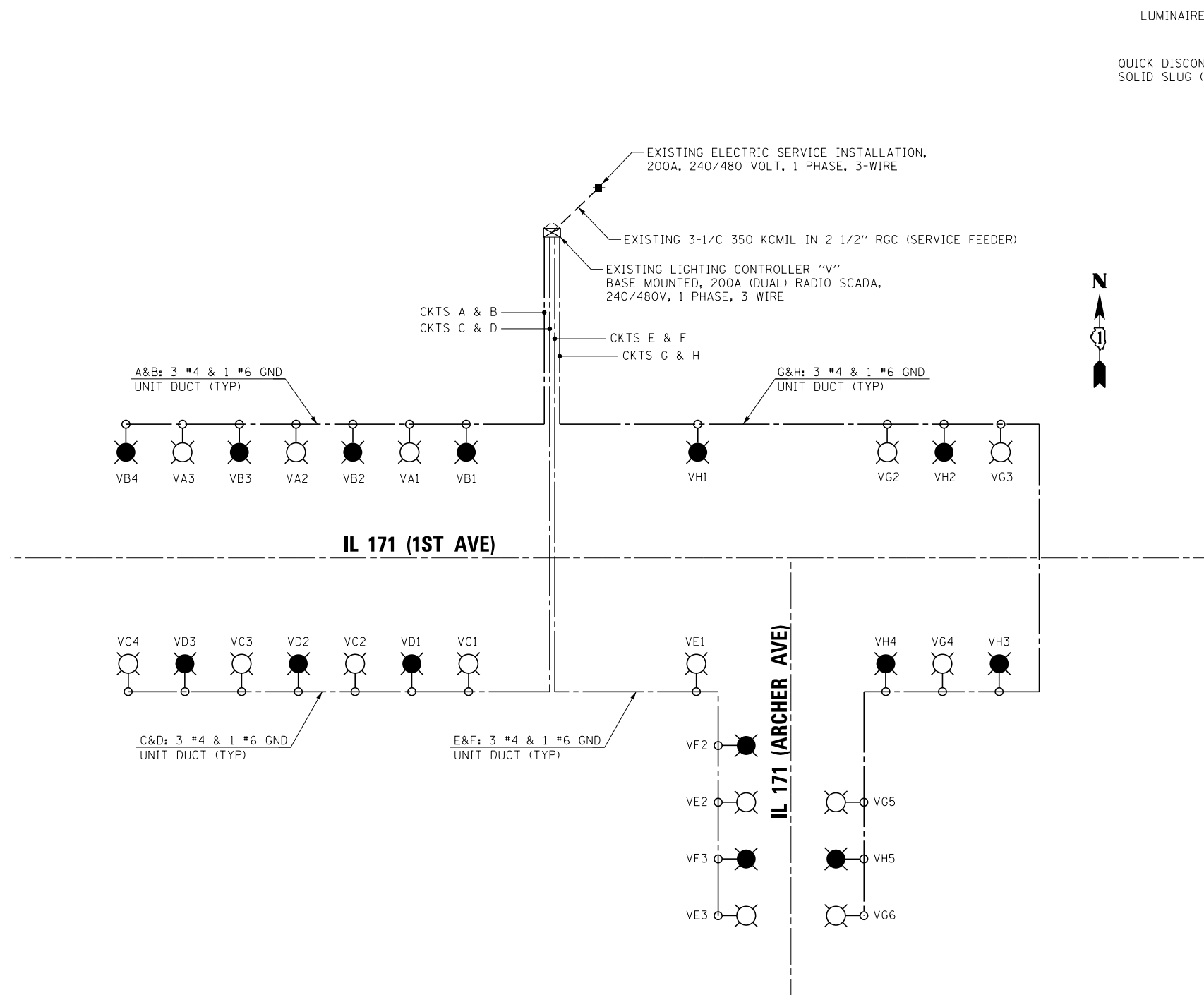
1. EXISTING LIGHTING INSTALLATIONS ARE "FOR INFORMATION ONLY".
2. NORTHBOUND UNDERPASS LIGHTS AT I-55 SHOWN ARE PROPOSED AND WILL BE INSTALLED BY OTHERS UNDER CONTRACT 60W78.
3. THE CONTRACTOR SHALL TAKE CONTROL AND MAINTAIN ALL LIGHTING CIRCUITS ON CONTROLLER "N" EXCEPT FOR THE EXISTING AND PROPOSED I-55 UNDERPASS LIGHTING WHICH WILL BE MAINTAINED BY CONTRACT 60W78. SEE SPECIAL PROVISION FOR ADDITIONAL INFORMATION.

TYPICAL HIGH MAST TOWER WIRING DIAGRAM



TYPICAL LIGHT POLE WIRING DIAGRAM





TYPICAL LIGHT POLE WIRING DIAGRAM

LOAD TABLE

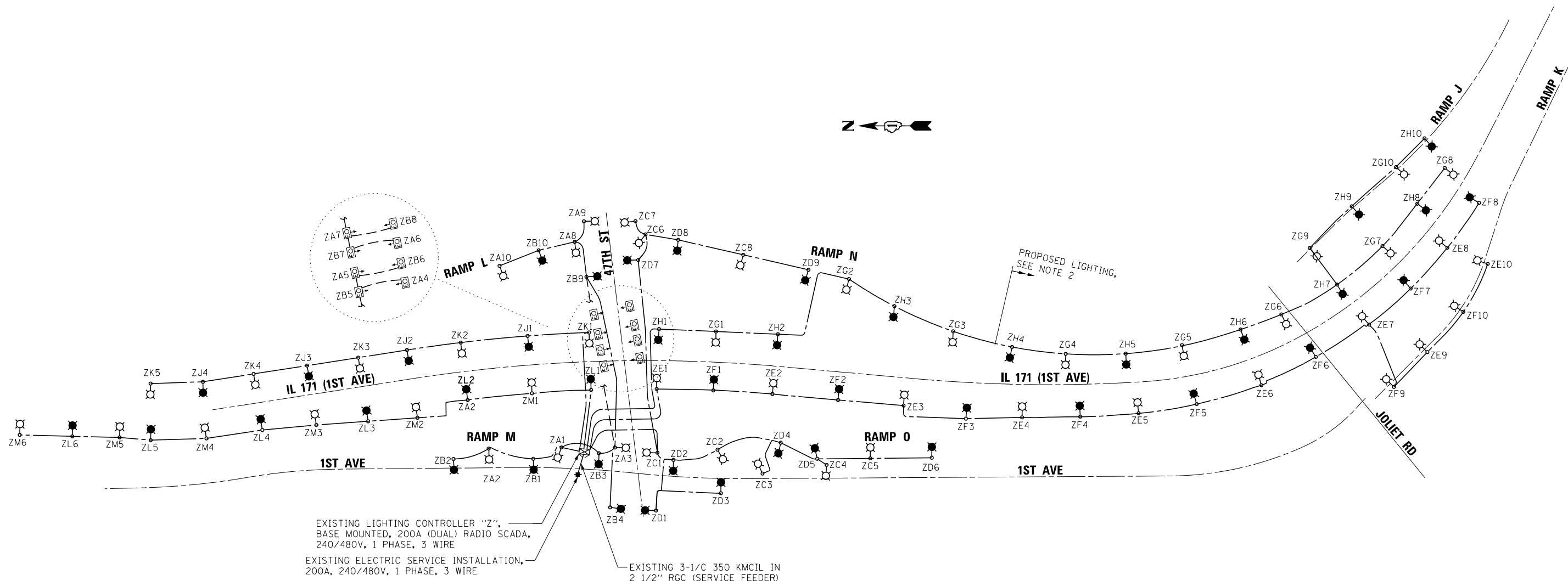
IDOT LIGHTING CONTROLLER "V" 240/480V					
RED PHASE	WATTS	AMPS	BLACK PHASE	WATTS	AMPS
A	1434	6.0	B	1912	8.0
C	1912	8.0	D	1434	6.0
E	1434	6.0	F	956	4.0
G	2390	10.0	H	2390	10.0
I	SPARE	SPARE	J	SPARE	SPARE
TOTAL	7170	30.0	TOTAL	6692	28.0

NOTES:

- EXISTING LIGHTING INSTALLATIONS ARE "FOR INFORMATION ONLY".
- SOUTHBOUND LIGHTS AND LIGHTS AT/EAST OF ARCHER AVE ARE EXISTING TO REMAIN.

LEGEND (THIS SHEET ONLY):

- UNIT DUCT OR CABLE IN CONDUIT
- ⊕ ELECTRIC GROUND ROD
- ⊗ EXISTING LIGHTING CONTROLLER, BASE MOUNTED, 240/480V, 200 AMP (DUAL) RADIO SCADA, 1 PHASE, 3 WIRE
- ⚡ ELECTRIC SERVICE INSTALLATION
- 240V 400W HPS MC3 HORIZONTAL MOUNT LUMINAIRE ON RED WIRE
- 240V 400W HPS MC3 HORIZONTAL MOUNT LUMINAIRE ON BLACK WIRE



EXISTING LIGHTING CONTROLLER "Z",
 BASE MOUNTED, 200A (DUAL) RADIO SCADA,
 240/480V, 1 PHASE, 3 WIRE

EXISTING ELECTRIC SERVICE INSTALLATION,
 200A, 240/480V, 1 PHASE, 3 WIRE

EXISTING 3-1/C 350 KMCIL IN
 2 1/2" RGC (SERVICE FEEDER)

PROPOSED LIGHTING,
 SEE NOTE 2

LEGEND

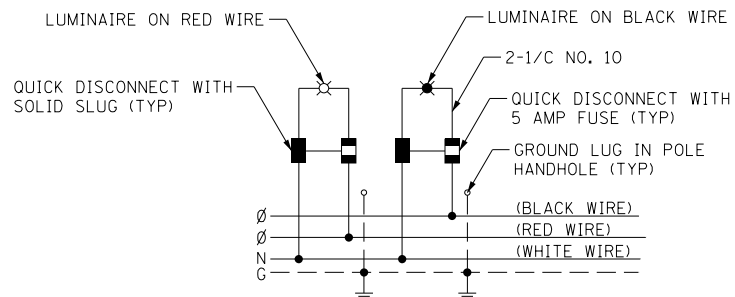
- UNIT DUCT OR CABLE IN CONDUIT
- ELECTRIC GROUND ROD
- ⊠ EXISTING LIGHTING CONTROLLER, BASE MOUNTED, 240/480V, 200 AMP (DUAL) RADIO SCADA, 1 PHASE, 3 WIRE
- ELECTRIC SERVICE INSTALLATION
- 240V 400W HPS MC3 HORIZONTAL MOUNT LUMINAIRE ON RED WIRE
- 240V 400W HPS MC3 HORIZONTAL MOUNT LUMINAIRE ON BLACK WIRE
- ⊕ UNDERPASS LUMINAIRE, 100W HPS, 240V ON RED WIRE
- ⊖ UNDERPASS LUMINAIRE, 100W HPS, 240V ON BLACK WIRE
- ⊞ EXISTING 240V 100W HPS UNDERPASS LUMINAIRE

LOAD TABLE

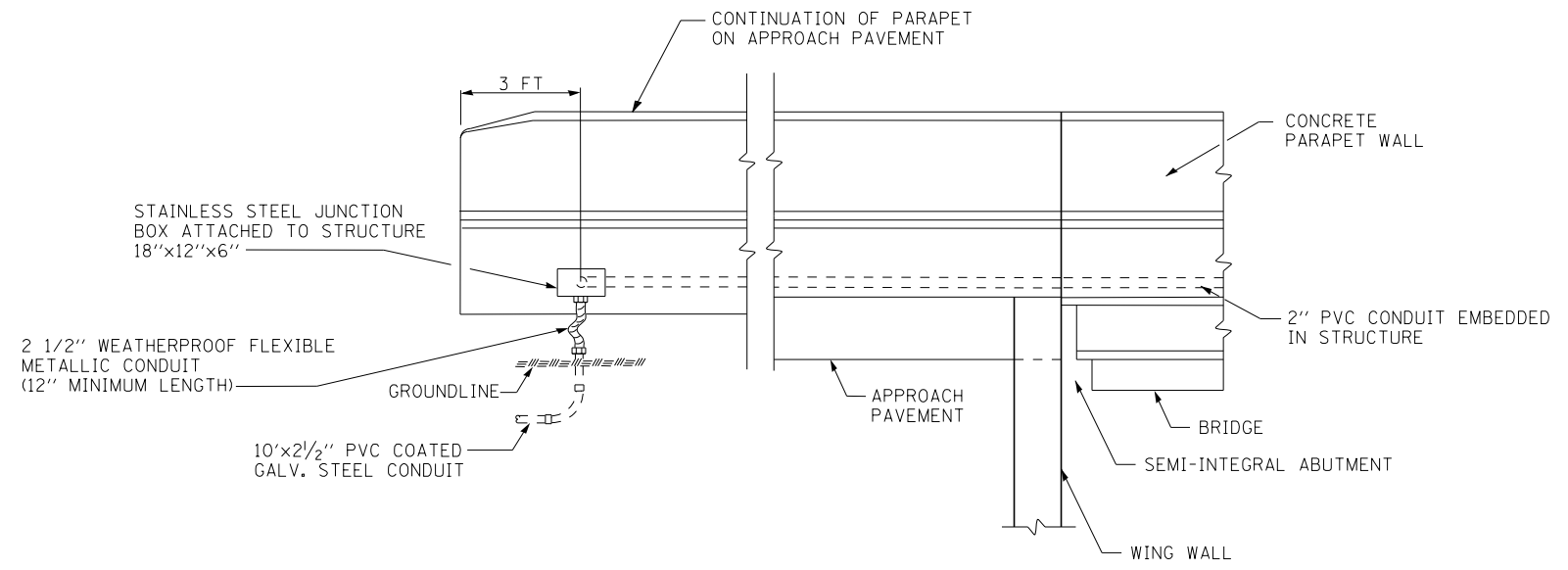
IDOT LIGHTING CONTROLLER "Z" 240/480V					
RED PHASE	WATTS	AMPS	BLACK PHASE	WATTS	AMPS
A	3348	14.0	B	3348	14.0
C	3824	15.9	D	4302	17.9
E	4780	19.9	F	4780	19.9
G	4780	19.9	H	4780	19.9
I	SPARE	SPARE	J	1912	8.0
K	2390	10.0	L	2868	12.0
M	2868	12.0	N	SPARE	SPARE
O	SPARE	SPARE	P	SPARE	SPARE
TOTAL	21990	91.7	TOTAL	21990	91.7

NOTES:

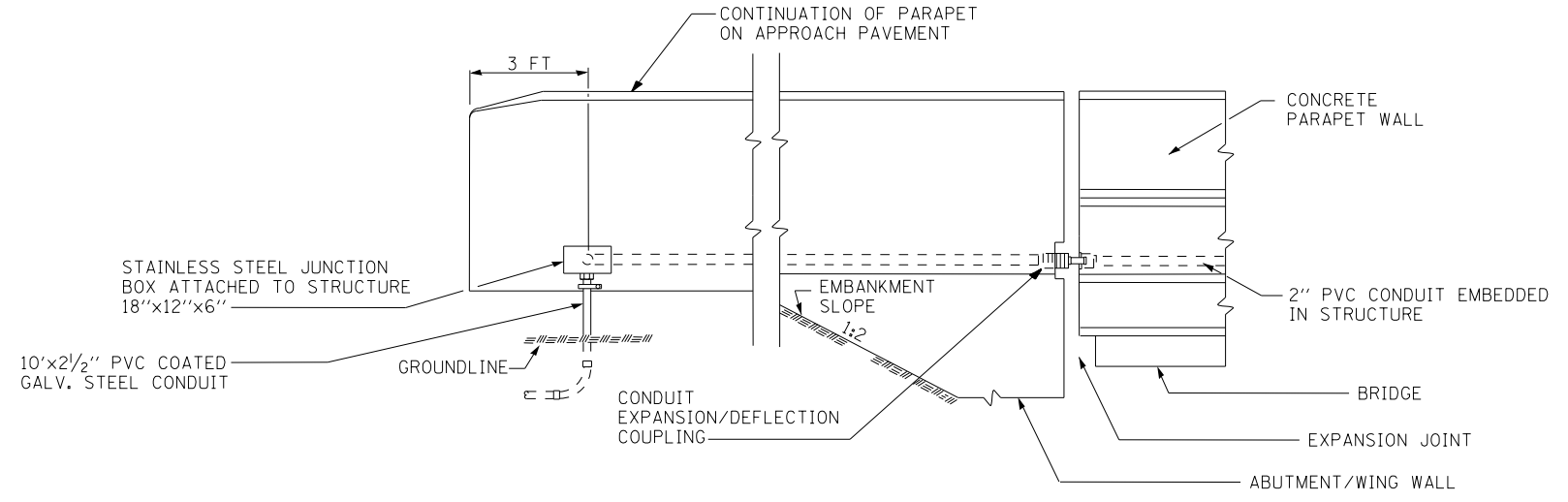
1. EXISTING LIGHTING INSTALLATIONS ARE "FOR INFORMATION ONLY".
2. SOUTHBOUND LIGHTS AND ALL LIGHTS AT THE 47TH STREET INTERCHANGE ARE EXISTING AND WILL REMAIN.



**TYPICAL LIGHT POLE
 WIRING DIAGRAM**



CONDUIT DETAIL
(SEMI-INTEGRAL ABUTMENT-SN 016-0489)



CONDUIT DETAIL
(OPEN ABUTMENT)

FILE NAME =	DESIGNED - GRR	REVISED -
... \D160W75-sh1-1L171-conduit-detail-NB-01.dgn	DRAWN - JLW	REVISED -
USER NAME = jworthington	CHECKED - GHT	REVISED -
PLOT DATE = 8/31/2015	DATE - 6/12/2015	REVISED -

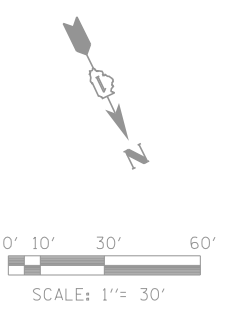
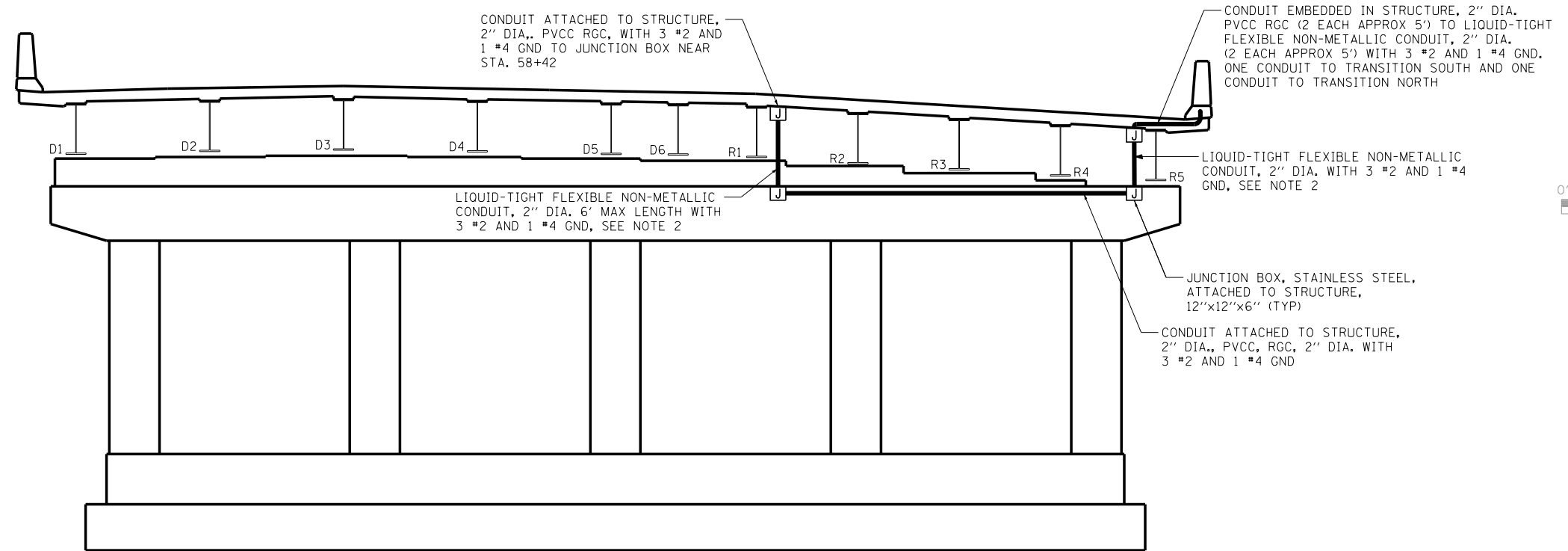


STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

CONDUIT TRANSITION DETAILS

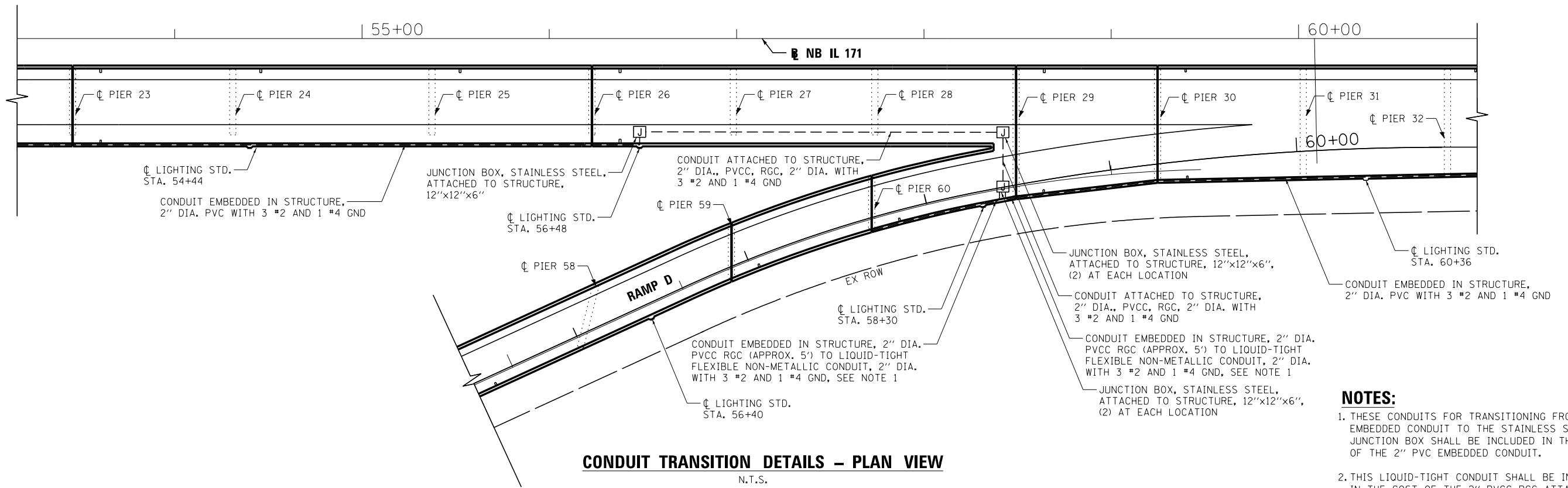
SCALE: N.T.S. SHEET 37 OF 61 SHEETS STA. TO STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372/373	2013-037B-R	COOK	787	207
CONTRACT NO. 60W75				
ILLINOIS FED. AID PROJECT				



CONDUIT TRANSITION DETAILS – ELEVATION AT PIER 29 –
CL BRG. PIER 29 – SN 016-2456


N.T.S.



CONDUIT TRANSITION DETAILS – PLAN VIEW

N.T.S.

- NOTES:**
1. THESE CONDUITS FOR TRANSITIONING FROM 2" PVC EMBEDDED CONDUIT TO THE STAINLESS STEEL JUNCTION BOX SHALL BE INCLUDED IN THE COST OF THE 2" PVC EMBEDDED CONDUIT.
 2. THIS LIQUID-TIGHT CONDUIT SHALL BE INCLUDED IN THE COST OF THE 2" PVCC RGC ATTACHED TO STRUCTURE.

FILE NAME =	DESIGNED - GRR	REVISED -		<p align="center">STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION</p>	CONDUIT TRANSITION DETAILS			F.A.P. RFE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
... \D160W75-sh1-IL171-conduit-detail-NB-02.dgn	DRAWN - JLW	REVISED -			372/373	2013-037B-R	COOK	787	208			
USER NAME = jworthington	CHECKED - GHT	REVISED -			CONTRACT NO. 60W75							
PLOT DATE = 8/31/2015	DATE - 6/12/2015	REVISED -			ILLINOIS FED. AID PROJECT							
				SCALE: N.T.S.	SHEET 38 OF 61 SHEETS	STA.	TO STA.					

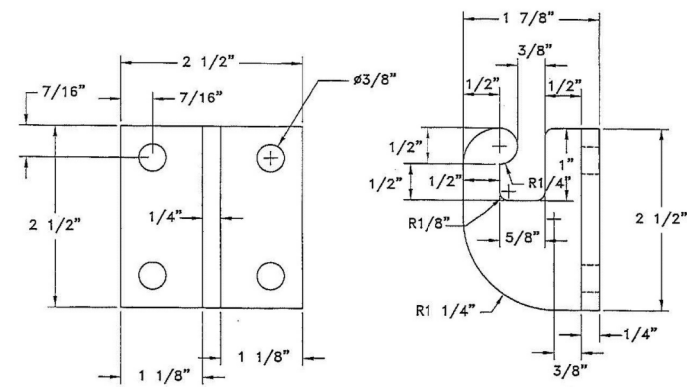


PLATE HOOK

ONE PER SWIVEL REQUIRED

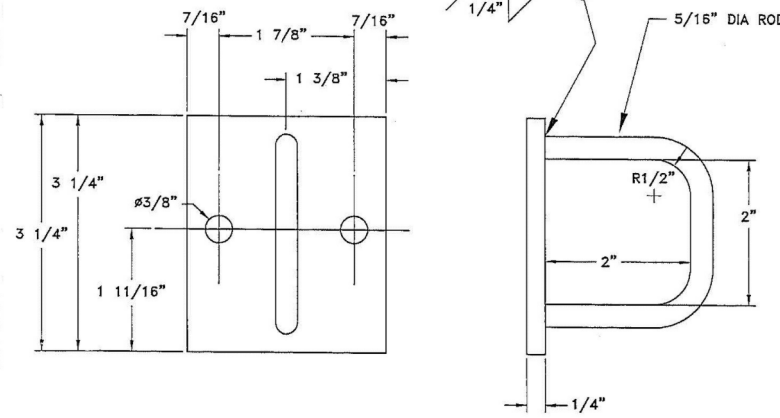
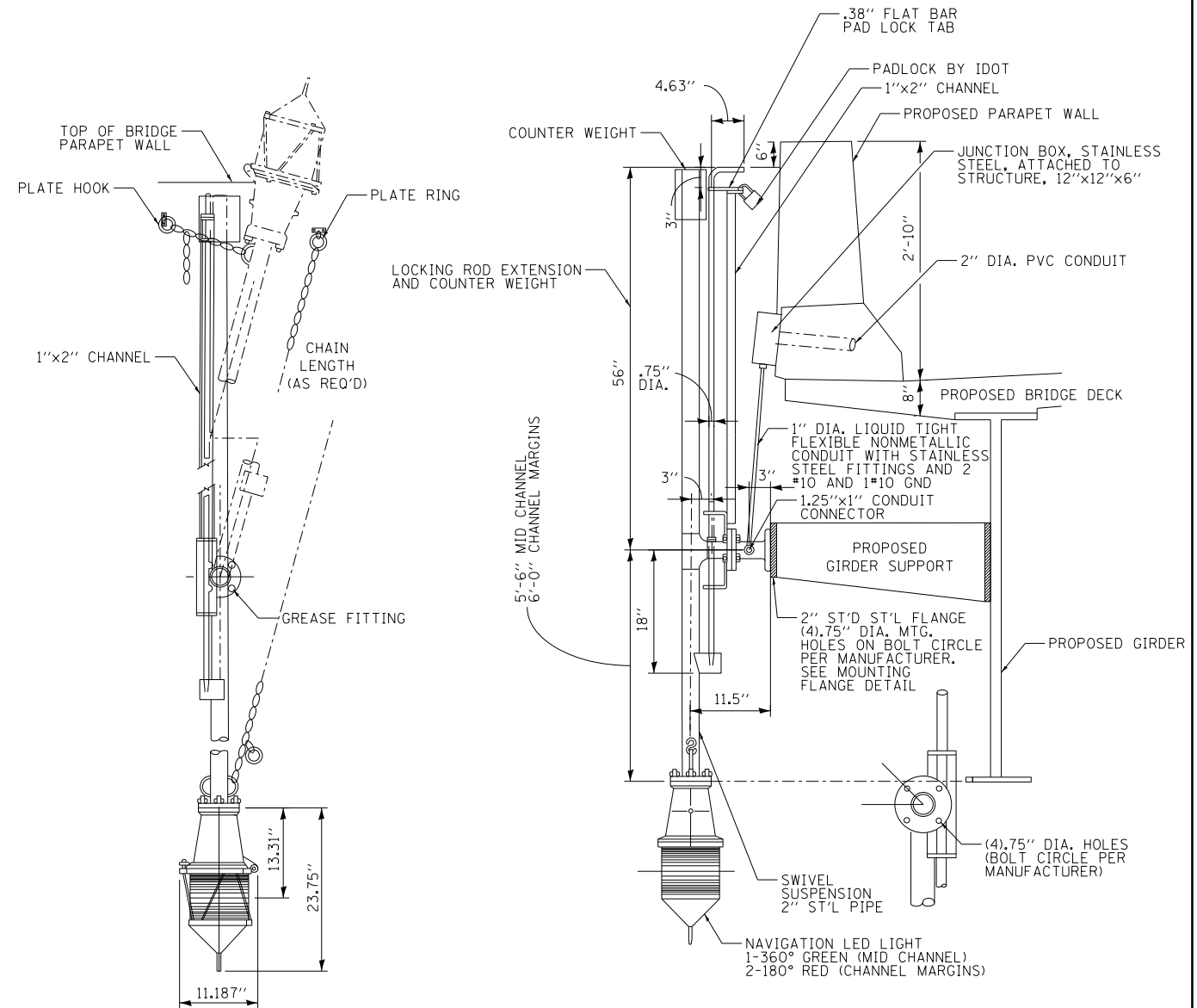


PLATE RING

ONE PER SWIVEL REQUIRED

MATERIAL:
CARBON STEEL
FINISH:
HOT DIP GALVANIZED



NAVIGATION LUMINAIRE

NAVIGATION LIGHT FIXTURE AND LOCKING SWIVEL ASSEMBLY FOR NB IL-171 BRIDGE

NOTES:

1. ALL DIMENSIONS ARE IN INCHES UNLESS NOTED OTHERWISE. CONTRACTOR SHALL VERIFY DIMENSIONS IN FIELD PRIOR TO SUBMITTING SHOP DRAWINGS.
2. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS TO THE ENGINEER FOR APPROVAL OF THE NAVIGATION LIGHT ASSEMBLY.
3. PLATE RING AND PLATE HOOK SHALL BE ATTACHED TO PARAPET WALL. SEE STRUCTURAL PLANS FOR PARAPET WALL DETAILS.
4. FOR DIMENSIONS OF OTHER STRUCTURAL MEMBERS, REFER TO STRUCTURAL DRAWINGS.
5. TOP OF NAVIGATION LIGHT HOUSING SHALL BE AT THE SAME ELEVATION AS THE BOTTOM OF ADJACENT GIRDER.
6. LIQUID TIGHT CONDUIT AND CABLES FROM JUNCTION BOX TO NAVIGATION LIGHT SHALL BE INCLUDED IN THE COST OF THE NAVIGATION LIGHTING SYSTEM.

FILE NAME =	DESIGNED - GRR	REVISED -
... \D160W75-sh-t-IL171-NavLight-01.dgn	DRAWN - JLW	REVISED -
USER NAME = jworthington	CHECKED - GHT	REVISED -
PLOT DATE = 6/17/2015	DATE - 6/12/2015	REVISED -

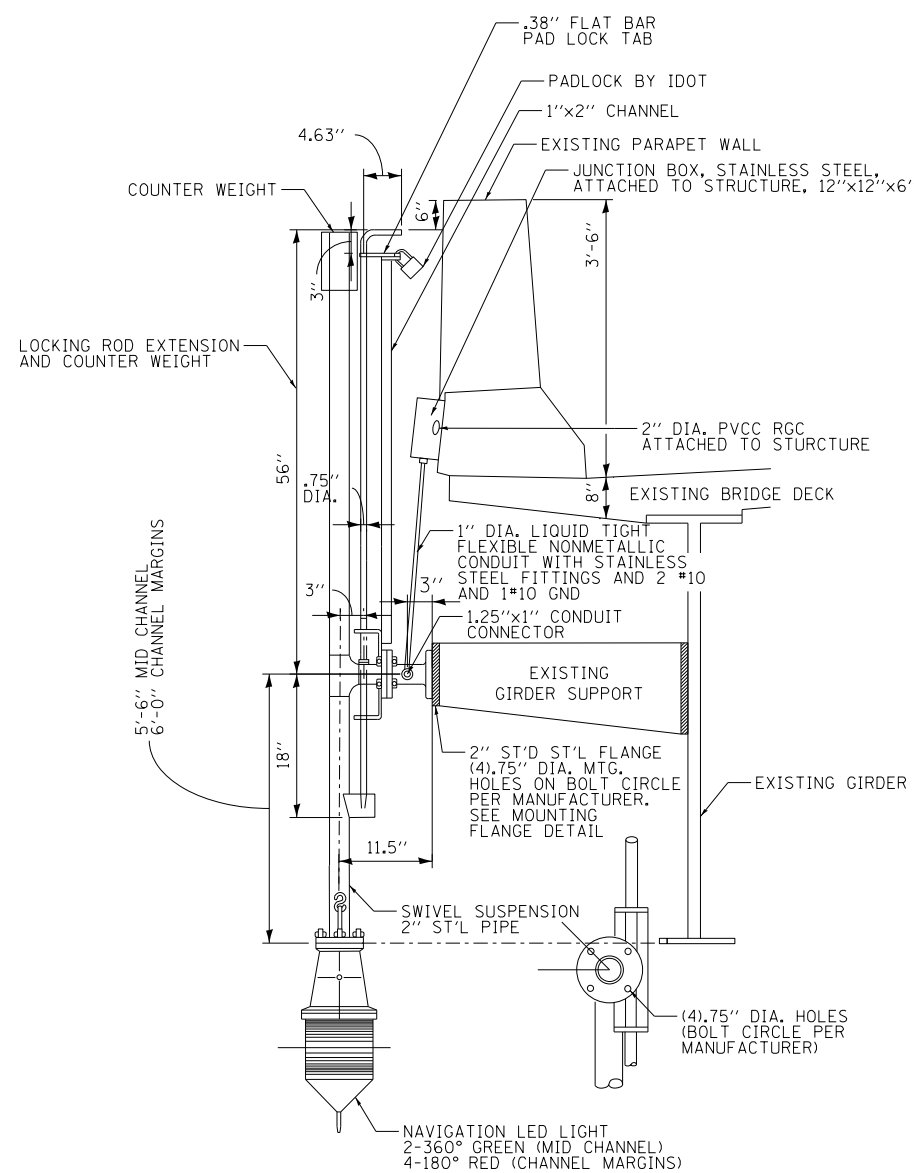


STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

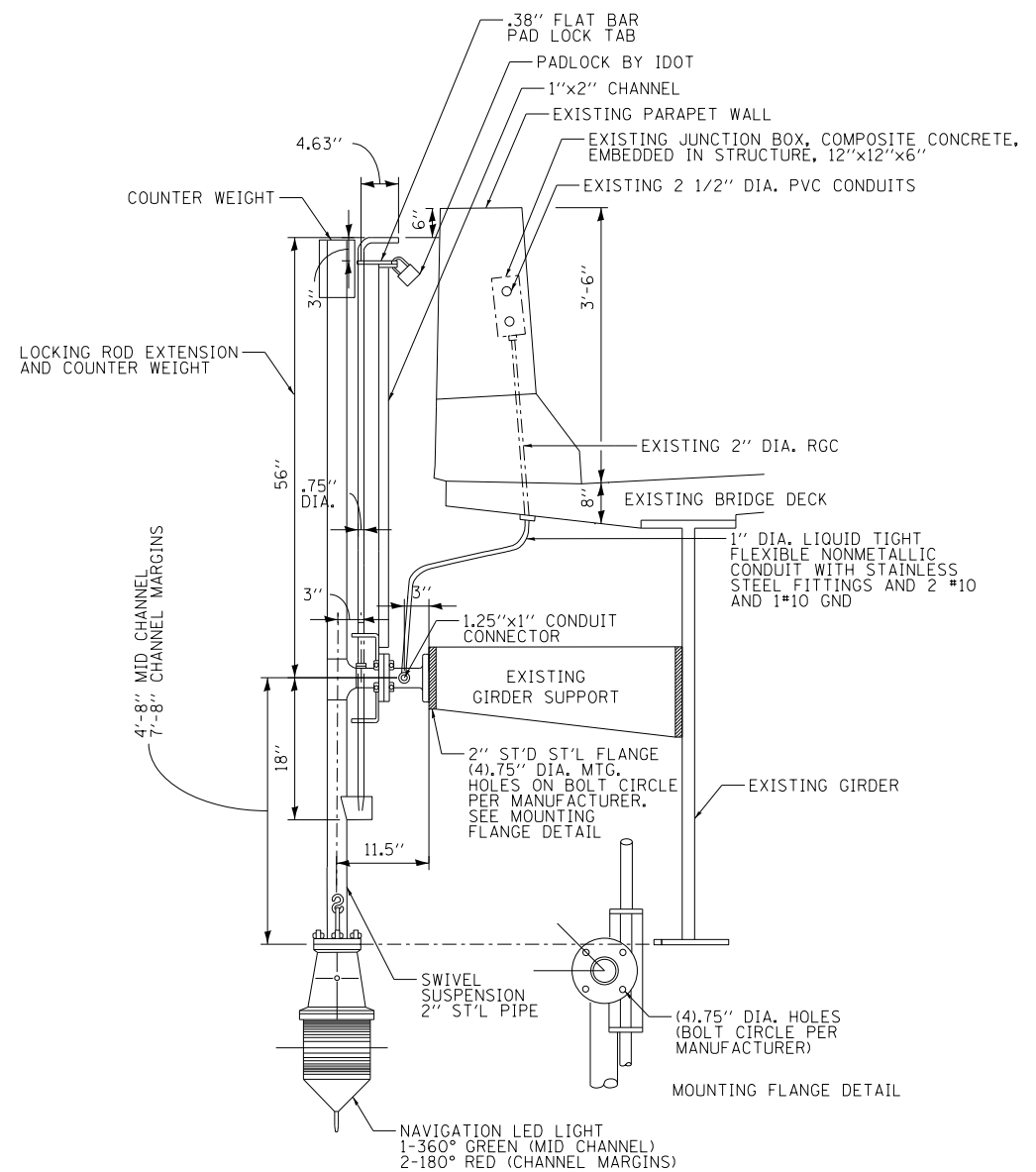
NAVIGATION LIGHTING DETAILS

SCALE: N.T.S. SHEET 39 OF 61 SHEETS STA. TO STA.

F.A.P. R.T.E.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372/373	2013-037B-R	COOK	787	209
CONTRACT NO. 60W75			ILLINOIS FED. AID PROJECT	




**NAVIGATION LIGHT FIXTURE
AND LOCKING SWIVEL ASSEMBLY
RAMP D BRIDGE (NB IL-171 TO NB I-55)**

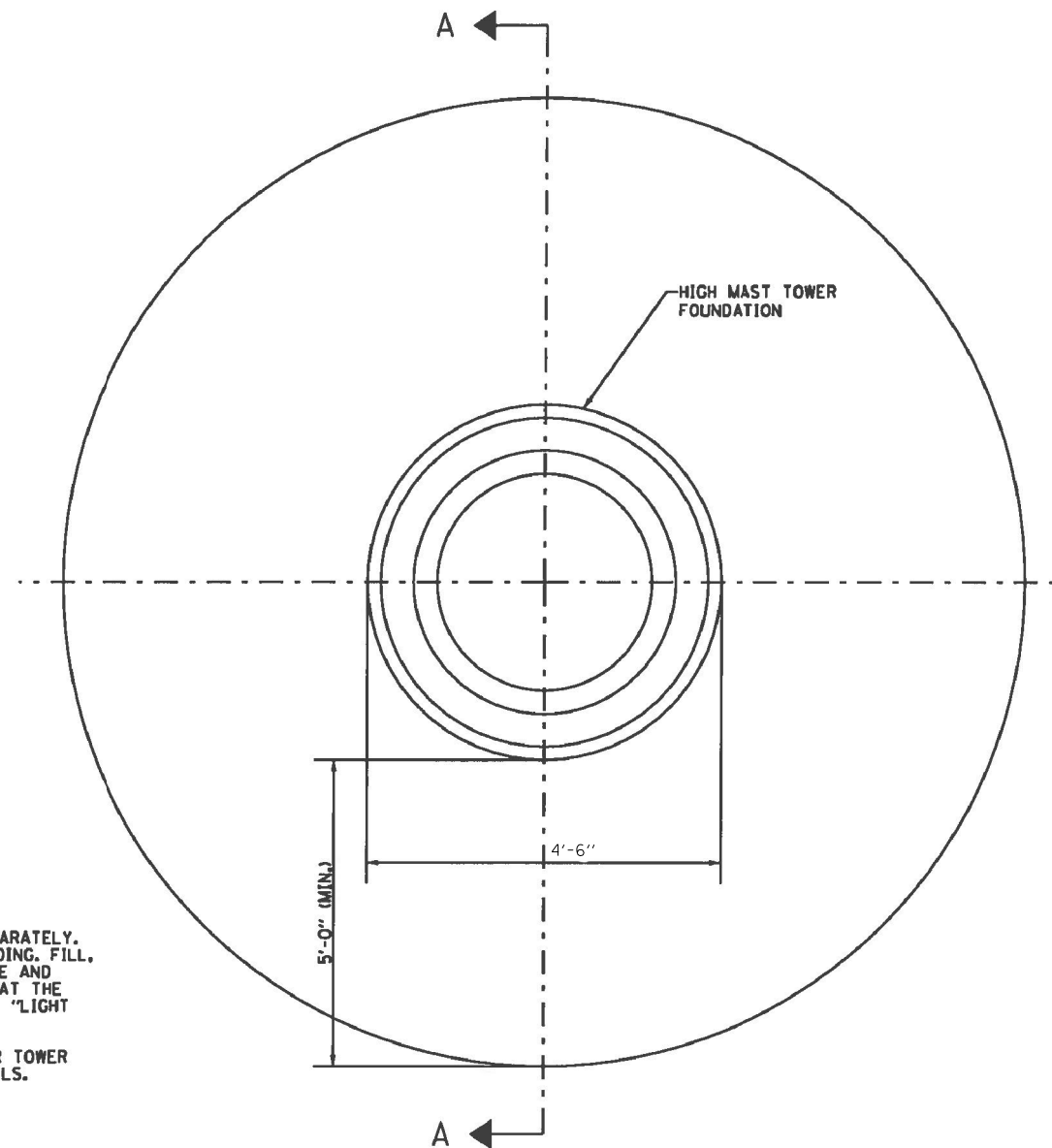


**NAVIGATION LIGHT FIXTURE
AND LOCKING SWIVEL ASSEMBLY
NB I-55 AND SB I-55 BRIDGES**

NOTES:

1. ALL DIMENSIONS ARE IN INCHES UNLESS NOTED OTHERWISE. CONTRACTOR SHALL VERIFY DIMENSIONS IN FIELD PRIOR TO SUBMITTING SHOP DRAWINGS.
2. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS TO THE ENGINEER FOR APPROVAL OF THE NAVIGATION LIGHT ASSEMBLY.
3. PLATE RING AND PLATE HOOK SHALL BE ATTACHED TO PARAPET WALL. SEE STRUCTURAL PLANS FOR PARAPET WALL DETAILS.
4. FOR DIMENSIONS OF OTHER STRUCTURAL MEMBERS, REFER TO STRUCTURAL DRAWINGS.
5. TOP OF NAVIGATION LIGHT HOUSING SHALL BE AT THE SAME ELEVATION AS THE BOTTOM OF ADJACENT GIRDER.
6. LIQUID TIGHT CONDUIT AND CABLES FROM JUNCTION BOX TO NAVIGATION LIGHT SHALL BE INCLUDED IN THE COST OF THE NAVIGATION LIGHTING SYSTEM.

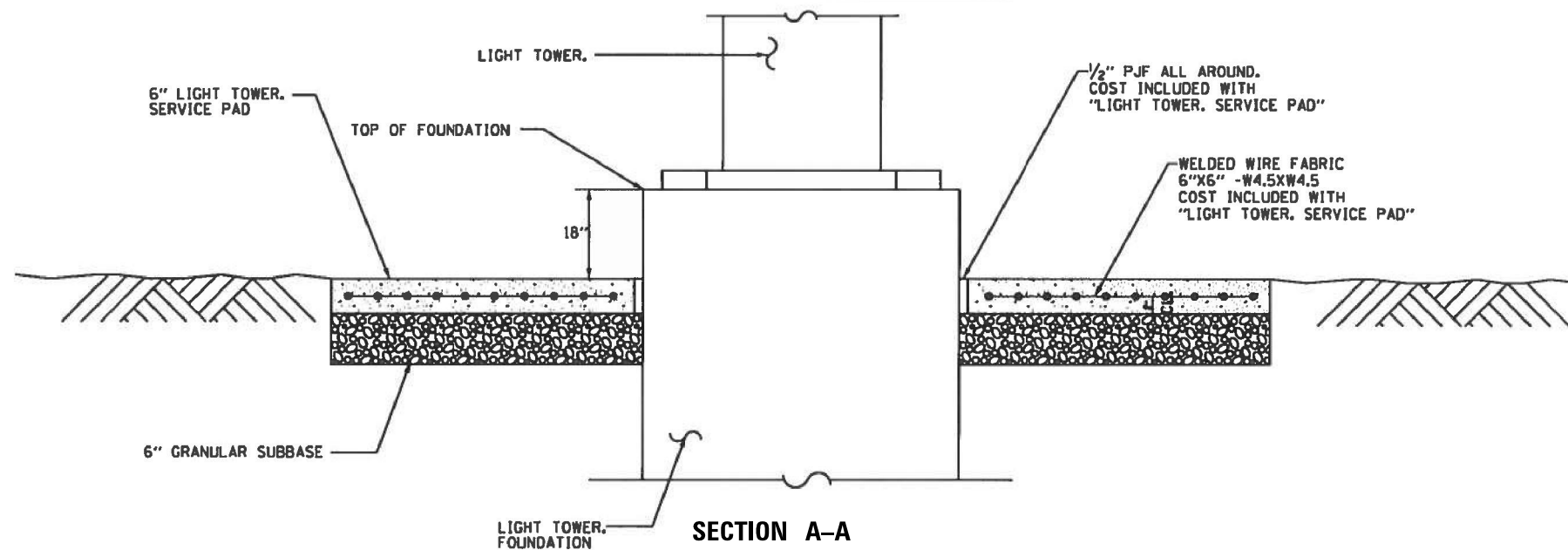
FILE NAME =	DESIGNED - GRR	REVISED -		STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	NAVIGATION LIGHTING DETAILS			F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
... \DI160W75-sh-t-IL171-NavLight-02.dgn	DRAWN - JLW	REVISED -						372/373	2013-037B-R	COOK	787	210
USER NAME = jworthington	CHECKED - GHT	REVISED -			CONTRACT NO. 60W75							
PLOT DATE = 6/17/2015	DATE - 6/12/2015	REVISED -			SCALE: N.T.S.	SHEET 40 OF 61 SHEETS	STA.	TO STA.	ILLINOIS FED. AID PROJECT			



NOTES:

1. THE LIGHT TOWER AND LIGHT TOWER FOUNDATION SHALL BE PAID FOR SEPARATELY. ALL REMAINING WORK INCLUDING GRADING, FILL, SERVICE PAD, RUSTICATION, CONCRETE AND REINFORCEMENT SHALL BE PAID FOR AT THE CONTRACT UNIT PRICE PER EACH FOR "LIGHT TOWER SERVICE PAD".
2. SEE IDOT-D1 DRAWING NO BE-506 FOR TOWER FOUNDATION AND GROUND WELL DETAILS.

TYPE A – LIGHT TOWER, SERVICE PAD DETAILS – PLAN VIEW



FILE NAME =	DESIGNED - GRR	REVISED -
...\\D160W75-shr-1L171-highmast-A.dgn	DRAWN - JLW	REVISED -
USER NAME = jworthington	CHECKED - GHT	REVISED -
PLOT DATE = 6/17/2015	DATE - 6/12/2015	REVISED -

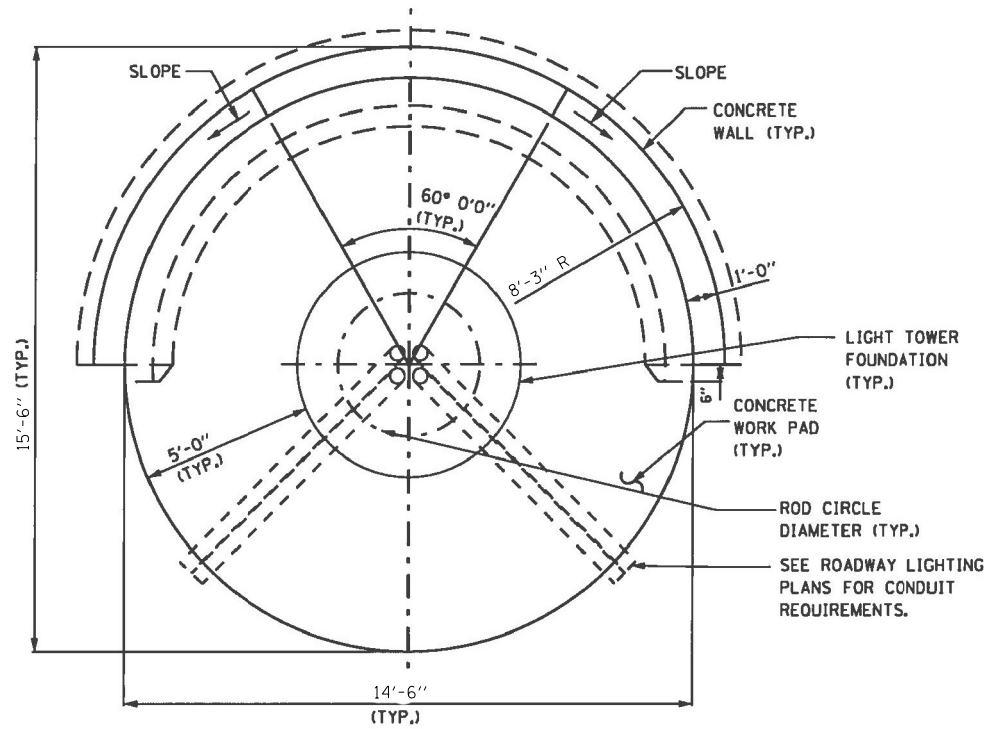


STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

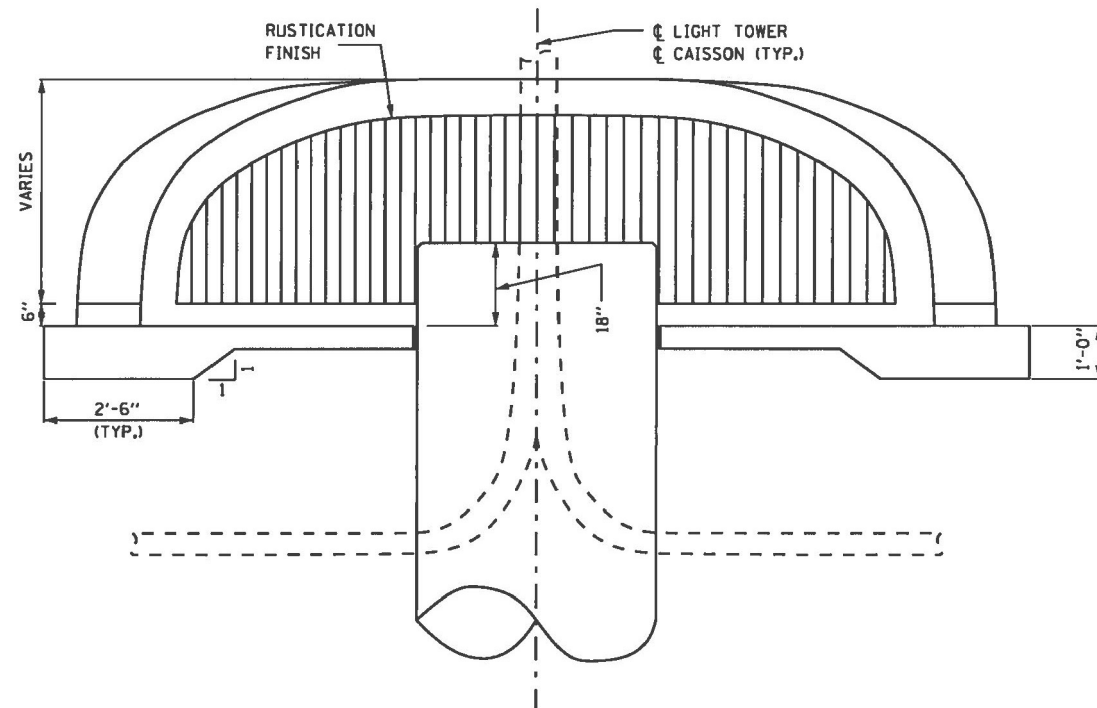
HIGH MAST LIGHT TOWER PAD DETAIL
TYPE A

SCALE: N.T.S. SHEET 41 OF 61 SHEETS STA. TO STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372/373	2013-037B-R	COOK	787	211
CONTRACT NO. 60W75			ILLINOIS FED. AID PROJECT	



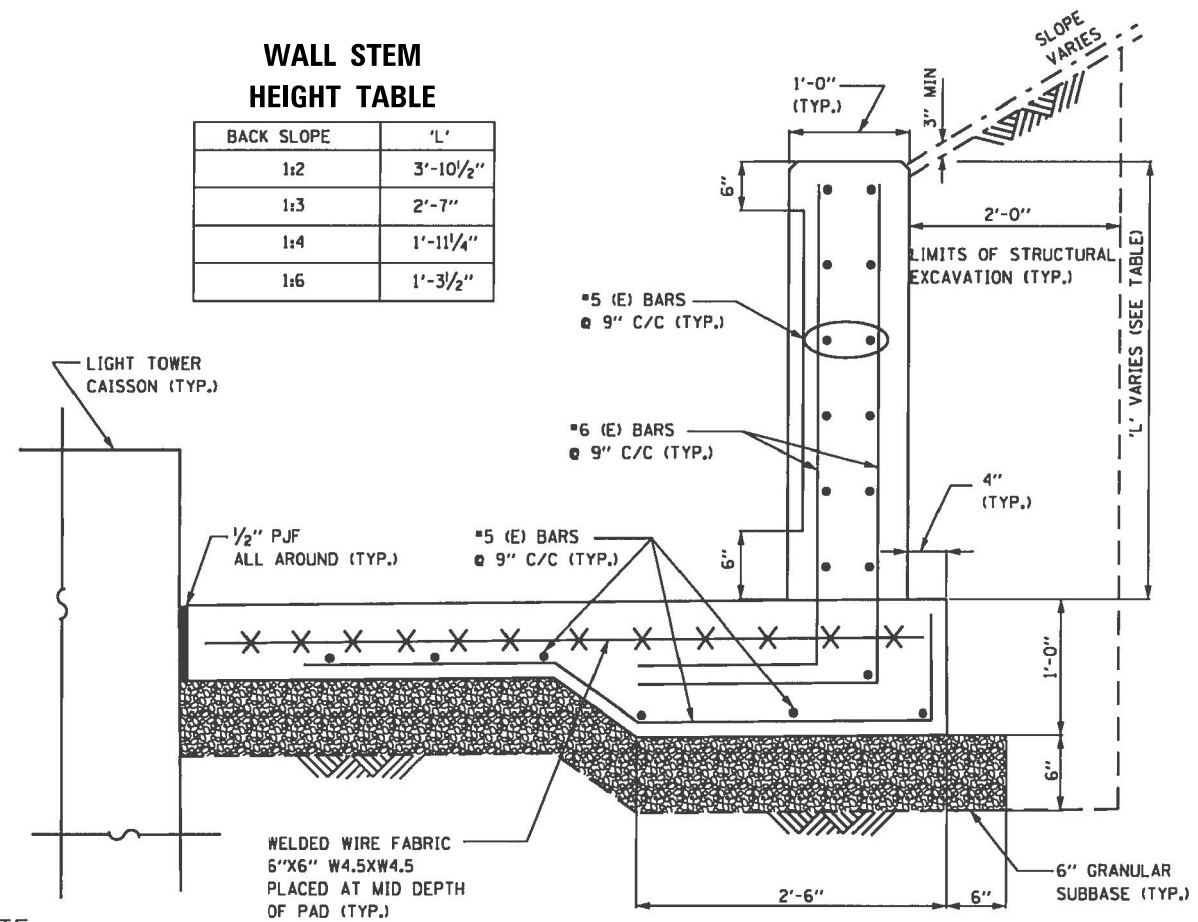
LIGHT TOWER, SERVICE PAD, SPECIAL DETAILS – PLAN VIEW



ELEVATION – WITH WALL

WALL STEM HEIGHT TABLE

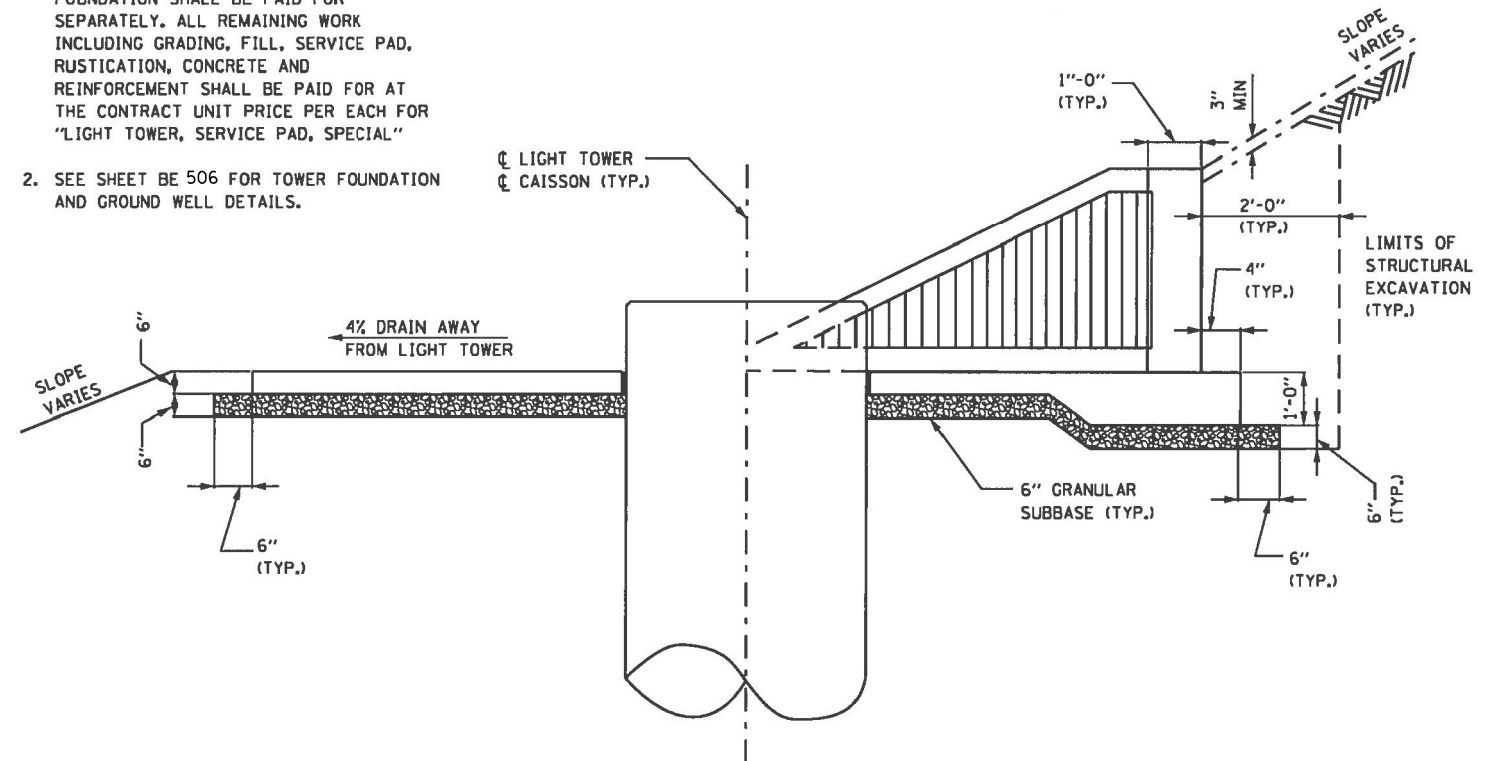
BACK SLOPE	'L'
1:2	3'-10 1/2"
1:3	2'-7"
1:4	1'-11 1/4"
1:6	1'-3 1/2"



SECTION THRU PAD-WITH WALL

NOTE:

1. THE LIGHT TOWER AND LIGHT TOWER FOUNDATION SHALL BE PAID FOR SEPARATELY. ALL REMAINING WORK INCLUDING GRADING, FILL, SERVICE PAD, RUSTICATION, CONCRETE AND REINFORCEMENT SHALL BE PAID FOR AT THE CONTRACT UNIT PRICE PER EACH FOR "LIGHT TOWER, SERVICE PAD, SPECIAL"
2. SEE SHEET BE 506 FOR TOWER FOUNDATION AND GROUND WELL DETAILS.



SECTION THRU EMBANKMENT – WITH WALL

FILE NAME =	DESIGNED - GRR	REVISED -
...ND160W75-shd-1L171-highmast-B.dgn	DRAWN - JLW	REVISED -
USER NAME = jworthington	CHECKED - GHT	REVISED -
PLOT DATE = 6/17/2015	DATE - 6/12/2015	REVISED -

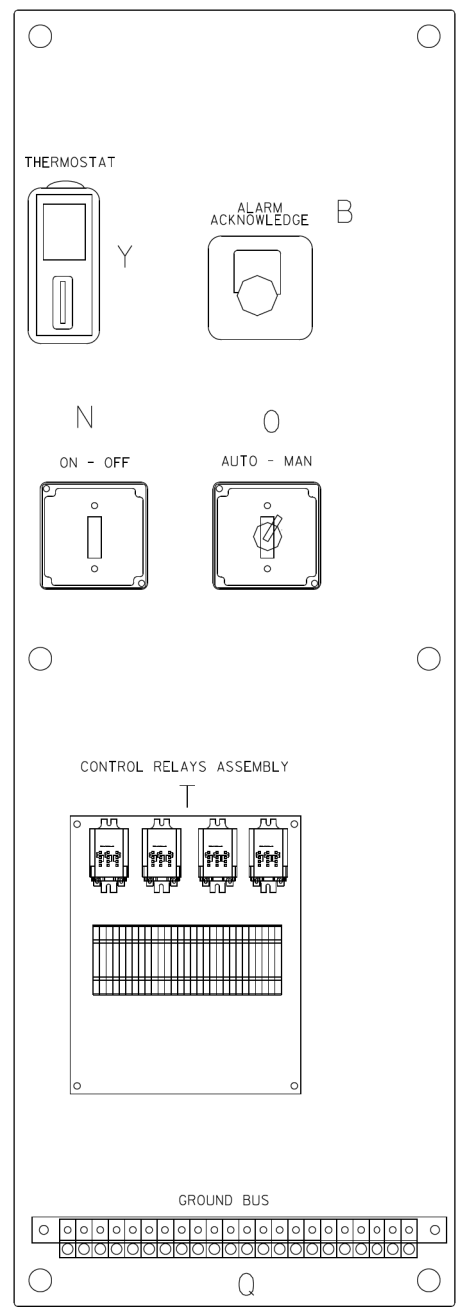
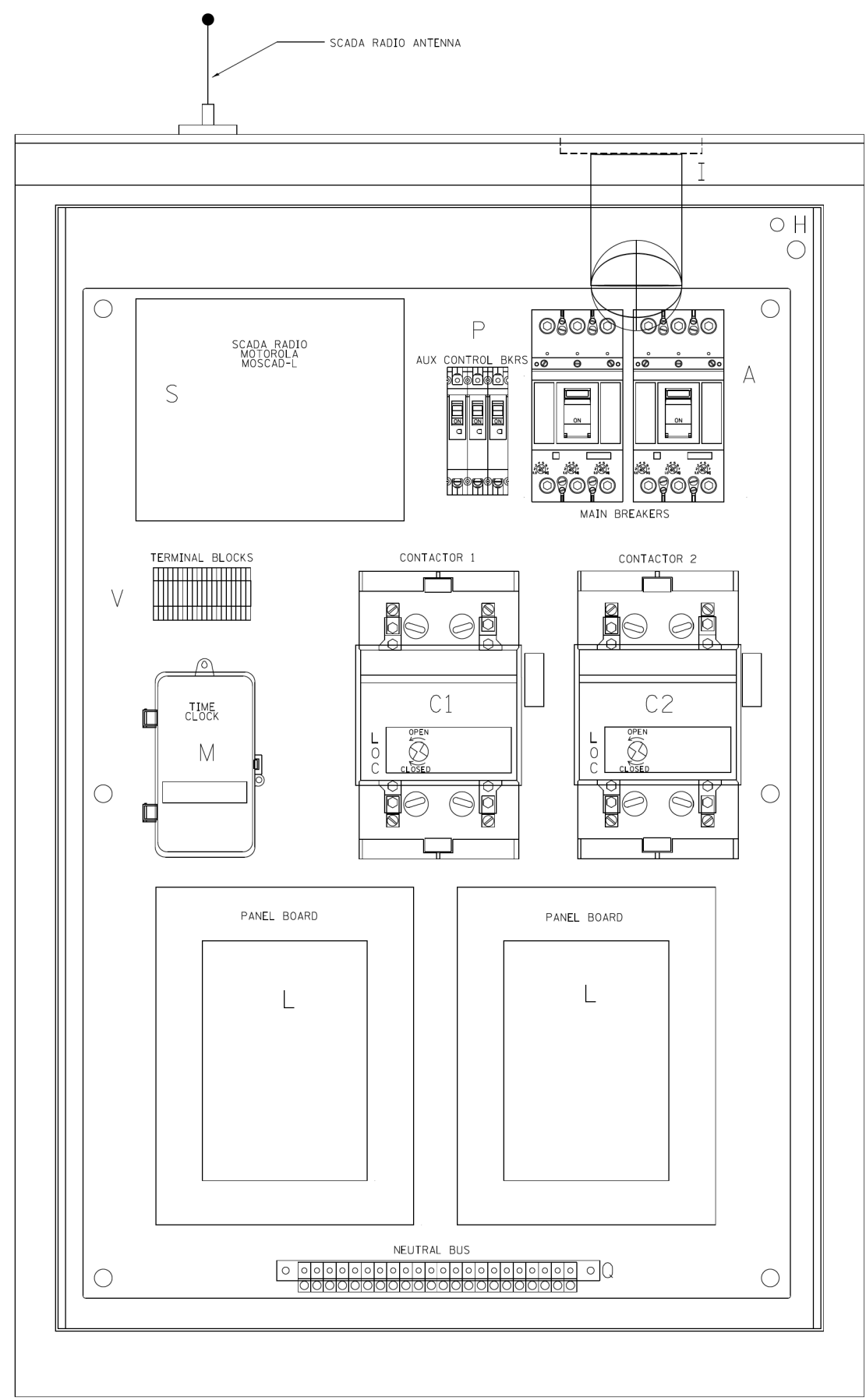


**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

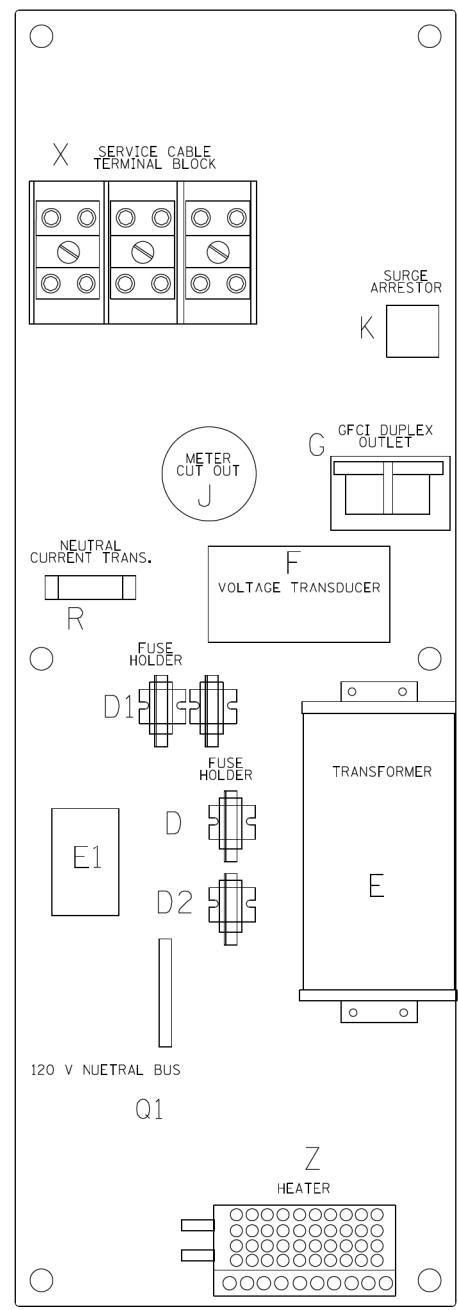
**HIGH MAST LIGHT TOWER PAD DETAIL
TYPE B**

SCALE: N.T.S. SHEET 42 OF 61 SHEETS STA. TO STA.

F.A.P. R.T.E.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372/373	2013-037B-R	COOK	787	212
CONTRACT NO. 60W75			ILLINOIS FED. AID PROJECT	



LEFT SIDE PANEL



RIGHT SIDE PANEL

BILL OF MATERIALS		
ITEM	QTY	DESCRIPTION
A	2	MAIN CIRCUIT BREAKERS 2 POLE 200 AMP WITH AUX CONTACT
B	1	ACKNOWLEDGE SWITCH, PUSH BUTTON WITH YELLOW INSERT
C1, C2*	2	CONTACTOR 2 POLE 200 AMP 240V COIL WITH AUX CONTACTS
D	1	FINGERSAFE FUSE HOLDER WITH KTK-20 FUSE
D1	2	FINGERSAFE FUSE HOLDER WITH KTK-1/2 FUSE
D2	1	FINGERSAFE FUSE HOLDER WITH KTK-2A FUSE
E	1	2.0 KVA 277V-240/120 TRANSFORMER
E1	1	0.25 KVA 240/120 - 24 VAC TRANSFORMER
F	1	VOLTAGE TRANSUDCER WITH COVERED TERMINALS
G	1	20 AMP GFCI DUPLEX OUTLET W/COVER
H	2	DOOR SWITCH
I	1	LIGHT FIXTURE
J	1	METER FITTING 1 PHASE 3 WIRE 200 AMP
K	1	SURGE ARRESTER
L	2	PANEL BOARD 480/240V 1 PHASE, 250 AMP COPPER BUS
M	1	2 CHANNEL DIGITAL TIME CLOCK
N	1	MOMENTARY SWITCH ON - OFF
O	1	SQUARE D, 9001KS11BH13, 2 POSITION SWITCH IN 9001KY1 ENCLOSURE OR APPROVED EQUAL
P	2	BREAKER 1P 15A
Q	2	COPPER GROUND AND NEUTRAL BUS 1 x 16 x 1/4
Q1	1	COPPER NEUTRAL BUS WITH 1 #6 AND 8 #12 CONDUCTOR POINTS
R	1	CURRENT TRANSUDCER
S	1	MOTOROLA MOSCAD-L RADIO, 240 V
T*	1	CONTROL RELAY ASSEMBLY 240V COILS WITH 4 3 PDT 25A RELAYS (W389ACX-15) (R1, R2, R3, R4) . QTY 32 TERMINAL BLOCKS
V	20	TERMINAL BLOCKS
X*	1	620 AMP SLPICE BLOCK
Y	1	40-80 DEG THERMOSTAT
Z	1	375 WATT HEATER

* TERMINALS SHALL BE COVERED WITH CLEAR PLEXIGLASS SHEET

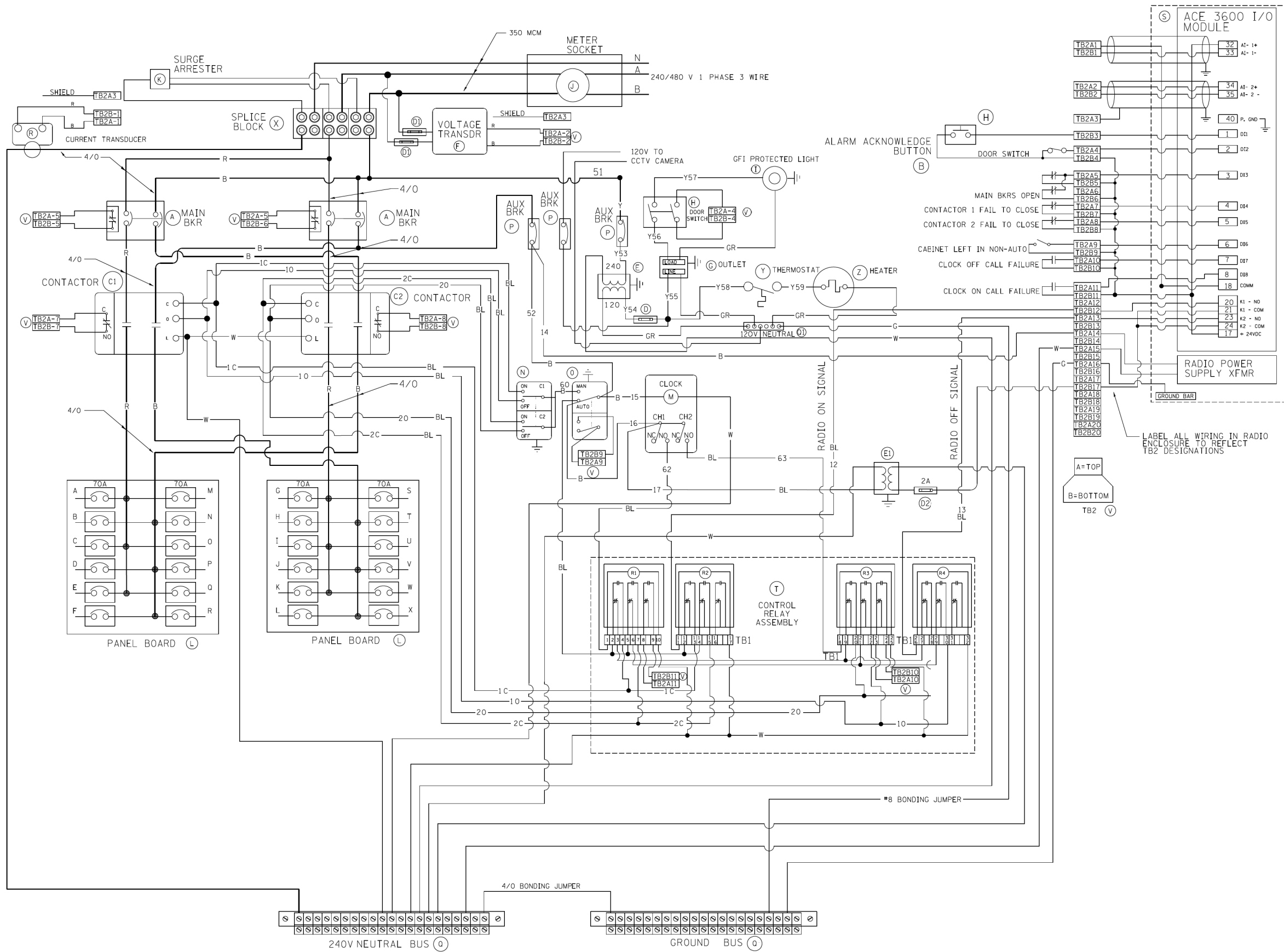
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		CHECKED -	REVISED - R. TOMSONS 03-10-10
		DATE -	REVISED - R. TOMSONS 03-29-12

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

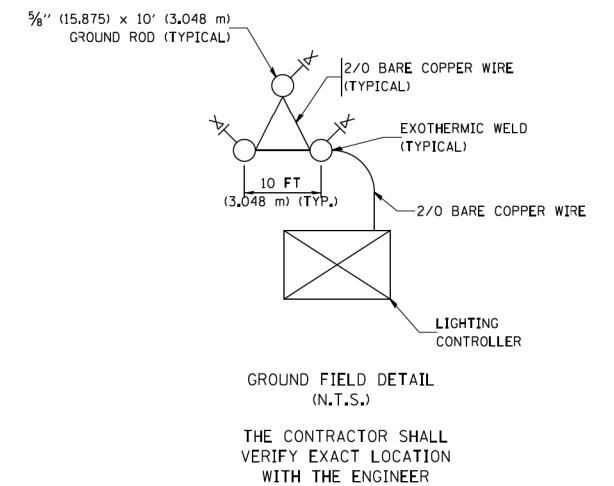
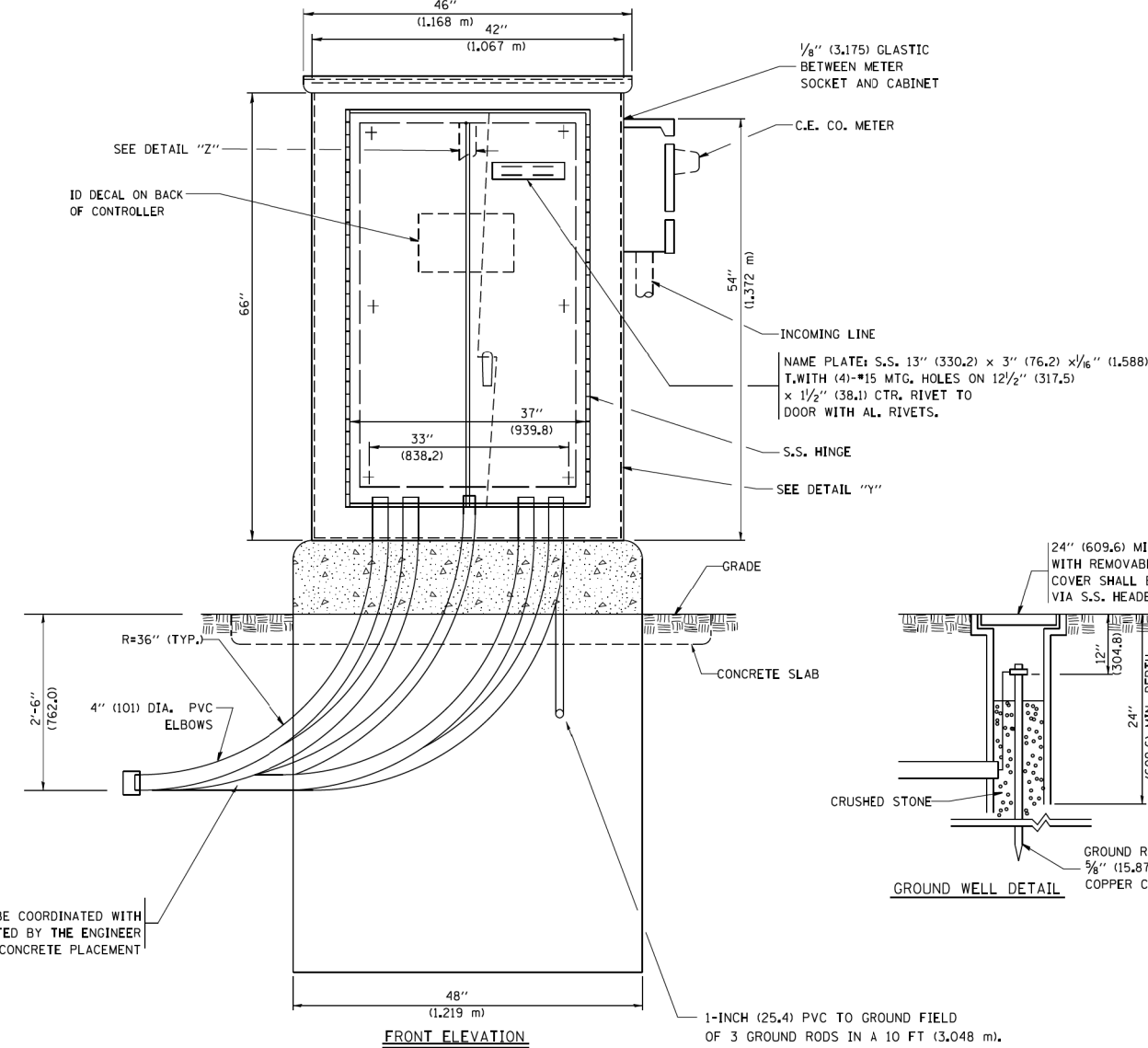
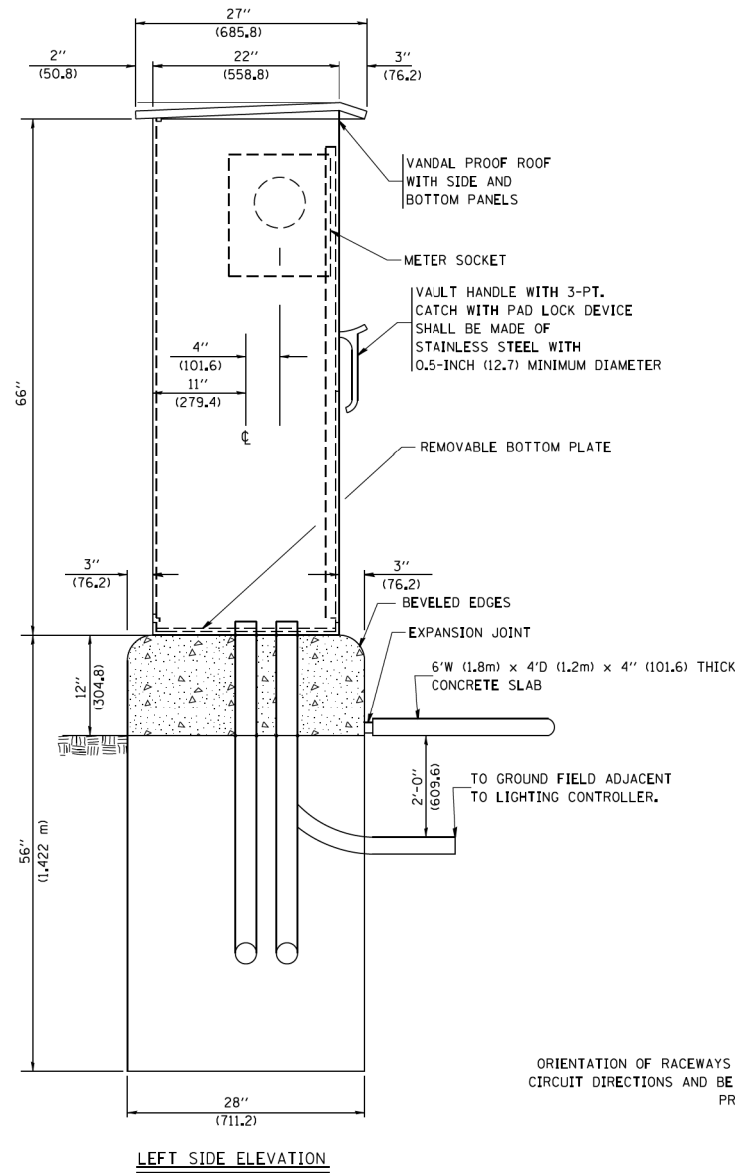
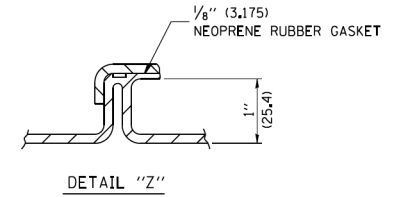
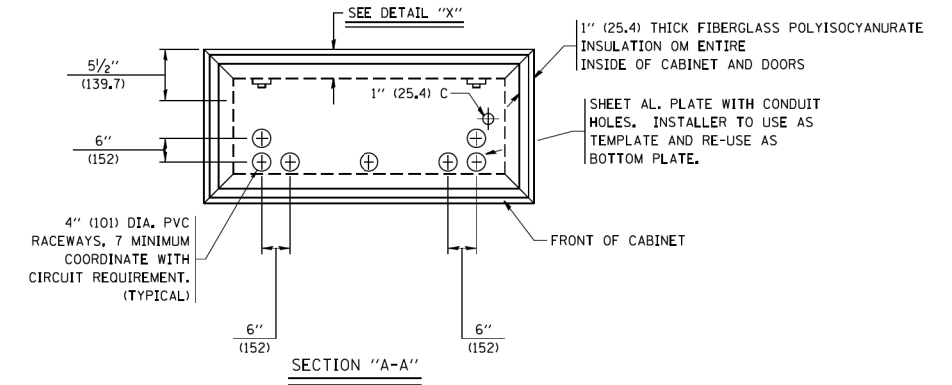
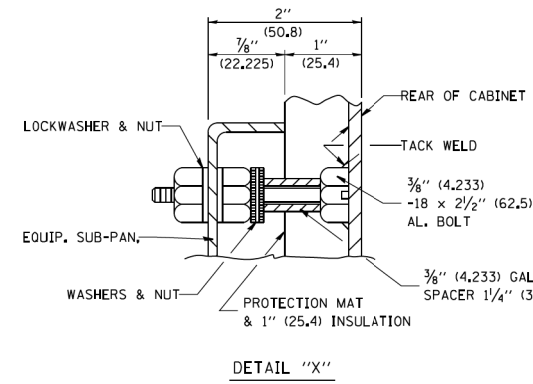
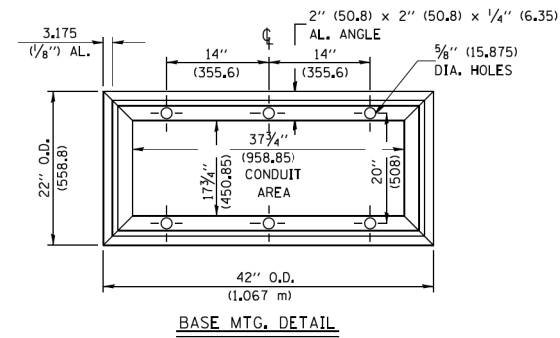
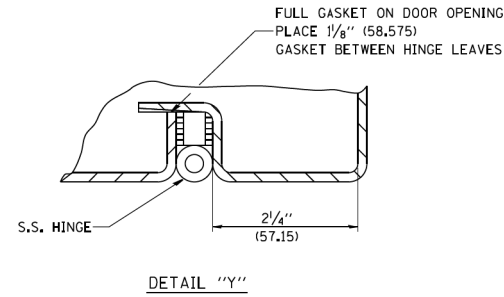
LIGHTING CONTROLLER, BASE MOUNTED, 480VOLT, 200AMP (DUAL) RADIO SCADA

