

F.A.P. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
885	(107A)B-1	JOHNSON	38	1

CONTRACT NO. 98776

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

PROPOSED HIGHWAY PLANS

F.A.P. 885 (ILLINOIS 146)

SECTION (107A)B-1

PROJECT NO. ACBRF-0885 (036)

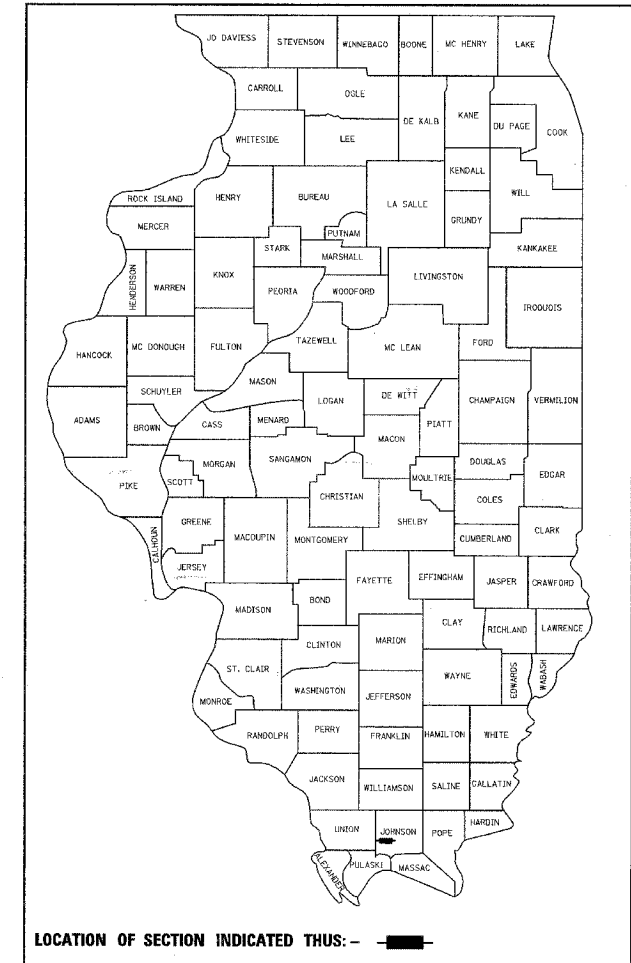
JOHNSON COUNTY

C-99-064-02

STRUCTURE REPLACEMENT OVER BUCK RUN CREEK

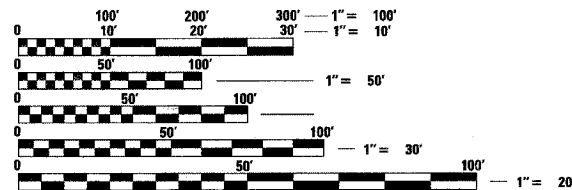
FOR INDEX OF SHEETS, SEE SHEET NO. 2
FOR SUMMARY OF QUANTITIES, SEE SHEETS NO. 3 AND 4

D-99-036-03



TRAFFIC DATA

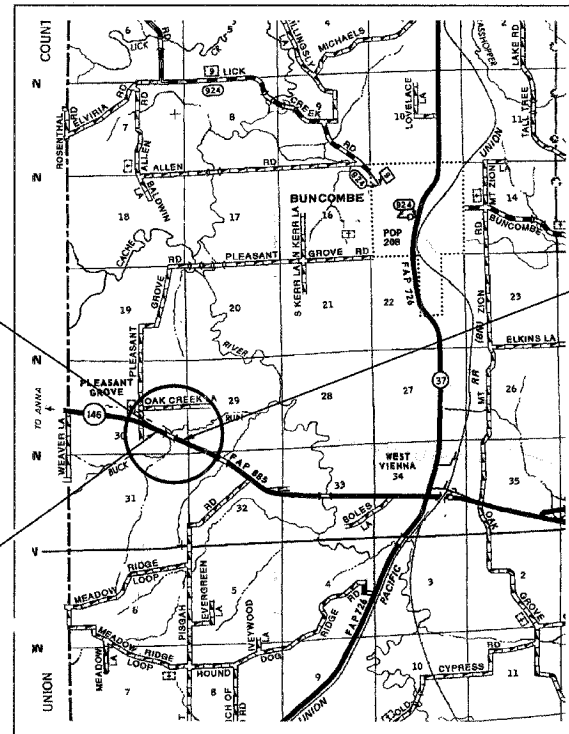
2006 ADT = 2420
20% TRUCKS
POSTED SPEED 55 MPH



PROPOSED BRIDGE
OVER BUCK RUN CREEK (1.2 MILES EAST
OF JOHNSON/UNION COUNTY LINE)
STRUCTURE NO. 044-0059
SINGLE SPAN 48" WEB PLATE GIRDER,
INTEGRAL ABUTMENT BRIDGE;
92'-0" BK TO BK ABUTMENTS; 36' BRIDGE
CLEAR WIDTH; 10° SKEW
CL STRUCTURE STA 628+25

PROPOSED PROJECT ENDS
STA 631+20

PROPOSED PROJECT BEGINS
STA 625+30



ELVIRA TOWNSHIP

FULL SIZE PLANS HAVE BEEN PREPARED USING STANDARD
ENGINEERING SCALES, REDUCED SIZED PLANS WILL NOT
CONFORM TO STANDARD SCALES, IN MAKING MEASUREMENTS
ON REDUCED PLANS, THE ABOVE SCALES MAY BE USED.

J.U.L.I.E.
JOINT UTILITY LOCATION INFORMATION FOR EXCAVATION
1-800-892-0123 WEBSITE: WWW.JULIE1CALL.COM

CONTRACT NO. 98776

ROADWAY LENGTH = 498 FT
BRIDGE LENGTH = 92 FT
NET LENGTH OF PROJECT = 590 FT
GROSS LENGTH OF PROJECT = 590 FT

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

SUBMITTED Aug 9 2007
Mark C. Lami
DEPUTY DIRECTOR OF HIGHWAYS, REGION ENGINEER

October 12, 2007
Eric S. Hamrick
ENGINEER OF DESIGN AND ENVIRONMENT

October 12, 2007
Milton R. Seer, P.E.
DIRECTOR OF HIGHWAYS, CHIEF ENGINEER

**PRINTED BY THE AUTHORITY
OF THE STATE OF ILLINOIS**

PLOT DATE = 7/30/2007
FILE NAME = c:\pro\pmts\98776\014-2013.dgn
PLOT SCALE = 50.0000 / IN.
USER NAME = halsteadw
PROJECT ENGINEER: DAVID PICHE
DESIGNER: MIKE STEPHENSON

GENERAL NOTES

THE THICKNESS OF THE HOT MIX ASPHALT MIXTURE SHOWN ON THE PLANS IS THE NOMINAL THICKNESS. DEVIATIONS FROM THE NOMINAL THICKNESS WILL BE PERMITTED WHEN SUCH DEVIATIONS OCCUR DUE TO IRREGULARITIES IN THE EXISTING SURFACE OR BASE ON WHICH THE HOT MIX ASPHALT MIXTURE IS PLACED.

FACTORS USED FOR QUANTITY CALCULATIONS ARE AS FOLLOWS:

ALL HOT MIX ASPHALT:	2.016 TONS/CU. YD.
HOT MIX ASPHALT MATERIALS ON PAVEMENT:	0.09 GAL./SQ. YD.
AGGREGATE (PRIME COAT)	0.0015 TONS/SQ. YD.
ALL AGGREGATE:	2.05 TONS/CU. YD.
RIPRAP	1.50 TONS/CU YD

PLAN DIMENSIONS AND DETAILS RELATIVE TO THE EXISTING STRUCTURE HAVE BEEN TAKEN FROM EXISTING PLANS AND ARE SUBJECT TO NOMINAL CONSTRUCTION VARIATIONS. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO VERIFY SUCH DIMENSIONS AND DETAILS IN THE FIELD AND MAKE NECESSARY APPROVED ADJUSTMENTS PRIOR TO CONSTRUCTION OR ORDERING OF MATERIALS. SUCH VARIATIONS SHALL NOT BE A CAUSE FOR ADDITIONAL COMPENSATION FOR A CHANGE IN THE SCOPE OF THE WORK. THE CONTRACTOR, HOWEVER, WILL BE PAID FOR THE ACTUAL QUANTITY FURNISHED AT THE UNIT PRICE BID FOR THE WORK. CONSTRUCTION PLANS ARE AVAILABLE FOR REVIEW AT THE DISTRICT 9 OFFICE.

IN ADDITION TO THE REQUIREMENTS OF ARTICLE 107.16 THE CONTRACTOR SHALL PROTECT THE SURFACE OF ALL BRIDGE DECK AND BRIDGE APPROACH PAVEMENTS IN A MANNER SATISFACTORY TO THE ENGINEER BEFORE ANY EQUIPMENT IS ALLOWED TO CROSS THE STRUCTURE. PROTECTION SHALL BE PROVIDED FOR ALL EQUIPMENT AS DEFINED IN ARTICLE 101.17 REGARDLESS IF TRACK MOUNTED OR WHEELED.

AT ALL LOCATIONS WHERE HOT MIX ASPHALT OR CONCRETE PAVEMENT JOINS AN EXISTING HOT MIX ASPHALT OR CONCRETE PAVEMENT, A SAWED JOINT SHALL BE CONSTRUCTED. THE COST OF THIS JOINT SHALL BE INCLUDED IN THE TYPE OF PAVEMENT BEING CONSTRUCTED.

QUANTITIES SHOWN IN THE PLANS FOR BRIDGE DECK GROOVING AND PROTECTIVE COAT INCLUDE THE BRIDGE, THE BRIDGE APPROACH PAVEMENTS, AND THE BRIDGE APPROACH PAVEMENT CONNECTORS (PCC) SPECIAL.

PROTECTIVE COAT SHALL BE APPLIED TO THE BRIDGE, THE BRIDGE APPROACH PAVEMENTS, AND THE BRIDGE APPROACH PAVEMENT CONNECTORS (PCC) SPECIAL IN ACCORDANCE WITH ARTICLE 503.19 OF THE STANDARD SPECIFICATIONS. THE PROTECTIVE COAT SHALL BE APPLIED REGARDLESS OF THE CURING METHOD USED. THE RATE OF APPLICATION FOR EACH COAT ON SAW CUT GROOVED AREAS SHALL BE 25 SQUARE YARDS PER GALLON OF MIXTURE.

REMOVAL OF EXISTING PRECAST CONCRETE BRIDGE SLABS (34 SQ YD) AND 10" PCC PAVEMENT (34 SQ YD) ARE INCLUDED IN THE QUANTITY FOR PAVEMENT REMOVAL.

ALL OBSTRUCTIONS WHICH ARE WITHIN THE CLEAR ZONE SHOWN ON THE TYPICAL SECTION, AND ARE NOT SHIELDED BY THE PROPOSED GUARDRAIL, SHALL BE REMOVED BETWEEN STA. 625+30 AND STA. 631+20. TYPICAL OBSTRUCTIONS ARE HEADWALLS, FOUNDATIONS, EXT. WHICH PROJECT (4 IN.) OR MORE ABOVE THE GROUNDLINE; AND TREES WHICH WILL MATURE TO A DIAMETER OF 4 IN OR GREATER.

TREES SHALL BE PRESERVED THROUGHOUT THIS SECTION AS SHOWN ON THE PLANS AND AS DIRECTED BY THE ENGINEER. GENERALLY, TREES OUTSIDE THE CLEAR ZONE, AND WHICH DO NOT INTERFERE WITH CONSTRUCTION, SHALL NOT BE DISTURBED.

IT WILL BE THE CONTRACTOR'S RESPONSIBILITY TO REMOVE ANY DEBRIS OR DIRT CAUSED BY CONSTRUCTION ACTIVITY THAT COVERS THE NEW RIPRAP AND NO ADDITIONAL COMPENSATION WILL BE ALLOWED.

ALL TEMPORARY EROSION CONTROL MEASURES SHALL BE LEFT IN PLACE UNTIL REMOVAL IS REQUIRED TO CONSTRUCT FINAL GRADE LINES.

THE QUANTITY OF SHORT TERM PAVEMENT MARKING SHOWN IN THE PLANS IS BASED ON ONE APPLICATION EACH FOR THE BINDER COURSE AND THE SURFACE COURSE.

THE QUANTITY OF TEMPORARY PAVEMENT MARKING SHOWN IN THE PLANS IS BASED ON ONE APPLICATION FOR THE SURFACE COURSE.

PRIOR TO PLACEMENT OF THE FINAL PAVEMENT MARKINGS THE RESIDENT ENGINEER SHOULD CONTACT THE BUREAU OF OPERATIONS AND ARRANGE FOR INSPECTION AND APPROVAL OF THE PAVEMENT MARKING LAYOUT.

STATIONING OF THE PROPOSED SURFACE SHALL BE REQUIRED. STAMP STATIONING EVERY 300 FEET ON ALTERNATING SIDES OF THE PAVEMENT AND AS DIRECTED BY THE ENGINEER. THE STATION SYMBOL STAMPS USED SHALL BE 5 1/2 " TALL AND OF A DESIGN APPROVED BY THE ENGINEER. THE STAMPS SHALL BE FURNISHED BY THE CONTRACTOR AND REMAIN HIS/HER PROPERTY.

COST OF REMOVING HOT MIX ASPHALT BASE COURSE WIDENING, 10" USED FOR STAGE I TRAFFIC IS INCLUDED IN "PAVED SHOULDER REMOVAL-SQ YD."

ATTAINMENT OF PROPER CROWN SHALL BE FULLY ACCOMPLISHED WITH THE HOT MIX ASPHALT BINDER COURSE.

THE ADVANCE DETECTOR LOOPS ARE TYPICALLY LOCATED 300 FEET IN ADVANCE OF THE STOP BAR. THE BUREAU OF OPERATIONS SHOULD APPROVE THE LOOP LOCATIONS PRIOR TO INSTALLATION.

THE CENTERLINE PAVEMENT MARKING SHOULD BE REMOVED FROM THE STOP BAR TO THE SAND ATTENUATORS OR DRUMS. EDGE LINE PAVEMENT MARKING SHOULD BE REMOVED IF A 10 FOOT LANE WIDTH CANNOT BE MAINTAINED. TEMPORARY EDGE LINES SHOULD BE INSTALLED WHEN THE EDGE LINES ARE REMOVED.

VERTICAL PANELS SHOWN ON STANDARD 701321 WILL NOT BE REQUIRED ON THE STAGE II NEW BRIDGE PARAPET. THE BARRIER WALL REFLECTORS SHALL BE INSTALLED PRIOR TO OPENING TO TRAFFIC.

ANY TIME THE CONCRETE BARRIER IS NOT IN THE PROPER POSITION, FLAGGERS SHALL BE IN PLACE TO CONTROL TRAFFIC. THE TEMPORARY TRAFFIC SIGNALS SHALL BE SET TO FLASH ALL RED.

"NARROW BRIDGE" SIGNS WITH ADVISORY TAGS "10 FT-0 IN" SHALL BE ERECTED BETWEEN "ROAD CONSTRUCTION AHEAD" AND THE "SIGNAL AHEAD" SIGNS FOR STAGE 1 TRAFFIC.

TRIM EDGES OF EXISTING HOT MIX ASPHALT SURFACE FLUSH WITH EXISTING PAVEMENT PRIOR TO CONSTRUCTING NEW BASE COURSE WIDENING.

THE HOT MIX ASPHALT BASE COURSE WIDENING, 10" CONSTRUCTED IN PRE-STAGE 1 MAY BE INCORPORATED INTO THE FINAL HOT MIX ASPHALT SHOULDERS, 8" DURING STAGE II CONSTRUCTION IF APPROVED BY THE ENGINEER. SUCH CHANGE WILL NOT BE A CAUSE FOR ADDITIONAL COMPENSATION, BUT THE CONTRACTOR WILL BE PAID FOR THE ACTUAL QUANTITY FURNISHED AT THE UNIT PRICE BID FOR THE WORK.

COMMITMENTS: NONE AS OF AUGUST 17, 2007, REFER TO COMMITMENT FILE FOR ANY COMMITMENTS AFTER THIS DATE.

