

INTERIOR GIRDER MOMENT TABLE

	0.4 Sp. 1 or 0.6 Sp. 4	Pier 1 or 3	0.5 Sp. 2 or 3	Pier 2	0.4 Sp. 5 or 0.6 Sp. 7	Pier 5 or 6	0.5 Sp. 6	
I _s	(in ⁴)	15,424	18,604	15,424	18,604	26,796	44,318	30,277
I _c (n)	(in ⁴)	33,605	39,672	33,605	39,672	56,071	83,808	61,025
I _c (3n)	(in ⁴)	25,039	29,385	25,039	29,385	41,852	62,985	45,776
I _c (cr)	(in ⁴)	-	22,217	-	22,217	-	50,269	-
S _s	(in ³)	709	882	709	882	966	1,652	1,086
S _c (n)	(in ³)	928	1,126	928	1,126	1,256	1,999	1,381
S _c (3n)	(in ³)	851	1,036	851	1,036	1,148	1,853	1,267
S _c (cr)	(in ³)	-	943	-	943	-	1,726	-
DC1	(k/')	0.811	0.838	0.811	0.838	0.836	0.923	0.853
M _{DC1}	(k)	586.4	1,053.9	422.5	952.0	758.6	1,948.7	828.2
DC2	(k/')	0.150	0.150	0.150	0.150	0.150	0.150	0.150
M _{DC2}	(k)	108.9	191.8	78.8	173.9	139.0	331.6	148.3
DW	(k/')	0.267	0.267	0.267	0.267	0.267	0.267	0.267
M _{DW}	(k)	193.6	341.0	140.1	309.2	247.0	589.5	263.6
M _{ℓ + IM}	(k)	1,103.5	1,228.0	1,014.9	1,226.6	1,419.6	1,827.5	1,429.7
M _u (Strength I)	(k)	3,090.7	4,217.4	2,612.8	4,017.8	3,976.8	6,932.7	4,117.9
Φ _r M _n	(k)	4,682.2	4,324.3	4,729.4	4,336.5	6,366.8	7,006.9	7,046.9
f _s DC1	(ksi)	9.93	14.34	7.15	12.95	9.42	14.15	9.15
f _s DC2	(ksi)	1.54	2.44	1.11	2.21	1.45	2.31	1.40
f _s DW	(ksi)	2.73	4.34	1.98	3.93	2.58	4.10	2.50
f _s (ℓ + IM)	(ksi)	14.27	15.63	13.12	15.61	13.56	12.71	12.42
f _s (Service II)	(ksi)	32.75	41.44	27.30	39.38	31.08	37.08	29.20
0.95R _n F _{yf}	(ksi)	47.50	47.50	47.50	47.50	47.50	47.50	47.50
f _s (Total)(Strength I)	(ksi)	-	-	-	-	-	-	-
Φ _r F _n	(ksi)	-	-	-	-	-	-	-
V _f	(k)	31.4	29.5	22.7	29.4	33.1	30.6	23.8

- I_s, S_s: Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total-Strength I, and Service II) due to non-composite dead loads (in⁴ and in³).
- I_c(n), S_c(n): Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections due to short-term composite live loads (in⁴ and in³).
- I_c(3n), S_c(3n): Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections, due to long-term composite (superimposed) dead loads (in⁴ and in³).
- I_c(cr), S_c(cr): Composite moment of inertia and section modulus of the steel and longitudinal deck reinforcement, used for computing f_s (Total-Strength I and Service II) in cracked sections, due to both short-term composite live loads and long-term composite (superimposed) dead loads (in⁴ and in³).
- DC1: Un-factored non-composite dead load (kips/ft.).
- M_{DC1}: Un-factored moment due to non-composite dead load (kip-ft.).
- DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).
- M_{DC2}: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).
- DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).
- M_{DW}: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).
- M_{ℓ + IM}: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).
- M_u (Strength I): Factored design moment (kip-ft.).
1.25 (M_{DC1} + M_{DC2}) + 1.5 M_{DW} + 1.75 M_{ℓ + IM}

- Φ_rM_n: Compact composite positive moment capacity computed according to Article 6.10.7.1 or non-slender negative moment capacity according to Article A6.1.1 or A6.1.2 (kip-ft).
- f_s DC1: Un-factored stress at edge of flange for controlling steel flange due to vertical non-composite dead loads as calculated below (ksi).
M_{DC1} / S_{nc}
- f_s DC2: Un-factored stress at edge of flange for controlling steel flange due to vertical composite dead loads as calculated below (ksi).
M_{DC2} / S_c(3n) or M_{DC2} / S_c(cr) as applicable.
- f_s DW: Un-factored stress at edge of flange for controlling steel flange due to vertical composite future wearing surface loads as calculated below (ksi).
M_{DW} / S_c(3n) or M_{DW} / S_c(cr) as applicable.
- f_s (ℓ + IM): Un-factored stress at edge of flange for controlling steel flange due to vertical composite live load plus impact loads as calculated below (ksi).
M_{ℓ + IM} / S_c(n) or M_{DW} / S_c(cr) as applicable.
- f_s (Service II): Sum of stresses as computed below (ksi).
f_sDC1 + f_sDC2 + f_sDW + 1.3 f_s(ℓ + IM)
- 0.95R_nF_{yf}: Composite stress capacity for Service II loading according to Article 6.10.4.2 (ksi).
- f_s (Total)(Strength I): Sum of stresses as computed below on non-compact section (ksi).
1.25 (f_sDC1 + f_sDC2) + 1.5 f_sDW + 1.75 f_s(ℓ + IM)
- Φ_rF_n: Non-Compact composite positive or negative stress capacity for Strength I loading according to Article 6.10.7 or 6.10.8 (ksi).
- V_f: Maximum factored shear range in span computed according to Article 6.10.10.