SCALE: NONE SHEET NO. 1 OF 4 SHEETS STA. TO STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372/373	2013-037B-R	COOK	787	213
BE-205A			CONTRACT NO. 60W75	
FED. ROAD DIST. NO. 1 ILLINOIS FED. AID PROJECT				



BILL OF MATERIALS		
ITEM #	QTY	DESCRIPTION
A	2	MAIN CIRCUIT BREAKERS 2 POLE 200 AMP WITH AUX CONTACT
B	1	ACKNOWLEDGE SWITCH, PUSH BUTTON WITH YELLOW INSERT
C1, C2	2	CONTACTOR 2 POLE 200 AMP 240V COIL WITH AUX CONTACTS
D	1	FINGERSAFE FUSE HOLDER WITH KTK-20A FUSE
D1	2	FINGERSAFE FUSE HOLDER WITH KTK-1/2 FUSE
D2	1	FINGERSAFE FUSE HOLDER WITH KTK- 2A FUSE
E	1	2.0 KVA 277V-240/120 TRANSFORMER
E1	1	0.25 KVA 240/120-24 VAC TRANSFORMER
F	1	VOLTAGE TRANSDUCER
G	1	15 AMP GFCI DUPLEX OUTLET W/COVER
H	2	DOOR SWITCH A-20GO-B7-K
I	1	LIGHT FIXTURE
J	1	METER FITTING 1 PHASE 3 WIRE 200 AMP
K	1	SURGE ARRESTER
L	2	PANEL BOARD 480/240V 1 PHASE, 250 AMP COPPER BUS
M	1	2 CHANNEL DIGITAL TIME CLOCK
N	1	MOMENTARY SWITCH ON - OFF
O	1	SQUARE D, 900IKS11BH3, 2 POSITION SWITCH IN 900IKY1 ENCLOSURE
P	2	BREAKER 1P 15A
Q	2	COPPER GROUND AND NEUTRAL BUS 1 x 16 x 1/4
Q1	1	COPPER NEUTRAL BUS WITH 1 1/0 AND #6 CONDUCTOR POINTS
R	1	CURRENT TRANSDUCER
S	1	MOTOROLA ACE 3600
T	1	CONTROL RELAY ASSEMBLY 240V COILS WITH 4 3 PDT 25A RELAYS (W389ACX-15) (R1, R2, R3, R4) . QTY 32 TERMINAL BLOCKS
V	20	TERMINAL BLOCKS
X	1	620 AMP SPLICE BLOCK
Y	1	40-80 DEG THERMOSTAT
Z	1	375 WATT HEATER



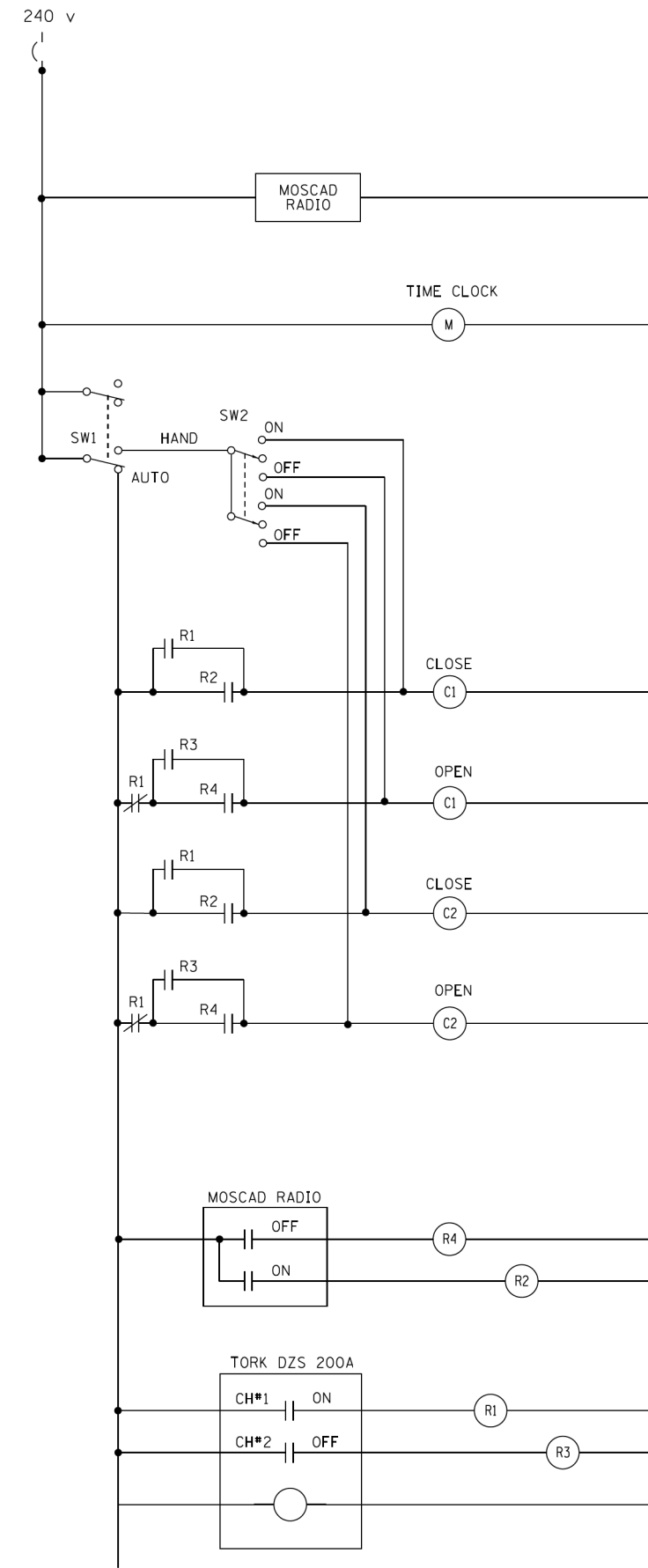
ORIENTATION OF RACEWAYS SHALL BE COORDINATED WITH CIRCUIT DIRECTIONS AND BE INSPECTED BY THE ENGINEER PRIOR TO CONCRETE PLACEMENT

FILE NAME =	USER NAME = drivakosgn	DESIGNED -	REVISED - R. TOMSONS 08-19-04	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	LIGHTING CONTROLLER, BASE MOUNTED, 480VOLT, 200AMP (DUAL) RADIO SCADA			F.A.P. RTE. 372/373	SECTION 2013-037B-R	COUNTY COOK	TOTAL SHEETS 787	SHEET NO. 215			
ca:\pwork\pwork\drivakosgn\d0108315\be205.dgn	PLOT SCALE = 50.000' / 1 in.	DRAWN -	REVISED - R. TOMSONS 05-11-09					SCALE: NONE			SHEET NO. 3 OF 4 SHEETS			STA. TO STA.	
	PLOT DATE = 3/29/2012	CHECKED -	REVISED - R. TOMSONS 03-10-10					CONTRACT NO. 60W75			FED. ROAD DIST. NO. 1 ILLINOIS FED. AID PROJECT				
		DATE -	REVISED - R. TOMSONS 03-29-12												

NOTES

- CABINET SHALL BE FABRICATED FROM 0.125-INCH (3.175) SHEET ALUMINUM #3003H14, FORMED AND ARC WELDED.
- ALL SCREWS AND HARDWARE SHALL BE PLATED, GALVANIZED, OR MADE OF BRASS, ALUMINUM OR STAINLESS STEEL, UNLESS OTHERWISE NOTED.
- NAME PLATE SHALL HAVE ENGRAVED 0.75-INCH (19.05) HIGH LETTERS FILLED IN BLACK: "STATE OF ILLINOIS LIGHTING CONTROLS" UNLESS OTHERWISE SPECIFIED.
- ONE INCH THICK POLYISOCYANURATE INSULATION SHALL BE INSTALL AND PERMANENTLY CEMENTED ON ALL SIDES OF THE CABINET AND DOORS.
- CABINET SHALL BE PRIMED AND PAINTED AS SPECIFIED.
- ELECTRIC UTILITY METER BOX SHALL BE MOUNTED ON THE SIDE OF CONTROL CABINET AS SHOWN ON THE PANEL LAYOUT DIAGRAM.
- THE COMPLETED CONTROLLER SHALL BE U.L. LISTED AS AN INDUSTRIAL CONTROL PANEL UNDER UL508.
- METAL MOUNTING PANEL SHALL BE FABRICATED FROM THE SAME MATERIAL AS THE CABINET AND SHALL BE FLANGED BACK 0.75-INCHES I.D. ON 4 SIDES.
- CIRCUIT BREAKERS AND CONTACTORS AND OTHER COMPONENTS SHALL BE MOUNTED ON 0.125-INCH (3.175) THICK GLASTIC INSULATION BACK PANEL.
- ALL DEVICES SHALL BE FRONT REMOVABLE.
- TIME CLOCK CHANNEL 1 N.O. CONTACT IS CLOSED NIGHT AND OPEN DAY (LIGHTS ON).
- SET LATITUDE TO 42 DEGREES, SET CH.1 TO 23 MINUTES AFTER ASTRONOMICAL SUNSET, 50 MINUTES BEFORE ASTRONOMICAL SUNRISE, SET CH.2 TO 60 MINUTES AFTER ASTRONOMICAL SUNSET (WITH A SIGNAL LENGTH OF 1 SECOND), +28 MINUTES AFTER ASTRONOMICAL SUNRISE (WITH A SIGNAL LENGTH OF 7 SECONDS.)
- BUS BAR SHALL HAVE 22 LUG TERMINALS SIZED TO ACCOMMODATE REQUIRED WIRE SIZES. 240V NEUTRAL BUS SHALL BE PAINTED WHITE, GROUND BUS SHALL BE PAINTED GREEN, AND THE 120V NEUTRAL BUS SHALL BE PAINTED GREY.
- ALL LUGS SHALL BE OF COPPER SCREWS AND CONNECTORS, SPRING HELD.
- ALL WIRING TERMINATIONS SHALL BE RATED NOT LESS THAN 75 DEGREE CENTIGRADE.
- ALL CONTROL WIRING SHALL BE 600V #12 TYPE MTW, SCADA WIRING SHALL BE #18.
- ALL POWER WIRING SHALL BE 600V TYPE RHH/RHW.
- ALL WIRING WITHIN THE CABINET SHALL BE COLOR CODED AS INDICATED:

R - RED	Y - YELLOW
B - BLACK	W - WHITE
BL - BLUE	G - GREEN
	GR - GREY
- MOSCAD I/O WIRING SHALL BE:
 - DIGITAL INPUT (DI) WIRING SHALL BE #18 MTW PURPLE.
 - ANALOG INPUT (AI) WIRING SHALL BE #18, 2/C SHIELDED.
 - AI AND DI WIRING MAY BE BUNDLED TOGETHER, BUT SHALL NOT BE BUNDLED WITH OTHER WIRING.
- ALL DIMENSIONS ARE IN INCHES (MILLIMETERS) UNLESS OTHERWISE INDICATED.
- SCHEMATIC SHOWN WITH BREAKER OPEN, CONTACTOR OPEN, CABINET DOOR CLOSED, CLOCK NOT ACTIVE (DE-ENERGIZED STATE).
- A LAMINATED COPY OF THE CIRCUIT SCHEMATIC AND SCADA I/O DIAGRAM (NO SMALLER THAN 11"x17" EACH) SHALL BE ATTACHED TO THE INSIDE OF THE CONTROLLER WITH STAINLESS STEEL SCREWS.



CONTROL CIRCUIT LADDER LOGIC DIAGRAM

MOSCAD I/O ASSIGNMENTS		
TERM	MOSCAD DESTINATION	DESCRIPTION OF INPUT
1	DIGITAL INPUT 1	ALARM KNOWLEDGE
2	DIGITAL INPUT 2	DOOR OPEN
3	DIGITAL INPUT 3	MAINS) BREAKER OPEN
4	DIGITAL INPUT 4	CONTACTOR 1 OPEN
5	DIGITAL INPUT 5	CONTACTOR 2 OPEN
6	DIGITAL INPUT 6	CABINET IN NON-AUTO
7	DIGITAL INPUT 7	BACK-UP CLOCK OFF CALL
8	DIGITAL INPUT 8	BACK-UP CLOCK ON CALL
17	24 V+	24+VDC
18	DI COMMON	COMMON
21	K1 C	K1 COMMON
22	K1 NO	LIGHTS ON CALL
24	K2 C	K2 COMMON
25	K2 NO	LIGHTS OFF CALL
32	ANALOG INPUT 1 (+)	CABINET NEUTRAL CURRENT
33	ANALOG INPUT 1 (-)	CABINET NEUTRAL CURRENT
34	ANALOG INPUT 2 (+)	CABINET SERVICE VOLTAGE
35	ANALOG INPUT 2 (-)	CABINET SERVICE VOLTAGE
40	P. GROUND	GROUND

ALL ANALOG INPUTS WILL BE 4-20 MA ONLY. DIGITAL OUTPUT RELAYS WILL BE ELECTRICALLY ENERGIZED AND MOMENTARILY HELD
MIXED I/O MODULE MODEL NUMBER V436

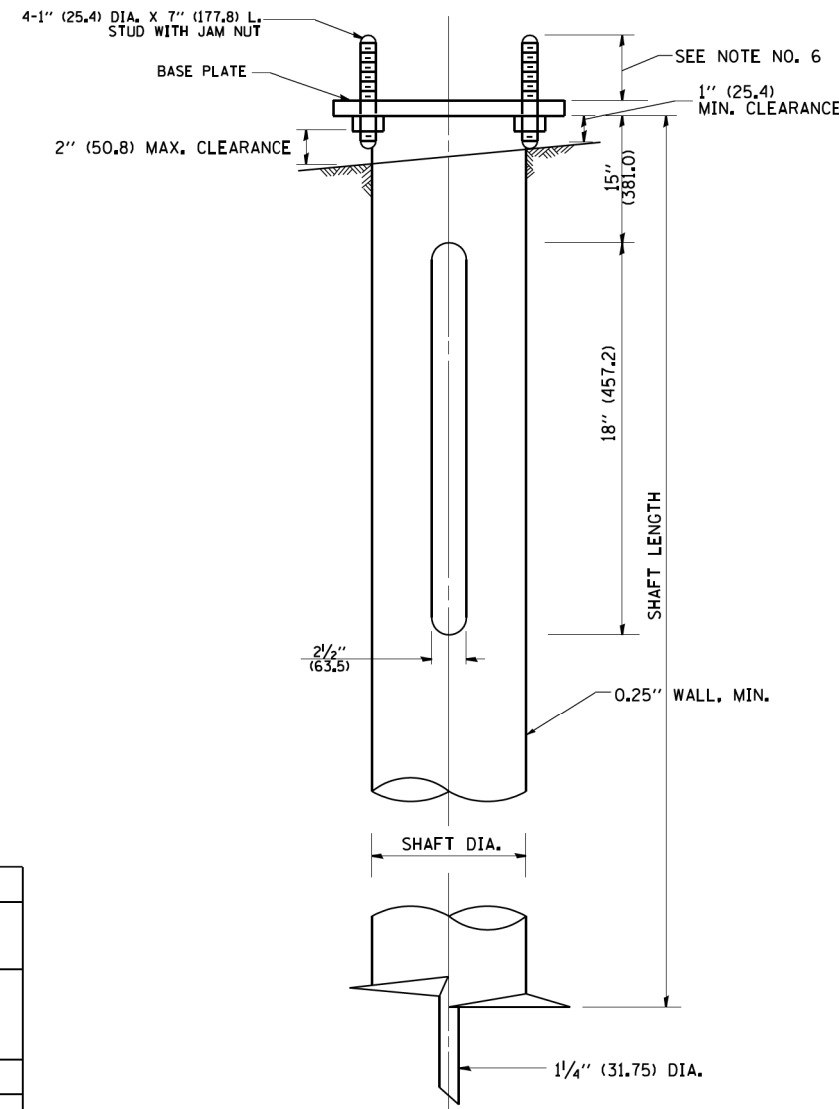
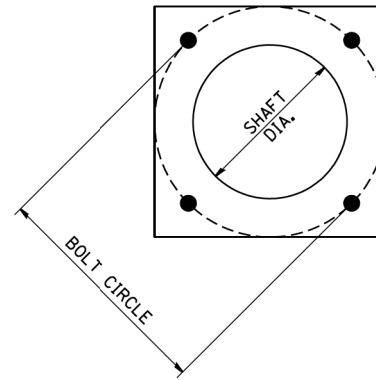
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ca:\pwork\pwork\drivakosgn\d0108315\be205.dgn		DRAWN -	REVISED - R. TOMSONS 05-11-09
	PLOT SCALE = 50.000' / in.	CHECKED -	REVISED - R. TOMSONS 03-10-10
	PLOT DATE = 3/29/2012	DATE -	REVISED - R. TOMSONS 03-29-12

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

LIGHTING CONTROLLER, BASE MOUNTED, 480VOLT, 200AMP (DUAL) RADIO SCADA

SCALE: NONE SHEET NO. 4 OF 4 SHEETS STA. TO STA.

F.A.P. RTE. 372/373	SECTION 2013-037B-R	COUNTY COOK	TOTAL SHEETS 787	SHEET NO. 216
BE-205D		CONTRACT NO. 60W75		
FED. ROAD DIST. NO. 1 ILLINOIS FED. AID PROJECT				



HELIX FOUNDATION SIZE

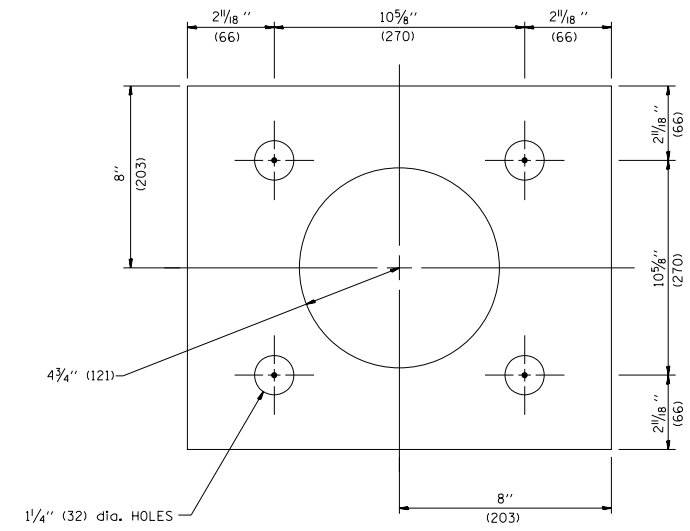
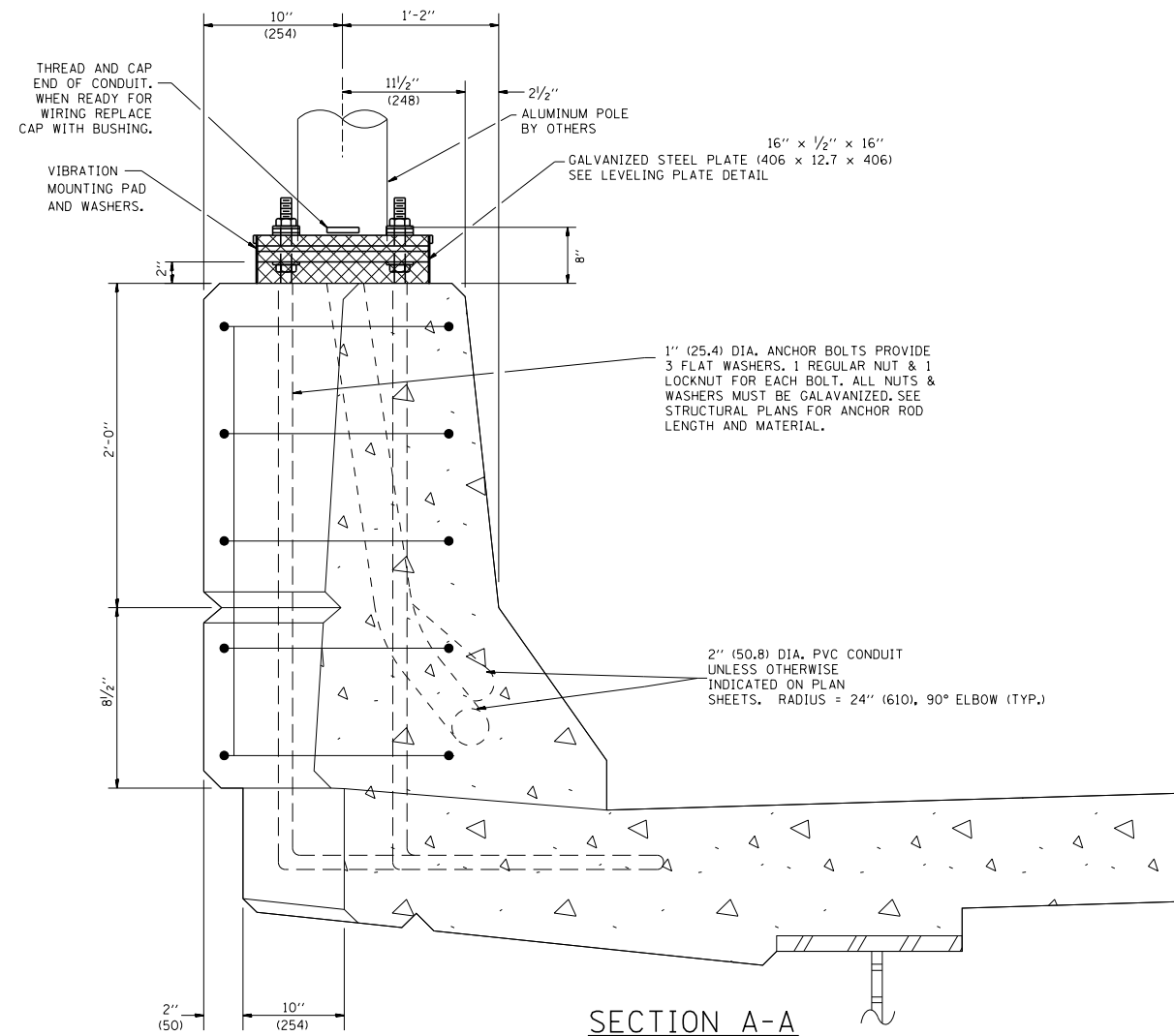
POLE MOUNTING HEIGHT	BOLT CIRCLE	SHAFT DIAMETER	SHAFT LENGTH	BASEPLATE
30 FT.	11 1/2"	8 5/8"	6 FT.	12"x12"x1"
31 FT.-35 FT.	11 1/2"	8 5/8"	6 FT.	12"x12"x1"
36 FT.-40 FT.	15"	8 5/8"	6 FT.	15"x15"x1 1/4"
41 FT.-45 FT.	15"	8 5/8"	6 FT.	15"x15"x1 1/4"
46 FT.-50 FT.	15"	10"	8 FT.	15"x15"x1 1/4"

METAL HELIX FOUNDATION MATERIALS

ITEM	MATERIAL REQUIREMENT
BASEPLATE	AASHTO M 270M, GRADE 36 (M270M, GRADE 250)
SHAFT	ASTM A 252, GRADE 2 (PHOSPHOROUS 0.04% MAXIMUM, SULFUR 0.05% MAXIMUM)
HELIX SCREW	AASHTO M 183 (ASTM A 635)
PILOT POINT	AASHTO M 270 (ASTM A 575)
ANCHOR RODS/STUDS	AASHTO M 314 (ASTM F 1554)
HEXAGON NUTS	AASHTO M 291M (ASTM A 563) GRADE DH, OR AASHTO M 292 (ASTM A 194) GRADE 2H
WASHERS	AASHTO M 293 (ASTM F 436)

NOTES:

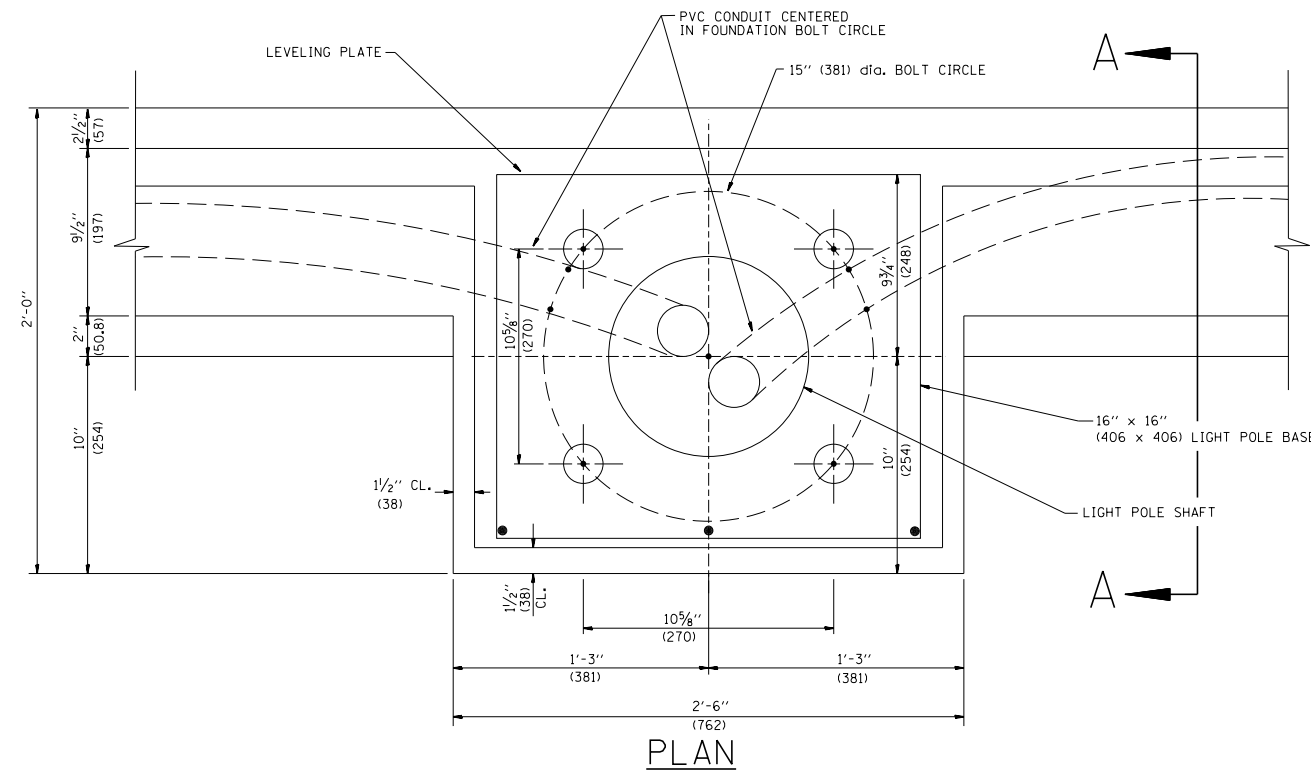
- ALL DIMENSION IN INCHES (MILLIMETERS) UNLESS OTHERWISE SHOWN.
- ALL MATERIAL SHALL BE GALVINIZED ACCORDING TO AASHTO M111, UNLESS OTHERWISE SPECIFIED.
- ALL WELDS SHALL BE CONTINUOUS AND NOT LESS THAN 1/4" (6.35 mm) FILLET WELDS. THE WELDED FOUNDATION SHALL BE CAPABLE OF WITHSTANDING 10,000 FT/LBS (13558.18 n.m) OF INSTALLATION TORQUE APPLIED ABOUT THE AXIS OF THE FOUNDATION.
- THE HELIX FOUNDATION SHAFT SHALL BE INSTALLED VERTICAL AND THE BASE PLATE SHALL BE IN LEVEL. THE BREAKAWAY COUPLINGS AND HARDWARE SHALL NOT BE USED TO ALIGN THE POLE INSTALLATION.
- THE CABLE TRENCH SHALL BE BACKFILLED AND FIRMLY COMPACTED BEFORE THE INSTALLATION OF THE LIGHT POLE.
- THE CONTRACTOR SHALL COORDINATE EXTENSION OF ANCHOR BOLTS ABOVE TOP OF THE BASE PLATE WITH THE BREAKAWAY DEVICE MANUFACTURER'S REQUIREMENTS.
- ANY VOIDS WITHIN THE METAL FOUNDATION SHALL BE FILLED WITH FINE AGGREGATE.
- METAL FOUNDATIONS SHALL BE INSTALLED IN UNDISTURBED SOIL. PREDRILLING A PILOT HOLE AND/OR BACKFILLING AROUND THE FOUNDATION IS NOT ALLOWED.
- THE METAL FOUNDATION SHALL NOT BE INSTALLED TO A TORQUE WHICH EXCEEDS THE MANUFACTURER'S MAXIMUM TORQUE RATING NOR SHALL IT BE INSTALLED TO AN INSTALLATION TORQUE VALUE OF LESS THAN 3,500 FT LB (4,750 KNM). METAL FOUNDATIONS THAT ARE NOT INSTALLED TO FULL INSTALLATION DEPTH OR DO NOT ACHIEVE THE MINIMUM INSTALLATION TORQUE SHALL BE REMOVED AND REPLACED WITH A CONCRETE FOUNDATION AT NO ADDITIONAL COST.
- THE BASEPLATE SHALL BE PERPENDICULAR TO THE SHAFT AXIS ($\pm 1^\circ$) AND THE HOLE CENTERLINE SHALL BE CONCENTRIC (± 0.188) TO THE SHAFT AXIS.
- THE PILOT POINT AND SHAFT AXIS SHALL BE CONCENTRIC (± 0.125) AND IN LINE ($\pm 2^\circ$).
- THE BASEPLATE SHALL BE STAMPED WITH THE MANUFACTURERS NAME AND DATE OF MANUFACTURE.
- GRADING AROUND LIGHT POLE FOUNDATIONS SHALL MEET THE REQUIREMENTS OF ARTICLE 836.03 OF THE STANDARD SPECIFICATIONS.



LEVELING PLATE DETAIL

NOTES

1. ALL DIMENSIONS ARE IN INCHES (MILLIMETERS) UNLESS OTHERWISE SHOWN.
2. THE COST OF ANCHOR BOLTS, LEVELING PLATE AND FOUNDATION IS INCLUDED IN THE COST OF THE BRIDGE STRUCTURE.
3. SEE STRUCTURAL PLANS FOR REBAR DETAILS.



PLAN

FILE NAME =	DESIGNED -	REVISED -
...\\D160W75-sh-t-1L171-BE-330-NB.dgn	DRAWN -	REVISED -
USER NAME = jworthington	CHECKED -	REVISED -
PLOT DATE = 6/17/2015	DATE - 6/12/2015	REVISED -

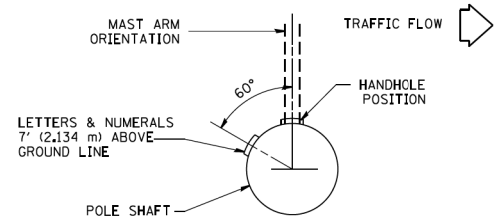


STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

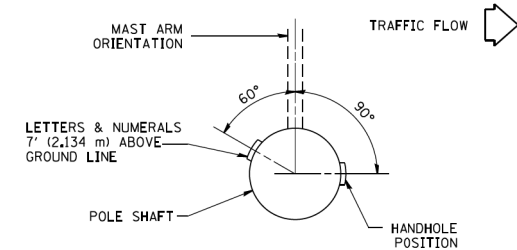
LIGHT POLE MOUNTED ON CONCRETE PARAPET
WALL, 15" (381 mm) BOLT CIRCLE

SCALE: SHEET 1 OF 1 SHEETS STA. TO STA.

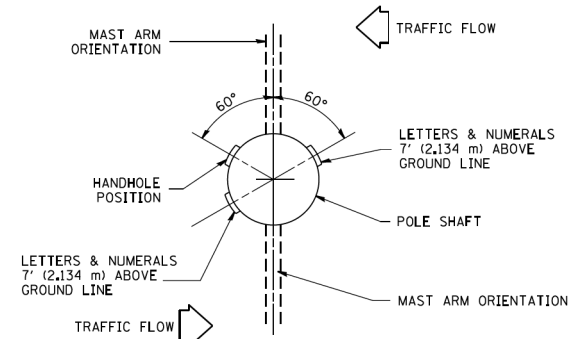
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372/373	2013-037B-R	COOK	787	219
BE-330		CONTRACT NO. 60W75		
ILLINOIS FED. AID PROJECT				



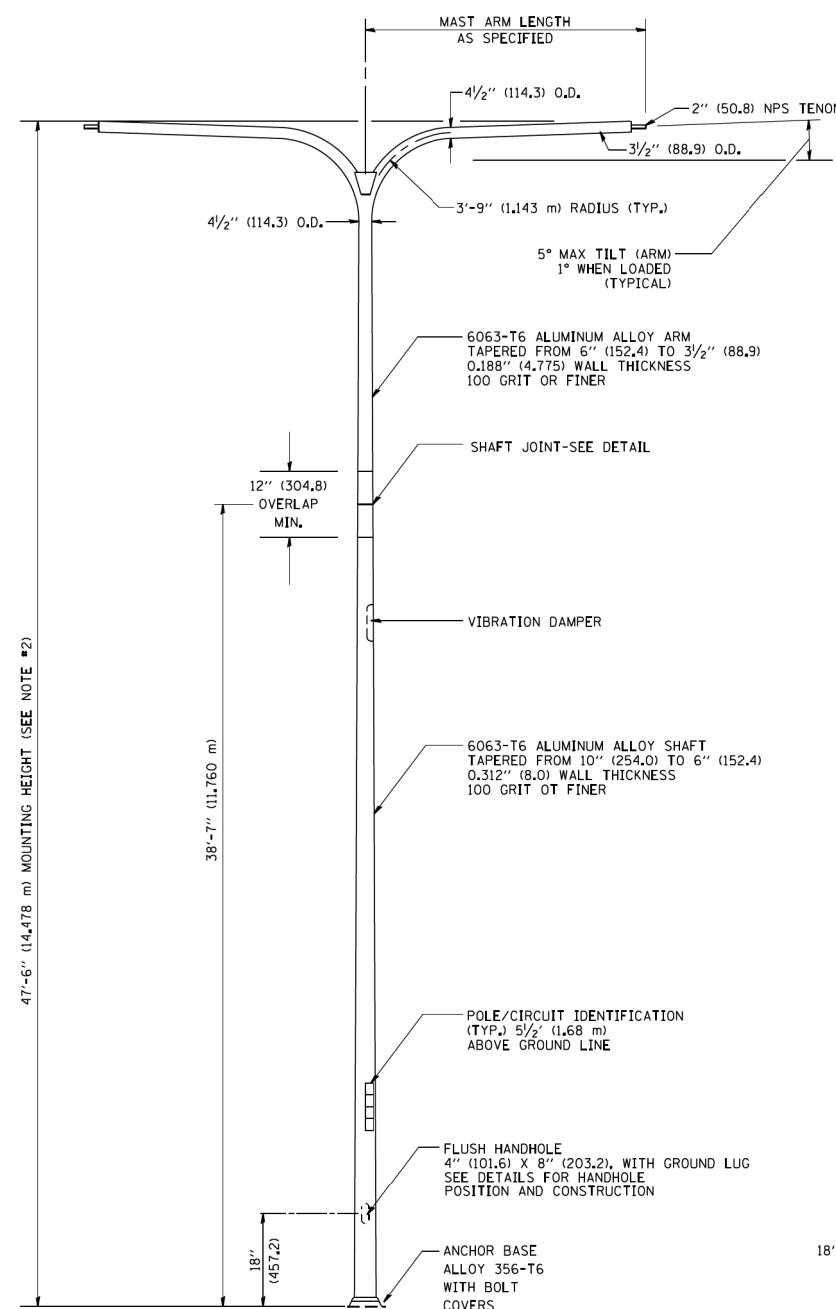
POSITION OF HANDHOLE AND POLE NUMBER FOR SINGLE MAST ARM POLES MOUNTED ON BRIDGE PARAPET OR BARRIER WALL



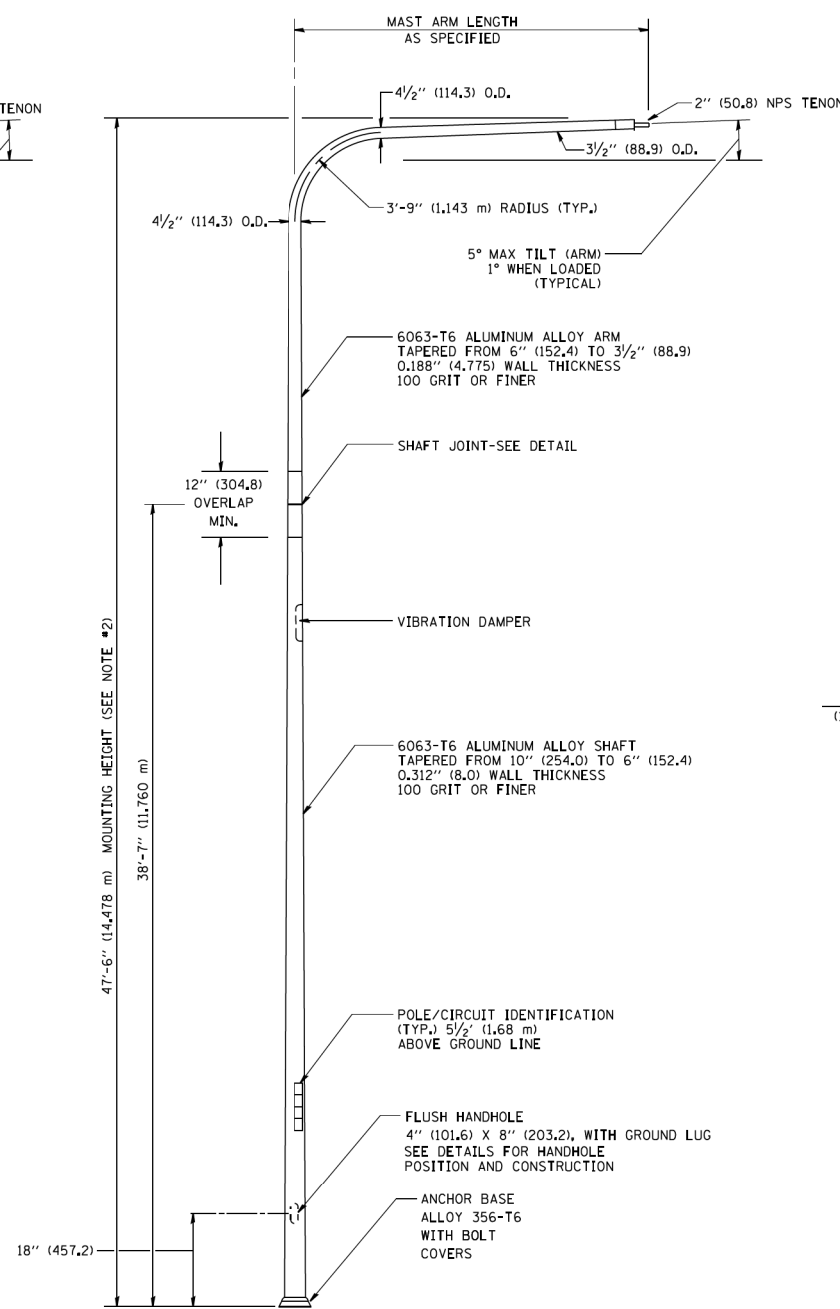
POSITION OF HANDHOLE AND POLE NUMBER FOR SINGLE MAST ARM POLES



POSITION OF HANDHOLE AND POLE NUMBER FOR TWIN MAST ARM POLES

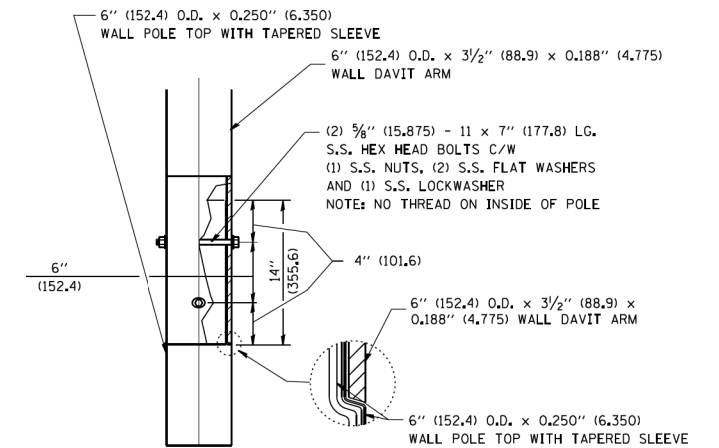


TWIN ARM POLE

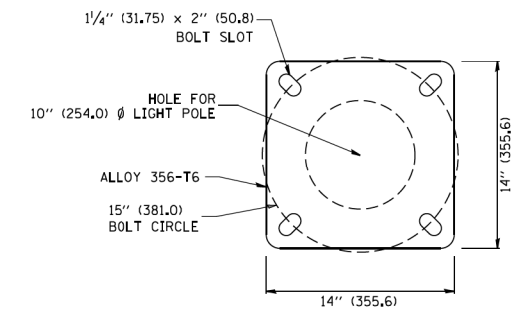


SINGLE ARM POLE

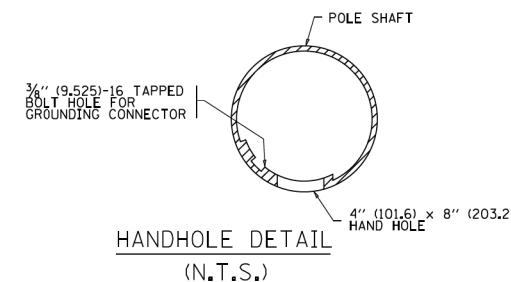
- NOTES:
- ALL DIMENSIONS ARE IN INCHES (MILLIMETERS) UNLESS OTHERWISE SHOWN.
 - MOUNTING HEIGHT IS DEFINED AS THE DISTANCE FROM THE CENTERLINE OF THE TENON TO THE BOTTOM OF THE ANCHOR BASE.
 - TWO PIECE SHAFT WILL BE MATCHED MARKED AND INTERCHANGEABLE BETWEEN DIFFERENT UNITS. FIELD DRILLING OF THE HOLES WILL NOT BE ALLOWED.
 - THE LIGHT POLE WILL MEET AASHTO DESIGN CRITERIA AS SPECIFIED.
 - THE INSTALLING CONTRACTOR WILL PROVIDE A UL LISTED GROUNDING CONNECTOR, BURNDY K2C23, T&B SP4DL OR APPROVED EQUAL.
 - LIGHT POLES WILL NOT BE INSTALLED WITHOUT MAST ARMS AND LUMINAIRES.
 - LIGHT POLES WILL BE SET PLUMB ON THE FOUNDATION WITHOUT THE USE OF LEVELING NUTS, WASHERS OR SHIMS.
 - LIGHTING UNIT IDENTIFICATION NUMBERS SHALL BE INSTALLED BEFORE THE LIGHTING UNIT IS ENERGIZED.



DAVIT ARM CONNECTION
[14" (355.6) OVERLAP SHOWN]



LIGHT POLE BASE PLATE DETAIL
(FOR POLE MOUNTED ON 15 INCH (381.0) BOLT CIRCLE FOUNDATION)



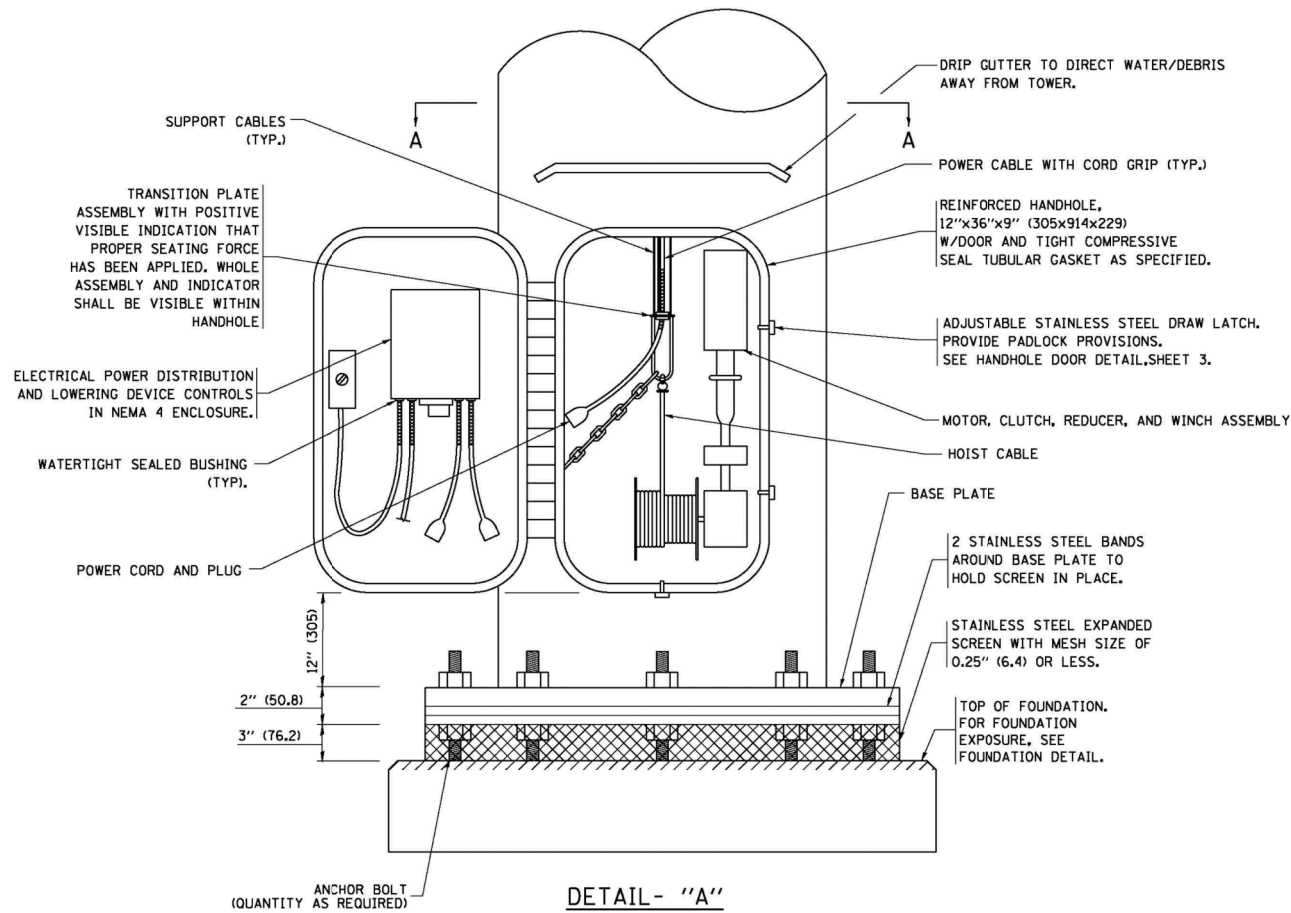
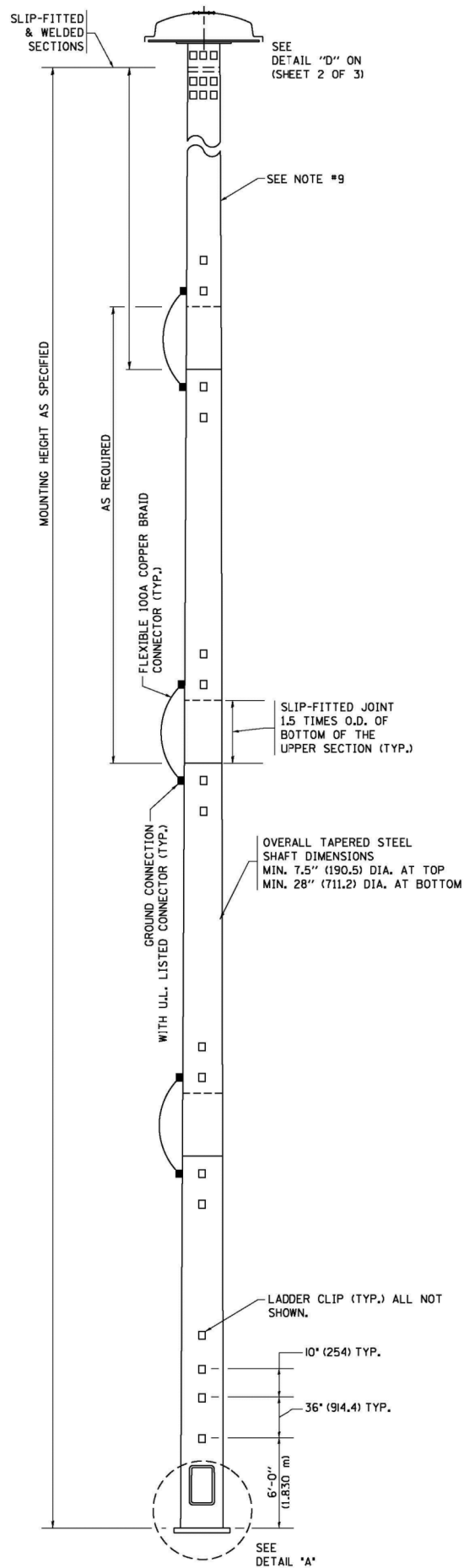
HANDHOLE DETAIL
(N.T.S.)

FILE NAME =	USER NAME = lveys	DESIGNED -	REVISED - D. DREW 05-07-92
ct:\pwork\pwork\lveys\0108315\be410.dgn		DRAWN - LEY	REVISED - R. TOMSONS 09-06-00
	PLOT SCALE = 50.0000 / 1	CHECKED -	REVISED - R. TOMSONS 09-02-03
	PLOT DATE = 4/4/2013	DATE -	REVISED - R. TOMSONS 01-18-13

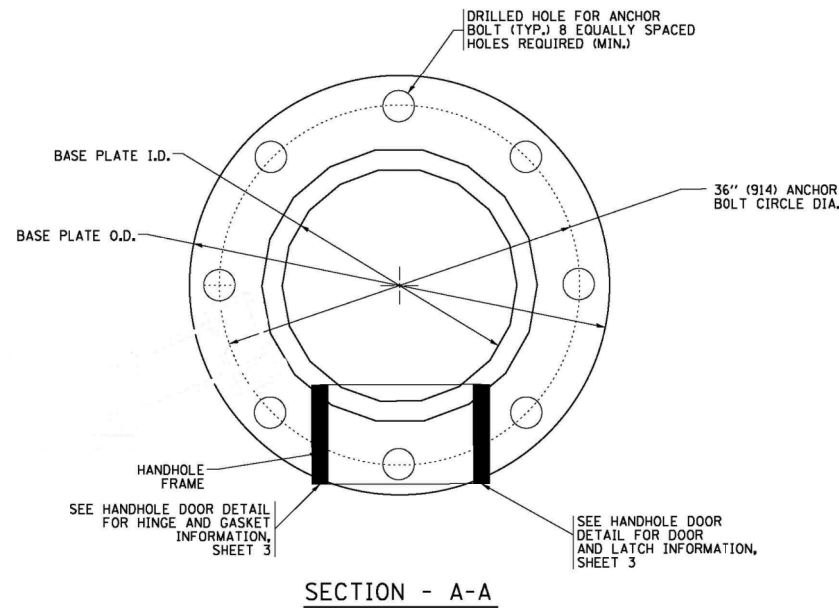
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

DAVIT LIGHT POLE			
47'-6" (14.478 m) MOUNTING HEIGHT			
SCALE: NONE	SHEET NO. 1 OF 1 SHEETS	STA.	TO STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372/373	2013-037B-R	COOK	787	220
BE-410		CONTRACT NO. 60W75		
FED. ROAD DIST. NO. 1 ILLINOIS FED. AID PROJECT				



DETAIL - "A"
3 CABLE LOWERING & SUPPORT MECHANISM SHOWN.



NOTES:

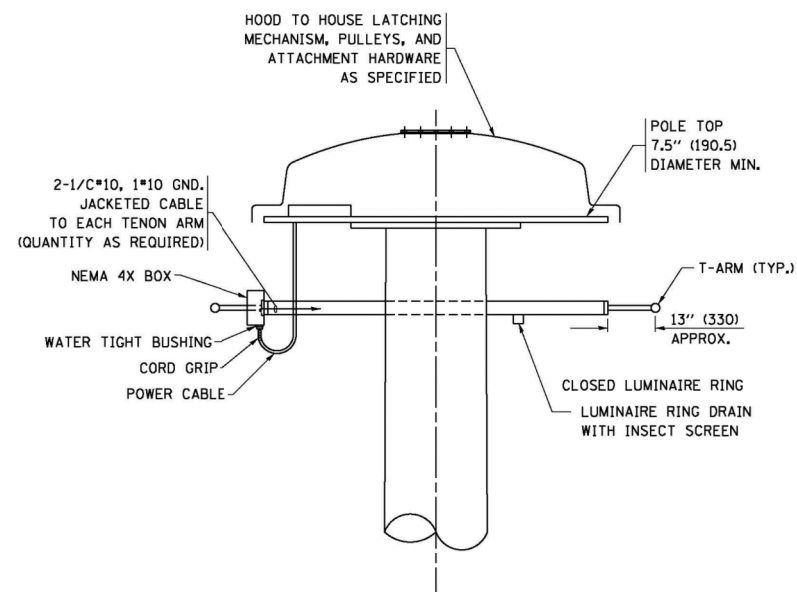
1. ALL DIMENSIONS ARE IN INCHES (MILLIMETERS) UNLESS OTHERWISE SHOWN.
2. THE DESIGN SHALL BE BASED UPON AASHTO "STANDARD SPECIFICATIONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS, LUMINAIRES AND TRAFFIC SIGNALS" CURRENT AT THE TIME THE PROJECT IS ADVERTISED AND A TOTAL COMBINED LUMINAIRE WEIGHT OF 720 LBS. (326 kg) AND HAVING A TOTAL PROJECTED AREA OF 24 SQ. FT. (7.3 sq. m).
3. ALL TOWER SHAFT COMPONENTS, INCLUDING, BUT NOT LIMITED TO THE SHAFT SECTIONS, BASE PLATE, LADDER CLIPS, HANDHOLE DOOR, HANDHOLE REINFORCING, RAIN GUTTER, AND BASE PLATE, SHALL BE FABRICATED FROM HIGH-STRENGTH, LOW ALLOY, STEEL WITH A MINIMUM YIELD STRENGTH OF 50,000 PSI (345 K PA) ACCORDING TO AASHTO M 223 (ASTM A 572 GR50)
4. THE ELECTRIC MOTOR, MOTOR GEAR REDUCER, WINCH DRUM ASSEMBLY AND AUTOMATIC SHUTOFF SWITCH OF THE LOWERING DEVICE SHALL BE ACCESSIBLE FROM THE FRONT OF THE TOWER FOR EASY REMOVAL AND MAINTENANCE. ALL COMPONENTS SHALL BE REMOVABLE THROUGH THE HANDHOLE.
5. THE LIGHT TOWER SHAFT SHALL HAVE LADDER CLIPS. CLIPS SHALL BEGIN 6 FT. (1.8 m) ABOVE THE BASE PLATE WITH ALTERNATE 1.8 m (900) AND 10 INCH (250) SPACING THEREAFTER, FOR THE ENTIRE LENGTH. THE TOP 10 FT. (3 m) OF THE POLE SHAFT SHALL HAVE 3 SETS OF CLIPS. EACH SET OF CLIPS SHALL BE 120 DEGREES APART. CLIPS SHALL BE 0.25 X 2 INCHES (6 X 50) WELDED TO THE SHAFT TO PRODUCE A SLOT 0.625 INCHES (15.9) DEEP AND 1.625 INCHES (41.3) LONG. THE TOP INSIDE EDGE SHALL BE CHAMFERED.
6. A COPPER BONDING JUMPER SHALL BOND SLIP-FIT POLE SECTIONS TOGETHER WITH A FLAT COPPER MESH AND STAINLESS STEEL GROUND LUGS.
7. ALL TOWER SHAFT HARDWARE, SUCH AS GROUND LUGS, JUNCTION BOXES, HARDWARE FOR THE HANDHOLE DOOR, INCLUDING THE HANDLE/LATCH MECHANISM, HINGE AND DOOR STOP, SHALL BE STAINLESS STEEL. ALL CONDUIT AND CONDUIT FITTINGS SHALL BE PVC COATED GALVANIZED STEEL.
8. THE ENTIRE TOWER INCLUDING THE SHAFT, HANDHOLE, HANDHOLE DOOR, BASE PLATE AND ALL OTHER ELEMENTS WELDED TO THE SHAFT SHALL BE GALVANIZED IN ACCORDANCE WITH AASHTO M 111 (ASTM A 123) AND THEN PAINTED AS SPECIFIED. THE LUMINAIRE RING SHALL BE PRIMED AND PAINTED AS SPECIFIED.
9. THE FINISH COAT SHALL BE ANSI 70, SKY GREY COLOR SAMPLE TO BE SUBMITTED FOR APPROVAL.
10. ALL MULTI-CONDUCTOR CABLES SHALL BE FITTED WITH A HEAT-SHRINK MULTI-LEG BOOT. THE BOOT SHALL MEET MILITARY SPECIFICATION MIL-I-81765/1.
11. THE LIGHT TOWER SHALL BE STRAIGHT AND CENTERED ON ITS LONGITUDINAL AXIS, UNDER NO-WIND CONDITIONS, SO WHEN EXAMINED WITH A TRANSIT FROM ANY DIRECTION, THE DEVIATION FROM THE NORMAL SHALL NOT EXCEED 1/8 IN. IN 3 FT (2 mm IN 1 m) WITHIN ANY 5 FT (1.5 m) OF HEIGHT, WITH TOTAL DEVIATION NOT TO EXCEED 3 IN. (75) FROM THE VERTICAL AXIS THROUGH THE CENTER OF THE POLE BASE.
12. PVC CONDUIT WILL NOT BE ALLOWED FOR ANY LIGHT TOWER COMPONENT.
13. COUNTER WEIGHTS TO BE INCLUDED AS A PART OF THE LIGHT TOWER PAY ITEM.

FILE NAME =	USER NAME = bauerdl	DESIGNED -	REVISED - 09-02-03
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	PLOT DATE = 2/27/2013	DATE -	REVISED - 02-27-13

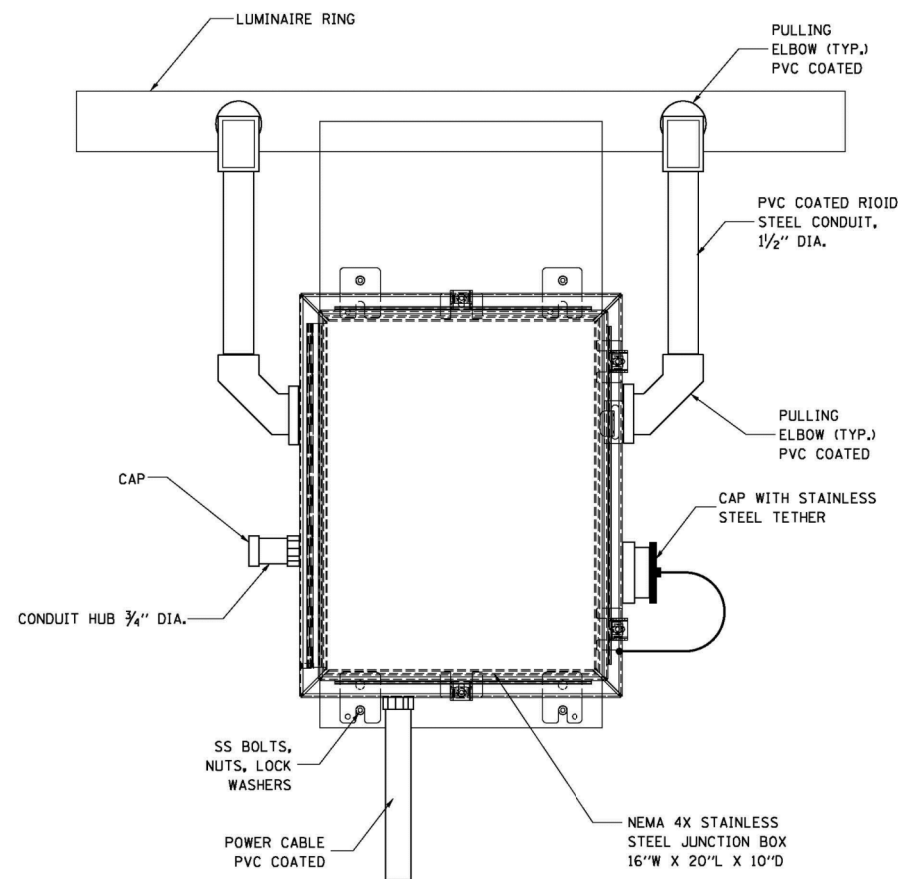
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

HIGH MAST LIGHT TOWER			
120 FT TO 140 FT (36 m TO 43 m)			
SCALE: NONE	SHEET NO. 1 OF 3 SHEETS	STA.	TO STA.

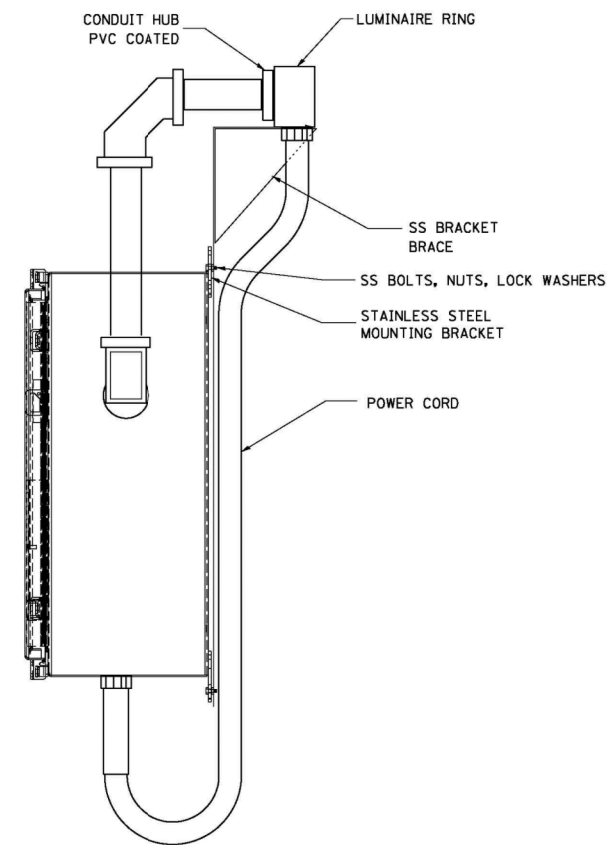
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372/373	2013-037B-R	COOK	787	221
BE-505 A		CONTRACT NO. 60W75		
FED. ROAD DIST. NO. 1 ILLINOIS FED. AID PROJECT				



DETAIL--"D"

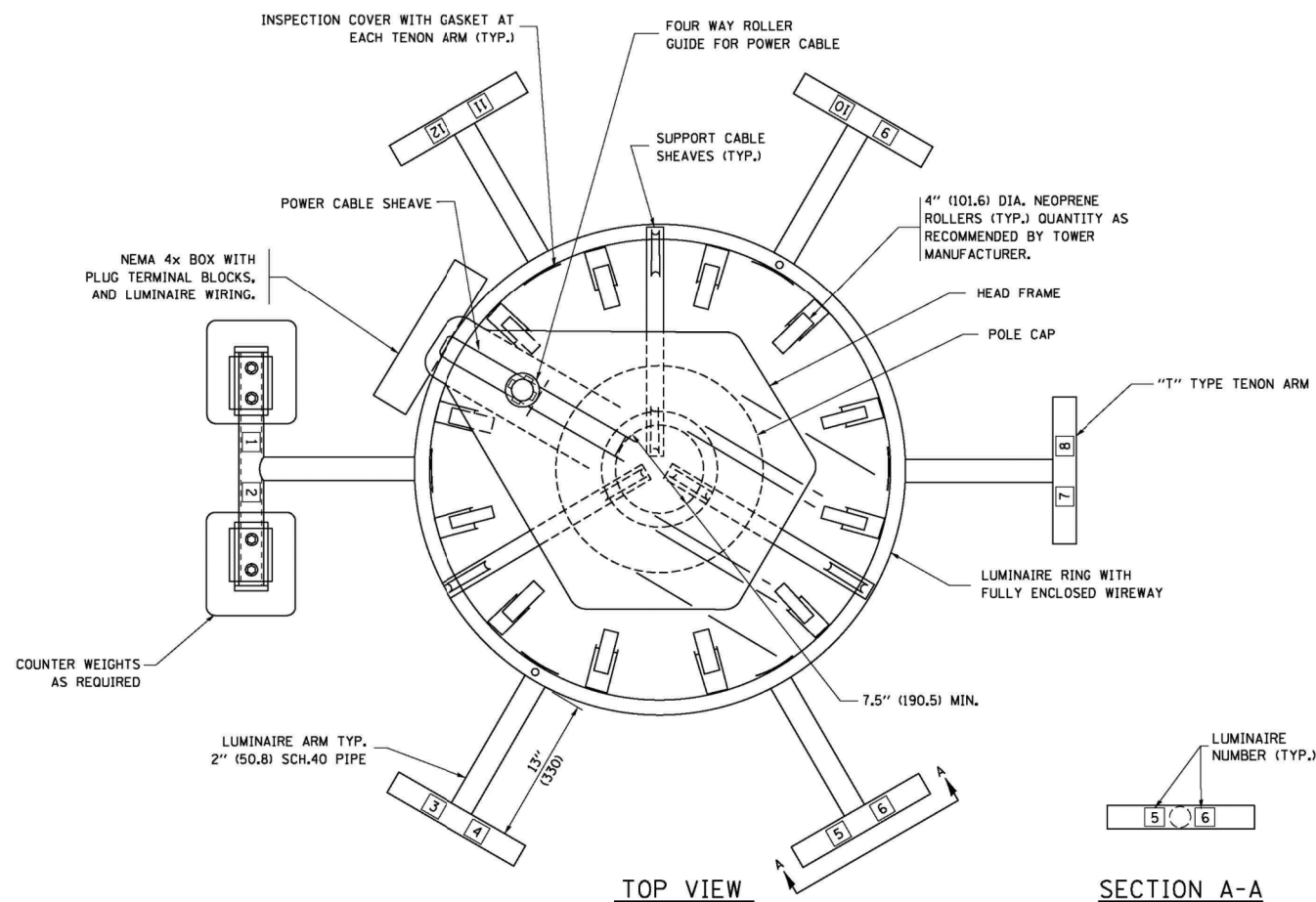


FRONT VIEW
N.T.S.



SIDE VIEW
N.T.S.

LUMINAIRE RING TERMINAL BOX



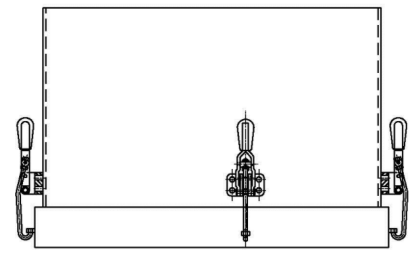
TOP VIEW

SECTION A-A

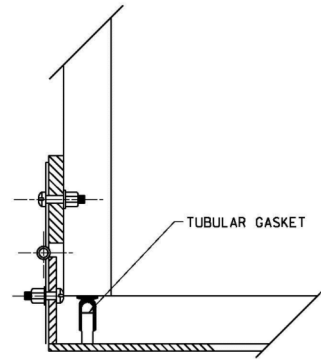
NOTES:

- LUMINAIRE WIRES SHALL EXTEND 24 INCHES (609 mm) LONGER THAN THEIR RESPECTIVE TENON ARM AND SHALL BE TRAINED BACK INTO THE ARM WHICH SHALL THEN BE CLOSED WITH A CAP AS SPECIFIED ALL WIRES SHALL BE CAPPED WITH HEAT SHRINK INSULATING BOOTS, CRIMP CAPS ARE UNACCEPTABLE. ALL RING WIRES SHALL BE TAGGED WITH WIRE MARKERS AT BOTH ENDS THE TENON ARMS SHALL ALSO BE TAGGED CORRESPONDING TO THE WIRING CONTAINED WITHIN.
- SPLICING WILL NOT BE ALLOWED WITHIN THE LUMINAIRE RING.
- ALL TOWER SHAFT HARDWARE, SUCH AS GROUND LUGS, JUNCTION BOXES, HARDWARE FOR THE HANDHOLE DOOR, INCLUDING THE HANDLE/LATCH MECHANISM, HINGE AND DOOR STOP, SHALL BE STAINLESS STEEL. ALL CONDUIT AND CONDUIT FITTINGS SHALL BE PVC COATED GALVANIZED STEEL.
- ALL MULTI-CONDUCTOR CABLES SHALL BE FITTED WITH A HEAT-SHRINK MULTI-LEG BOOT. THE BOOT SHALL MEET MILITARY SPECIFICATION MIL-I-81765/1.

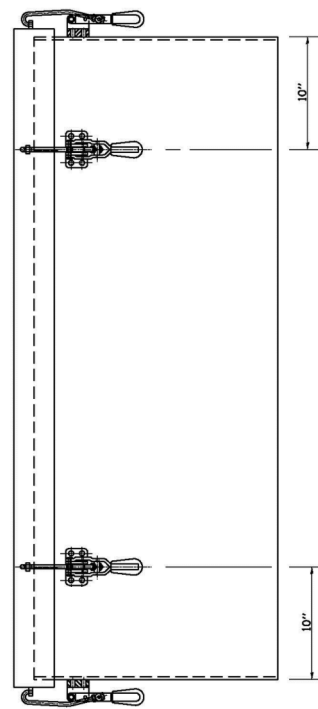
FILE NAME =	USER NAME = bauerdl	DESIGNED -	REVISED - 09-02-03	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	HIGH MAST LIGHT TOWER 120 FT TO 140 FT (36 m TO 43 m)			F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
ca:\pwork\pwork\baue\dl\0108315\be505.dgn		DRAWN -	REVISED - 03-22-10		372/373	2013-037B-R	COOK	787	222			
PLOT SCALE = 50.0000' / 1"		CHECKED -	REVISED - 09-02-10		BE-505B			CONTRACT NO. 60W75				
PLOT DATE = 2/27/2013		DATE -	REVISED - 02-27-13		SCALE: NONE	SHEET NO. 2 OF 3 SHEETS	STA.	TO STA.	FED. ROAD DIST. NO. 1 ILLINOIS FED. AID PROJECT			



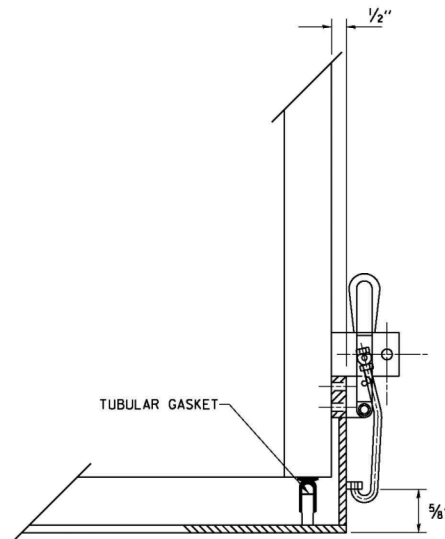
TOP VIEW



HINGE DETAIL

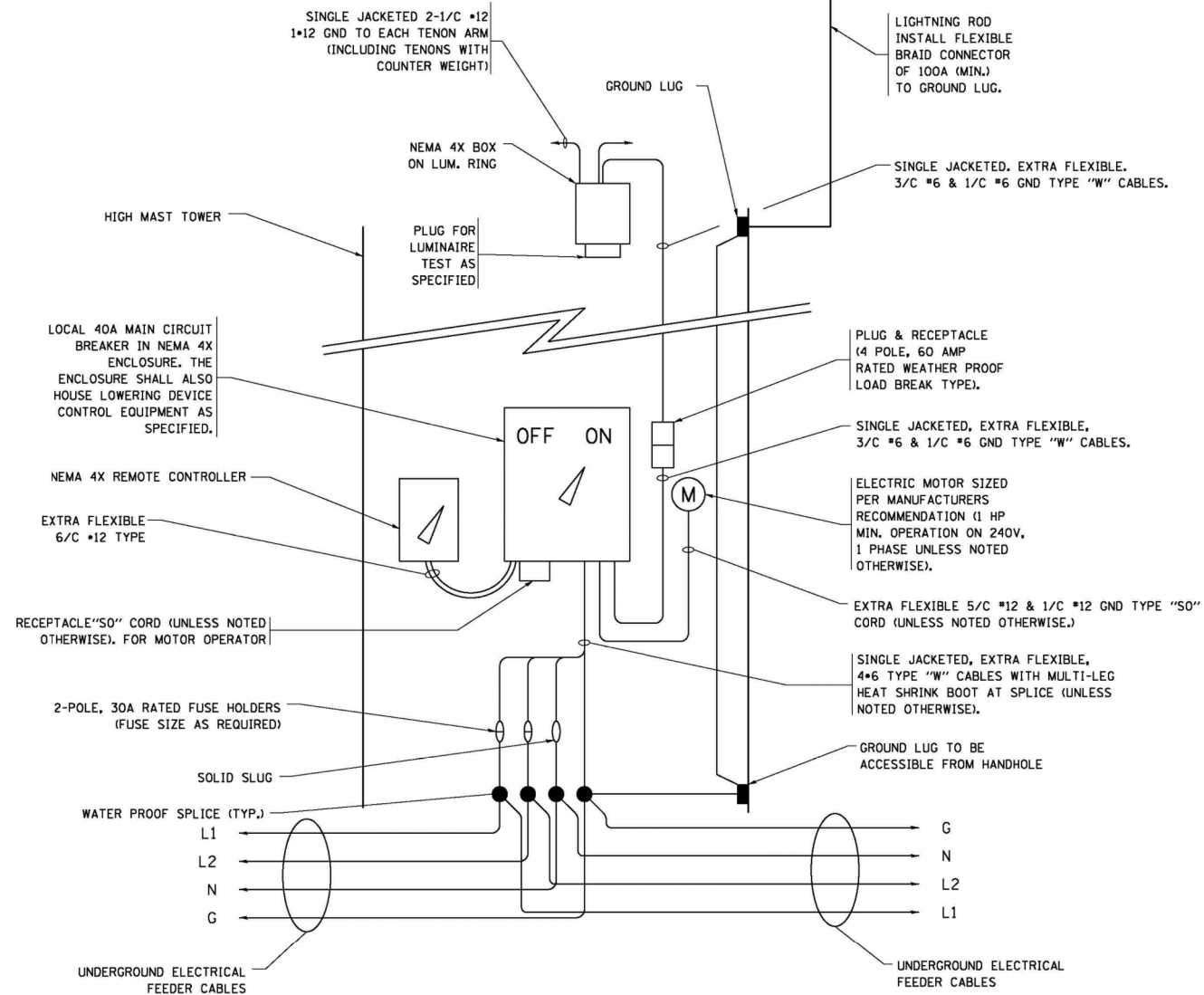


SIDE VIEW

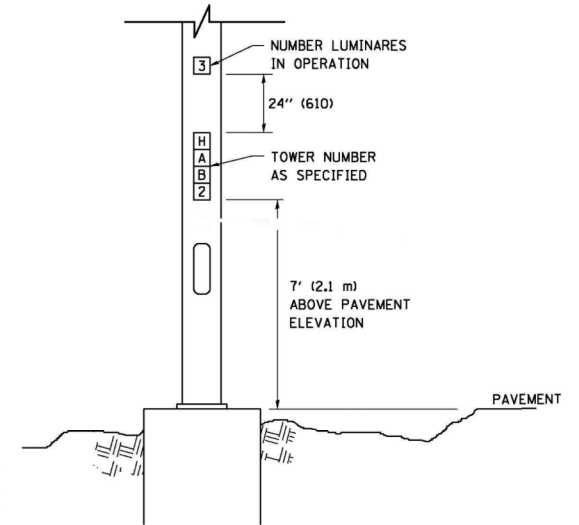


LATCH DETAIL

HANDHOLE DOOR DETAILS



HIGH MAST POLE WIRING DIAGRAM



LIGHT TOWER NUMBERING DETAIL

FILE NAME =	USER NAME = bauerdl	DESIGNED -	REVISED - 09-02-03
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		CHECKED -	REVISED - 09-02-10
		DATE -	REVISED - 02-27-13

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

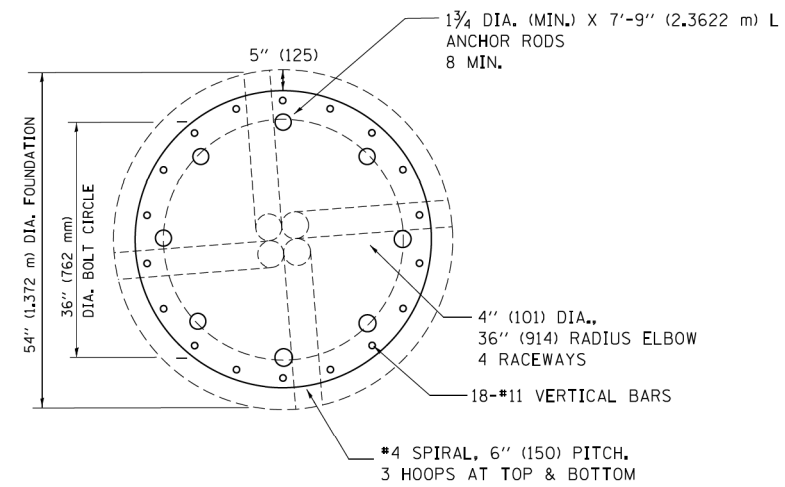
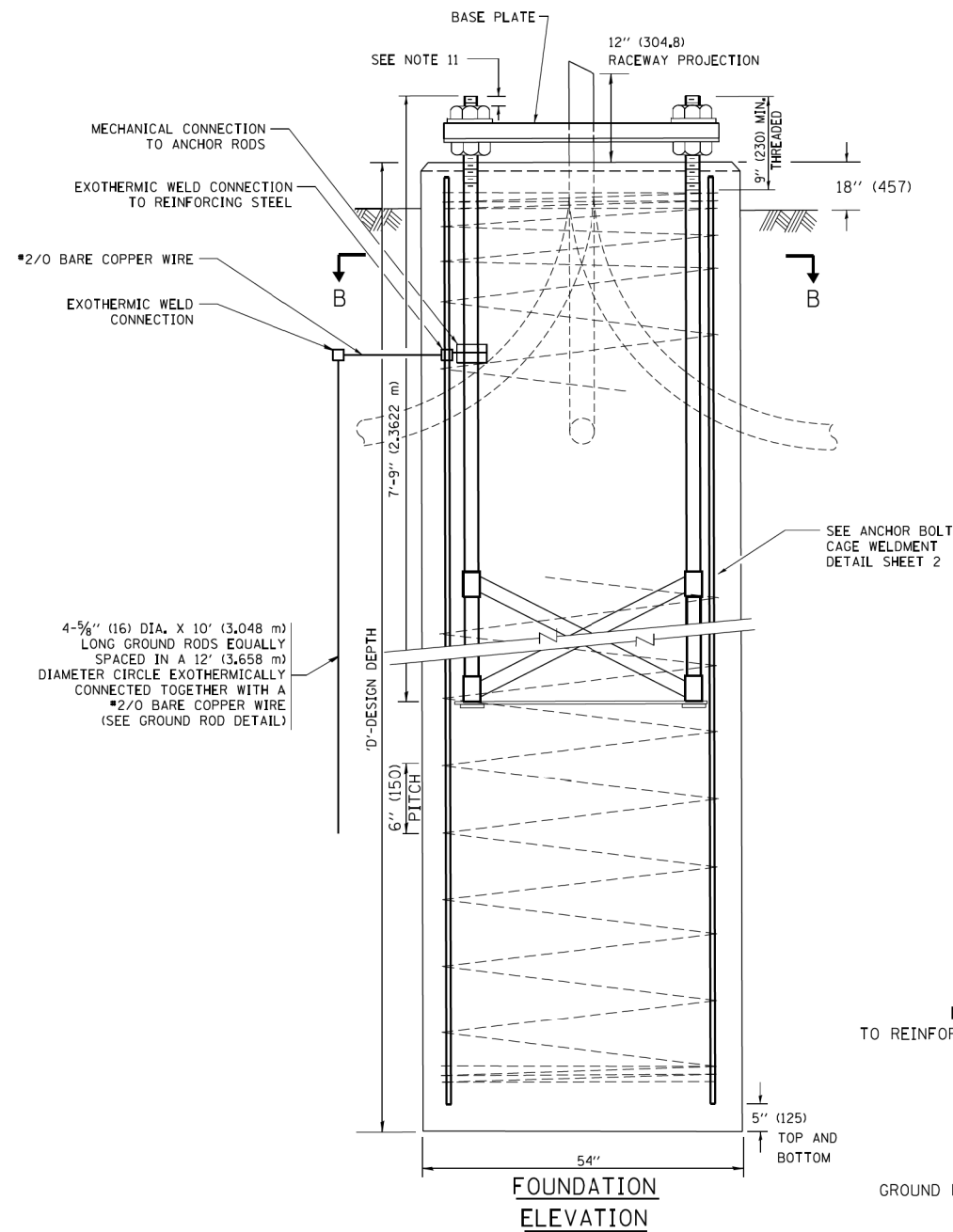
HIGH MAST LIGHT TOWER			
120 FT TO 140 FT (36 m TO 43 m)			
SCALE: NONE	SHEET NO. 3 OF 3	SHEETS	STA. TO STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372/373	2013-037B-R	COOK	787	223
BE-505 C		CONTRACT NO. 60W75		
FED. ROAD DIST. NO. 1 ILLINOIS FED. AID PROJECT				

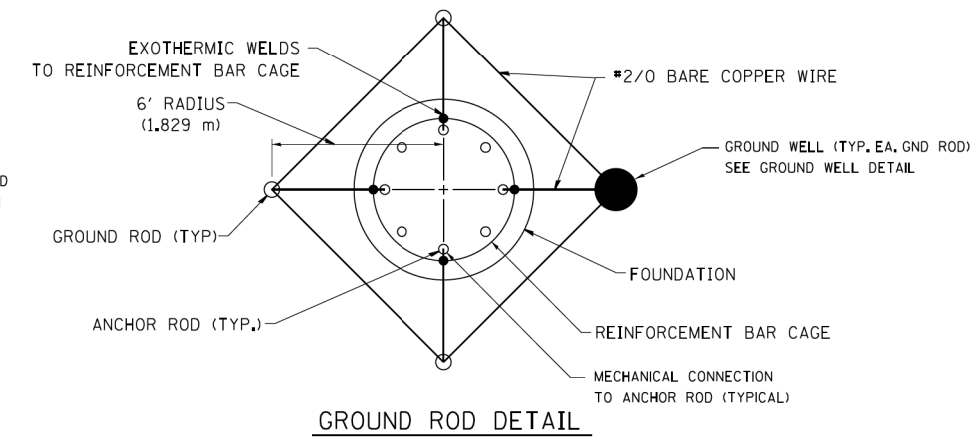
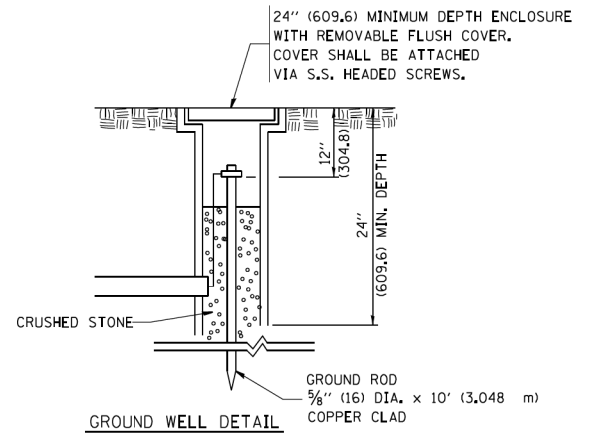
SOIL CONSISTENCY		SHAFT LENGTH (D) TABLE			
		AVERAGE STRENGTH Q _u In tsf (Q _u In kPa)	LIGHT TOWER MOUNTING HEIGHT		
			120 FT. (37 m)	130 FT. (40 m)	140 FT. (43 m)
SOFT		<0.5 (<50)	25'-0" (7.6 m)	26'-6" (8.0 m)	27'-6" (8.3 m)
	MEDIUM	0.5 TO 1 (50 TO 100)	20'-6" (6.2 m)	21'-6" (6.4 m)	22'-0" (6.7 m)
COHESIVE	STIFF	1 TO 2 (100 TO 200)	17'-6" (5.2 m)	18'-0" (5.4 m)	18'-6" (5.5 m)
	VERY STIFF	2 TO 4 (200 TO 400)	15'-0" (4.5 m)	15'-6" (4.6 m)	16'-0" (4.7 m)
	HARD	>4 (>400)	13'-6" (4.0 m)	13'-6" (4.1 m)	14'-0" (4.2 m)
		N In BLOWS/FT. (N In BLOWS/0.3m)			
	VERY LOOSE	<5 (<5)	19'-0" (6.3 m)	20'-0" (6.0 m)	20'-6" (6.2 m)
	LOOSE	5 TO 10 (5 TO 10)	17'-6" (5.7 m)	18'-0" (5.5 m)	18'-6" (5.6 m)
GRANULAR	MEDIUM	10 TO 25 (10 TO 25)	16'-6" (5.5 m)	17'-0" (5.2 m)	17'-6" (5.3 m)
	DENSE	25 TO 50 (25 TO 50)	15'-6" (5.2 m)	16'-6" (4.9 m)	16'-6" (5.0 m)
	VERY DENSE	>50 (>50)	15'-0" (4.5 m)	15'-6" (4.7 m)	16'-0" (4.8 m)

DESIGN NOTES

- (1) ALL DIMENSIONS IN INCHES (MILLIMETERS) UNLESS OTHERWISE SHOWN
- (2) THE ANCHOR RODS SHALL BE VERTICAL NO ADJUSTMENT SHALL BE ALLOWED AFTER THE FOUNDATION IS PLACED.
- (3) THE GAP BETWEEN THE FOUNDATION AND THE BASE PLATE SHALL BE ENCLOSED WITH A STAINLESS STEEL SCREEN FASTENED WITH A STAINLESS STEEL BAND.
- (4) THE TOP OF THE FOUNDATION TO 18" (450) BELOW GRADE SHALL BE FORMED.
- (5) SURFACE WATER WILL NOT BE PERMITTED TO ENTER THE HOLE AND ALL WATER WHICH MAY HAVE INFILTRATED INTO THE HOLE SHALL BE REMOVED BEFORE PLACING CONCRETE.
- (6) THE LIGHT TOWER SHALL NOT BE ERECTED UNTIL AFTER THE CONCRETE HAS BEEN CURED ACCORDING TO ARTICLE 1020.13.
- (7) ANCHOR RODS SHALL BE STRAIGHT AND SHALL BE ACCORDING TO AASHTO M 314 OR ASTM F1554, GRADE 725(GRADE 105) AND GALVANIZED ACCORDING TO ARTICLE 1006.9.
- (8) ANCHOR ROD INFORMATION SHALL BE SUBMITTED FOR APPROVAL AND SHALL BE FULLY COORDINATED FOR APPROVAL WITH TOWER MANUFACTURER REQUIREMENTS.
- (9) REINFORCEMENT BARS SHALL BE ACCORDING TO ARTICLE 1006.10
- (10) TWO ANCHOR RODS OPPOSITE EACH OTHER SHALL HAVE THE ANCHOR ROD THREADS PEENED AFTER NUTS ARE INSTALLED.
- (11) A MINIMUM OF THREE FULL THREADS SHALL REMAIN EXPOSED AFTER LIGHT TOWER IS INSTALLED.
- (12) ALL GROUNDING INDICATED IN THE PLANS SHALL BE INCLUDED IN THE COST OF THE LIGHT TOWER FOUNDATION AND SHALL NOT BE PAID FOR SEPARATELY.
- (13) CUT NUTS, OR JAM NUTS, ARE NOT ALLOWED
- (14) ANCHOR ROD QUANTITY, DIAMETER, AND LENGTH SHALL BE DETERMINED BY THE TOWER MANUFACTURER AND APPROVED BY THE ENGINEER. EACH FOUNDATION SHALL HAVE A MINIMUM OF 8 ANCHOR RODS.
- (15) COORDINATE THE ROD CIRCLE DIAMETER OF THE TOWER WITH THE DIAMETER OF THE ANCHOR ROD CAGE.
- (16) THE FOUNDATION SHALL BE POURED MONOLITHICALLY AND SHALL HAVE NO CONSTRUCTION JOINTS.