HIGHWAY STANDARDS

000001-04	STANDARD SYMBOLS, ABBREVIATIONS AND PATTERNS
280001-03	TEMPORARY EROSION CONTROL SYSTEMS
420001-06	PAVEMENT JOINTS
420401-05	BRIDGE APPROACH PAVEMENT
421001-01	REINFORCEMENT FOR CONTINUOUSLY REINFORCED PCC PAVEMENT
482001-01	BITUMINOUS SHOULDER ADJACENT TO FLEXIBLE PAVEMENT
515001-02	NAME PLATE FOR BRIDGES
542401	METAL END SECTION FOR PIPE CULVERTS
609006-03	BRIDGE APPROACH PAVEMENT (DRAIN DETAIL)
630001-07	STEEL PLATE BEAM GUARDRAIL
630201-04	PCC/BITUMINOUS STABILIZATION AT STEEL PLATE BEAM GUARDRAIL
631031-06	TRAFFIC BARRIER TERMINAL, TYPE 6
635011-01	REFLECTOR MARKER & MOUNTING DETAILS
701006-02	OFF-ROAD OPERATIONS, 2L 2W, 4.5 m (15') TO 600 mm (24") AWAY, FOR SPEEDS \geq 45 MPH
701201-02	LANE CLOSURE, 2L 2W, DAY ONLY, ON-ROAD TO 600 mm (24") OFF-ROAD, FOR SPEEDS \geq 45 MPH
701301-02	LANE CLOSURE, 2L 2W, SHORT TIME OPERATIONS, FOR SPEEDS \geq 45 MPH
701321-08	LANE CLOSURE, 2L, 2W, BRIDGE REPAIR WITH BARRIER
701326-02	LANE CLOSURE, 2L, 2W, PAVEMENT WIDENING, FOR SPEEDS \geq 45 MPH
702001-06	TRAFFIC CONTROL DEVICES
704001-03	TEMPORARY CONCRETE BARRIER
780001-01	TYPICAL PAVEMENT MARKINGS

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F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
885	(107A)B-1	JOHNSON	38	2
STA. 624+80		TO STA. 631+60		
FED. ROAD DIST. NO.		ILLINOIS	FED. AID PROJECT	
CONTRACT NO. 98776				

Prepared By:	<i>Joe Zlanich</i> DISTRICT STUDIES & PLANS ENGINEER
Examined By:	<i>James Travis Emery</i> DISTRICT LAND ACQUISITION ENGINEER
Examined By:	<i>Carrie Nelson</i> DISTRICT PROGRAM DEVELOPMENT ENGINEER
Examined By:	<i>Kevin Kammes</i> DISTRICT OPERATIONS ENGINEER
Examined By:	<i>Joseph Lewis</i> DISTRICT CONSTRUCTION ENGINEER
Examined By:	<i>Bruce W. Reeder</i> DISTRICT MATERIALS ENGINEER
Examined By:	<i>Jim Smith</i> DISTRICT PROJECT IMPLEMENTATION ENGINEER
Examined By:	<i>James H. Garton</i> ASSISTANT REGIONAL ENGINEER
Approved By:	<i>Mark C. Linn</i> DEPUTY DIRECTOR OF HIGHWAYS, REGION ENGINEER
DATE	Aug 9 2007

PLOT DATE = 8/1/2007
 FILE NAME = c:\p\o\ess\3923823\044-0013.dgn
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 USER NAME = bob@arad

SUMMARY OF QUANTITIES

F.A.P. RTG.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
885	(107A)B-1	JOHNSON	38	3
STA.		TO STA.		
FED. ROAD DIST. NO.		ILLINOIS	FED. AID PROJECT	
CONTRACT NO. 98776				

RURAL - JOHNSON COUNTY			
HBP FUNDING			
80% FEDERAL;		20% STATE	
CONSTRUCTION TYPE CODE X071-2A			
SN 044-0059			
CODE NUMBER	ITEM DESCRIPTION	UNIT	QUANTITY
20100110	TREE REMOVAL (6 TO 15 UNITS DIAMETER)	UNIT	2
20100210	TREE REMOVAL (OVER 15 UNITS DIAMETER)	UNIT	2
20200100	EARTH EXCAVATION	CU YD	95
20300100	CHANNEL EXCAVATION	CU YD	508
20400100	BORROW EXCAVATION	CU YD	1161
20700400	POROUS GRANULAR EMBANKMENT, SPECIAL	CU YD	229
25000350	SEEDING, CLASS 7	ACRE	0.4
25000400	NITROGEN FERTILIZER NUTRIENT	POUND	64
25000500	PHOSPHORUS FERTILIZER NUTRIENT	POUND	48
25000600	POTASSIUM FERTILIZER NUTRIENT	POUND	48
25000700	AGRICULTURAL GROUND LIMESTONE	TON	0.8
25001010	SEEDING, CLASS 2 (MODIFIED)	ACRE	0.4
25100115	MULCH, METHOD 2	ACRE	0.6
25100630	EROSION CONTROL BLANKET	SQ YD	1224
28000250	TEMPORARY EROSION CONTROL SEEDING	POUND	40
28000400	PERIMETER EROSION BARRIER	FOOT	1152
28100107	STONE RIPRAP, CLASS A4	SQ YD	39
28100109	STONE RIPRAP, CLASS A5	SQ YD	974
28200200	FILTER FABRIC	SQ YD	1013
35600716	HOT-MIX ASPHALT BASE COURSE WIDENING, 10"	SQ YD	288
40600100	<i>BITUMINOUS</i> MATERIALS (PRIME COAT)	GALLON	127
40600300	AGGREGATE (PRIME COAT)	TON	2
40600982	HOT-MIX ASPHALT SURFACE REMOVAL - BUTT JOINT	SQ YD	160
40600990	TEMPORARY RAMP	SQ YD	26
40603090	HOT - MIX ASPHALT BINDER COURSE, IL - 19.0, N90	TON	161
40603320	HOT - MIX ASPHALT SURFACE COURSE, MIX "C", N90	TON	103
42001165	BRIDGE APPROACH PAVEMENT	SQ YD	249
42001300	PROTECTIVE COAT	SQ YD	326
44000100	PAVEMENT REMOVAL	SQ YD	356
44001114	HOT-MIX ASPHALT SURFACE REMOVAL (ASBESTOS)	SQ YD	208
44004250	PAVED SHOULDER REMOVAL	SQ YD	124
48100700	AGGREGATE SHOULDERS, TYPE A, 8"	SQ YD	58
48203029	HOT - MIX ASPHALT SHOULDERS, 8"	SQ YD	566

RURAL - JOHNSON COUNTY			
HBP FUNDING			
80% FEDERAL;		20% STATE	
CONSTRUCTION TYPE CODE X071-2A			
SN 044-0059			
CODE NUMBER	ITEM DESCRIPTION	UNIT	QUANTITY
48203100	HOT - MIX ASPHALT SHOULDERS	TON	34
50100100	REMOVAL OF EXISTING STRUCTURES	EACH	1
50200100	STRUCTURE EXCAVATION	CU YD	261
50300100	FLOOR DRAINS	EACH	12
50300225	CONCRETE STRUCTURES	CU YD	29.8
50300255	CONCRETE SUPERSTRUCTURE	CU YD	141.4
50300260	BRIDGE DECK GROOVING	SQ YD	645
50300280	CONCRETE ENCASEMENT	CU YD	5.6
50300300	PROTECTIVE COAT	SQ YD	445
50500105	FURNISHING AND ERECTING STRUCTURAL STEEL	L SUM	1
50500505	STUD SHEAR CONNECTORS	EACH	1782
50800205	REINFORCEMENT BARS, EPOXY COATED	POUND	33800
50800515	BAR SPLICERS	EACH	421
51201600	FURNISHING STEEL PILES HP12X53	FOOT	632
51202305	DRIVING PILES	FOOT	632
51204650	PILE SHOES	EACH	16
51500100	NAME PLATES	EACH	1
52100520	ANCHOR BOLTS, 1"	EACH	24
54215547	METAL END SECTIONS 12"	EACH	2
59100100	GEOCOMPOSITE WALL DRAIN	SQ YD	111
60100945	PIPE DRAINS 12"	FOOT	44
60109580	PIPE UNDERDRAINS FOR STRUCTURES 4"	FOOT	128
60900240	TYPE C INLET BOX, STANDARD 609006	EACH	2
60900515	CONCRETE THRUST BLOCKS	EACH	2
* 63000000	STEEL PLATE BEAM GUARDRAIL, TYPE A	FOOT	375
* 63100085	TRAFFIC BARRIER TERMINAL, TYPE 6	EACH	4
* 63100167	TRAFFIC BARRIER TERMINAL, TYPE 1, SPECIAL (TANGENT)	EACH	3
63200310	GUARDRAIL REMOVAL	FOOT	484
66600105	FURNISHING AND ERECTING RIGHT OF WAY MARKERS	EACH	12
67000400	ENGINEER'S FIELD OFFICE, TYPE A	CAL MO	9
67100100	MOBILIZATION	L SUM	1

* SPECIALTY ITEMS

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 USER NAME = halstoadx

SCHEDULES OF QUANTITIES

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
B85	(107A)B-1	JOHNSON	38	6
STA.	TO STA.			
FED. ROAD DIST. NO.	ILLINOIS	FED. AID PROJECT		
CONTRACT NO. 98776				

SHOULDER SCHEDULE

LOCATION STATION TO STATION	HOT-MIX ASPHALT BASE COURSE WIDENING, 10"	HOT-MIX ASPHALT SHOULDERS, 8"	HOT-MIX ASPHALT SHOULDERS	AGGREGATE SHOULDERS TYPE A, 8"
	SQ YD	SQ YD	TONS	SQ YD
626+40 TO 627+78.4 LT	62			
628+70.9 TO 630+10 LT	62			
626+60 TO 627+39.7 RT	77			
629+10.3 TO 630+00 RT	87			
625+30 TO 627+75 LT		157		
628+67 TO 631+20 LT		209		
625+30 TO 627+82.2 RT		131		
628+74.2 TO 631+20 RT		69		
626+60 TO 627+39.7 RT			16	
629+10.3 TO 630+00 RT			18	
625+30 TO 626+20.5 LT				29
630+29.3 TO 631+20 RT				29
TOTALS	288	566	34	58

TEMPORARY CONCRETE BARRIER SCHEDULE

LOCATION STATION TO STATION	STAGE	TEMPORARY CONCRETE BARRIER FOOT	RELOCATE TEMPORARY CONCRETE BARRIER FOOT
626+12.5 TO 630+37.5	I	425	
626+37.5 TO 630+12.5	II		375
TOTALS		425	375

TREE REMOVAL SCHEDULE

DIAMETER	STA.	6 TO 5 UNITS	>15 UNITS
6"	627+00 RT	1 UNIT	
10"	628+10 RT	1 UNIT	
16"	625+97 LT		1 UNIT
41"	627+15 LT		1 UNIT
TOTALS		2	2

TERMINALS AND GUARDRAIL SCHEDULE

LOCATION STATION TO STATION	TRAFFIC BARRIER TERMINAL			TYPE 6 EACH	SPBGR TYPE A FOOT	GUARDRAIL REMOVAL FOOT	TERMINAL MARKER DIRECT APPLIED EACH	GUARDRAIL MARKER EACH	BARRIER WALL MARKER EACH
	TANGENT (TEMP.) EACH	TANGENT (RE-ERECT) EACH	TANGENT EACH						
627+39.0 TO 627+82.1 RT				1					
627+32.7 TO 627+75.8 LT				1					
628+67.8 TO 629+10.9 LT				1					
628+74.1 TO 629+17.2 RT				1					
626+01.5 TO 627+39.0 RT					137.5				
626+82.7 TO 627+32.7 LT					50				
629+10.9 TO 630+48.4 LT					137.5				
629+17.2 TO 629+67.2 RT					50				
625+51.5 TO 626+01.5 RT			1				1		
626+32.7 TO 626+82.7 LT		1							
630+48.4 TO 630+98.4 LT			1				1		
629+67.2 TO 630+17.2 RT			1				1		
627+53.4 TO 628+03.4 LT	1						1		
625+76.4 TO 627+78.4 RT						202			
627+38.4 TO 627+78.4 LT						40			
628+70.9 TO 630+72.9 LT						202			
628+70.9 TO 629+10.9 RT						40			
625+51.5 TO 627+82.2 RT								4	
626+32.7 TO 627+75.8 LT								3	
628+67.8 TO 630+98.5 LT								4	
628+74.2 TO 630+17.3 RT								3	
627+75.8 TO 628+67.8 LT									3
627+82.2 TO 628+74.2 RT									3
TOTALS	1	1	3	4	375	484	4	14	6

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SCHEDULES OF QUANTITIES

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
885	(107A)B-1	JOHNSON	38	7
STA.	TO STA.			
FED. ROAD DIST. NO.	ILLINOIS	FED. AID PROJECT		
CONTRACT NO. 98776				

EARTHWORK SCHEDULE

LOCATION STATION TO STATION	CHANNEL EXCAVATION (UNSUITABLE)	*EARTH EXCAVATION	SHRINKAGE FACTOR FOR EARTH EXCAVATION	EARTH EXCAVATION TO BE USED IN EMBANKMENT, ADJUSTED FOR SHRINKAGE	**EMBANKMENT	EARTHWORK BALANCE WASTE (+) OR SHORTAGE (-)	SHRINKAGE FACTOR FOR BORROW EXCAVATION	BORROW EXCAVATION
	CU YD	CU YD	%	CU YD	CU YD	CU YD	%	CU YD
625+10 TO 627+79		52	25	39	787	-748	25	-997
627+79 TO 628+71	508							
628+71 TO 631+40		43	25	32	155	-123	25	-164
TOTALS	508	95						-1161

*CUTS FROM CROSS SECTIONS
**FILLS FROM CROSS SECTIONS

PAVEMENT REMOVAL AND MISCELLANEOUS SCHEDULE

LOCATION STATION TO STATION	PAVEMENT REMOVAL	HOT-MIX ASPHALT SURFACE REMOVAL BUTT-JOINT	TEMPORARY RAMP	PAVED SHOULDER REMOVAL
	SQ YD	SQ YD	SQ YD	SQ YD
627+39.7 TO 627+98.3	177			
628+50.9 TO 629+10.3	179			
624+75 TO 625+05		80		
631+45 TO 631+75		80		
624+75 TO 624+80			13	
631+70 TO 631+75			13	
626+40 LT TO 627+78.4 LT				62
628+70.9 LT TO 630+10 LT				62
TOTALS	356	160	26	124

HOT-MIX ASPHALT PAVEMENT SCHEDULE

LOCATION STATION TO STATION	HOT-MIX ASPHALT BINDER COURSE IL-19.0, N90	HOT-MIX ASPHALT SURFACE COURSE MIX C, N90	HOT-MIX ASPHALT MATERIALS (PRIME COAT)	AGGREGATE (PRIME COAT)
	TON	TON	GALLON	TON
625+30 TO 627+39.7	82			
629+10.3 TO 631+20	79			
624+75 TO 627+39.7		51.5	63.5	1
629+10.3 TO 631+75		51.5	63.5	1
TOTAL	161	103	127	2

EROSION CONTROL

LOCATION STATION TO STATION	PERIMETER EROSION BARRIER	MULCH METHOD 2	EROSION CONTROL BLANKET	STONE RIPRAP CLASS A4	FILTER FABRIC
	FOOT	ACRES	SQ YD	SQ YD	SQ YD
625+10 TO 631+40 RT	583	0.3	680		
625+10 TO 631+40 LT	569	0.3	544		
625+30 TO 626+00 RT				31	31
627+67.6 RT				4	4
627+61.4 LT				4	4
TOTALS	1152	0.6	1224	39	***39

*** FILTER FABRIC FOR STONE RIPRAP CLASS A4 IS ADDITIONAL TO QUANTITY NEEDED FOR STONE RIPRAP CLASS A5

BRIDGE APPROACH SCHEDULE

LOCATION STATION TO STATION	CONNECTOR (PCC) SPECIAL	BRIDGE APPROACH PAVEMENT	PROTECTIVE COAT 42001300	BRIDGE DECK GROOVING	TYPE C INLET BOX	METAL END SECTIONS 12"	PIPE DRAINS 12"	CONCRETE THRUST BLOCK
	SQ YD	SQ YD	SQ YD	SQ YD	EACH	EACH	FOOT	EACH
627+39.7 TO 627+49	38.5		38.5	35				
629+01 TO 629+10.3	38.5		38.5	35				
627+49 TO 627+79		124.5	124.5	113.5	2	2	44	2
628+71 TO 629+01		124.5	124.5	113.5				
TOTALS	77	249	326	* 297	2	2	44	2

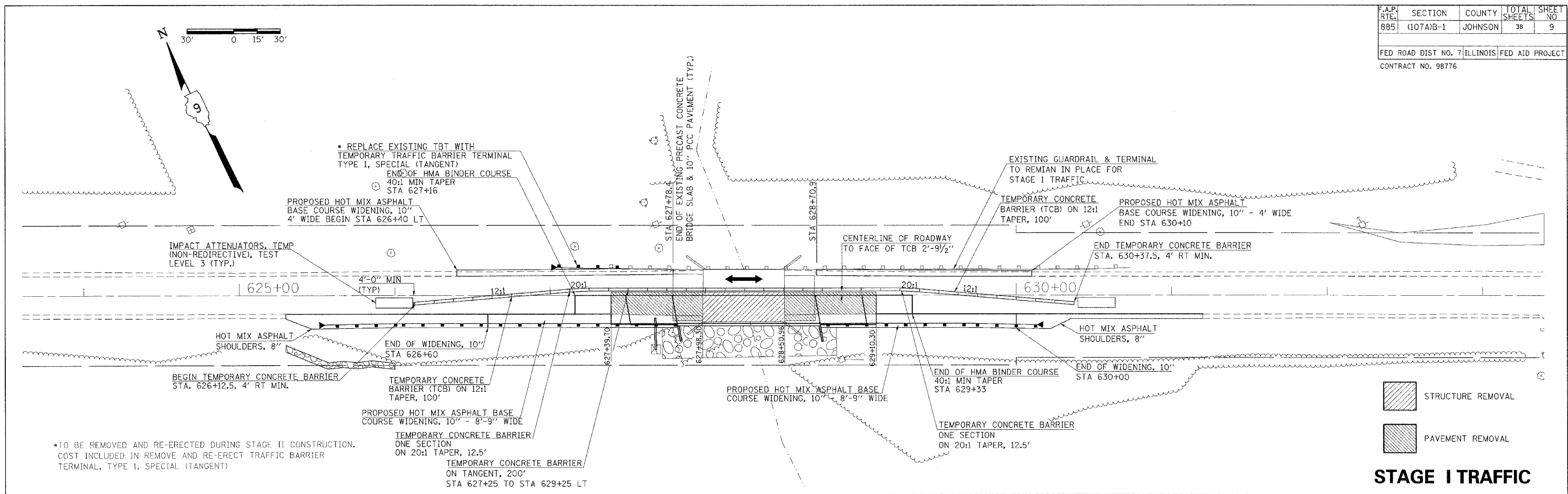
* GROOVING FOR CONNECTOR (PCC) SPECIAL AND BRIDGE PAVEMENT IS ADDITIONAL TO QUANTITY NEEDED FOR BRIDGE DECK

SEEDING SCHEDULE

LOCATION STATION TO STATION	TEMPORARY EROSION CONTROL SEEDING	SEEDING CLASS 7	SEEDING CLASS 2 (MODIFIED)	NITROGEN (N)	PHOSPHOROUS (P)	POTASSIUM (K)	AGRICULTURAL GROUND LIMESTONE
	POUND	ACRES	ACRES	POUND	POUND	POUND	TON
625+10 TO 631+40 RT	20	0.2	0.2	32	24	24	0.4
625+10 TO 631+40 LT	20	0.2	0.2	32	24	24	0.4
TOTALS	40	0.4	0.4	64	48	48	0.8