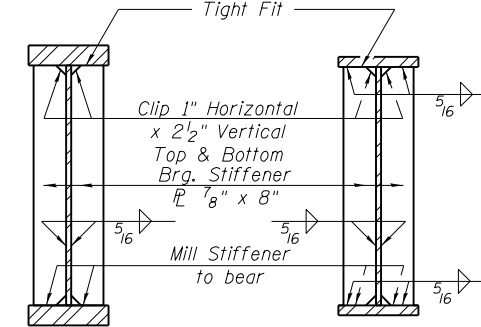
INTERIOR GIRDER REACTION TABLE

	S. Abut.	Pier 1 or 3	Pier 2	S. Pier 4	N. Pier 4	Pier 5 or 6	N. Abut.	
R _{DC1}	(k)	30.9	101.7	95.6	32.4	37.3	140.7	35.8
R _{DC2}	(k)	5.7	18.5	17.4	6.0	6.8	23.9	6.5
R _{DW}	(k)	10.2	32.9	31.0	10.7	12.0	42.5	11.5
R _{ℓ + IM}	(k)	88.1	140.8	140.8	107.2	112.9	163.2	92.5
R _{Total}	(k)	134.9	293.9	284.8	156.3	169.0	370.3	146.3

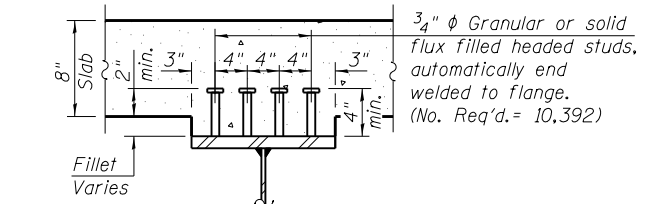
TOP OF WEB ELEVATIONS*

	Girder #1	Girder #2	Girder #3	Girder #4	Girder #5	Girder #6
℄ Brg. S. Abut.	559.55	559.63	559.69	559.67	559.56	559.43
℄ Brg. Pier 1	560.02	560.10	560.16	560.13	560.01	559.88
℄ Splice 1	560.09	560.17	560.23	560.20	560.08	559.95
℄ Brg. Pier 2	560.63	560.71	560.78	560.76	560.64	560.52
℄ Splice 2	560.76	560.84	560.91	560.89	560.78	560.65
℄ Brg. Pier 3	561.02	561.11	561.19	561.18	561.08	560.97
℄ Splice 3	561.05	561.15	561.23	561.23	561.13	561.02
℄ S. Brg. Pier 4	561.10	561.21	561.30	561.30	561.21	561.11
℄ N. Brg. Pier 4	561.10	561.20	561.30	561.30	561.21	561.11
℄ Splice 4	560.88	560.99	561.10	561.11	561.03	560.94
℄ Brg. Pier 5	560.77	560.89	561.00	561.02	560.94	560.86
℄ Splice 5	560.67	560.79	560.90	560.92	560.86	560.77
℄ Splice 6	560.16	560.28	560.40	560.42	560.35	560.27
℄ Brg. Pier 6	560.00	560.13	560.24	560.26	560.20	560.12
℄ Splice 7	559.85	559.97	560.09	560.11	560.04	559.96
℄ Brg. N. Abut.	559.43	559.56	559.67	559.70	559.63	559.55

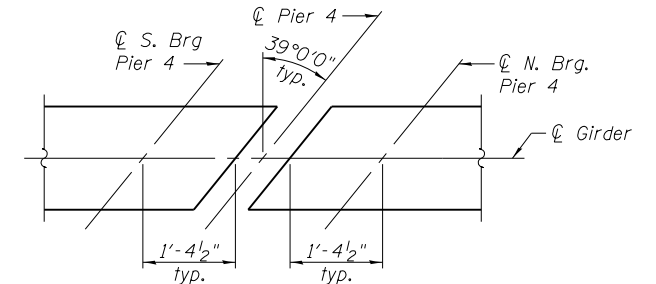
*For fabrication only.



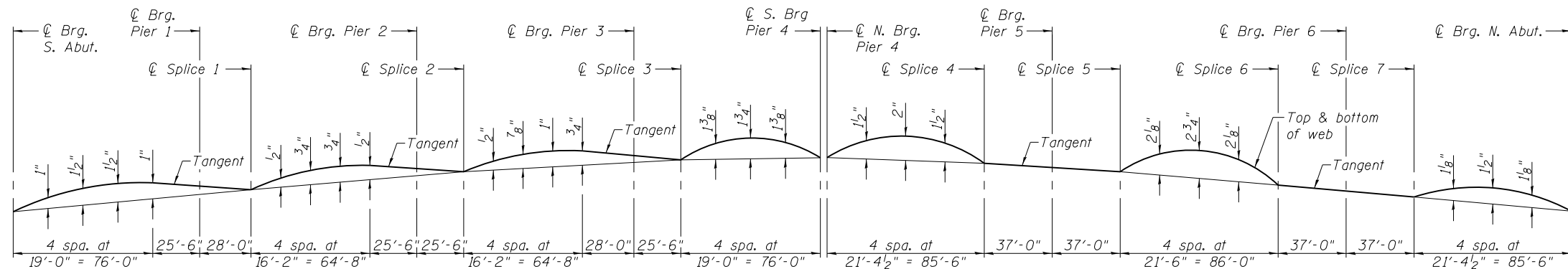
SECTION AT PIERS 1, 2, 3, 5 & 6



SECTION A-A



GIRDER END TREATMENT DETAIL



CAMBER DIAGRAM