SECTION-B-B



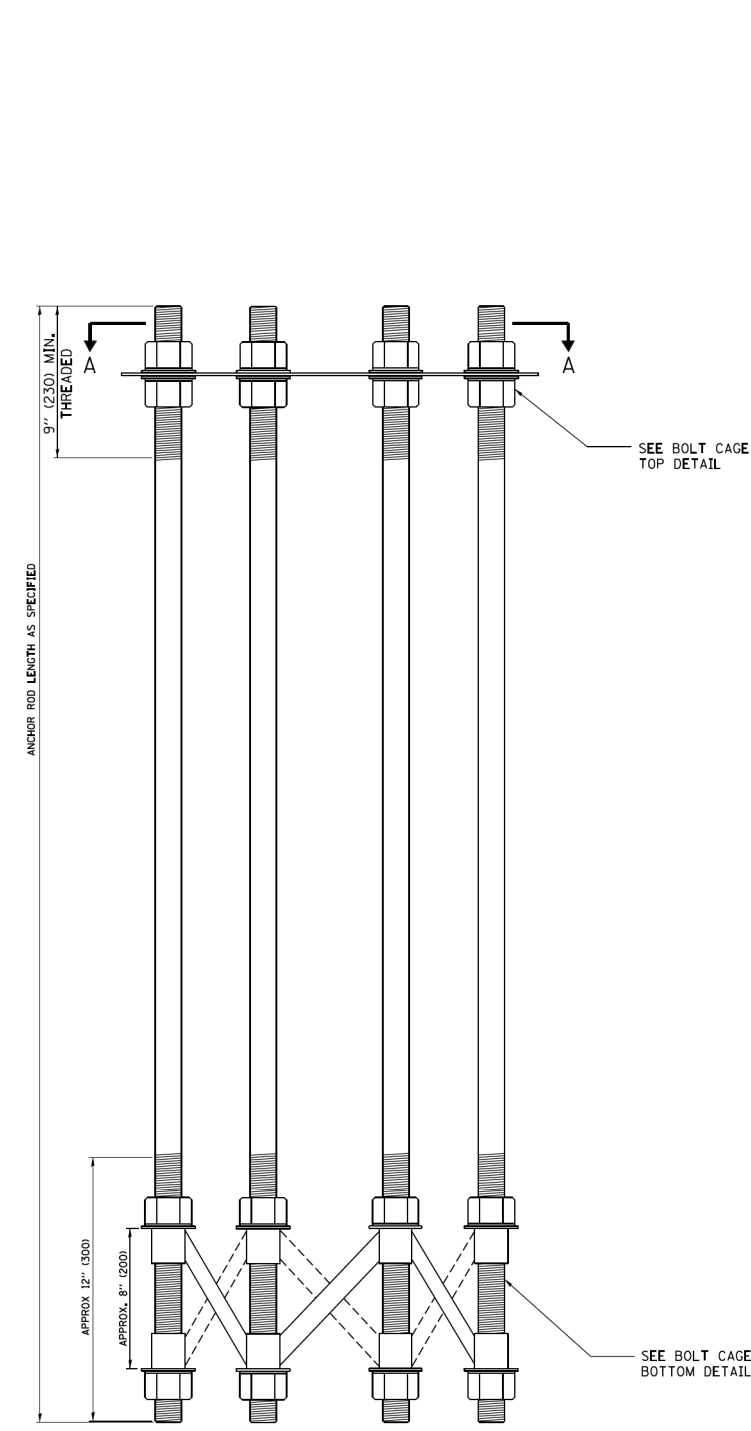
GROUND ROD DETAIL

FILE NAME =	USER NAME = bauerdl	DESIGNED - R. TOMSONS	REVISED - R. TOMSONS 09-02-10
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	PLOT SCALE = 50.000' / in.	CHECKED -	REVISED -
	PLOT DATE = 2/27/2013	DATE - 03-12-10	REVISED -

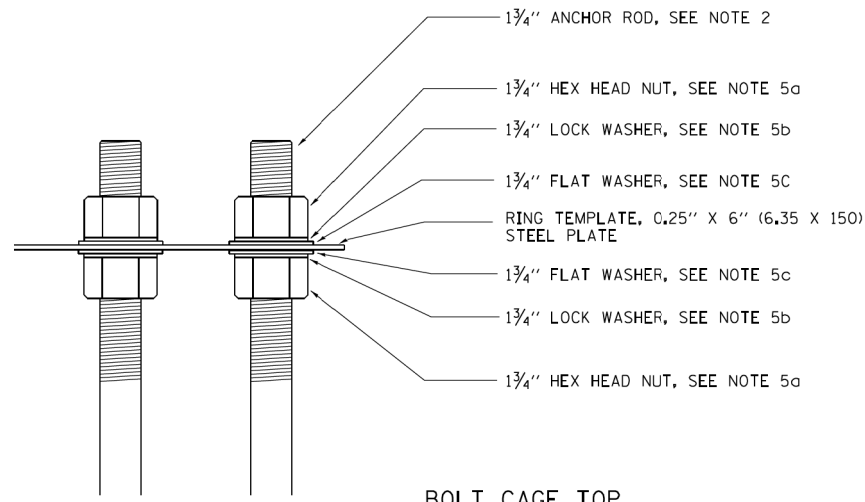
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

HIGH MAST LIGHT TOWER			
120 FT TO 140 FT FOUNDATION DETAIL			
SCALE: NONE	SHEET NO. 1 OF 2 SHEETS	STA.	TO STA.

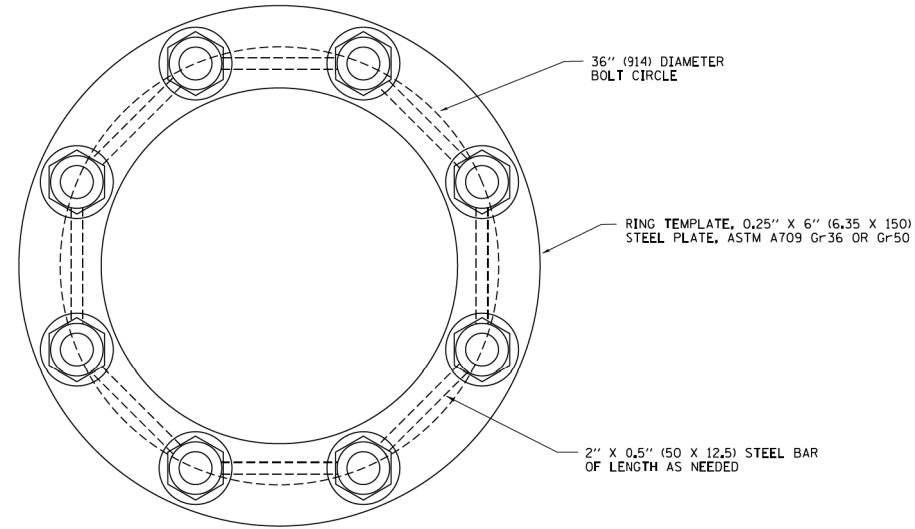
F.A.P. RTE. 372/373	SECTION 2013-037B-R	COUNTY COOK	TOTAL SHEETS 787	SHEET NO. 224
BE-506A		CONTRACT NO. 60W75		
FED. ROAD DIST. NO. 1 ILLINOIS FED. AID PROJECT				



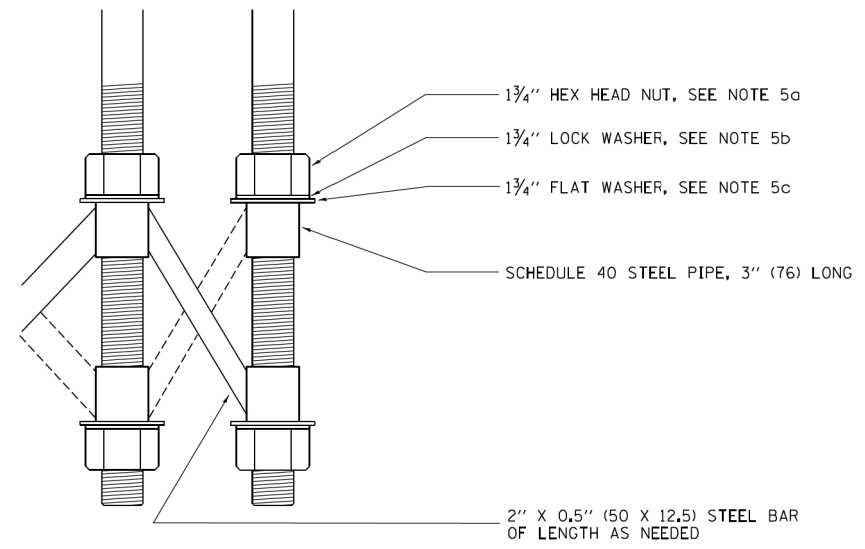
ANCHOR BOLT CAGE



BOLT CAGE TOP



SECTION A-A



BOLT CAGE BOTTOM

NOTES

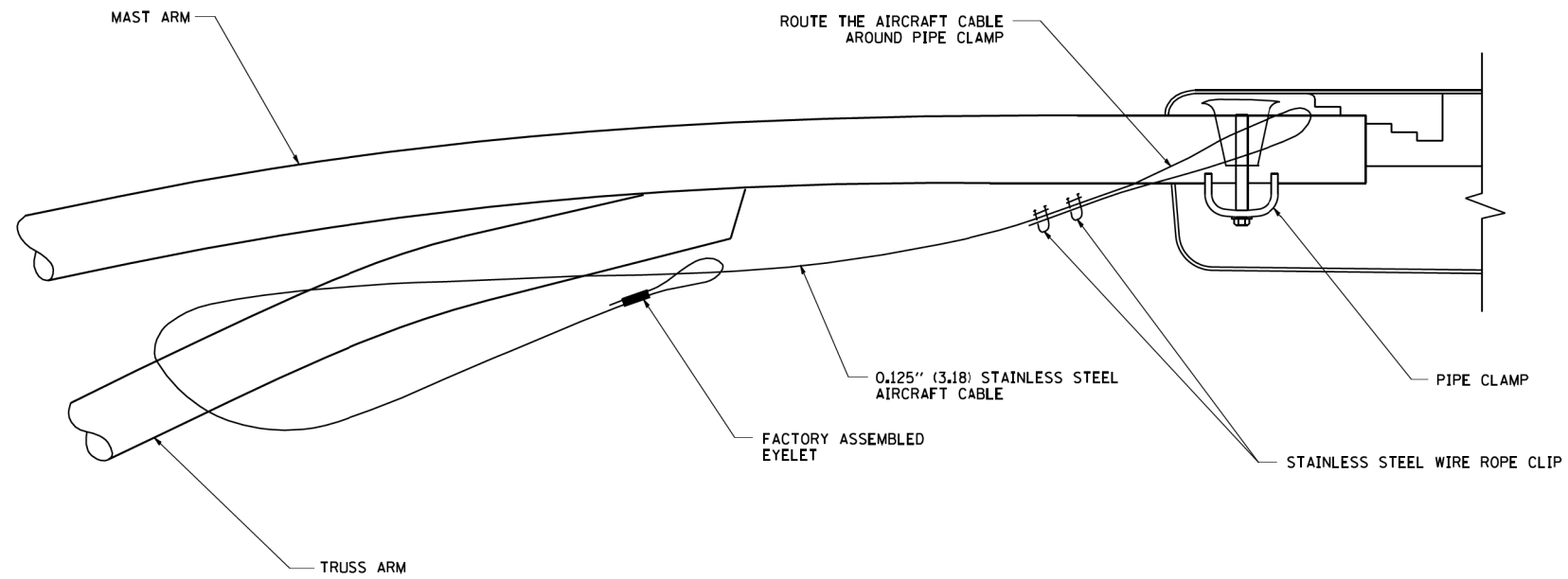
1. ALL DIMENSIONS IN INCHES (MILLIMETERS) UNLESS OTHERWISE SHOWN
2. ANCHOR RODS SHALL BE STRAIGHT AND SHALL BE ACCORDING TO AASHTO M 314 OR ASTM F1554, GRADE 725 (GRADE 105) AND GALVANIZED ACCORDING TO ARTICLE 1006.09.
3. ANCHOR ROD INFORMATION SHALL BE SUBMITTED FOR APPROVAL AND SHALL BE FULLY COORDINATED WITH TOWER MANUFACTURERS REQUIREMENTS.
4. CUT NUTS, OR JAM NUTS, ARE NOT ALLOWED
5. ANCHOR ROD CAGE HARDWARE SHALL BE IN ACCORDANCE WITH THE FOLLOWING:
 - a) 1 3/4" HEX HEAD NUTS
AASHTO M291, GRADE C, C3, D ,DH OR DH3
HOT DIPPED GALVANIZED AASHTO M 232
 - b) 1 3/4" HELICAL LOCK WASHERS
ANSI/ASME B18.21.1
I.D. 1.758 - 1.778
O.D. 2.596 MAX.
WIDTH 0.383 MIN.
THICKNESS 0.469 MIN.
HARDNESS 26-45 ROCKWELL C
HOT DIPPED GALVANIZED AASHTO M232
 - c) 1 3/4" FLAT WASHERS
AASHTO M293
O.D. 3.25
I.D. 1.875
THICKNESS 0.16 - 0.25
HARDNESS 26-45 ROCKWELL C.
HOT DIPPED GALVANIZED AASHTO M232
6. THE SHAFT LENGTHS SHALL BE BASED ON SOIL BORINGS IN THE PLANS AND OR A DETERMINATION OF SOIL CONDITIONS BY THE ENGINEER.
7. ALL FOUNDATION REINFORCEMENT STEEL SHALL BE EPOXY COATED.
8. THE FOUNDATION SHALL BE POURED MONOLITHICALLY AND SHALL HAVE NO CONSTRUCTION JOINTS.
9. ANCHOR RODS AND ALL ASSOCIATED HARDWARE ARE SHOWN AS MINIMUMS. SIZING SHALL BE DETERMINED BY THE TOWER MANUFACTURER AND APPROVED BY THE ENGINEER. EACH FOUNDATION SHALL HAVE A MINIMUM OF 8 ANCHOR RODS.

FILE NAME =	USER NAME = bauerdl	DESIGNED - R. TOMSONS 09-02-10	REVISED - R. TOMSONS 02-27-13
ct:\pwork\pwork\baueard1\d0108315\be506.dgn		DRAWN -	REVISED -
		CHECKED -	REVISED -
		DATE -	REVISED -
PLOT SCALE = 50.000' / in.			
PLOT DATE = 2/27/2013			

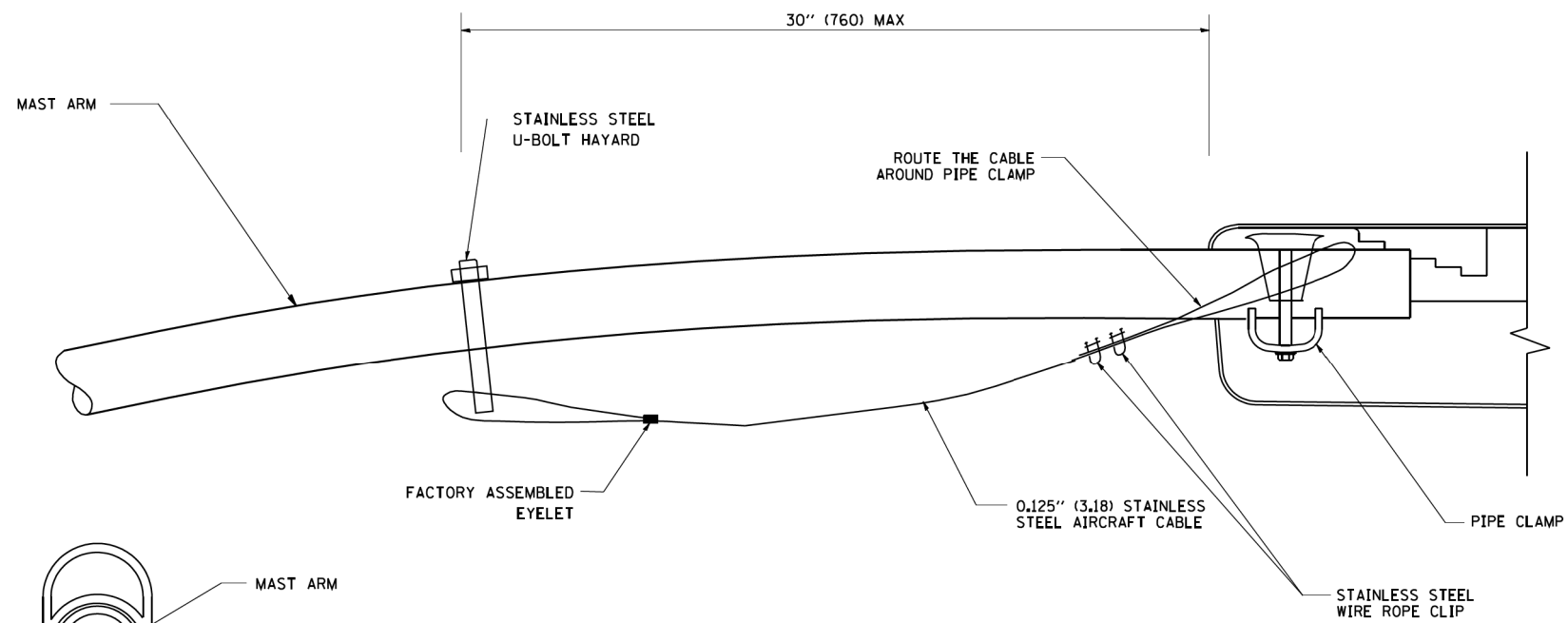
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

HIGH MAST LIGHT TOWER			
120 FT TO 140 FT FOUNDATION DETAIL			
SCALE: NONE	SHEET NO. 2 OF 2 SHEETS	STA.	TO STA.

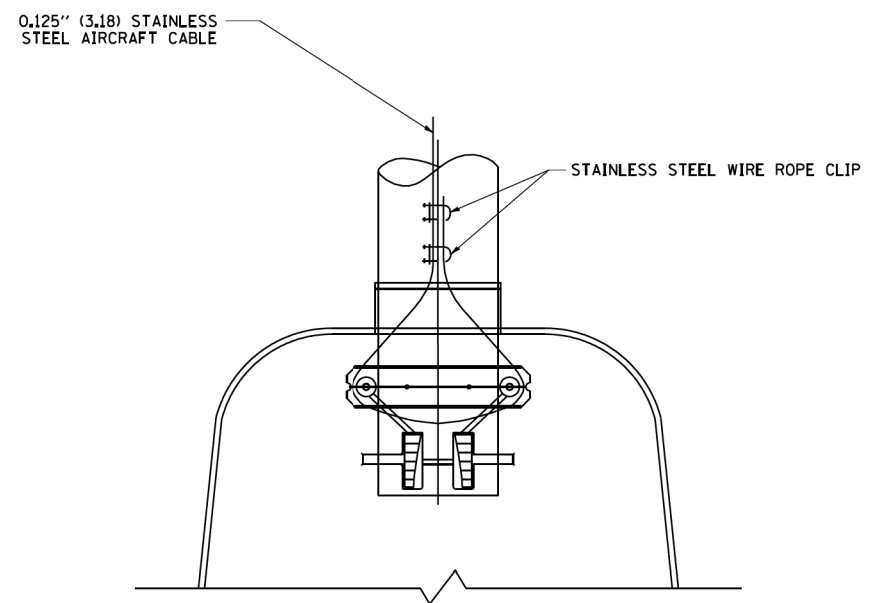
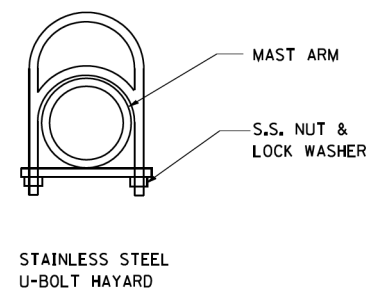
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372/373	2013-037B-R	COOK	787	225
BE-506B		CONTRACT NO. 60W75		
FED. ROAD DIST. NO. 1 ILLINOIS FED. AID PROJECT				



SIDE VIEW (TRUSS ARM)
N.T.S.



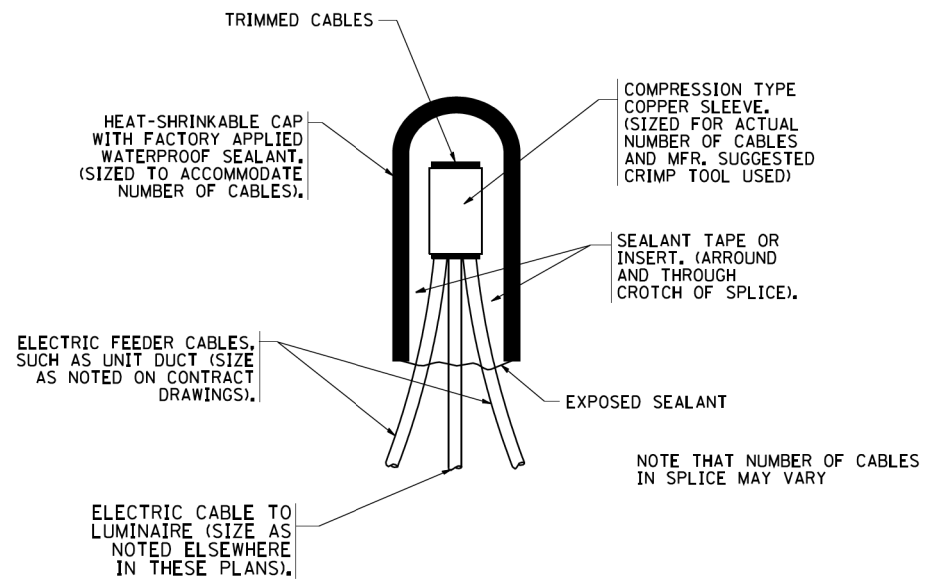
SIDE VIEW (SINGLE MEMBER OR DAVIT ARM)
N.T.S.



BOTTOM VIEW
N.T.S.

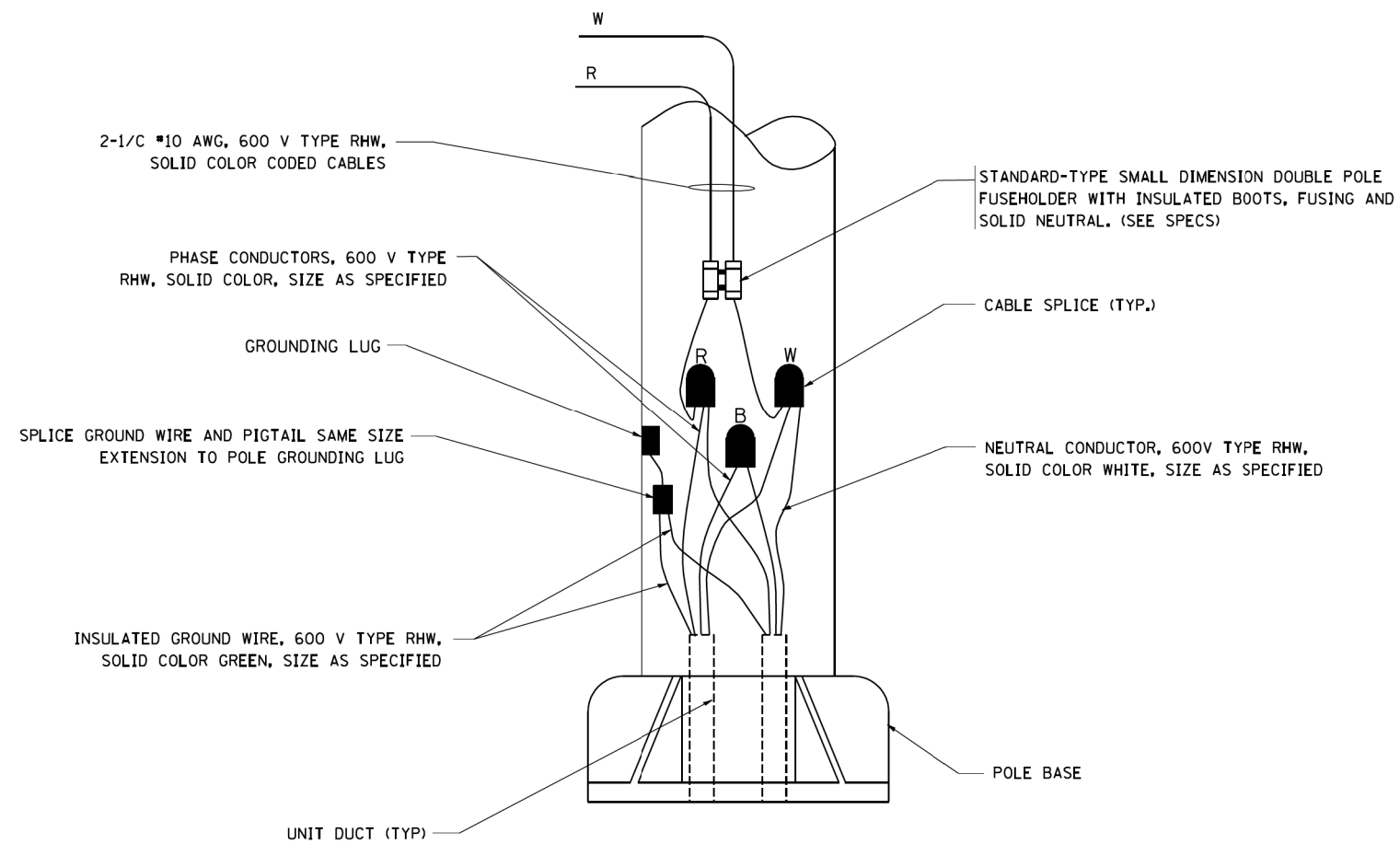
- NOTES:**
1. ALL DIMENSIONS ARE IN MILLIMETERS (INCHES) UNLESS OTHERWISE SHOWN.
 2. CONTRACTOR SHALL ADJUST THE WIRE CLIP TO ELIMINATE ANY SLACK FROM THE WIRE ROPE.
 3. THE 0.125" (3.18) STAINLESS STEEL AIRCRAFT CABLE SHALL REMAIN VISIBLE FROM THE GROUND LEVEL.
 4. THE BREAKING STRENGTH OF THE CABLE SHALL BE 1700 LBS. MIN.

FILE NAME = W:\diststd\22x34\be701.dgn	USER NAME = gegl1enobt	DESIGNED -	REVISED - 08-08-03	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	LUMINAIRE SAFETY CABLE ASSEMBLY			F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	PLOT SCALE = 50.000 ' / IN.	DRAWN -	REVISED -					372/373	2013-037B-R	COOK	787	226
PLOT DATE = 1/4/2008	CHECKED -	REVISED -	SCALE: NONE	SHEET NO. 1 OF 1 SHEETS	STA.	TO STA.	BE-701 CONTRACT NO. 60W75					
	DATE -	REVISED -	FED. ROAD DIST. NO. 1 ILLINOIS FED. AID PROJECT									



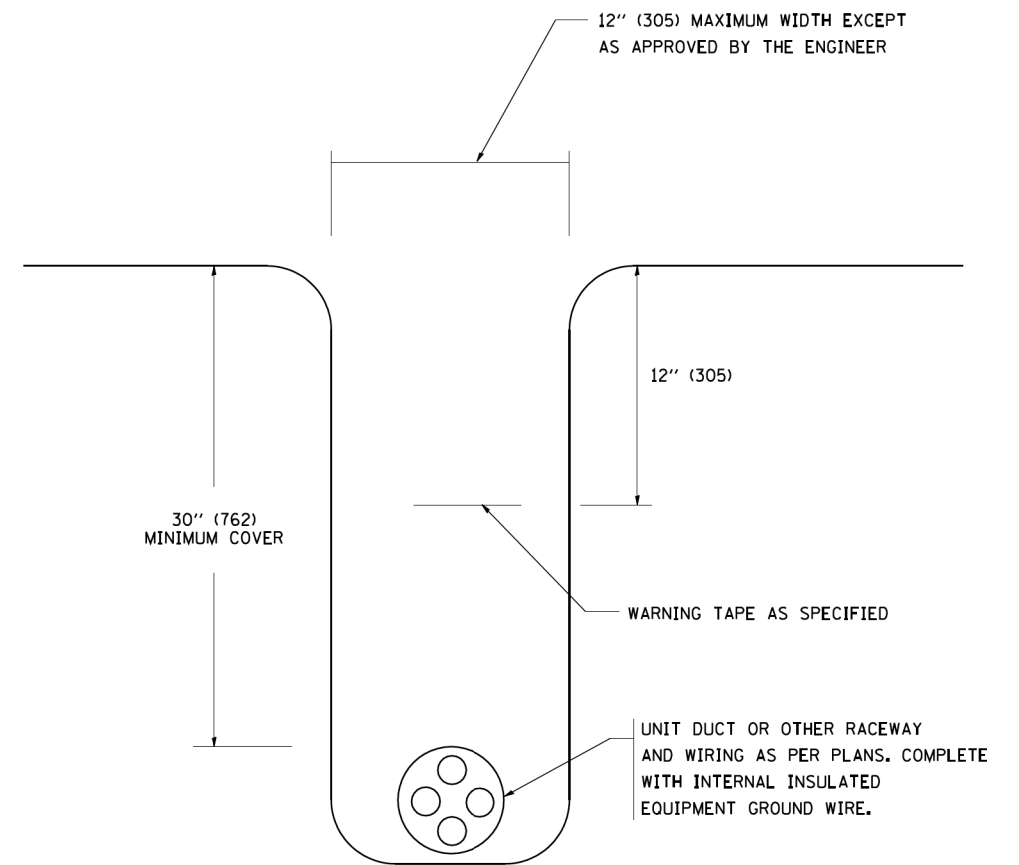
TYPICAL SPLICE DETAIL

N.T.S.



POLE WIRING DETAIL

N.T.S.



TYPICAL WIRING IN TRENCH DETAIL

N.T.S.

FILE NAME = W:\diststd\22x34\be702.dgn

USER NAME = gaglienobt

DESIGNED -	REVISED - 08-08-03
DRAWN -	REVISED -
CHECKED -	REVISED -
DATE -	REVISED -

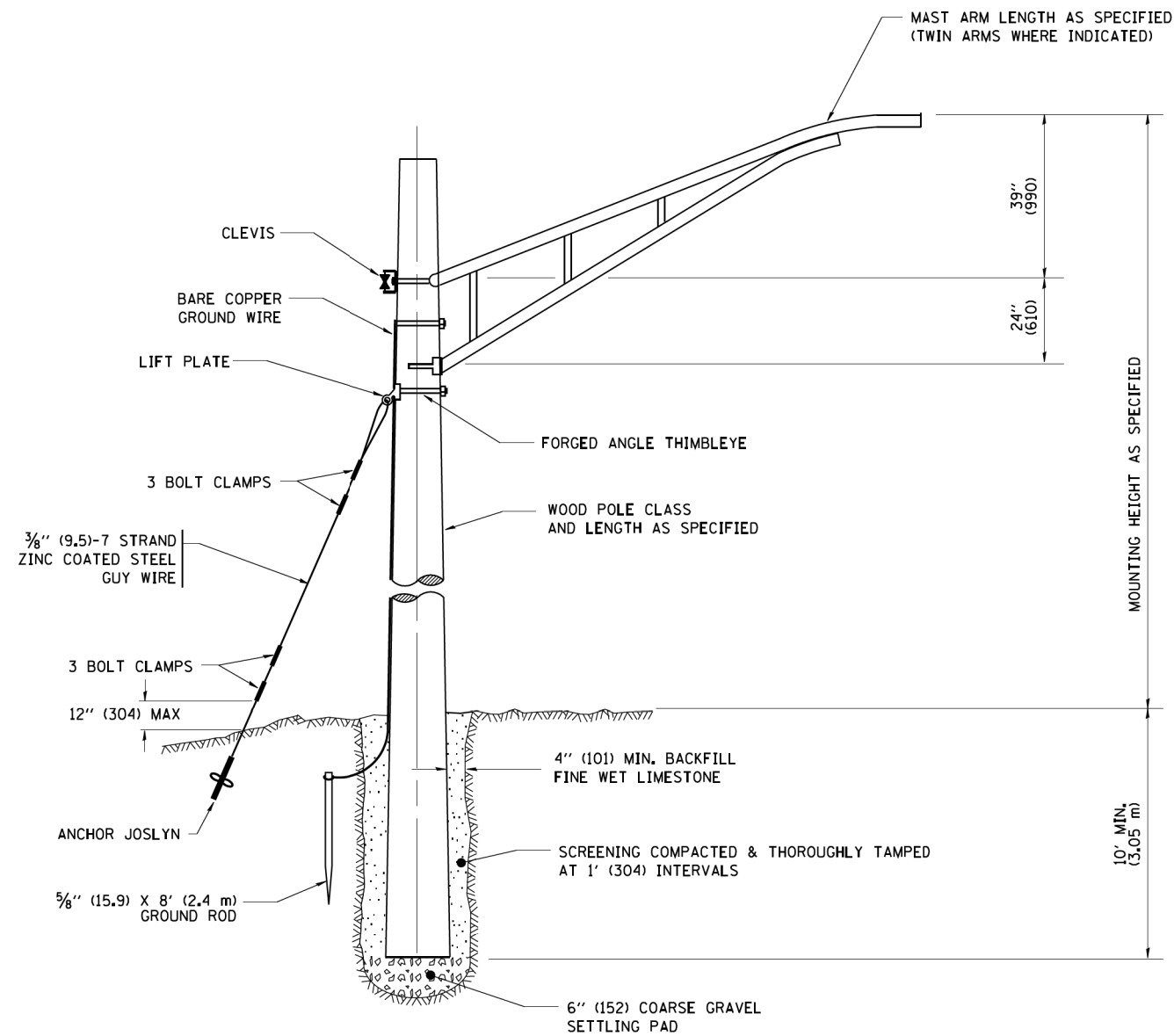
PLOT SCALE = 50.000 ' / IN.
PLOT DATE = 1/4/2008

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

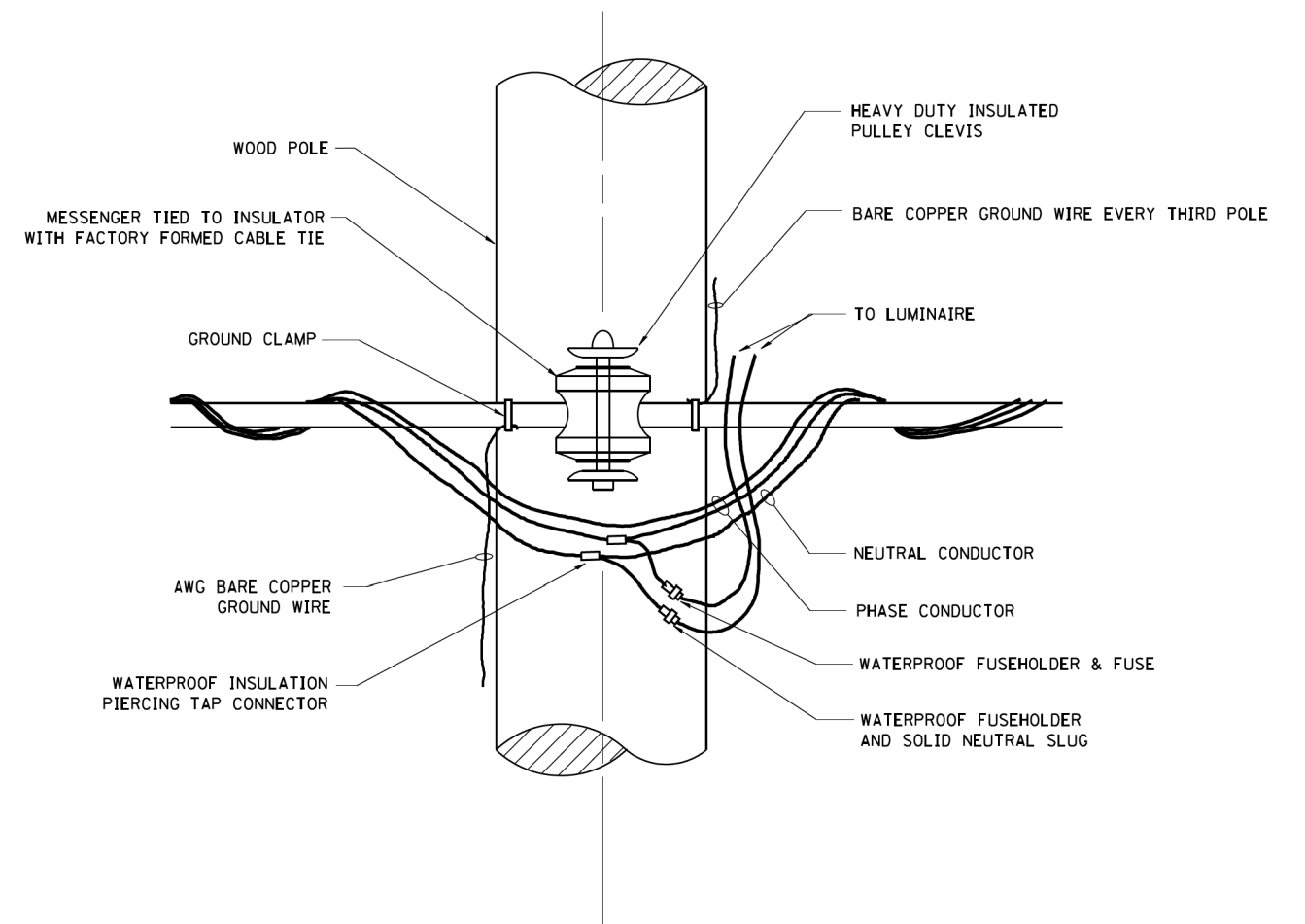
**MISC. ELECTRICAL DETAILS
SHEET A**

SCALE: NONE SHEET NO. 1 OF 1 SHEETS STA. TO STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372/373	2013-037B-R	COOK	787	227
BE-702		CONTRACT NO. 60W75		
FED. ROAD DIST. NO. 1 ILLINOIS FED. AID PROJECT				



TEMPORARY LIGHT POLE DETAIL

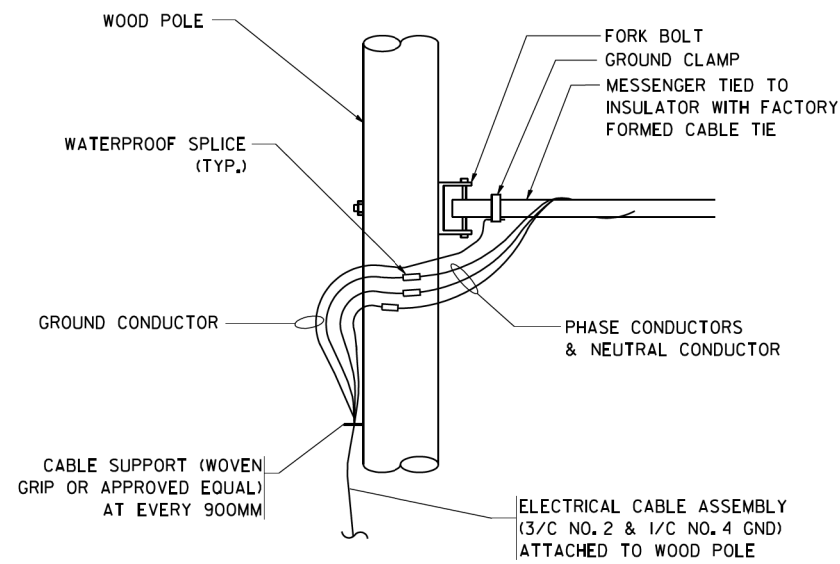


TEMPORARY LIGHT POLE ATTACHMENT DETAIL

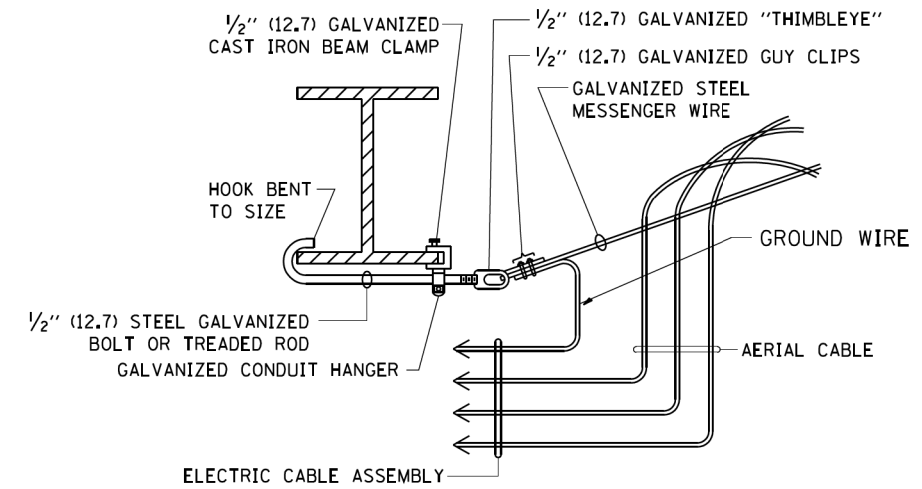
NOTES:

1. ALL DIMENSIONS IN INCHES (MILLIMETERS) UNLESS OTHERWISE INDICATED

FILE NAME = W:\diststd\22x34\be800.dgn	USER NAME = gegl1enobt	DESIGNED - DRAWN -	REVISED - 08-08-03 REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	TEMPORARY LIGHT POLE DETAILS			F.A.P. RTE. 372/373	SECTION 2013-037B-R	COUNTY COOK	TOTAL SHEETS 787	SHEET NO. 229
					SCALE: NONE	SHEET NO. 1 OF 1 SHEETS	STA.	TO STA.	BE-800		CONTRACT NO. 60W75	
					FED. ROAD DIST. NO. 1 ILLINOIS FED. AID PROJECT							



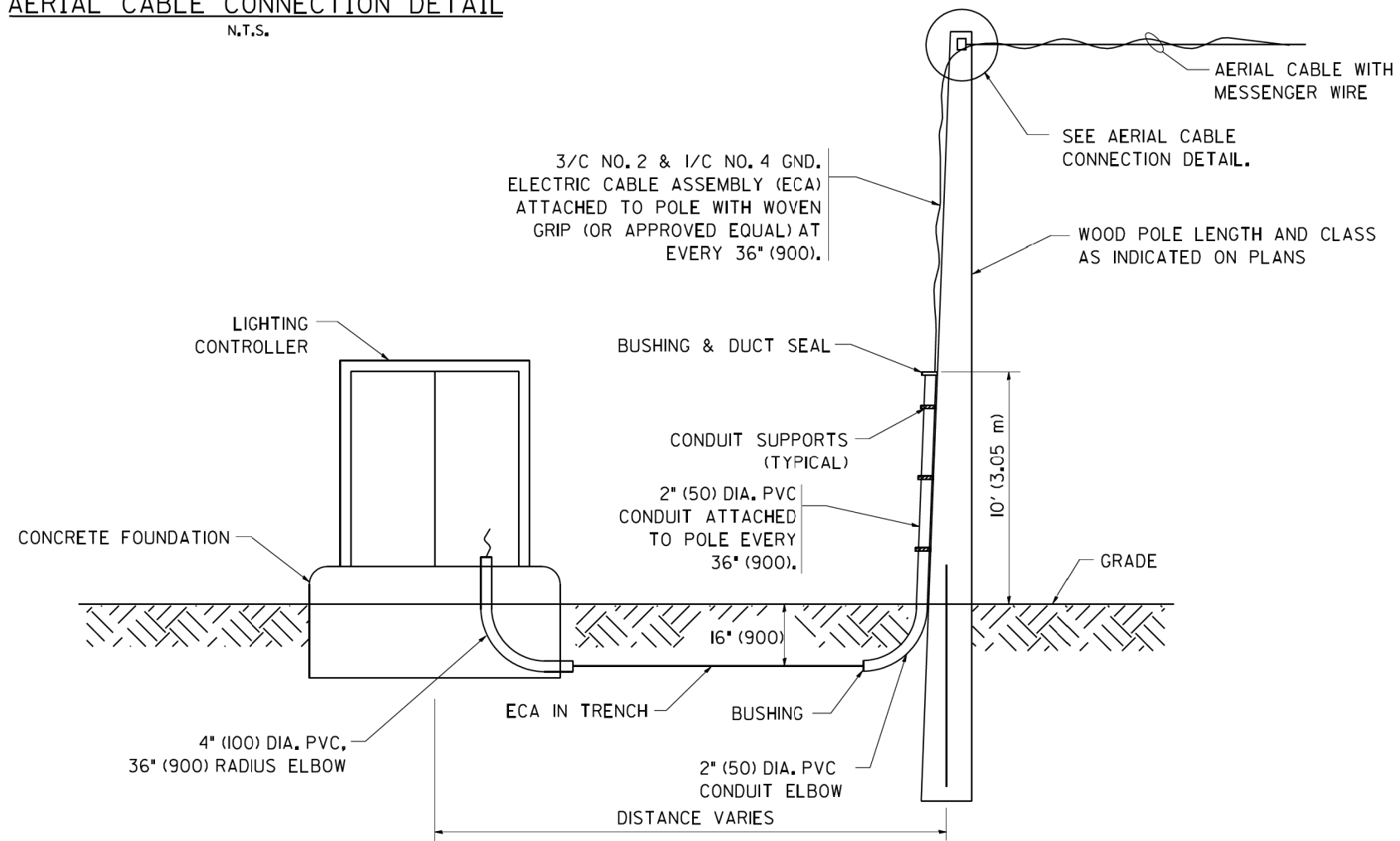
AERIAL CABLE CONNECTION DETAIL
N.T.S.



AERIAL CABLE ATTACHED TO STRUCTURE
NOT TO SCALE

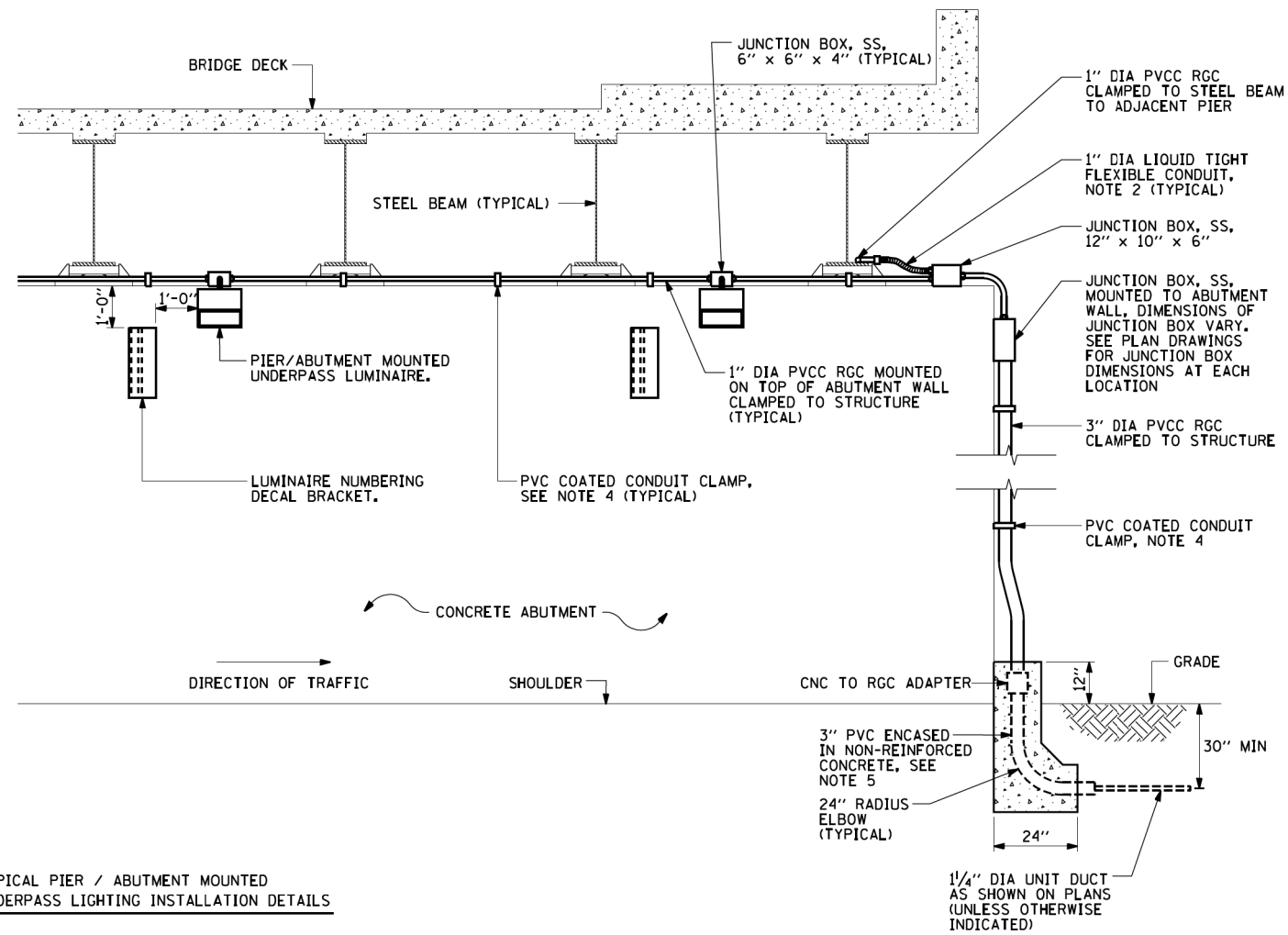
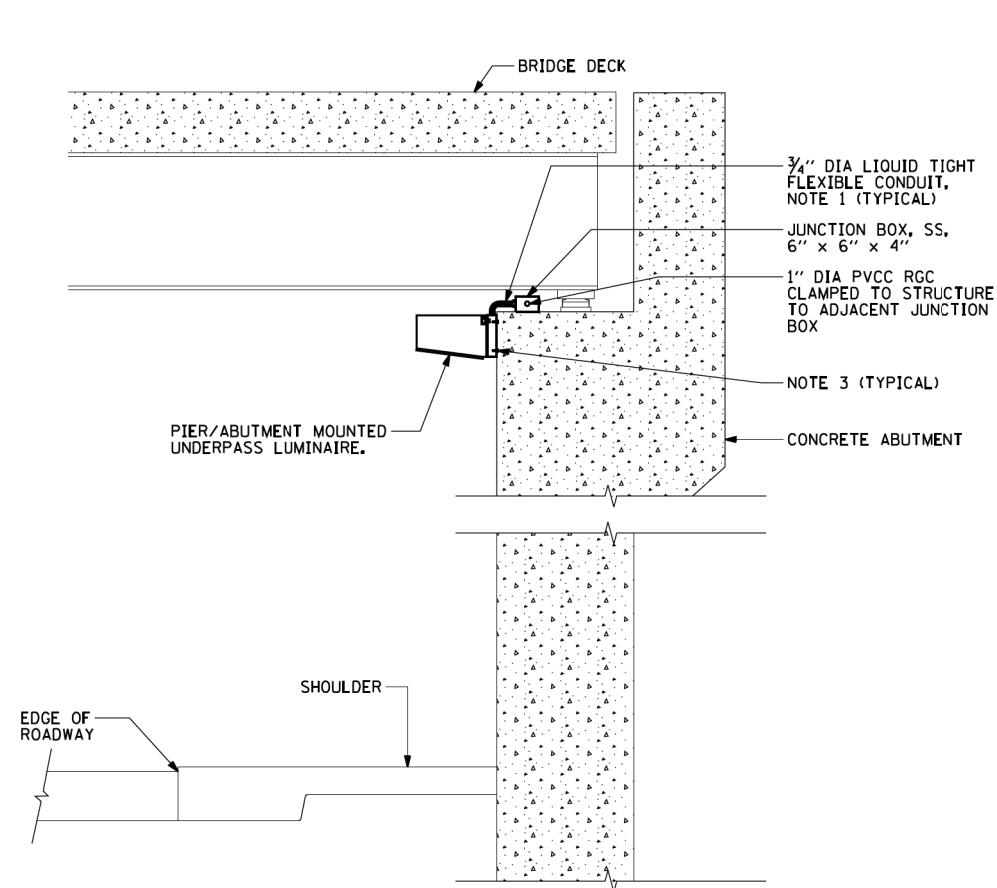
NOTES:

1. ALL DIMENSIONS IN INCHES (MILLIMETERS) UNLESS OTHERWISE INDICATED.
2. SEE PROPOSED LIGHTING PLAN FOR CONDUIT, CABLE AND ROUTING.
3. THE CONTRACTOR SHALL PROVIDE INTERMEDIATE SUPPORTS TO MAINTAIN MINIMUM CLEARANCES. REFER TO AERIAL CABLE ATTACHED TO STRUCTURE DETAIL.
4. COST OF SPLICES AND MOUNTING HARDWARE SHALL BE INCLUDED IN THE UNIT PRICE FOR AERIAL CABLE.

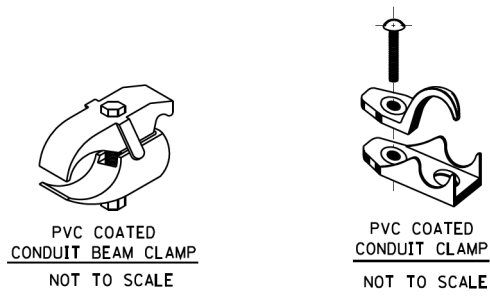
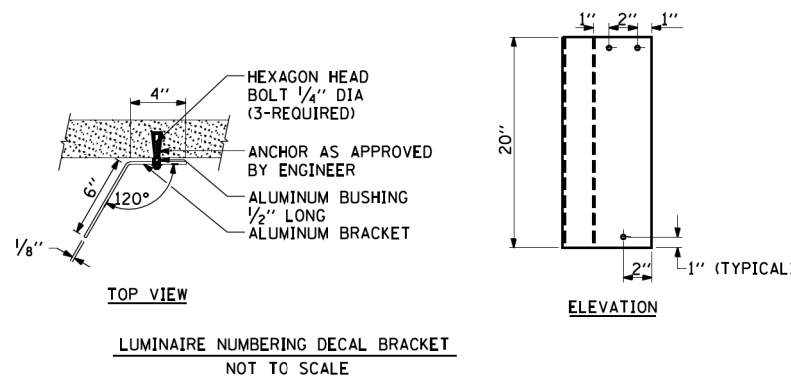


WOOD POLE TO LIGHTING CONTROLLER WIRING CONNECTION DETAIL
N.T.S.

FILE NAME = W:\diststd\22x34\be801.dgn	USER NAME = gegl1enobt	DESIGNED - DRAWN -	REVISED - 08-08-03 REVISED - REVISED - REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	TEMPORARY AERIAL CABLE INSTALLATION			F.A.P. RTE. 372/373	SECTION 2013-037B-R	COUNTY COOK	TOTAL SHEETS 787	SHEET NO. 230
PLOT SCALE = 50.000' / IN. PLOT DATE = 1/4/2008	CHECKED - DATE -	SCALE: NONE	SHEET NO. 1 OF 1 SHEETS		STA.	TO STA.	BE-801		CONTRACT NO. 60W75		FED. ROAD DIST. NO. 1 ILLINOIS FED. AID PROJECT	



TYPICAL PIER / ABUTMENT MOUNTED UNDERPASS LIGHTING INSTALLATION DETAILS



- NOTES:**
- LIQUID TIGHT FLEXIBLE METAL CONDUIT, MAXIMUM LENGTH 6'-0", TYPICAL FOR EACH INSTANCE AS SHOWN, PROVIDE PVC COATED RIGID GALVANIZED STEEL CONDUIT AS REQUIRED NOT TO EXCEED 6'-0" OF FLEXIBLE LIQUID TIGHT METAL CONDUIT. LIQUID TIGHT FLEXIBLE METAL CONDUIT WILL BE INCLUDED IN THE COST OF THE CONDUIT ATTACHED TO STRUCTURE, OF THE CORRESPONDING DIA., GALVANIZED STEEL, PVC COATED PAY ITEM EXCEPT THAT THE COST OF THE 3/4" DIA. RIGID STEEL CONDUIT AND 3/4" DIA. FLEXIBLE CONDUIT SHALL BE INCLUDED IN THE LUMINAIRE INSTALLATION.
 - UNDERPASS LUMINAIRE MOUNTED TO FACE OF PIER OR ABUTMENT WALL. MOUNTING HEIGHT OF 1" BELOW THE TOP OF PIER OR ABUTMENT WALL TYPICAL FOR ALL PIER/ABUTMENT MOUNTED UNDERPASS LUMINAIRES UNLESS OTHERWISE NOTED.
 - EXPANSION ANCHOR, POWDER ACTUATED FASTENERS WILL NOT BE ALLOWED. EXPANSION ANCHOR MUST BE SIZED IN ACCORDANCE WITH MANUFACTURERS REQUIREMENTS.
 - SECURE THE CONDUIT WITH PVC COATED CONDUIT CLAMPS OR CONDUIT BEAM CLAMPS AS SHOWN AT 5'-0" INTERVALS FOR LATERALS AND WITHIN 2'-0" MAXIMUM FROM ANY JUNCTION BOX, FLEXIBLE CONDUIT, OR CHANGE IN DIRECTION. ALL PVC COATED CONDUIT CLAMPS OR BEAM CLAMPS SHALL BE INCLUDED WITH THE COST OF THE CONDUIT ATTACHED TO STRUCTURE, OF THE CORRESPONDING DIA., GALVANIZED STEEL, PVC COATED PAY ITEM.
 - THE CONCRETE ENCASED CONDUIT TRANSITION SHALL BE INCLUDED IN THE COST OF THE GALVANIZED RIGID STEEL CONDUIT PAY ITEMS.
 - ALL CONDUIT ATTACHED TO STRUCTURE SHALL BE PVC COATED RIGID STEEL CONDUIT (PVCC RGC) TYPICAL.

FILE NAME = W:\diststd\22x34\be902.dgn	USER NAME = gegl1enobt	DESIGNED -	REVISED - 01-25-05	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	PIER /ABUTMENT MOUNTED UNDERPASS LUMINAIRE INSTALLATION DETAILS			F.A.P. RTE. 372/373	SECTION 2013-037B-R	COUNTY COOK	TOTAL SHEETS 787	SHEET NO. 231
	PLOT SCALE = 50.000 ' / IN.	DRAWN -	REVISD -		SCALE: NONE	SHEET NO. 1 OF 1 SHEETS	STA.	TO STA.	BE-902		CONTRACT NO. 60W75	
	PLOT DATE = 1/4/2008	CHECKED -	REVISD -		FED. ROAD DIST. NO. 1 ILLINOIS FED. AID PROJECT							
		DATE -	REVISD -									



SOIL BORING LOG

GSJ Job No. 10025
Page 1 of 2
Date 9/26/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St. LOGGED BY CW
SECTION 2013-037B-R LOCATION SE 1/4, SEC. 12, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD HSA/MUD ROTARY HAMMER TYPE CME Automatic

STRUCT. NO.	DEPTH	LOG	UCS	MOIST	Surface Water Elev.	Stream Bed Elev.	DEPTH	LOG	UCS	MOIST
	(ft)	(/6")	(tsf)	(%)	n/a	n/a	(ft)	(/6")	(tsf)	(%)
					Groundwater Elev.: First Encounter <u>Dry to 10.0'</u> ft Upon Completion <u>n/a</u> ft After <u> </u> Hrs. <u> </u> ft					
12.0" ASPHALT					CLAY LOAM-brown & gray-stiff to very stiff (Fill) (continued)					
CRUSHED STONE-dense (Fill)	30						2			
	16			4			3	1.2	24	
	14						3	B		
CLAY LOAM-brown & gray-stiff to very stiff (Fill)	3						2			
	3	1.0		21			3	1.1	18	
	5	P					5	B		
	2						2			
	3	2.0		17			2	1.1	24	
	4	P					4	B		
	4						2			
	5	3.0		17			2			23
	7	P					3			
	3						2			
	5	1.8		21			3	1.2	28	
	7	B					3	B		
	2						3			
	2	1.0		20			4	1.5	25	
	3	P					6	P		
	3						4			
	5	2.0		17			6	1.2	15	
	5	P					8	B		
	10				SANDY CLAY LOAM-gray-dense to very dense		21			
	8	2.2		17			26	7.1	12	
	11	B					26	B		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

GSJ Job No. 10025
Page 2 of 2
Date 9/26/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St. LOGGED BY CW
SECTION 2013-037B-R LOCATION SE 1/4, SEC. 12, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD HSA/MUD ROTARY HAMMER TYPE CME Automatic

STRUCT. NO.	DEPTH	LOG	UCS	MOIST	Surface Water Elev.	Stream Bed Elev.	DEPTH	LOG	UCS	MOIST
	(ft)	(/6")	(tsf)	(%)	n/a	n/a	(ft)	(/6")	(tsf)	(%)
					Groundwater Elev.: First Encounter <u>Dry to 10.0'</u> ft Upon Completion <u>n/a</u> ft After <u> </u> Hrs. <u> </u> ft					
SANDY CLAY LOAM-gray-dense to very dense (continued)										
							23			
							24	9.1	12	
							32	B		
							21			
							50/5"	3.5	12	
							45			
							47			
							50/5"	5.1	10	
							32			
							33	6.7	17	
End Of Boring @ -50.0'. Boring backfilled with cuttings.							50/5"	B		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

GSJ Job No. 10025
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Date 9/5/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St. LOGGED BY NW
SECTION 2013-037B-R LOCATION SE 1/4, SEC. 12, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD MUD ROTARY HAMMER TYPE CME Automatic

STRUCT. NO.	DEPTH	LOG	UCS	MOIST	Surface Water Elev.	Stream Bed Elev.	DEPTH	LOG	UCS	MOIST
	(ft)	(/6")	(tsf)	(%)	n/a	n/a	(ft)	(/6")	(tsf)	(%)
					Groundwater Elev.: First Encounter <u> </u> ft Upon Completion <u> </u> ft After <u> </u> Hrs. <u> </u> ft					
8.0" ASPHALT					CLAY LOAM-brown & gray-medium stiff to stiff (Fill) (continued)					
CLAY LOAM-dark brown & gray-stiff to very stiff (Fill)	7						3			
	4	2.5		16			3	0.7	23	
	4	B					3	B		
	5						3			
	9						3	1.1	21	
	12						4	B		
	4						6			
	6	2.7		14			5			
	8	B					9			
	3						4			
	5	1.5		18			4	1.0	21	
	6	P					4	B		
	5						4			
	5	4.5		19			5	0.7	21	
	7	P					7	B		
	4						3			
	3	1.0		21			5	1.5	19	
	4	P					6	P		
	3						3			
	3	0.5		23			3			
	4	B					3			
	3						3			
	3	0.6		19			13			
	3	B					17			
							23			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

GSI Job No. 10025
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Date 9/5/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St. LOGGED BY NW
SECTION 2013-037B-R LOCATION SE 1/4, SEC. 12, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD MUD ROTARY HAMMER TYPE CME Automatic

STRUCT. NO. Station	DEPTH H	BLOW S	UCS Qu	MOIST T	Surface Water Elev.		Groundwater Elev.:	
					n/a	ft	ft	ft
	11							
	16	7.0	12					
	22	B						
	13							
	12	4.8	12					
	13	B						
	6							
	10	6.5	12					
	15	B						
	7							
	25		13					
	23							

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

GSI Job No. 10025
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Date 9/26/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St. LOGGED BY CW
SECTION 2013-037B-R LOCATION SE 1/4, SEC. 12, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD HSA/MUD ROTARY HAMMER TYPE CME Automatic

STRUCT. NO. Station	DEPTH H	BLOW S	UCS Qu	MOIST T	Surface Water Elev.		Groundwater Elev.:	
					n/a	ft	ft	ft
	25							
	9		3					
	5							
	4							
	7	1.5	21					
	19	P						
	9							
	35		9					
	17							
	8							
	9		8					
	7							
	6							
	6							
	6							
	7		15					
	10							
	5		16					
	7	1.7						
	10	B						
	5							
	5	2.0	11					
	5							

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

GSI Job No. 10025
Page 2 of 2
Date 9/26/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St. LOGGED BY CW
SECTION 2013-037B-R LOCATION SE 1/4, SEC. 12, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD HSA/MUD ROTARY HAMMER TYPE CME Automatic

STRUCT. NO. Station	DEPTH H	BLOW S	UCS Qu	MOIST T	Surface Water Elev.		Groundwater Elev.:	
					n/a	ft	ft	ft
	2							
	9		11					
	21							
	16							
	18		15					
	25							
	23							
	50/5'		13					
	38							
	28		13					
	50/5'							

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

FILE NAME =	DESIGNED -	REVISED -
... \D160W75-sh-t-HMT-sb-NB-02.dgn	DRAWN -	REVISED -
USER NAME = jworthington	CHECKED -	REVISED -
PLOT DATE = 6/17/2015	DATE - 6/12/2015	REVISED -



STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

HIGH MAST TOWER FOUNDATION
SOIL BORINGS

SCALE: SHEET 2 OF 19 SHEETS STA. TO STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372/373	2013-037B-R	COOK	787	233
CONTRACT NO. 60W75				
ILLINOIS FED. AID PROJECT				



SOIL BORING LOG

GSJ Job No. 10025
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Date 9/18/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St. LOGGED BY CW
SECTION 2013-037B-R LOCATION SE 1/4, SEC. 12, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD HSA/MUD ROTARY HAMMER TYPE CME Automatic

STRUCT. NO.	DEPTH (ft)	SOIL	UCS (tsf)	M O I S T (%)	Surface Water Elev. (ft)	Stream Bed Elev. (ft)	DEPTH (ft)	SOIL	UCS (tsf)	M O I S T (%)
12.0" ASPHALT										
6.0" CRUSHED STONE										
CLAY LOAM-brown & gray-medium stiff to very stiff (Fill) (continued)										
	10						3			
	10			6			3	0.6		17
	8						4	B		
	6						5			
	7	2.8		17			12	2.0		23
	8	B					9	P		
	7						5			
	8	2.8		15			5	0.6		18
	11	B					5	B		
	6						5			
	50/1	1.9		19			7	1.5		18
	10	B					8	P		
	10						6			
	10	3.3		18			8	1.0		19
	7	P					8	P		
	8						6			
	4	2.0		17			9	1.0		18
	8	P					8	B		
	7						3			
	7	1.0		20			7	1.0		23
	7	B					7	P		
	5						4			
	5	1.0		18			5	0.5		21
	7	B					7	P		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

GSJ Job No. 10025
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Date 9/18/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St. LOGGED BY CW
SECTION 2013-037B-R LOCATION SE 1/4, SEC. 12, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD HSA/MUD ROTARY HAMMER TYPE CME Automatic

STRUCT. NO.	DEPTH (ft)	SOIL	UCS (tsf)	M O I S T (%)	Surface Water Elev. (ft)	Stream Bed Elev. (ft)	DEPTH (ft)	SOIL	UCS (tsf)	M O I S T (%)
12.0" ASPHALT										
CRUSHED STONE-medium dense (Fill)										
CLAY LOAM-brown & gray-medium stiff to very stiff (Fill) (continued)										
	15						4	1.0		25
	10			5			5	P		
	10						3			
SILTY CLAY LOAM-gray-loose										
	3						3	1.0		37
	12						3	P		
	9						-45			
CLAY LOAM with FRACTURED ROCK-gray-dense to very dense										
	18						22	1.5		15
	22						30	B		
	30						21			
	21						30			16
	30						50/5'			
	50/5'									

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

GSJ Job No. 10025
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Date 9/27/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St. LOGGED BY CW
SECTION 2013-037B-R LOCATION SE 1/4, SEC. 12, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD HSA/MUD ROTARY HAMMER TYPE CME Automatic

STRUCT. NO.	DEPTH (ft)	SOIL	UCS (tsf)	M O I S T (%)	Surface Water Elev. (ft)	Stream Bed Elev. (ft)	DEPTH (ft)	SOIL	UCS (tsf)	M O I S T (%)
12.0" ASPHALT										
CRUSHED STONE-medium dense (Fill)										
CLAY LOAM-brown & gray-stiff to hard (Fill)										
	15						3			
	10			5			6	2.7		19
	10						12	B		
	6						10			
	7	2.8		17			16	2.2		20
	8	B					16	B		
	7						7			
	8	2.8		15			4	4.3		17
	11	B					7	B		
	6						8			
	50/1	1.9		19			8	5.3		17
	10	B					11	B		
	10						3			
	10	3.3		18			3	1.9		20
	7	P					7	B		
	8						4			
	4	2.0		17			4	1.8		21
	8	P					7	B		
	7						6			
	7	1.0		20			8	1.6		22
	7	B					11	B		
	5						6			
	5	1.0		18			6	2.0		20
	7	B					10	B		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

GSI Job No. 10025
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Date 9/27/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St.
SECTION 2013-037B-R LOCATION SW 1/4, SEC. 12, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD HSA/MUD ROTARY HAMMER TYPE CME Automatic

Table with columns: DEPTH (ft), BLOW COUNT (blows/6"), UC (tsf), MOISTURE (%), SOIL DESCRIPTION, and ELEVATION (ft). Includes groundwater data and soil layers like FRAGMENTED ROCK and CLAY LOAM.

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

GSI Job No. 10025
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Date 9/17/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St.
SECTION 2013-037B-R LOCATION SW 1/4, SEC. 12, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD HSA/MUD ROTARY HAMMER TYPE CME Automatic

Table with columns: DEPTH (ft), BLOW COUNT (blows/6"), UC (tsf), MOISTURE (%), SOIL DESCRIPTION, and ELEVATION (ft). Includes groundwater data and soil layers like ASPHALT, CRUSHED STONE, and CLAY LOAM.

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

GSI Job No. 10025
Page 2 of 2
Date 9/17/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St.
SECTION 2013-037B-R LOCATION SW 1/4, SEC. 12, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD HSA/MUD ROTARY HAMMER TYPE CME Automatic

Table with columns: DEPTH (ft), BLOW COUNT (blows/6"), UC (tsf), MOISTURE (%), SOIL DESCRIPTION, and ELEVATION (ft). Includes groundwater data and soil layers like CRUSHED STONE and SANDY CLAY LOAM.

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

Table with columns: FILE NAME, DESIGNED, REVISED, DRAWN, CHECKED, PLOT DATE, DATE, REVISED.



STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

HIGH MAST TOWER FOUNDATION
SOIL BORINGS

SCALE: SHEET 6 OF 19 SHEETS STA. TO STA.

Table with columns: F.A.P RFE., SECTION, COUNTY, TOTAL SHEETS, SHEET NO., CONTRACT NO., ILLINOIS FED. AID PROJECT.