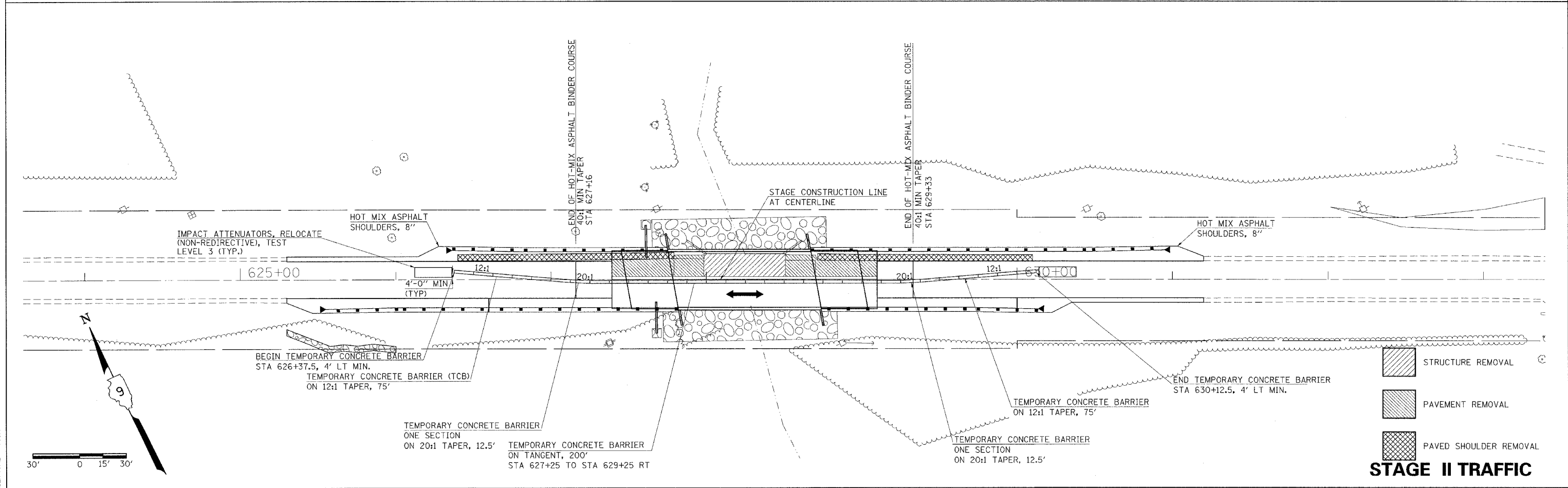
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FILE NAME = c:\pav\pcc\pcc13\344-0813.dgn
PLOT SCALE = 50.0000 / IN.
USER NAME = hst10001

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
885	(107A)B-1	JOHNSON	38	9
FED ROAD DIST NO. 7		ILLINOIS		FED AID PROJECT
CONTRACT NO. 98776				



- STRUCTURE REMOVAL
- PAVEMENT REMOVAL

STAGE I TRAFFIC



- STRUCTURE REMOVAL
- PAVEMENT REMOVAL
- PAVED SHOULDER REMOVAL

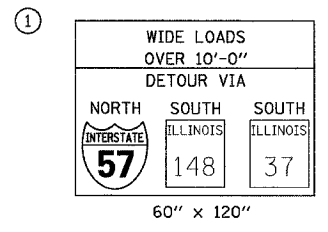
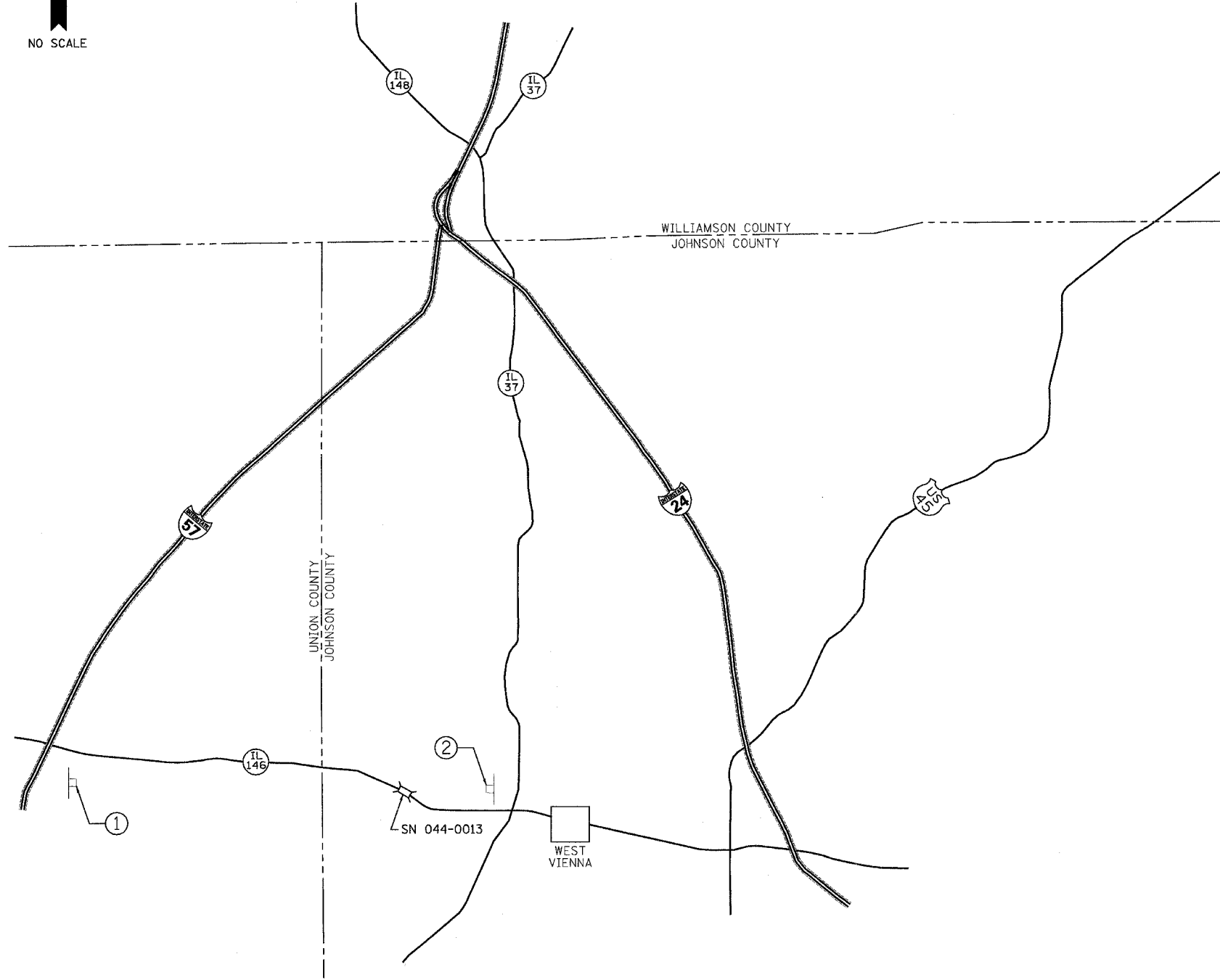
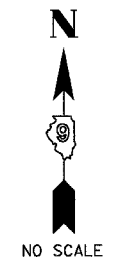
STAGE II TRAFFIC

STAGING PLAN

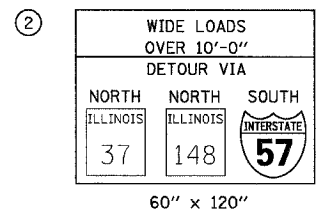
PLOT DATE = 7/28/2007
 FILE NAME = c:\projects\98776\98776.dgn
 PLOT SCALE = 3/8" = 1' / IN.
 USER NAME = halsstead

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO
885	(107A)B-1	JOHNSON	38	10
STA. 624+80		TO STA. 631+60		
FED. ROAD DIST. NO.	ILLINOIS	FED. AID PROJECT		
CONTRACT NO. 98776				

DETOUR SIGNING FOR LANE WIDTH RESTRICTION



TO BE USED:
STAGE I ONLY



TO BE USED:
STAGE I ONLY

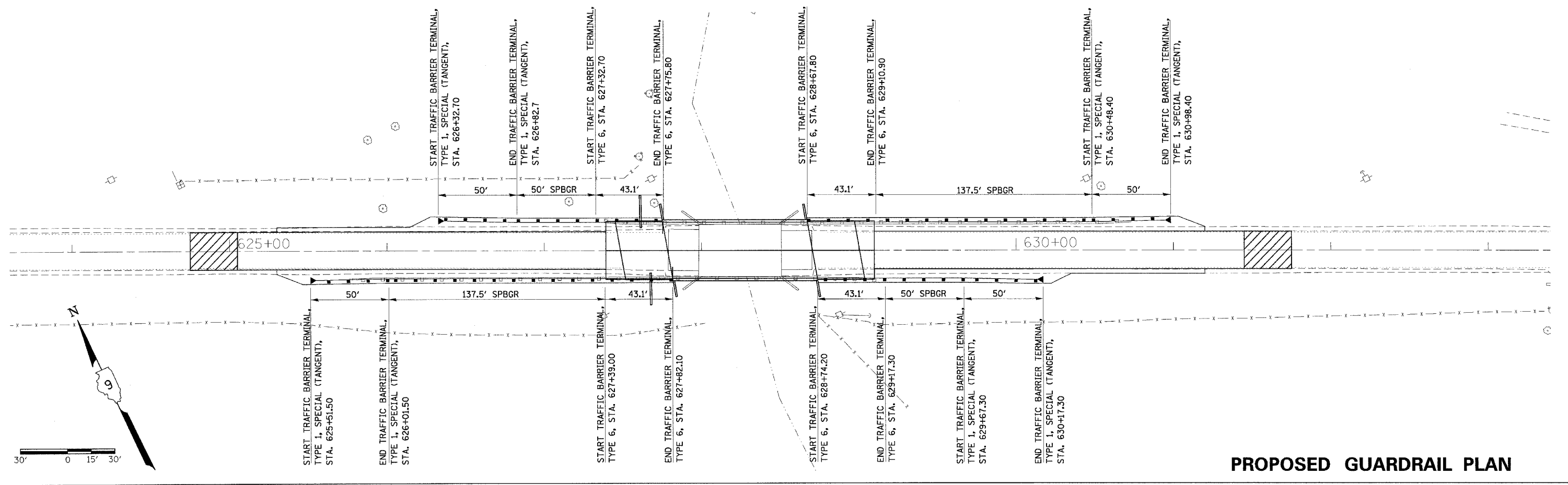
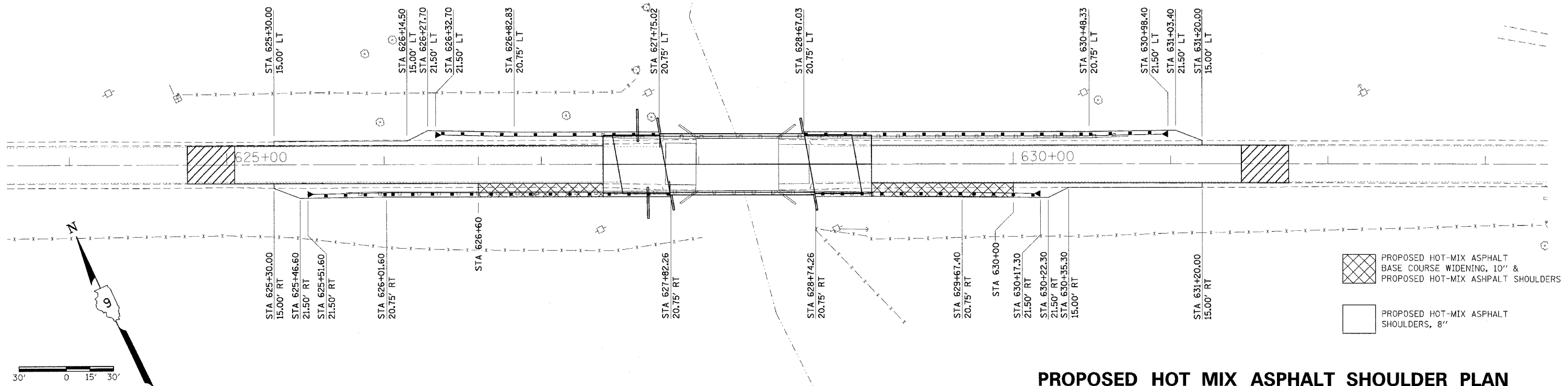
NOTES

1. THE CONTRACTOR SHALL FURNISH, ERECT, MAINTAIN, AND REMOVE THE POSTS AND SIGNS AT THE LOCATIONS SHOWN AND AS DIRECTED BY THE RESIDENT ENGR./TECH. ALL SIGNS SHALL BE POST MOUNTED.
2. THE CONTRACTOR SHALL GIVE I.D.O.T. BUREAU OF OPERATIONS, PERMITS SECTION, TWO WEEKS NOTICE BEFORE IMPLEMENTING ANY LANE WIDTH RESTRICTIONS.
3. THE ABOVE NOTED WORK, INCLUDING SIGNS, POSTS, HARDWARE, AND LABOR SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE, EACH, FOR TRAFFIC CONTROL AND PROTECTION, STD 701321 AND NO OTHER COMPENSATION WILL BE ALLOWED.

PLOT DATE = 7/30/2007
FILE NAME = c:\p\projects\1038003\044-0013.dgn
PLOT SCALE = 80/0000 / IN.
USER NAME = holstendts

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO
885	(107A)B-1	JOHNSON	38	11

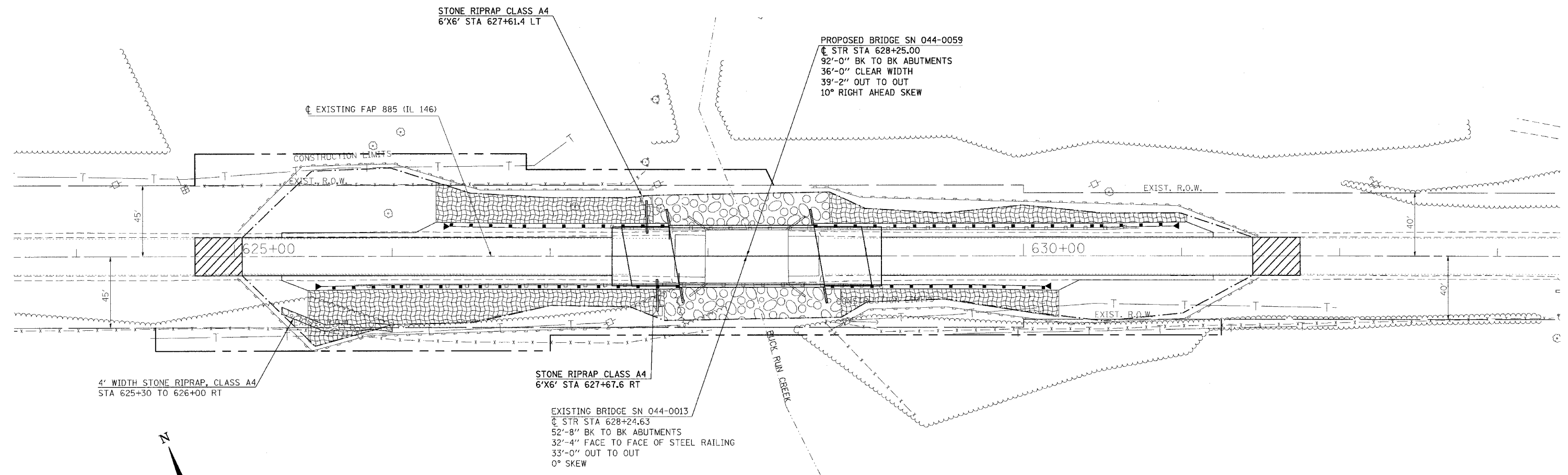
FED ROAD DIST NO. 7 ILLINOIS FED AID PROJECT
CONTRACT NUMBER 98776



PROPOSED GUARDRAIL PLAN
HOT MIX ASPHALT SHOULDER AND GUARDRAIL PLAN

PLOT DATE = 7/30/2007
FILE NAME = s:\projects\98776\98776.dwg
PLOT SCALE = 1/8" = 1'-0"
USER NAME = jharrington

CONTRACT NUMBER 98776				
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO
885	(107A)B-1	JOHNSON	38	12
FED ROAD DIST NO. 7 ILLINOIS FED AID PROJECT				



4' WIDTH STONE RIPRAP, CLASS A4
STA 625+30 TO 626+00 RT

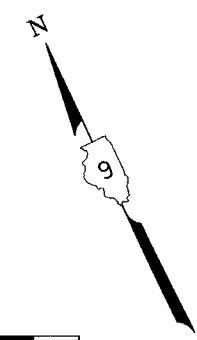
STONE RIPRAP CLASS A4
6'X6' STA 627+61.4 LT

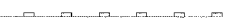
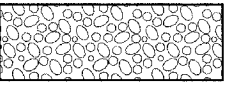
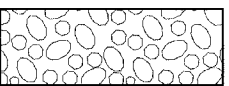
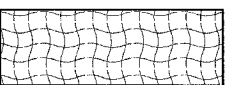
PROPOSED BRIDGE SN 044-0059
 C STR STA 628+25.00
 92'-0" BK TO BK ABUTMENTS
 36'-0" CLEAR WIDTH
 39'-2" OUT TO OUT
 10° RIGHT AHEAD SKEW

C EXISTING FAP 885 (IL 146)

STONE RIPRAP CLASS A4
6'X6' STA 627+67.6 RT

EXISTING BRIDGE SN 044-0013
 C STR STA 628+24.63
 52'-8" BK TO BK ABUTMENTS
 32'-4" FACE TO FACE OF STEEL RAILING
 33'-0" OUT TO OUT
 0° SKEW

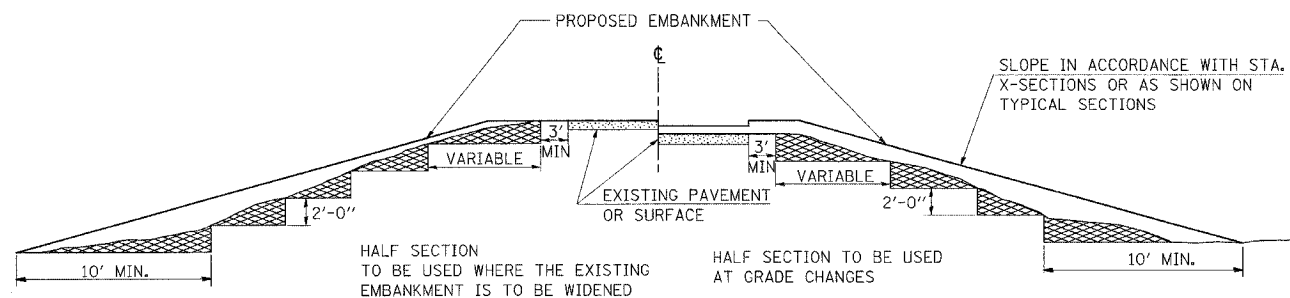


-  PERIMETER EROSION BARRIER
-  STONE RIPRAP, CLASS A4
-  STONE RIPRAP, CLASS A5
-  EROSION CONTROL BLANKET

PLOT DATE = 7/29/2007
 FILE NAME = c:\projects\98776\107a\107a_b1\107a_b1.dgn
 PLOT SCALE = 3/8"=1'-0"
 USER NAME = hals123456

EROSION CONTROL PLAN

TYPICAL CROSS SECTION SHOWING STEP CONSTRUCTION ON EXISTING FILL



HALF SECTION TO BE USED WHERE THE EXISTING EMBANKMENT IS TO BE WIDENED

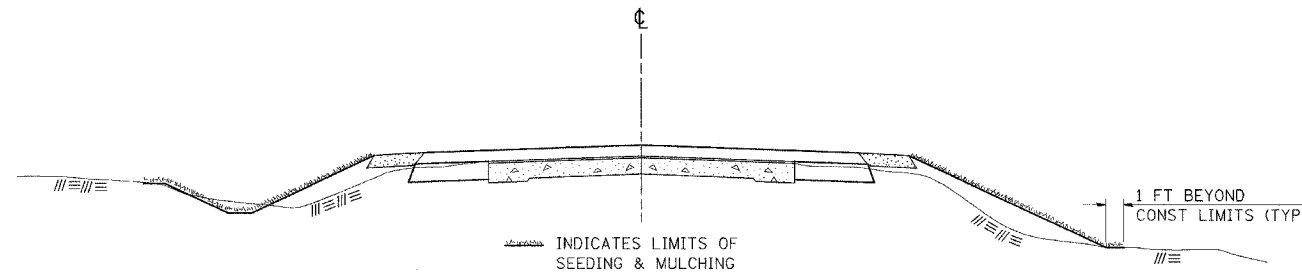
HALF SECTION TO BE USED AT GRADE CHANGES

MATERIAL TO BE REMOVED AND REPLACED IN THE EMBANKMENT IN ACCORDANCE WITH ART. 205.04 OF THE STANDARD SPECIFICATION. COST TO BE INCLUDED IN THE VARIOUS ITEMS OF EXCAVATION AND NO ADDITIONAL COMPENSATION WILL BE ALLOWED BECAUSE OF THIS WORK.

REVISIONS
 REDRAWN 2-15-89
 REVISED 8-15-94
 CHECKED 6-3-99
 REVISED
 STD. 9-16

SEEDING & MULCHING

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO
885	(107A)B-1	JOHNSON	38	13
STA.		TO STA.		
FED. ROAD DIST. NO.		ILLINOIS	FED. AID PROJECT	
CONTRACT NO. 98776				



GENERAL NOTES

IN GENERAL, ALL EARTH SURFACES DISTURBED DURING CONSTRUCTION OPERATIONS SHALL BE SEEDED AND MULCHED UPON COMPLETION OF ALL GRADING OPERATIONS.

FERTILIZER NUTRIENTS AND LIMESTONE SHALL BE APPLIED TO ALL SEEDED AREAS.

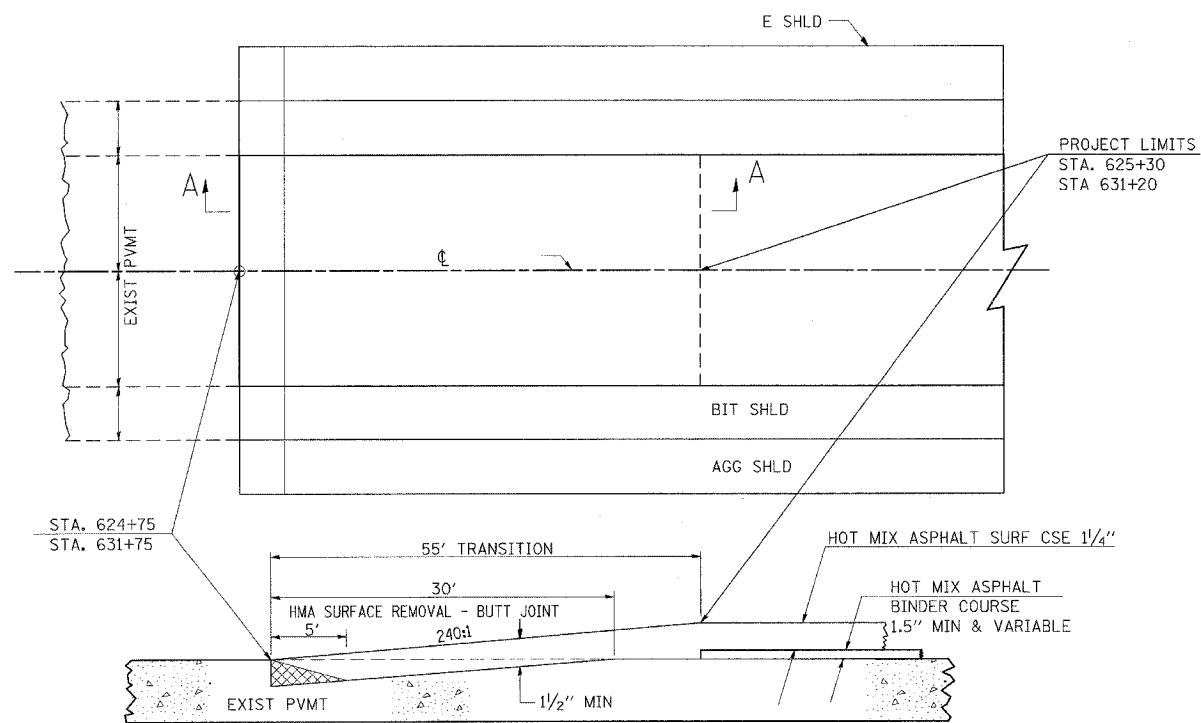
THE RATES OF APPLICATION OF FERTILIZER, MULCH AND LIMESTONE SHALL BE AS SPECIFIED IN THE SPECIAL PROVISIONS.

SECTIONS 250 AND 251 OF THE STANDARD SPECIFICATIONS SHALL GOVERN THIS WORK EXCEPT AS SPECIFIED HEREIN OR AS NOTED IN THE SPECIAL PROVISIONS.

REVISIONS
 REDRAWN 2-15-89
 REVISED 8-15-94
 REVISED 6-3-99
 REVISED

STD. 9-12

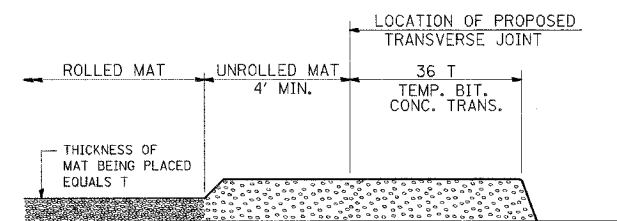
BUTT JOINT



SECTION A-A

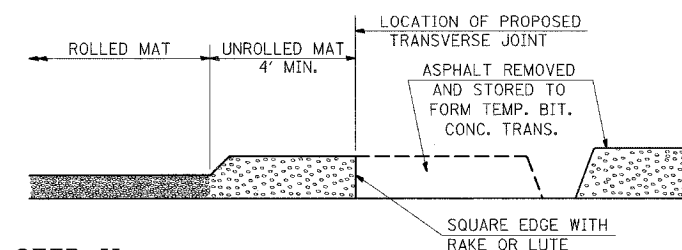
REVISIONS
 DRAWN 10-17-90
 REVISED
 REVISED
 REVISED
 STD. 9-86

TEMPORARY BITUMINOUS CONCRETE TRANSITIONS



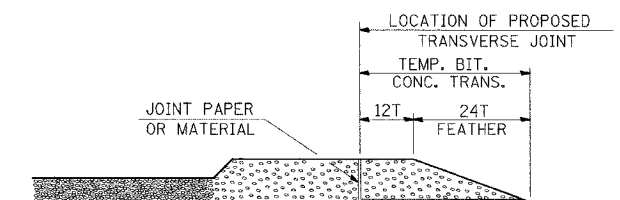
STEP I

- PLACE BITUMINOUS MAT, LENGTH 36 TIMES THE THICKNESS OF THE MAT BEING PLACED PAST THE PROPOSED TRANSVERSE JOINT LOCATION USING NORMAL OPERATING PROCEDURES.
- EXTREME CARE SHOULD BE TAKEN TO MAINTAIN ENOUGH MATERIAL IN FRONT OF THE SCREED TO MAINTAIN REQUIRED PAVING DEPTH.



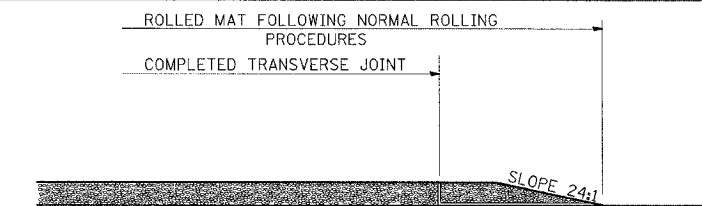
STEP II

- MOVE THE PAVER OUT OF THE WAY AND REMOVE THE ASPHALT FROM THE AREA OF THE PROPOSED TEMPORARY BITUMINOUS CONCRETE TRANSITION.
- SQUARE UP THE END OF THE MAT WITH A RAKE OR LUTE.
- NOTE THAT THE MAT WITHIN 4' OF THE END OF JOINT IS NOT TO BE ROLLED AT THIS TIME.



STEP III

- JOINT PAPER OR OTHER PRESELECTED JOINT MATERIAL IS THEN PLACED IN THE CLEARED AREA AND THE EXCESS ASPHALT USED TO HAND FORM A TRANSITION TO THE DIMENSIONS SHOWN ABOVE.
- NOTE THAT IN CONSTRUCTING THE TRANSITION, THE MAT DEPTH IS CONTINUED AS PART OF THE TRANSITION BEFORE FORMING THE FEATHER.



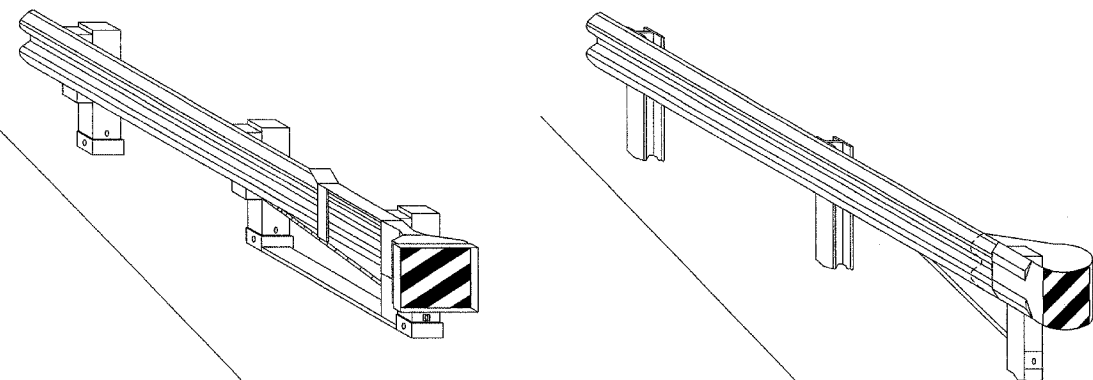
STEP IV

- COMPLETE TEMPORARY TRANSITION BY ROLLING.
- TO RESUME PAVING, AT THE JOINT, REMOVE TEMPORARY TRANSITION AND DISPOSE OF THE MATERIAL ACCORDING TO ART. 202.03 OF THE STD. SPECS. (COST INCLUDED IN THE CONTRACT).
- CONSTRUCTING THE TEMPORARY TRANSITIONS WILL NOT BE PAID FOR SEPARATELY BUT WILL BE INCLUDED IN THE BITUMINOUS MATERIAL BEING PLACED.

REVISIONS
 REDRAWN 2-15-89
 REVISED 8-15-94
 REVISED
 REVISED

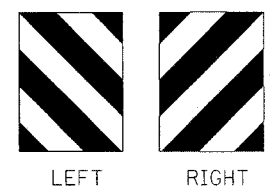
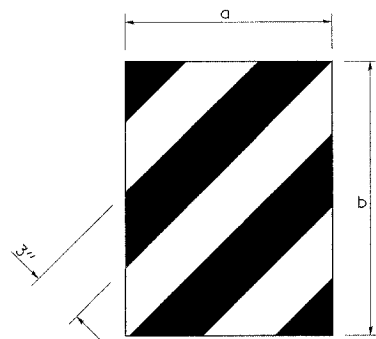
STD. 9-26

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
885	(107A)B-1	JOHNSON	38	14
STA.		TO STA.		
FED. ROAD DIST. NO.		ILLINOIS	FED. AID PROJECT	
CONTRACT NO. 98776				



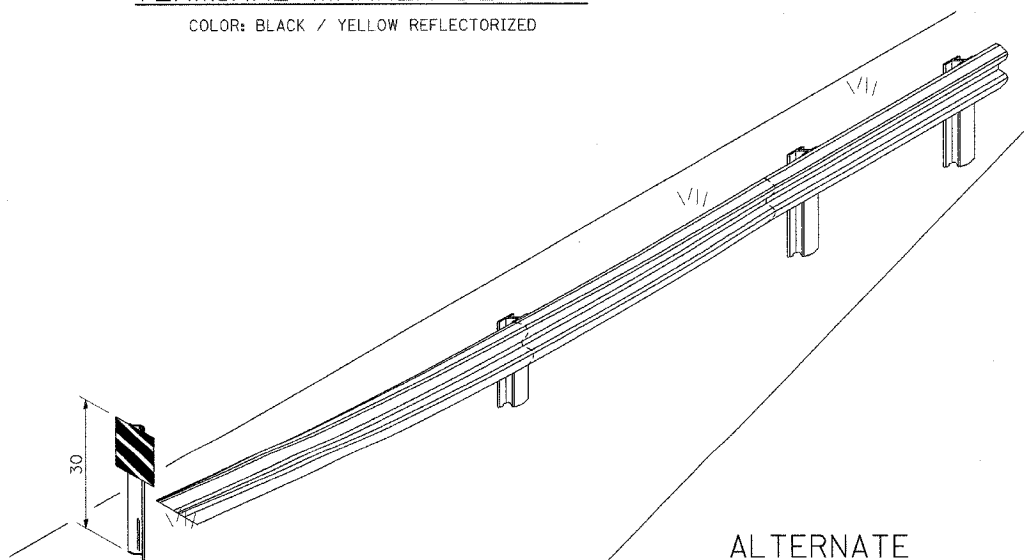
CASE I

CASE II

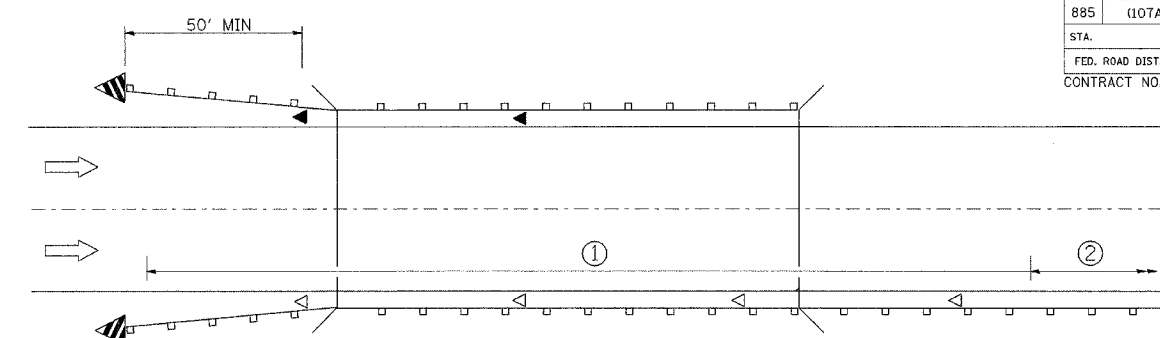


DIMENSION	CASE I	CASE II
a	15"	18"
b	20"	16"

TERMINAL MARKER DETAILS
COLOR: BLACK / YELLOW REFLECTORIZED



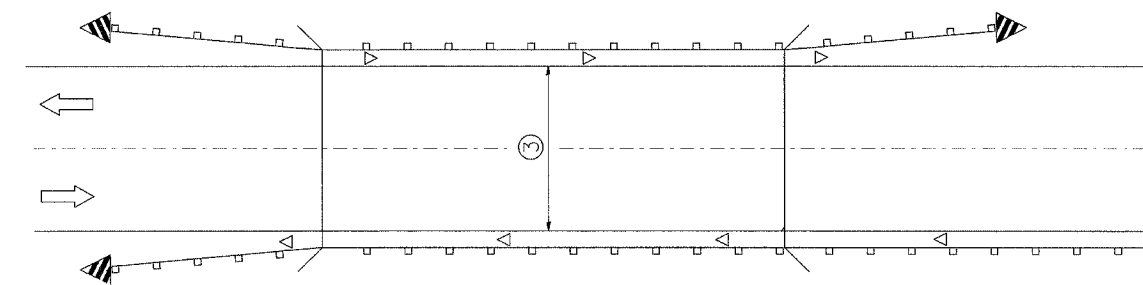
ALTERNATE
POST MOUNT ONLY WITH
TWISTED OR SLOPING TERMINAL
WHERE SHEETING CANNOT BE
APPLIED DIRECTLY TO TERMINAL.