SOIL BORING LOG

GSI Job No. 10025
Page 2 of 2
Date 9/4/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St. LOGGED BY CW
SECTION 2013-037B-R LOCATION SW 1/4, SEC. 12, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD MUD ROTARY HAMMER TYPE Diedrich Automatic

STRUCT. NO.	DEPTH	BLOW	UCS	MOIST	Surface Water Elev.		Groundwater Elev.:
					n/a	ft	
	7				n/a		588.2
	10	23			n/a		588.2
	15						
SILT-gray-medium dense to dense (continued)							
	568.20						
SILTY GRAVEL FRACTURED-gray-very dense							
	5						
	6	26					
	10						
	17						
	564.20	50/2	9				
Auger Refusal @ -47.0'. End Of Boring. Boring backfilled with cuttings.							
	-50						
	-55						
	-60						

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

GSI Job No. 10025
Page 1 of 2
Date 8/22/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St. LOGGED BY KD
SECTION 2013-037B-R LOCATION SW 1/4, SEC. 12, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD HSA/MUD ROTARY HAMMER TYPE CME Automatic

STRUCT. NO.	DEPTH	BLOW	UCS	MOIST	Surface Water Elev.		Groundwater Elev.:
					n/a	ft	
	8				n/a		588.2
	14	14			n/a		588.2
	14						
13.0" ASPHALT							
	613.32						
5.0" CRUSHED STONE							
	612.90						
CLAY LOAM-dark brown & gray-stiff to hard (Fill)							
	5	3.8	14				
	5	P					
	5						
	5	4.4	15				
	5	B					
	3						
	3	3.4	20				
	3	B					
	2						
	4	2.3	22				
	4	P					
	-10						
	5						
	8	1.3	25				
	8	P					
	19						
	14	4.5	14				
	11	P					
	-15						
	8						
	6	4.5	17				
	6	P					
	6						
	7	3.0	13				
	7	P					
	-20						

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

GSI Job No. 10025
Page 2 of 2
Date 8/22/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St. LOGGED BY KD
SECTION 2013-037B-R LOCATION SW 1/4, SEC. 12, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD HSA/MUD ROTARY HAMMER TYPE CME Automatic

STRUCT. NO.	DEPTH	BLOW	UCS	MOIST	Surface Water Elev.		Groundwater Elev.:
					n/a	ft	
	7				n/a		588.2
	14	14			n/a		588.2
	14						
Drillers Observation: Weathered Fractured Rock (continued)							
	572.40						
Drillers Observation: Possible Bedrock							
	570.90						
End Of Boring @ -43.5'. Boring backfilled with cuttings.							
	-45						
	-50						
	-55						
	-60						

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

FILE NAME =	DESIGNED -	REVISED -
...\\D160W75-sh-t-HMT-sb-NB-08.dgn	DRAWN -	REVISED -
USER NAME = jworthington	CHECKED -	REVISED -
PLOT DATE = 6/17/2015	DATE - 6/12/2015	REVISED -



STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

HIGH MAST TOWER FOUNDATION
SOIL BORINGS

SCALE: SHEET 8 OF 19 SHEETS STA. TO STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372/373	2013-037B-R	COOK	787	239
CONTRACT NO. 60W75				
ILLINOIS FED. AID PROJECT				



SOIL BORING LOG

GSJ Job No. 10025
Page 1 of 1
Date 8/28/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St. LOGGED BY KD
SECTION 2013-037B-R LOCATION SW 1/4, SEC. 12, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD Hollow Stem Auger HAMMER TYPE Diedrich Automatic

STRUCT. NO.	DEPTH	B	L	U	M	Surface Water Elev.	Stream Bed Elev.	DEPTH	B	U	M
	(ft)	(/6")	(tsf)	(%)				(ft)	(/6")	(tsf)	(%)
BORING NO. LB-15 Station 41+73 (BL Ramp B) Offset 64.40ft Left Ground Surface Elev. 598.30 ft											
12.0" CLAYEY TOPSOIL-dark brown to black (Fill)	597.30				10			577.80			
CLAY LOAM-dark brown-hard (Fill)	8							24			
	10	4.5			10			50/4"			13
	12	P									
								575.30			
	5							50/5"			9
	5	4.5			17						
	5	P						-25			
592.80								572.80			
CRUSHED STONE-loose to medium dense (Fill)								50/3"			16
	8				6						
	7							50/4"			8
	5										
	6							50/4"			14
	4				14						
	5							-30			
	-10							567.80			
								50/3"			17
	8										
	10				3						
	8							50/3"			
								50/1"			
	6				6			564.30			
	7										
	9							-35			
	-15							562.30			
	7										
	7				7						
	10										
580.30											
SILTY CLAY LOAM-gray-medium dense											
	5										
	7	3.5			14						
	8	P						-40			
	-20										

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

GSJ Job No. 10025
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Date 8/27/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St. LOGGED BY KD
SECTION 2013-037B-R LOCATION SW 1/4, SEC. 12, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD Hollow Stem Auger HAMMER TYPE Diedrich Automatic

STRUCT. NO.	DEPTH	B	L	U	M	Surface Water Elev.	Stream Bed Elev.	DEPTH	B	U	M
	(ft)	(/6")	(tsf)	(%)				(ft)	(/6")	(tsf)	(%)
BORING NO. LB-16 Station 53+57 (BL Ramp A) Offset 57.60ft Left Ground Surface Elev. 603.40 ft											
18.0" TOPSOIL								582.90			
	3				24						
	7										
	12	4.5			16						
	8	P									
	3										
	8	2.8			17						
	5	P									
	4										
	3										
	7	2.0			17						
	17	P									
	-10							-30			
592.90								575.40			
CRUSHED STONE-medium dense to dense (Fill)											
	13										
	15				4						
	25										
	17										
	17				6						
	8										
	-15										
	10										
	10				15						
	12										
566.40											
Drillers Observation: Possible Bedrock											
	6										
	6				9						
	7										
	-20										

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

GSJ Job No. 10025
Page 1 of 1
Date 9/5/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St. LOGGED BY CW
SECTION 2013-037B-R LOCATION SE 1/4, SEC. 12, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD MUD ROTARY HAMMER TYPE Diedrich Automatic

STRUCT. NO.	DEPTH	B	L	U	M	Surface Water Elev.	Stream Bed Elev.	DEPTH	B	U	M
	(ft)	(/6")	(tsf)	(%)				(ft)	(/6")	(tsf)	(%)
BORING NO. LB-17 Station 22+85 (BL Ramp C) Offset 50.80ft Right Ground Surface Elev. 602.50 ft											
8.0" TOPSOIL-black	602.00							582.00			
	5				18						
	5	2.8			18						
	6	P									
	3										
	3	1.0			37						
	3	P									
	2										
	2	2.8			44						
	3	P									
	2										
	2	1.0			50						
	4	P									
	-10										
591.00											
SILTY CLAY-dark brown & gray-stiff (Fill)											
	4	1.0			23						
	14	P									
	12										
	9				4						
	7										
	-15										
	50/4"				6						
	50/4"				8						
	-20										

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

GSI Job No. 10025
Page 1 of 2
Date 9/12/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St. LOGGED BY TZ
SECTION 2013-037B-R LOCATION SE 1/4, SEC. 11, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD Hollow Stem Auger HAMMER TYPE CME Automatic

STRUCT. NO.	DEPTH	BL	UCS	MOIST	Surface Water Elev.	DEPTH	BL	UCS	MOIST
Station	(ft)	(/6")	(tsf)	(%)	n/a ft	(ft)	(/6")	(tsf)	(%)
BORING NO. LB-20 Station 44+38 (BL Ramp A) Offset 5.60ft Right Ground Surface Elev. 617.20 ft									
12.0" ASPHALT	618.20								
12.0" CRUSHED STONE	615.20	12	2.5	18					
CLAY to CLAY LOAM-brown & gray-stiff to hard (Fill)		6	P						
		3							
		3	1.5	23					
		4	P						
		-3							
		3							
		5	2.8	22					
		6	P						
		5							
		7	4.5	12					
		12	P						
		-10							
		3							
		4	2.9	13					
		8	B						
		3							
		6	2.0	20					
		7	B						
		-15							
		45							
		32	2.5	15					
		19	P						
		6							
		12							
		12							
		50/4"							

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

GSI Job No. 10025
Page 2 of 2
Date 9/12/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St. LOGGED BY TZ
SECTION 2013-037B-R LOCATION SE 1/4, SEC. 11, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD Hollow Stem Auger HAMMER TYPE CME Automatic

STRUCT. NO.	DEPTH	BL	UCS	MOIST	Surface Water Elev.	DEPTH	BL	UCS	MOIST
Station	(ft)	(/6")	(tsf)	(%)	n/a ft	(ft)	(/6")	(tsf)	(%)
BORING NO. LB-20 Station 44+38 (BL Ramp A) Offset 5.60ft Right Ground Surface Elev. 617.20 ft									
SAND & GRAVEL-gray-dense (continued)	578.70								
		50							
SANDY CLAY LOAM with FRACTURED ROCK-gray-very dense		50/2"	4.5	9					
		50/2"							
		50/2"							
		19	4.5	6					
		-45							
		49							
		50/5"	4.5	11					
		18	P						
		23							
		18							
		39	4.5	10					
		6	P						
		567.20							
End Of Boring @ -50.0'. Boring backfilled with cuttings.									
		6							
		7							
		12							
		16							
		11							
		7							
		-55							
		6							
		16							
		18							
		27							

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

GSI Job No. 10025
Page 1 of 2
Date 9/12/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St. LOGGED BY CW
SECTION 2013-037B-R LOCATION SE 1/4, SEC. 11, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD HSA/MUD ROTARY HAMMER TYPE CME Automatic

STRUCT. NO.	DEPTH	BL	UCS	MOIST	Surface Water Elev.	DEPTH	BL	UCS	MOIST
Station	(ft)	(/6")	(tsf)	(%)	n/a ft	(ft)	(/6")	(tsf)	(%)
BORING NO. LB-21 Station 24+36 (BL Ramp G) Offset 3.80ft Right Ground Surface Elev. 620.10 ft									
13.0" ASPHALT									
		4							
5.0" CRUSHED STONE	618.60								
CLAY LOAM-brown & gray-stiff to hard		4	4.1	17					
		4	B						
		4							
		6	5.7	17					
		8	B						
		-5							
		5							
		6	4.0	19					
		7	B						
		3							
		4	1.3	18					
		4	B						
		-10							
		50/3"							
		609.60							
SANDY CLAY LOAM with STONE-dark brown-medium dense (Fill)		4							
		5							
		5							
		11							
		11							
		7							
		-15							
		4							
		14							
		14							
		18							
		12							
		5							
		-20							

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

GSJ Job No. 10025
Page 2 of 2
Date 9/13/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St. LOGGED BY NW
SECTION 2013-037B-R LOCATION SW 1/4, SEC. 12, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD HSA/MUD ROTARY HAMMER TYPE CME Automatic

STRUCT. NO. Station	DEPTH H	BLOW S	UCS Qu	MOIST T	Surface Water Elev.		Groundwater Elev.:
					n/a ft	n/a ft	
							Dry to 10.0' ft
BORING NO. LB-23							n/a ft
Station 38+49 (BL IL171)							
Offset 68.70ft Right							
Ground Surface Elev. 622.20 ft							
CLAY LOAM-gray-hard	581.70						
	9						
	13	4.7	18				
	17	B					
	7						
	10	5.0	16				
	12	B					
SILTY LOAM-gray-dense	576.70						
	12						
	18						
	23		19				
SANDY CLAY LOAM with GRAVEL-gray-dense	574.20						
	14						
	19		10				
End Of Boring @ -50.0'. Boring backfilled with cuttings.	572.20	-50	25				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

GSJ Job No. 10025
Page 1 of 1
Date 9/9/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St. LOGGED BY CW
SECTION 2013-037B-R LOCATION SE 1/4, SEC. 11, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD MUD ROTARY HAMMER TYPE Diedrich Automatic

STRUCT. NO. Station	DEPTH H	BLOW S	UCS Qu	MOIST T	Surface Water Elev.		Groundwater Elev.:
					n/a ft	n/a ft	
							Dry to 10.0' ft
BORING NO. LB-24							n/a ft
Station 12+31 (BL Ramp G)							
Offset 32.30ft Right							
Ground Surface Elev. 598.00 ft							
12.0" TOPSOIL-black	597.00			21			
CLAY LOAM-dark brown to black-very loose to loose		5		23			
		4					
		3					
		1		18			
		2					
		2					
		2	1.0	26			
		3	P				
		3					
		3		13			
		4					
SILTY CLAY-dark brown to black-medium stiff (Fill)	587.50			38			
		3	0.6				
		4	B				
CLAY LOAM with STONE-dark brown & gray-medium dense (Fill)	585.00			11			
		6					
		5					
CRUSHED STONE-medium dense to very dense (Fill)	582.50			19			
		5					
		4					
		6					
		28					
		34		8			
		36					

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

GSJ Job No. 10025
Page 1 of 2
Date 9/6/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St. LOGGED BY NW
SECTION 2013-037B-R LOCATION SE 1/4, SEC. 12, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD HSA/MUD ROTARY HAMMER TYPE CME Automatic

STRUCT. NO. Station	DEPTH H	BLOW S	UCS Qu	MOIST T	Surface Water Elev.		Groundwater Elev.:
					n/a ft	n/a ft	
							Dry to 10.0' ft
BORING NO. LB-25							n/a ft
Station 38+69 (BL IL171)							
Offset 59.80ft Left							
Ground Surface Elev. 622.00 ft							
12.0" ASPHALT	621.00						
6.0" CRUSHED STONE	620.50			7			
CLAY to CLAY LOAM-brown & gray-very stiff to hard (Fill)		2	3.0	15			
		4	P				
		2					
		4	2.3	22			
		4	B				
		4					
		2					
		3	4.9	16			
		8	B				
		9					
		7		10			
		10					
SILTY CLAY LOAM with GRAVEL-gray-medium dense (Fill)	614.00			9			
		7					
		12					
		6	1.3	21			
		7	B				
		4					
		6	1.8	19			
		10	P				
		5					
		7		22			
		13					
		6					
		7	1.8	17			
		7	P				
		5					

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

GSJ Job No. 10025
Page 2 of 2
Date 9/6/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St. LOGGED BY NW
SECTION 2013-037B-R LOCATION SE 1/4, SEC. 12, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD HSA/MUD ROTARY HAMMER TYPE CME Automatic

STRUCT. NO.	DEPTH	BL	UCS	MOIST	Surface Water Elev.	Stream Bed Elev.
BORING NO. LB-25					n/a	n/a
Station 38+69 (BL IL171)						
Offset 59.80ft Left						
Ground Surface Elev. 622.00 ft						
	581.50					
SILTY LOAM-brown & gray-loose (continued)	11					
CLAY LOAM-gray-very dense	24	1.8	11			
	50/4"	P				
	579.00					
SANDY LOAM-gray-very dense	50/5"					
	-45		13			
	576.50					
SANDY CLAY LOAM with GRAVEL-gray-very dense	40		11			
	50/4"					
	48					
	50/1"		9			
	572.00	-50				
End Of Boring @ -50.0'. Boring backfilled with cuttings.						
	-55					
	-60					

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

GSJ Job No. 10025
Page 1 of 2
Date 8/27/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St. LOGGED BY NW
SECTION 2013-037B-R LOCATION SE 1/4, SEC. 11, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD Hollow Stem Auger HAMMER TYPE CME Automatic

STRUCT. NO.	DEPTH	BL	UCS	MOIST	Surface Water Elev.	Stream Bed Elev.
BORING NO. LB-26					n/a	n/a
Station 315+27 (CL I-55)						
Offset 60.40ft Right						
Ground Surface Elev. 602.20 ft						
	601.03					
14.0" ASPHALT	9					
CRUSHED STONE-medium dense	6		3			
	6					
	599.20					
CLAY LOAM-brown & gray-hard (Fill)	3	5.8	19			
	7	B				
	-5					
	5					
	10	4.3	14			
	11	P				
	6					
	9	4.5	13			
	-10	39	P			
	591.70					
CRUSHED STONE-medium dense to dense (Fill)	4		8			
	9					
	14					
	6					
	5		6			
	-15					
	3					
	5		11			
	12					
	7					
	9		9			
	14					
	-20					

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

GSJ Job No. 10025
Page 2 of 2
Date 8/27/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St. LOGGED BY NW
SECTION 2013-037B-R LOCATION SE 1/4, SEC. 11, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD Hollow Stem Auger HAMMER TYPE CME Automatic

STRUCT. NO.	DEPTH	BL	UCS	MOIST	Surface Water Elev.	Stream Bed Elev.
BORING NO. LB-26					n/a	n/a
Station 315+27 (CL I-55)						
Offset 60.40ft Right						
Ground Surface Elev. 602.20 ft						
	581.20					
SAND & GRAVEL-gray-very dense (continued)	15					
Drillers Observation: COBBLES & BOULDERS	50/2"		10			
	559.70					
Drillers Observation: Possible Bedrock						
	557.70					
End Of Boring @ -44.5'. Boring backfilled with cuttings.	-45					
	-50					
	-55					
	-60					

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

FILE NAME =	DESIGNED -	REVISED -
...\\D160W75-sh-t-HMT-sb-NB-15.dgn	DRAWN -	REVISED -
USER NAME = jworthington	CHECKED -	REVISED -
PLOT DATE = 6/17/2015	DATE - 6/12/2015	REVISED -



STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

HIGH MAST TOWER FOUNDATION
SOIL BORINGS

SCALE: SHEET 15 OF 19 SHEETS STA. TO STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372/373	2013-037B-R	COOK	787	246
CONTRACT NO. 60W75			ILLINOIS FED. AID PROJECT	



SOIL BORING LOG

GSJ Job No. 10025
Page 1 of 2
Date 9/10/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St. LOGGED BY NW
SECTION 2013-037B-R LOCATION SW 1/4, SEC. 12, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD HSA/MUD ROTARY HAMMER TYPE Geo Probe

STRUCT. NO. Station	DEPTH (ft)	BLOW COUNT (/6")	UNIFIED CLASSIFICATION (tsf)	MOISTURE CONTENT (%)	Surface Water Elev. n/a ft	Stream Bed Elev. n/a ft	DEPTH (ft)	BLOW COUNT (/6")	UNIFIED CLASSIFICATION (tsf)	MOISTURE CONTENT (%)	Groundwater Elev.: First Encounter Dry to 10.0' ft Upon Completion n/a ft After Hrs. n/a
16.5" ASPHALT	617.13	17									
CLAY LOAM-dark brown & gray-stiff to hard (Fill)		5		16							
		3									
		3									
		5	3.3	17							
		7	P								
		4									
		12	3.0	11							
		6	P								
		3									
		5	3.9	16							
		6	B								
		3									
		4	1.4	20							
		6	B								
		4									
		7	4.1	16							
		9	B								
		3									
		7	2.7	17							
		10	B								
		4									
		5	1.5	22							
		6	B								

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

GSJ Job No. 10025
Page 2 of 2
Date 9/10/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St. LOGGED BY NW
SECTION 2013-037B-R LOCATION SW 1/4, SEC. 12, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD HSA/MUD ROTARY HAMMER TYPE Geo Probe

STRUCT. NO. Station	DEPTH (ft)	BLOW COUNT (/6")	UNIFIED CLASSIFICATION (tsf)	MOISTURE CONTENT (%)	Surface Water Elev. n/a ft	Stream Bed Elev. n/a ft	DEPTH (ft)	BLOW COUNT (/6")	UNIFIED CLASSIFICATION (tsf)	MOISTURE CONTENT (%)	Groundwater Elev.: First Encounter Dry to 10.0' ft Upon Completion n/a ft After Hrs. n/a
		17									
		24	5.9	13							
		35	B								
		20									
		7	6.2	12							
		32	B								
		4									
		14									
		28		19							
		35									
		10									
		14		16							
		19									
		25		21							
		50/4"									
		5									
		6		1							
		5									
		6		6							
		7									
		8									
		9									
		12	4.9	19							
		12	B								
		10									
		13	3.6	19							
		15	B								

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

GSJ Job No. 10025
Page 1 of 2
Date 9/16/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St. LOGGED BY CW
SECTION 2013-037B-R LOCATION SW 1/4, SEC. 12, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD HSA/MUD ROTARY HAMMER TYPE Geo Probe

STRUCT. NO. Station	DEPTH (ft)	BLOW COUNT (/6")	UNIFIED CLASSIFICATION (tsf)	MOISTURE CONTENT (%)	Surface Water Elev. n/a ft	Stream Bed Elev. n/a ft	DEPTH (ft)	BLOW COUNT (/6")	UNIFIED CLASSIFICATION (tsf)	MOISTURE CONTENT (%)	Groundwater Elev.: First Encounter Dry to 10.0' ft Upon Completion n/a ft After Hrs. n/a
		625.15									
		624.65									
		5									
		5		14							
		5									
		2									
		3	2.3	17							
		6	B								
		8		23							
		9		18							
		4									
		4	1.8	18							
		5	P								
		8									
		20	1.5	19							
		23	P								
		7									
		5		12							
		11									
		6									
		8	2.7	18							
		11	B								
		6									
		7	1.8	19							
		9	B								
		5									
		7	2.6	19							
		12	B								

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

FILE NAME =	DESIGNED -	REVISED -
... \D160W75-sh-t-HMT-sb-NB-16.dgn	DRAWN -	REVISED -
USER NAME = jworthington	CHECKED -	REVISED -
PLOT DATE = 6/17/2015	DATE - 6/12/2015	REVISED -



STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

HIGH MAST TOWER FOUNDATION
SOIL BORINGS

SCALE: SHEET 16 OF 19 SHEETS STA. TO STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372/373	2013-037B-R	COOK	787	247
CONTRACT NO. 60W75				
ILLINOIS FED. AID PROJECT				



SOIL BORING LOG

GSJ Job No. 10025
Page 2 of 2
Date 9/16/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St. LOGGED BY CW
SECTION 2013-037B-R LOCATION SW 1/4, SEC. 12, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD HSA/MUD ROTARY HAMMER TYPE Geo Probe

STRUCT. NO.	DEPTH	BLOW	UCS	MOIST	Surface Water Elev.	Stream Bed Elev.	Groundwater Elev.:	First Encounter	Upon Completion	After	Hrs.
	(ft)	(/6")	(tsf)	(%)	n/a	n/a	n/a	Dry to 10.0'	n/a	n/a	
	4		1.5	21							
	7	8	B								
	8										
582.90											
	16										
	50/5'		16								
580.40											
	28										
	35	48	B	12							
	48										
	30										
	50/3'		2.7	12							
	50										
	575.90										

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

GSJ Job No. 10025
Page 1 of 1
Date 9/10/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St. LOGGED BY CW
SECTION 2013-037B-R LOCATION SE 1/4, SEC. 11, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD Hollow Stem Auger HAMMER TYPE Diedrich Automatic

STRUCT. NO.	DEPTH	BLOW	UCS	MOIST	Surface Water Elev.	Stream Bed Elev.	Groundwater Elev.:	First Encounter	Upon Completion	After	Hrs.
	(ft)	(/6")	(tsf)	(%)	n/a	n/a	n/a	Dry to 10.0'	n/a	n/a	
	7			18							
	10	4.5	P	12							
	13										
603.50											
	6										
	30	4.0	P	13							
	12										
581.50											
	6										
	50/5'										
579.00											
	3										
	5	4.0	P	12							
	10										
576.50											
	9										
	10	3.5	P	17							
	12										
594.00											
	13										
	6										
	13										
	8										
	14										
	14										
	6										
	6										
	5										
	8										
	8										
	11										

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SOIL BORING LOG

GSJ Job No. 10025
Page 1 of 1
Date 9/11/13

ROUTE FAP 372/373 (IL 171) DESCRIPTION IL Route 171 from 47th St. to 55th St. LOGGED BY CW
SECTION 2013-037B-R LOCATION SE 1/4, SEC. 11, TWP. T38N, RNG. R12E, 3rd PM
COUNTY Cook DRILLING METHOD Hollow Stem Auger HAMMER TYPE Diedrich Automatic

STRUCT. NO.	DEPTH	BLOW	UCS	MOIST	Surface Water Elev.	Stream Bed Elev.	Groundwater Elev.:	First Encounter	Upon Completion	After	Hrs.
	(ft)	(/6")	(tsf)	(%)	n/a	n/a	n/a	Dry to 10.0'	n/a	n/a	
	5										
	6	2.5	P	23							
	4										
578.00											
	7										
	7	5.1	B	14							
	7										
574.00											
	3										
	5	2.7	B	16							
	9										
593.00											
	27										
	28										
	32										
	14										
	6										
	8										
	4										
	4										
	3										
	9										
	7										
	3										
583.00											
	3										
	3										
	5										

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

FILE NAME =	DESIGNED -	REVISED -
... \D160W75-sh-t-HMT-sb-NB-17.dgn	DRAWN -	REVISED -
USER NAME = jworthington	CHECKED -	REVISED -
PLOT DATE = 6/17/2015	DATE - 6/12/2015	REVISED -



STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

HIGH MAST TOWER FOUNDATION
SOIL BORINGS

SCALE: SHEET 17 OF 19 SHEETS STA. TO STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372/373	2013-037B-R	COOK	787	248
CONTRACT NO. 60W75				
ILLINOIS FED. AID PROJECT				

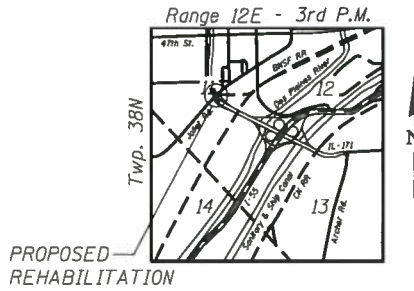
Benchmark: Square cut in N.W. corner of Traffic Control Box north of Joliet Road 20' ± east of the IL 171 Bridge - El. 611.99

Existing Structure:

The structure is a six simple span, composite multi-beam bridge with a 7½ inch reinforced concrete deck. The original structure was built in 1964. In 1994, the approach slabs, deck, parapets, and joints were replaced, one fascia beam was added to the east, slopewalls, piers and abutments were widened, steel painted, fixed and expansion bearings were replaced, and substructures were repaired. In 1998, substructure concrete was repaired.

A crossover shall be utilized to maintain traffic during construction.

No salvage.



LOCATION SKETCH

DESIGN SPECIFICATIONS

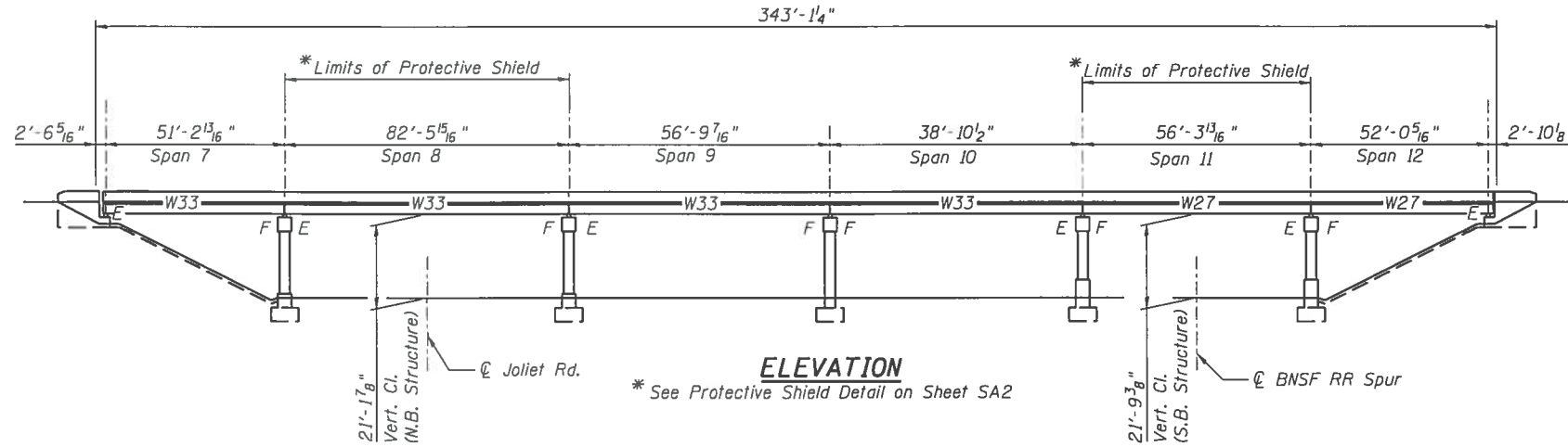
2002 AASHTO Standard Specifications for highway Bridges, 17th Edition

DESIGN STRESSES

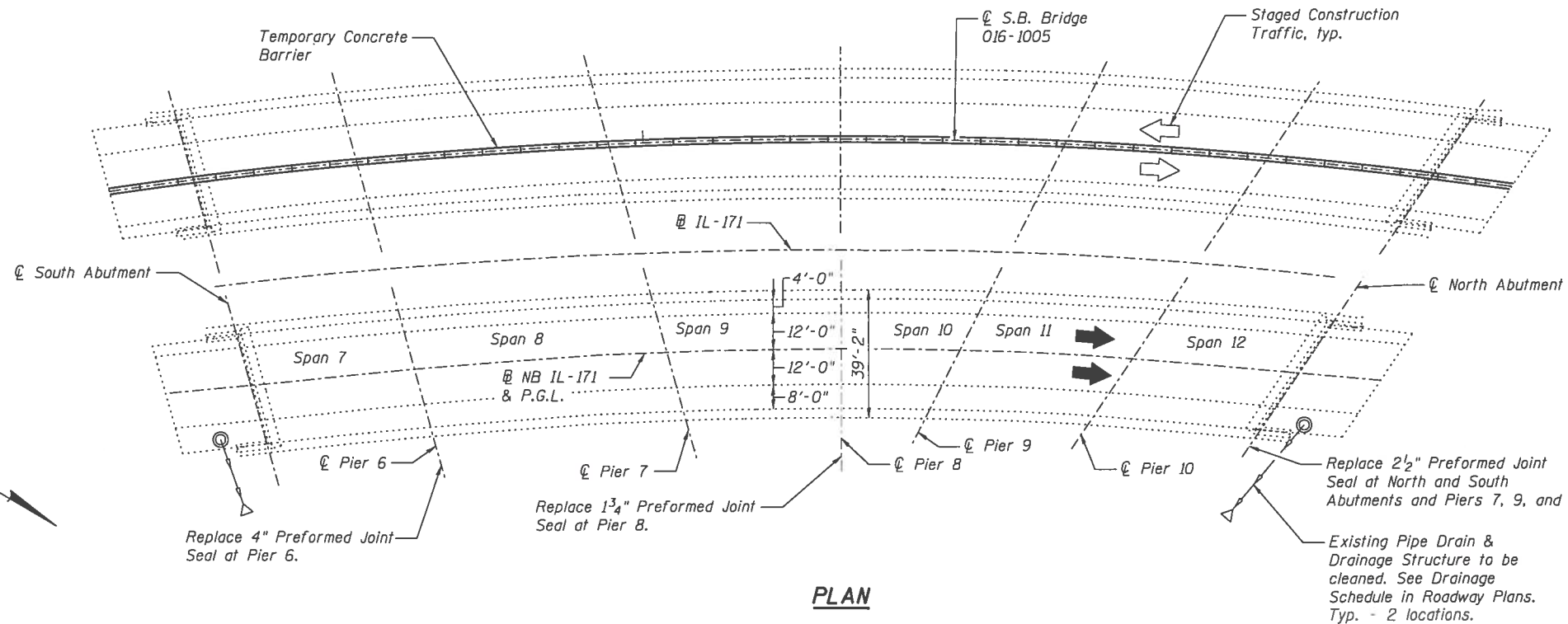
$f'_c = 3,500$ psi
 $f_y = 60,000$ psi (reinforcement)
 $f_y = 36,000$ psi (existing structural steel)
 $f_y = 50,000$ psi (proposed structural steel repair)

SCOPE OF WORK

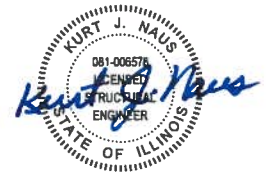
1. Repair abutments and piers with epoxy crack sealing.
2. Structural concrete repair of abutments and piers.
3. Replace all preformed joint seals.
4. Vegetation removal.
5. Clean existing deck floor drains.
6. Clean and seal bridge deck, approach slabs and parapets.
7. Remove abandoned conduit attached to girder.
8. Deck slab repairs.
9. Structural steel repair on the west fascia girder of Span 8.



ELEVATION



PLAN



EXPIRATION DATE 11-30-2016
 DATE: 08-12-2015

GENERAL PLAN AND ELEVATION
NB IL-171 OVER JOLIET ROAD & BNSF RR SPUR
FAP 372 - SECTION 2013-037B-R
COOK COUNTY
STATION 77+49.49
STRUCTURE NO. 016-1006

benesch
 engineers - scientists - planners
 Alfred Benesch & Company
 205 North Michigan Avenue, Suite 2400
 Chicago, Illinois 60601
 312-565-0450 Job No. 10093

FILE NAME =	USER NAME = jaurber	DESIGNED - CMK	REVISED -
816-1006-60W75-001-gpe.dgn	PLOT SCALE =	CHECKED - JAW	REVISED -
	PLOT DATE = 6/18/2015	DRAWN - CMK	REVISED -
		CHECKED - JAW	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SHEET NO. SA1 OF SAT SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372	2013-037B-R	COOK	787	251
			CONTRACT NO. 60W75	
ILLINOIS FED. AID PROJECT				

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GENERAL NOTES

- Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.
- Protective Coat shall be applied to the entire top surface of the deck and approach slabs and the tops and inside vertical faces of all parapets. All surfaces to be sealed shall be cleaned thoroughly prior to sealer application.
- A crossover shall be utilized to maintain traffic during construction.
- There are 13 floor drains on this bridge. The floor drains shall be cleaned with the bridge deck prior to application of the Protective Coat. Cost included with "Protective Coat".
- Cost of vegetation removal to be included with "Selective Clearing". See Special Provision.
- The existing structural steel coating contains lead. The Contractor shall take all precautions to deal with the presence of lead on this project.
- The Inorganic Zinc Rich Primer/Acrylic/Acrylic Paint System shall be used for shop and field painting of new structural steel. Only Inorganic Zinc Rich Primer shall be applied to the new structural steel in the shop under this contract and is included in "Structural Steel Repair". The intermediate and top coats shall be applied under a separate painting contract.
- Existing structural steel shall only be cleaned and painted as required by the Special Provision "Cleaning and Painting Contact Surface Areas of Existing Steel Structures".

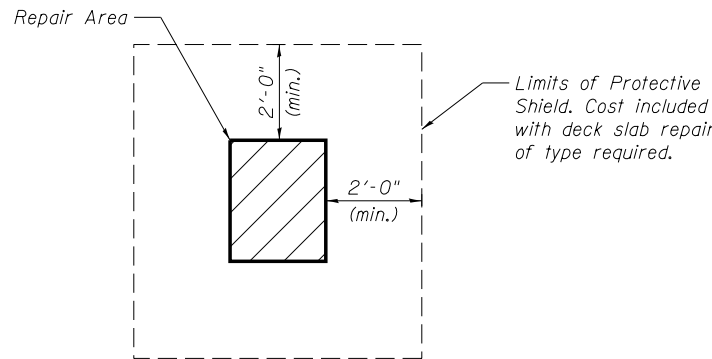
INDEX OF SHEETS

- SA1 General Plan and Elevation
- SA2 General Notes, Bill of Material and Index of Sheets
- SA3 Structural Steel Repairs - Span 8
- SA4 Substructure Repairs - South Abutment
- SA5 Substructure Repairs - Piers 6 & 7
- SA6 Substructure Repairs - Piers 8 & 9
- SA7 Substructure Repairs - Pier 10

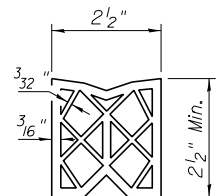
For existing bridge plans, see Sheets SAX1 thru SAX7, immediately following Sheet SA7.

TOTAL BILL OF MATERIAL

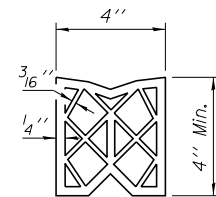
ITEM	UNIT	SUPER	SUB	TOTAL
Protective Coat	Sq Yd	1,926		1,926
* Preformed Joint Seal 1 ³ / ₄ "	Foot	37.5		37.5
* Preformed Joint Seal 2 ¹ / ₂ "	Foot	207.5		207.5
* Preformed Joint Seal 4"	Foot	38.5		38.5
** Epoxy Crack Injection	Foot		37	37
Remove Conduit Attached to Structure	Foot	686		686
Structural Steel Repair	Pound	3,580		3,580
** Structural Repair of Concrete (Depth Equal to or Less Than 5 Inches)	Sq Ft		153	153
** Structural Repair of Concrete (Depth Greater than 5 Inches)	Sq Ft		31	31
*** Deck Slab Repair (Full Depth, Type I)	Sq Yd	5.0		5.0
*** Deck Slab Repair (Full Depth, Type II)	Sq Yd	5.0		5.0
*** Deck Slab Repair (Partial)	Sq Yd	10.0		10.0
**** Selective Clearing	Unit		2	2



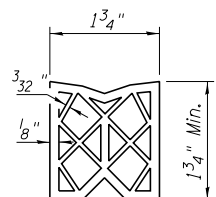
PROTECTIVE SHIELD DETAIL



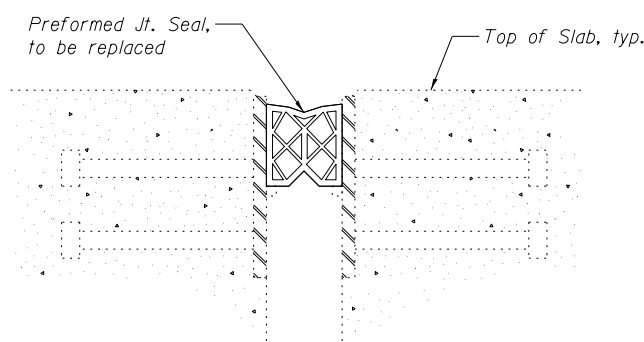
PREFORMED JOINT SEAL (2¹/₂")
(Abutments, Piers 7, 9, & 10)



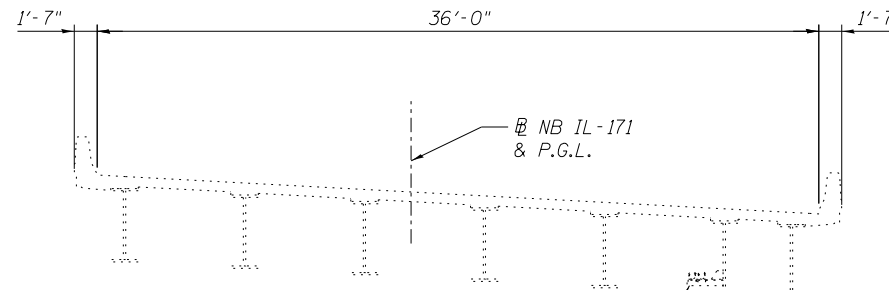
PREFORMED JOINT SEAL (4")
(Pier 6)



PREFORMED JOINT SEAL (1³/₄")
(Pier 8)



**PREFORMED JOINT SEAL
PLACEMENT DETAIL**



SECTION - CONDUIT REMOVAL LOCATION
(Looking North)

Existing conduit, wiring, U-Bolts, hardware, expansion couplings, sleeves, and junction boxes to be completely removed. Cost included with "Remove Conduit Attached to Structure". See Special Provision.

Existing support angle to remain.

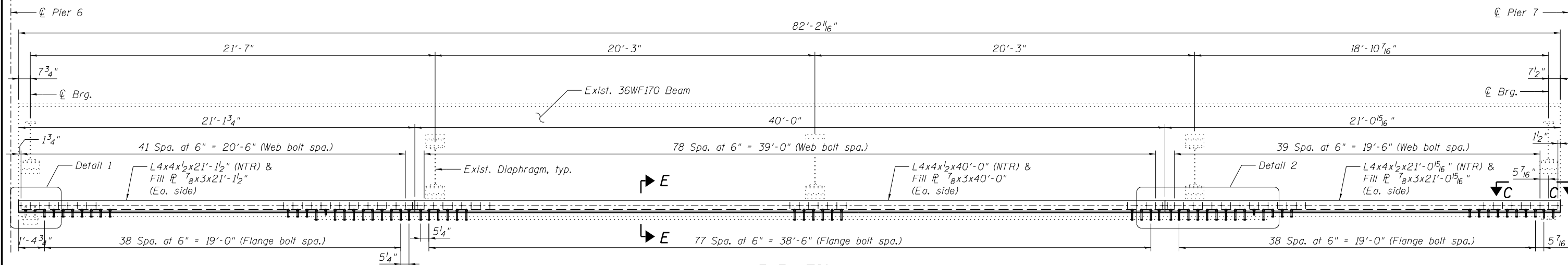
CONDUIT REMOVAL DETAIL

- * Includes cost of Removal and Disposal of Existing Joint Seals.
- ** Quantity includes a contingency (above the amounts shown in the bills of material) to account for uncertainties associated with the condition of the existing substructure and the age of the original inspection (2008-9). Actual repair areas will be determined by the Engineer in the field.
- *** The quantity shown is an estimate. Actual repair areas and locations shall be determined by the Engineer and shown on As-Built plans.
- **** The quantity for this work is estimated. The intent for this work is to remove accumulations of rubbish, vegetation, etc., on the existing slopewalls.

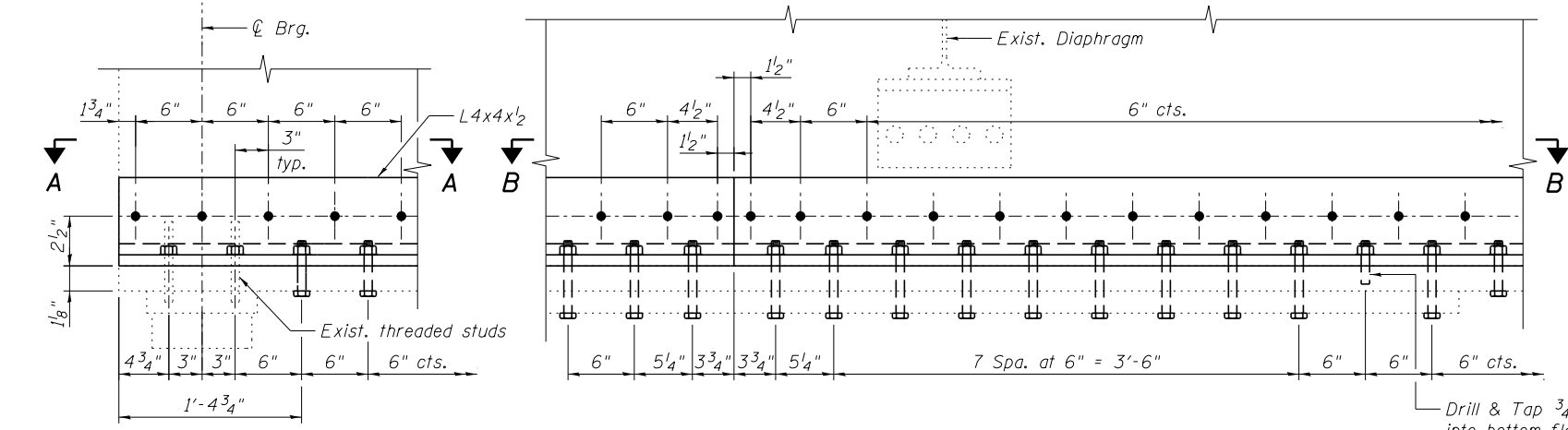
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016-1006-60W75-002-notes.dgn	PLOT SCALE =	CHECKED - JAW	REVISED -
	PLOT DATE = 6/12/2015	DRAWN - CMK	REVISED -
		CHECKED - JAW	REVISED -

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372	2013-037B-R	COOK	787	252
CONTRACT NO. 60W75				
ILLINOIS FED. AID PROJECT				

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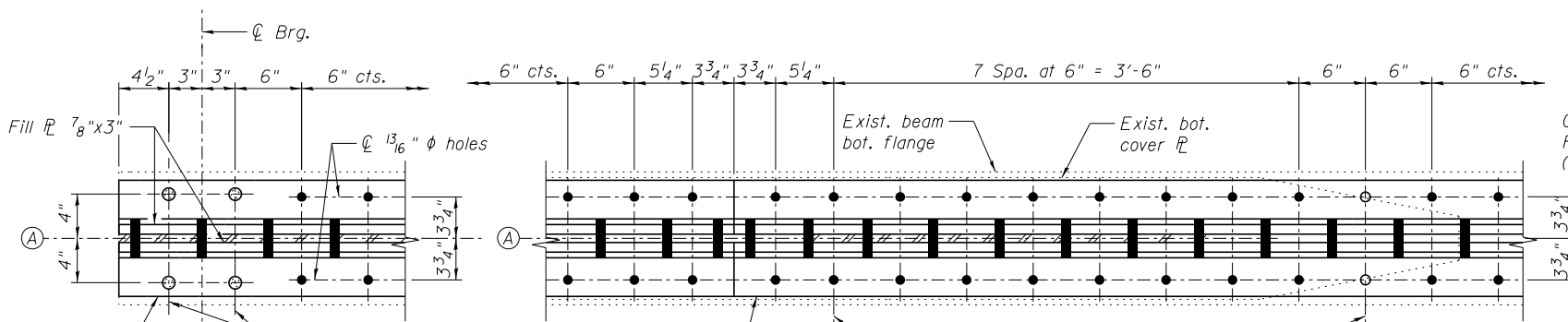


ELEVATION



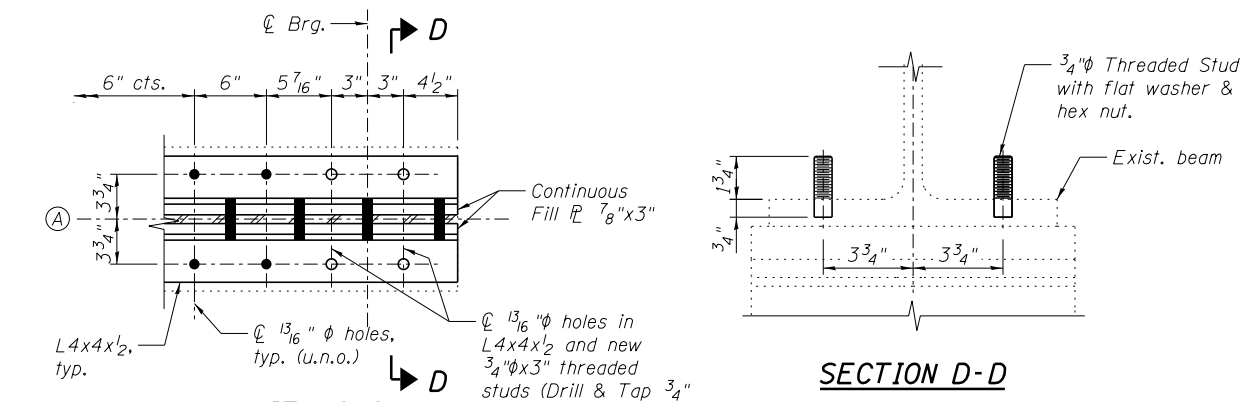
DETAIL 1

DETAIL 2



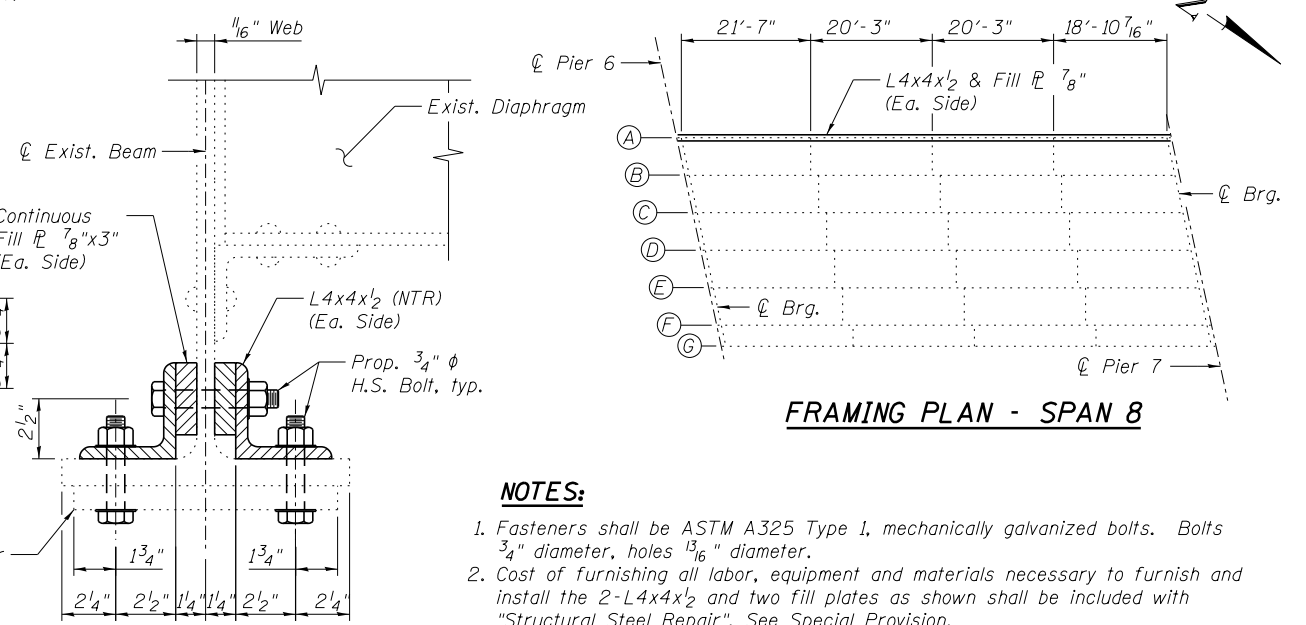
VIEW A-A

VIEW B-B



VIEW C-C

SECTION D-D



FRAMING PLAN - SPAN 8



SECTION E-E

NOTES:

- Fasteners shall be ASTM A325 Type 1, mechanically galvanized bolts. Bolts 3/4" diameter, holes 13/16" diameter.
- Cost of furnishing all labor, equipment and materials necessary to furnish and install the 2-L4x4x1/2 and two fill plates as shown shall be included with "Structural Steel Repair". See Special Provision.
- Angles and fill plates shall conform to the requirements of AASHTO M270 Grade 50.
- Cost of field drilling included with "Structural Steel Repair".
- Contractor shall verify locations of existing diaphragms, dimensions and hole locations and make necessary approved adjustments prior to construction or ordering of materials. Cost included with "Structural Steel Repair".
- Load carrying components designated "NTR" shall conform to the Impact Testing Requirement, Zone 2.
- All steel repairs shall be done with no live load on the bridge.

BILL OF MATERIAL

ITEM	UNIT	TOTAL
Structural Steel Repair	Pound	3,580

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**STRUCTURAL STEEL REPAIRS - SPAN 8
STRUCTURE NO. 016-1006**

SHEET NO. SA3 OF SAT SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372	2013-037B-R	COOK	787	253
				CONTRACT NO. 60W75

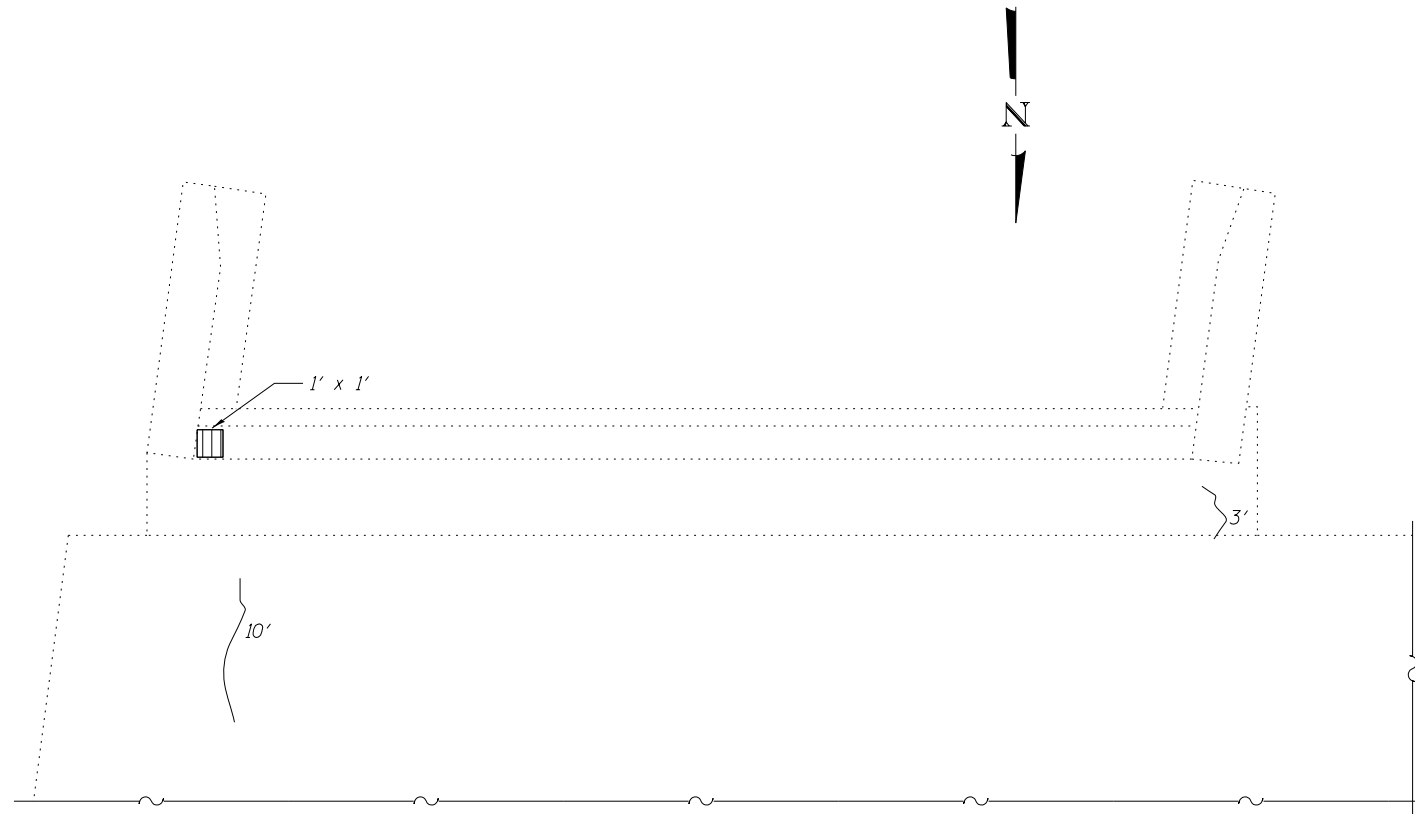
ILLINOIS FED. AID PROJECT

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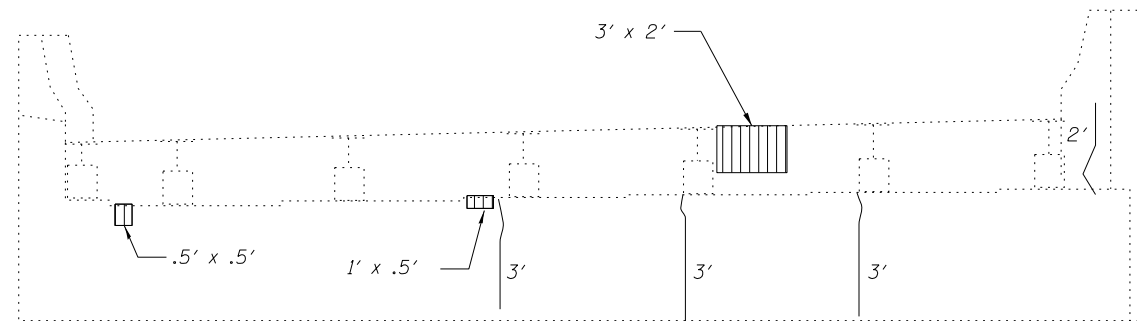
Alfred Benesch & Company
205 North Michigan Avenue, Suite 2400
Chicago, Illinois 60601
312-565-0450 Job No. 10093

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016-1006-60W75-003-Steel.Repairs.dgn		CHECKED - AJK	REVISIONS -
		DRAWN - RMG	REVISIONS -
		CHECKED - JLS	REVISIONS -

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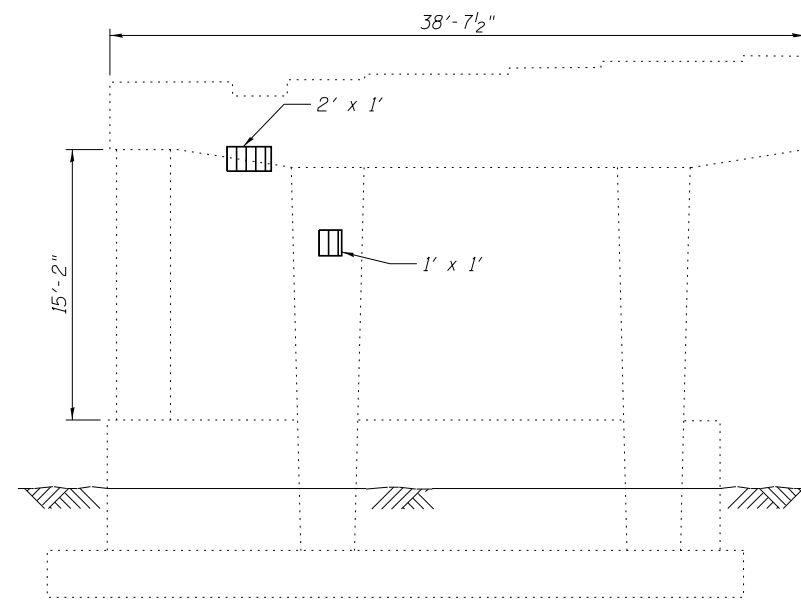
PLAN - SOUTH ABUTMENT



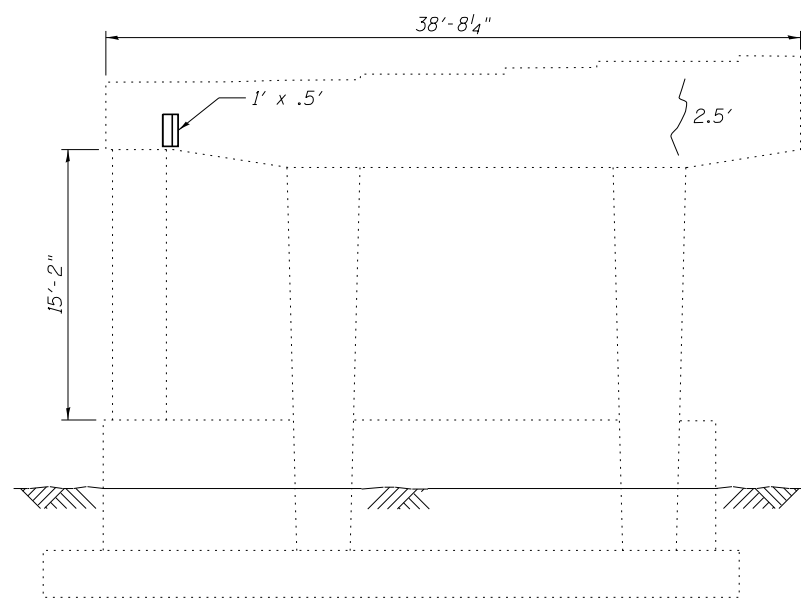
ELEVATION - SOUTH ABUTMENT

BILL OF MATERIAL

SYMBOL	ITEM	UNIT	QUANTITY
	Structural Repair of Concrete (Depth Equal to or Less Than 5 Inches)	Sq. Ft.	8
	Epoxy Crack Injection	Foot	24



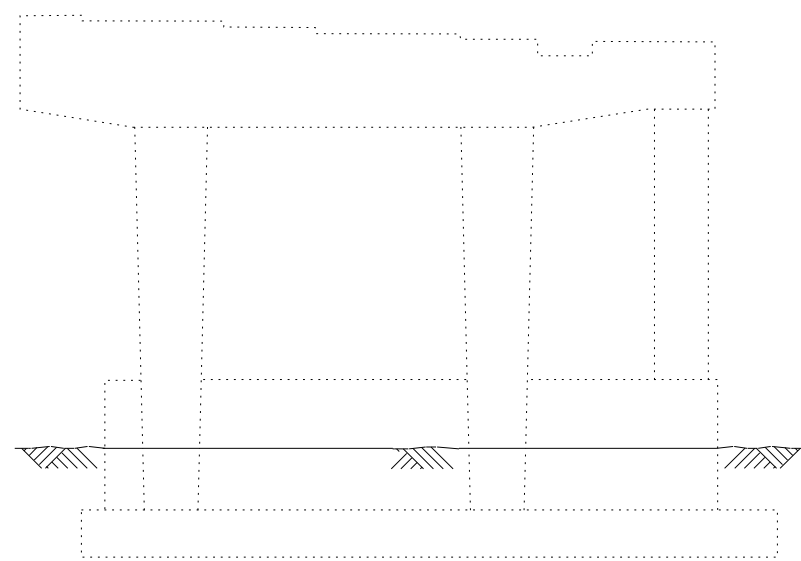
PIER 6
(Looking South)



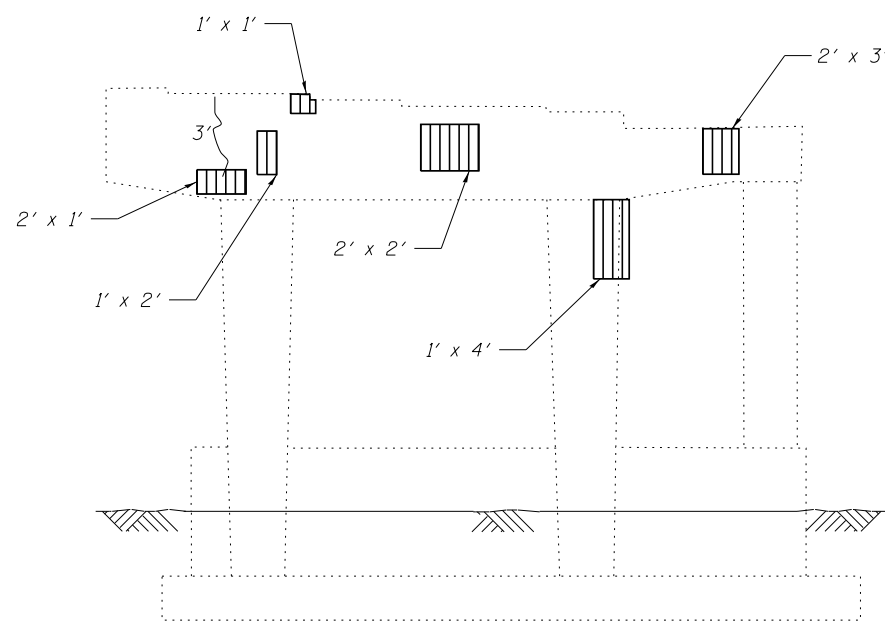
PIER 7
(Looking South)

BILL OF MATERIAL

SYMBOL	ITEM	UNIT	QUANTITY
	Structural Repair of Concrete (Depth Equal to or Less Than 5 Inches)	Sq. Ft.	23
	Epoxy Crack Injection	Foot	6



PIER 6
(Looking North)



PIER 7
(Looking North)



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205 North Michigan Avenue, Suite 2400
Chicago, Illinois 60601
312-565-0450 Job No. 10093

FILE NAME =
016-1006-60W75-005-piers 6&7.dgn

USER NAME = jsurber
PLOT SCALE =
PLOT DATE = 6/10/2015

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CHECKED - JAW

REVISED -
REVISED -
REVISED -
REVISED -

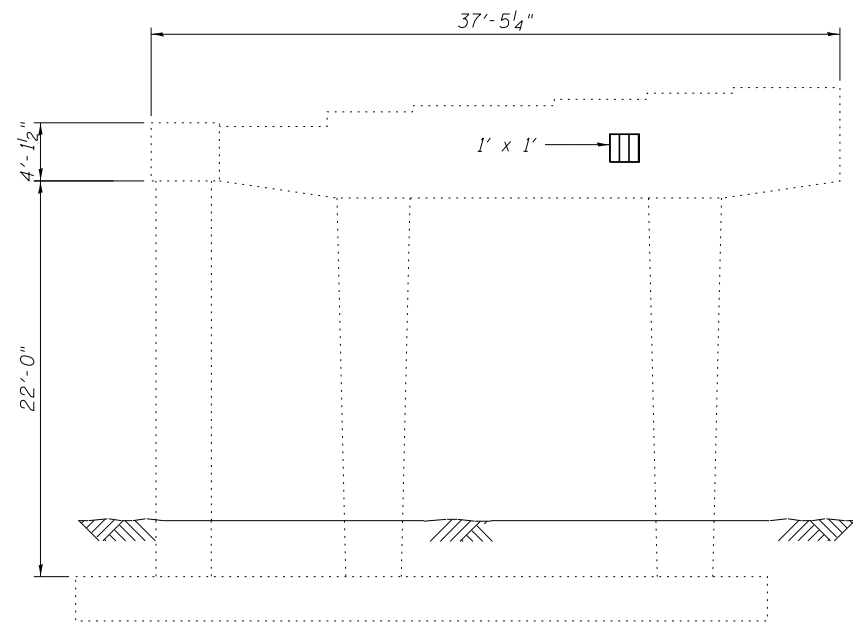
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**SUBSTRUCTURE REPAIRS - PIERS 6 & 7
STRUCTURE NO. 016-1006**

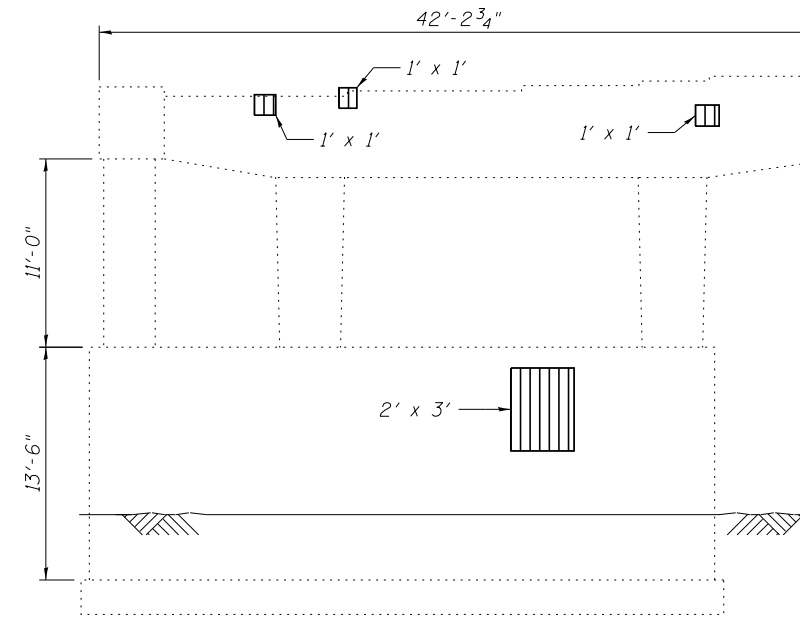
SHEET NO. SA5 OF SA7 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372	2013-037B-R	COOK	787	255
			CONTRACT NO. 60W75	
ILLINOIS FED. AID PROJECT				

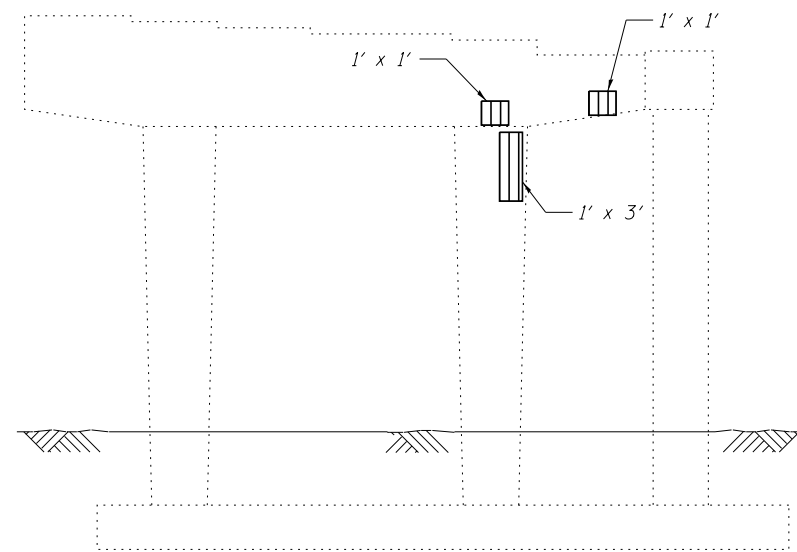
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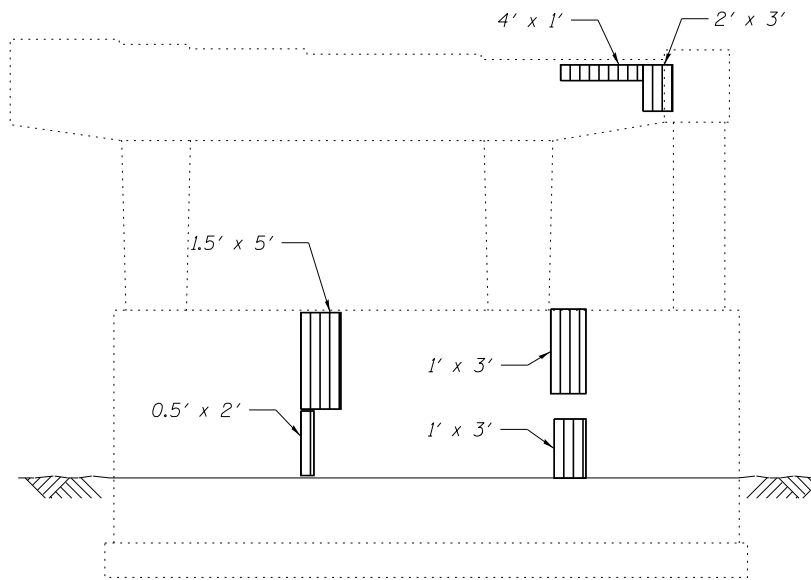
PIER 8
(Looking South)



PIER 9
(Looking South)



PIER 8
(Looking North)



PIER 9
(Looking North)

BILL OF MATERIAL

SYMBOL	ITEM	UNIT	QUANTITY
	Structural Repair of Concrete (Depth Equal to or Less Than 5 Inches)	Sq. Ft.	40

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312-565-0450 Job No. 10093

FILE NAME =	USER NAME = jsurber	DESIGNED - CMK	REVISED -
		CHECKED - JAW	REVISED -
		DRAWN - CMK	REVISED -
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	PLOT DATE = 6/10/2015		

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

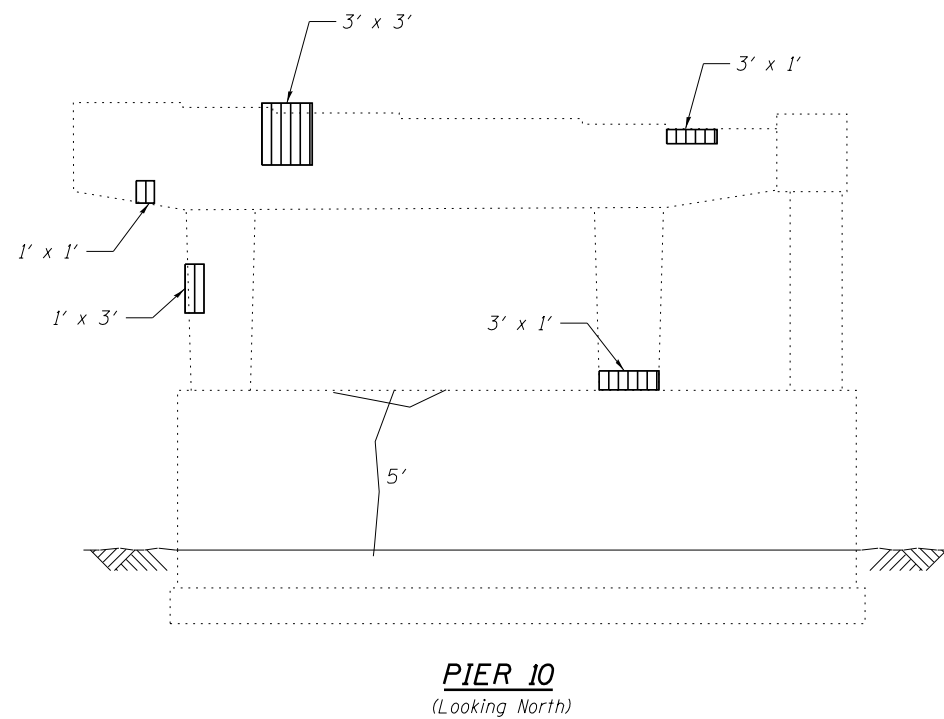
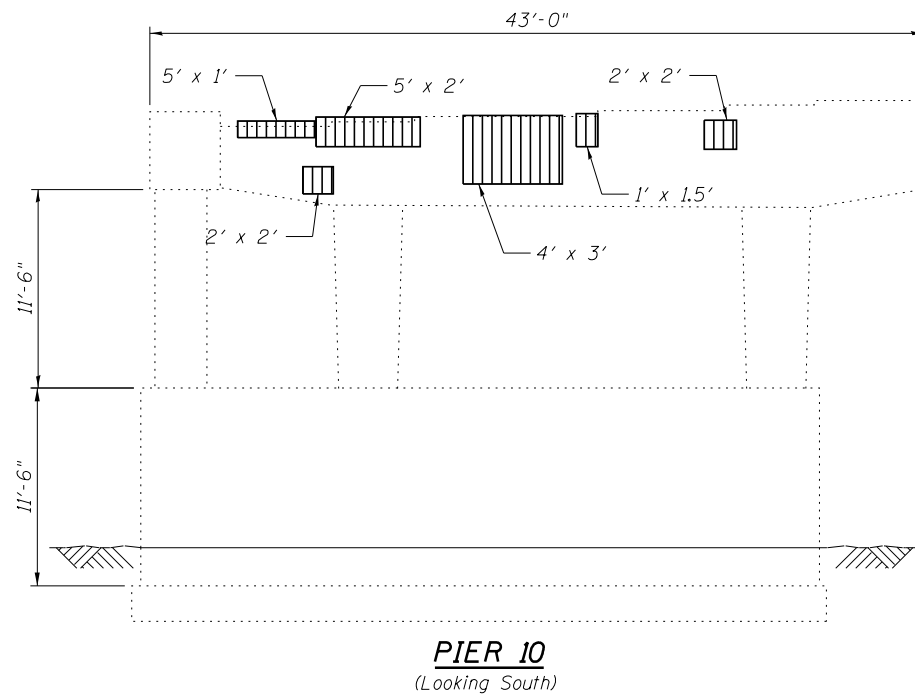
SUBSTRUCTURE REPAIRS - PIERS 8 & 9
STRUCTURE NO. 016-1006

SHEET NO. SA6 OF SA7 SHEETS



F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372	2013-037B-R	COOK	787	256
CONTRACT NO. 60W75				

ILLINOIS FED. AID PROJECT

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BILL OF MATERIAL

SYMBOL	ITEM	UNIT	QUANTITY
	Structural Repair of Concrete (Depth Equal to or Less Than 5 Inches)	Sq. Ft.	56
	Epoxy Crack Injection	Foot	5

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

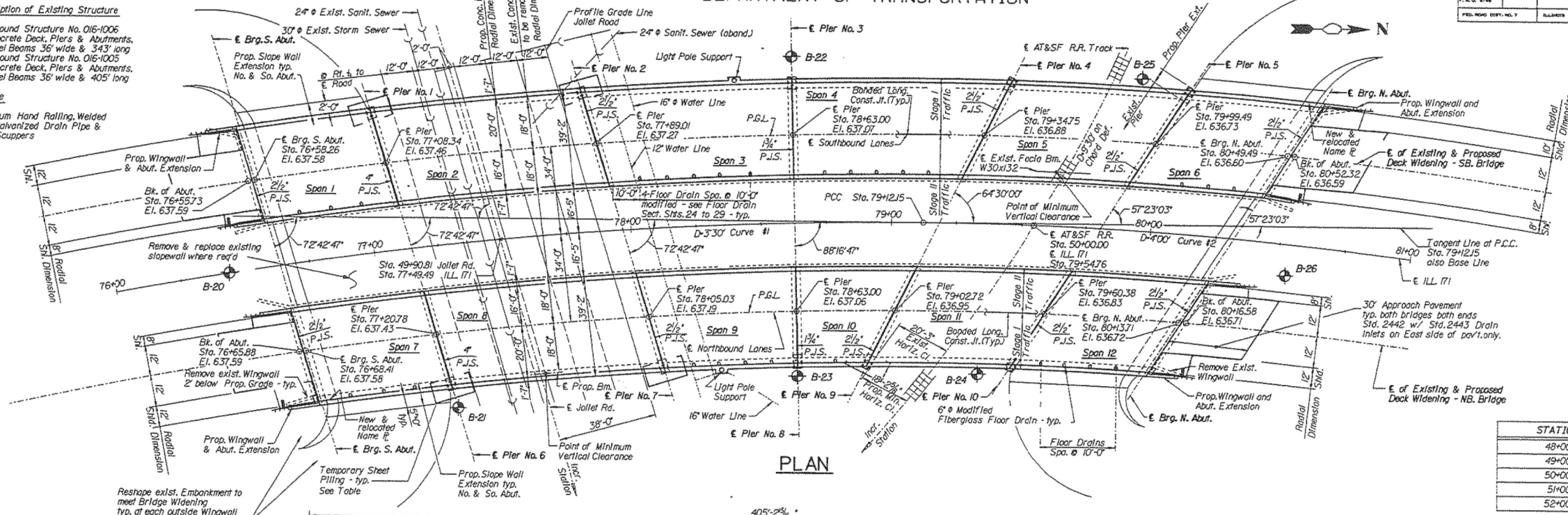
ROUTE NO.	SECTION	COUNTY	SHEET	DATE
F.A.U. 2746	606-628	COOK	383	106
FED. ROAD DIST. NO. 7	BLDG. NO.	FED. AID PROJECT		

SHEET NO. S-1
SHEETS 5-48

Benchmark
Cut in NW Corner of Traffic Control Box north of Joliet Road 20' ± east of the ILL. 171 Bridge - El. 611.99

Description of Existing Structure
Northbound Structure No. 016-1006
Concrete Deck, Piers & Abutments.
Steel Beams 36" wide & 343' long
Southbound Structure No. 016-1005
Concrete Deck, Piers & Abutments.
Steel Beams 36" wide & 405' long

Salvage
Aluminum Hand Railing, Welded Steel Galvanized Drain Pipe & Deck Scaupers



STATION 77+49.4
BUILT 199 BY
STATE OF ILLINOIS
FAU.RT. 2746 SEC. 606-628(B)
F.A. PROJ. STPM-1005(11)
LOADING HS20
STR. NO. 016-1005
NAME PLATE
SOUTHBOUND

STATION 77+49.4
BUILT 199 BY
STATE OF ILLINOIS
FAU.RT. 2746 SEC. 606-628(B)
F.A. PROJ. STPM-1005(11)
LOADING HS20
STR. NO. 016-1006
NAME PLATE
NORTHBOUND

STATION	TRACK ELEVATION
48+00	612.0
49+00	610.99
50+00	610.61
51+00	610.31
52+00	610.20

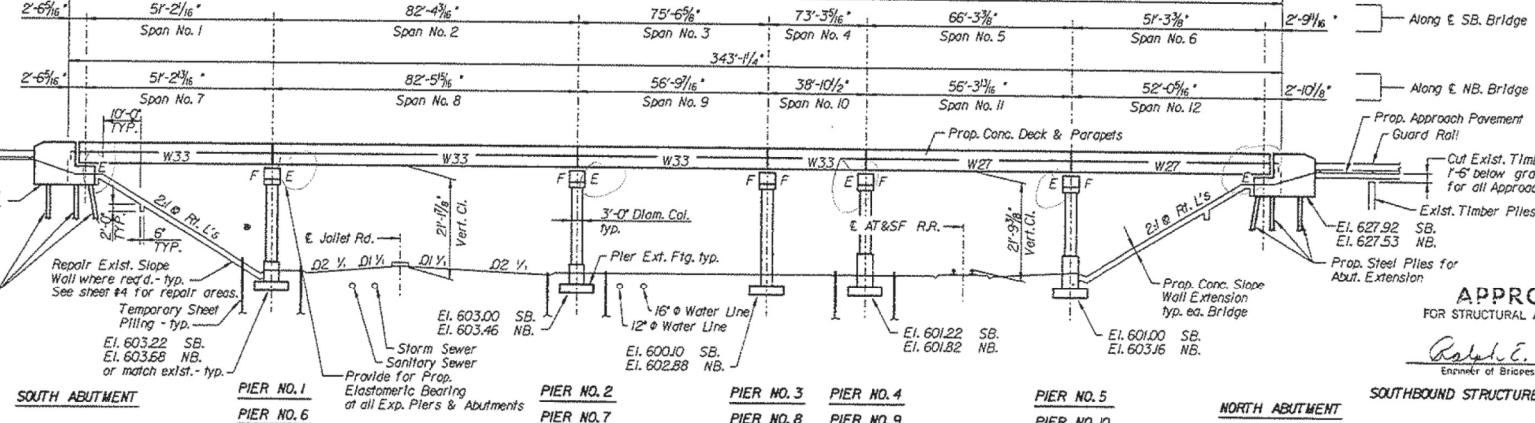
AT&SF R.R. TOP OF TRACK ELEVATION

NOTE:
Use Traffic Terminal Type 6 for Approach end of bridges and Traffic Terminal Type 5 for Exit end of bridges.
SB - Southbound Bridge
NB - Northbound Bridge

HORIZONTAL CURVE DATA
Along ILL. 171

	CURVE # 1	CURVE # 2
Δc	31°12'18"	22°24'00"
Dc	3°30'00"	4°00'00"
Rc	1637.02	1432.39
Tc	457.14	283.62
Lc	891.57	560.00
LC	880.59	556.44
Ec	62.63	27.81

DESIGNED	RC
CHECKED	ELD
DRAWN	MF
CHECKED	ELD



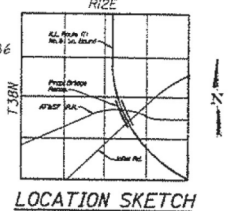
STAGING INSTRUCTIONS
STAGE I:
Outer median lanes shall be used for traffic while inner lanes are reconstructed with new deck.
STAGE II:
Inner lanes of reconstructed bridge shall be used for traffic while outer lanes are reconstructed with new deck.

ELEVATION NORTHBOUND STRUCTURE
ELEVATION SOUTHBOUND STRUCTURE (SIMILAR)
(Looking West)

DESIGN SPECIFICATIONS
AASHTO (1992)
LOADING HS20-44
Allow 25#/#sq. ft. for future wearing surface.

DESIGN STRESSES
New Deck & Rehab. Structures
 $f_c = 3,500$ psi
 $f_y = 60,000$ psi (Reinf.)
 $f_y = 36,000$ psi (New Beams) M150 Gr. 36
Existing Structures
 $f_c = 1,400$ psi
 $f_s = 20,000$ psi (Reinf.)
 $f_y = 36,000$ psi (Beams)
Soil Pressure All. = 5,200 #/#sq. Ft.

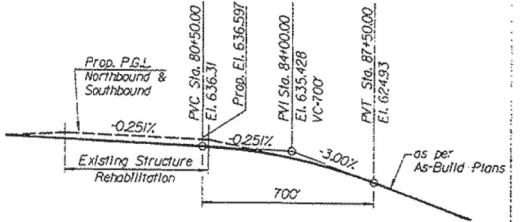
SEISMIC DATA
S.P.C. - A
A - 0.04
Site Coeff. - 1.0



APPROVED
FOR STRUCTURAL ADEQUACY ONLY
Robert E. Anderson
Engineer of Bridges and Structures

TEMPORARY SHEET PILING
TOP & BOTTOM ELEVATION

PIER	TOP EL.	BOTTOM EL.
PIER 1	611.5	600.2
PIER 2	611.5	600.0
PIER 6	610.0	600.5
PIER 7	610.0	600.4



PROFILE GRADE LINES
ILL. Route 171
along E Northbound & Southbound Structures

GENERAL PLAN
BRIDGE REHABILITATION ILLINOIS ROUTE 171
NORTHBOUND & SOUTHBOUND
OVER JOLIET RD. & AT&SF R.R.
FAU 2746 SECTION 606-628 VB
STATION 77+49.4
COOK COUNTY
STRUCTURE NO. 016-1005 S.B.
STRUCTURE NO. 016-1006 N.B.



FOR INFORMATION ONLY

FILE NAME =	USER NAME =	DESIGNED -	REVISIONS
016-1006-60675-008-Existing gpe.dgn	jsurber	CMK	CMK
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		CMK	CMK
		JAW	JAW

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

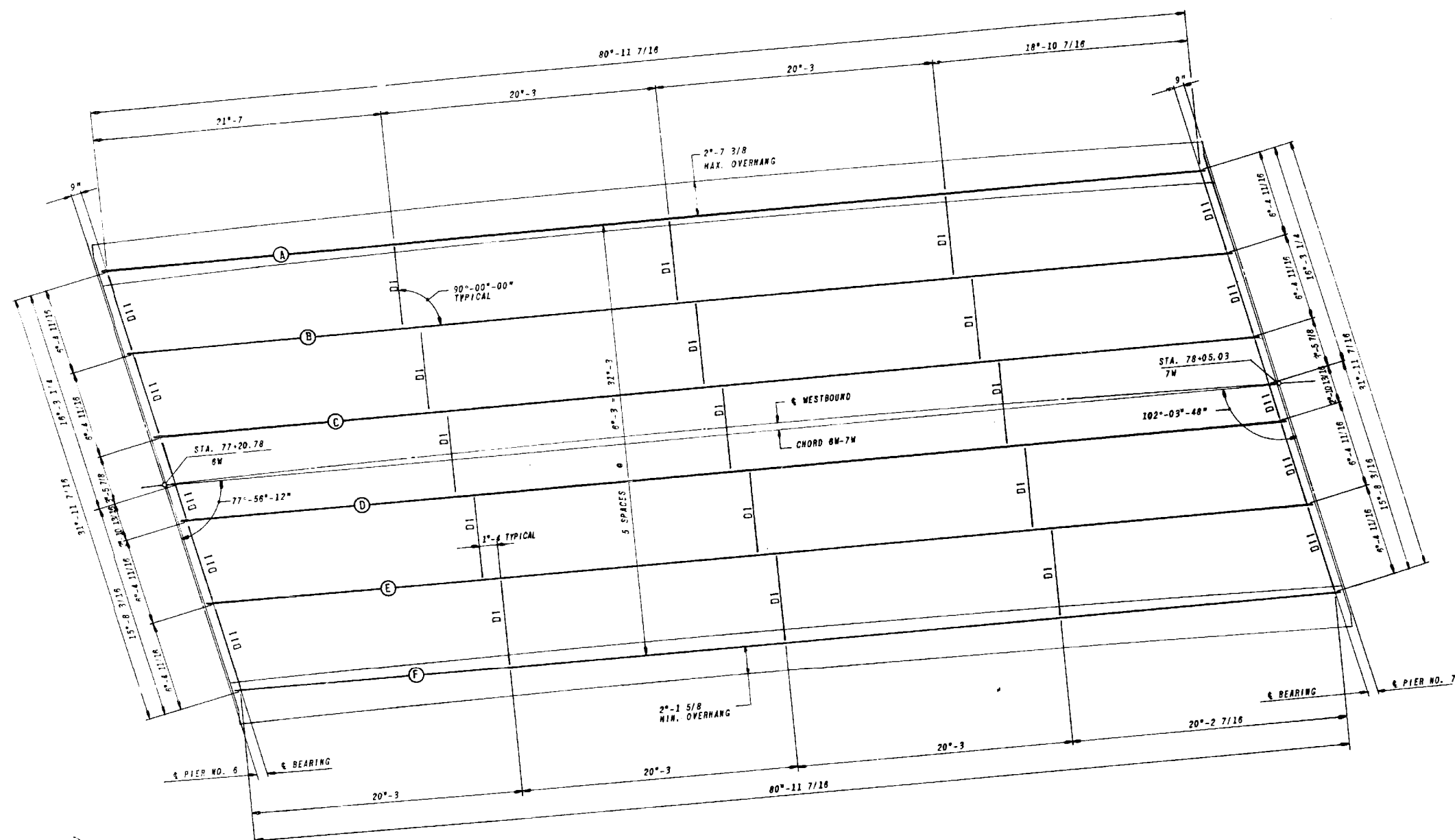
EXISTING PLAN INFORMATION 1 OF 7
STRUCTURE NO. 016-1006
SHEET NO. SAX1 OF SAX7 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372	2013-037B-R	COOK	787	258
				CONTRACT NO. 60W75

ILLINOIS FED. AID PROJECT

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F.P. NO.	SECTION	EXPRESSWAYS	TOTAL SHEETS	SHEET NO.
121	0606-628 W.B.	SOUTHWEST	87	39
STA.	TO STA.			
FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT		



FRAMING PLAN SPAN NO. 8 WEST BOUND

NOTE: SEE SHEET "SUMMARY OF BEAMS WESTBOUND" FOR ELEVATIONS TOP OF STEEL, LENGTH OF COVERPLATES, SHEAR CONNECTOR SPACINGS AND DETAIL DIMENSIONS.

REVISIONS		ILLINOIS DIVISION OF HIGHWAYS	
NAME	DATE	SOUTHWEST EXPRESSWAY	
		F.A. RT. 133	
		LAWDALE AVE STRUCTURE OVER	
		JOLIET ROAD & SANTA FE RY. SPUR	
		SECTION 0606-628WB	
		FRAMING PLAN SPAN NO. 8 W.B.	
DESIGNED	S.T.-P.X.	SCALE: VERT.	DRAWN BY D.U.
REVIEWED	C.W.W.	DATE 2-18-1964	CHECKED BY L.E.E.