① SPACING 80 FEET MAX FOR FIRST 400 FEET OR CURVE SPACING SHOWN IN STANDARD 635001, WHICHEVER IS LESS (MIN 4 REFLECTORS REGARDLESS OF LENGTH).

② AFTER 400 FEET, TRANSITION TO NORMAL DELINEATOR SPACING SHOWN IN STANDARD 635001, AND CONTINUE AS REQUIRED.

ONE-WAY TRAFFIC

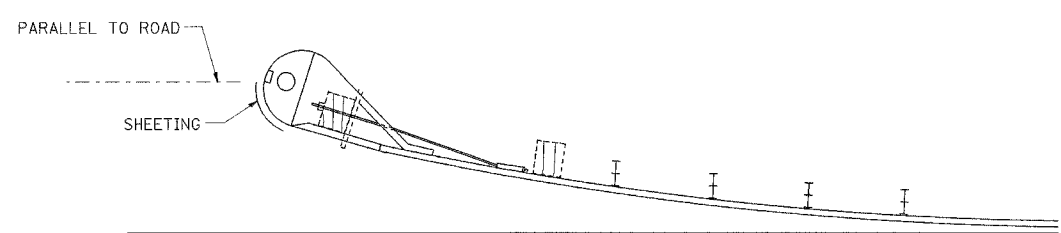


③ BIDIRECTIONAL SILVER/SILVER SHOULD BE USED IN LIEU OF MONODIRECTIONAL SILVER ON BOTH SIDES OF TWO-LANE BRIDGES WHERE THE PAVEMENT IS LESS THAN 24 IN WIDER THAN THE PAVEMENT APPROACHING THE BRIDGE.

- ◁ MONODIRECTIONAL SILVER
- ◀ MONODIRECTIONAL AMBER
- ▤ TERMINAL MARKER - BLACK/YELLOW LEFT OR RIGHT AS APPROPRIATE

TWO-WAY TRAFFIC

GUARDRAIL / BARRIER WALL /
BRIDGE RAIL REFLECTORS



SHEETING POSITION: CASE II

ALL DIMENSIONS ARE IN INCHES
UNLESS OTHERWISE SHOWN.

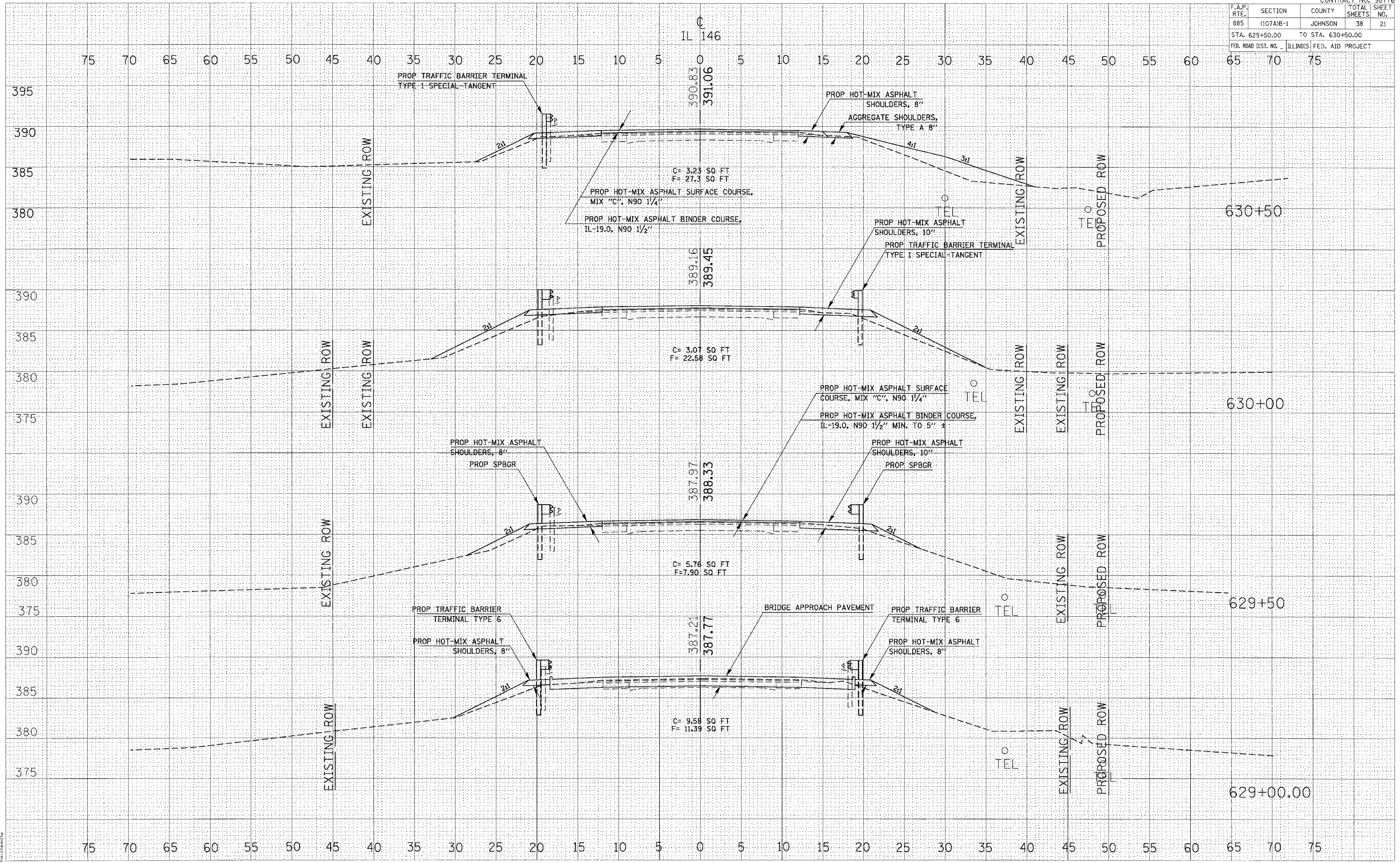
REFLECTOR AND TERMINAL
MARKER PLACEMENT

DETAIL

REFLECTOR AND MARKER PLACEMENT

PLOT DATE = 7/30/2007
FILE NAME = C:\p5\met\98776\044-0813.dgn
PLOT SCALE = 5/8"=1'-0"
USER NAME = hls

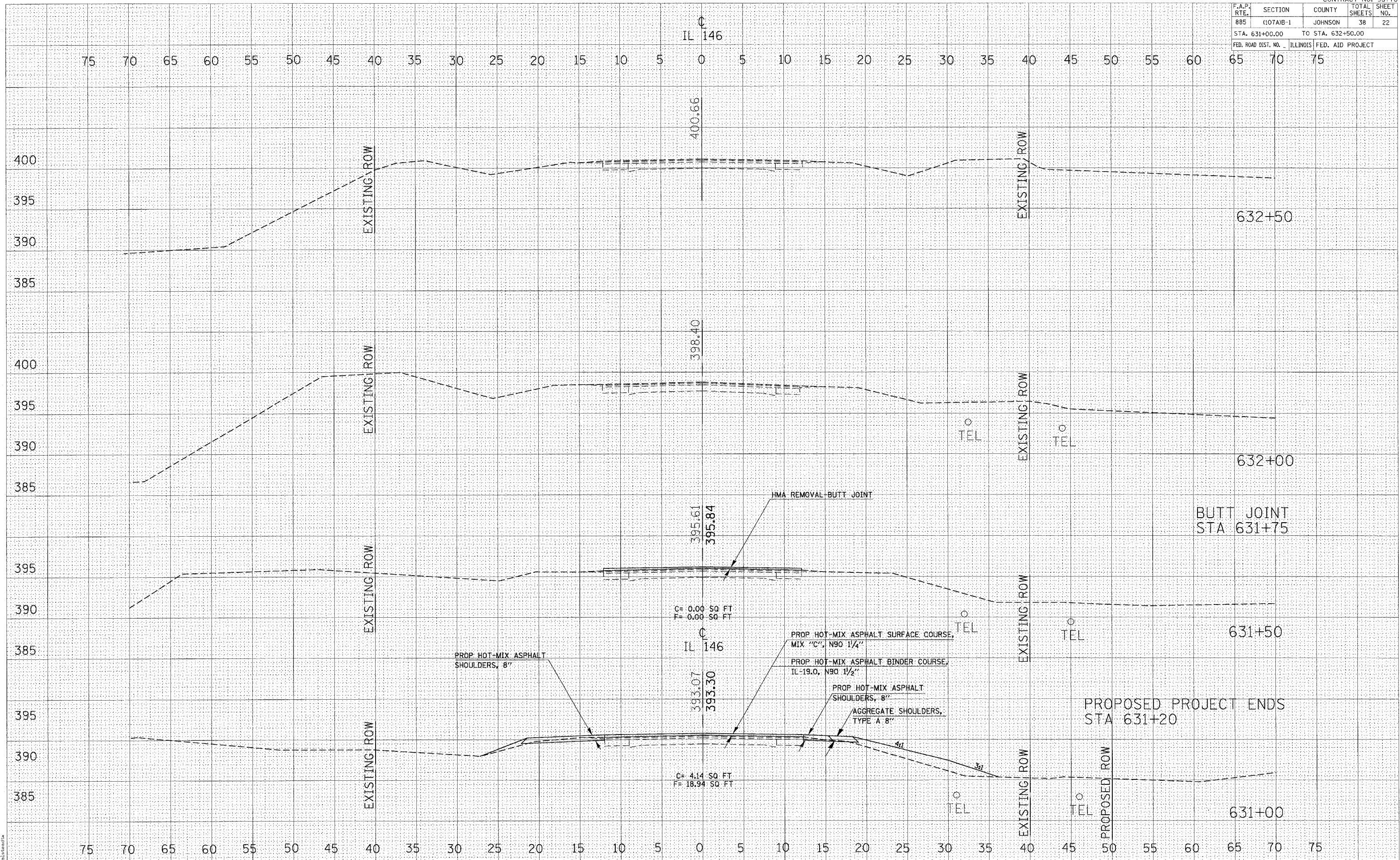
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
885	(107A)B-1	JOHNSON	38	21
STA. 629+50.00		TO STA. 630+50.00		
FED. ROAD DIST. NO. ILLINOIS		FED. AID PROJECT		



CROSS SECTIONS - IL 146

7/30/2007
 c:\projects\1033683\644-01\13a.dgn
 5:00PM / JN
 2007/07/30

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
885	(107A)B-1	JOHNSON	38	22
STA. 631+00.00		TO STA. 632+50.00		
FED. ROAD DIST. NO.	ILLINOIS	FED. AID PROJECT		



7/30/2007 10:03:03 AM 044-0013.mxd

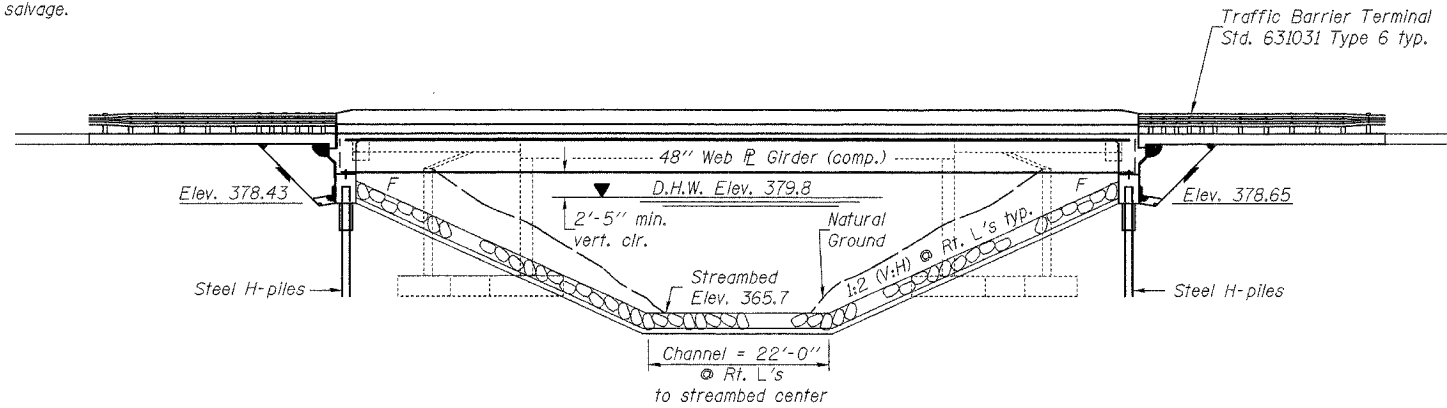
Bench Mark: Square out in NW wingwall @ Sta. 627+93, Elev. 384.92

Existing Structure: S.N. 044-0013 Built in 1930 as SBI Rte. 146 Sec. 107A at Sta. 628+28 as a simple span reinforced concrete T-beam 52'-8" Bk.-Bk. abutment supported on timber piles. Bridge widening in 1983 at Sta. 628+24.63 with PPC deck beams & bituminous overlay. Traffic maintained utilizing stage construction.

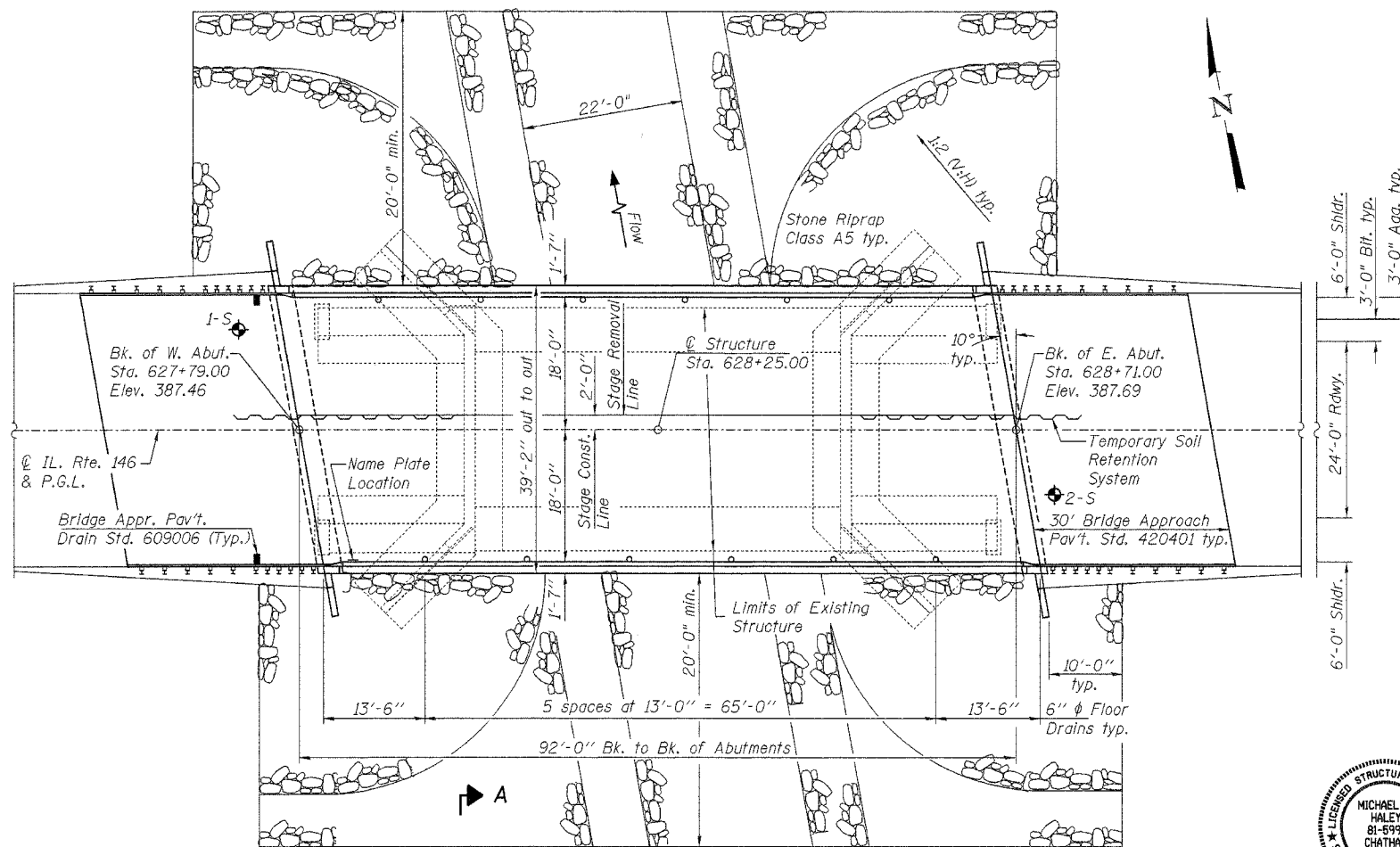
No salvage.

ROUTE NO.	SECTION	COUNTY	SHEET NO.	SHEET NO.
F.A.P. 885	(107A) B-1	Johnson	38	23
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT-	

Contract # 98776



ELEVATION



PLAN

INDEX OF SHEETS

1. General Plan
2. General Notes & Details
3. Stage Construction Details
4. Deck Elevations
5. Approach Pavement Elevations
6. Superstructure
7. Superstructure Details
8. Concrete End Diaphragms
9. Framing Plan & Steel Details
10. West Abutment
11. East Abutment
12. Temporary Concrete Barrier
13. Bar Splicer Assembly Details
14. Steel Pile Details
15. Soil Borings-1
16. Soil Borings-2

LOADING HL-93

Allow 50#/sq. ft. for future wearing surface.

DESIGN SPECIFICATIONS

2004 LRFD AASHTO w/2005 & 2006 Interims

DESIGN STRESSES

FIELD UNITS

- $f'_c = 3,500$ psi
- $f_y = 60,000$ psi (reinforcement)
- $f_y = 50,000$ psi (Structural Steel, M270 Gr. 50)

SEISMIC DATA

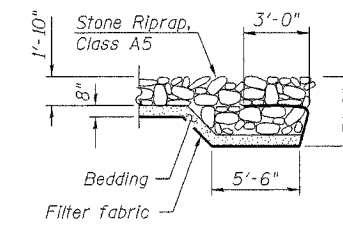
Seismic Performance Zone (SPZ) = 3
 Bedrock Acceleration Coefficient (A) = 0.20g
 Site Coefficient (S) = 1.0

WATERWAY INFORMATION

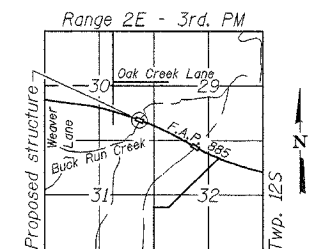
Exist. Low Grade Elev. 386.77 @ Sta. 628+00 (Exist. Align.)
 Prop. Low Grade Elev. 387.45 @ Sta. 627+75 (Prop. Align.)