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205 North Michigan Avenue, Suite 2400
Chicago, Illinois 60601
312-565-0450 Job No. 10093

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016-1006-60W75-009-Exist fr plan1.dgn	PLOT SCALE =	CHECKED - JAW	REVISED -
	PLOT DATE = 6/10/2015	DRAWN - CMK	REVISED -
		CHECKED - JAW	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

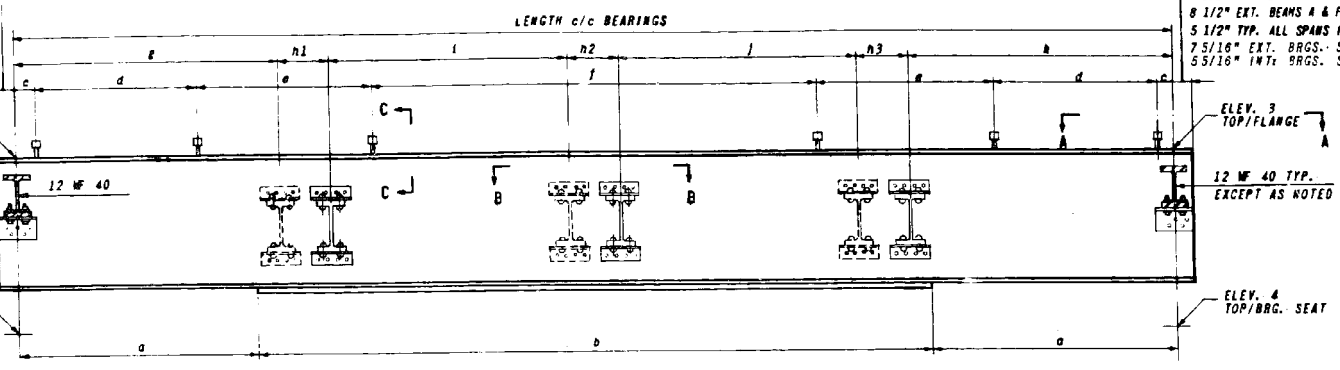
EXISTING PLAN INFORMATION 2 OF 7
STRUCTURE NO. 016-1006
SHEET NO. SAX2 OF SAX7 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372	2013-037B-R	COOK	787	259
				CONTRACT NO. 60W75
ILLINOIS FED. AID PROJECT				

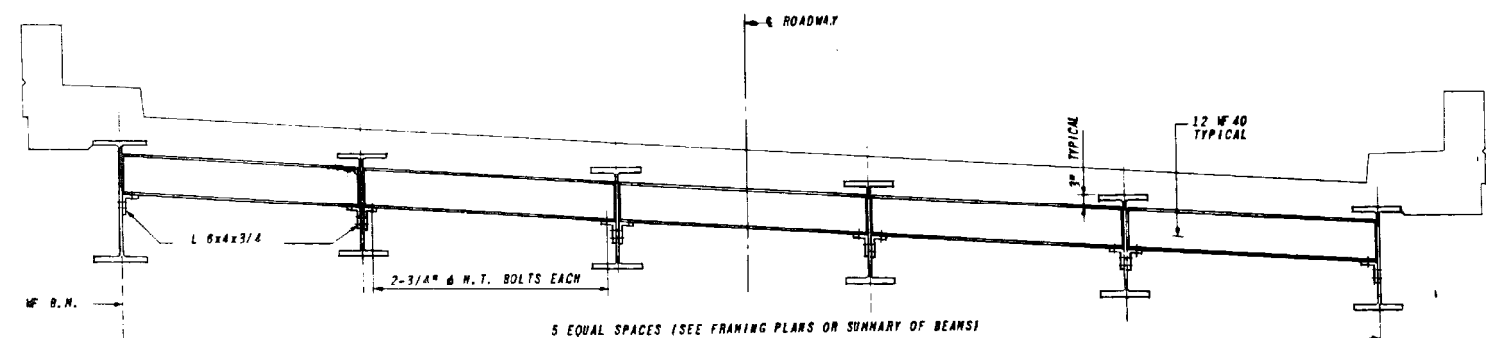
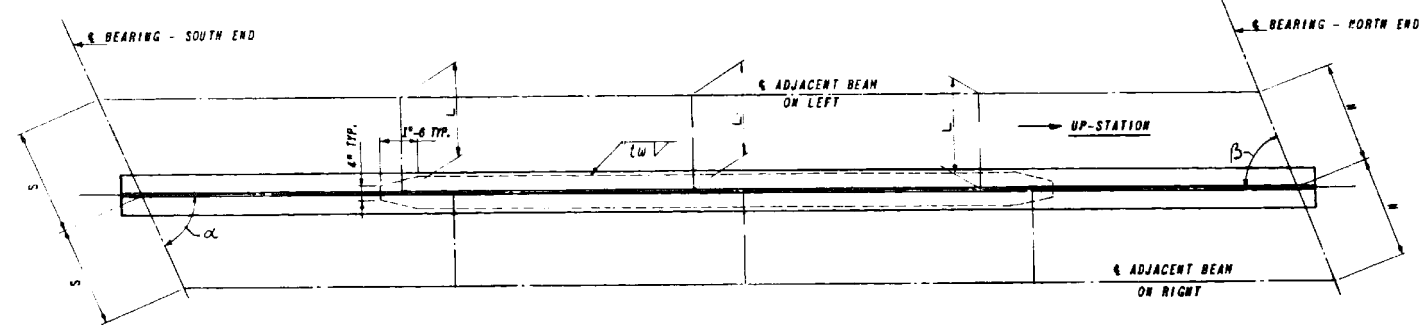
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F.A. RT.	SECTION	EXPRESSWAY	TOTAL SHEETS	SHEET NO.
132	0606-628 FB	SOUTHWEST		43A
STA.	TO STA.			
FED. ROAD DIST. NO. 1	ILLINOIS	FED. AID PROJECT		

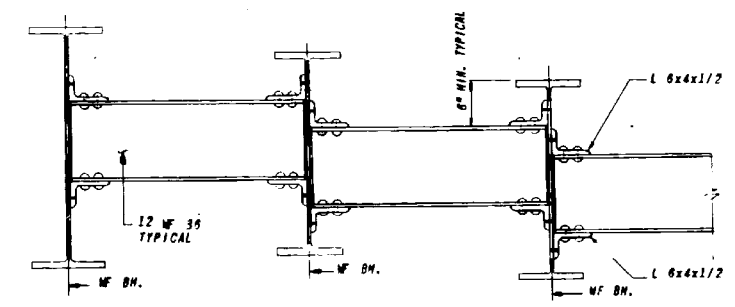
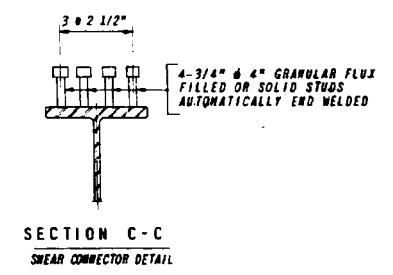
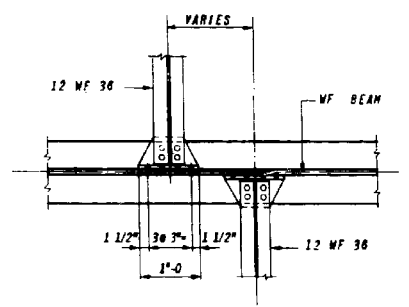
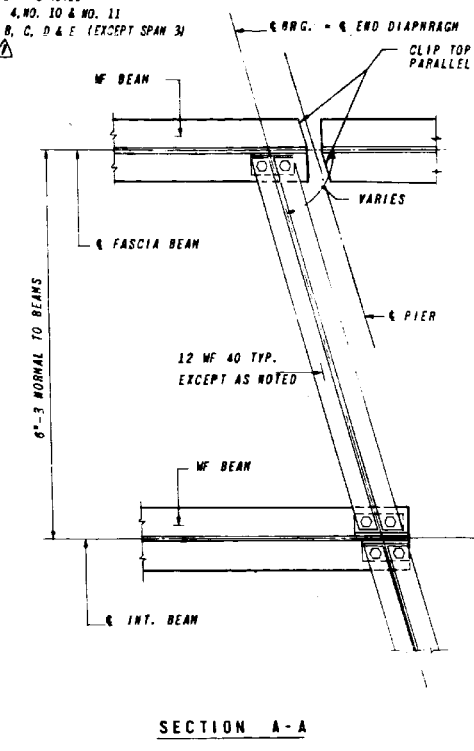
5 3/8" INT. BM. SPAN 3
 7 5/8" EXT. BM. SPAN 3
 7 1/2" TYP. EXT. BEAMS A & F, EXCEPT AS NOTED
 7 3/4" EXT. BEAMS A & F, SPAN NO. 2,
 NO. 8 & NO. 9
 8 3/8" EXT. BEAMS A & F, SPAN NO. 6,
 NO. 11 & NO. 12
 10" EXT. BEAMS A & F, SPAN NO. 5
 5 1/2" TYP. ALL SPANS. INT. BEAMS B,
 C, D & E (EXCEPT SPAN 3)



ELEVATION
TYPICAL INTERIOR GIRDER



SUMMARY OF END DIAPHRAGM											
SPAN NO.	DIAPHRAGM	C/C BRG.	REQ'D	SPAN NO.	DIAPHRAGM	C/C BRG.	REQ'D	SPAN NO.	DIAPHRAGM	C/C BRG.	REQ'D
1	D2	6'-4	8	4	D5	6'-3	5	6	D9	7'-1	10
	DE1	6'-7	2		D6	6'-11 7/16	5		D10	6'-4 1/16	8
2	D3	6'-4 9/16	10	5	D7	6'-9 11/16	3	7	DE10	6'-7 3/8	2
	D4	6'-5 3/8	3		D8	7'-3	3		D11	6'-4 11/16	10
3	D5	6'-3	3	9	DE6	6'-11 7/8	1	8	D12	6'-5 1/2	4
	DE2	6'-1 1/4	1		D7	7'-3 15/16	1		D5	6'-3	4
	DE3	6'-5 7/16	1		DE9	6'-9 11/16	1		DE11	6'-1 1/4	1
	DE4	6'-0 1/8	1		DE9	6'-10 5/8	1		DE12	5'-10 7/8	1
	DE5	6'-5 7/8	1								
								11	D14	6'-10 9/10	5
									D15	7'-4 1/4	5
								12	D16	7'-4 1/4	10



TYPICAL INTERMEDIATE DIAPHRAGM

REQ'D: 107 D1 = 6'-3
 2 EACH D41 = 6'-5, D412 = 6'-6 1/4, D413 = 5'-10 7/8
 1 EACH D42 = 6'-3 5/8, D43 = 6'-4 3/8, D44 = 6'-5 1/8
 D45 = 5'-11 3/16, D46 = 5'-11 1/2, D47A = 6'-1 15/16,
 D47 = 5'-11 3/4, D48 = 6'-5 15/16, D49 = 6'-2 13/16,
 D410 = 6'-2 3/4, D411 = 5'-9 3/8, D414 = 5'-11
 10 D415 = 6'-4 13/16

This sheet (13A) replaces sheet (43) of the original contract drawings. All revisions made by the consultant. R DE 46-65

AS BUILT

ILLINOIS DIVISION OF HIGHWAYS
SOUTHWEST EXPRESSWAY

FA. RT 133
LAWDALE AVE. STRUCTURE OVER
JOLIET ROAD & SANTA FE RR SPUR
SECTION 0606-628VB
STRUCTURAL STEEL DETAILS

REVISIONS	
NAME	DATE
L.D. BARRETT	3/29/65

SCALE: VERT. DATE 2-18-1964
 DRAWN BY J.W.
 CHECKED BY L.R.K.

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Alfred Benesch & Company
205 North Michigan Avenue, Suite 2400
Chicago, Illinois 60601
312-565-0450 Job No. 10093

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DEPARTMENT OF TRANSPORTATION

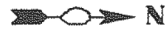
EXISTING PLAN INFORMATION 3 OF 7
STRUCTURE NO. 016-1006

F.A.P. RT.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372	2013-037B-R	COOK	787	260
				CONTRACT NO. 60W75
ILLINOIS FED. AID PROJECT				

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	PLOT DATE = 6/10/2015	DRAWN - CMK	REVISED -
		CHECKED - JAW	REVISED -

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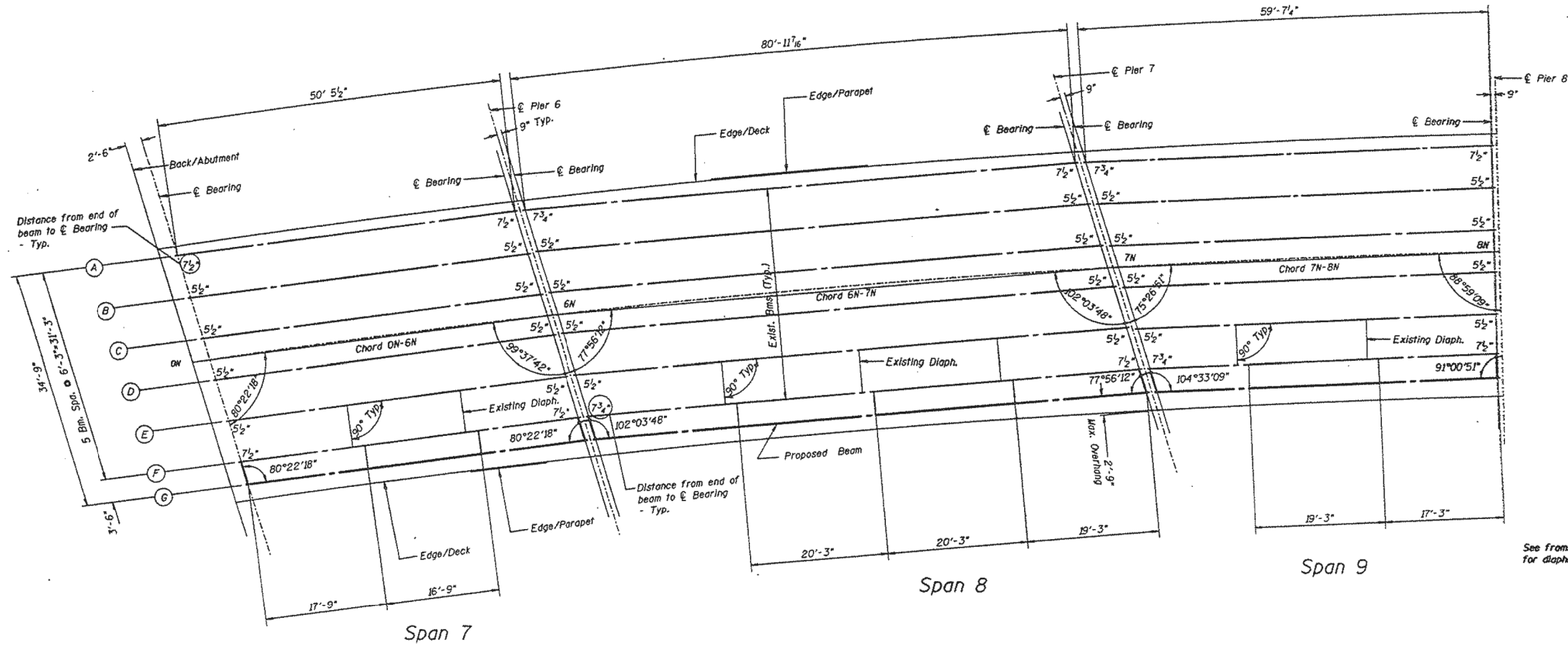
ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	SHEET NO. S-13 SHEETS S-4B
D.B.L.	606-628B	COOK	383	118	
F.A.U. 2746					
FED. ROAD DIST. NO. 7	BLANDED	FED. AID PROJECT			



Note: For table of new beam Moments and Shears, see next sheet.

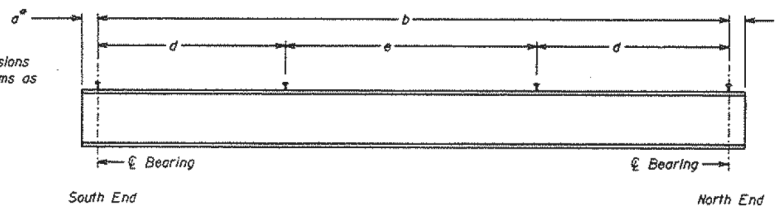
Contractor shall exercise extreme care to not damage the existing beams during removal of the concrete deck.

Saw cutting of existing slab over stringers will not be allowed.



See framing plan for spans 1, 2, & 3 for diaphragm connection details.

"a" & "c" Dimensions for existing beams as indicated on the Framing Plan.



Notes:

All new beams shall have 3/8" x 4" shear studs, 3 across.
See Section A-A, Sheet S-11
All new end diaphragms are W12x40.
New intermediate diaphragms are W12x35, 3'-6" long.
Contractor shall leave existing studs in place, inspecting them for possible damage.
Top of Steel elevations are for fabrication use only.

DESIGNED	JAA
CHECKED	ELD
DRAWN	JAA
CHECKED	ELD

NEW BEAM DATA

Span	Bm Size	a	b	c	d	e	Top of Steel		Bearing Ht.		End Diaph. Length	
							South	North	South	North	South	North
7	W33x118	7 1/2"	50'-5 1/2"	7 1/2"	25 @ 8"	H @ 1'-1 1/2"	635.01	635.69	E 3/4"	F 1'-1 1/2"	3'-8 1/2"	3'-6 1/2"
8	W33x221	7 1/2"	80'-11 7/16"	7 1/2"	33 @ 10"	20 @ 1'-3"	635.68	635.49	E 1/4"	F 1'-2"	3'-7"	3'-7"
9	W33x118	7 1/2"	59'-7 1/4"	7 1/2"	24 @ 8"	H @ 1'-1 1/2"	635.49	635.37	E 3/4"	F 1'-1 1/2"	3'-7 1/2"	3'-6"
10	W33x118	7 1/2"	27'-7 1/4"	8 1/2"	14 @ 8"	7 @ 1'-1 1/2"	635.36	635.26	E 3/4"	F 1'-1 1/2"	3'-6"	3'-11"
11	W27x84	8 1/2"	51'-6 1/4"	8 1/2"	32 @ 6"	19 @ 10"	635.26	635.08	F 1'-1 1/2"	E 3/4"	3'-10"	4'-1 1/2"
12	W27x84	8 1/2"	51'-2 1/4"	7 1/2"	33 @ 6"	18 @ 10"	635.07	634.93	F 1'-1 1/2"	E 3/4"	4'-0 1/2"	4'-0 1/2"

FRAMING PLAN SPANS 7, 8, & 9

BRIDGE REHABILITATION ILLINOIS ROUTE 171
NORTHBOUND
OVER JOLIET RD. & AT&SF R.R.
FAU 2746 SECTION 606-628 VB
STATION 77+49.4
COOK COUNTY
STRUCTURE NO. 016-1006 N.B.

BOWMAN, BARRETT & ASSOCIATES INC.
CONSULTING ENGINEERS
222 N. MICHIGAN AVENUE SUITE 2500 CHICAGO, ILLINOIS 60601
JOB NO. 124

benesch
engineers · scientists · planners
Alfred Benesch & Company
205 North Michigan Avenue, Suite 2400
Chicago, Illinois 60601
312-565-0450 Job No. 10093

FOR INFORMATION ONLY

FILE NAME =	016-1006-60W75-011-Exist fr plan2.dgn
USER NAME =	jsurber
PLOT SCALE =	
PLOT DATE =	6/10/2015

DESIGNED -	CMK	REVISED -	
CHECKED -	JAW	REVISED -	
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**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**EXISTING PLAN INFORMATION 4 OF 7
STRUCTURE NO. 016-1006**

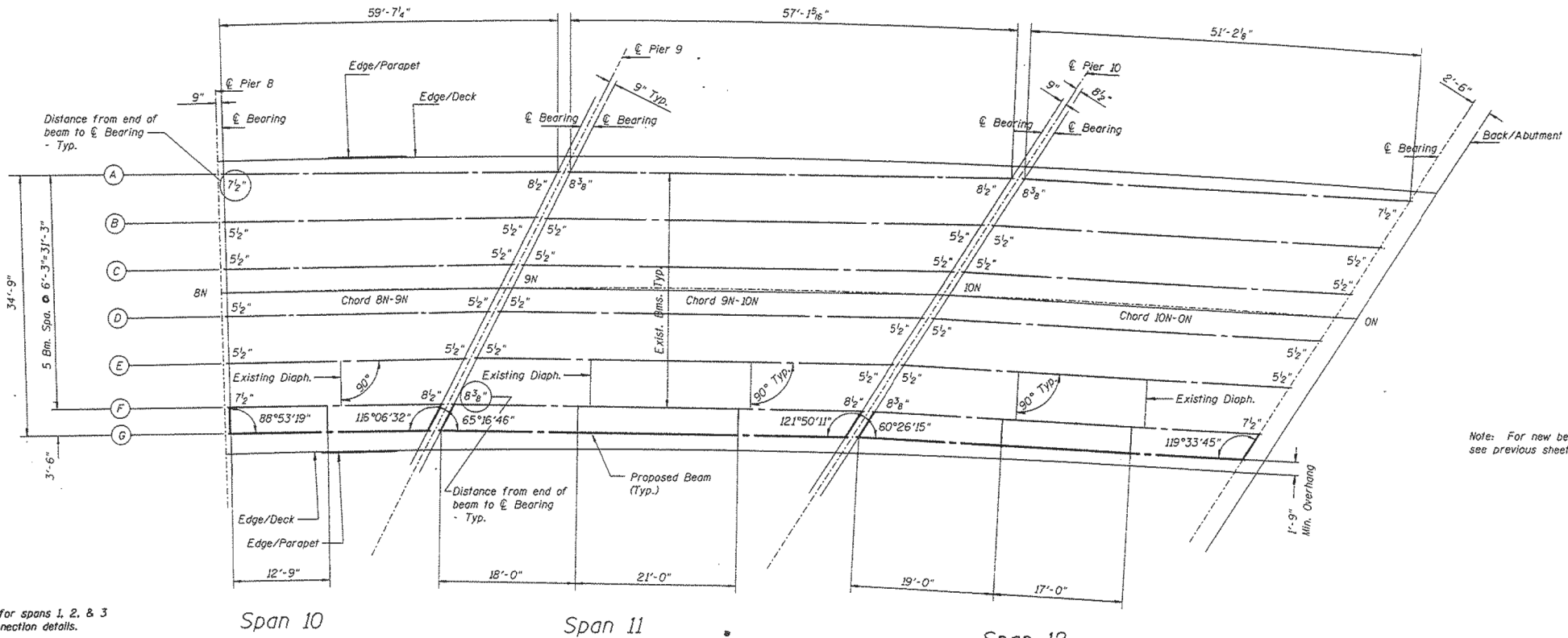
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372	2013-037B-R	COOK	787	261
CONTRACT NO. 60W75				
ILLINOIS FED. AID PROJECT				

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ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	SHEET NO. 5-34 SHEETS 5-48
S. B. I.	606-628VB	COOK	383	119	
F. A. U. 2746					
FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT			

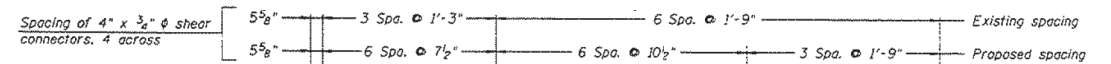
Contractor shall exercise extreme care to not damage the existing beams during removal of the concrete deck.

Saw cutting of existing slab over stringers will not be allowed.



See framing plan for spans 1, 2, & 3 for diaphragm connection details.

Note: For new beam dimensions and other data, see previous sheet.



	Sp. 4	Sp. 5	Sp. 7	Sp. 12
Similar for Spans	2, 3, 8		1, 9, 10	6, 11
Sections	W33X221	W27X178	W33X118	W27X84
I _s (in ⁴)	12800	6900	5900	2850
I _c (in ⁴)	22410	13503	13337	7404
S _s (in ³)	755	503	359	214
S _c (in ³)	942	929	498	313
Z (in ³)	855	567	415	244
M (K/ft)	0.643	0.600	0.540	0.506
M _R (K)	539	335	172	168
s _R (K/ft)	0.488	0.488	0.488	0.488
M _S (K)	409	272	155	162
M _L (K)	483	373	253	262
M (Imp)	117	97	72	74
S _y (M _t -I) (K)	1000	784	544	561
M _c (K)	2533	1809	1131	1159
M _s (K)	2632	1960	1243	1273
f _s (non-comp.) (k.s.i.)	8.6	8.0	5.7	9.5
f _{sR} (comp) (k.s.i.)	14.4	13.6	9.9	16.4
f _s (k.s.i.)	12.9	14.6	13.1	21.5
f _s (Overload) (k.s.i.)	27.3	28.2	23.1	37.9
f _s (Total) (k.s.i.)	-	-	-	-
VR (K)	33	33	31	32

	Sp. 4	Sp. 5	Sp. 7	Sp. 12
Similar for Spans	2, 3, 8		1, 9, 10	6, 11
Sections	W33X221	W27X178	W33X118	W27X84
R _L + R _R (K)	46.3	36.4	25.9	25.7
R _L (K)	22.6	25.8	24.5	24.6
Imp. (K)	6.5	6.7	7.0	7.0
R (Total) (K)	79.4	68.9	57.4	57.3

I_s and S_s are the moment of inertia and section modulus of the steel section used in computing f_s (Total & Overload).
I_c and S_c are the moment of inertia and section modulus of the composite section used in computing f_s (Total & Overload).
VR is the maximum live load + impact shear range in span.
Z is the plastic section modulus used to determine the Fully Plastic Moments in the non-composite areas.
M_a (Applied Moment) = 1.3(M_R + M_S + 5/8(M_L + I)).
M_u is the Full Plastic Moment Capacity for Compact, Braced section.
f_s (Overload) is the sum of the stresses due to M_R + M_S + 5/8(M_L + I).
f_s (Total) is the sum of the stresses due to 1.3(M_R + M_S + 5/8(M_L + I)).

DESIGNED	JAA
CHECKED	ELD
DRAWN	JAA
CHECKED	ELD

Beam F, Span 10

Showing location of 48 additional shear connectors

FRAMING PLAN SPANS 10, 11, & 12

BRIDGE REHABILITATION ILLINOIS ROUTE 171
NORTHBOUND
OVER JOLIET RD. & AT&SF R.R.
FAU 2746 SECTION 606-628 VB
STATION 77+49.4
COOK COUNTY

STRUCTURE NO. 016-1006 N.B.

BOWMAN, BARRETT & ASSOCIATES INC.
CONSULTING ENGINEERS
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JOB NO. 024

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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

EXISTING PLAN INFORMATION 5 OF 7
STRUCTURE NO. 016-1006

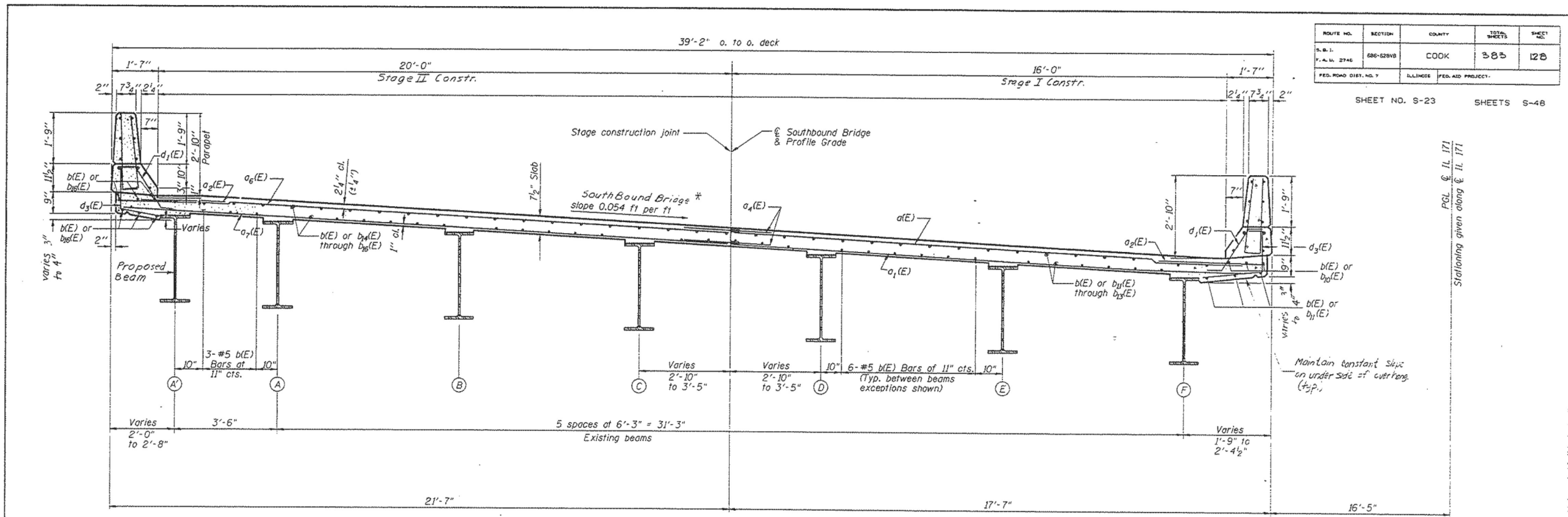
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372	2013-037B-R	COOK	787	262
CONTRACT NO. 60W75			ILLINOIS FED. AID PROJECT	

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312-565-0450 Job No. 10093

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016-1006-60W75-012-Exist fr plan3.dgn	PLOT SCALE =	CHECKED - JAW	REVISIONS -
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		CHECKED - JAW	REVISIONS -

SHEET NO. SAX5 OF SAX7 SHEETS

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ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
S. B. 1	606-628VB	COOK	385	128
F.A. 2746				
FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT		

SHEET NO. S-23 SHEETS 5-48

NORTHBOUND BRIDGE CROSS SECTION
(Looking South)
SOUTHBOUND BRIDGE CROSS SECTION
(Looking North)

Bars shown are specific to span #4; other spans are similar.

* Southbound Bridge & Northbound Bridge are symmetrical by 180° rotation, & keep the superelevation of both bridges slope down toward the East

Notes: See Sheet #s S11 thru S22 for superstructure details and Sheet #s S24 thru S29 for Bills of Material. Reinforcement bars designated (E) shall be epoxy coated. Bars indicated thus 20 x 3-#5 etc. indicates 20 lines of bars with 3 lengths per line. See Sheet #s S24 thru S29 for parapet reinforcement.

DESIGNED	MK
CHECKED	JAA
DRAWN	EF
CHECKED	JAA

BOWMAN, BARRETT & ASSOCIATES INC.
CONSULTING ENGINEERS
225 N. MICHIGAN AVENUE SUITE 2800 CHICAGO, ILLINOIS 60611
JOB NO. 124

DECK CROSS SECTION
BRIDGE REHABILITATION ILLINOIS ROUTE 171
NORTHBOUND & SOUTHBOUND
OVER JOLIET RD. & AT&SF R.R.
FAU 2746 SECTION 606-628 VB
STATION 77+49.4
COOK COUNTY
STRUCTURE NO. 016-1005 SOUTHBOUND
STRUCTURE NO. 016-1006 NORTHBOUND

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Chicago, Illinois 60601
312-565-0450 Job No. 10093

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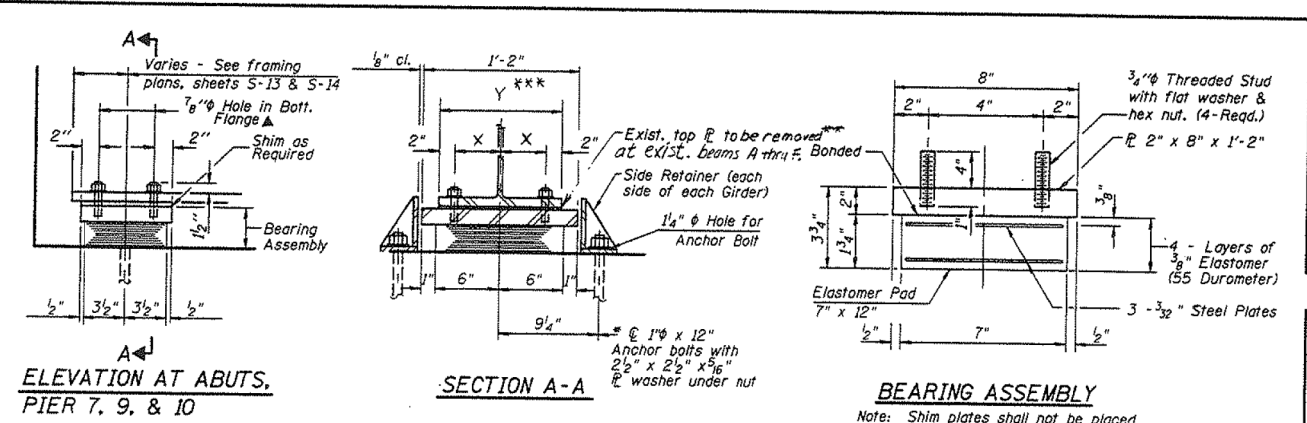
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

EXISTING PLAN INFORMATION 6 OF 7
STRUCTURE NO. 016-1006

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372	2013-037B-R	COOK	787	263
				CONTRACT NO. 60W75
ILLINOIS FED. AID PROJECT				

SHEET NO. SAX6 OF SAX7 SHEETS

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ELEVATION AT ABUTS, PIER 7, 9, & 10

SECTION A-A

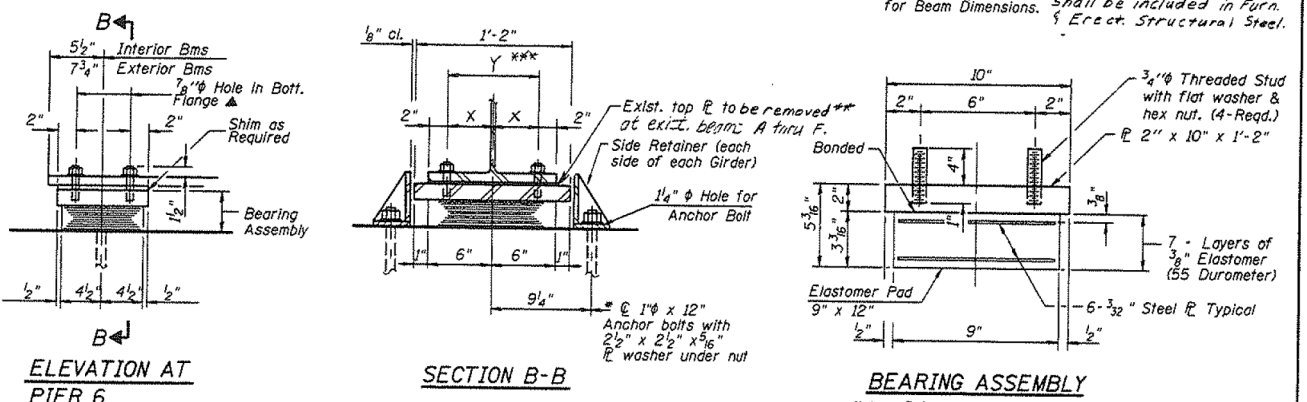
BEARING ASSEMBLY

TYPE I ELASTOMERIC EXPANSION BEARING NORTH AND SOUTH ABUTS, PIERS 7, 9 & 10 NORTHBOUND BRIDGE

Note: Shim plates shall not be placed under Bearing Assembly.

** The Contractor shall remove the existing top flange welded to the bottom flange by the air arc method. Cost shall be included in Furn. & Erect. Structural Steel.

*** Dimensions vary. See Table 2 for Beam Dimensions.



ELEVATION AT PIER 6

SECTION B-B

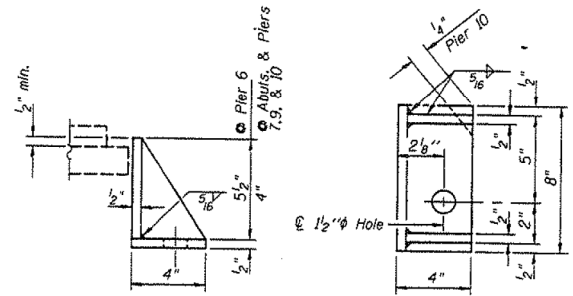
BEARING ASSEMBLY

TYPE I ELASTOMERIC EXPANSION BEARING PIER 6 NORTHBOUND BRIDGE

Note: Shim plates shall not be placed under Bearing Assembly.

NOTE:
Existing steel shall not be raised more than 1/8" above existing elevation for removal of existing exp. bearings. Jack all beams simultaneously. The max. difference in elevation of two adjacent beams shall not be more than 1/8" during jacking. Anchor bolts shall be removed or cut off and ground flush with top of concrete and sealed with epoxy. If anchor bolts are removed, exist. holes shall be filled with non-shrink grout.

Note: 7/8" Holes shall be field drilled in existing flange by Contractor at existing beams A thru F. 1/2" holes shall be shop drilled for new beams G. Cost shall be included in Furn. & Erect. Structural Steel. (L. Sum)



SIDE RETAINER

Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates. Weight included with Structural Steel.

BILL OF MATERIAL

Item	Unit	Total
Elastomeric Bearing Assembly Type 1	Each	42

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
FAU 2746	606-628VB	COOK	121	5-16

SHEETS 5-48

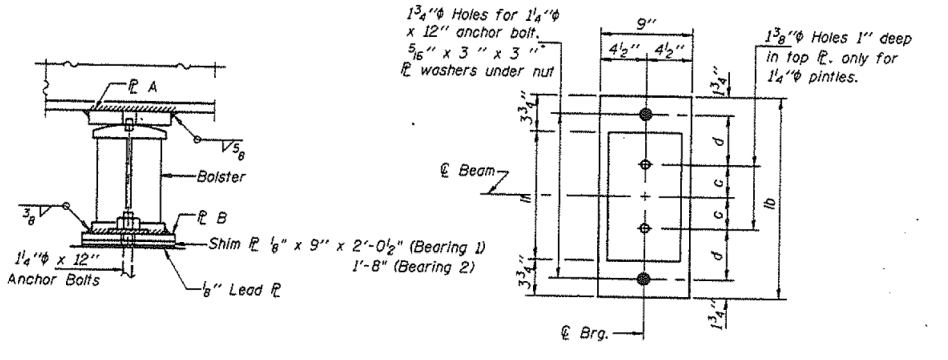
TABLE 2, North Bound Bridge

BEAM	N. ABUT.		PIER 6		PIER 7		PIER 9		PIER 10		S. ABUT.	
	X	Y	X	Y	X	Y	X	Y	X	Y	X	Y
A	3 1/4"	6 1/2"	4"	8"	3 1/4"	6 1/2"	3 1/4"	6 1/2"	3 1/4"	6 1/2"	3 1/4"	6 1/2"
B	3 1/4"	6 1/2"	4"	8"	3 1/4"	6 1/2"	3 1/4"	6 1/2"	3 1/4"	6 1/2"	3 1/4"	6 1/2"
C	3 1/4"	6 1/2"	4"	8"	3 1/4"	6 1/2"	3 1/4"	6 1/2"	3 1/4"	6 1/2"	3 1/4"	6 1/2"
D	3 1/4"	6 1/2"	4"	8"	3 1/4"	6 1/2"	3 1/4"	6 1/2"	3 1/4"	6 1/2"	3 1/4"	6 1/2"
E	3 1/4"	6 1/2"	4"	8"	3 1/4"	6 1/2"	3 1/4"	6 1/2"	3 1/4"	6 1/2"	3 1/4"	6 1/2"
F	4"	8"	4"	8"	4"	8"	4"	8"	3 1/4"	6 1/2"	3 1/4"	6 1/2"

FIXED BEARING TABLE

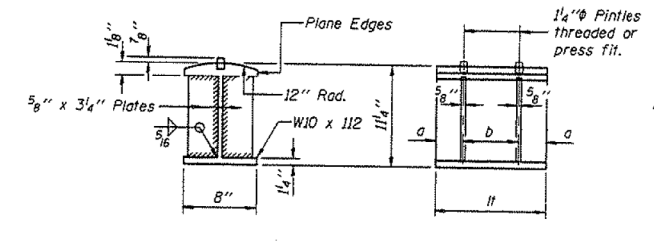
FIXED BEARING NO.	PIERS	PLATE A	PLATE B	11	1b	a	b	c	d	No. req'd
1	7	1 3/4" X 9" X 1'-5"	1" X 9" X 2'-0 1/2"	17"	24 1/2"	3"	20 5/8"	5"	5"	1
2	6,8,9,10	1 3/8" X 9" X 1'-0 1/2"	3/8" X 9" X 1'-8"	12 1/2"	20"	3"	20 3/4"	3 1/4"	5"	5

NOTE: 2 Brgs. Pier 8



PIER
SEE TABLE FOR LOCATIONS AND DIMENSIONS

PLAN
SEE TABLE FOR LOCATIONS AND DIMENSIONS



DETAIL OF BOLSTER

SEE TABLE FOR LOCATIONS AND DIMENSIONS

Note: for detail of 1/4" pntle, see sat. 5-15.

ELASTOMERIC & FIXED BEARING DETAILS

BRIDGE REHABILITATION ILLINOIS ROUTE 171 NORTHBOUND & SOUTHBOUND OVER JOLIET RD. & AT&SF R.R. FAU 2746 SECTION 606-628 VB STATION 77+49.4 COOK COUNTY STRUCTURE NO. 016-1006 NORTHBOUND

BOWMAN, BARRETT & ASSOCIATES INC. CONSULTING ENGINEERS 333 N. MICHIGAN AVENUE SUITE 2800 CHICAGO, ILLINOIS 60601 JOB NO. 04

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STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

EXISTING PLAN INFORMATION 7 OF 7 STRUCTURE NO. 016-1006

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372	2013-037B-R	COOK	787	264

CONTRACT NO. 60W75

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205 North Michigan Avenue, Suite 2400
Chicago, Illinois 60601
312-565-0450 Job No. 10093

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	jsurber	CMK	JAW	CMK	JAW	-	-	-	-

PLOT SCALE =
PLOT DATE = 6/10/2015

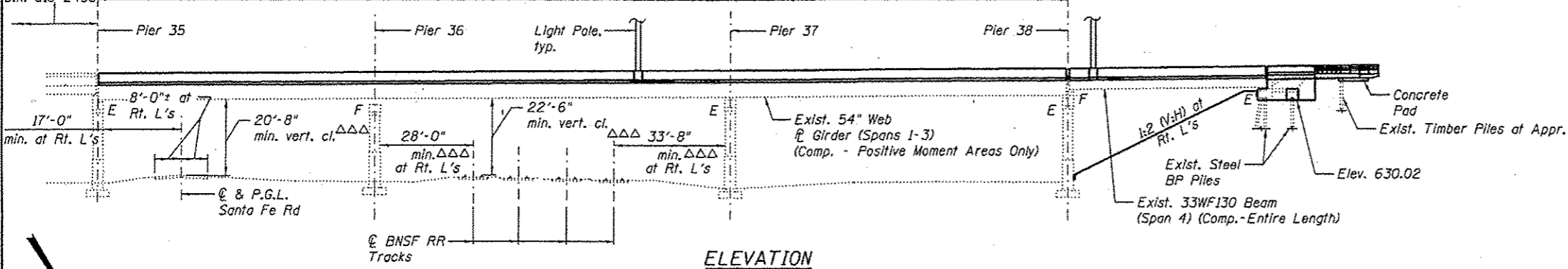
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Bench Mark: Chiseled square of NE corner of NB IL-171 bridge wingwall over BNSF R.R. El. 640.61

Existing Structure: S.N. 016-2454 (NB) was built in 1964 as F.A. Rte. 133, Section 0606-627VB at Sta. 65+06.90. Existing structure consists of a three span reinforced concrete deck on seven continuous 54" steel welded plate girders in spans 1 thru 3 and a single span reinforced concrete deck on eight steel 33" WF beams in span 4. The reinforced concrete deck is 8 1/2" thick, including a 1/2" microsilica concrete overlay. The substructure consists of open stub abutments founded on steel piles at the north end and by multi-column circular piers founded on spread footings. The south pier is shared with S.N. 016-2456 (NB). S.N. 016-2454 measures 363'-9" from back of north abutment to centerline of south pier, with an out-to-out deck width varying from 50'-3 1/4" to 57'-4". Piers 35, 36, and 37 are at a skew angle of 6°08'00" and Pier 38 and the North Abutment are at a skew angle of 15°03'45". The concrete deck is to be removed and replaced with traffic maintained utilizing crossovers. No salvage.

All Elevations in the proposed plans are based on NAVD88 Datum. Elevations in the existing plans are based on the NGVD29 Datum. NGVD29 Elev. 584.50 = NAVD88 Elev. 584.22.

S.N. 016-2456 Limits of Proposed and Existing Protective Shield (See Notes)



Notes:
Existing Protective Shield to be removed. (Cast Included in "Removal of Existing Deck No. 1").
No freefall drains will be permitted over Santa Fe Road and the BNSF RR tracks or within 10 ft. of cross arms of a railroad pole line.
BNSF RR Milepost 12.08 is located approximately at the @ IL-171.

DESIGN SPECIFICATIONS

2002 AASHTO Standard Specifications for Highway Bridges, 17th Edition

DESIGN STRESSES

FIELD UNITS (New Construction)

f'c = 3,500 psi
fy = 60,000 psi (Reinforcement)
fy = 50,000 psi (M270 Grade 50)

FIELD UNITS (Exst. Construction)

f'c = 3,500 psi
fy = 40,000 psi (Reinforcement)
fy = 36,000 psi (Structural Steel)

LOADING HS20-44

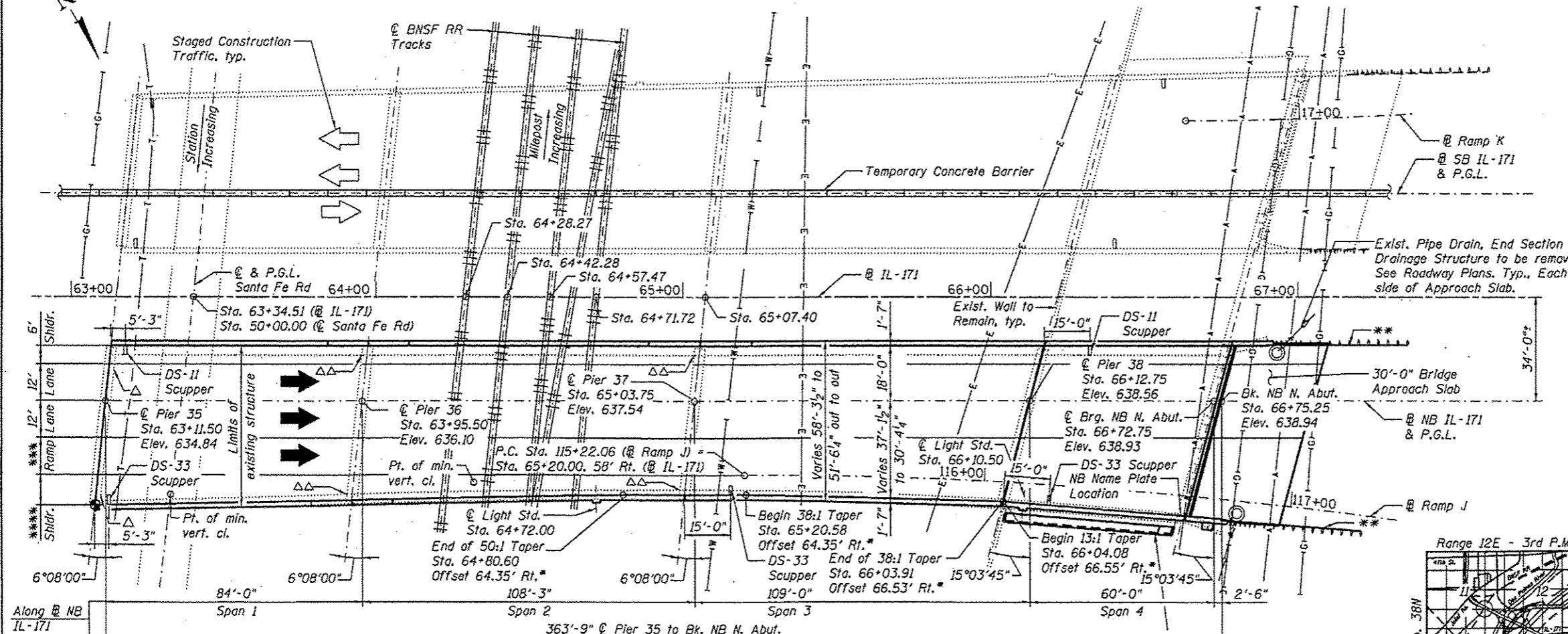
No future wearing surface allowed.

SEISMIC DATA

Seismic Performance Category (SPC) = A
Bedrock Acceleration Coefficient (A) = 0.04g
Site Coefficient (S) = 1.0



L. Stock 6/9/15
Lynn M. Stock
Licensed Structural Engineer
State of Illinois No. 081-006848
Expires 11/30/2016



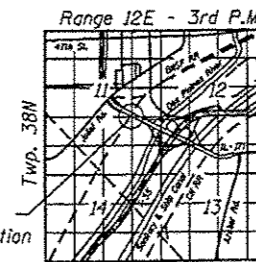
EXISTING UTILITY LEGEND

- S — Underground Storm Sewer
- A — Aerial Electric Line
- G — Underground Gas Line
- E — Underground Electric Line
- T — Underground Telephone Line
- W — Underground Water Line
- ⊙ Existing Drainage Structure

APPROVED

For Structural Adequacy Only

L. Carl Ramsey
Engineer of Bridges & Structures



LOCATION SKETCH

- Δ Remove and replace existing closed drainage system. See sheets SB24 and SB25.
- ΔΔ Remove existing closed drainage system.
- ΔΔΔ Based on Phase I survey information dated 08/11/2008
- Offset measured perpendicular to @ IL-171
- Traffic Barrier Terminal Type 6 Std. 631031
- Ramp width varies 12'-0" to 18'-7 1/4", measured perpendicular to @ NB IL-171
- Shoulder width varies 6'-3 1/4" to 9'-8 1/4", measured perpendicular to @ NB IL-171

PLAN

	USER NAME = Lin.41	DESIGNED - RCB	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	SHEET NO. SB1 OF SB34 SHEETS
	FILE NAME =	CHECKED - JRS	REVISED -		
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	PLGT DATE = 6/8/2015 3:39:04 PM	CHECKED - JRS	REVISED -		
					F.A.P. RT. 372 SECTION 2013-037B-R COUNTY COOK TOTAL SHEETS 787 SHEET NO. 265 CONTRACT NO. 60W75 ILLINOIS FED. AID PROJECT

GENERAL NOTES

- All new fasteners shall be ASTM A325 Type 1, mechanically galvanized bolts. Bolts 7/8 in. Ø, holes 1 1/16 in. Ø, unless otherwise noted.
- No field welding is permitted except as specified in the contract documents.
- The Contractor shall test the existing welds by non-destructive methods within 2 ft. of the end of the existing cover plates for cracks after removal of the existing concrete deck. Dye penetrant (PT), magnetic particle (MT), or other approved testing method shall be performed by qualified personnel approved by the Engineer. If cracks are found, report them to the Bureau of Bridges and Structures for disposition. The cost of testing is included in Removal of Existing Concrete Deck. The cost of crack repair, if necessary, will be paid for according to Article 109.04 of the Standard Specifications.
- Reinforcement bars designated (E) shall be epoxy coated.
- Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and other loose or potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete.

As directed by the Engineer, existing construction accessories welded to the top flange of beams and girders shall be removed. The weld areas shall be ground flush and inspected for cracks using magnetic particle testing (MT) or dye penetrant testing (PT) by qualified personnel approved by the Engineer. Any cracks that cannot be removed by grinding 1/4 in. deep shall be identified and reported to the Bureau of Bridges and Structures for further disposition. The cost of removing welded accessories, grinding and inspecting weld areas and grinding cracks will be paid for according to Article 109.04 of the Standard Specifications.
- If the Contractor elects to use cantilever forming brackets on the exterior beams or girders, the brackets shall be placed at the same locations required for the hardwood blocks in Article 503.06(b) of the Standard Specifications. If additional cantilever forming brackets are required, hardwood blocking shall be wedged between the exterior and first interior beam at each of these additional bracket locations.
- Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work; however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.
- Concrete Sealer shall be applied to the exposed faces of the reconstructed backwall and widened portions of the north abutment.
- The Inorganic Zinc Rich Primer/Acrylic/Acrylic Paint System shall be used for shop and field painting of new structural steel. Only Inorganic Zinc Rich Primer shall be applied to the new structural steel in the shop under this contract and is included in "Structural Steel Repair". The intermediate and top coats shall be applied under a separate painting contract.
- The existing structural steel coating contains lead. The Contractor shall take all precautions to deal with the presence of lead on this project.
- Existing structural steel shall only be cleaned and painted as required by the Special Provision "Cleaning and Painting Contact Surface Areas of Existing Steel Structures".
- The embankment configuration shown shall be the minimum that must be placed and compacted prior to construction of the abutments.
- The top flange of the existing structural steel is anticipated to have been painted with lead paint. Lead paint removal shall be completed in areas of the top flange that are to receive proposed stud shear connectors. See Special Provisions for "Containment and Disposal of Lead Paint Cleaning Residues."

INDEX OF SHEETS

SB1	General Plan and Elevation
SB2	General Notes, Bill of Material, and Index of Sheets
SB3	Details
SB4-SB5	Stage Construction Details
SB6	Temporary Concrete Barrier for Stage Construction
SB7-SB11	Top of Slab Elevations
SB12	Top of Approach Slab Elevations
SB13-SB15	Superstructure
SB16-SB17	Superstructure Details
SB18	Concrete Parapet Slip Forming Option
SB19-SB20	Bridge Approach Slab Details
SB21	Preformed Joint Strip Seal
SB22	Drainage Scupper, DS-II
SB23	Drainage Scupper, DS-33
SB24-SB25	Pier Drainage System Details
SB26	Framing Plan
SB27	Structural Steel Details
SB28	North Abutment Concrete Removal and Repair Details
SB29-SB31	North Abutment Modification Details
SB32-SB33	Pier Concrete Repair and Drainage System removal Details
SB34	Bar Splicer Assembly Details

For Existing Bridge Plans, see Sheets SBX1 thru SBX14 immediately following Sheet SB34.

SCOPE OF WORK

- Remove existing concrete deck and microsilica concrete overlay and replace with new 8" reinforced concrete deck.
- Make new deck composite in positive moment areas only by adding shear studs to all girders and beams where not already installed.
- Remove and replace existing expansion joints.
- Remove and replace approach slab.
- Remove and replace backwall.
- Repair spalls, delaminations and open cracks in substructures using structural repair of concrete and epoxy crack injection as shown.
- Repair slopewall using epoxy crack injection as shown.
- Modify wingwalls and slopewall as shown for the new deck width.
- Retrofit steel superstructure fatigue prone details at cover plates as shown.
- Remove all bottom wind bracing lateral angles.
- Remove wind bracing gusset plates where shown.
- Remove and dispose of existing electrical conduits and junction boxes attached to the beams and/or deck.
- Remove and replace the existing closed drainage system at Pier 35.
- Remove the existing closed drainage systems at Piers 36 and 37.
- Perform miscellaneous repairs including repairing unseated anchor bolts, and removing debris and vegetation at slopewalls.
- Remove and replace existing roadway lighting. See electrical plans for cost.

TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Concrete Removal	Cu. Yd.		21.6	21.6
Removal of Existing Concrete Deck No. 1	Each	1		1
Protective Shield	Sq. Yd.	1,772		1,772
Structure Excavation	Cu. Yd.		103	103
Concrete Structures	Cu. Yd.		49.0	49.0
Concrete Superstructure	Cu. Yd.	702.8		702.8
Bridge Deck Grooving	Sq. Yd.	2,116		2,116
Protective Coat	Sq. Yd.	2,565		2,565
Stud Shear Connectors	Each	1,642		1,642
Reinforcement Bars, Epoxy Coated	Pound	185,110	4,570	189,680
Bar Splicers	Each	57		57
Slope Wall 4 Inch	Sq. Yd.		20	20
Name Plates	Each		1	1
Preformed Joint Strip Seal	Foot	168.0		168.0
Concrete Sealer	Sq. Yd.		299	299
Epoxy Crack Injection	Foot		72	72
Geocomposite Wall Drain	Sq. Yd.		44	44
Remove Conduit Attached to Structure	Foot	756		756
Granular Backfill for Structures	Cu. Yd.		103	103
Structural Steel Removal	Pound	10,140		10,140
Structural Steel Repair	Pound	4,700		4,700
Containment and Disposal of Lead Paint Cleaning Residues No. 1	L. Sum	1		1
Cleaning Bridge Seats	Sq. Ft.		854	854
Structural Repair of Concrete (Depth Equal to or Less Than 5 Inches)	Sq. Ft.		318	318
Structural Repair of Concrete (Depth Greater than 5 Inches)	Sq. Ft.		64	64
Drainage Scuppers, DS-II	Each	2		2
Drainage Scuppers, DS-33	Each	3		3
Drainage System	L. Sum		1	1
Pipe Underdrains for Structures 4"	Foot		64	64
Selective Clearing	Unit		1	1
Temporary Shoring and Cribbing	Each		3	3

* Quantity includes a contingency (above the amounts shown in the bills of material) to account for uncertainties associated with the condition of the existing substructure and the age of the original inspection (2008-09). Actual repair areas will be determined by the Engineer in the field.

** The quantity for this work is estimated. The intent for this work is to remove accumulations of rubbish, vegetation, etc., on the existing slopewall. See special provisions.

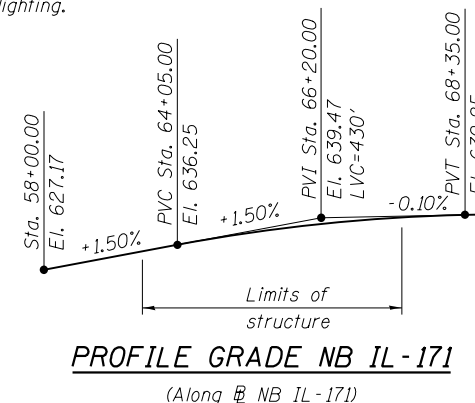
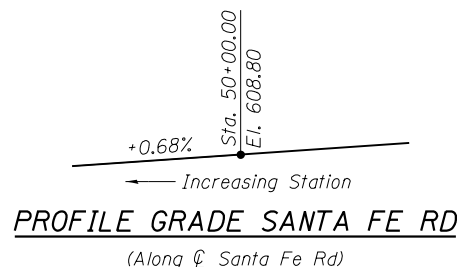
STATION 65+07.40
RE-BUILT 201_ BY
STATE OF ILLINOIS
F.A.P. RT. 372
SEC. 2013-037B-R
LOADING HS20
STR. NO. 016-2454

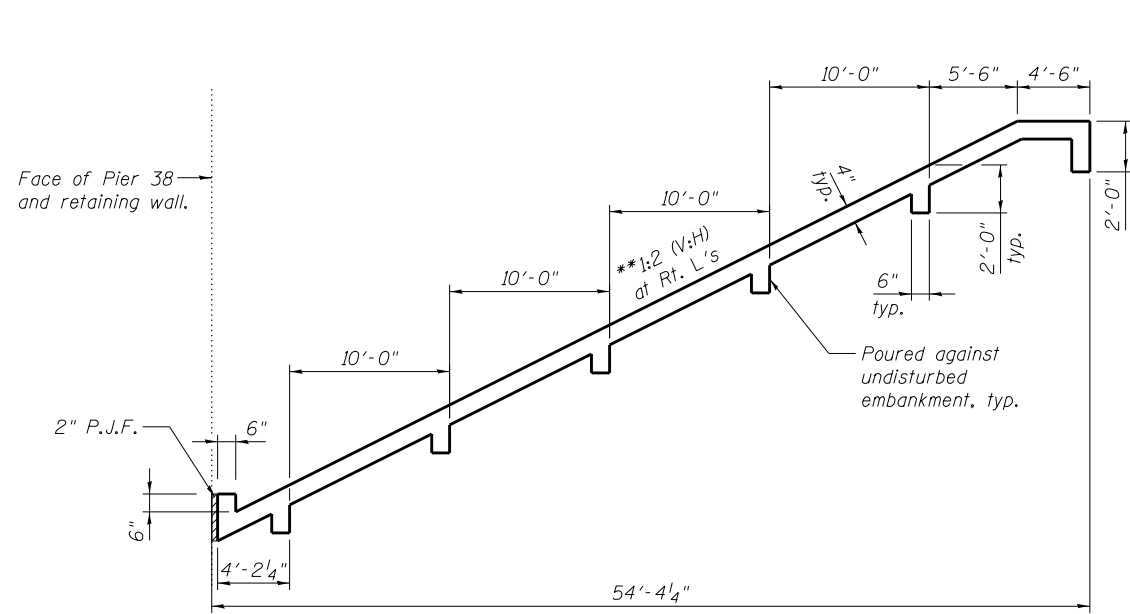
NAME PLATE
(See Std. 515001)

Existing Name Plate shall be cleaned and relocated next to the new Name Plate. Cost included with Name Plates.

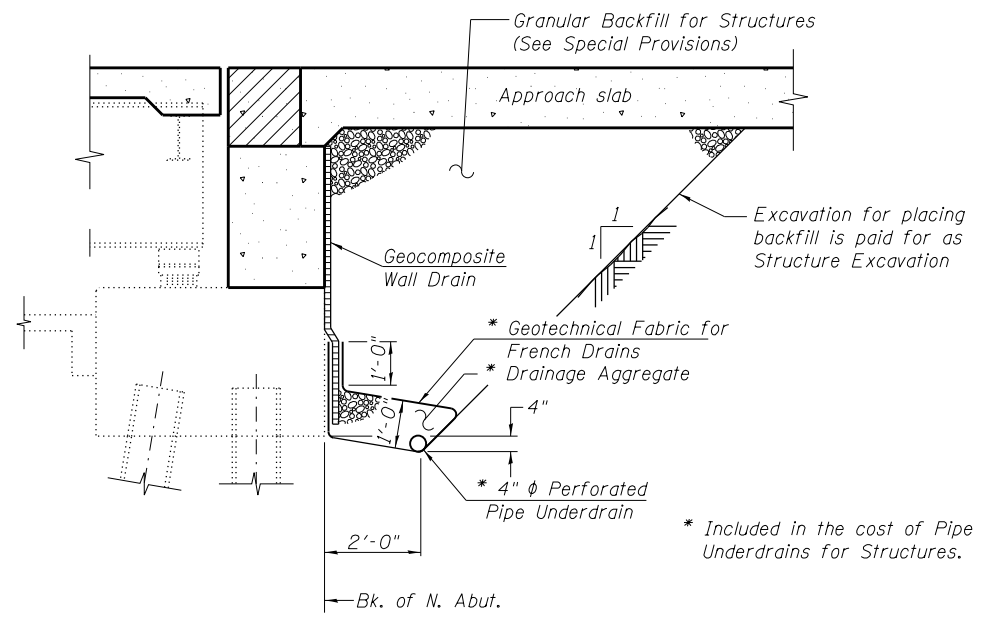
CURVE DATA - @ RAMP J

P.I. Sta. = 116+72.53
Δ = 11°05'22" (RT)
D = 3°41'47"
R = 1,550.00'
L = 300.00'
T = 150.47'
E = 7.2865'
P.C. Sta. = 115+22.06
P.T. Sta. = 118+22.06
S.E. = 3.9%





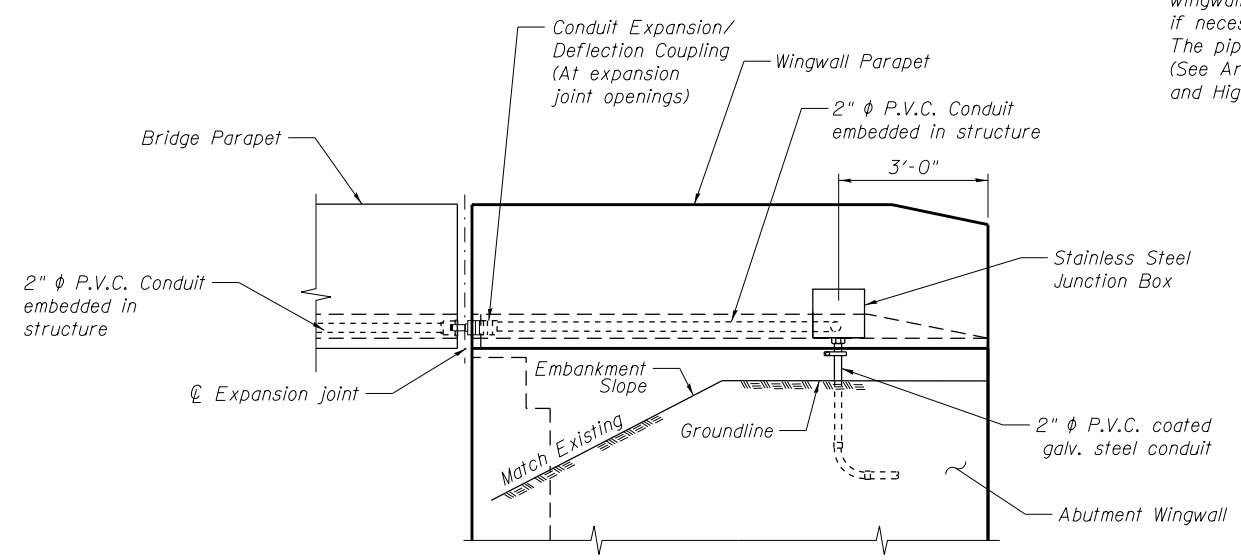
SECTION THRU SLOPEWALL
 (Horizontal dimensions at Rt. L's to Abutment)
 ** Slope to match existing slopewall configuration.



SECTION THRU ABUTMENT
 (Horizontal dimensions at Rt. L's to Abutment)

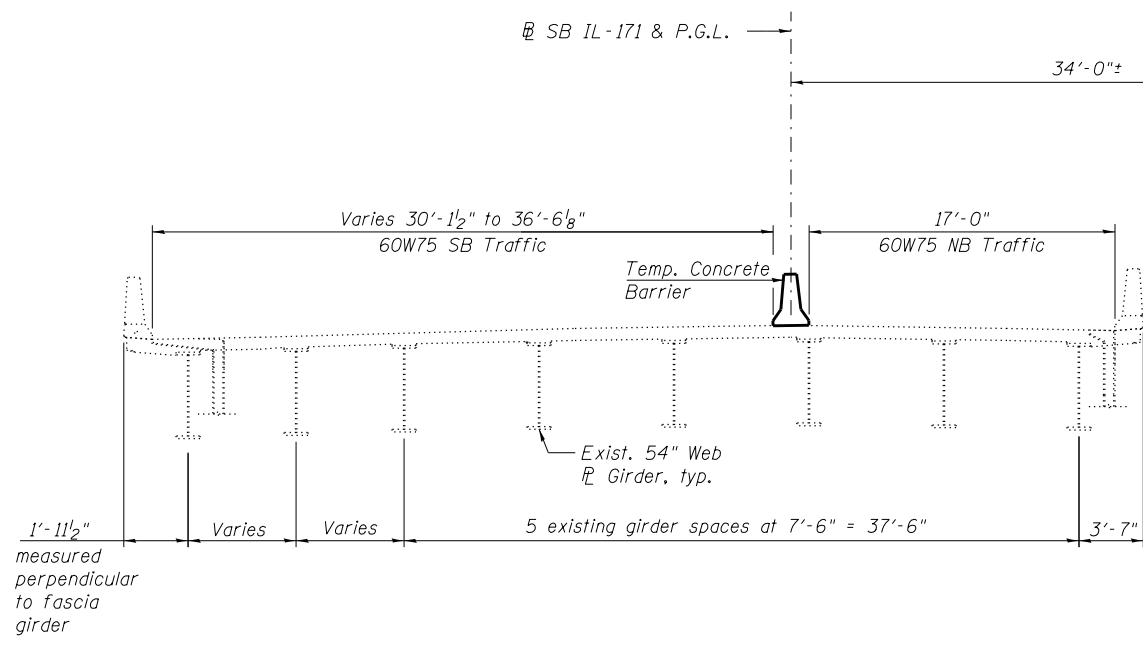
All drainage system components shall extend parallel to the abutment back wall until they intersect the wingwalls. The pipe shall extend under the wingwall, if necessary, until intersecting the side slopes. The pipe shall drain into concrete headwalls. (See Article 601.05 of the Standard Specifications and Highway Standard 601101).

Note:
 See Sheet SB28 for additional slopewall details.

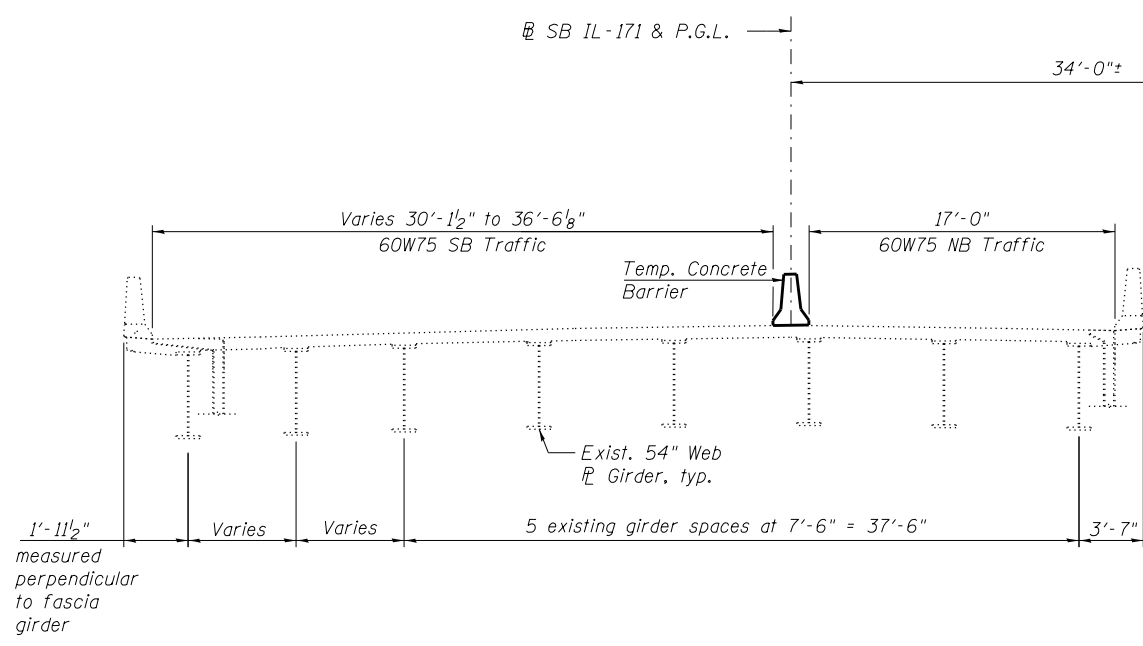
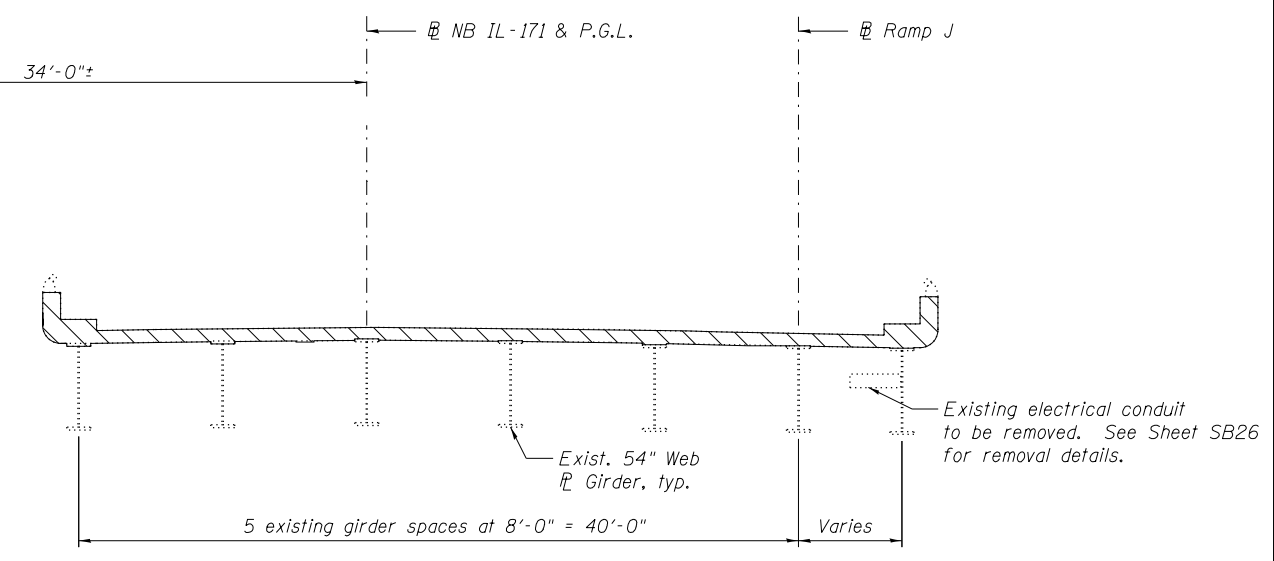


CONDUIT DETAIL
 (East Parapet only)

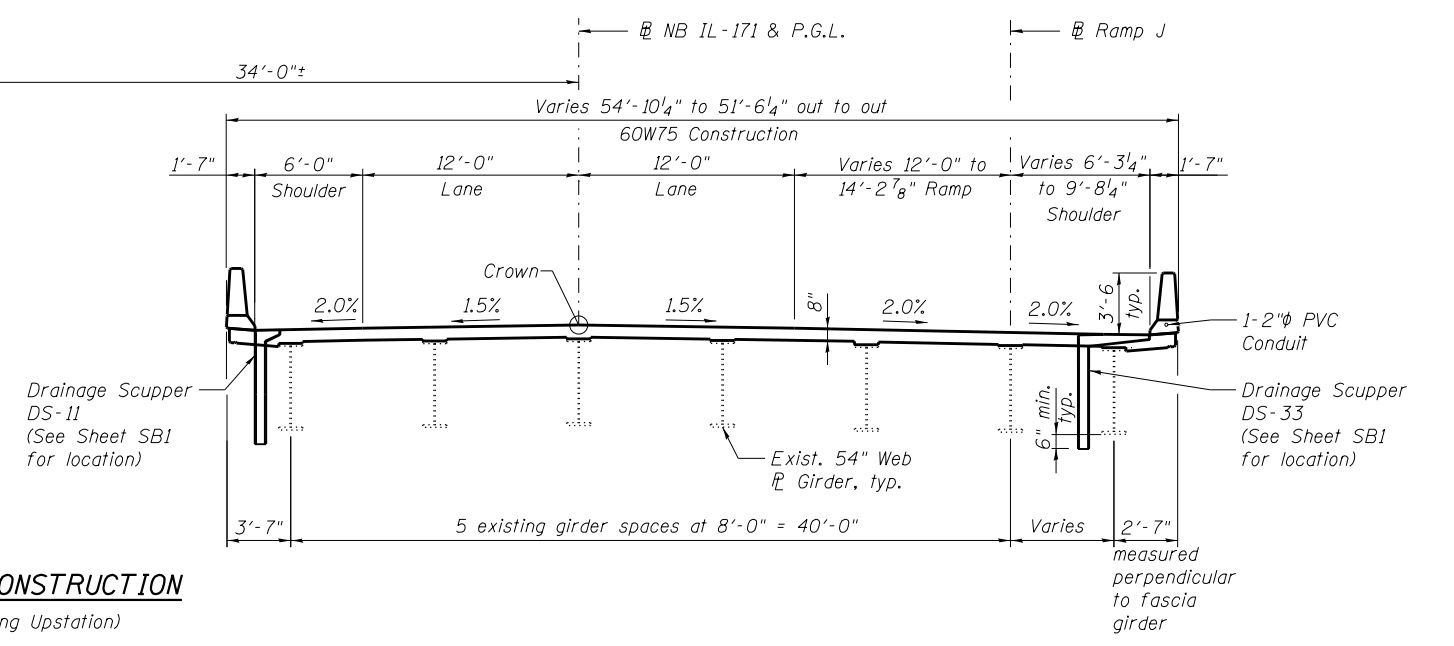
For Conduit, Junction Box & Couplings, see Electrical Plans.



CONTRACT 60W75 REMOVAL
(Spans 1 thru 3, Looking Upstation)



CONTRACT 60W75 CONSTRUCTION
(Spans 1 thru 3, Looking Upstation)



LEGEND

Removal of Existing Concrete Deck No. 1

Notes:
All dimensions are measured perpendicular to NB or SB IL-171 unless noted otherwise.
Temporary Concrete Barrier shall not be anchored to deck.
Refer to Sheet SB6 for Temporary Concrete Barrier.
See Roadway Plans for Maintenance of Traffic and Pay Item for Temporary Concrete Barrier.

(Sheet 1 of 2)



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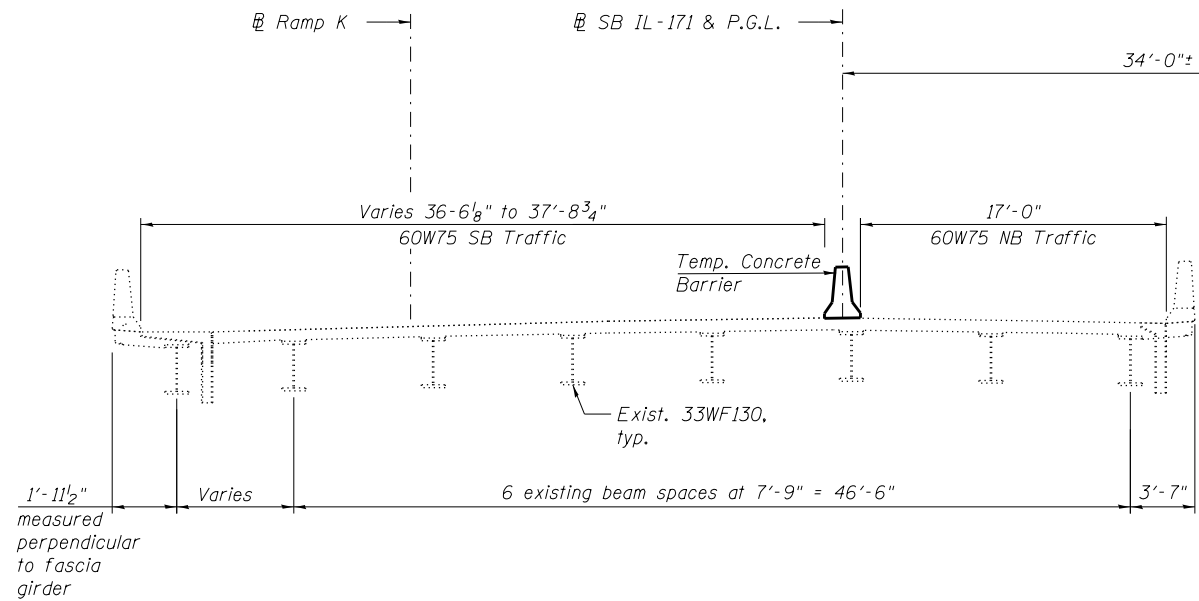
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**STAGE CONSTRUCTION DETAILS
STRUCTURE NO. 016-2454**

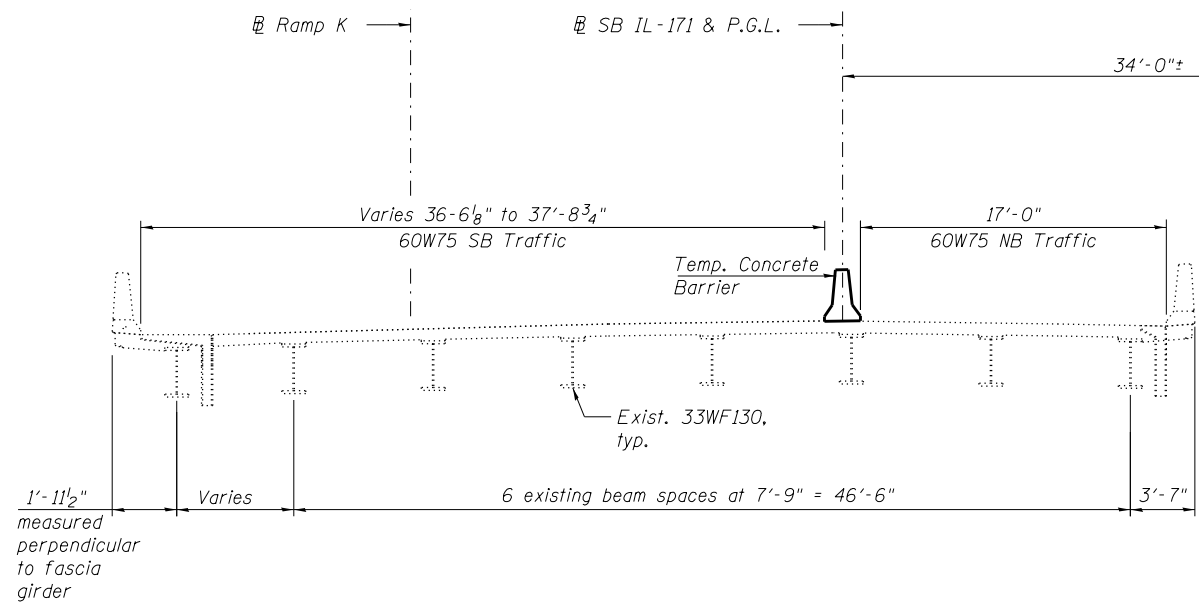
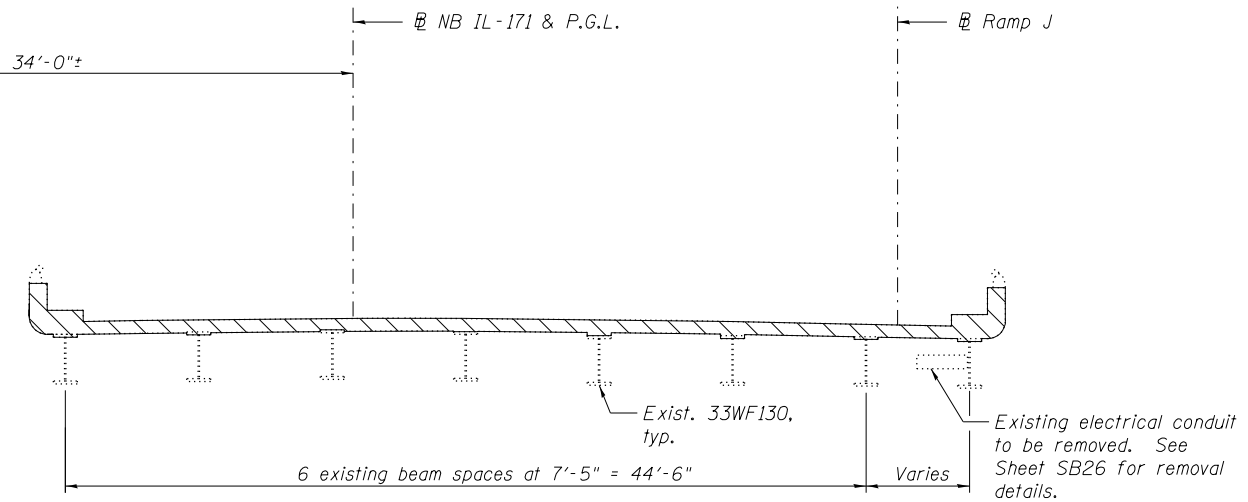
SHEET NO. SB4 OF SB34 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372	2013-037B-R	COOK	787	268
CONTRACT NO. 60W75				

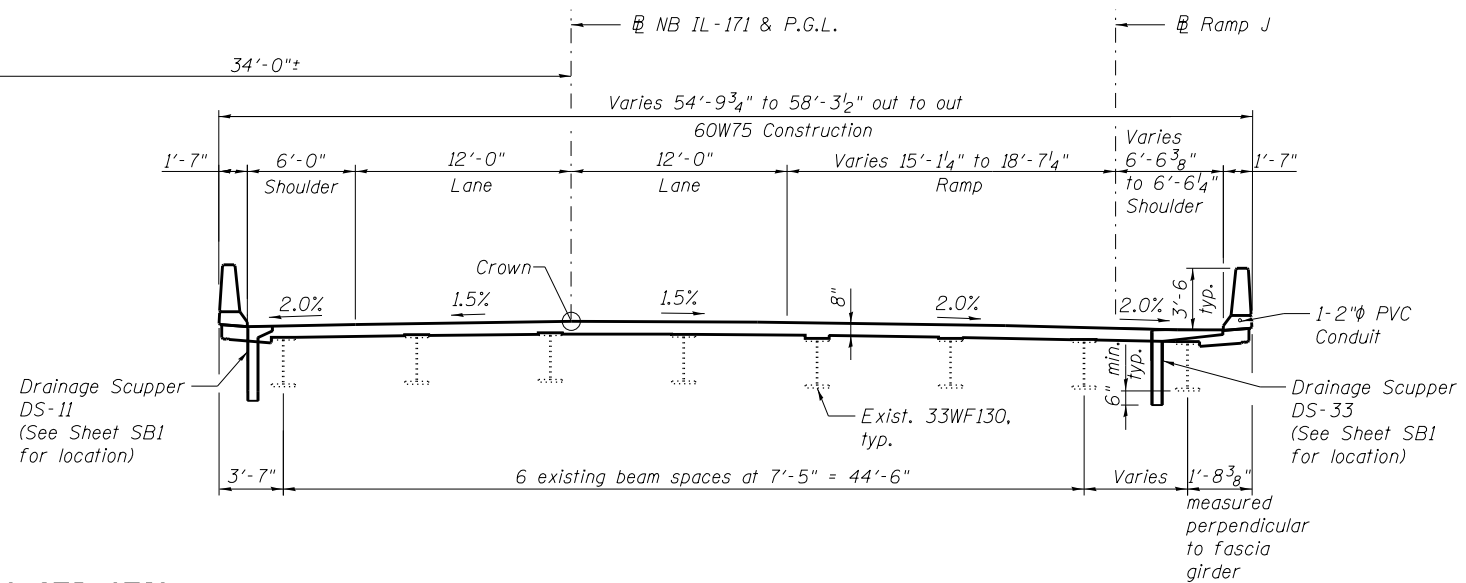
ILLINOIS FED. AID PROJECT



CONTRACT 60W75 REMOVAL
(Span 4, Looking Upstation)



CONTRACT 60W75 CONSTRUCTION
(Span 4, Looking Upstation)



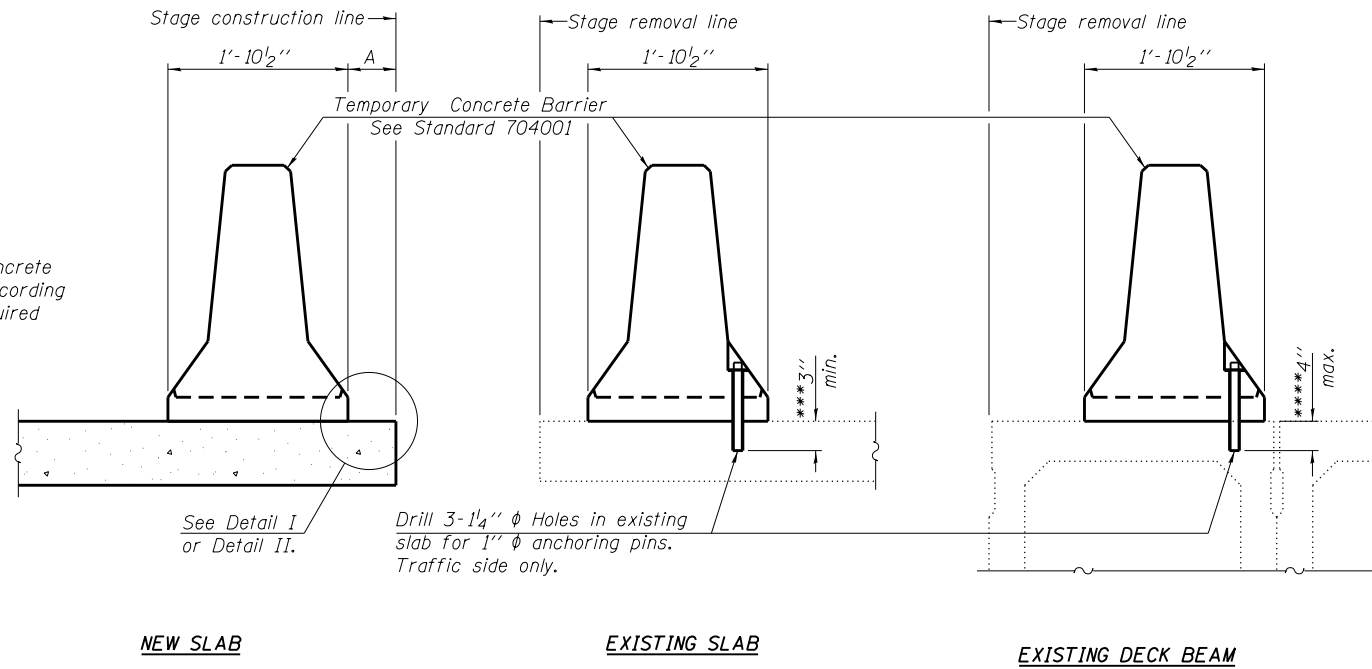
LEGEND

Removal of Existing Concrete Deck No. 1

Notes:
All dimensions are measured perpendicular to NB or SB IL-171 unless noted otherwise.
Temporary Concrete Barrier shall not be anchored to deck.
Refer to Sheet SB6 for Temporary Concrete Barrier.
See Roadway Plans for Maintenance of Traffic and Pay Item for Temporary Concrete Barrier.

(Sheet 2 of 2)

When "A" is 3'-1" or less, the temporary concrete barrier shall be anchored to the new slab according to Detail I or Detail II. No anchorage is required when "A" is greater than 3'-1".



SECTIONS THRU SLAB OR DECK BEAM

NOTES

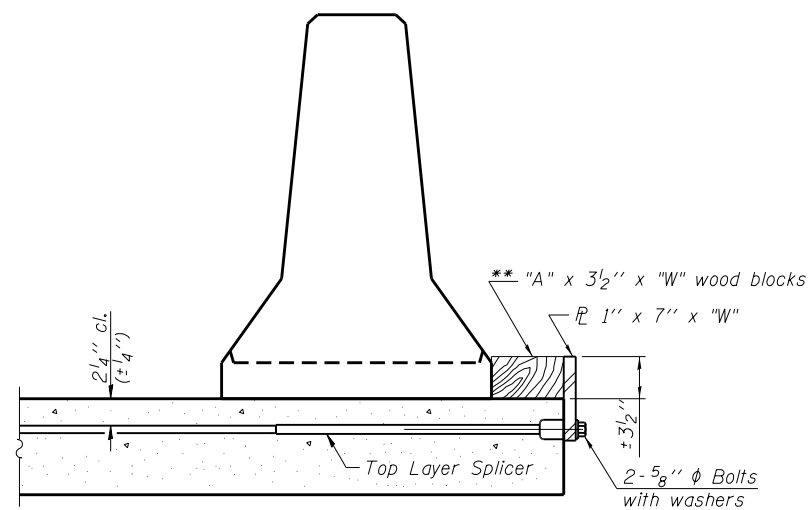
Detail I - With Bar Splicer or Couplers:
Connect one (1) 1" x 7" x "W" steel \bar{L} to the top layer of couplers with 2-5/8" ϕ bolts screwed to coupler at approximate \bar{C} of each barrier panel.

Detail II - With Extended Reinforcement Bars:
Connect one (1) 1" x 7" x "W" steel \bar{L} to the concrete slab or concrete wearing surface with 2-5/8" ϕ Expansion Anchors or cast in place inserts spaced between the top layer of reinforcement at approximate \bar{C} of each barrier panel.

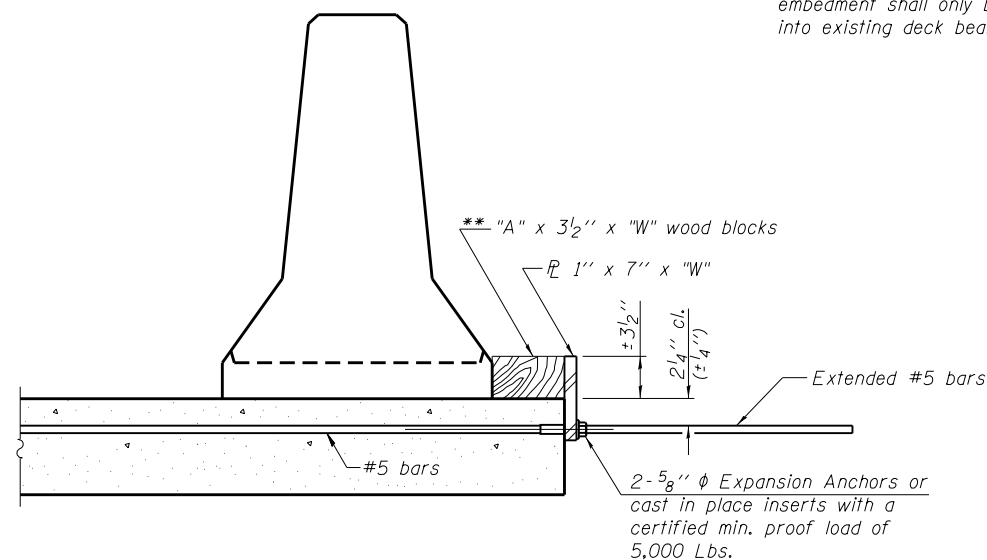
Cost of retainer assembly is included with Temporary Concrete Barrier. The 1" x 7" x "W" plate shall not be removed until stage II construction forms and all reinforcement bars are in place and the concrete is ready to be placed.

*** Dimension shown is minimum required embedment into concrete.
If hot-mix asphalt wearing surface is present, minimum embedment shall be in addition to wearing surface depth.

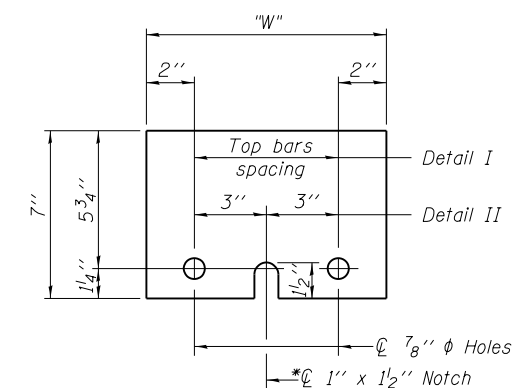
**** If existing deck beam is to remain in place after stage construction, embedment shall only be into wearing surface and not into existing deck beam concrete.



DETAIL I



DETAIL II



STEEL RETAINER \bar{L} 1" x 7" x "W"

* Required only with Detail II

RETAINER ASSEMBLY

** Wood blocks may be omitted when required to provide minimum stage traffic lane width. When the wood blocks are omitted, the concrete barrier shall be in direct contact with the steel retainer plate.

R-27

1-12-15



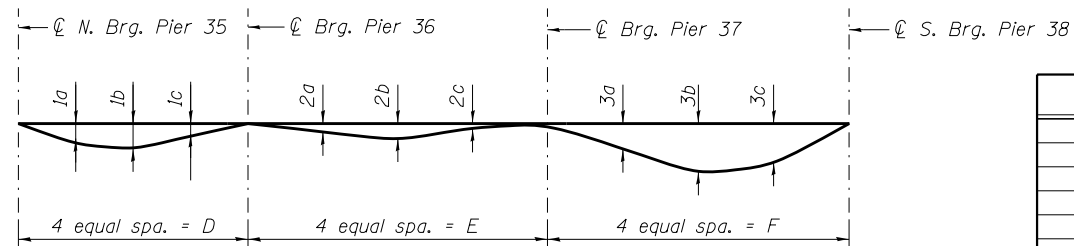
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**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**TEMPORARY CONCRETE BARRIER FOR STAGE CONSTRUCTION
STRUCTURE NO. 016-2454**

SHEET NO. SB6 OF SB34 SHEETS

F.A.P. RE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372	2013-037B-R	COOK	787	270
CONTRACT NO. 60W75			ILLINOIS FED. AID PROJECT	



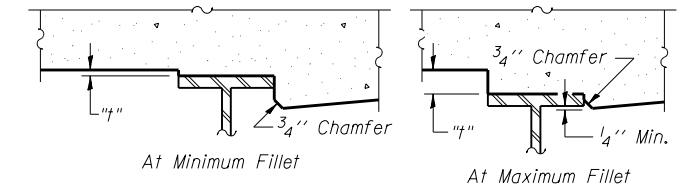
DEAD LOAD DEFLECTION DIAGRAM

(Includes weight of concrete only.)

Note: The above deflections are not for use in the field if the Engineer is working from the "Theoretical Grade Elevations Adjusted For Dead Load Deflection" as shown on Sheets SB9, SB10 and SB11.

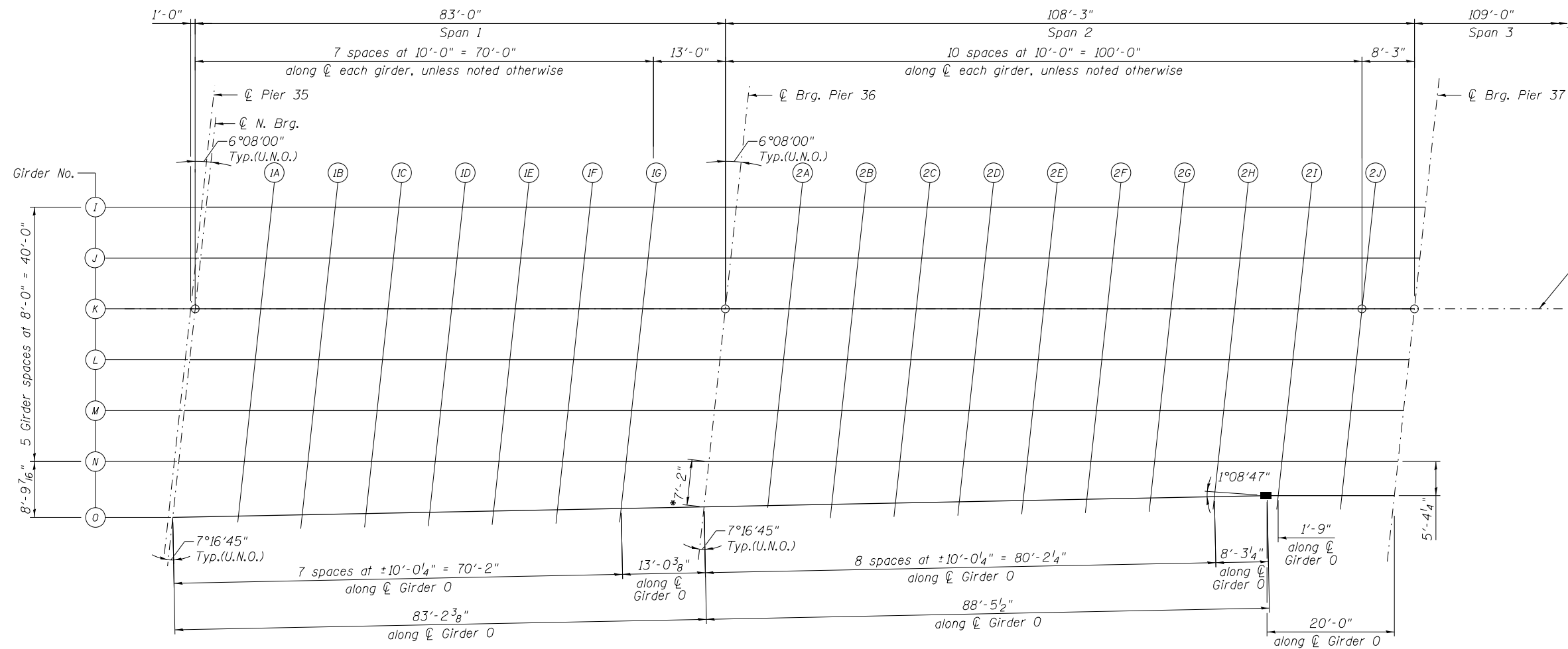
DEAD LOAD DEFLECTION TABLE

Girder	1a	1b	1c	2a	2b	2c	3a	3b	3c	D	E	F
I												110'-8 1/2"
J		3/4"	3/8"		3/8"	0"	7/8"	1 1/4"	1 3/8"			109'-5"
K	5/8"			1/4"	1/2"			1 1/2"	1 1/4"	83'-0"	108'-3"	108'-1 1/2"
L		5/8"	1/4"			1/8"	3/4"	1 3/8"	1 1/8"			106'-10"
M								1 3/8"	1 1/8"			105'-6 1/2"
N		3/4"	3/8"		3/8"		5/8"	1 1/4"	1"			104'-3"
O	1/2"	5/8"	1/4"				1/2"	1"	3/4"	83'-2 3/8"	108'-5 1/2"	102'-9 7/8"



To determine "t": After all structural steel has been erected, elevations of the top flanges of the beams shall be taken at intervals shown below and on sheet SB8. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflection" shown on sheets SB9, SB10 and SB11 minus 8" slab thickness, equals the fillet heights "t" above top flange of beams.

FILLET HEIGHTS

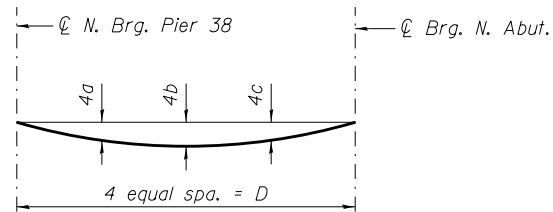


PARTIAL PLAN - SPANS 1 & 2

*Measured along C Pier

Note:
Contractor shall supply top of steel elevation survey data at all screed points to the Engineer for approval before beginning deck formwork operations.

(Sheet 1 of 5)



DEAD LOAD DEFLECTION DIAGRAM

(Includes weight of concrete only.)

Note: The above deflections are not for use in the field if the Engineer is working from the "Theoretical Grade Elevations Adjusted For Dead Load Deflection" as shown on sheet SB9, SB10 and SB11.

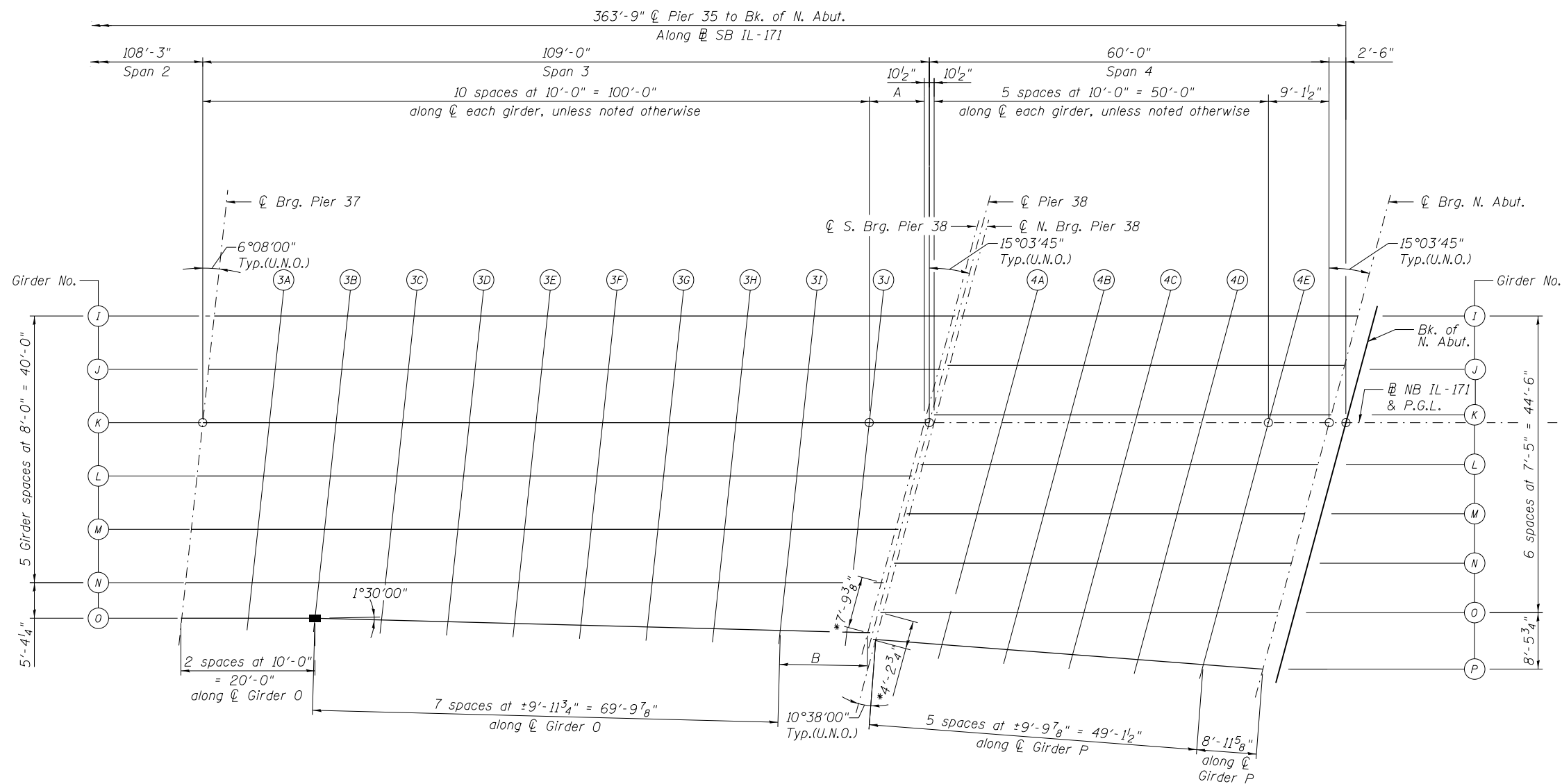
DEAD LOAD DEFLECTION TABLE

Girder	4a	4b	4c	D
I	3/4"		3/4"	
J				
K				
L	5/8"	1"	5/8"	59'-1 1/2"
M				
N				
O		7/8"		
P	1/2"	5/8"	1/2"	58'-1 1/8"

For Span 3 deflection data see Sheet SB7.

DISTANCE ALONG C GIRDER FROM SCREED LINE 3J TO C S. BRG. AT PIER 38

Girder	Distance A	Distance B
I	10'-8 1/2"	—
J	9'-5"	—
K	8'-1 1/2"	—
L	6'-10"	—
M	5'-6 1/2"	—
N	—	14'-3"
O	—	13'-0"



PARTIAL PLAN - SPANS 3 & 4

*Measured along C Brg.

Notes:
For fillet height details see Sheet SB7.
Contractor shall supply top of steel elevation survey data at all screed points to the Engineer for approval before beginning deck formwork operations.

(Sheet 2 of 5)

GIRDER I

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ PIER 35	63+13.22	-16.00	634.61	634.61
☉ N. BRG. PIER 35	63+14.22	-16.00	634.62	634.62
1A	63+24.22	-16.00	634.77	634.80
1B	63+34.22	-16.00	634.92	634.97
1C	63+44.22	-16.00	635.07	635.13
1D	63+54.22	-16.00	635.22	635.28
1E	63+64.22	-16.00	635.37	635.42
1F	63+74.22	-16.00	635.52	635.56
1G	63+84.22	-16.00	635.67	635.69
☉ BRG. & ☉ PIER 36	63+97.22	-16.00	635.87	635.87
2A	64+07.22	-16.00	636.02	636.02
2B	64+17.22	-16.00	636.17	636.18
2C	64+27.22	-16.00	636.31	636.33
2D	64+37.22	-16.00	636.45	636.48
2E	64+47.22	-16.00	636.59	636.62
2F	64+57.22	-16.00	636.72	636.75
2G	64+67.22	-16.00	636.85	636.86
2H	64+77.22	-16.00	636.97	636.98
2I	64+87.22	-16.00	637.09	637.09
2J	64+97.22	-16.00	637.21	637.20
☉ BRG. & ☉ PIER 37	65+05.47	-16.00	637.30	637.30
3A	65+15.47	-16.00	637.41	637.44
3B	65+25.47	-16.00	637.52	637.57
3C	65+35.47	-16.00	637.63	637.71
3D	65+45.47	-16.00	637.72	637.84
3E	65+55.47	-16.00	637.82	637.96
3F	65+65.47	-16.00	637.91	638.06
3G	65+75.47	-16.00	638.00	638.15
3H	65+85.47	-16.00	638.09	638.21
3I	65+95.47	-16.00	638.17	638.26
3J	66+05.47	-16.00	638.24	638.30
☉ S. BRG. PIER 38	66+16.18	-16.00	638.32	638.32
☉ PIER 38 SPAN 3	66+17.06	-16.00	638.33	638.33
☉ PIER 38 SPAN 4	66+17.06	-16.00	638.33	638.33
☉ N. BRG. PIER 38	66+17.93	-16.00	638.34	638.34
4A	66+27.93	-16.00	638.40	638.45
4B	66+37.93	-16.00	638.47	638.54
4C	66+47.93	-16.00	638.53	638.62
4D	66+57.93	-16.00	638.59	638.66
4E	66+67.93	-16.00	638.64	638.68
☉ BRG. N. ABUT.	66+77.06	-16.00	638.69	638.69
BK. N. ABUT.	66+79.56	-16.00	638.70	638.70

GIRDER J

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ PIER 35	63+12.36	-8.00	634.74	634.74
☉ N. BRG. PIER 35	63+13.36	-8.00	634.75	634.75
1A	63+23.36	-8.00	634.90	634.93
1B	63+33.36	-8.00	635.05	635.10
1C	63+43.36	-8.00	635.20	635.26
1D	63+53.36	-8.00	635.35	635.41
1E	63+63.36	-8.00	635.50	635.55
1F	63+73.36	-8.00	635.65	635.68
1G	63+83.36	-8.00	635.80	635.81
☉ BRG. & ☉ PIER 36	63+96.36	-8.00	636.00	636.00
2A	64+06.36	-8.00	636.15	636.15
2B	64+16.36	-8.00	636.29	636.31
2C	64+26.36	-8.00	636.44	636.46
2D	64+36.36	-8.00	636.58	636.61
2E	64+46.36	-8.00	636.71	636.75
2F	64+56.36	-8.00	636.85	636.88
2G	64+66.36	-8.00	636.98	637.00
2H	64+76.36	-8.00	637.10	637.11
2I	64+86.36	-8.00	637.22	637.22
2J	64+96.36	-8.00	637.34	637.33
☉ BRG. & ☉ PIER 37	65+04.61	-8.00	637.43	637.43
3A	65+14.61	-8.00	637.55	637.56
3B	65+24.61	-8.00	637.65	637.70
3C	65+34.61	-8.00	637.76	637.84
3D	65+44.61	-8.00	637.86	637.96
3E	65+54.61	-8.00	637.95	638.08
3F	65+64.61	-8.00	638.05	638.18
3G	65+74.61	-8.00	638.13	638.27
3H	65+84.61	-8.00	638.22	638.33
3I	65+94.61	-8.00	638.30	638.39
3J	66+04.61	-8.00	638.38	638.42
☉ S. BRG. PIER 38	66+14.03	-8.00	638.45	638.45
☉ PIER 38 SPAN 3	66+14.90	-8.00	638.45	638.45
☉ PIER 38 SPAN 4	66+15.06	-8.58	638.45	638.45
☉ N. BRG. PIER 38	66+15.93	-8.58	638.45	638.45
4A	66+25.93	-8.58	638.52	638.56
4B	66+35.93	-8.58	638.59	638.66
4C	66+45.93	-8.58	638.65	638.73
4D	66+55.93	-8.58	638.71	638.78
4E	66+65.93	-8.58	638.76	638.80
☉ BRG. N. ABUT.	66+75.06	-8.58	638.81	638.81
BK. N. ABUT.	66+77.56	-8.58	638.82	638.82

GIRDER K

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ PIER 35	63+11.50	0.00	634.84	634.84
☉ N. BRG. PIER 35	63+12.50	0.00	634.86	634.86
1A	63+22.50	0.00	635.01	635.03
1B	63+32.50	0.00	635.16	635.21
1C	63+42.50	0.00	635.31	635.37
1D	63+52.50	0.00	635.46	635.52
1E	63+62.50	0.00	635.61	635.66
1F	63+72.50	0.00	635.76	635.79
1G	63+82.50	0.00	635.91	635.92
☉ BRG. & ☉ PIER 36	63+95.50	0.00	636.10	636.10
2A	64+05.50	0.00	636.25	636.26
2B	64+15.50	0.00	636.40	636.41
2C	64+25.50	0.00	636.54	636.57
2D	64+35.50	0.00	636.69	636.72
2E	64+45.50	0.00	636.82	636.86
2F	64+55.50	0.00	636.96	636.99
2G	64+65.50	0.00	637.08	637.11
2H	64+75.50	0.00	637.21	637.22
2I	64+85.50	0.00	637.33	637.33
2J	64+95.50	0.00	637.45	637.44
☉ BRG. & ☉ PIER 37	65+03.75	0.00	637.54	637.54
3A	65+13.75	0.00	637.66	637.67
3B	65+23.75	0.00	637.76	637.81
3C	65+33.75	0.00	637.87	637.94
3D	65+43.75	0.00	637.97	638.07
3E	65+53.75	0.00	638.06	638.19
3F	65+63.75	0.00	638.16	638.29
3G	65+73.75	0.00	638.25	638.37
3H	65+83.75	0.00	638.33	638.44
3I	65+93.75	0.00	638.41	638.49
3J	66+03.75	0.00	638.49	638.53
☉ S. BRG. PIER 38	66+11.87	0.00	638.55	638.55
☉ PIER 38 SPAN 3	66+12.75	0.00	638.56	638.56
☉ PIER 38 SPAN 4	66+13.06	-1.17	638.54	638.54
☉ N. BRG. PIER 38	66+13.94	-1.17	638.55	638.55
4A	66+23.94	-1.17	638.62	638.66
4B	66+33.94	-1.17	638.69	638.76
4C	66+43.94	-1.17	638.75	638.83
4D	66+53.94	-1.17	638.81	638.88
4E	66+63.94	-1.17	638.86	638.90
☉ BRG. N. ABUT.	66+73.06	-1.17	638.91	638.91
BK. N. ABUT.	66+75.56	-1.17	638.92	638.92

(Sheet 3 of 5)



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**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**TOP OF SLAB ELEVATIONS
STRUCTURE NO. 016-2454**

SHEET NO. SB9 OF SB34 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372	2013-037B-R	COOK	787	273
ILLINOIS FED. AID PROJECT			CONTRACT NO. 60W75	

NB IL-171 & P.G.L.

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ PIER 35	63+11.50	0.00	634.84	634.84
☉ N. BRG. PIER 35	63+12.50	0.00	634.86	634.86
1A	63+22.50	0.00	635.01	635.03
1B	63+32.50	0.00	635.16	635.21
1C	63+42.50	0.00	635.31	635.37
1D	63+52.50	0.00	635.46	635.52
1E	63+62.50	0.00	635.61	635.66
1F	63+72.50	0.00	635.76	635.79
1G	63+82.50	0.00	635.91	635.92
☉ BRG. & ☉ PIER 36	63+95.50	0.00	636.10	636.10
2A	64+05.50	0.00	636.25	636.26
2B	64+15.50	0.00	636.40	636.41
2C	64+25.50	0.00	636.54	636.57
2D	64+35.50	0.00	636.69	636.72
2E	64+45.50	0.00	636.82	636.86
2F	64+55.50	0.00	636.96	636.99
2G	64+65.50	0.00	637.08	637.11
2H	64+75.50	0.00	637.21	637.22
2I	64+85.50	0.00	637.33	637.33
2J	64+95.50	0.00	637.45	637.44
☉ BRG. & ☉ PIER 37	65+03.75	0.00	637.54	637.54
3A	65+13.75	0.00	637.66	637.67
3B	65+23.75	0.00	637.76	637.81
3C	65+33.75	0.00	637.87	637.94
3D	65+43.75	0.00	637.97	638.07
3E	65+53.75	0.00	638.06	638.19
3F	65+63.75	0.00	638.16	638.29
3G	65+73.75	0.00	638.25	638.37
3H	65+83.75	0.00	638.33	638.44
3I	65+93.75	0.00	638.41	638.49
3J	66+03.75	0.00	638.49	638.53
☉ S. BRG. PIER 38	66+11.87	0.00	638.55	638.55
☉ PIER 38 SPAN 3	66+12.75	0.00	638.56	638.56
☉ PIER 38 SPAN 4	66+12.75	0.00	638.56	638.56
☉ N. BRG. PIER 38	66+13.63	0.00	638.56	638.56
4A	66+23.63	0.00	638.64	638.68
4B	66+33.63	0.00	638.70	638.77
4C	66+43.63	0.00	638.76	638.84
4D	66+53.63	0.00	638.82	638.89
4E	66+63.63	0.00	638.88	638.92
☉ BRG. N. ABUT.	66+72.75	0.00	638.93	638.93
BK. N. ABUT.	66+75.25	0.00	638.94	638.94

GIRDER L

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ PIER 35	63+10.64	8.00	634.71	634.71
☉ N. BRG. PIER 35	63+11.64	8.00	634.72	634.72
1A	63+21.64	8.00	634.87	634.90
1B	63+31.64	8.00	635.02	635.07
1C	63+41.64	8.00	635.17	635.23
1D	63+51.64	8.00	635.32	635.38
1E	63+61.64	8.00	635.47	635.52
1F	63+71.64	8.00	635.62	635.66
1G	63+81.64	8.00	635.77	635.79
☉ BRG. & ☉ PIER 36	63+94.64	8.00	635.97	635.97
2A	64+04.64	8.00	636.12	636.12
2B	64+14.64	8.00	636.27	636.28
2C	64+24.64	8.00	636.41	636.44
2D	64+34.64	8.00	636.55	636.59
2E	64+44.64	8.00	636.69	636.73
2F	64+54.64	8.00	636.82	636.86
2G	64+64.64	8.00	636.95	636.98
2H	64+74.64	8.00	637.08	637.09
2I	64+84.64	8.00	637.20	637.20
2J	64+94.64	8.00	637.32	637.32
☉ BRG. & ☉ PIER 37	65+02.89	8.00	637.42	637.42
3A	65+12.89	8.00	637.53	637.54
3B	65+22.89	8.00	637.63	637.68
3C	65+32.89	8.00	637.74	637.81
3D	65+42.89	8.00	637.84	637.94
3E	65+52.89	8.00	637.94	638.05
3F	65+62.89	8.00	638.03	638.15
3G	65+72.89	8.00	638.12	638.24
3H	65+82.89	8.00	638.20	638.30
3I	65+92.89	8.00	638.29	638.36
3J	66+02.89	8.00	638.36	638.39
☉ S. BRG. PIER 38	66+09.72	8.00	638.42	638.42
☉ PIER 38 SPAN 3	66+10.60	8.00	638.42	638.42
☉ PIER 38 SPAN 4	66+11.07	6.25	638.45	638.45
☉ N. BRG. PIER 38	66+11.94	6.25	638.46	638.46
4A	66+21.94	6.25	638.53	638.57
4B	66+31.94	6.25	638.60	638.67
4C	66+41.94	6.25	638.66	638.74
4D	66+51.94	6.25	638.72	638.79
4E	66+61.94	6.25	638.78	638.81
☉ BRG. N. ABUT.	66+71.07	6.25	638.83	638.83
BK. N. ABUT.	66+73.57	6.25	638.84	638.84

GIRDER M

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ PIER 35	63+09.78	16.00	634.56	634.56
☉ N. BRG. PIER 35	63+10.78	16.00	634.57	634.57
1A	63+20.78	16.00	634.72	634.75
1B	63+30.78	16.00	634.87	634.92
1C	63+40.78	16.00	635.02	635.08
1D	63+50.78	16.00	635.17	635.23
1E	63+60.78	16.00	635.32	635.37
1F	63+70.78	16.00	635.47	635.50
1G	63+80.78	16.00	635.62	635.63
☉ BRG. & ☉ PIER 36	63+93.78	16.00	635.82	635.82
2A	64+03.78	16.00	635.97	635.97
2B	64+13.78	16.00	636.12	636.13
2C	64+23.78	16.00	636.26	636.29
2D	64+33.78	16.00	636.40	636.44
2E	64+43.78	16.00	636.54	636.58
2F	64+53.78	16.00	636.67	636.71
2G	64+63.78	16.00	636.80	636.83
2H	64+73.78	16.00	636.93	636.94
2I	64+83.78	16.00	637.05	637.05
2J	64+93.78	16.00	637.17	637.17
☉ BRG. & ☉ PIER 37	65+02.03	16.00	637.27	637.27
3A	65+12.03	16.00	637.38	637.39
3B	65+22.03	16.00	637.49	637.53
3C	65+32.03	16.00	637.59	637.66
3D	65+42.03	16.00	637.69	637.78
3E	65+52.03	16.00	637.79	637.90
3F	65+62.03	16.00	637.88	638.00
3G	65+72.03	16.00	637.97	638.08
3H	65+82.03	16.00	638.06	638.15
3I	65+92.03	16.00	638.14	638.20
3J	66+02.03	16.00	638.22	638.24
☉ S. BRG. PIER 38	66+07.57	16.00	638.26	638.26
☉ PIER 38 SPAN 3	66+08.44	16.00	638.27	638.27
☉ PIER 38 SPAN 4	66+09.07	13.67	638.32	638.32
☉ N. BRG. PIER 38	66+09.95	13.67	638.32	638.32
4A	66+19.95	13.67	638.40	638.44
4B	66+29.95	13.67	638.46	638.53
4C	66+39.95	13.67	638.53	638.61
4D	66+49.95	13.67	638.59	638.66
4E	66+59.95	13.67	638.65	638.68
☉ BRG. N. ABUT.	66+69.07	13.67	638.70	638.70
BK. N. ABUT.	66+71.57	13.67	638.71	638.71

(Sheet 4 of 5)



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**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**TOP OF SLAB ELEVATIONS
STRUCTURE NO. 016-2454**

SHEET NO. SB10 OF SB34 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372	2013-037B-R	COOK	787	274
ILLINOIS FED. AID PROJECT				

GIRDER N

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ PIER 35	63+08.92	24.00	634.38	634.38
☉ N. BRG. PIER 35	63+09.92	24.00	634.40	634.40
1A	63+19.92	24.00	634.55	634.58
1B	63+29.92	24.00	634.70	634.75
1C	63+39.92	24.00	634.85	634.91
1D	63+49.92	24.00	635.00	635.06
1E	63+59.92	24.00	635.15	635.20
1F	63+69.92	24.00	635.30	635.33
1G	63+79.92	24.00	635.45	635.46
☉ BRG. & ☉ PIER 36	63+92.92	24.00	635.64	635.64
2A	64+02.92	24.00	635.79	635.80
2B	64+12.92	24.00	635.94	635.95
2C	64+22.92	24.00	636.09	636.11
2D	64+32.92	24.00	636.23	636.26
2E	64+42.92	24.00	636.37	636.40
2F	64+52.92	24.00	636.50	636.53
2G	64+62.92	24.00	636.63	636.66
2H	64+72.92	24.00	636.76	636.77
2I	64+82.92	24.00	636.88	636.88
2J	64+92.92	24.00	637.00	637.00
☉ BRG. & ☉ PIER 37	65+01.17	24.00	637.10	637.10
3A	65+11.17	24.00	637.21	637.22
3B	65+21.17	24.00	637.32	637.35
3C	65+31.17	24.00	637.42	637.48
3D	65+41.17	24.00	637.52	637.61
3E	65+51.17	24.00	637.62	637.72
3F	65+61.17	24.00	637.71	637.82
3G	65+71.17	24.00	637.80	637.90
3H	65+81.17	24.00	637.89	637.97
3I	65+91.17	24.00	637.97	638.02
☉ S. BRG. PIER 38	66+05.42	24.00	638.08	638.08
☉ PIER 38 SPAN 3	66+06.29	24.00	638.09	638.09
☉ PIER 38 SPAN 4	66+07.08	21.08	638.15	638.15
☉ N. BRG. PIER 38	66+07.95	21.08	638.16	638.16
4A	66+17.95	21.08	638.23	638.27
4B	66+27.95	21.08	638.30	638.37
4C	66+37.95	21.08	638.37	638.45
4D	66+47.95	21.08	638.43	638.50
4E	66+57.95	21.08	638.49	638.52
☉ BRG. N. ABUT.	66+67.08	21.08	638.54	638.54
BK. N. ABUT.	66+69.58	21.08	638.55	638.55

GIRDER O

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ PIER 35	63+07.97	32.81	634.19	634.19
☉ N. BRG. PIER 35	63+08.98	32.79	634.21	634.21
1A	63+19.00	32.59	634.36	634.39
1B	63+29.02	32.39	634.52	634.56
1C	63+39.04	32.19	634.67	634.73
1D	63+49.06	31.99	634.83	634.88
1E	63+59.08	31.78	634.98	635.03
1F	63+69.11	31.58	635.13	635.16
1G	63+79.13	31.38	635.29	635.30
☉ BRG. & ☉ PIER 36	63+92.16	31.12	635.49	635.49
2A	64+02.18	30.92	635.64	635.65
2B	64+12.20	30.72	635.80	635.81
2C	64+22.22	30.52	635.95	635.97
2D	64+32.24	30.32	636.09	636.12
2E	64+42.26	30.12	636.24	636.27
2F	64+52.28	29.92	636.37	636.40
2G	64+62.31	29.72	636.51	636.53
2H	64+72.33	29.52	636.64	636.65
2I	64+82.35	29.35	636.77	636.77
2J	64+92.35	29.35	636.89	636.88
☉ BRG. & ☉ PIER 37	65+00.60	29.35	636.98	636.98
3A	65+10.60	29.35	637.09	637.11
3B	65+20.60	29.35	637.20	637.23
3C	65+30.57	29.61	637.30	637.35
3D	65+40.54	29.87	637.40	637.47
3E	65+50.51	30.14	637.49	637.57
3F	65+60.48	30.40	637.58	637.66
3G	65+70.46	30.66	637.66	637.74
3H	65+80.43	30.92	637.75	637.81
3I	65+90.40	31.18	637.82	637.86
☉ S. BRG. PIER 38	66+03.39	31.51	637.92	637.92
☉ PIER 38 SPAN 3	66+04.26	31.54	637.92	637.92
☉ PIER 38 SPAN 4	66+05.08	28.50	637.99	637.99
☉ N. BRG. PIER 38	66+05.96	28.50	638.00	638.00
4A	66+15.96	28.50	638.07	638.11
4B	66+25.96	28.50	638.14	638.21
4C	66+35.96	28.50	638.21	638.28
4D	66+45.96	28.50	638.27	638.33
4E	66+55.96	28.50	638.33	638.36
☉ BRG. N. ABUT.	66+65.08	28.50	638.38	638.38
BK. N. ABUT.	66+67.58	28.50	638.39	638.39

GIRDER P

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
☉ PIER 38 SPAN 4	66+04.02	32.43	637.90	637.90
☉ N. BRG. PIER 38	66+04.88	32.49	637.91	637.91
4A	66+14.68	33.25	637.97	638.00
4B	66+24.47	34.01	638.02	638.07
4C	66+34.27	34.77	638.07	638.12
4D	66+44.06	35.53	638.12	638.16
4E	66+53.86	36.29	638.16	638.18
☉ BRG. N. ABUT.	66+62.80	36.98	638.20	638.20
BK. N. ABUT.	66+65.25	37.17	638.21	638.21

(Sheet 5 of 5)



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**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**TOP OF SLAB ELEVATIONS
STRUCTURE NO. 016-2454**

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372	2013-037B-R	COOK	787	275
CONTRACT NO. 60W75			ILLINOIS FED. AID PROJECT	

SHEET NO. SB11 OF SB34 SHEETS

WEST EDGE OF SHOULDER

Location	Station along NB IL-171	Offset from NB IL-171 & P.G.L.	Theoretical Grade Elevations
S. End of Appr. Pav't.	66+79.58	-18.00	638.66
A1	66+89.58	-18.00	638.71
A2	66+99.58	-18.00	638.75
N. End of Appr. Pav't.	67+09.58	-18.00	638.79

CROSS SLOPE BREAK 1

Location	Station along NB IL-171	Offset from NB IL-171 & P.G.L.	Theoretical Grade Elevations
S. End of Appr. Pav't.	66+77.96	-12.00	638.77
A1	66+87.96	-12.00	638.82
A2	66+97.96	-12.00	638.86
N. End of Appr. Pav't.	67+07.96	-12.00	638.90

NB IL-171 & P.G.L

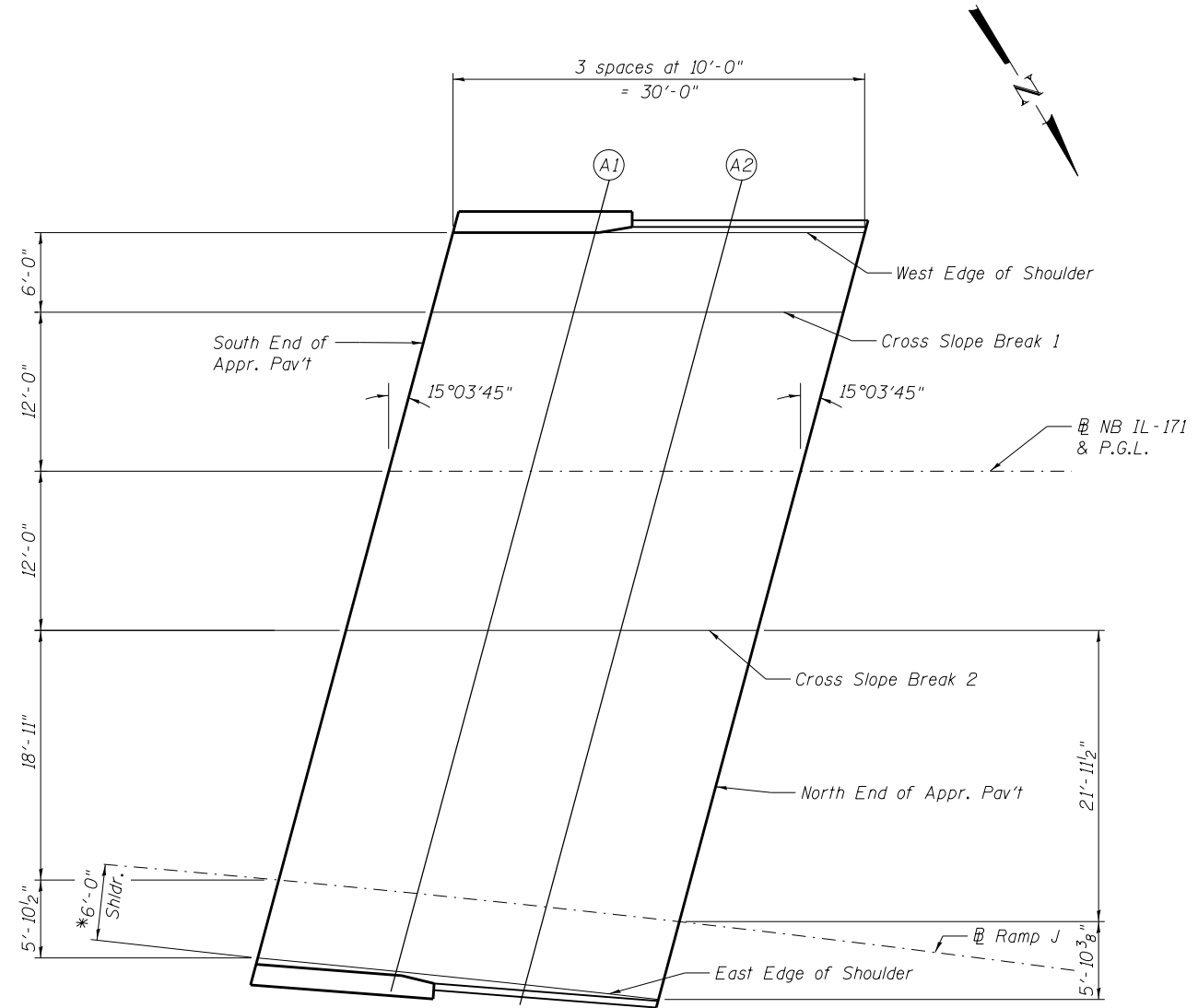
Location	Station along NB IL-171	Offset from NB IL-171 & P.G.L.	Theoretical Grade Elevations
S. End of Appr. Pav't.	66+74.73	0.00	638.94
A1	66+84.73	0.00	638.99
A2	66+94.73	0.00	639.03
N. End of Appr. Pav't.	67+04.73	0.00	639.07

CROSS SLOPE BREAK 2

Location	Station along NB IL-171	Offset from NB IL-171 & P.G.L.	Theoretical Grade Elevations
S. End of Appr. Pav't.	66+71.50	12.00	638.74
A1	66+81.50	12.00	638.79
A2	66+91.50	12.00	638.84
N. End of Appr. Pav't.	67+01.50	12.00	638.88

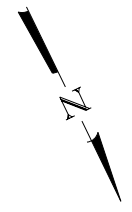
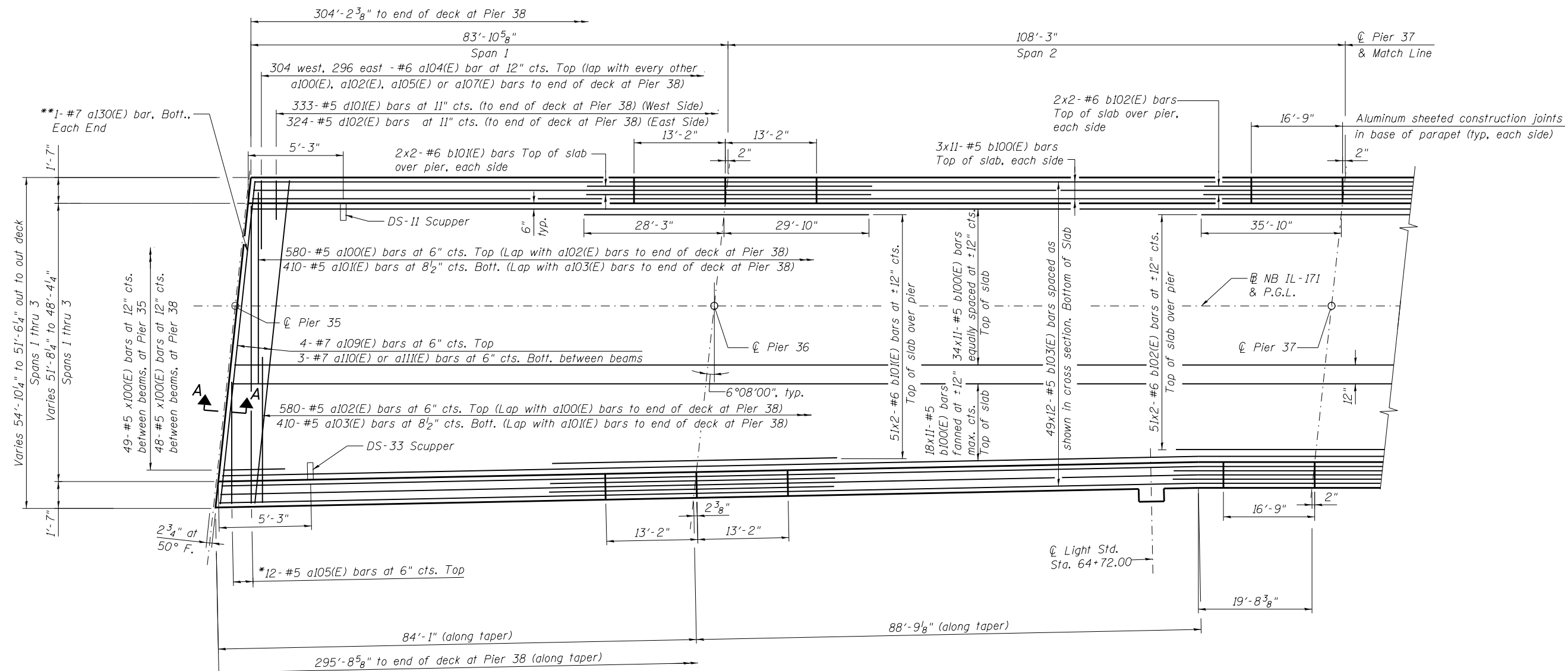
EAST EDGE OF SHOULDER

Location	Station along NB IL-171	Offset from NB IL-171 & P.G.L.	Theoretical Grade Elevations
S. End of Appr. Pav't.	66+64.83	36.79	638.21
A1	66+74.58	37.74	638.24
A2	66+84.31	38.74	638.27
N. End of Appr. Pav't.	66+94.02	39.81	638.29



* Dimension measured perpendicular to NB Ramp J

PLAN

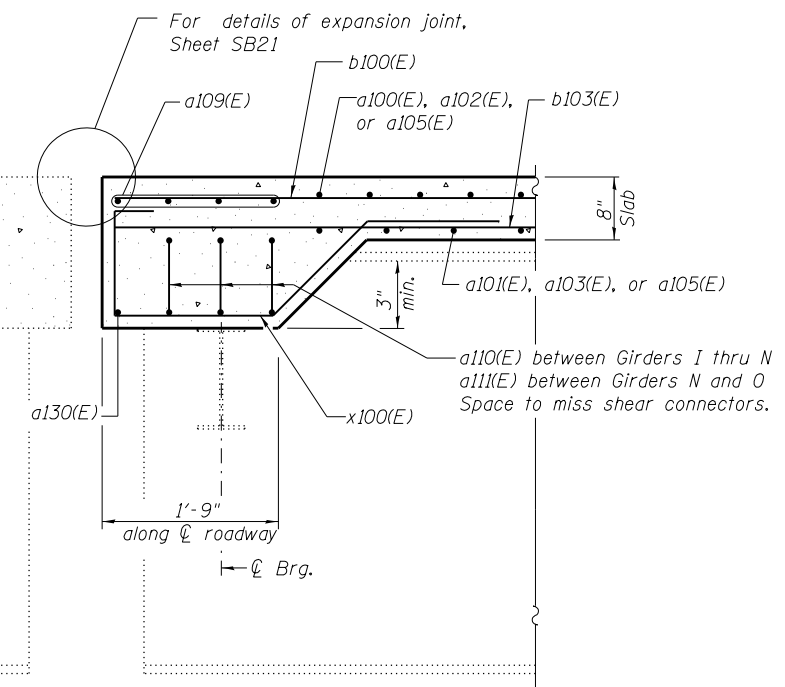


PARTIAL PLAN - SPANS 1 & 2

- * Order full length. Cut as shown and use remainder of bars in bottom of deck at same spacing as top bars. See bar cutting diagram on Sheet SB17.
- ** Cut to fit skew.

Notes:
 See Sheets SB16 & SB17 for superstructure details and Bill of Material.
 Bars indicated thus 20x3- #5 etc. indicates 20 lines of bars with 3 lengths per line.
 Dimensions are based on a Rolled Rail Strip Seal Joint. If the Contractor elects to use the Welded Rail Strip Seal Joint, deck dimensions may require adjustments to satisfy the details on Sheet SB21.

MINIMUM BAR LAP
 #5 bar = 2'-7"
 #6 bar = 3'-1"



SECTION A-A

(Sheet 1 of 3)



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**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**SUPERSTRUCTURE
 STRUCTURE NO. 016-2454**

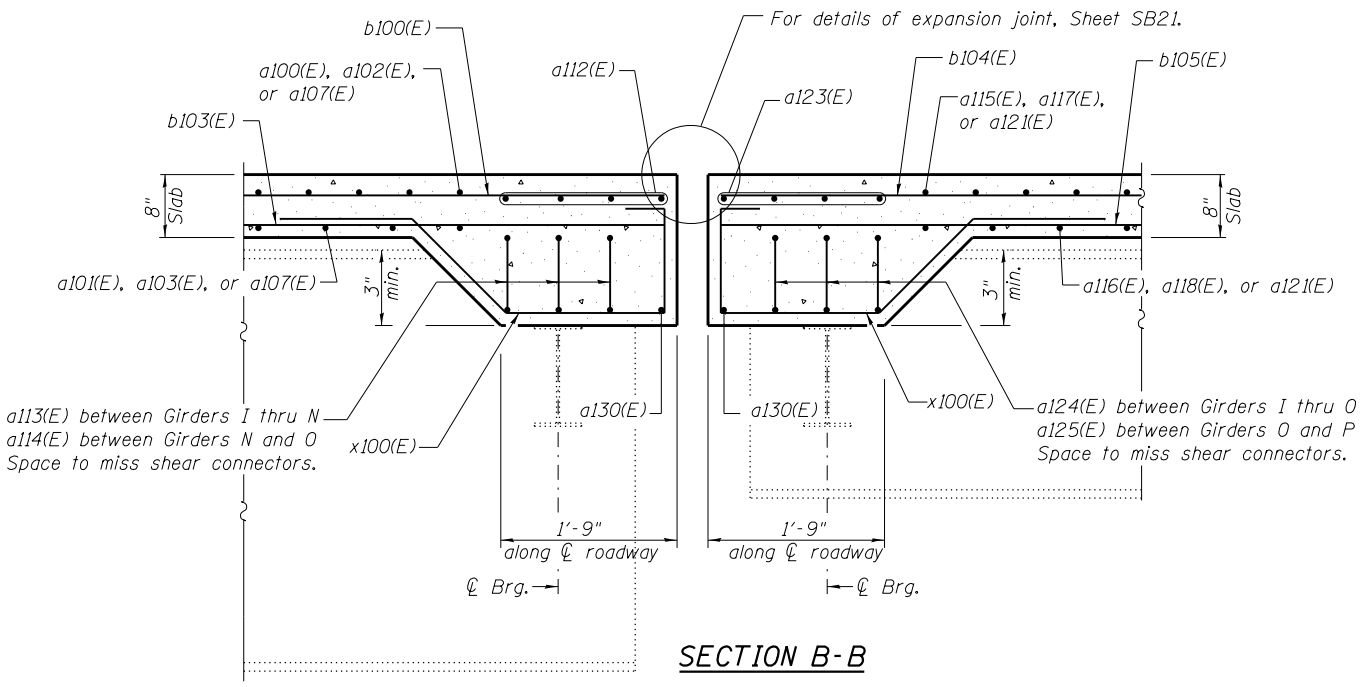
SHEET NO. SB13 OF SB34 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372	2013-037B-R	COOK	787	277
CONTRACT NO. 60W75				
ILLINOIS FED. AID PROJECT				

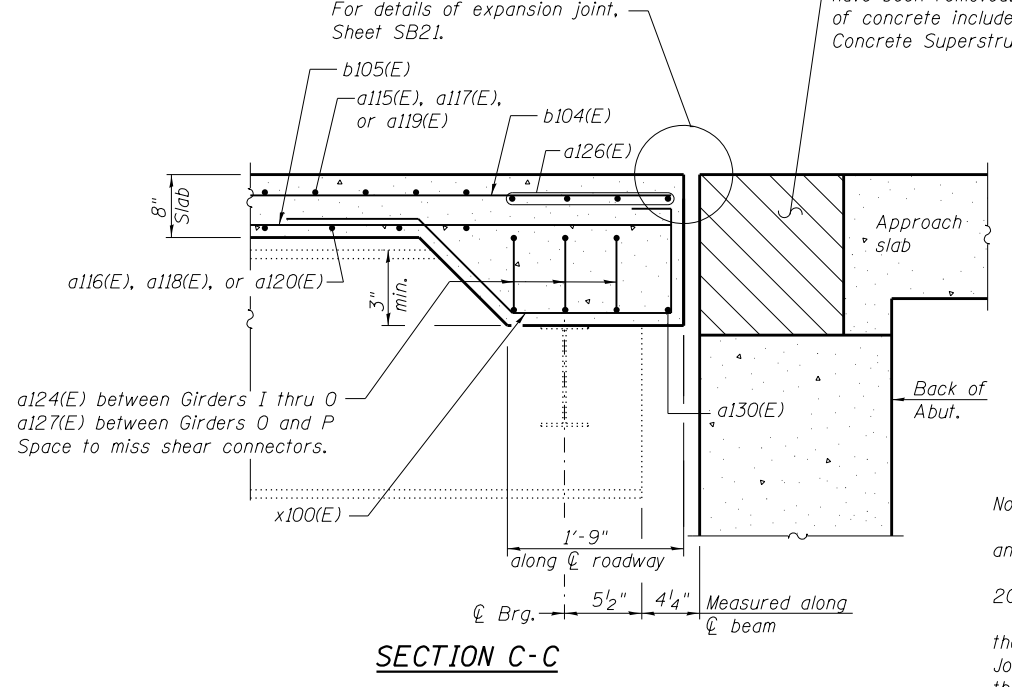


PARTIAL PLAN - SPANS 3 & 4

* Order full length. Cut as shown and use remainder of bars in bottom of deck at same spacing as top bars. See bar cutting diagram on Sheet SB17.
 ** Cut to fit skew.



SECTION B-B



SECTION C-C

Hatched area to be poured after superstructure forms have been removed. Quantity of concrete included with Concrete Superstructure.

MINIMUM BAR LAP
 #5 bar = 2'-7"
 #6 bar = 3'-1"

Notes:
 See Sheets SB16 & SB17 for superstructure details and Bill of Material.
 Bars indicated thus 20x3-#5 etc. indicates 20 lines of bars with 3 lengths per line.
 Dimensions are based on a Rolled Rail Strip Seal Joint. If the Contractor elects to use the Welded Rail Strip Seal Joint, deck dimensions may require adjustments to satisfy the details on Sheet SB21.

(Sheet 2 of 3)



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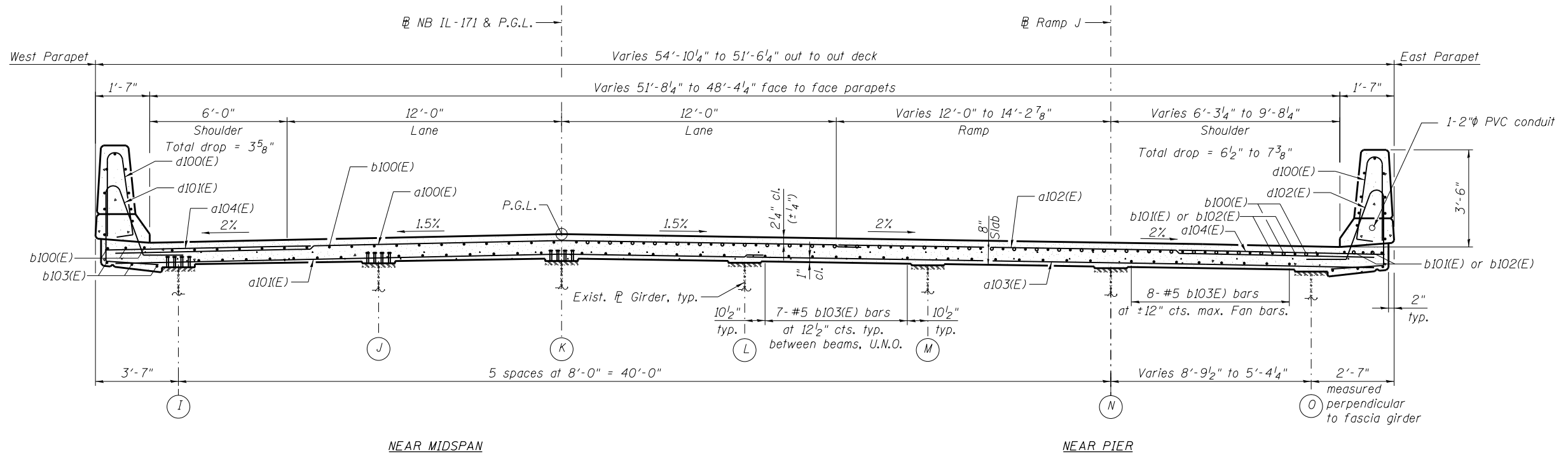
DESIGNED - RGB
 CHECKED - JRS
 DRAWN - RMH
 CHECKED - JRS
 REVISED -
 REVISED -
 REVISED -
 REVISED -

**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**SUPERSTRUCTURE
 STRUCTURE NO. 016-2454**

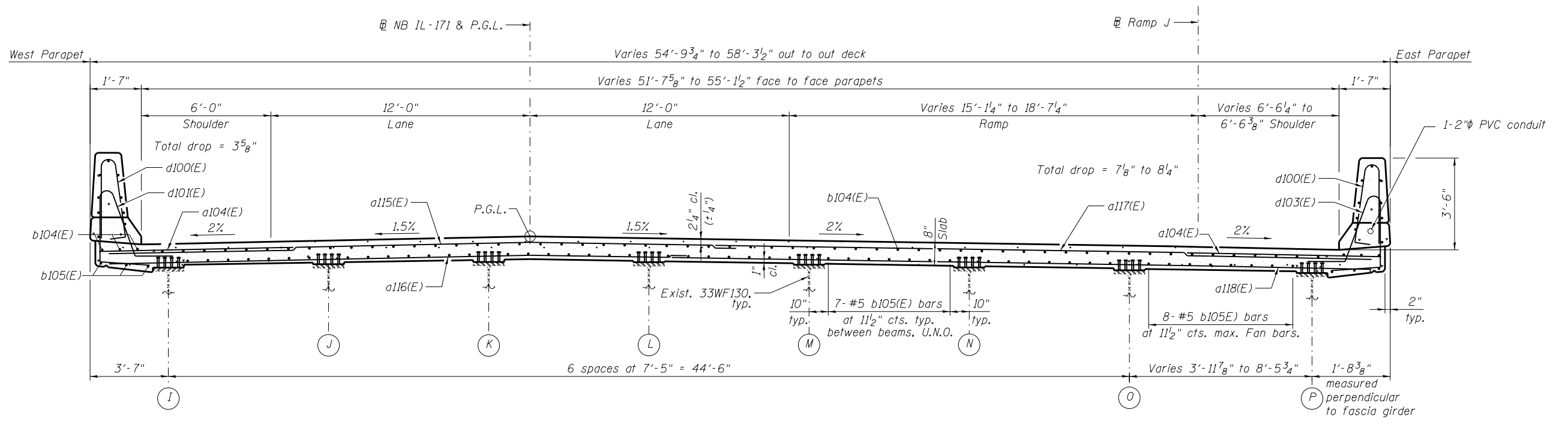
SHEET NO. SB14 OF SB34 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372	2013-037B-R	COOK	787	278
ILLINOIS FED. AID PROJECT			CONTRACT NO. 60W75	



CROSS SECTION
(Spans 1 thru 3, Looking Upstation)

MINIMUM BAR LAP
#5 bar = 2'-7"

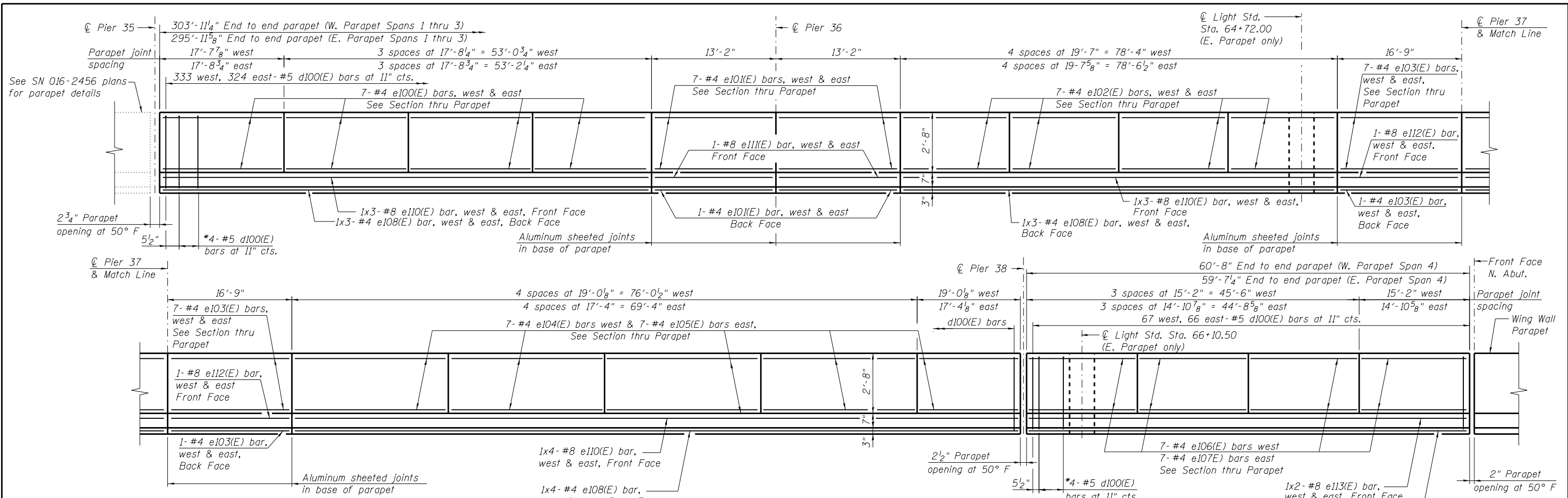


CROSS SECTION
(Span 4, Looking Upstation)

Note:
All dimensions are measured perpendicular to NB IL-171 unless noted otherwise.

(Sheet 3 of 3)

<p>LIN ENGINEERING, LTD. Consulting Engineers Westmont, Illinois</p>	USER NAME = Lin_41 FILE NAME = ...18162454.68W75_815_decksection.dgn PLOT SCALE = PLOT DATE = 6/8/2015 3:39:11 PM	DESIGNED - RGB CHECKED - JRS DRAWN - RMH CHECKED - JRS	REVISED - REVISED - REVISED - REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	SUPERSTRUCTURE STRUCTURE NO. 016-2454	F.A.P. RT. = 372 SECTION = 2013-037B-R COUNTY = COOK TOTAL SHEETS = 787 SHEET NO. = 279	ILLINOIS FED. AID PROJECT	
	SHEET NO. SB15 OF SB34 SHEETS					CONTRACT NO. 60W75		



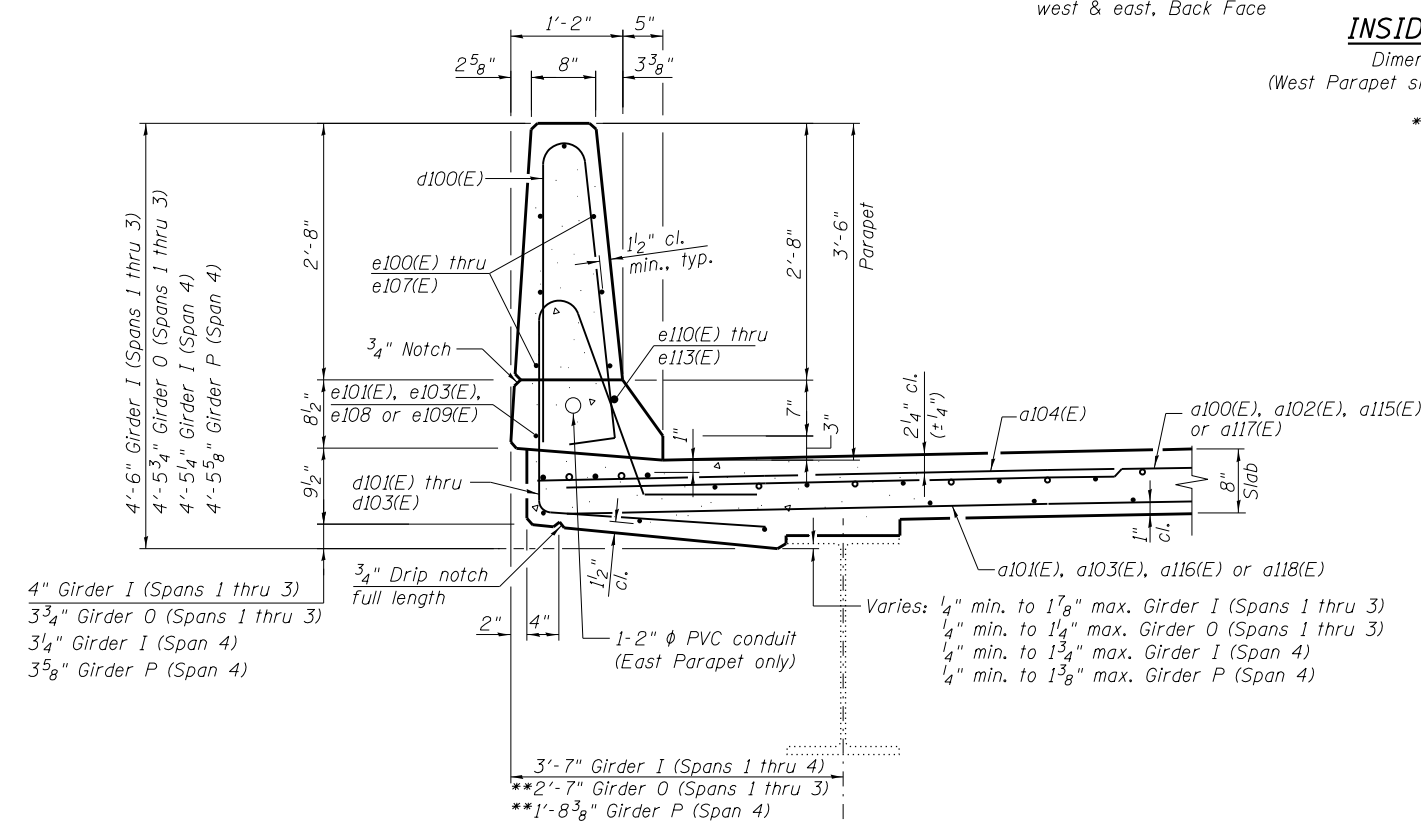
INSIDE ELEVATION OF PARAPET

Dimensions measured along toe of parapet.
 (West Parapet shown, East Parapet opposite hand and as noted)

*Typical at parapet ends and each
 side of aluminum sheeted joints.

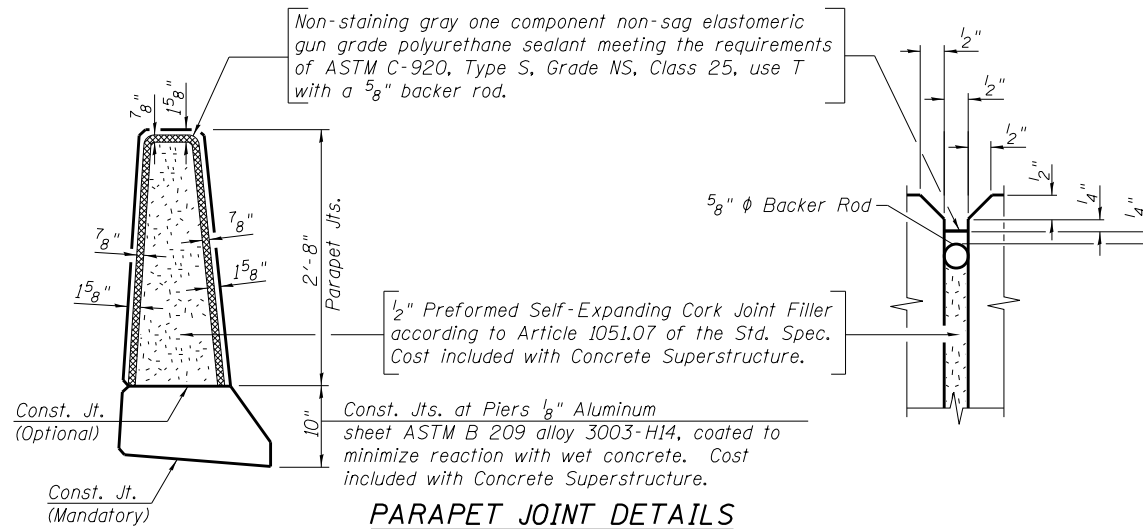
MINIMUM BAR LAP

(Parapet)
 #4 bar = 2'-0"
 #8 bar = 5'-2"



SECTION THRU PARAPET

** Measured perpendicular to Fascia Girder

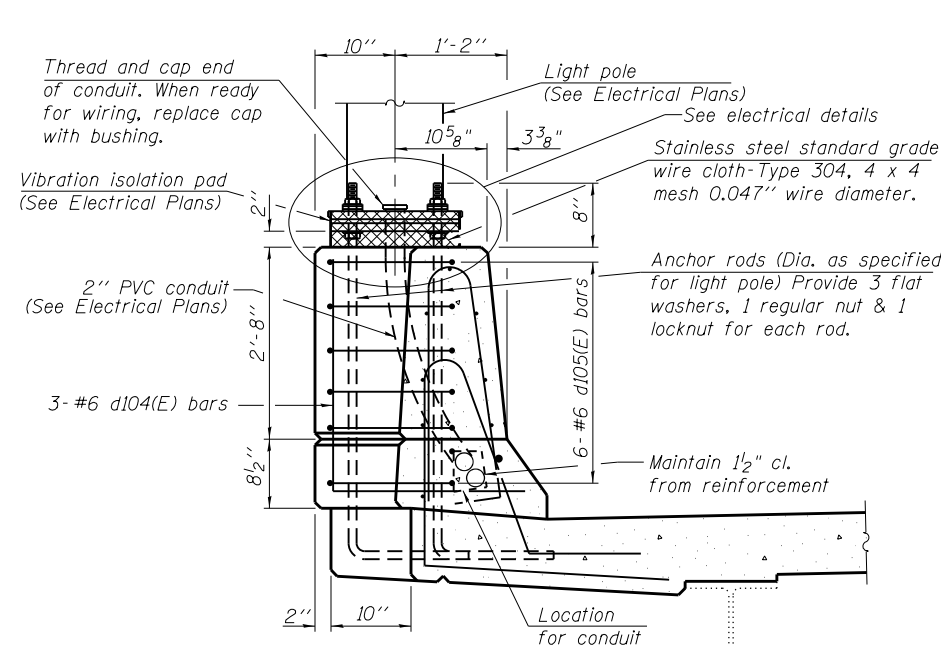


PARAPET JOINT DETAILS

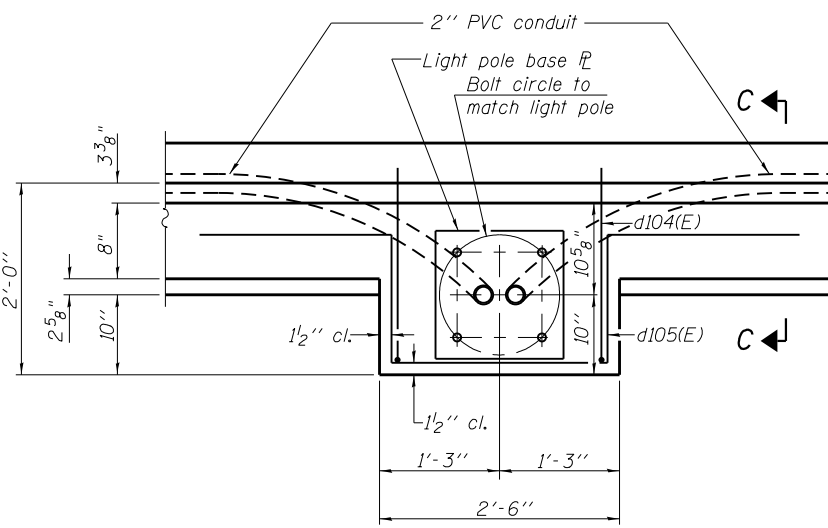
Note:
 Bars indicated thus 1x2- #8 etc.
 indicates 1 line of bars with 2 lengths per line.

(Sheet 1 of 2)

<p>LIN ENGINEERING, LTD. Consulting Engineers Westmont, Illinois</p>	USER NAME = Lin_41	DESIGNED - RGB	REVISED -	<p align="center">STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION</p>	<p align="center">SUPERSTRUCTURE DETAILS STRUCTURE NO. 016-2454</p>	F.A.P. RTE. = 372	SECTION = 2013-037B-R	COUNTY = COOK	TOTAL SHEETS = 787	SHEET NO. = 280
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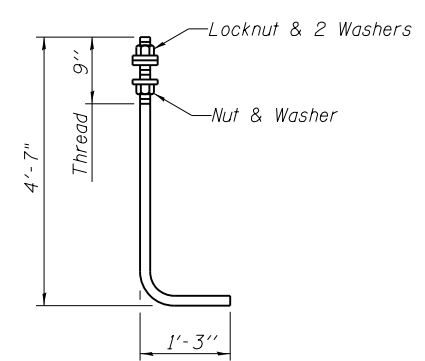


SECTION C-C



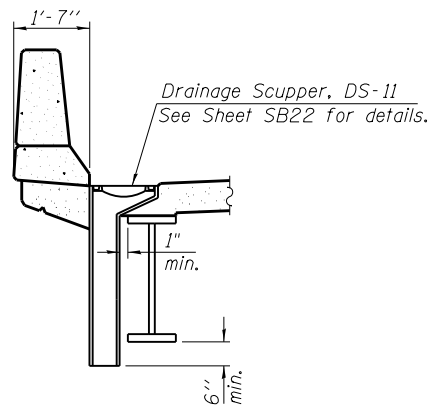
PLAN
LIGHT POLE FOUNDATION DETAILS

Cost of anchor rods is included with "Concrete Superstructure".

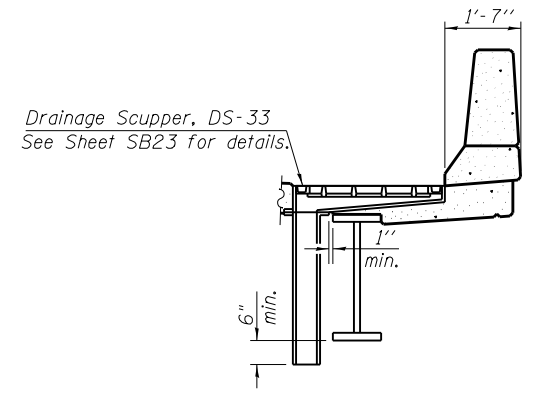


ANCHOR ROD

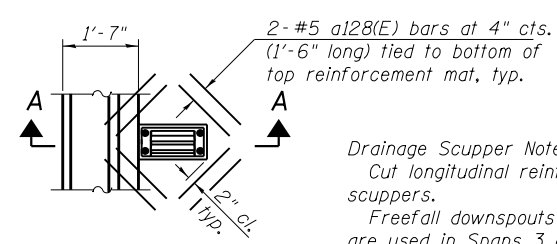
Diameter as specified for light poles. (ASTM F 1554 Grade 105) Full length hot dipped galvanized.



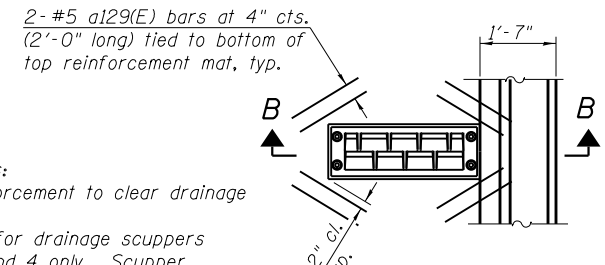
SECTION A-A
Girder I (Span 1 & 4)



SECTION B-B
Girder O (Span 1 & 3)
Girder P (Span 4)



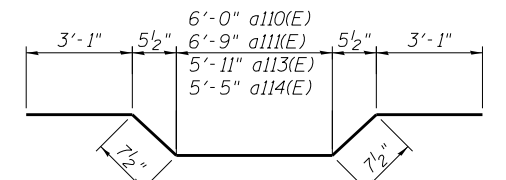
PLAN



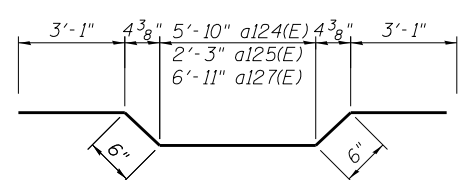
PLAN

Drainage Scupper Notes:
Cut longitudinal reinforcement to clear drainage scuppers.
Freefall downspouts for drainage scuppers are used in Spans 3 and 4 only. Scupper discharge in Span 1 is collected by drainage system. See Sheet SB24.

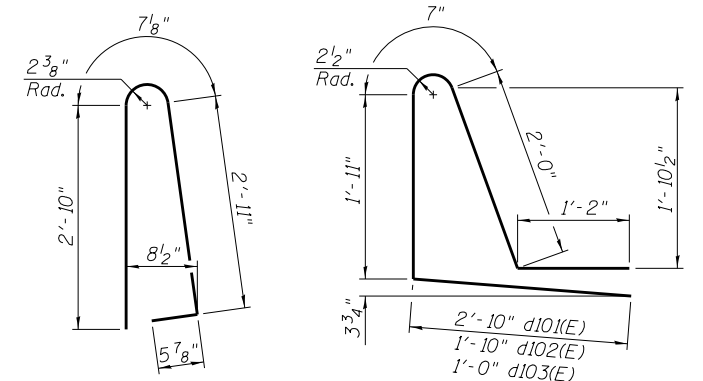
DRAINAGE SCUPPER REINFORCEMENT DETAILS



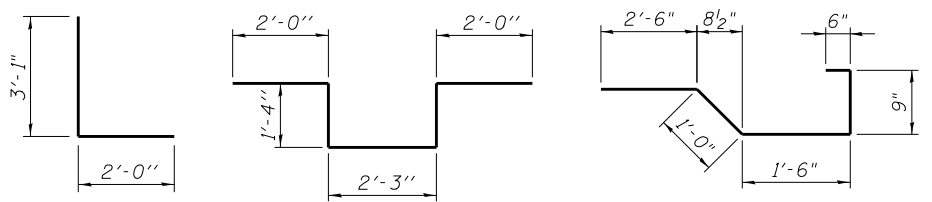
BARS a110(E), a111(E), a113(E), a114(E)



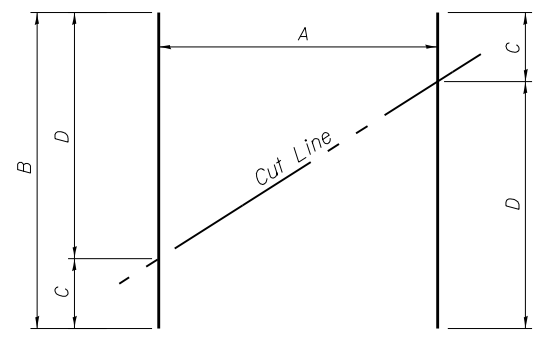
BARS a124(E), a125(E), a127(E)



BAR d100(E) **BARS d101(E), d102(E), d103(E)**



BAR d104(E) **BAR d105(E)** **BARS x100(E)**



CUTTING DIAGRAM

Order full length. Cut as shown and use remainder of bars in bottom of deck at same spacing as top bars.

	A	B	C	D
12- #5 a105(E) bars	57'-3"	3'-1"	54'-2"	
28- #5 a107(E) bars	55'-10"	2'-10"	53'-0"	
26- #5 a121(E) bars	56'-11"	2'-9"	54'-2"	

SUPERSTRUCTURE BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a100(E)	580	#5	32'-3"	—
a101(E)	410	#5	28'-3"	—
a102(E)	580	#5	24'-7"	—
a103(E)	410	#5	28'-7"	—
a104(E)	711	#6	6'-6"	—
a105(E)	12	#5	57'-3"	—
a107(E)	28	#5	55'-10"	—
a109(E)	4	#7	54'-11"	—
a110(E)	15	#7	13'-5"	—
a111(E)	3	#7	14'-2"	—
a112(E)	4	#7	55'-3"	—
a113(E)	15	#7	13'-4"	—
a114(E)	3	#7	12'-10"	—
a115(E)	84	#5	30'-2"	—
a116(E)	64	#5	26'-6"	—
a117(E)	84	#5	30'-1"	—
a118(E)	64	#5	33'-9"	—
a119(E)	29	#5	57'-7"	—
a120(E)	23	#5	57'-7"	—
a121(E)	26	#5	56'-11"	—
a123(E)	4	#7	55'-3"	—
a124(E)	36	#7	13'-0"	—
a125(E)	3	#7	9'-5"	—
a126(E)	4	#7	60'-0"	—
a127(E)	3	#7	14'-1"	—
a128(E)	16	#5	1'-6"	—
a129(E)	24	#5	2'-0"	—
a130(E)	4	#7	60'-0"	—
b100(E)	638	#5	30'-0"	—
b101(E)	110	#6	30'-7"	—
b102(E)	110	#6	34'-11"	—
b103(E)	588	#5	27'-9"	—
b104(E)	124	#5	31'-6"	—
b105(E)	168	#5	21'-11"	—
d100(E)	918	#5	6'-10"	—
d101(E)	400	#5	8'-6"	—
d102(E)	324	#5	7'-6"	—
d103(E)	66	#5	6'-8"	—
d104(E)	6	#6	5'-1"	—
d105(E)	12	#6	8'-11"	—
e100(E)	56	#4	17'-5"	—
e101(E)	32	#4	12'-11"	—
e102(E)	56	#4	19'-4"	—
e103(E)	32	#4	16'-6"	—
e104(E)	35	#4	18'-9"	—
e105(E)	35	#4	17'-1"	—
e106(E)	28	#4	14'-11"	—
e107(E)	28	#4	14'-7"	—
e108(E)	20	#4	27'-5"	—
e109(E)	6	#4	21'-6"	—
e110(E)	20	#8	29'-7"	—
e111(E)	4	#8	12'-11"	—
e112(E)	4	#8	16'-6"	—
e113(E)	4	#8	32'-10"	—
x100(E)	206	#5	6'-3"	—
Reinforcement Bars, Epoxy Coated	Pound	162,490		
Concrete Superstructure	Cu. Yd.	608.6		
Bridge Deck Grooving	Sq. Yd.	1,932		
Protective Coat	Sq. Yd.	2,359		

Notes:
For conduit and other electrical components see Electrical Plans.
The conduit shall be PVC pipe, Sch. 40.