Flood	Freq. Yr.	Q C.F.S.	Opening Sq. Ft.		Nat. H.W.E.	Head - Ft.		Headwater El.	
			Exist.	Prop.		Exist.	Prop.	Exist.	Prop.
Design	10	1940	414.9	631.4	378.8	0.3	0.3	379.1	379.1
Base	50	2930	464.9	707.8	379.8	0.7	0.4	380.5	380.2
Max. Calc.	100	3350	484.9	739.5	380.2	0.6	0.2	380.8	380.4
	500	4340	519.9	796.5	380.9	1.0	0.3	381.9	381.2

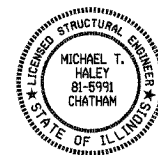
10 year velocity through existing bridge = 4.7 fps 10 year velocity through prop. bridge = 3.1 fps



SECTION A-A



LOCATION SKETCH



Michael J. Haley
 Michael T. Haley
 Licensed Structural Engineer
 State of Illinois No. 81-5991
 Expires 11/30/2008

8-16-07
 Date

APPROVED
 FOR STRUCTURAL ADEQUACY ONLY

Ralph E. Anderson (TSP)
 ENGINEER OF BRIDGES AND STRUCTURES

Design Scour Elevation (feet)	W. Abutment	E. Abutment
	378.43	378.65

REVISIONS	
NAME	DATE

ILLINOIS DEPARTMENT OF TRANSPORTATION
 GENERAL PLAN
 ILLINOIS ROUTE 146 OVER
 BUCK RUN CREEK
 F.A.P. ROUTE 885 - SEC. (107A)B-1
 JOHNSON COUNTY
 STA. 628+25.00
 STRUCTURE NO. 044-0059

8/2/2007 1:35:08 PM ...\\044-0059.dgn

Contract # 98776

GENERAL NOTES

Fasteners shall be AASHTO M164 Type 1, mechanically galvanized bolts. Bolts 7/8 in. ϕ , holes 15/16 in. ϕ , unless otherwise noted.

Calculated weight of Structural Steel = 80,150 lbs. (AASHTO M270, Grade 50)
11,820 lbs. (AASHTO M270, Grade 36)

No field welding is permitted except as specified in the contract documents.

Reinforcement bars shall conform to the requirements of ASTM A 706 Gr 60 (IL Modified). See Special Provisions

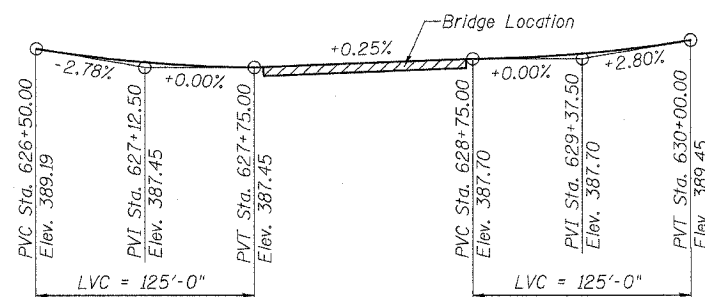
Reinforcement bars designated (E) shall be epoxy coated.

The Inorganic Zinc Rich Primer / Acrylic / Acrylic Paint System shall be used for shop and field painting of new structural steel except where otherwise noted. The color of the final finish coat for all interior steel surfaces shall be gray, Munsell No. 5B 7/L. The color of the final finish coat for the exterior and bottom flange of the fascia beams shall be Interstate Green, Munsell No. 7.5G 4/8. See Special Provision for "Cleaning and Painting New Metal Structures".

Layout of slope protection system may be varied in the field to suit ground conditions as directed by the Engineer.

TOTAL BILL OF MATERIAL

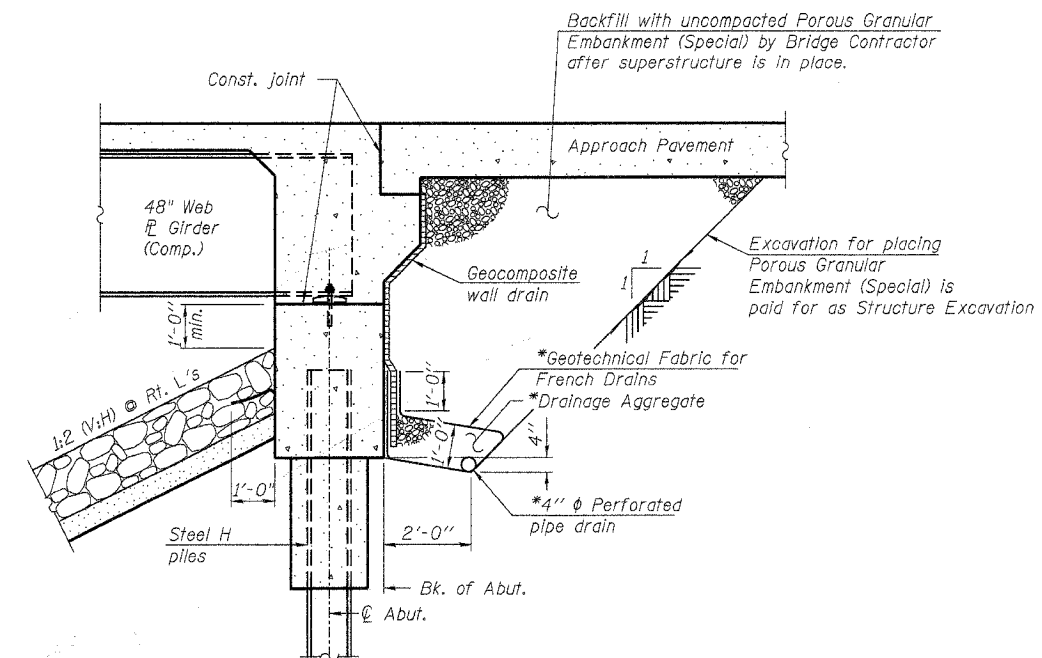
ITEM	UNIT	SUPER	SUB	TOTAL
Removal of Existing Structures	Each	1	-	1
Structure Excavation	Cu. Yd.	-	261	261
Geocomposite Wall Drain	Sq. Yd.	-	111	111
Pipe Underdrains for Structures 4"	Foot	-	128	128
Temporary Soil Retention System	Sq. Ft.	-	648	648
Porous Granular Embankment (Special)	Cu. Yd.	-	229	229
Concrete Structures	Cu. Yd.	-	29.8	29.8
Concrete Superstructure	Cu. Yd.	141.4	-	141.4
Bridge Deck Grooving	Sq. Yd.	348	-	348
Reinforcement Bars, Epoxy Coated	Pound	27600	6200	33800
Bar Splicers	Each	401	20	421
Furnishing and Erecting Structural Steel	Lump Sum	1	-	1
Stud Shear Connectors	Each	1782	-	1782
Protective Coat	Sq. Yd.	445	-	445
Floor Drains	Each	12	-	12
Driving Piles	Foot	-	632	632
Furnishing Steel Piles HP12x 53	Foot	-	632	632
Name Plates	Each	1	-	1
Stone Riprap, Class A5	Sq. Yd.	-	974	974
Filter Fabric	Sq. Yd.	-	974	974
Concrete Encasement	Cu. Yd.	-	5.6	5.6
Anchor Bolts, 1"	Each	-	24	24
Pile Shoes	Each	-	16	16



PROFILE GRADE
(along ϕ Roadway)

STATION 628+25.00
BUILT 20 BY
STATE OF ILLINOIS
F.A.P. RT. 885 SEC. (107A)B-1
LOADING HL-93
STR. NO. 044-0059

NAME PLATE
See Std. 515001



SECTION THRU INTEGRAL ABUTMENT

(Horiz. dim. @ Rt. L's)

* Included in the cost of Pipe Underdrains for Structures.

Note:
All drainage system components shall extend to 2'-0" from the end of each wingwall except an outlet pipe shall extend until intersecting with the side slopes. The pipes shall drain into concrete headwalls. (See Article 601.05 of the Standard Specifications and Highway Standard 601101).

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8/2/2007

Lin Engineering, Ltd.
Consulting Engineers
Chatham, Illinois

Designed By: KAH
Checked By: MTH
Date: 3/2007
File: 044-0059.DGN

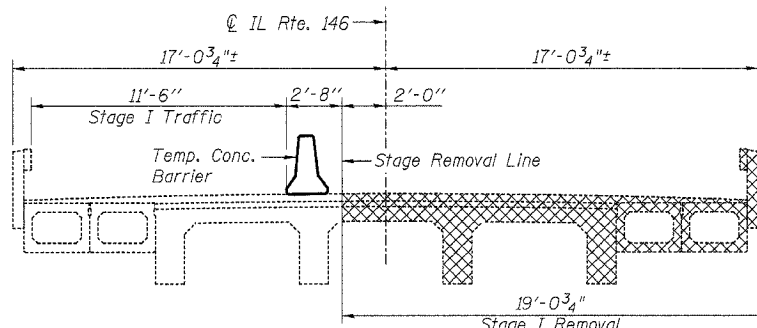
REVISIONS	
NAME	DATE

ILLINOIS DEPARTMENT OF TRANSPORTATION
GENERAL NOTES & DETAILS
ILLINOIS ROUTE 146 OVER
BUCK RUN CREEK
F.A.P. ROUTE 885 - SEC. (107A)B-1
JOHNSON COUNTY
STA. 628+25.00
STRUCTURE NO. 044-0059

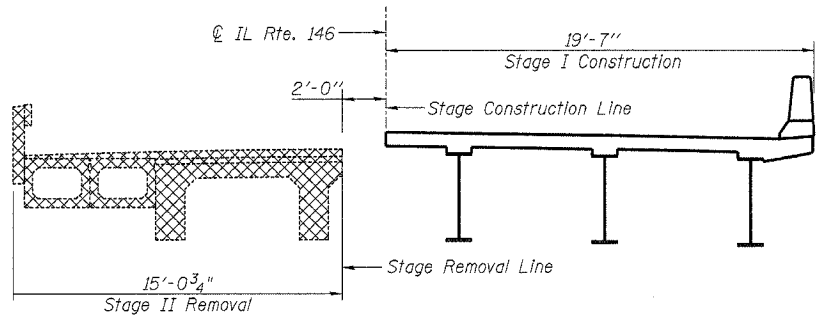
ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
F.A.P. 885	(107A) B-1	Johnson	38	25
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT-	

SHEET NO. 3
16 SHEETS

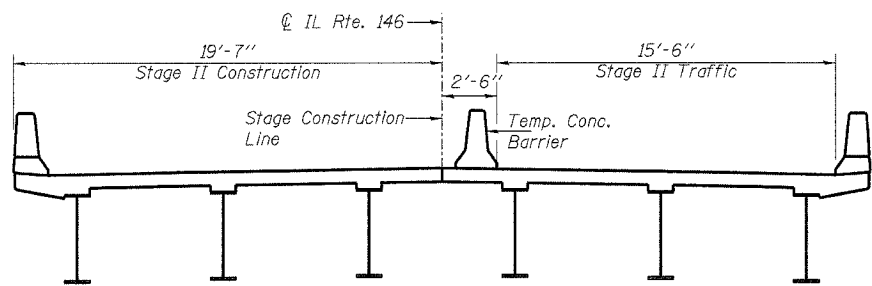
Contract # 98776



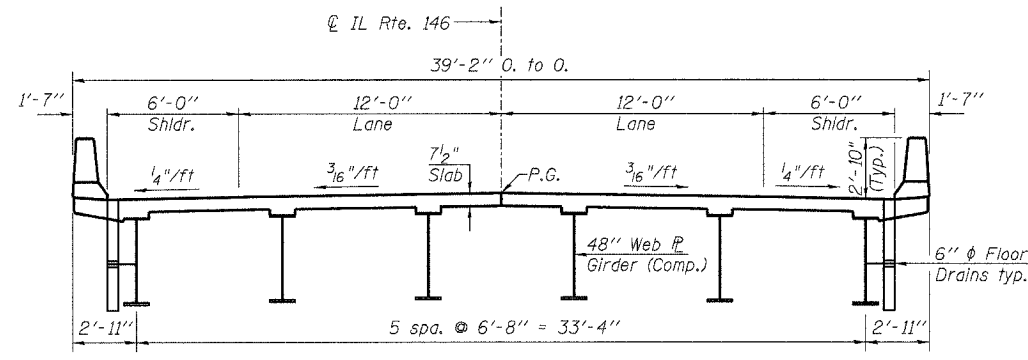
STAGE I REMOVAL & TRAFFIC
(Looking East)



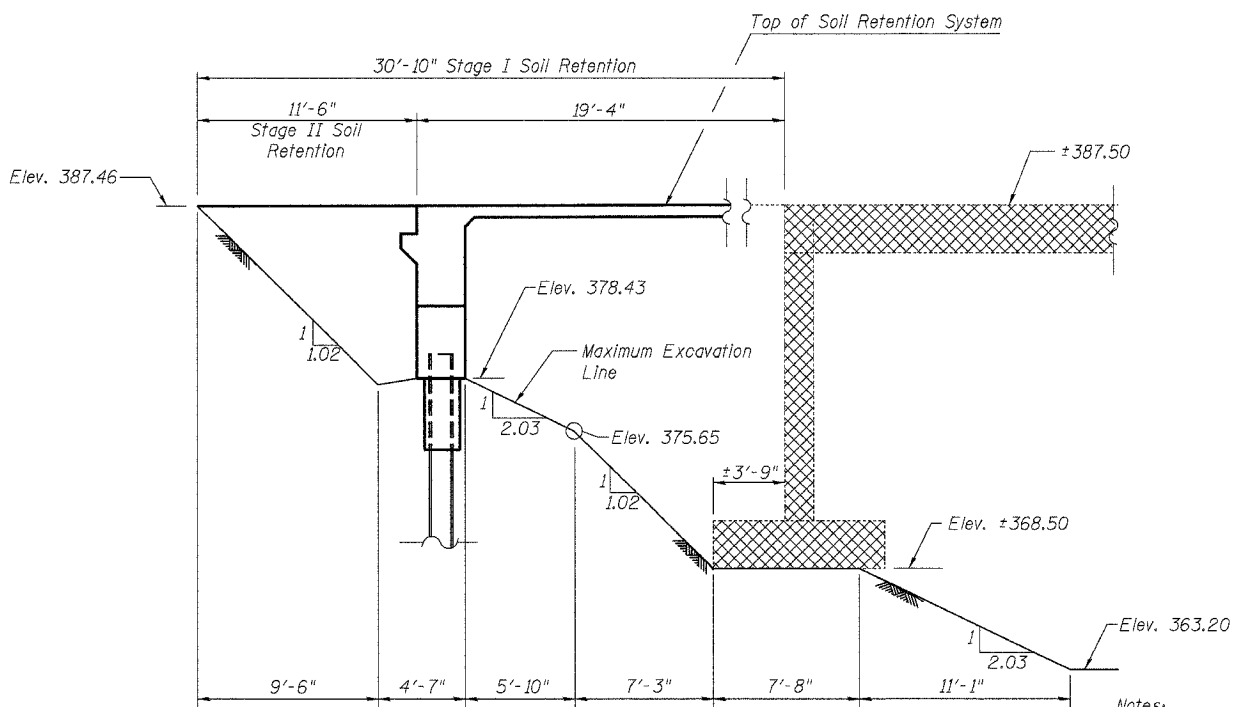
STAGE I CONSTRUCTION & STAGE II REMOVAL
(Looking East)



STAGE II CONSTRUCTION & TRAFFIC
(Looking East)



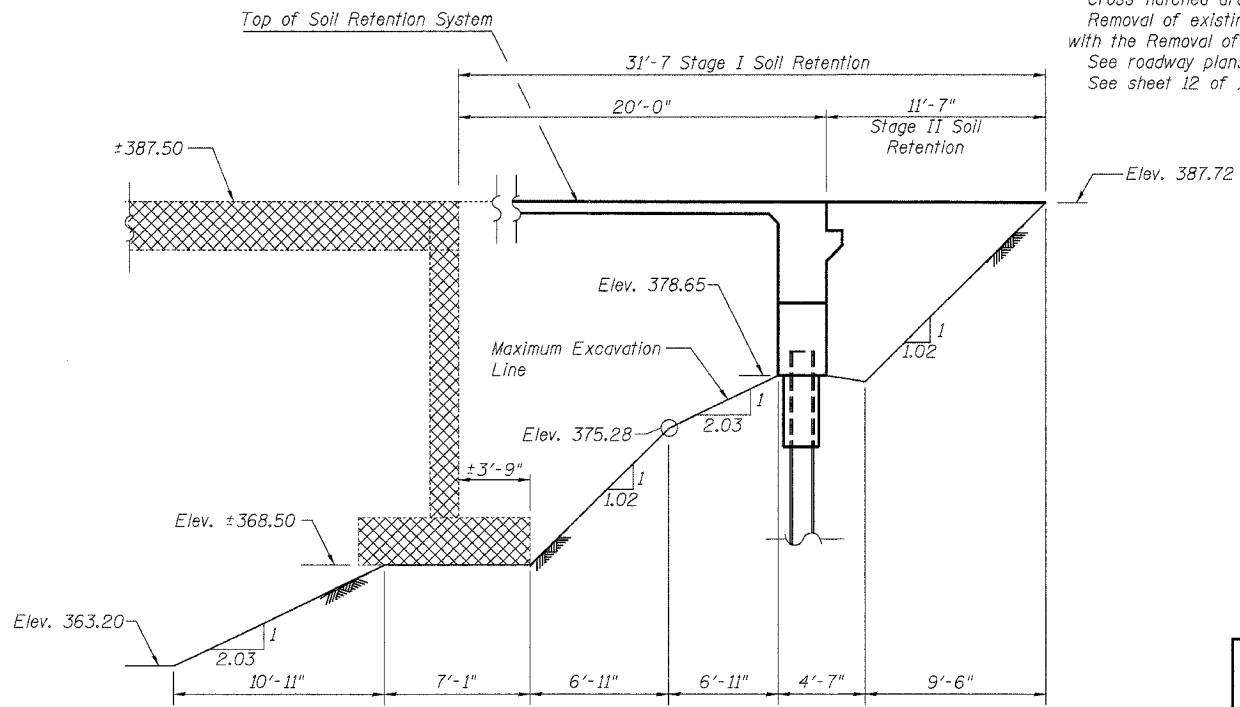
PROPOSED CROSS SECTION
(Looking East)



TEMPORARY SOIL RETENTION SYSTEM AT WEST ABUT.
(Horizontal dimensions along CL roadway)

Notes:

A cantilevered sheet piling design does not appear feasible and additional members or other retention systems may be necessary. The Contractor shall submit a temporary soil retention system design including plan details and calculations for review and acceptance by the Engineer.
Location of Stage Removal & Construction lines is also applicable to existing abutments.
Cross-hatched areas indicate removal of existing structures.
Removal of existing bridge railing and bituminous wearing surface is included with the Removal of Existing Structure.
See roadway plans for quantity of Temporary Concrete Barrier.
See sheet 12 of 16 for details of Temporary Concrete Barrier.