(Sheet 2 of 2)

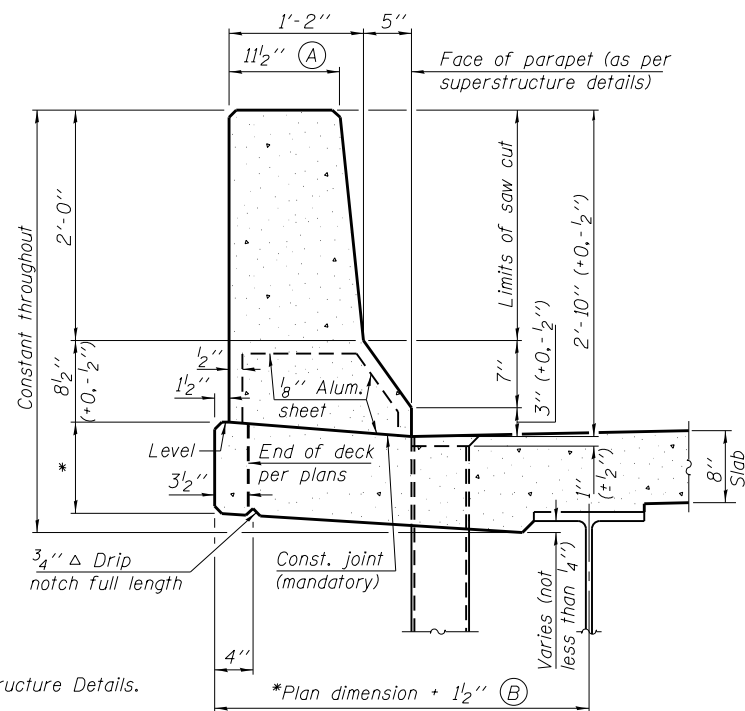


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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SUPERSTRUCTURE DETAILS
STRUCTURE NO. 016-2454
SHEET NO. SB17 OF SB34 SHEETS

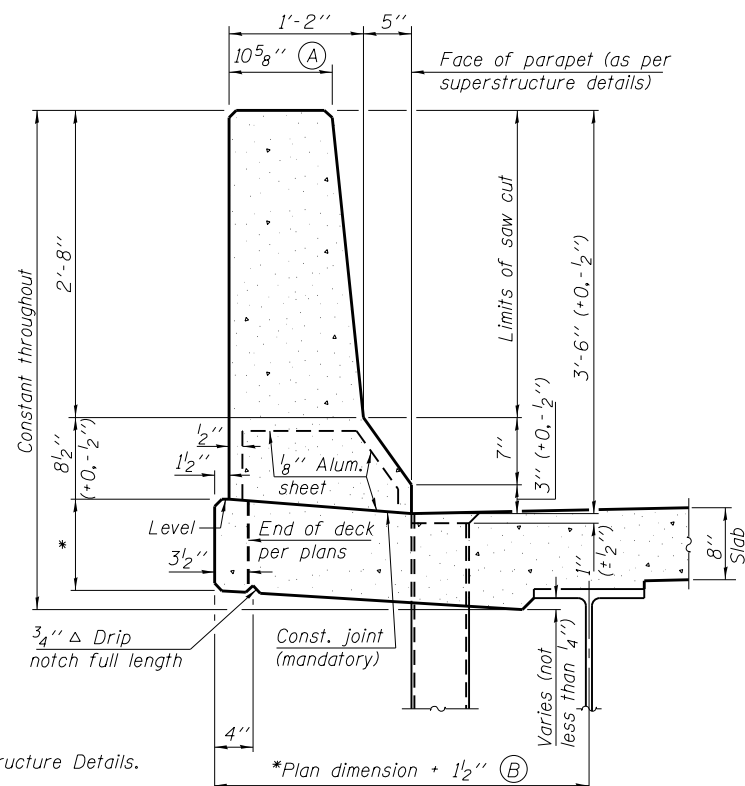
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372	2013-037B-R	COOK	787	281
			CONTRACT NO. 60W75	
ILLINOIS FED. AID PROJECT				



34" F SHAPE PARAPET SECTION

(Showing dimensions)

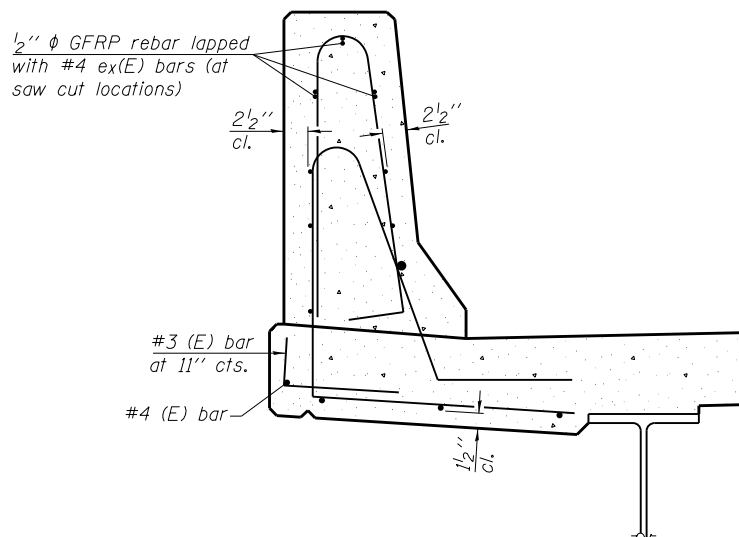
*See Superstructure Details.



42" F SHAPE PARAPET SECTION

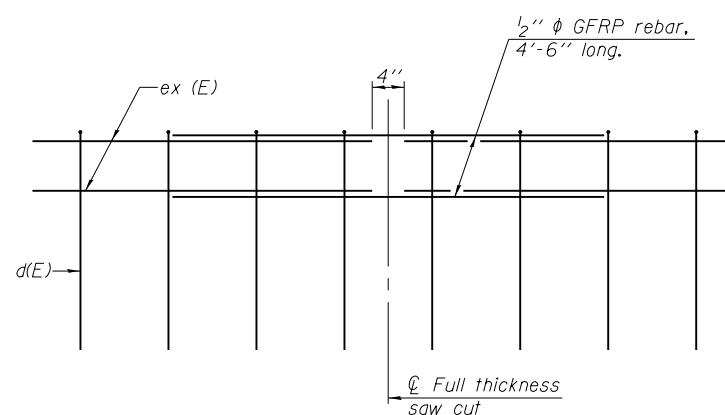
(Showing dimensions)

*See Superstructure Details.



SECTION

(34" parapet shown - 42" parapet similar)
(Showing reinforcement clearances for slip forming and additional reinforcement bars)



GFRP REBAR STIFFENING DETAIL

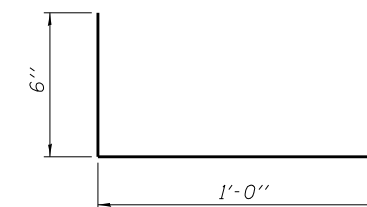
(Place as shown in parapet section at each parapet joint location.)

GENERAL NOTES

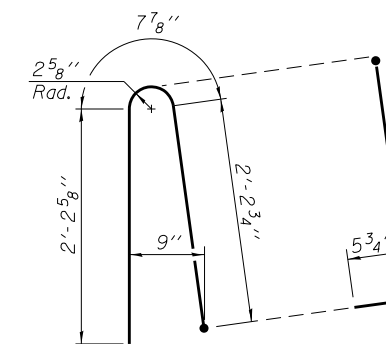
All dimensions shall remain the same as shown on superstructure details, except dimensions A and B which are to be revised as shown to provide additional clearance. Additional concrete needed to revise dimension A and B = 0.0165 cu. yds./ft. for 34" parapet or = 0.0223 cu. yds./ft. for 42" parapet.

Place aluminum sheet in curb portion at and near piers. Full thickness saw cut at all joint locations in lieu of cork joint filler.

Steel superstructure shown. Other superstructure types similar.

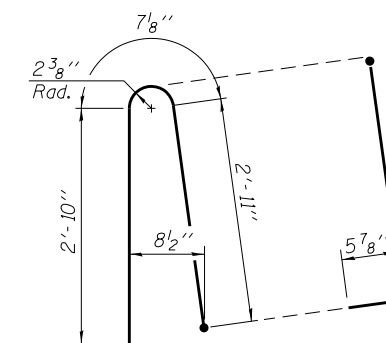


#3 (E) BAR



ALTERNATE BAR d(E)

(For 34" parapet when conduit is present)



ALTERNATE BAR d(E)

(For 42" parapet when conduit is present)

SFP 34-42

8-16-12



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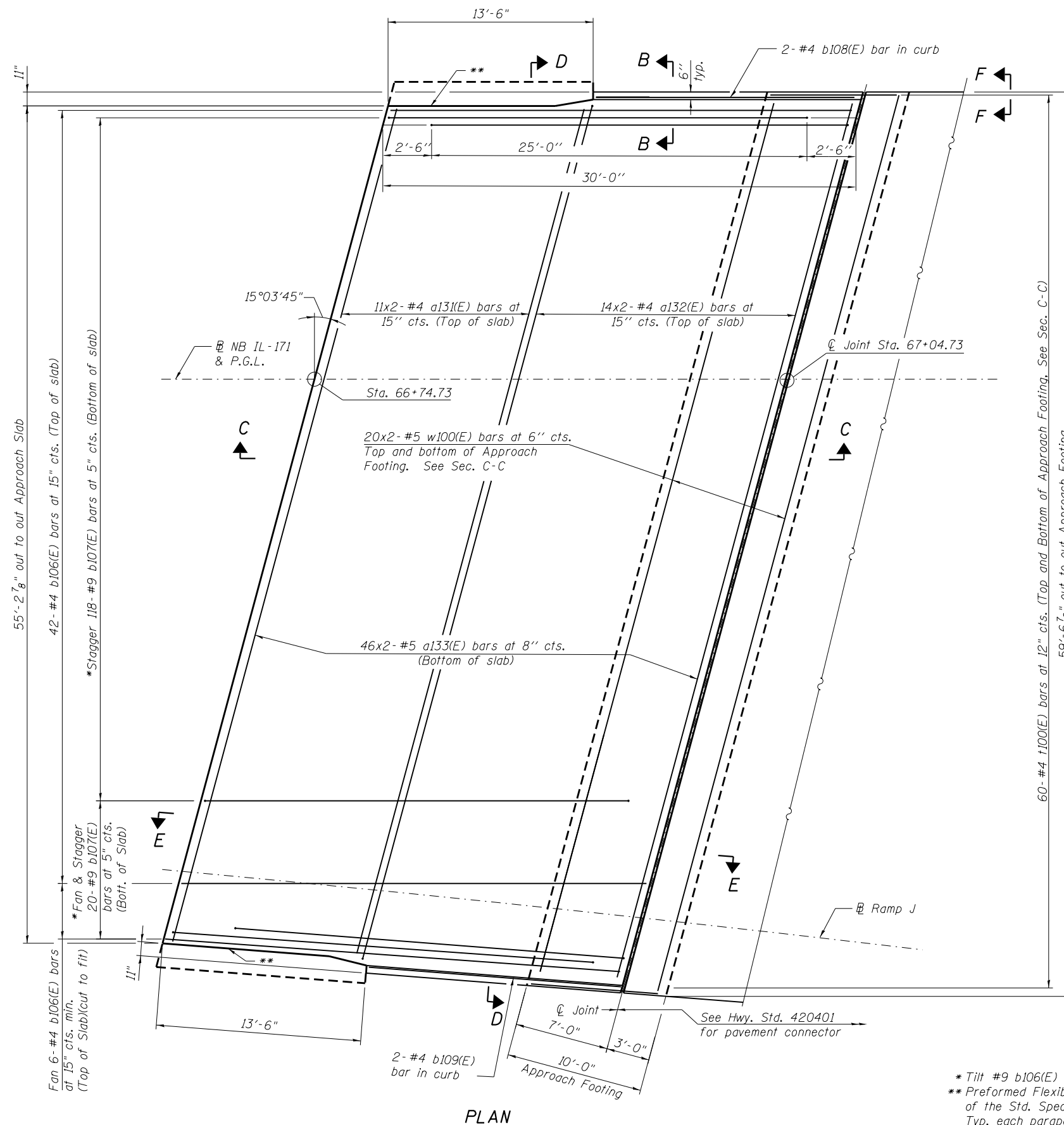
REVISED -
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

CONCRETE PARAPET SLIP FORMING OPTION
STRUCTURE NO. 016-2454

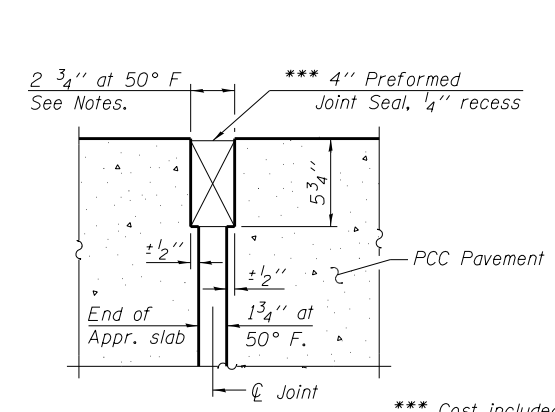
SHEET NO. SB18 OF SB34 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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CONTRACT NO. 60W75				
ILLINOIS FED. AID PROJECT				



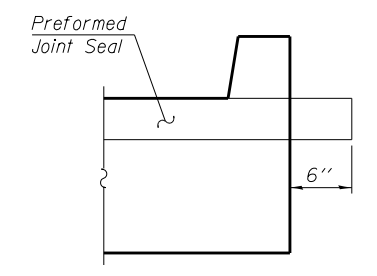
PLAN

Notes:
 See sheet SB20 for Sections C-C & D-D and View E-E.
 a131(E), a132(E), and a133(E) bar spacings measured along \varnothing Rdwy.
 The joint opening shall be determined per Article 520.04 except that on jointless structures, the distance described as the bridge length between the nearest fixed bearings each way from the joint shall be taken as half the bridge length plus the approach slab length. The minimum dimension shall be 1/2" for installation purposes.
 Bars indicated thus 12x2-#4 etc. indicates 12 lines of bars with 2 lengths per line.

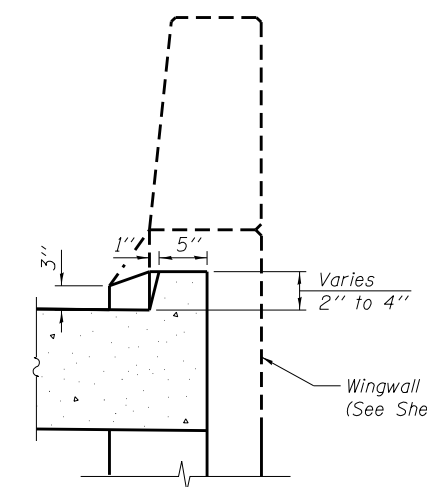


DETAIL A

*** Cost included with Concrete Superstructure.



VIEW F-F



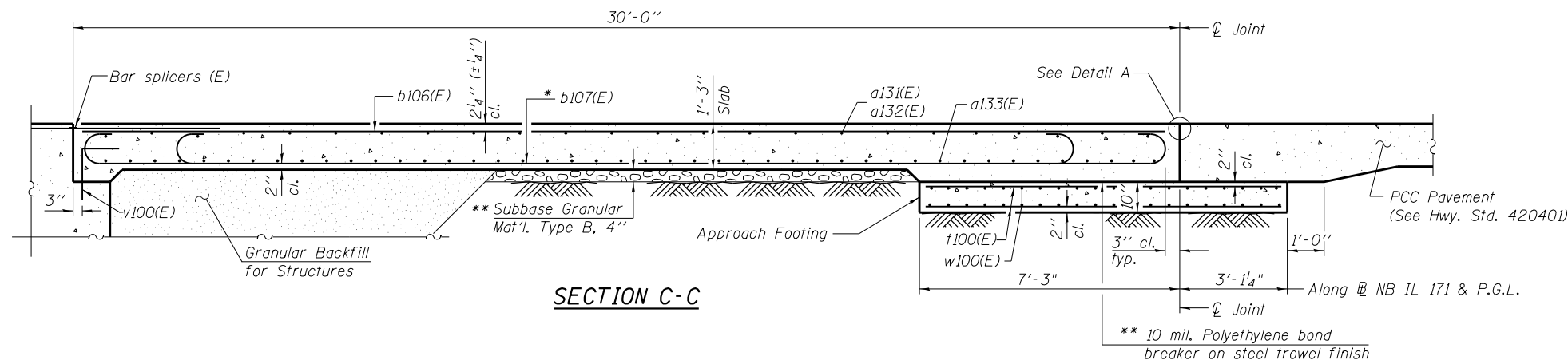
VIEW B-B

MINIMUM BAR LAP
 (Approach)
 #4 bar = 2'-7"
 #5 bar = 3'-3"

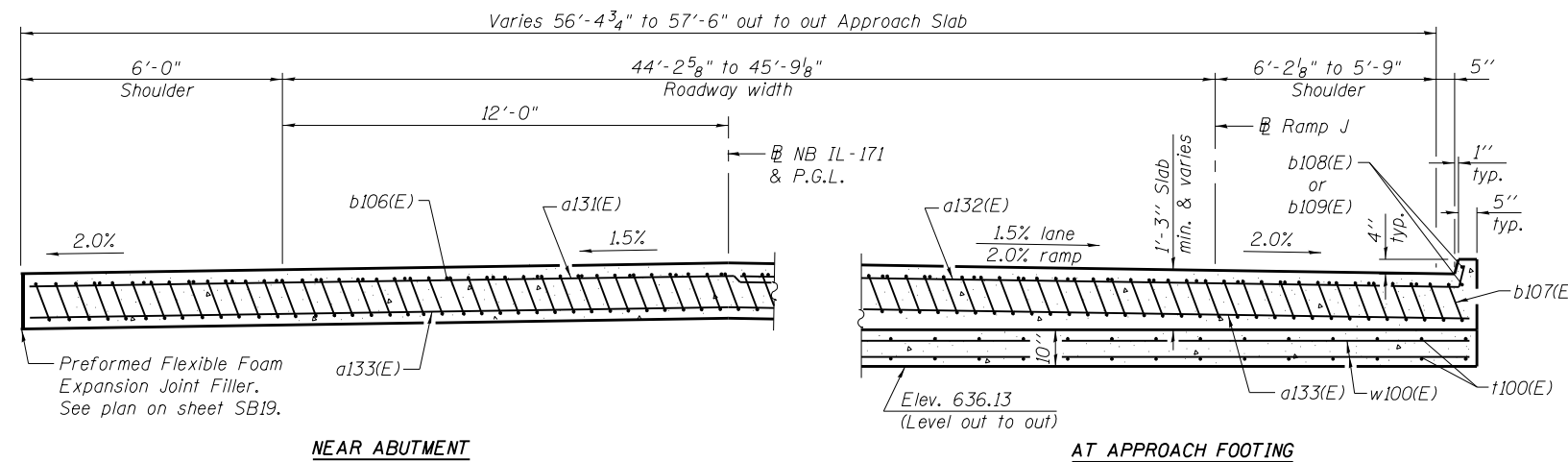
* Tilt #9 b106(E) bars as required to maintain clearance.
 ** Preformed Flexible Foam Expansion Joint Filler according to Article 1051.09 of the Std. Specifications; full depth of slab, full length of parapet. Typ. each parapet. Cost included with "Concrete Superstructure".

(Sheet 1 of 2)

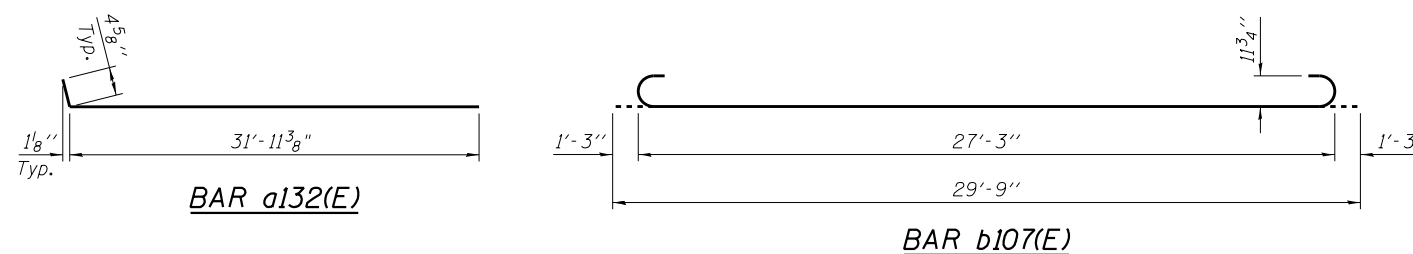
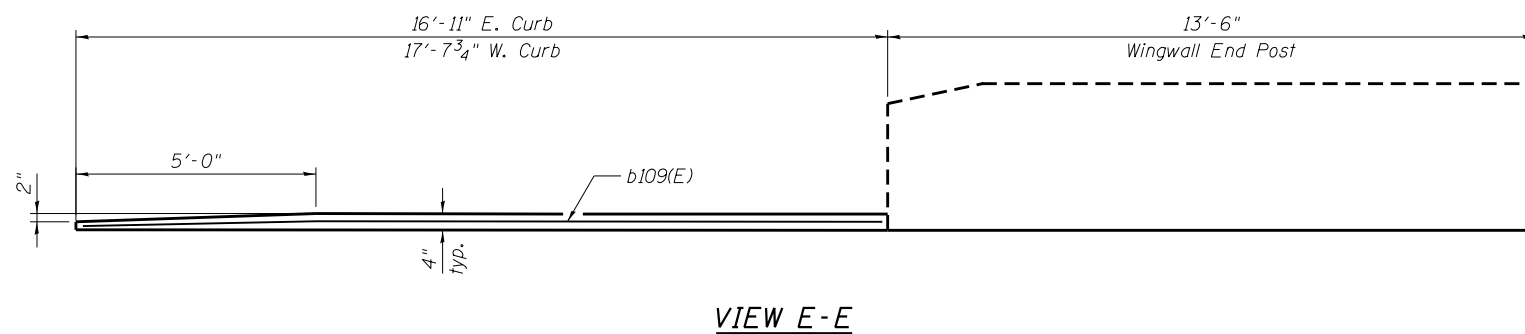
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	PLOT DATE = 6/8/2015 3:39:13 PM	CHECKED - JRS	REVISED -			CONTRACT NO. 60W75				



Notes:
 See sheet SB19 for Detail A and View B-B.
 Approach slab concrete shall be paid for as Concrete Superstructure.
 Approach footing concrete shall be paid for as Concrete Structures.
 Reinforcement shall be paid for as Reinforcement Bars, Epoxy Coated.
 For v100(E) bar details, see sheet SB29.
 The approach footing maximum applied service bearing pressure (Qmax) = 2.0 ksf.
 For bar splicer details, see sheet SB34.
 Cost of excavation for approach footing included with Concrete Structures.
 For Granular Backfill for Structures and drainage treatment details, see sheet SB3.
 For Wingwall End Post details, see sheet SB30 thru SB31.
 See Special Provisions for approach slab removal and Roadway Plans for approach slab removal quantities.



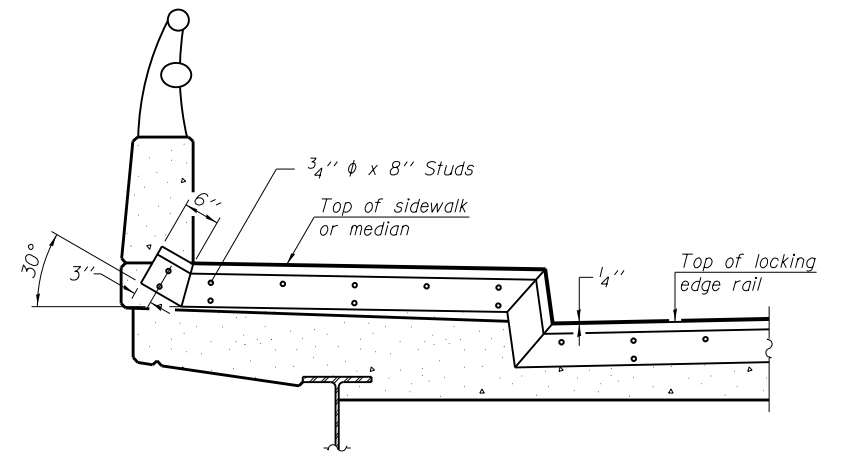
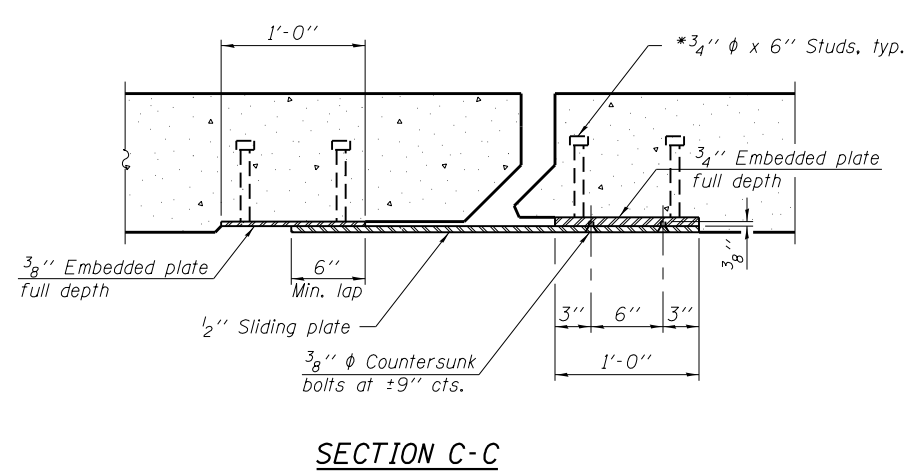
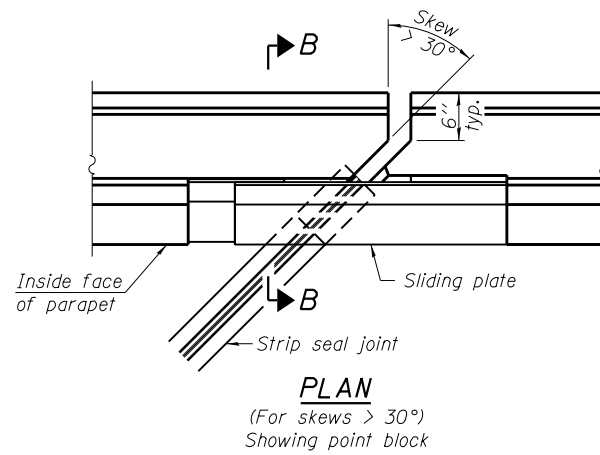
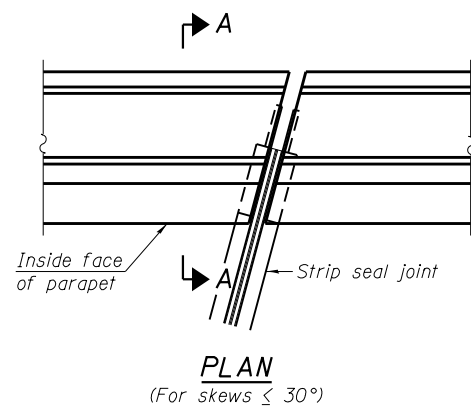
* Tilt #9 b107(E) bars as required to maintain clearance.
 ** Cost included with Concrete Superstructure.



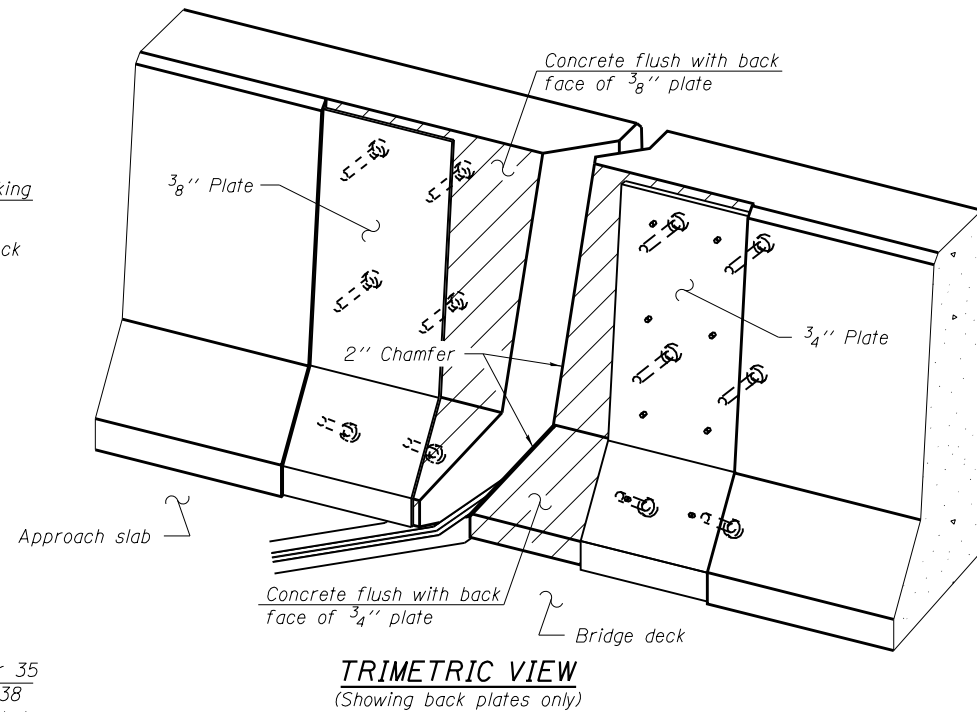
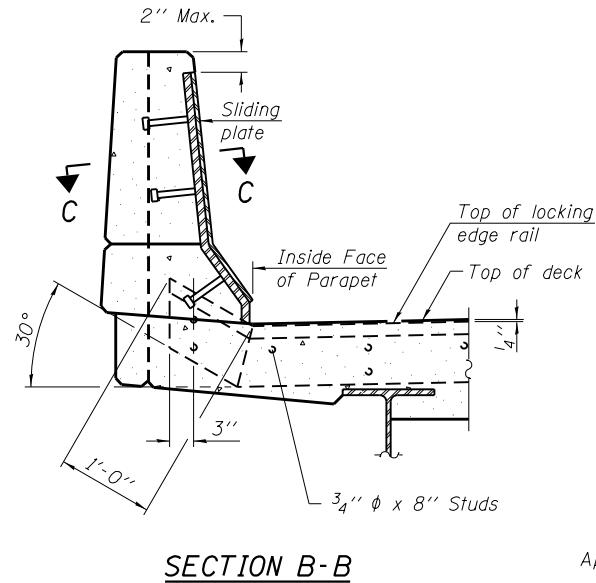
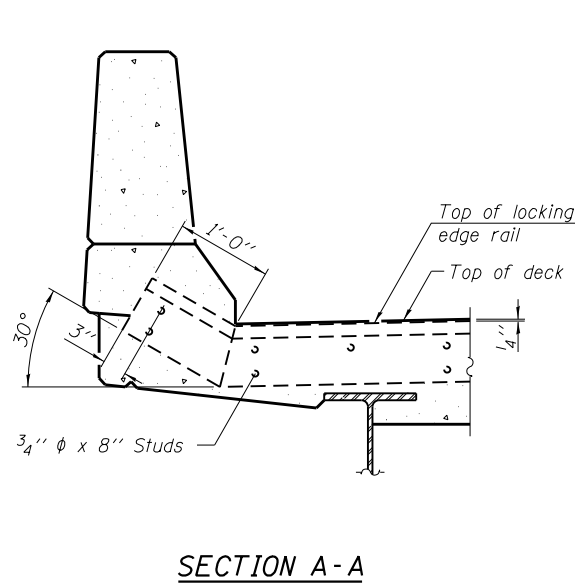
APPROACH BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a131(E)	22	#4	30'-2"	—
a132(E)	28	#4	32'-4"	—
a133(E)	92	#5	32'-5"	—
b106(E)	48	#4	29'-8"	—
b107(E)	138	#9	29'-9"	—
b108(E)	2	#4	17'-2"	—
b109(E)	2	#4	16'-6"	—
t100(E)	120	#4	9'-11"	—
w100(E)	80	#5	32'-5"	—
Concrete Superstructure		Cu. Yd.	94.2	
Concrete Structures		Cu. Yd.	19.0	
Bridge Deck Grooving		Sq. Yd.	184	
Protective Coat		Sq. Yd.	206	
Reinforcement Bars, Epoxy Coated		Pound	22,620	

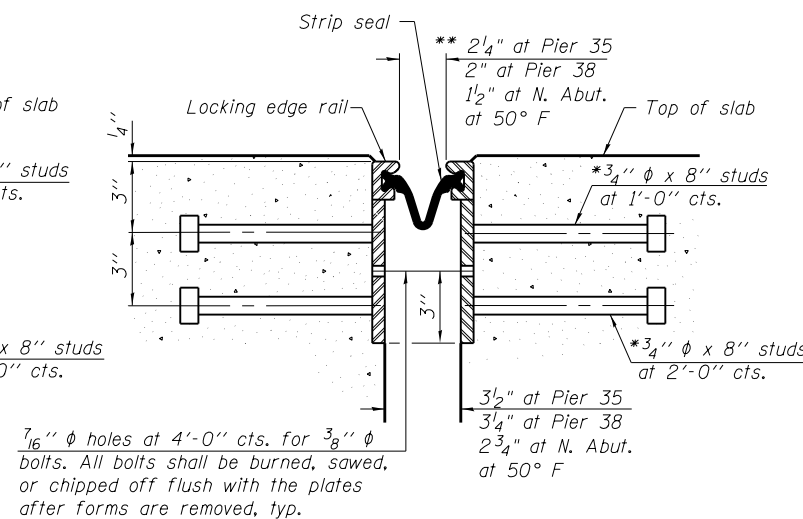
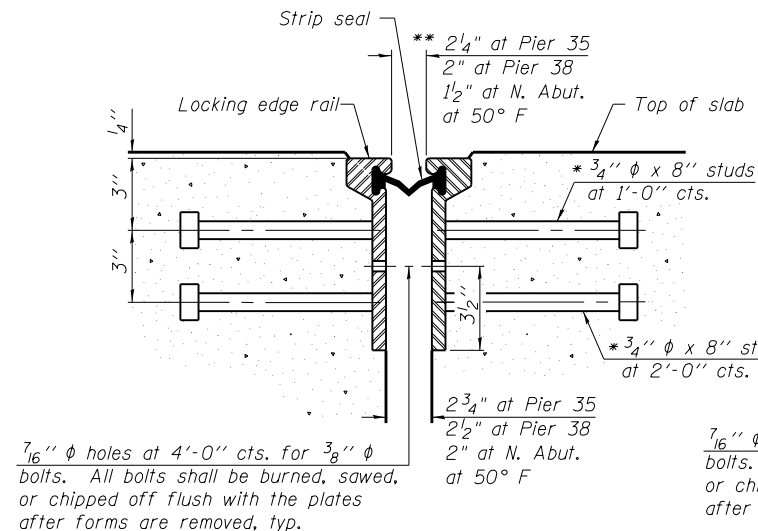
(Sheet 2 of 2)



TYPICAL END TREATMENT AT SIDEWALK OR MEDIAN
Shorter plates with a single row of studs at 12" cts. may be necessary on medians which are shallower than 9". See manufacturer's recommendation.



Notes:
The strip seal shall be made continuous and shall have a minimum thickness of 1/4". The configuration of the strip seal shall match the configuration of the Locking Edge Rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.
The Locking Edge Rails depicted are conceptual only, except for the minimum dimensions shown. The actual configuration of the Locking Edge Rails and matching strip seal may vary from manufacturer to manufacturer. Flanged edge rails will not be allowed. Locking Edge Rails may be spliced at slope discontinuities.
The manufacturer's recommended installation methods shall be followed.
The joint opening and deck dimensions detailed on the superstructure are based on a rolled rail expansion joint. If the Contractor elects to use the welded rail expansion joint, the opening and deck dimensions shall be modified according to the dimensions detailed on this sheet. Required modifications shall be made at no additional cost to the State.
All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.
Maximum space between rail segments shall be 3/16", sealed with a suitable sealant. Joints in rails within 10 ft. of curbs shall be welded.
Parapet plates and anchorage studs for skews $> 30^\circ$ included in the cost of Preformed Joint Strip Seal.



ROLLED EXTRUDED RAIL **WELDED RAIL**

LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue.
Rolled rail shown, welded rail similar.

LOCKING EDGE RAILS

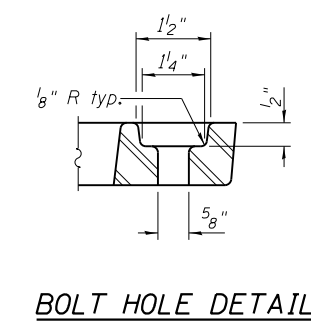
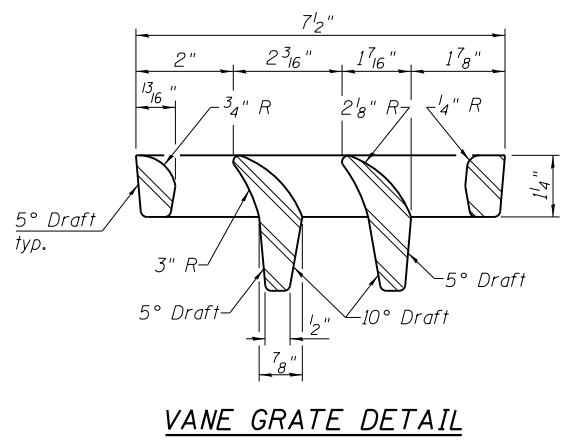
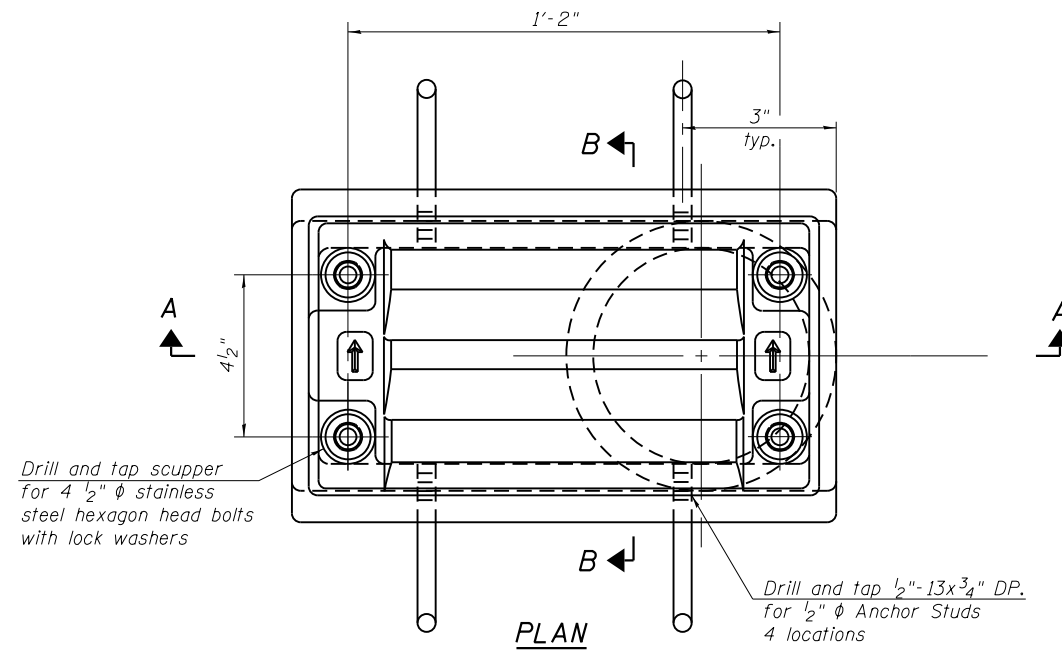
BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	168.0

* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.
** Strip Seal at Pier 35 shall be installed when the ambient temperature is at or below 80° F.

EJ-SSJ

1-27-12



Notes:

All cast iron parts shall be gray iron conforming to the requirements of AASHTO M 105, Class 35B.

Bolts, anchor studs, washers and nuts shall conform to the requirements of ASTM A 307 and shall be galvanized according to AASHTO M 232.

Downspouts located on the exterior side of a fascia beam shall be painted under a separate painting contract.

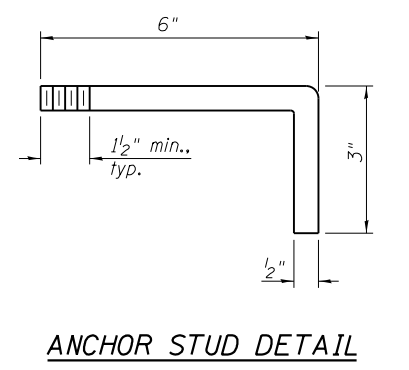
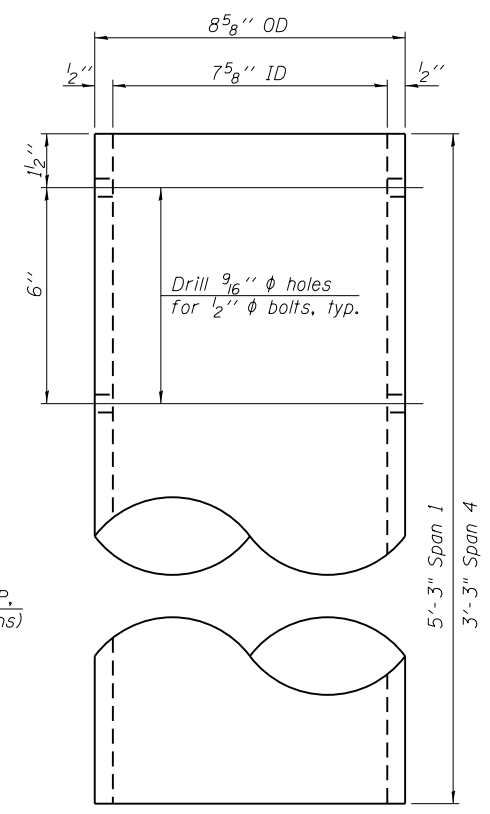
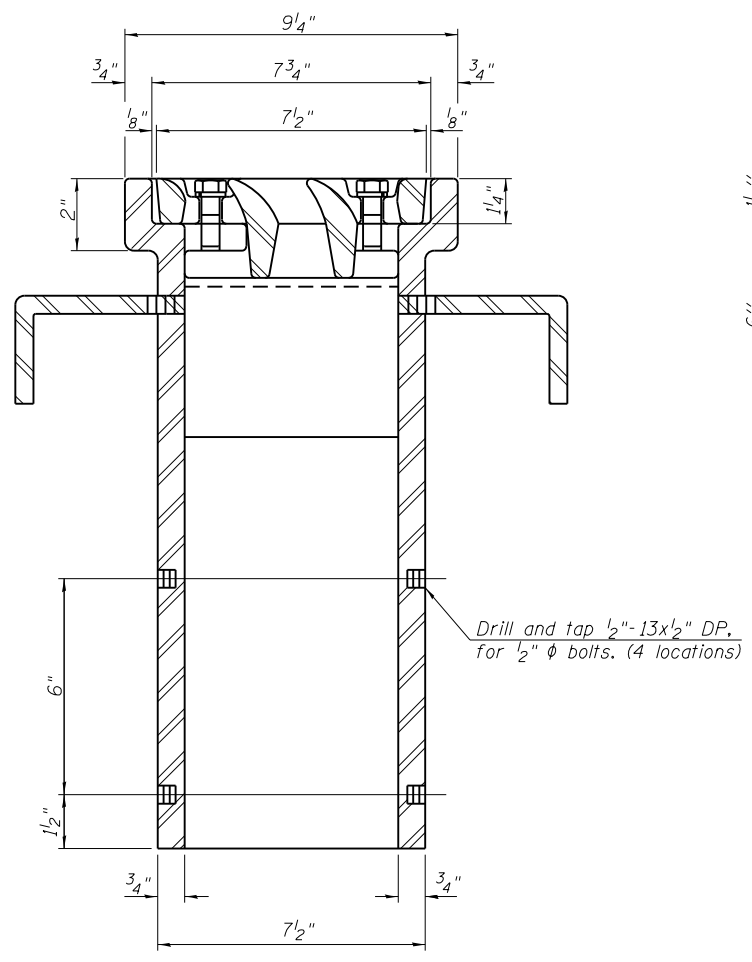
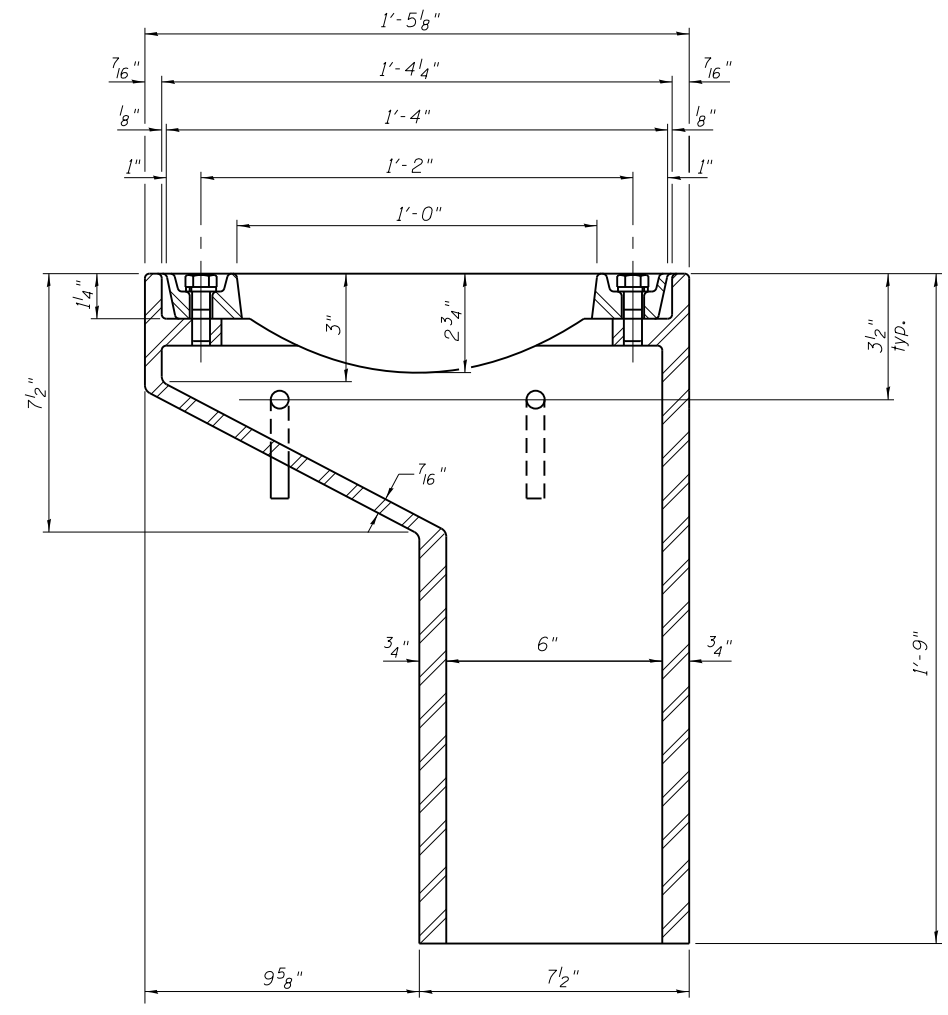
As an alternate, bolts, anchor studs, washers and nuts may be stainless steel according to Article 1006.29(d) of the Standard Specifications.

Structural steel weldments of equal sections and of the same configuration may be substituted for the cast iron scupper frame. Fillet or full penetration welds shall be used for the weldments. Details shall be submitted to the Engineer for approval. Structural steel weldments shall not be substituted for the cast iron scupper grate. Structural steel frames and downspouts shall be galvanized according to AASHTO M111.

The Contractor shall take appropriate measures to assure that Protective Coat is not applied to the scupper.

Cost of the Grate, Frame, Downspout, Anchor Studs, Bolts, Washers and Nuts including complete installation of the scupper shall be paid for at the contract unit price each for Drainage Scupper, DS-11.

Alternate fiberglass downspout conforming to ASTM D 2996 with a short-time rupture strength hoop tensile stress of 30,000 psi min. may be used in lieu of the cast iron or steel equivalent.



See Sheet SB17 for scupper location relative to parapet.

BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Drainage Scupper, DS-11	Each	2

DS-11

7-1-10

LE LIN ENGINEERING, LTD.
Consulting Engineers
Westmont, Illinois

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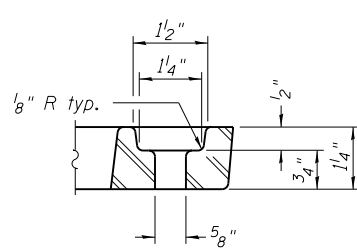
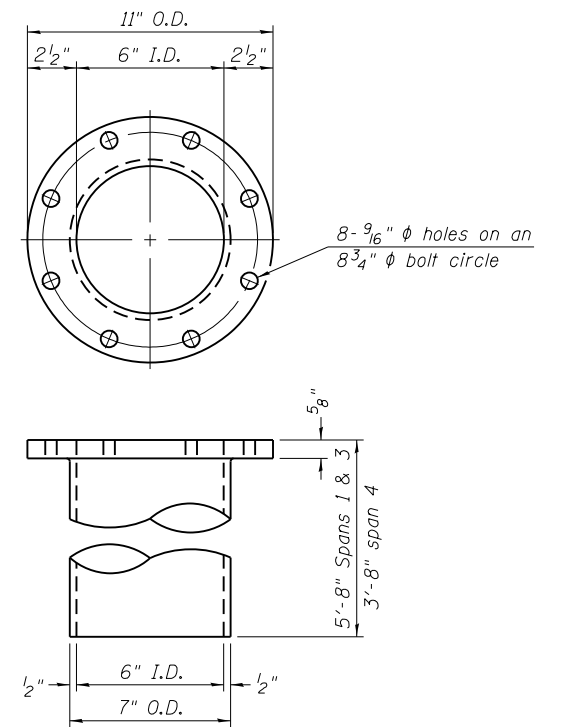
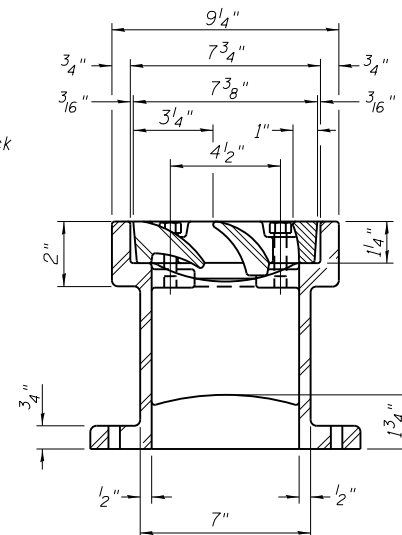
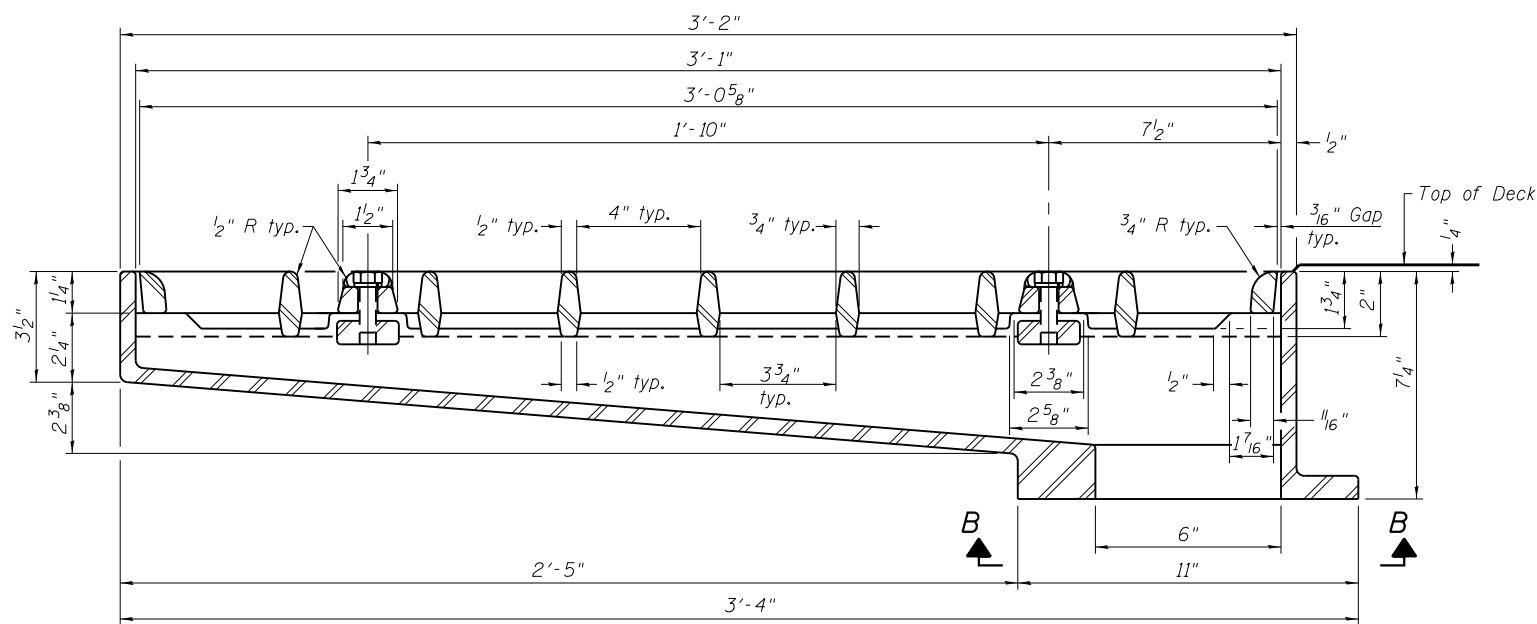
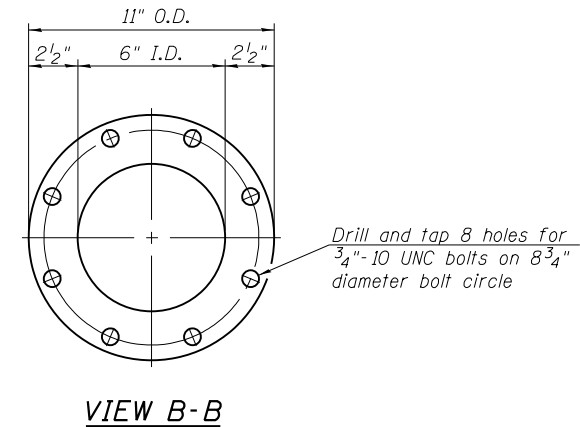
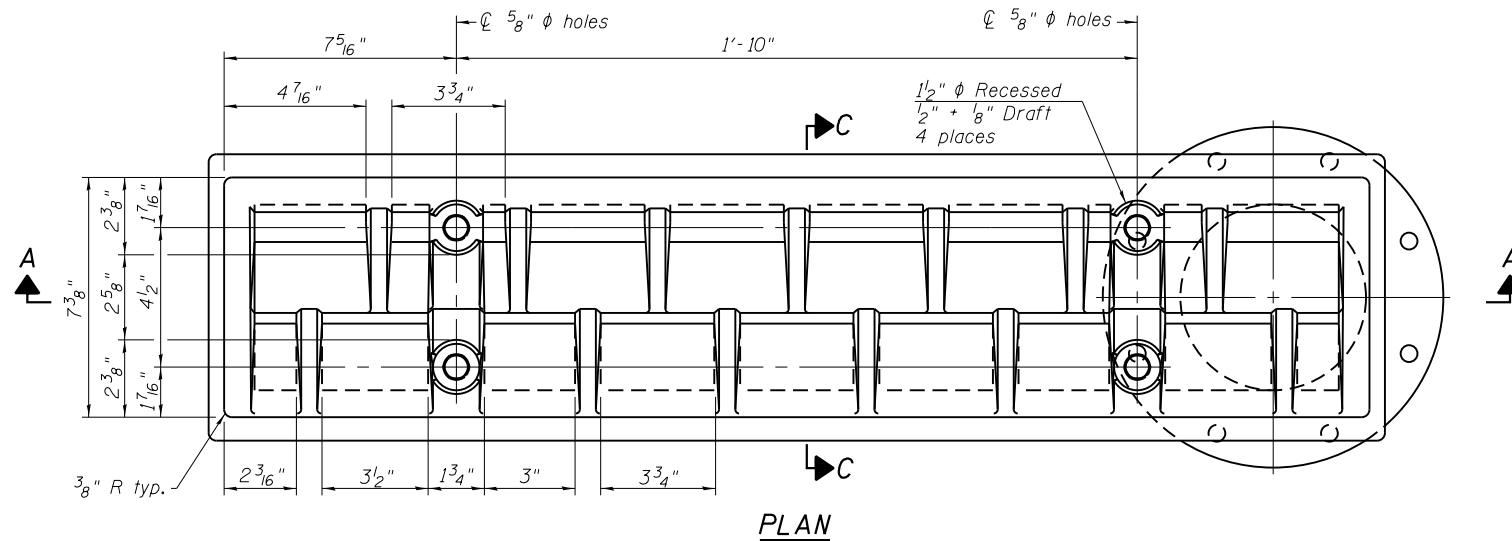
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**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

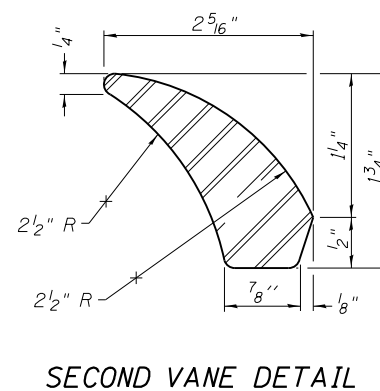
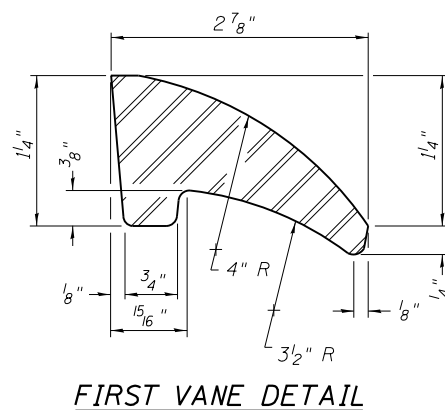
**DRAINAGE SCUPPER, DS-11
STRUCTURE NO. 016-2454**

SHEET NO. SB22 OF SB34 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372	2013-037B-R	COOK	787	286
CONTRACT NO. 60W75				
ILLINOIS FED. AID PROJECT				



SECTION A-A
See Sheet SB17 for scupper location relative to parapet.



BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Drainage Scupper, DS-33	Each	3

Notes:
 All cast iron parts shall be gray iron conforming to the requirements of AASHTO M 105, Class 35B.
 Bolts, anchor studs, washers and nuts shall conform to the requirements of ASTM A 307 and shall be galvanized according to AASHTO M 232.
 Downspouts located on the exterior side of a fascia beam shall be painted under a separate painting contract.
 As an alternate, bolts, anchor studs, washers and nuts may be stainless steel according to Article 1006.29(d) of the Standard Specifications.
 Structural steel weldments of equal sections and of the same configuration may be substituted for the cast iron scupper frame. Fillet or full penetration welds shall be used for the weldments. Details shall be submitted to the Engineer for approval. Structural steel weldments shall not be substituted for the cast iron scupper grate. Structural steel frames and downspouts shall be galvanized according to AASHTO M111.
 The Contractor shall take appropriate measures to assure that Protective Coat is not applied to the scupper.
 Cost of the Grate, Frame, Downspout, Anchor Studs, Bolts, Washers and Nuts including complete installation of the scupper shall be paid for at the contract unit price each for Drainage Scupper, DS-33.
 Alternate fiberglass downspout conforming to ASTM D 2996 with a short-time rupture strength hoop tensile stress of 30,000 psi min. may be used in lieu of the cast iron or steel equivalent.

DS-33

7-1-10



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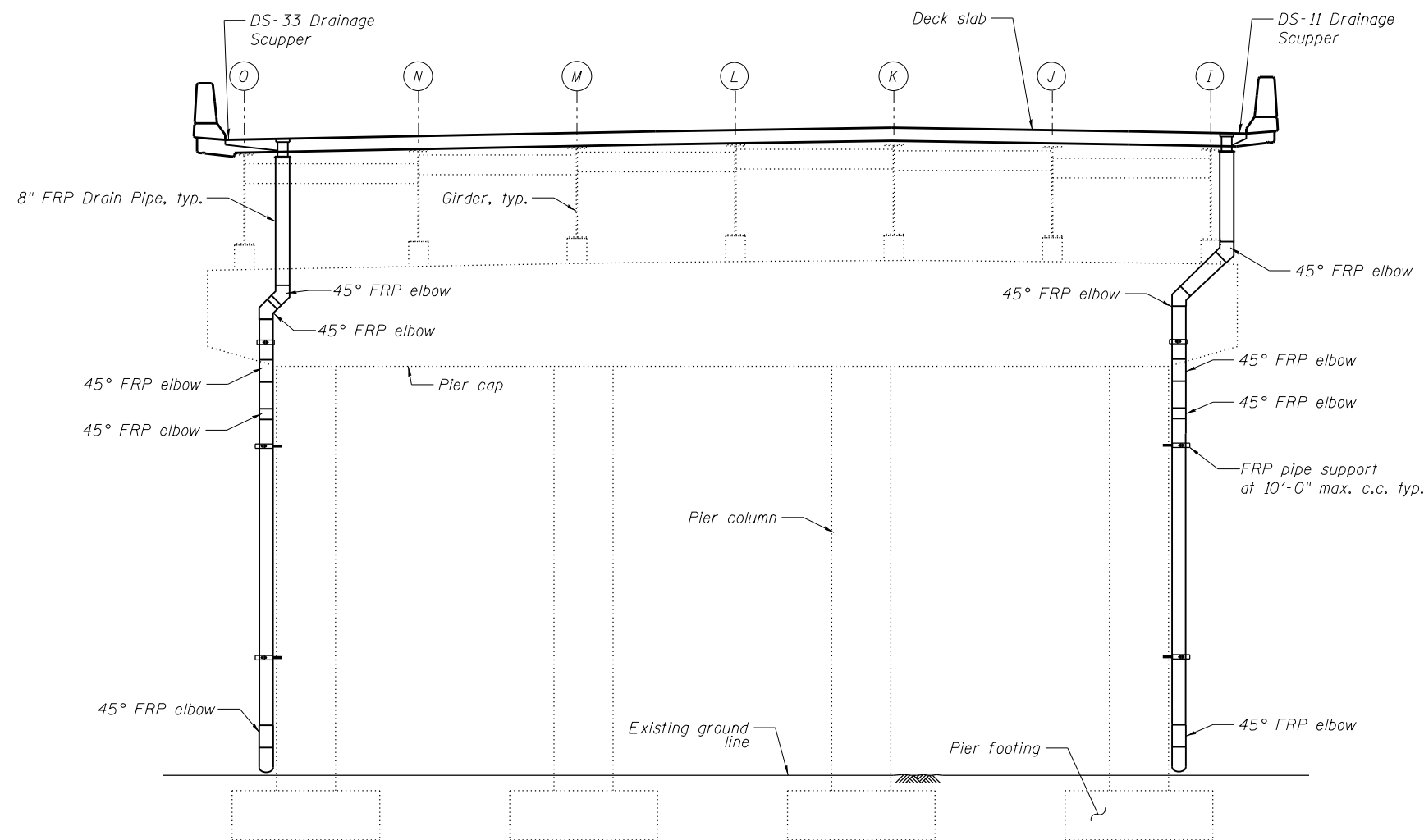
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**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

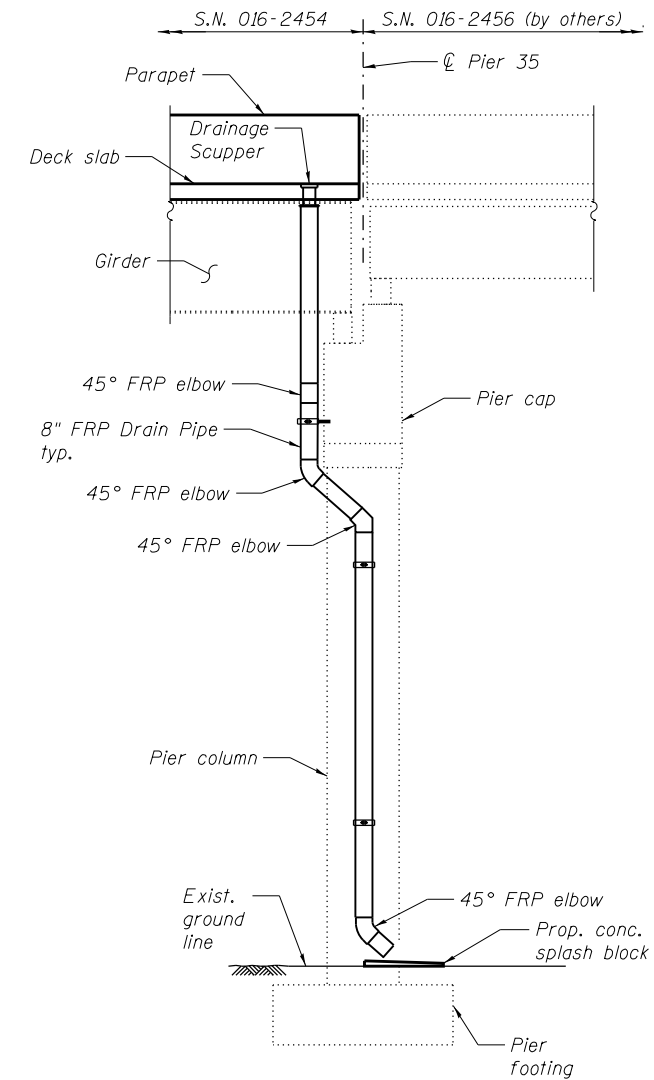
**DRAINAGE SCUPPER, DS-33
 STRUCTURE NO. 016-2454**

SHEET NO. SB23 OF SB34 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372	2013-037B-R	COOK	787	287
CONTRACT NO. 60W75				
ILLINOIS FED. AID PROJECT				



ELEVATION - PIER 35
(Looking Downstation)



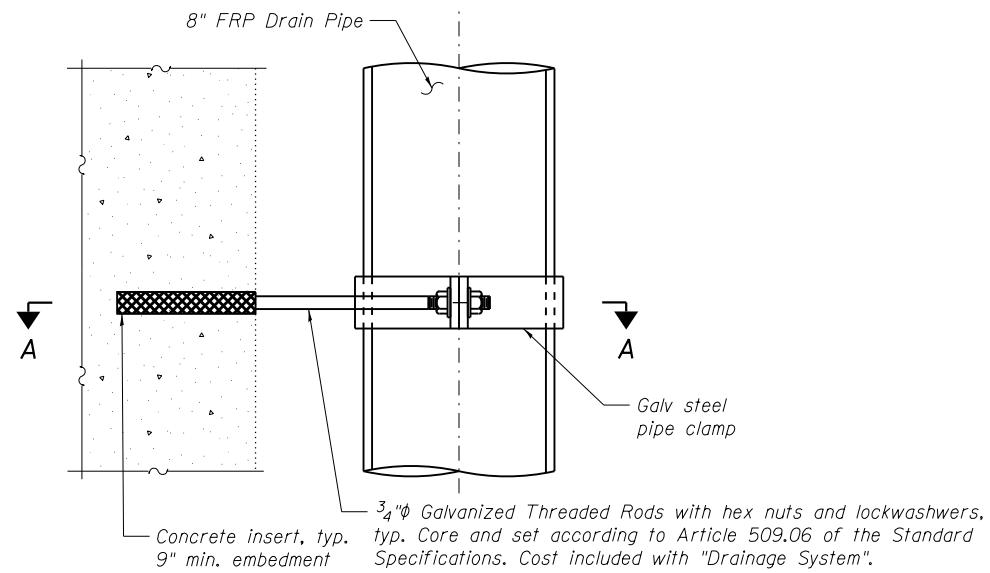
END VIEW - PIER 35
(Looking East)

Notes:
 Closed Drainage system shall be fabricated and installed in accordance with Special Provision "Drainage System".
 The surface of the fiberglass shall be free of bond inhibiting agents.

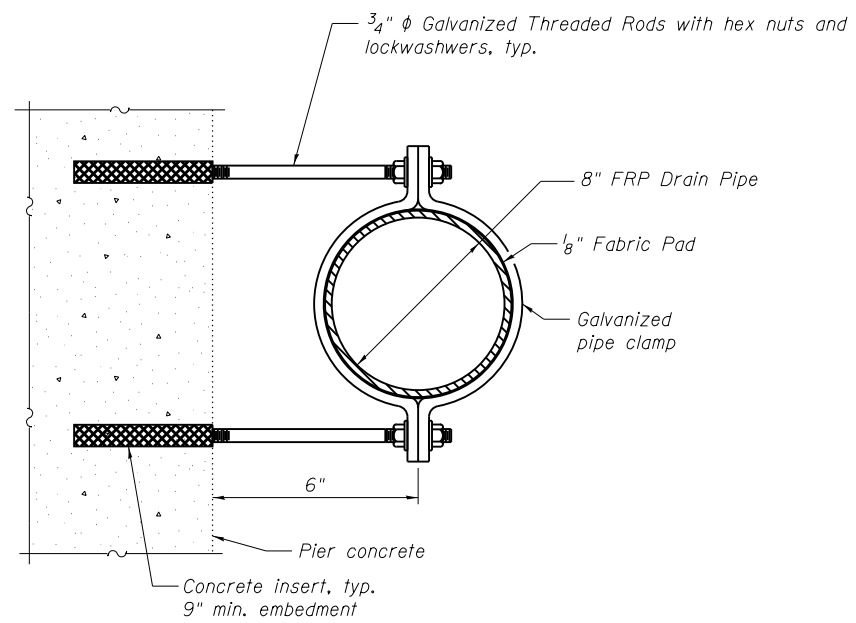
BILL OF MATERIAL

ITEM	UNIT	TOTAL
Drainage System	L. Sum	1

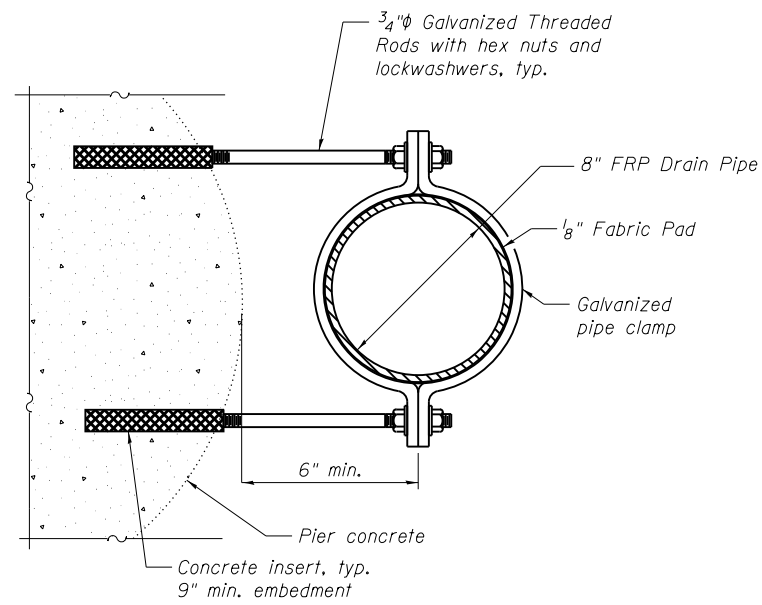
(Sheet 1 of 2)



PIPE SUPPORT PLAN AT PIER

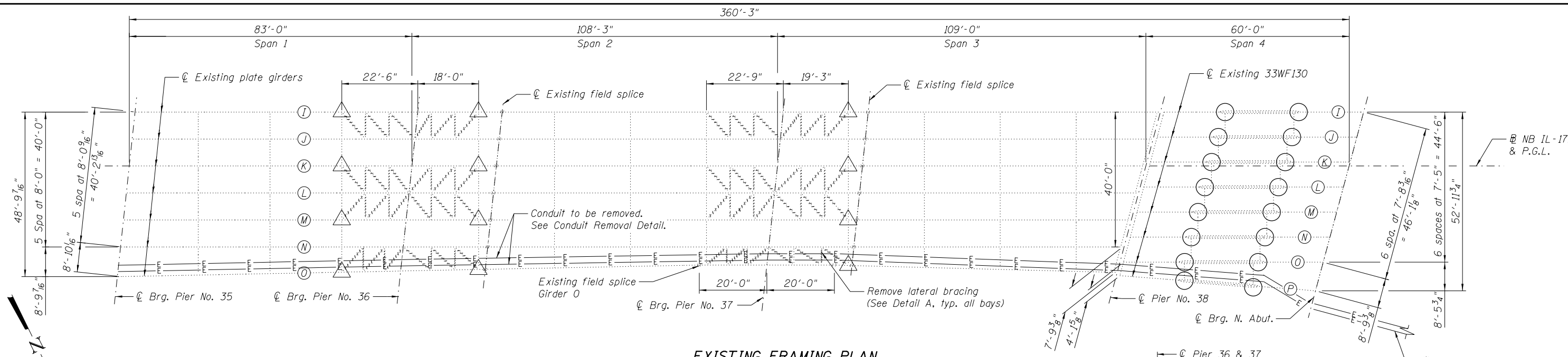


SECTION A-A
Pier Cap

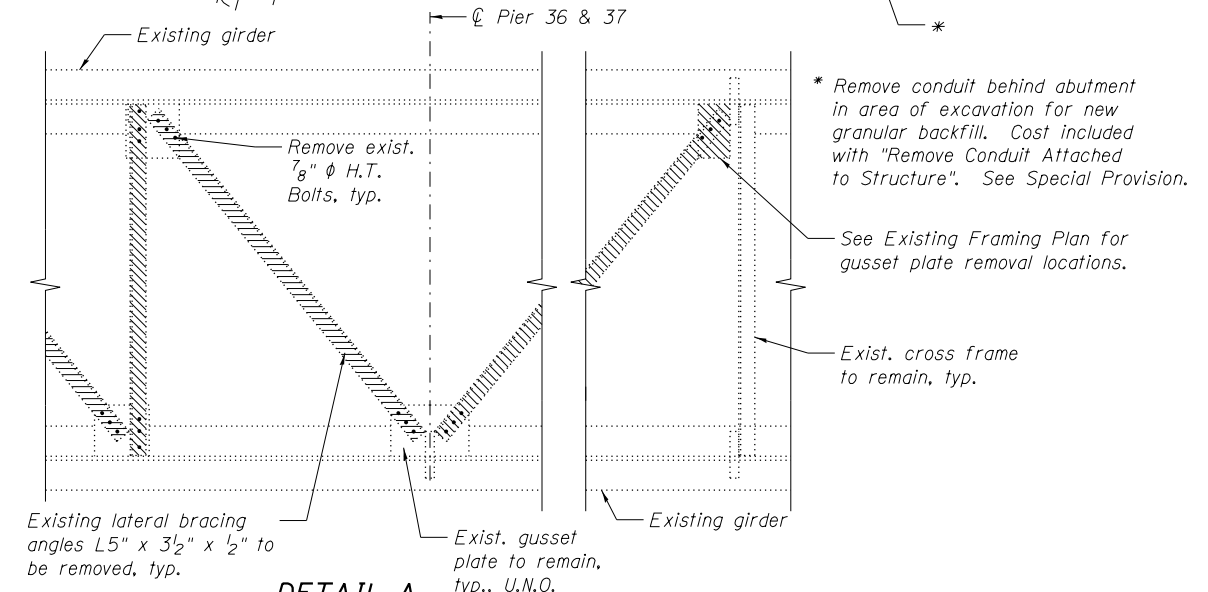


SECTION A-A
Pier Column

(Sheet 2 of 2)



EXISTING FRAMING PLAN

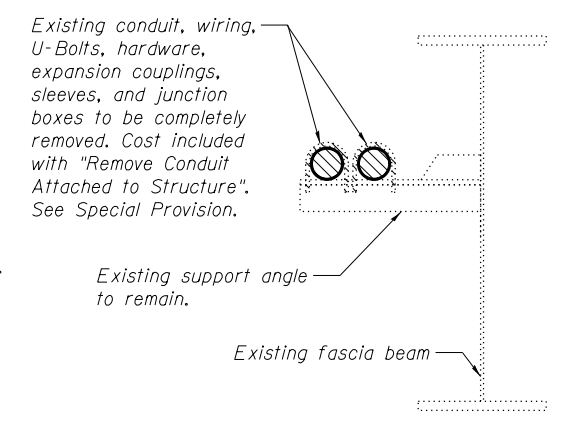


DETAIL A

Removal of existing lateral bracing angles and connecting H.T. bolts is included in the unit price of "Structural Steel Removal". See Special Provision.

LEGEND

- Location of cover plate ends to be retrofitted per "Structural Steel Repair" Special Provision. (16 locations)
- △ Remove existing gusset plates per "Structural Steel Removal" Special Provision. See Detail A. (12 locations)
- Remove lateral bracing angles per "Structural Steel Removal" Special Provision.



CONDUIT REMOVAL DETAIL

	0.4 Span 1	Pier 36	0.5 Span 2	Pier 37	0.6 Span 3	0.5 Span 4
I_s	19,198	32,273	20,731	47,092	29,331	8,238
$I_c(n)$	52,444	---	49,439	---	75,000	26,781
$I_c(3n)$	39,398	---	37,988	---	54,922	18,911
S_s	773	1,148	744	1,660	1,264	556
$S_c(n)$	1,080	---	1,002	---	1,638	866
$S_c(3n)$	1,001	---	929	---	1,530	782
Q	0.997	1.218	1.002	1.297	1.060	0.965
M_Q	470	868	330	1,384	911	411
s_Q	0.151	---	0.151	---	0.151	0.130
M_{sQ}	76	---	70	---	140	57
M_L	758.2	606.8	813.4	792.1	1,031.3	528.6
M_I	182.3	137.5	174.4	169.8	221.2	143.6
$5_3 [M_L + I]$	1,567	1,241	1,646	1,603	2,088	1,120
M_a	2,748	2,742	2,661	3,884	4,079	2,065
M_u	3,360	---	4,448	---	5,194	3,425
$f_s Q$ non-comp	7.3	9.1	5.3	10.0	8.6	8.9
$f_s Q$ (comp)	0.9	0.0	0.9	0.0	1.1	0.9
$f_s 5_3 [M_L + M_I]$	17.4	13.0	19.7	11.6	15.3	15.5
f_s (Overload)	25.6	22.1	26.0	21.6	25.0	25.3
f_s (Total)	---	28.7	---	28.1	---	---
VR	69.5	---	55.3	---	68.7	50.2

I_s, S_s : Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total and Overload) due to non-composite dead loads (in^4 and in^3).

$I_c(n), S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total and Overload) due to short-term composite live loads (in^4 and in^3).

$I_c(3n), S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total and Overload) due to long-term composite (superimposed) dead loads (in^4 and in^3).

Q : Un-factored non-composite dead load (kips/ft.).

M_Q : Un-factored moment due to non-composite dead load (kip-ft.).

s_Q : Un-factored long-term composite (superimposed) dead load (kips/ft.).

M_{sQ} : Un-factored moment due to long-term composite (superimposed) dead load (kip-ft.).

M_L : Un-factored live load moment (kip-ft.).

M_I : Un-factored moment due to impact (kip-ft.).

M_a : Factored design moment (kip-ft.).
 $1.3 [M_Q + M_{sQ} + \frac{5}{3} (M_L + M_I)]$

M_u : Compact composite moment capacity according to AASHTO LFD 10.50.1.1 or compact non-composite moment capacity according to AASHTO LFD 10.48.1 (kip-ft.).

f_s (Overload): Sum of stresses as computed from the moments below (ksi).
 $M_Q + M_{sQ} + \frac{5}{3} (M_L + M_I)$

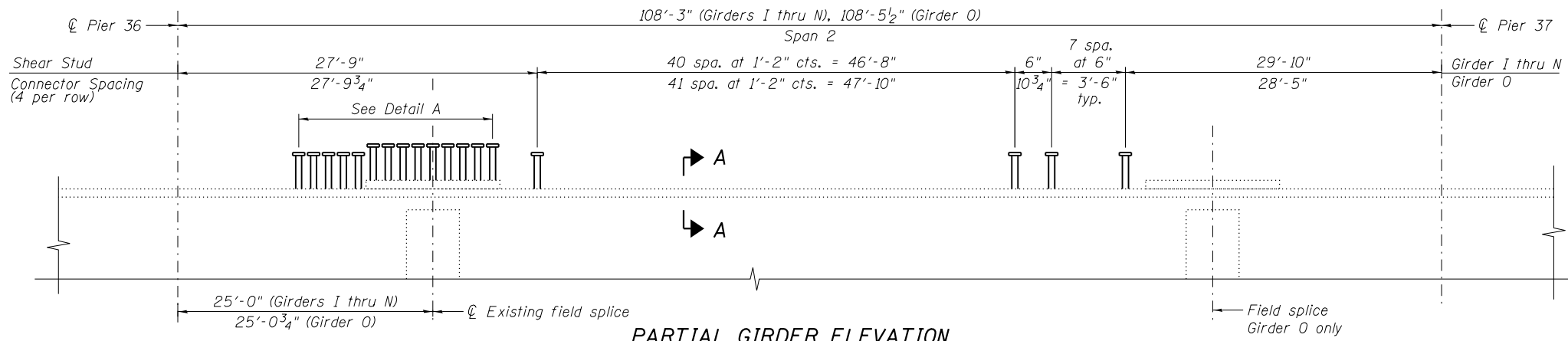
f_s (Total): Sum of stresses as computed from the moments below on non-compact section (ksi).
 $1.3 [M_Q + M_{sQ} + \frac{5}{3} (M_L + M_I)]$

VR: Maximum $t +$ impact shear range within the composite portion of the span for stud shear connector design (kips).

	Pier 35 North	Pier 36	Pier 37	Pier 38 South	Pier 38 North	North Abutment
R_Q	36.3	116.7	147.8	51.0	32.4	32.4
R_L	50.3	70.0	77.7	51.8	45.7	45.7
R_I	12.1	15.9	16.7	11.1	12.4	12.4
R_{Total}	98.7	202.6	242.2	113.8	90.5	90.5

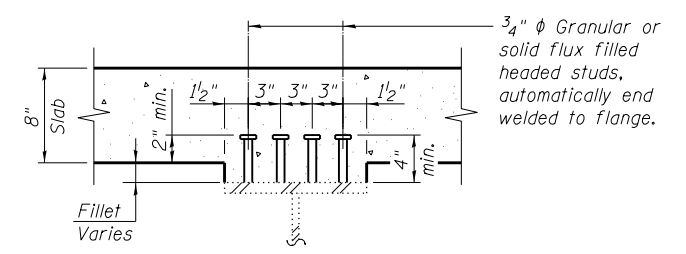
* Compact section
 ** Braced non-compact and partially braced section

Notes:
 No additional studs are being added to Spans 1, 3 and 4. For Cover Plate Retrofit Details, see sheet SB27.
 The Engineer will inspect all existing bearing anchor bolts to ascertain their condition. Any damaged anchor bolts shall be reported to the BBS for further direction. The Contractor shall provide all means and access for the Engineer to perform the anchor bolt inspections. All costs associated with providing the access shall be considered included in the unit price for "Structural Steel Repair".

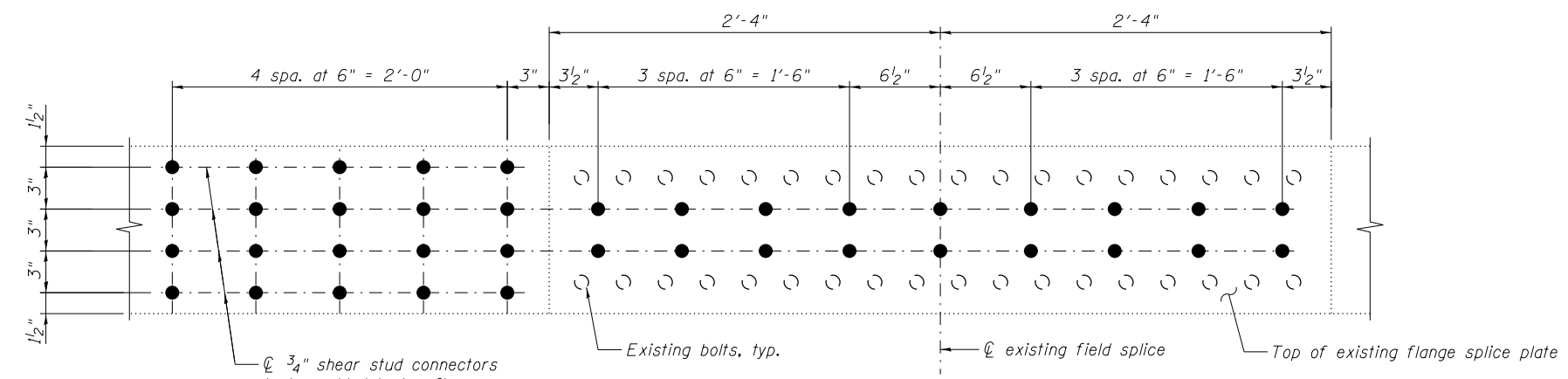


PARTIAL GIRDER ELEVATION

(No existing shear studs in Span 2.
No new shear studs shall be added to Spans 1, 3 and 4.)



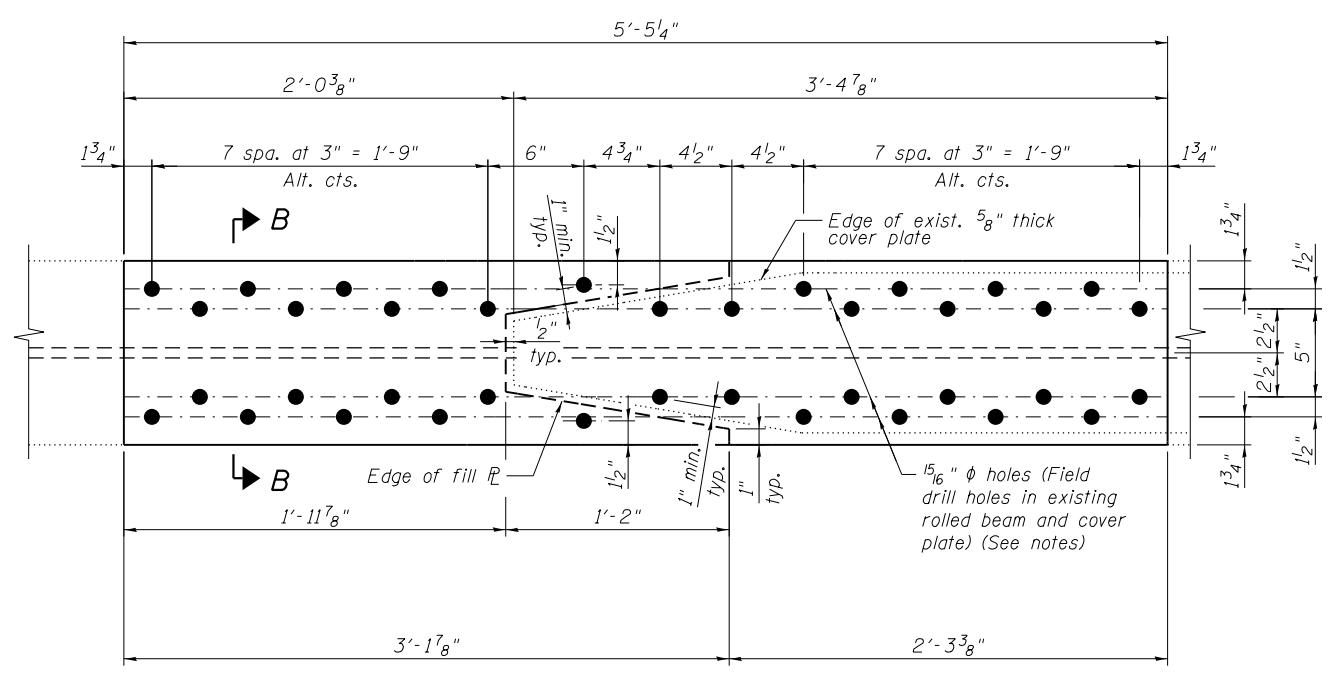
SECTION A-A



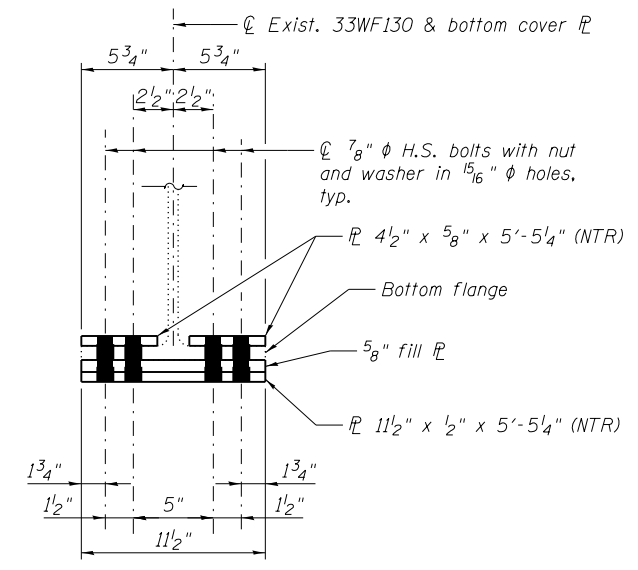
DETAIL A

BILL OF MATERIAL

ITEM	UNIT	TOTAL
Stud Shear Connectors	Each	1,642
Structural Steel Removal	Pound	10,140
Structural Steel Repair	Pound	4,700



COVER PLATE RETROFIT DETAIL (SPAN 4)
(Bottom Flanges)



SECTION B-B

Notes:
Load carrying components designated "NTR" shall conform to the Impact Testing Requirement, Zone 2.
Structural steel plates shall conform to the requirements of AASHTO M270 Grade 50.
See Sheet SB26 for locations of cover plate retrofits. Cost of cover plate retrofit and associated field drilling to be included with "Structural Steel Repair".



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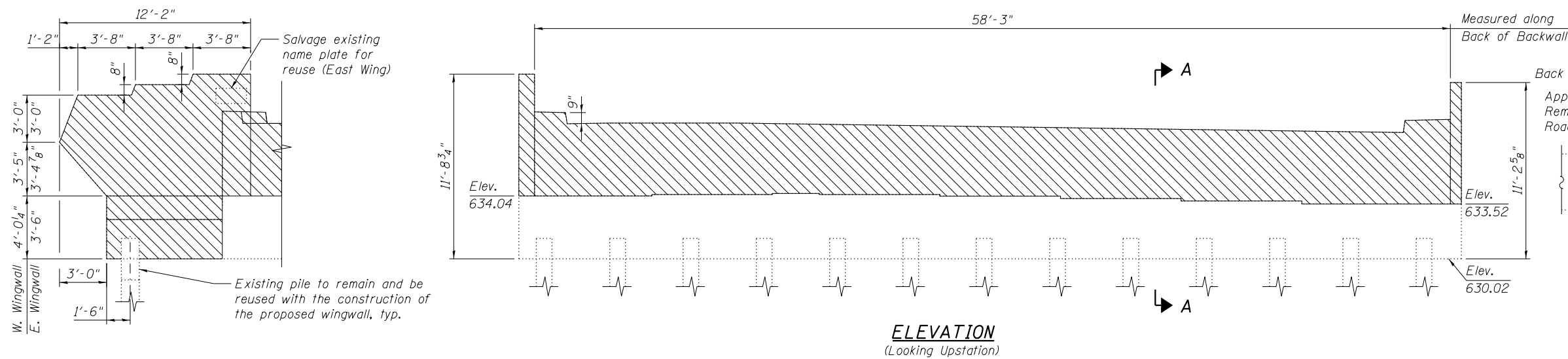
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

STRUCTURAL STEEL DETAILS
STRUCTURE NO. 016-2454
SHEET NO. SB27 OF SB34 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372	2013-037B-R	COOK	787	291

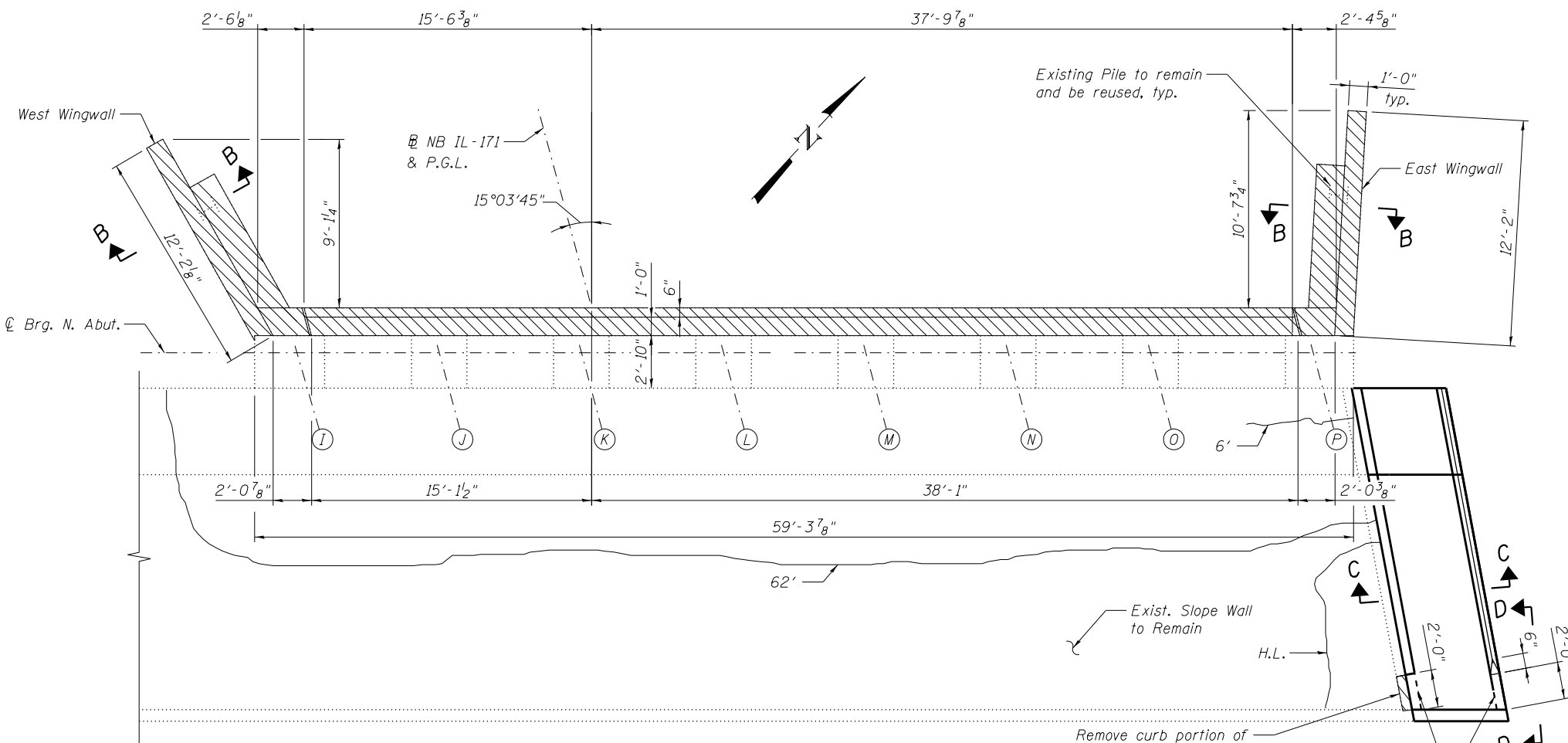
CONTRACT NO. 60W75
ILLINOIS FED. AID PROJECT



TYPICAL WINGWALL ELEVATION

BEARING SEAT ELEVATIONS

Beam No.	N. Abut.
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J	634.12
K	634.18
L	634.12
M	634.02
N	633.86
O	633.70
P	633.52

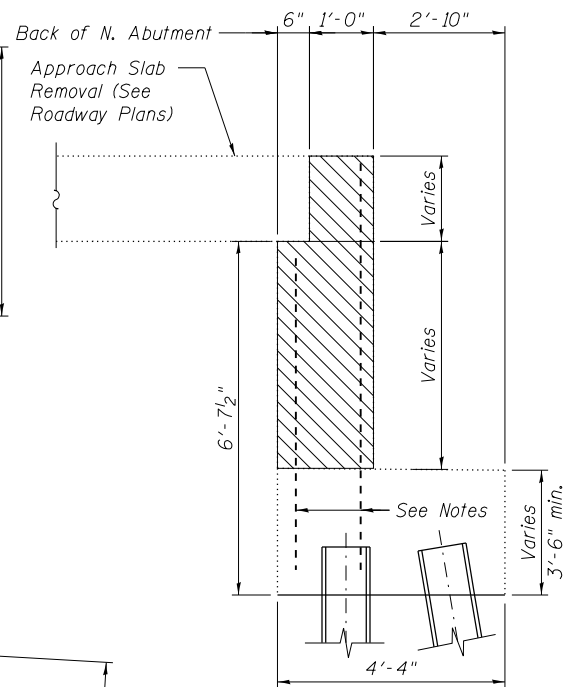


PLAN

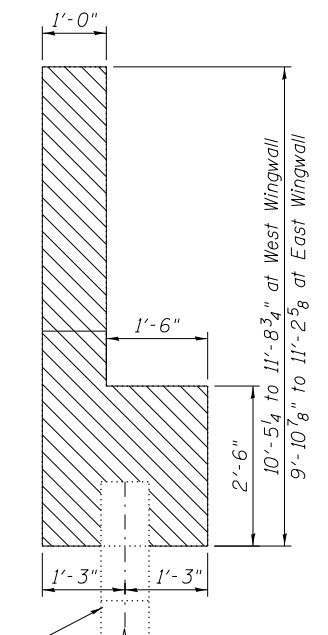
LEGEND

- Concrete Removal
- Hairline Crack (Not to be repaired)
- Epoxy Crack Injection

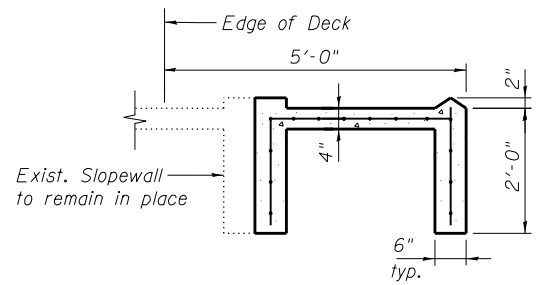
Notes:
 Existing reinforcement shall be cleaned and incorporated into the new construction. Cost included with "Concrete Removal".
 Any reinforcement bars that are damaged during concrete removal operations shall be repaired or replaced using an approved bar splicer or anchorage system. Cost included in "Concrete Removal".
 Slope wall shall be reinforced with welded wire fabric, 6 in. X 6 in. W4.0 x W4.0, weighing 58 lbs per 100 sq ft. Refer to Sheet SB3 for additional slopewall details.
 Embankment required for the slopewall widening shall be paid for as "Furnished Excavation". See Roadway Plans.



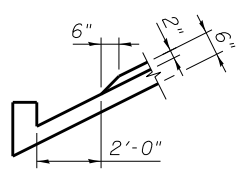
SECTION A-A



SECTION B-B



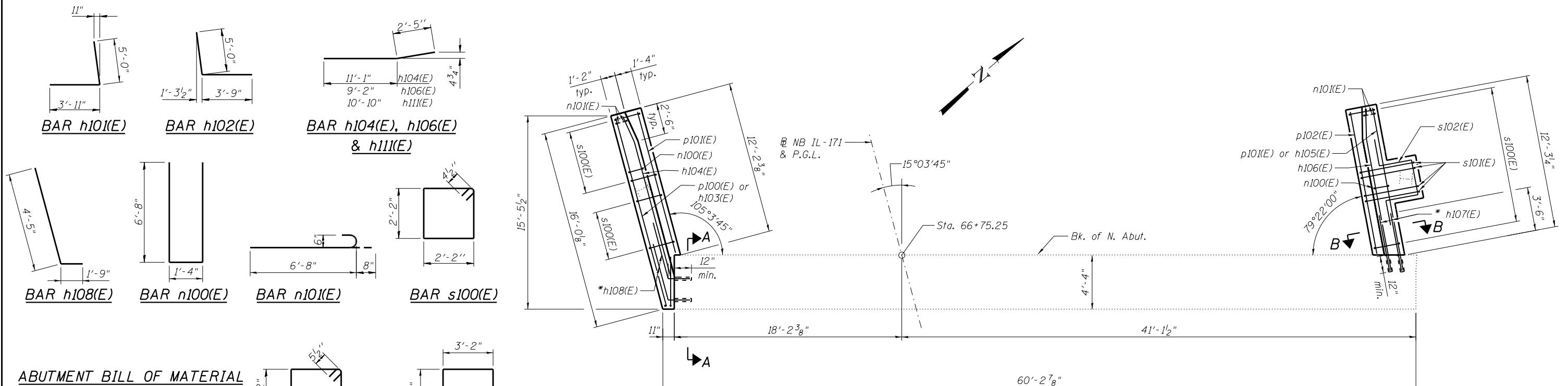
SECTION C-C



VIEW D-D

BILL OF MATERIAL

ITEM	UNIT	TOTAL
Concrete Removal	Cu. Yd.	21.6
Epoxy Crack Injection	Foot	62
Slope Wall 4 Inch	Sq. Yd.	20



ABUTMENT BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h100(E)	16	#5	30'-1"	—
h101(E)	12	#5	8'-11"	└
h102(E)	12	#5	8'-9"	└
h103(E)	1	#4	15'-8"	—
h104(E)	1	#4	13'-6"	—
h105(E)	1	#4	11'-10"	—
h106(E)	1	#4	11'-7"	—
h107(E)	6	#7	4'-6"	—
h108(E)	6	#7	6'-2"	—
h109(E)	11	#4	12'-9"	—
h110(E)	11	#4	13'-6"	—
h111(E)	14	#4	13'-3"	—
h112(E)	10	#6	30'-5"	—
n100(E)	21	#6	14'-8"	└
n101(E)	12	#6	7'-4"	└
p100(E)	3	#7	15'-8"	—
p101(E)	6	#7	11'-10"	—
p102(E)	3	#7	11'-6"	—
s100(E)	26	#4	9'-5"	└
s101(E)	4	#5	14'-3"	└
s102(E)	3	#5	11'-4"	└
v100(E)	58	#5	3'-9"	└
v101(E)	58	#4	3'-8"	└
v102(E)	29	#6	7'-11"	└
v103(E)	6	#6	7'-2"	└
v104(E)	22	#6	7'-11"	└
v105(E)	58	#5	3'-1"	└
v106(E)	58	#5	4'-5"	└
v107(E)	57	#5	4'-1"	└
Structure Excavation	Cu. Yd.		103	
Concrete Structures	Cu. Yd.		30.0	
Reinforcement Bars, Epoxy Coated	Pound		4,570	
Concrete Sealer	Sq. Ft.		299	

PLAN - PILE CAP

* Drill and grout 12" into existing abutment cap per Article 584 of the Standard Specifications. (Cost included with Concrete Structures).

PLAN - PILE LAYOUT

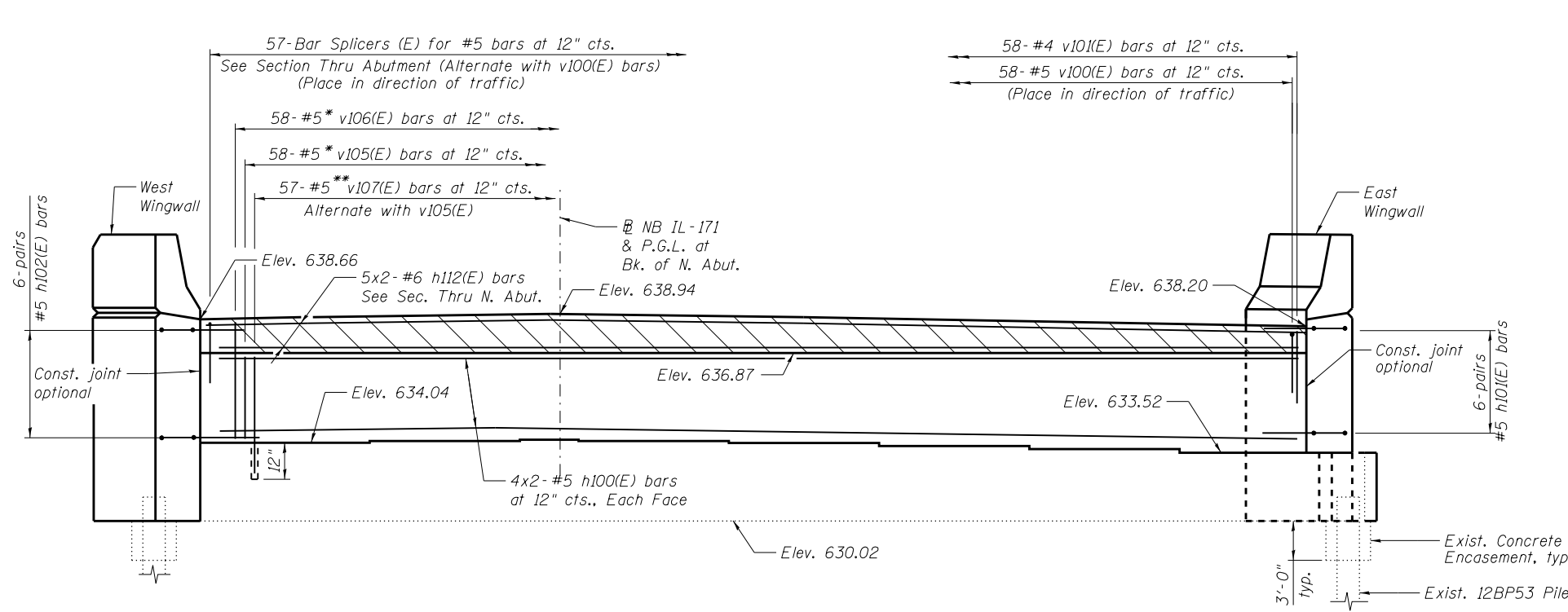
** Centered about Exist. 12BP53 Pile.

PILE DATA

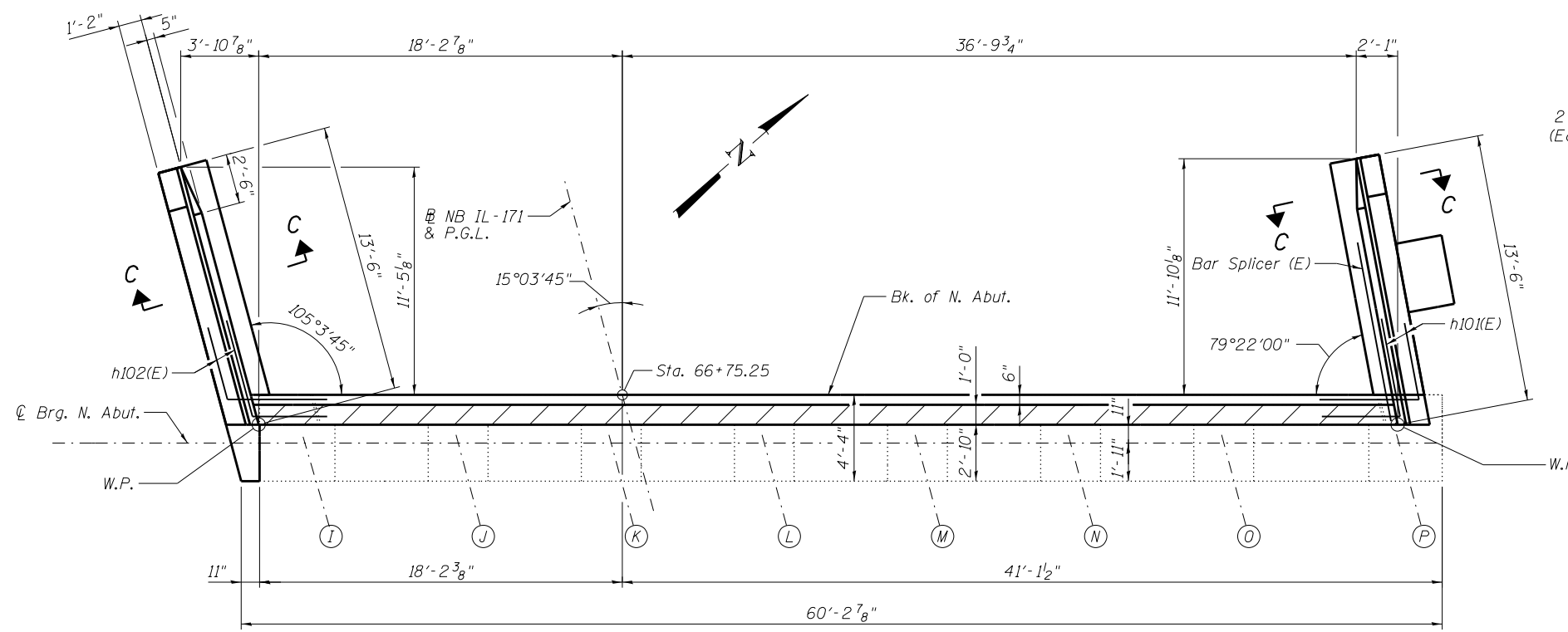
Type: HPI2x53
 Nominal Required Bearing: 264 k
 Allowable Resistance Available: 88 k
 Est. Length: 38'-0"
 No. Production Piles: 2
 No. Test Piles: 0

Note:
 See Sheet SB31 for Sections A-A and B-B.
 See Sheet SB30 for Section C-C.
 See Sheet SB31 for Section C'-C'.

(Sheet 1 of 3)



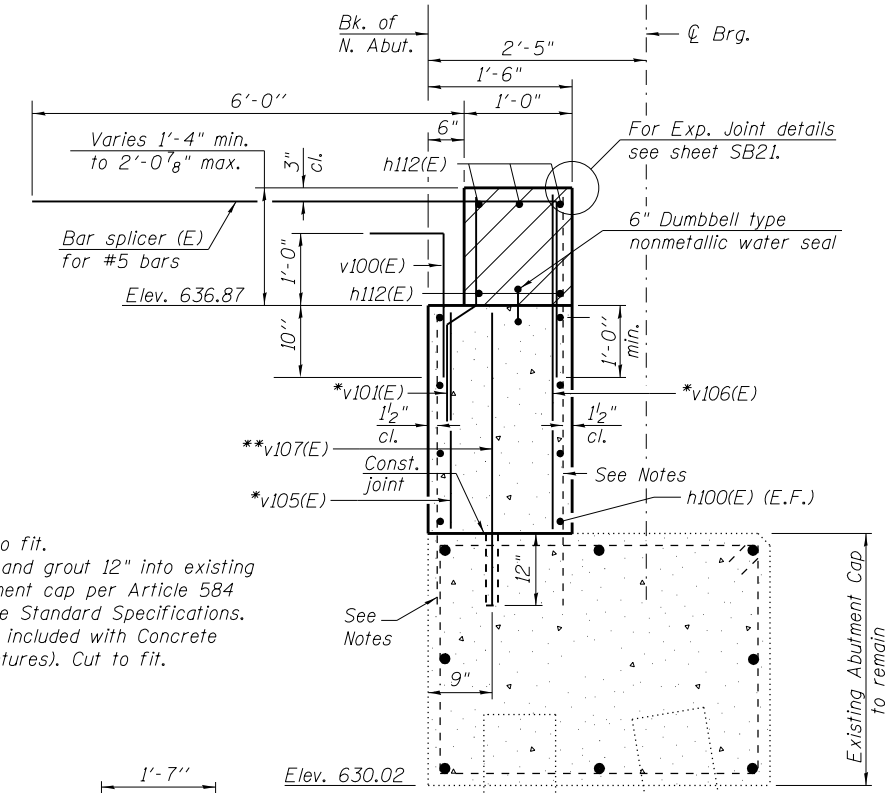
ELEVATION
(Looking Upstation)



TOP VIEW

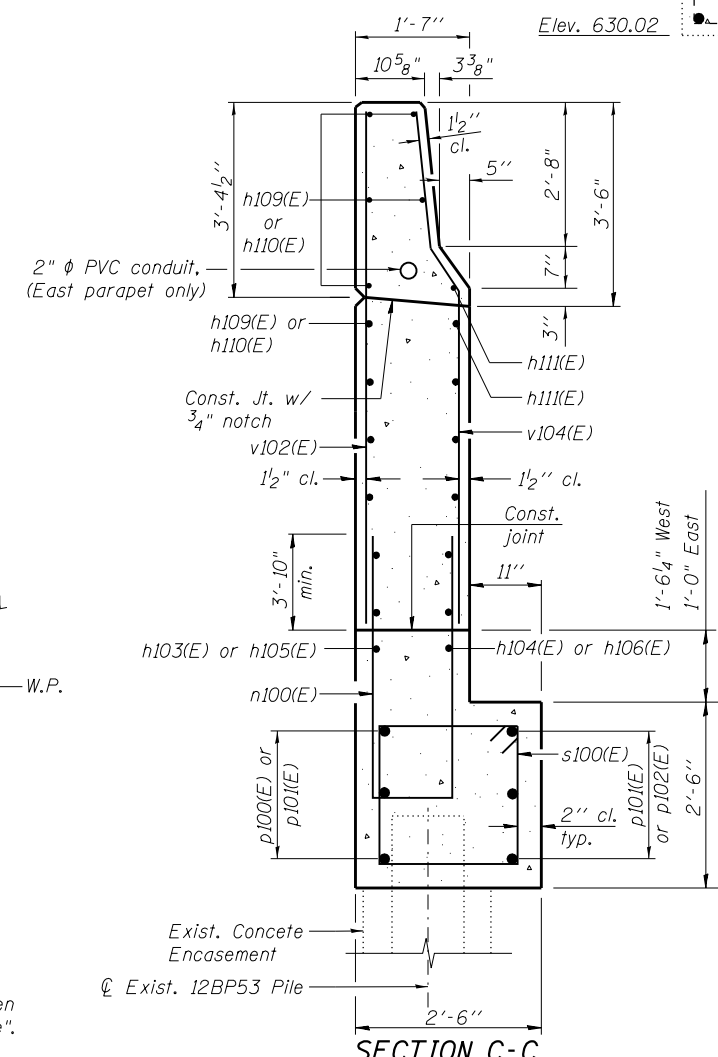
MINIMUM BAR LAP
#5 bar = 3'-3"
#6 bar = 3'-10"

Notes:
Existing reinforcement shall be cleaned and incorporated into the new construction. Cost included with "Concrete Removal".
Hatched area to be poured after superstructure false work has been removed. Quantity of concrete included with "Concrete Superstructure".
Concrete Sealer shall be applied to areas of newly placed concrete along the abutment backwall and cap.
For details of Bar Splicers, see sheet SB34.



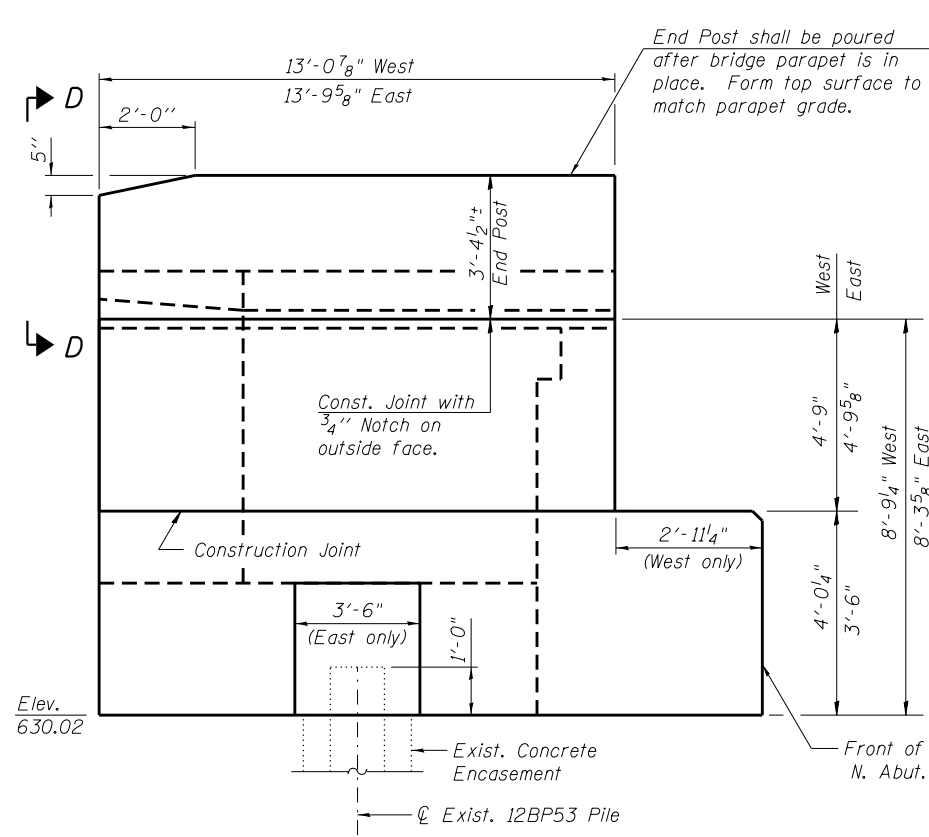
SEC. THRU N. ABUT.

* Cut to fit.
** Drill and grout 12" into existing abutment cap per Article 584 of the Standard Specifications. (Cost included with Concrete Structures). Cut to fit.



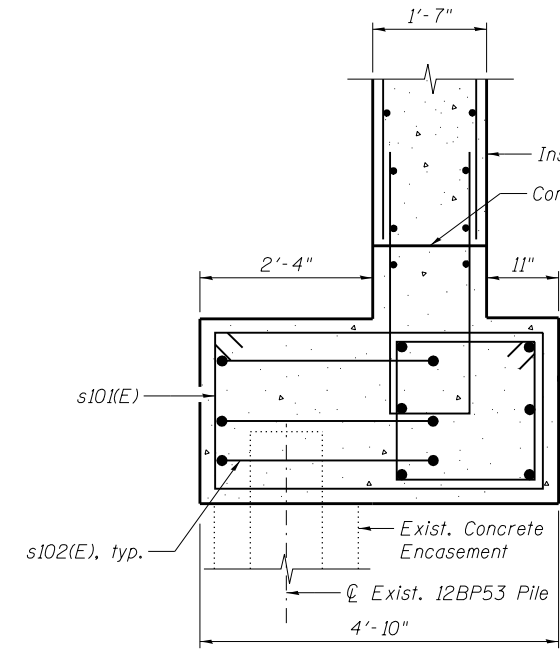
SECTION C-C
(East and West Wingwalls except as noted in Section C'-C')

(Sheet 2 of 3)



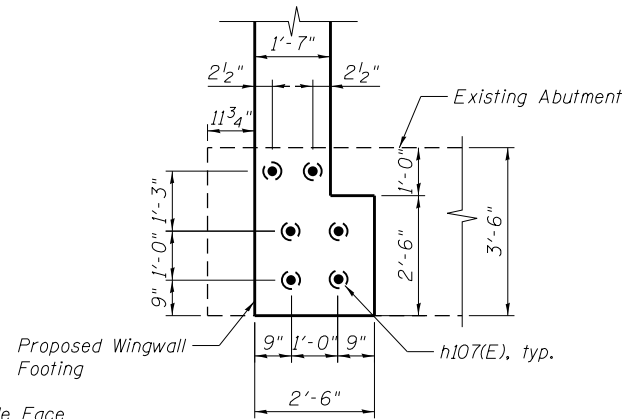
WINGWALL ELEVATION

Showing Dimensions
West Wingwall shown looking East
(East Wingwall opposite hand, Similar)



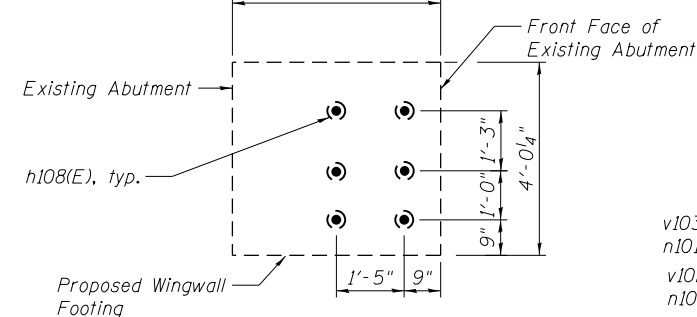
SECTION C'-C'

(East Wingwall at Exist. 12BP53 Pile)



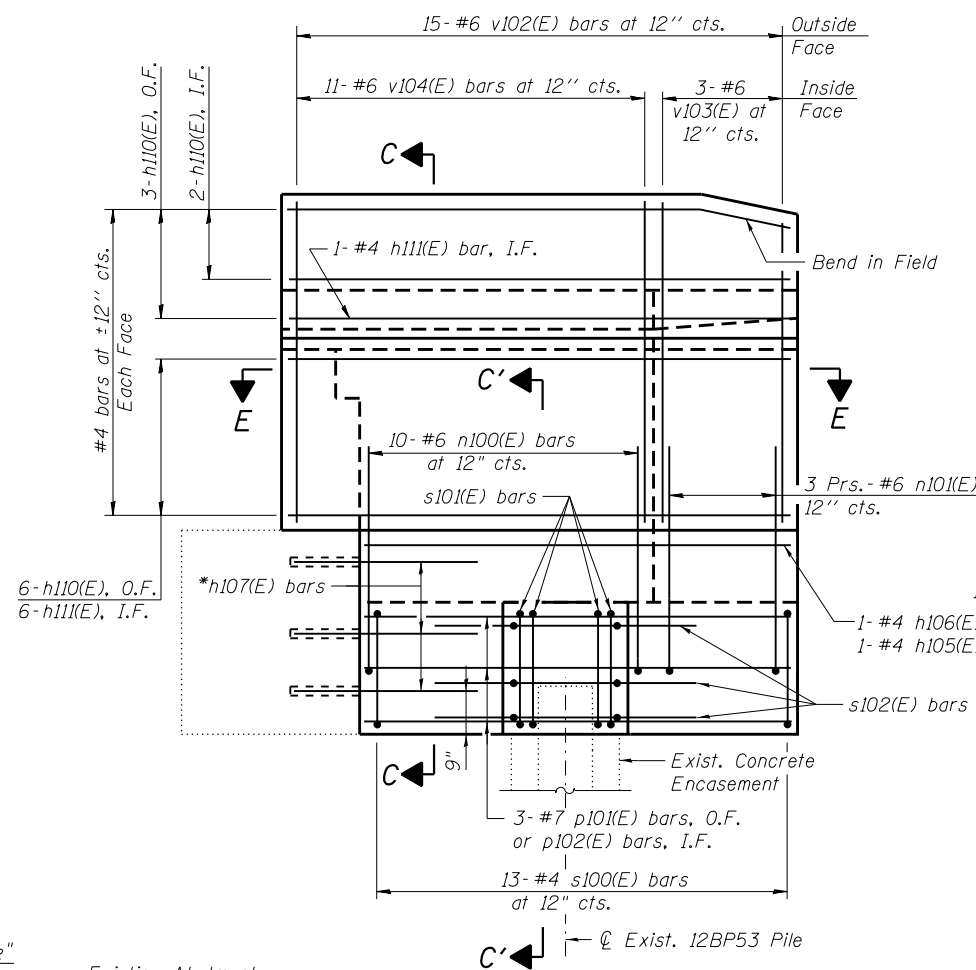
SECTION B-B

Proposed Wingwall Footing



SECTION A-A

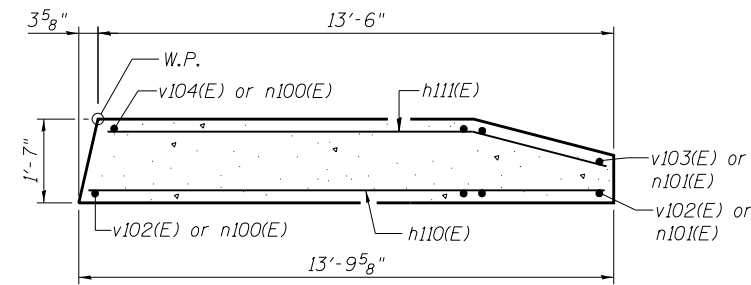
(Shown at rt. angles to existing abutment)



EAST WINGWALL ELEVATION

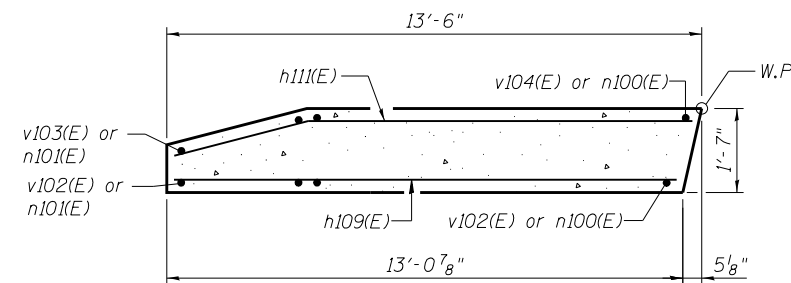
Showing Reinforcement
(Looking West)

* Drill and grout 12" into existing abutment cap per Article 584 of the Standard Specifications. (Cost included with Concrete Structures).



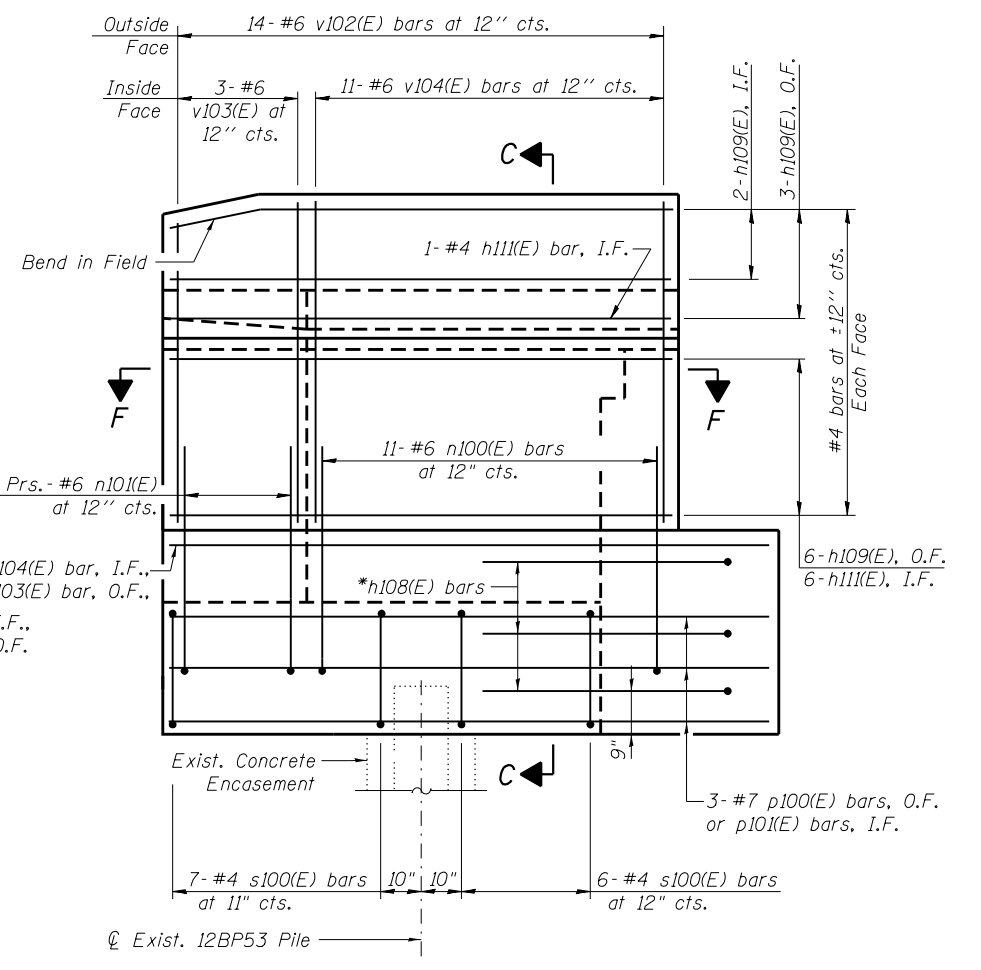
SECTION E-E

East Wingwall



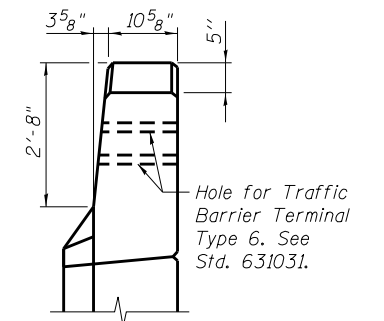
SECTION F-F

West Wingwall



WEST WINGWALL ELEVATION

Showing Reinforcement
(Looking East)

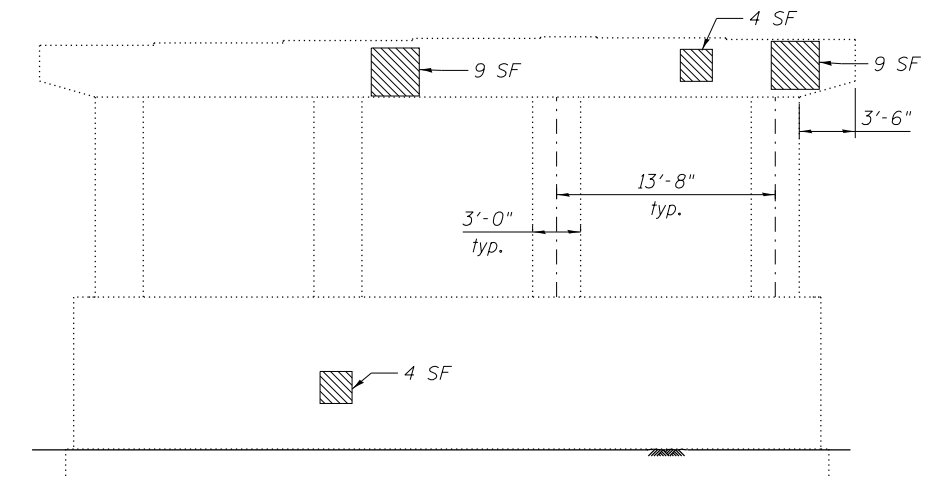
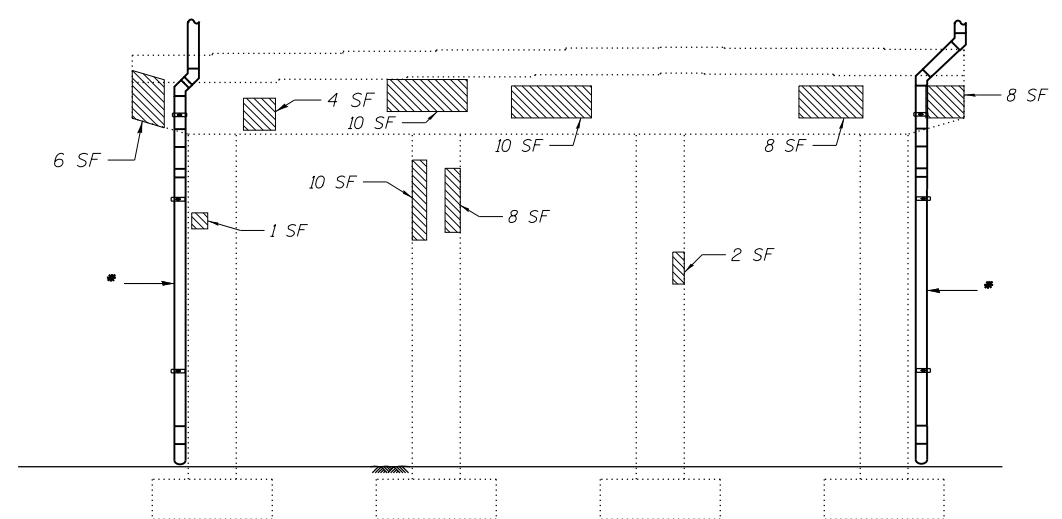
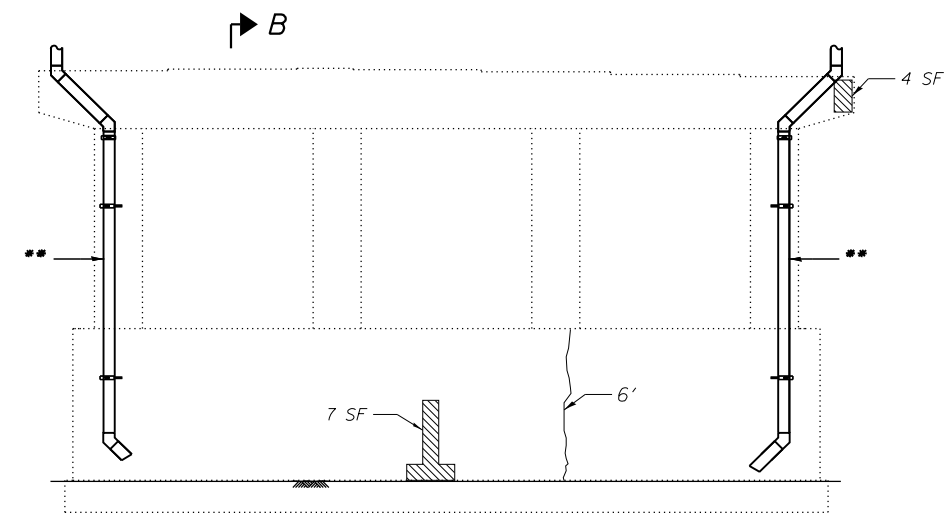
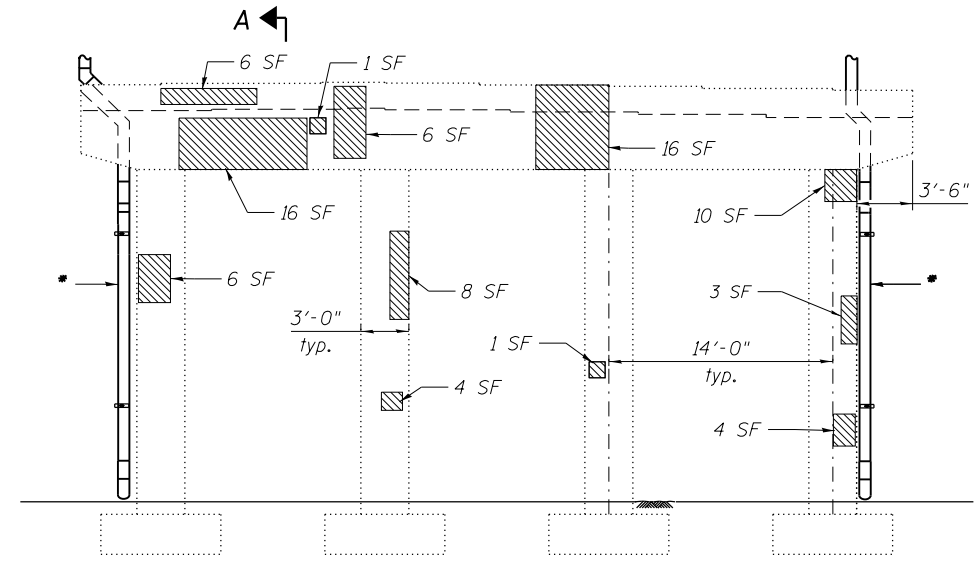
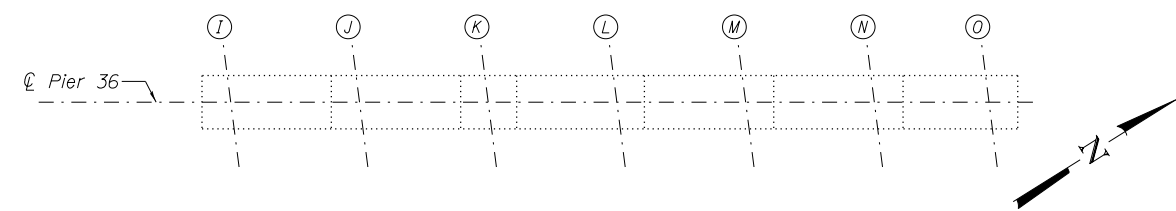
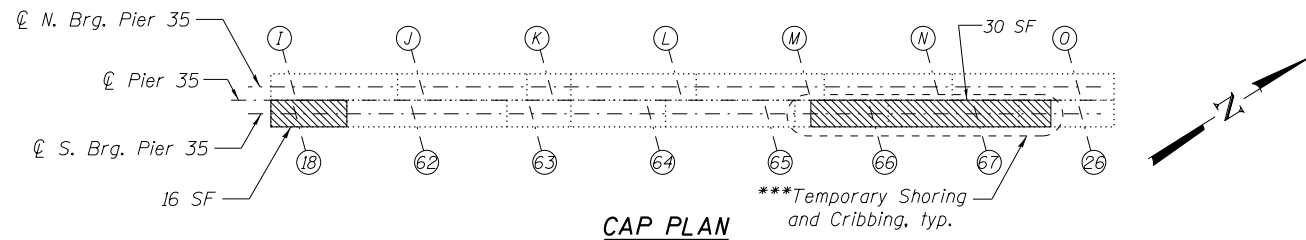


VIEW D-D

Notes:
Quantity of concrete in end post included with "Concrete Superstructure" on Sheet SB20.
Quantity of protective coat on end post included with "Protective Coat" on Sheet SB20.
See Std. 631031 for Traffic Barrier Terminal Type 6 Connection. See Sheet SB30 for Section C-C.

(Sheet 3 of 3)

<p>LIN ENGINEERING, LTD. Consulting Engineers Westmont, Illinois</p>	USER NAME = Lin20_Nick FILE NAME = ...8162454_68875_831_abutment.dwg PLOT SCALE = PLOT DATE = 6/9/2015 9:13:08 AM	DESIGNED - RGB CHECKED - JRS DRAWN - RMH CHECKED - JRS	REVISED - REVISED - REVISED - REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	NORTH ABUTMENT MODIFICATION DETAILS STRUCTURE NO. 016-2454	F.A.P. RTE. = 372 SECTION = 2013-037B-R COUNTY = COOK TOTAL SHEETS = 787 SHEET NO. = 295	CONTRACT NO. 60W75 ILLINOIS FED. AID PROJECT
	SHEET NO. SB31 OF SB34 SHEETS						



SECTION A-A

SECTION B-B

PIER 35
(Looking Upstation, Looking North)

PIER 36
(Looking Upstation, Looking North)

PIER 35
(Looking Downstation, Looking South)

PIER 36
(Looking Downstation, Looking South)

**** REACTION TABLE
(Service Load Per Beam)

LOCATION	BEAM DEAD LOAD(k)
Pier 35 N. Brg.	6
Pier 35 S. Brg.	8

LEGEND

- Structural Repair of Concrete (Depth equal to or less than 5 inches)
- * Existing closed drainage system to be removed & replaced. See Sheet SB24 for proposed drainage system details.
- ** Existing closed drainage system to be removed.
- 6' — } Crack (6 L.F.)

*** The installation of Temporary Shoring and Cribbing shall not commence until the deck has been removed entirely. Coordinate with the construction on Str. No. 016-2456.

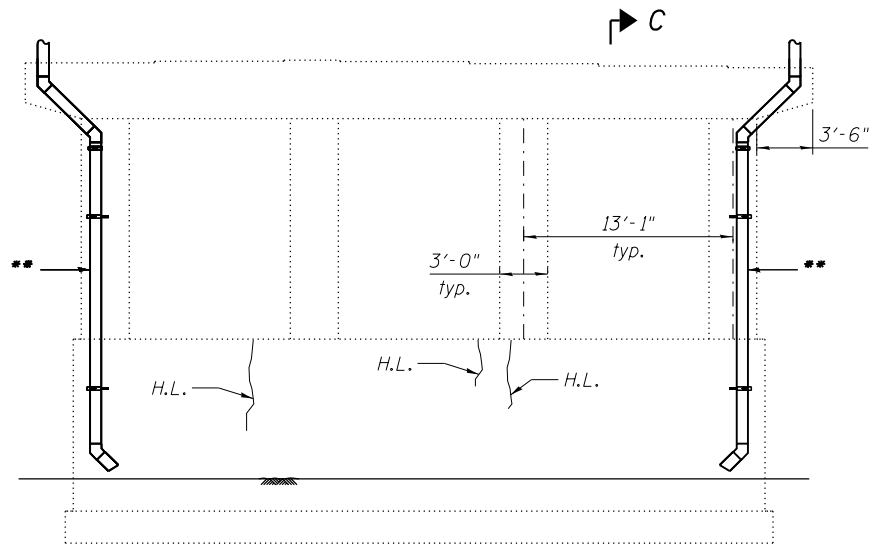
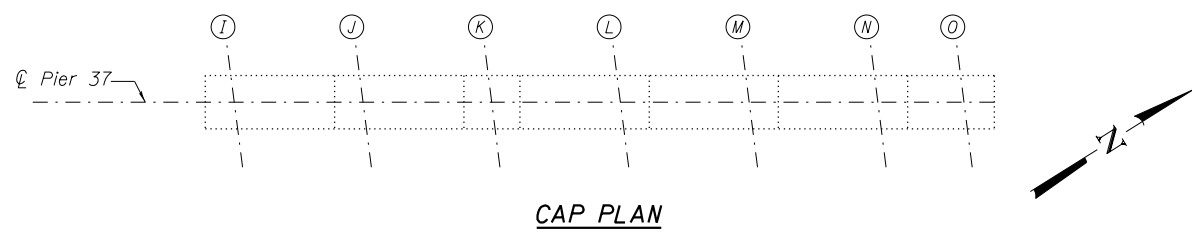
**** Reactions provided are for the steel beam dead load only.

BILL OF MATERIAL

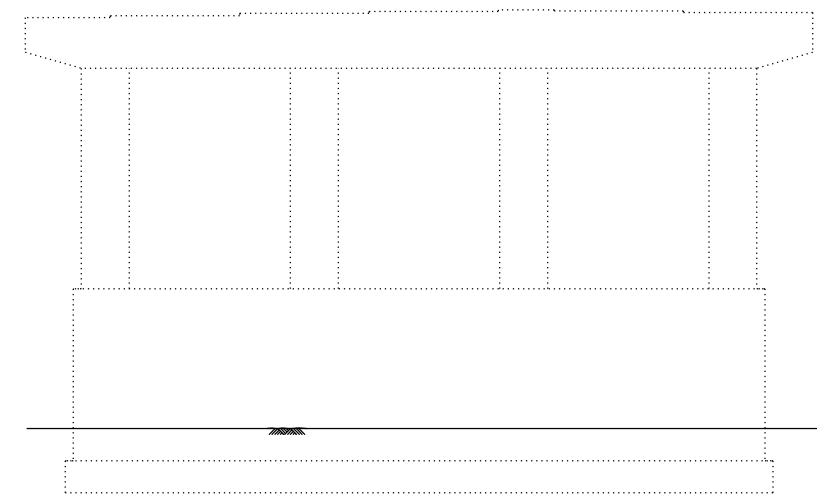
ITEM	UNIT	TOTAL
Structural Repair of Concrete (Depth equal to or less than 5 inches)	Sq. Ft.	231
Temporary Shoring and Cribbing	Each	3
Epoxy Crack Injection	Foot	6

Notes:
The existing closed drainage systems at Piers 35 and 36 shall be removed. The existing bolts in self drilling anchors shall be removed and the holes plugged. The plugged locations shall be coated with epoxy coating. Cost included with "Removal of Existing Concrete Deck No. 1". Replacement of existing closed drainage systems is included in the "Drainage System" pay item.

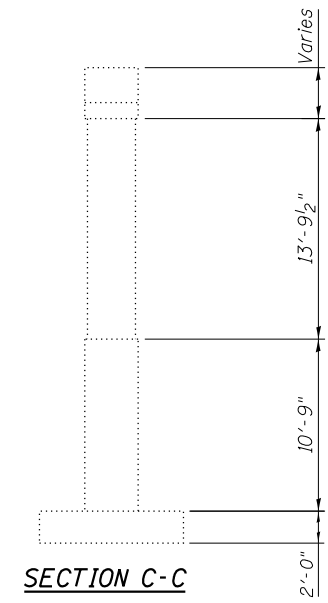
(Sheet 1 of 2)



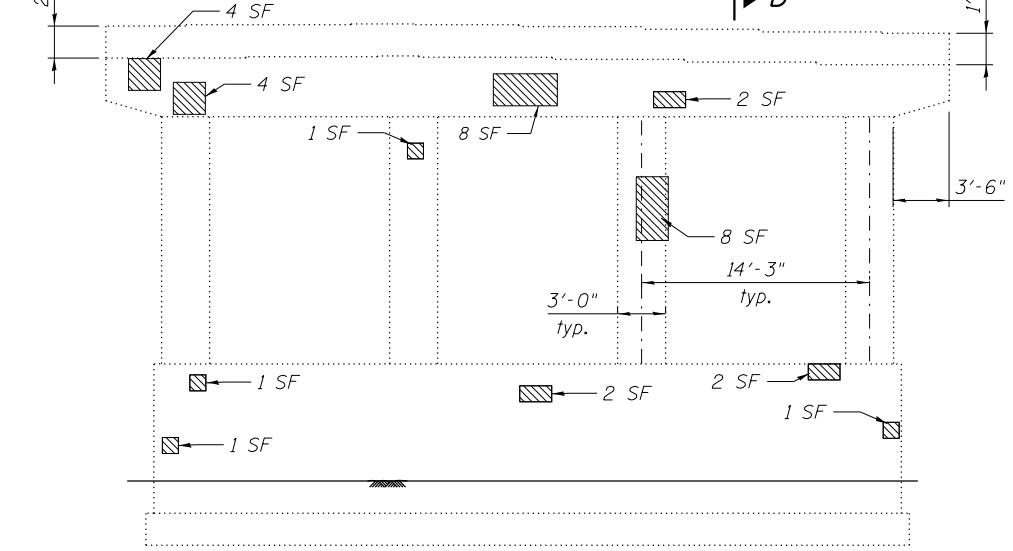
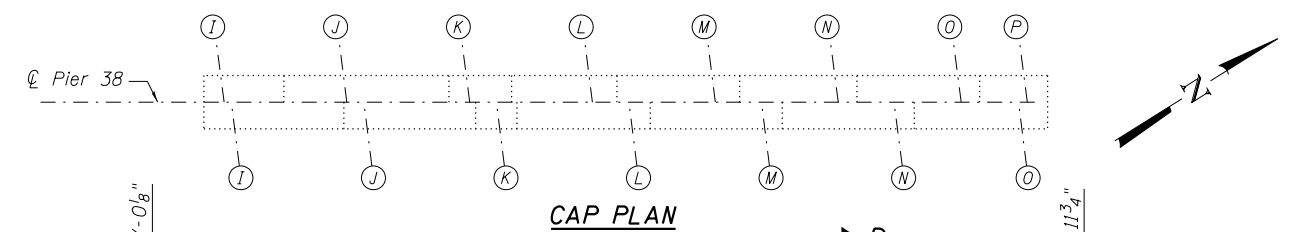
PIER 37
(Looking Upstation, Looking North)



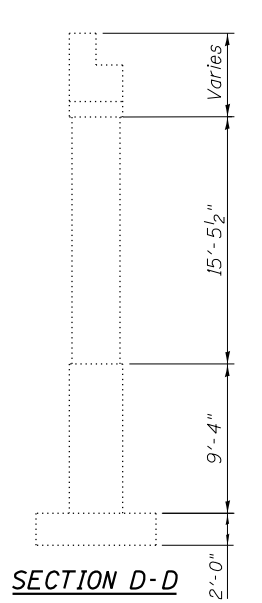
PIER 37
(Looking Downstation, Looking South)



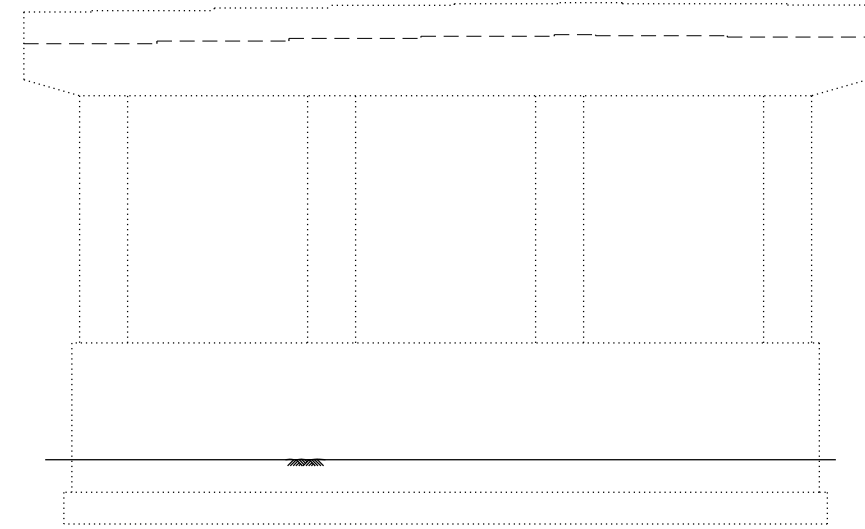
SECTION C-C



PIER 38
(Looking Upstation, Looking North)



SECTION D-D



PIER 38
(Looking Downstation, Looking South)

LEGEND

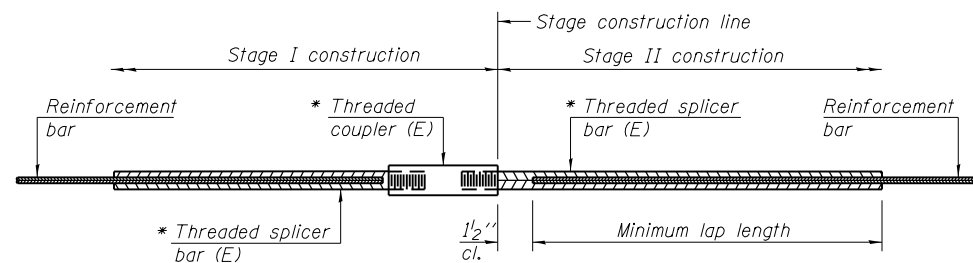
- Structural Repair of Concrete (Depth equal or less than 5 inches)
- Hairline Crack (Not to be repaired)
- **** Existing closed drainage system to be removed.

BILL OF MATERIAL

ITEM	UNIT	TOTAL
Structural Repair of Concrete (Depth equal to or less than 5 inches)	Sq. Ft.	34

Note:
The existing closed drainage systems at Pier 37 shall be removed. The existing bolts in self drilling anchors shall be removed and the holes plugged. The plugged locations shall be coated with epoxy coating. Cost included with "Removal of Existing Concrete Deck No. 1".

(Sheet 2 of 2)



STANDARD BAR SPLICER ASSEMBLY

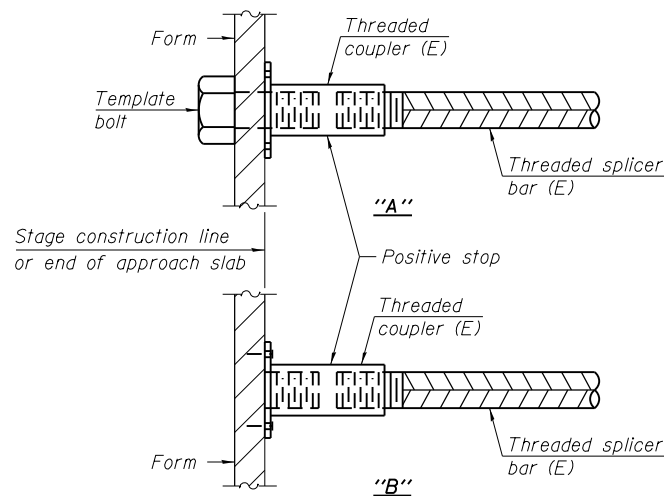
Minimum Lap Lengths						
Bar size to be spliced	Table 1	Table 2	Table 3	Table 4	Table 5	Table 6
3, 4	1'-5"	1'-11"	2'-1"	2'-4"	2'-7"	2'-11"
5	1'-9"	2'-5"	2'-7"	2'-11"	3'-3"	3'-8"
6	2'-1"	2'-11"	3'-1"	3'-6"	3'-10"	4'-5"
7	2'-9"	3'-10"	4'-2"	4'-8"	5'-2"	5'-10"
8	3'-8"	5'-1"	5'-5"	6'-2"	6'-9"	7'-8"
9	4'-7"	6'-5"	6'-10"	7'-9"	8'-7"	9'-8"

- Table 1: Black bar, 0.8 Class C
- Table 2: Black bar, Top bar lap, 0.8 Class C
- Table 3: Epoxy bar, 0.8 Class C
- Table 4: Epoxy bar, Top bar lap, 0.8 Class C
- Table 5: Epoxy bar, Class C
- Table 6: Epoxy bar, Top bar top, Class C

Threaded splicer bar length = min. lap length + 1 1/2" + thread length

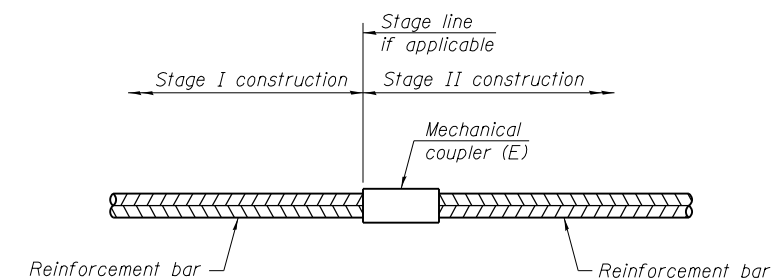
* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

Location	Bar size	No. assemblies required	Table for minimum lap length



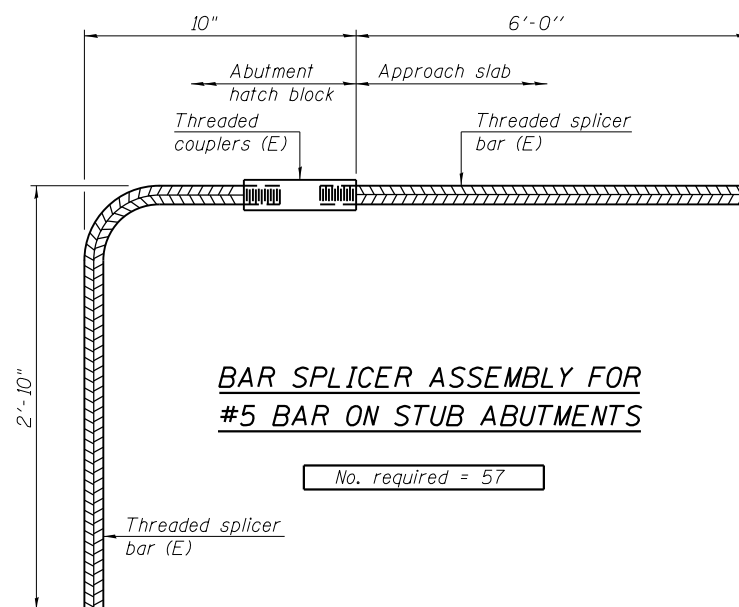
INSTALLATION AND SETTING METHODS

"A" : Set bar splicer assembly by means of a template bolt.
 "B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms.
 (E) : Indicates epoxy coating.



STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required



BAR SPLICER ASSEMBLY FOR #5 BAR ON STUB ABUTMENTS

No. required = 57

NOTES

Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.
 All reinforcement shall be lapped and tied to the splicer bars.
 Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications.
 See approved list of bar splicer assemblies and mechanical splicers for alternatives.

BSD-1

8-31-12



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 DRAWN - RMH
 CHECKED - JRS

REVISED -
 REVISED -
 REVISED -
 REVISED -

STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

BAR SPLICER ASSEMBLY DETAILS
 STRUCTURE NO. 016-2454

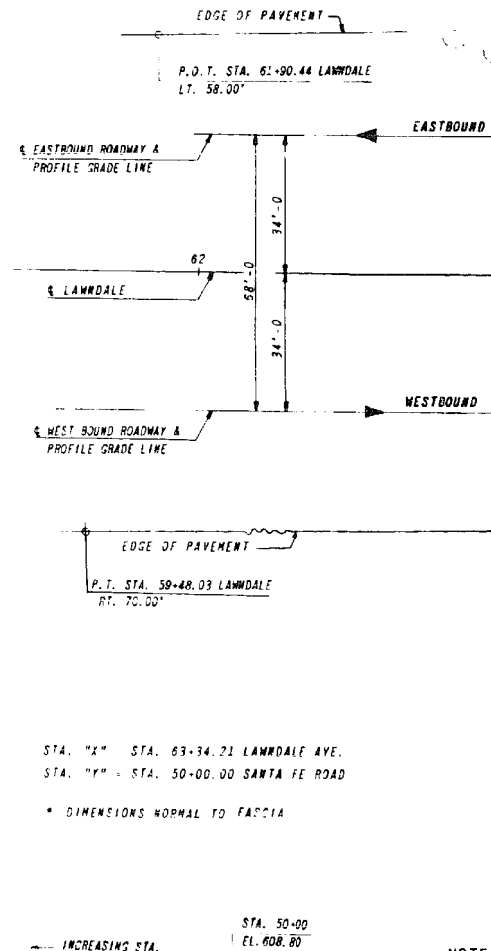
SHEET NO. SB34 OF SB34 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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CONTRACT NO. 60W75				ILLINOIS FED. AID PROJECT

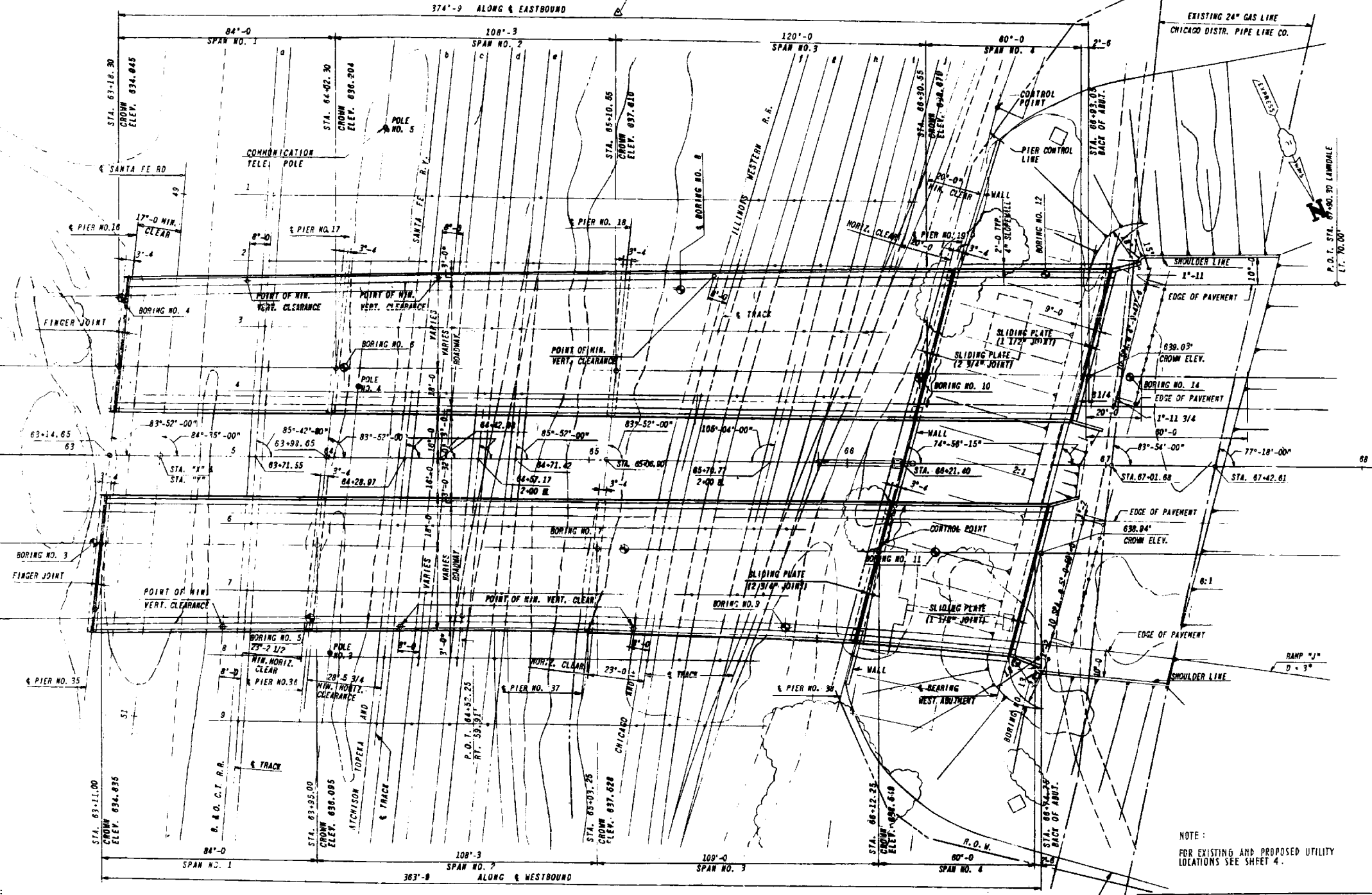
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133	0606-627 VB	SOUTHWEST	62	5
STA.	TO STA.			
FED. ROAD DIST. NO. 3	ILLINOIS	FED. AID PROJECT		

ELEVATIONS TOP OF HIGH RAIL (ADD 600 TO ALL ELEV.)									
ROW TRACK	1	2	3	4	5	6	7	8	9
a	6.99	6.88	6.86	6.84	6.58	6.68	6.50	6.58	6.82
b	6.85	6.86	6.89	6.93	6.90	6.84	6.96	6.96	7.01
c	7.25	7.27	7.29	7.30	7.37	7.36	7.37	7.39	7.40
d	7.41	7.40	7.41	7.41	7.43	7.44	7.45	7.48	7.53
e	6.99	7.11	7.20	7.29	7.27	7.18	7.12	7.06	7.00
f	6.83	6.85	6.82	6.83	6.85	6.85	6.81	6.83	6.79
g	6.81	6.84	6.86	6.86	6.85	6.89	6.90	6.91	6.89
h	6.93	6.92	6.93	6.91	6.85	6.86	6.80	6.83	6.85
i	6.87	5.81	6.85	6.80	6.80	6.82	6.75	6.81	-
j	6.98	6.92	6.88	6.82	-	-	-	-	-

NOTE: LOCATION OF EXISTING HIGH RAIL ELEVATIONS ARE INDICATED BY POINT.



NOTE: FOR FALSEWORK AT THE A.T. & S.T. RY. SYSTEM SEE SHEET "PAVEMENT SECTION".



PLAN

NOTE: FOR EXISTING AND PROPOSED UTILITY LOCATIONS SEE SHEET 4.

REVISIONS		ILLINOIS DIVISION OF HIGHWAYS SOUTHWEST EXPRESSWAY F.A. RT. 133 LAWDALE AVE. STRUCTURE OVER A.T. & S.F. RY. C.&I.W. R.R. & B.&O.C.T. R.R. SECTION 0606-627 VB GENERAL PLAN
NAME	DATE	
DESIGNED ST.		SCALE: HORIZ. 1"=20' VERT. DATE 11-20-63 DRAWN BY E.G. CHECKED BY I.D.B.
REVIEWED CWM		

(Sheet 1 of 14)

FOR INFORMATION ONLY

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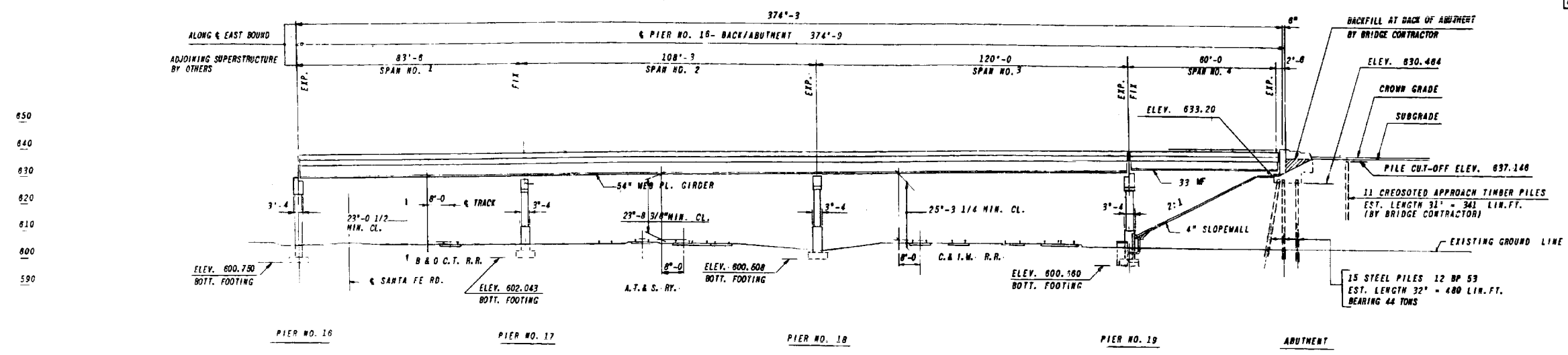
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

EXISTING PLANS (FOR INFORMATION ONLY)
STRUCTURE NO. 016-2454

SHEET NO. SBX1 OF SBX14 SHEETS

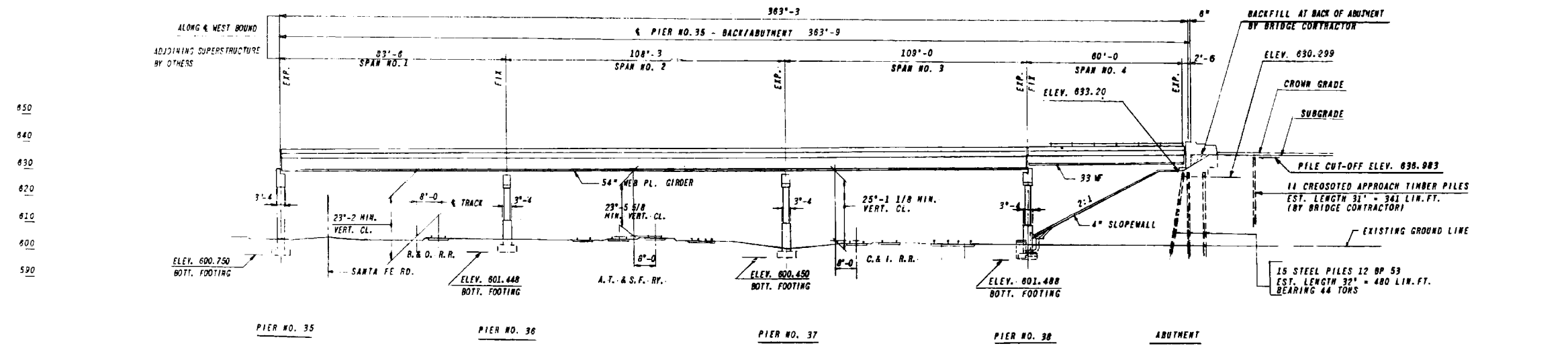
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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			CONTRACT NO. 60W75	
ILLINOIS FED. AID PROJECT				

F.A. RT.	SECTION	EXPRESSWAY	TOTAL SHEETS	SHEET NO.
133	0606-627 VB	SOUTHWEST	62	6
STA.		TO STA.		
PRO. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT	



ELEVATION EASTBOUND STRUCTURE

D. H. NO. 4
EL. 605.97
STA. 85+12
LT. 170'



ELEVATION WESTBOUND STRUCTURE

REVISIONS	
NAME	DATE
DESIGNED ST	
REVIEWED CWN	

ILLINOIS DIVISION OF HIGHWAYS	
-SOUTHWEST EXPRESSWAY	
F.A. RT. 133	
LAWDALE AVE. STRUCTURE OVER	
A.T. & S.F. RY., C.&I.W. R.R. & B.&O.C.T. R.R.	
SECTION 0606-627 VB	
ELEVATIONS	
SCALE: HORIZ. 1"=20'	DATE: 11-20-15
VERT. 1"=20'	DRAWN BY: E.G.
	CHECKED BY: J.D.P.

FOR INFORMATION ONLY

(Sheet 2 of 14)

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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

EXISTING PLANS (FOR INFORMATION ONLY)
STRUCTURE NO. 016-2454

SHEET NO. SBX2 OF SBX14 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
372	2013-037B-R	COOK	787	300
CONTRACT NO. 60W75				ILLINOIS FED. AID PROJECT