TEMPORARY SOIL RETENTION SYSTEM AT EAST ABUT.
(Horizontal dimensions along CL roadway)

REVISIONS	
NAME	DATE

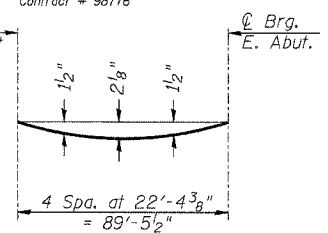
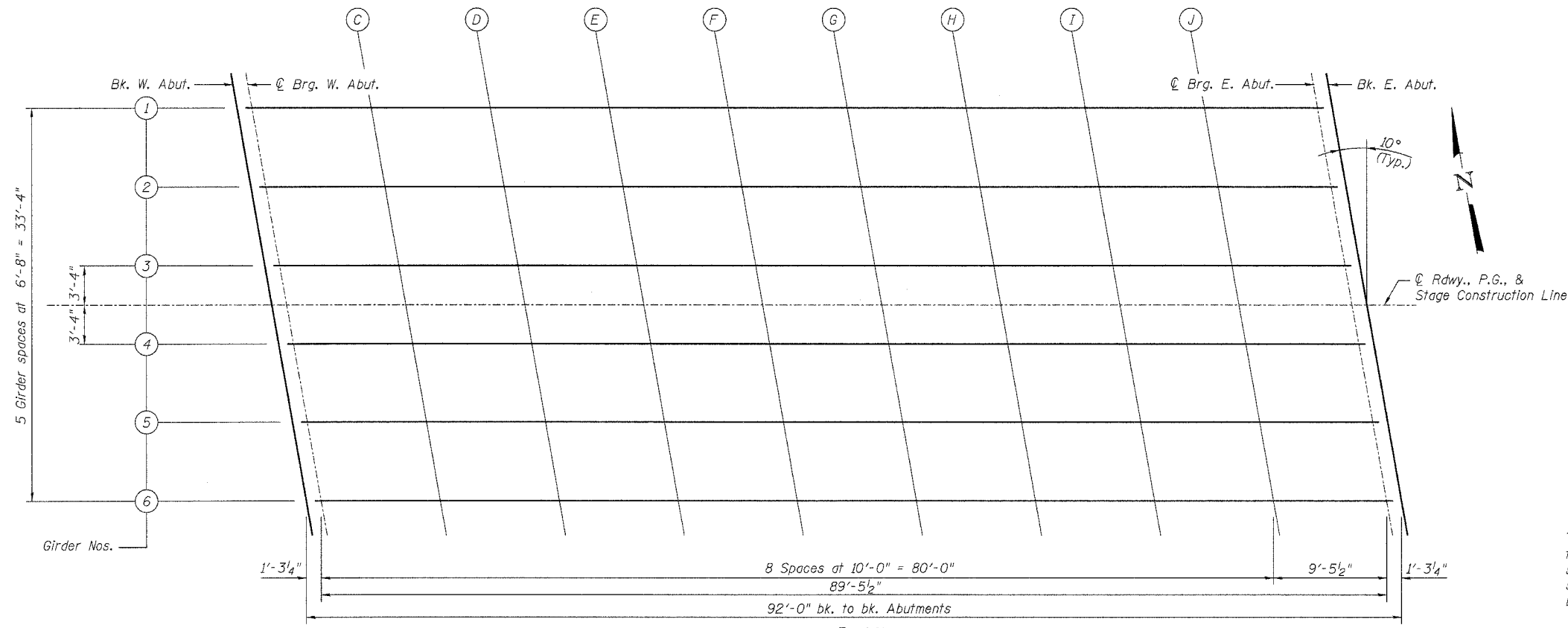
LIN ENGINEERING, LTD.
Consulting Engineers
Chatham, Illinois

Designed By: KKH
Checked By: MTH
Date: 3/2007

Drawn By: AJJ
File: 044-0059.DGN

ILLINOIS DEPARTMENT OF TRANSPORTATION
STAGE CONSTRUCTION DETAILS
ILLINOIS ROUTE 146 OVER
BUCK RUN CREEK
F.A.P. ROUTE 885 - SEC. (107A)B-1
JOHNSON COUNTY
STA. 628+25.00
STRUCTURE NO. 044-0059

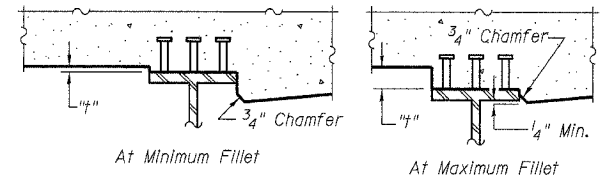
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DEAD LOAD DEFLECTION DIAGRAM

(Includes weight of concrete only)

Note: The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections as shown below.



To determine "4": After all structural steel has been erected, elevations of the top flanges of the beams shall be taken at intervals shown below. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflection" shown below, minus slab thickness, equals the fillet heights "4" above top flange of beams.

FILLET HEIGHTS

PLAN

GIRDER 1

Location	Station	Offset (ft)	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. of W. Abut.	627+76.11	-16.67	387.17	387.17
☉ Brg. W. Abut.	627+77.38	-16.67	387.17	387.17
C	627+87.38	-16.67	387.20	387.26
D	627+97.38	-16.67	387.22	387.33
E	628+07.38	-16.67	387.25	387.40
F	628+17.38	-16.67	387.27	387.44
G	628+27.38	-16.67	387.30	387.47
H	628+37.38	-16.67	387.32	387.47
I	628+47.38	-16.67	387.35	387.46
J	628+57.38	-16.67	387.37	387.43
☉ Brg. E. Abut.	628+66.84	-16.67	387.39	387.39
Bk. of E. Abut.	628+68.11	-16.67	387.40	387.40

GIRDER 2

Location	Station	Offset (ft)	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. of W. Abut.	627+77.26	-10.00	387.30	387.30
☉ Brg. W. Abut.	627+78.53	-10.00	387.30	387.30
C	627+88.53	-10.00	387.33	387.39
D	627+98.53	-10.00	387.35	387.47
E	628+08.53	-10.00	387.38	387.54
F	628+18.53	-10.00	387.40	387.58
G	628+28.53	-10.00	387.43	387.61
H	628+38.53	-10.00	387.45	387.61
I	628+48.53	-10.00	387.48	387.59
J	628+58.53	-10.00	387.50	387.56
☉ Brg. E. Abut.	628+67.99	-10.00	387.53	387.53
Bk. of E. Abut.	628+69.26	-10.00	387.53	387.53

GIRDER 3

Location	Station	Offset (ft)	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. of W. Abut.	627+78.42	-3.33	387.41	387.41
☉ Brg. W. Abut.	627+79.69	-3.33	387.41	387.41
C	627+89.69	-3.33	387.43	387.50
D	627+99.69	-3.33	387.46	387.58
E	628+09.69	-3.33	387.48	387.64
F	628+19.69	-3.33	387.51	387.69
G	628+29.69	-3.33	387.53	387.71
H	628+39.69	-3.33	387.56	387.72
I	628+49.69	-3.33	387.58	387.70
J	628+59.69	-3.33	387.61	387.67
☉ Brg. E. Abut.	628+69.15	-3.33	387.63	387.63
Bk. of E. Abut.	628+70.42	-3.33	387.64	387.64

☉ ROADWAY, P.G., & STAGE CONSTRUCTION LINE

Location	Station	Offset (ft)	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. of W. Abut.	627+79.00	0.00	387.46	387.46
☉ Brg. W. Abut.	627+80.27	0.00	387.46	387.46
C	627+90.27	0.00	387.49	387.55
D	628+00.27	0.00	387.51	387.63
E	628+10.27	0.00	387.54	387.70
F	628+20.27	0.00	387.56	387.74
G	628+30.27	0.00	387.59	387.77
H	628+40.27	0.00	387.61	387.77
I	628+50.27	0.00	387.64	387.75
J	628+60.27	0.00	387.66	387.72
☉ Brg. E. Abut.	628+69.73	0.00	387.69	387.69
Bk. of E. Abut.	628+71.00	0.00	387.69	387.69

GIRDER 4

Location	Station	Offset (ft)	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. of W. Abut.	627+79.58	3.33	387.41	387.41
☉ Brg. W. Abut.	627+80.85	3.33	387.41	387.41
C	627+90.85	3.33	387.44	387.50
D	628+00.85	3.33	387.46	387.58
E	628+10.85	3.33	387.49	387.65
F	628+20.85	3.33	387.51	387.69
G	628+30.85	3.33	387.54	387.72
H	628+40.85	3.33	387.56	387.72
I	628+50.85	3.33	387.59	387.70
J	628+60.85	3.33	387.61	387.67
☉ Brg. E. Abut.	628+70.31	3.33	387.64	387.64
Bk. of E. Abut.	628+71.58	3.33	387.64	387.64

GIRDER 5

Location	Station	Offset (ft)	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. of W. Abut.	627+80.74	10.00	387.31	387.31
☉ Brg. W. Abut.	627+82.01	10.00	387.31	387.31
C	627+92.01	10.00	387.34	387.40
D	628+02.01	10.00	387.36	387.48
E	628+12.01	10.00	387.39	387.54
F	628+22.01	10.00	387.41	387.59
G	628+32.01	10.00	387.44	387.61
H	628+42.01	10.00	387.46	387.62
I	628+52.01	10.00	387.49	387.60
J	628+62.01	10.00	387.51	387.57
☉ Brg. E. Abut.	628+71.47	10.00	387.53	387.53
Bk. of E. Abut.	628+72.74	10.00	387.54	387.54

GIRDER 6

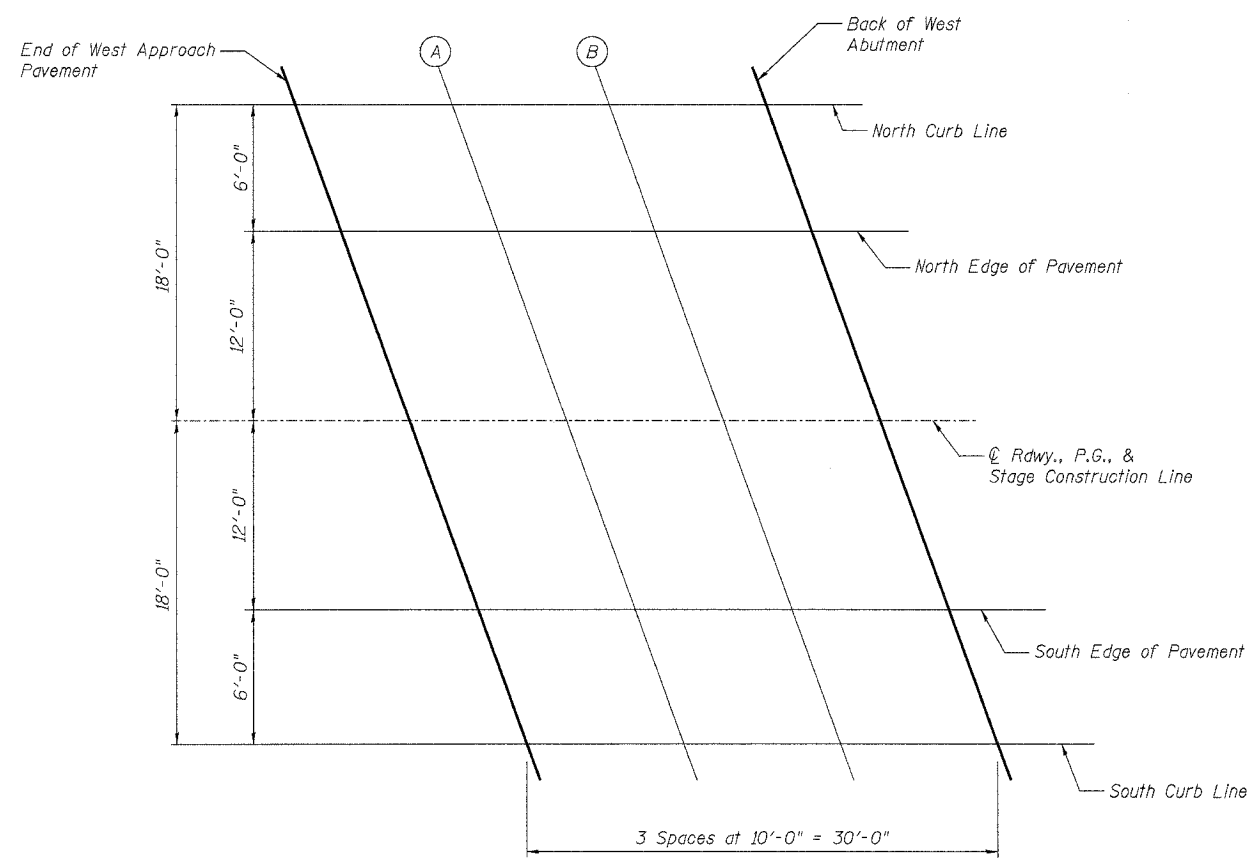
Location	Station	Offset (ft)	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. of W. Abut.	627+81.89	16.67	387.18	387.18
☉ Brg. W. Abut.	627+83.16	16.67	387.19	387.19
C	627+93.16	16.67	387.21	387.27
D	628+03.16	16.67	387.24	387.35
E	628+13.16	16.67	387.26	387.41
F	628+23.16	16.67	387.29	387.46
G	628+33.16	16.67	387.31	387.48
H	628+43.16	16.67	387.34	387.48
I	628+53.16	16.67	387.36	387.47
J	628+63.16	16.67	387.39	387.44
☉ Brg. E. Abut.	628+72.62	16.67	387.41	387.41
Bk. of E. Abut.	628+73.89	16.67	387.41	387.41

ILLINOIS DEPARTMENT OF TRANSPORTATION
DECK ELEVATIONS
 ILLINOIS ROUTE 146 OVER
 BUCK RUN CREEK
 F.A.P. ROUTE 885 - SEC. (107A)B-1
 JOHNSON COUNTY
 STA. 628+25.00
 STRUCTURE NO. 044-0059

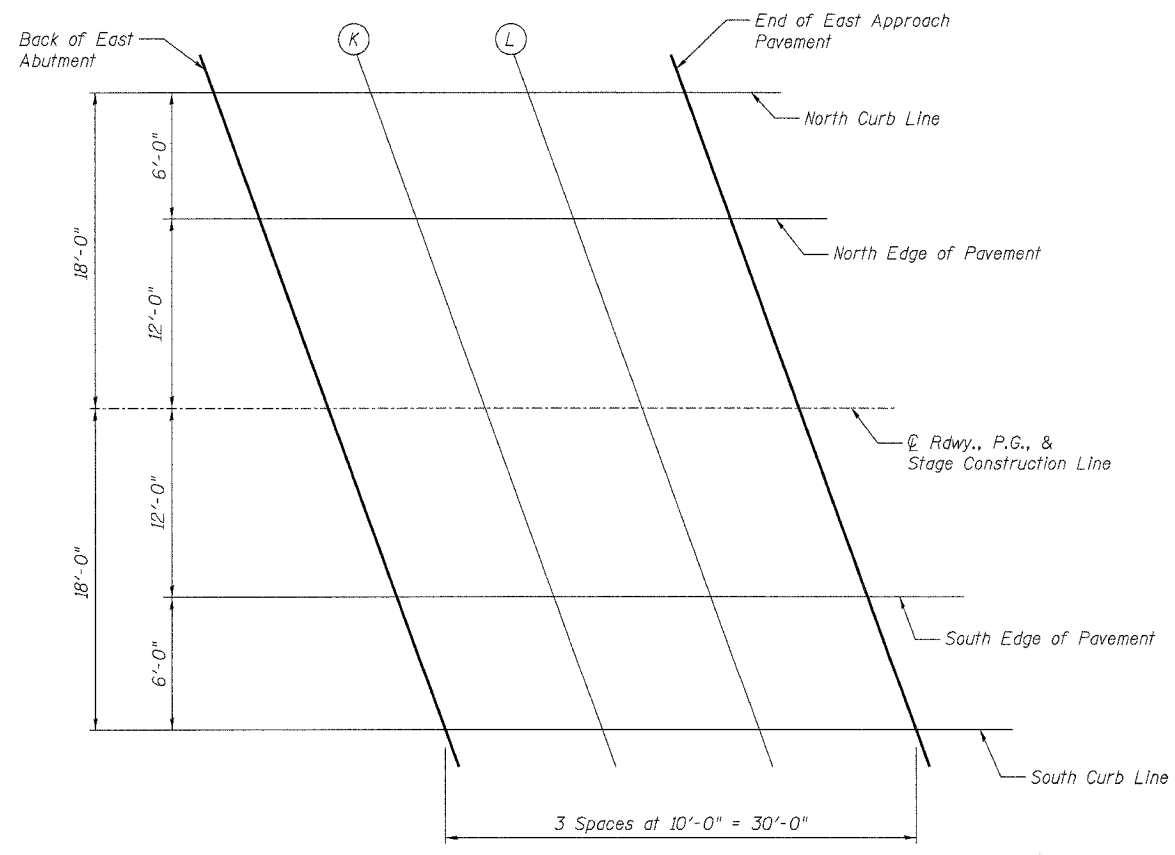
Lin Engineering, Ltd.
 Consulting Engineers
 Channah, Illinois
 Designed By: KWH
 Checked By: WTH
 Date: 3/2007
 Drawn By: AJF
 File: 044-0059.DGN

REVISIONS	
NAME	DATE

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PLAN - WEST APPROACH



PLAN - EAST APPROACH

NORTH CURB LINE

Location	Station	Offset (ft)	Theoretical Grade Elevations
End W. Appr. Pav't.	627+45.87	-18.00	387.23
A	627+55.87	-18.00	387.18
B	627+65.87	-18.00	387.15
Bk. W. Abut.	627+75.87	-18.00	387.14
Bk. E. Abut.	628+67.87	-18.00	387.37
K	628+77.87	-18.00	387.39
L	628+87.87	-18.00	387.41
End E. Appr. Pav't	628+97.87	-18.00	387.45

NORTH EDGE OF PAVEMENT

Location	Station	Offset (ft)	Theoretical Grade Elevations
End W. Appr. Pav't.	627+46.92	-12.00	387.35
A	627+56.92	-12.00	387.30
B	627+66.92	-12.00	387.27
Bk. W. Abut.	627+76.92	-12.00	387.27
Bk. E. Abut.	628+68.92	-12.00	387.50
K	628+78.92	-12.00	387.51
L	628+88.92	-12.00	387.53
End E. Appr. Pav't	628+98.92	-12.00	387.58

CL RDWY., P.G., & STAGE CONSTRUCTION LINE

Location	Station	Offset (ft)	Theoretical Grade Elevations
End W. Appr. Pav't.	627+49.00	0.00	387.53
A	627+59.00	0.00	387.48
B	627+69.00	0.00	387.46
Bk. W. Abut.	627+79.00	0.00	387.46
Bk. E. Abut.	628+71.00	0.00	387.69
K	628+81.00	0.00	387.70
L	628+91.00	0.00	387.73
End E. Appr. Pav't	629+01.00	0.00	387.78

SOUTH EDGE OF PAVEMENT

Location	Station	Offset (ft)	Theoretical Grade Elevations
End W. Appr. Pav't.	627+51.08	12.00	387.33
A	627+61.08	12.00	387.29
B	627+71.08	12.00	387.27
Bk. W. Abut.	627+81.08	12.00	387.28
Bk. E. Abut.	628+73.08	12.00	387.51
K	628+83.08	12.00	387.52
L	628+93.08	12.00	387.55
End E. Appr. Pav't	629+03.08	12.00	387.60

SOUTH CURB LINE

Location	Station	Offset (ft)	Theoretical Grade Elevations
End W. Appr. Pav't.	627+52.13	18.00	387.20
A	627+62.13	18.00	387.16
B	627+72.13	18.00	387.14
Bk. W. Abut.	627+82.13	18.00	387.16
Bk. E. Abut.	628+74.13	18.00	387.39
K	628+84.13	18.00	387.40
L	628+94.13	18.00	387.43
End E. Appr. Pav't	629+04.13	18.00	387.48

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LIN ENGINEERING, LTD.
Consulting Engineers
Channah, Illinois

Designed By: KHH Checked By: WTH Drawn By: AJF
Date: 3/2007 File: 044-0059.DGN

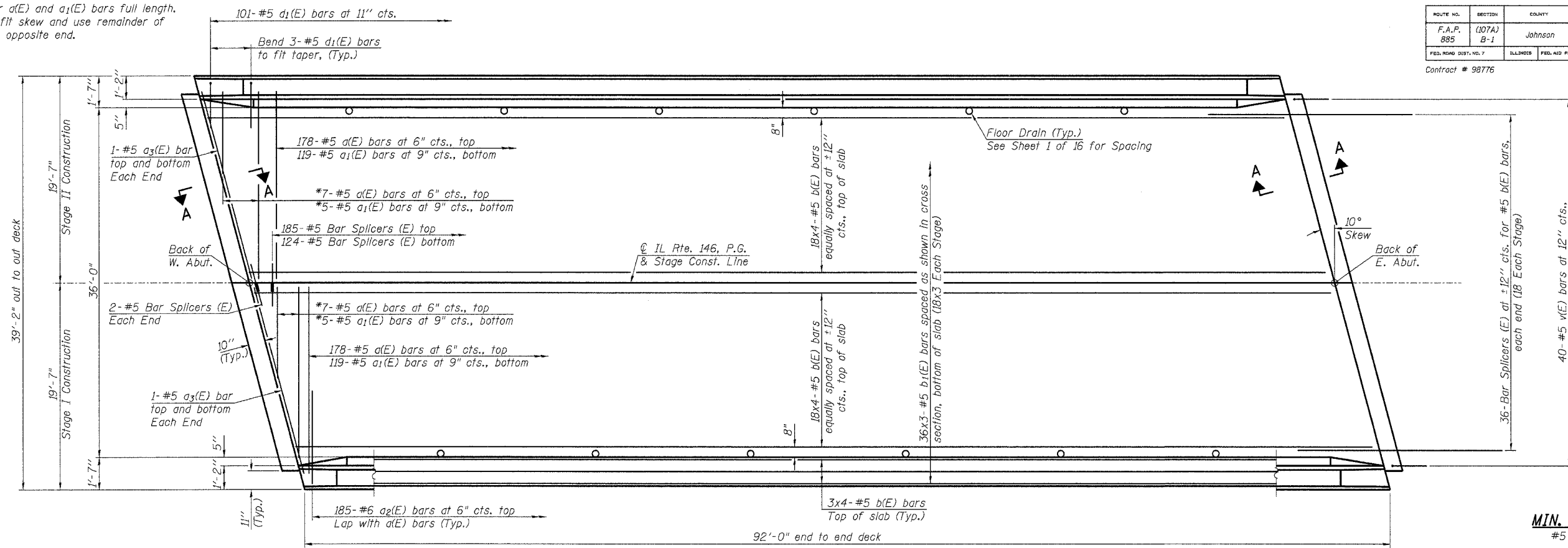
REVISIONS	
NAME	DATE

ILLINOIS DEPARTMENT OF TRANSPORTATION
APPROACH PAVEMENT ELEVATIONS
ILLINOIS ROUTE 146 OVER
BUCK RUN CREEK
F.A.P. ROUTE 885 - SEC. (107A)B-1
JOHNSON COUNTY
STA. 628+25.00
STRUCTURE NO. 044-0059

* Order a(E) and a₁(E) bars full length. Cut to fit skew and use remainder of bars in opposite end.

ROUTE NO. F.A.P. 885	SECTION (107A) B-1	COUNTY Johnson	SHEET 38	DATE 28	SHEET NO. 6 16 SHEETS
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT-		

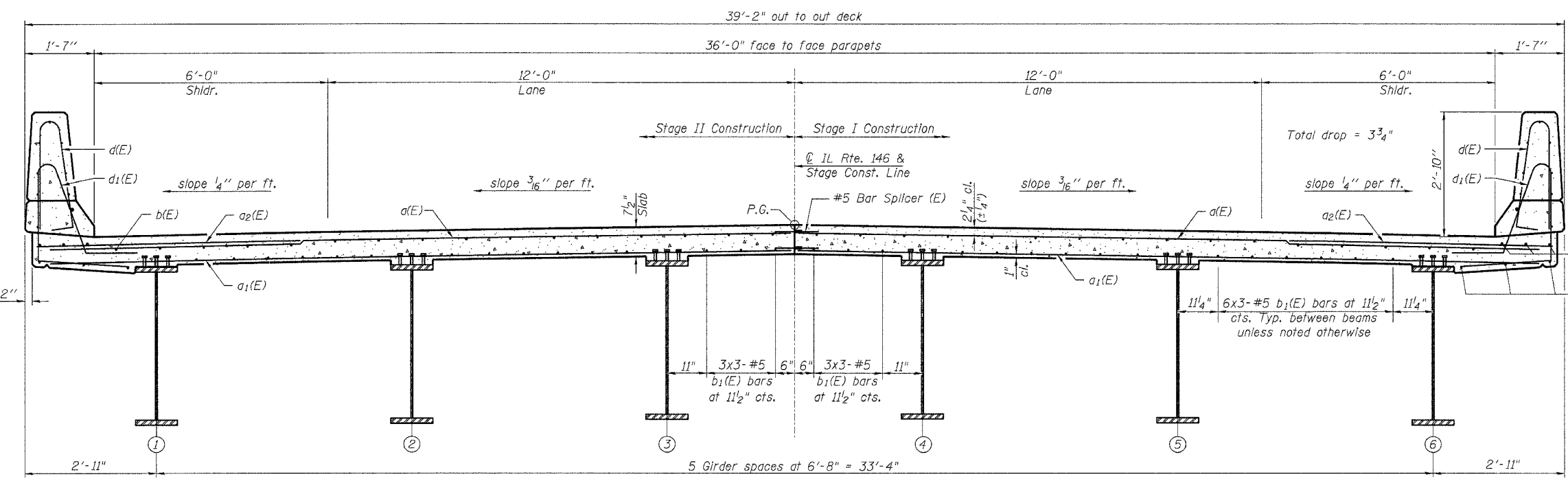
Contract # 98776



PLAN

MIN. BAR LAP
#5 bar = 1'-8"

Notes:
See Sheet 7 of 16 for superstructure details and Bill of Material.
Bars indicated thus 36 x 3-#5 etc. Indicates 36 lines of bars with 3 lengths per line.
See Sheet 7 of 16 for parapet reinforcement.
See Sheet 8 of 16 for section A-A.
See Sheet 13 of 16 for Bar Splicer Details.



CROSS SECTION
(Looking East)

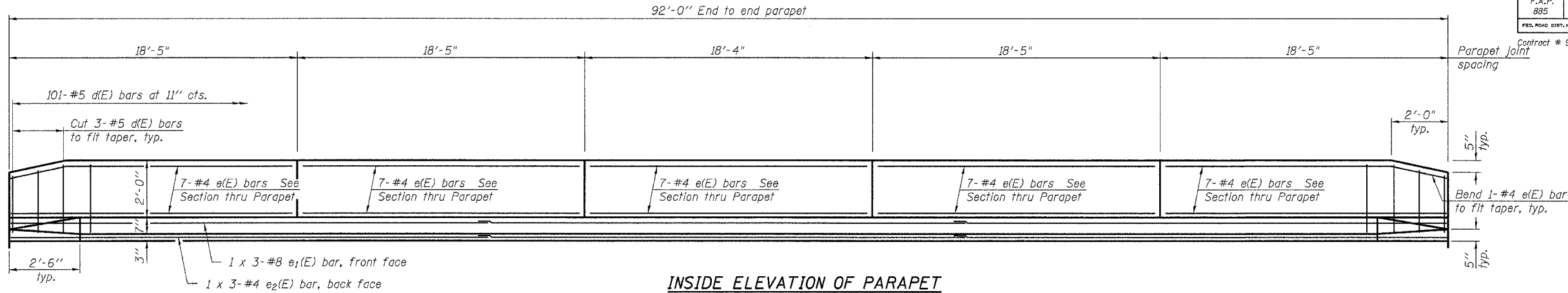
ILLINOIS DEPARTMENT OF TRANSPORTATION
SUPERSTRUCTURE
ILLINOIS ROUTE 146 OVER
BUCK RUN CREEK
F.A.P. ROUTE 885 - SEC. (107A)B-1
JOHNSON COUNTY
STA. 628+25.00
STRUCTURE NO. 044-0059

Lin Engineering, Ltd.
Consulting Engineers
Chatham, Illinois

REVISIONS	
NAME	DATE

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Contract # 98776



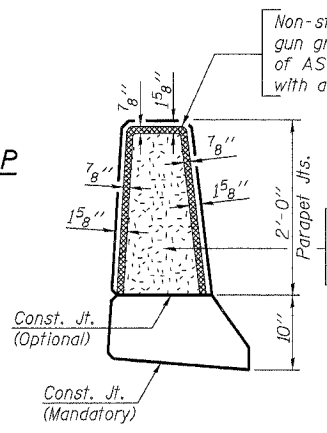
INSIDE ELEVATION OF PARAPET

SUPERSTRUCTURE BILL OF MATERIAL

Bar	No.	Size	Length	Shape	
d(E)	370	#5	18'-11"	—	
a1(E)	124	#5	18'-7"	—	
a2(E)	370	#6	6'-0"	—	
a3(E)	8	#5	19'-1"	—	
b(E)	168	#5	24'-2"	—	
b1(E)	108	#5	31'-8"	—	
d(E)	202	#5	5'-7"	⌒	
d1(E)	202	#5	7'-6"	⌒	
e(E)	70	#4	18'-2"	—	
e1(E)	6	#8	32'-10"	—	
e2(E)	6	#4	31'-6"	—	
m(E)	8	#6	18'-8"	—	
m1(E)	12	#6	19'-6"	—	
m2(E)	24	#6	8'-5"	—	
m3(E)	8	#6	5'-5"	—	
m4(E)	4	#6	2'-0"	—	
m5(E)	4	#6	2'-7"	—	
s(E)	82	#5	5'-9"	⌒	
s1(E)	72	#4	12'-4"	⌒	
v(E)	80	#5	3'-4"	⌒	
Reinforcement Bars, Epoxy Coated				Pound	27600
Concrete Superstructure				Cu. Yds.	141.4
Floor Drains				Each	12

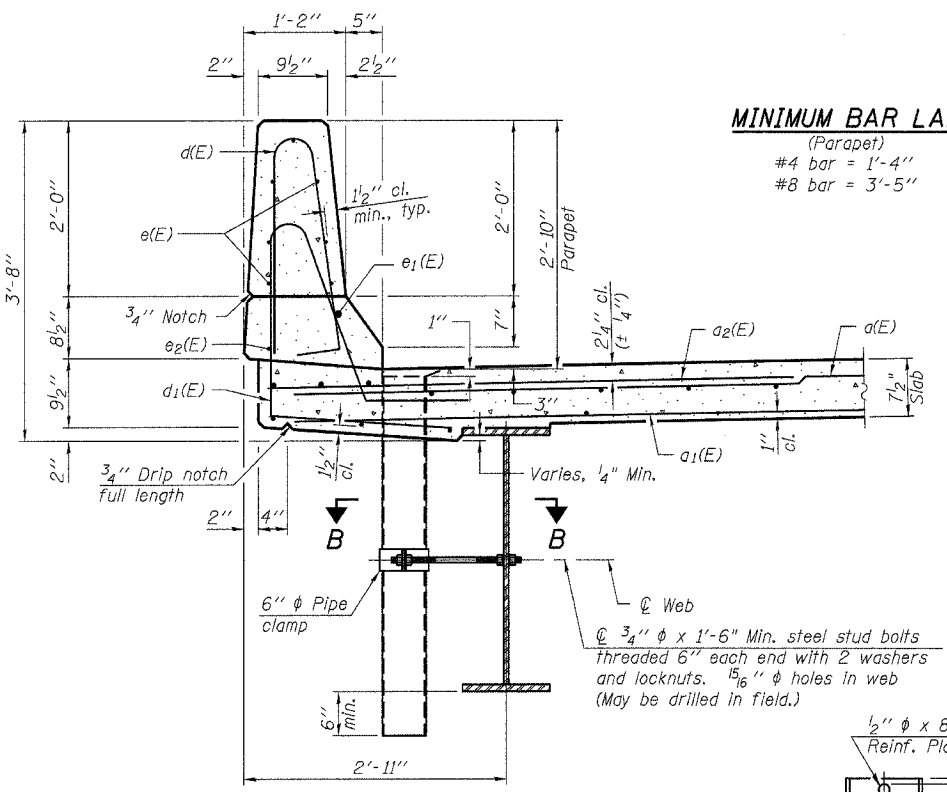
Bars Indicated thus 1 x 3 #8 etc. Indicates 1 line of bars with 3 lengths per line.

MINIMUM BAR LAP
(Parapet)
#4 bar = 1'-4"
#8 bar = 3'-5"

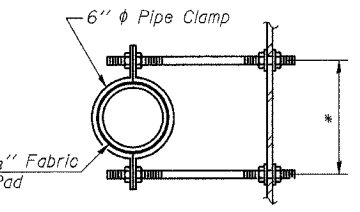


PARAPET JOINT DETAILS

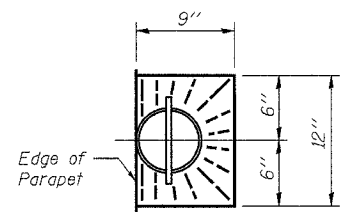
Notes:
The exterior surfaces of the floor drains shall be painted with the finish coat as specified in the special provisions for Cleaning and Painting New Metal Structures. The exterior surfaces of the drains shall be cleaned according to Steel Structures Painting Council's Spec. SSPC-SPI prior to painting.
Fiberglass pipe shall conform to ASTM D 2996, with short-time rupture strength hoop tensile stress of 30,000 p.s.i. minimum.



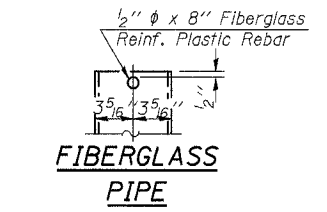
SECTION THRU PARAPET



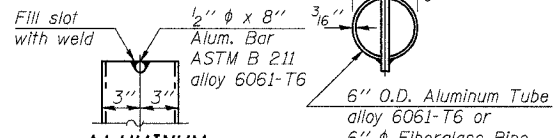
SECTION B-B
* Dimension as required by Pipe Clamp



TOP PLAN

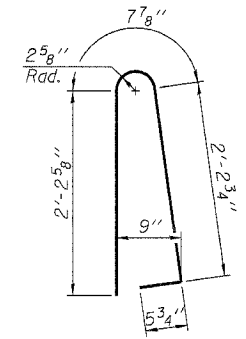


FIBERGLASS PIPE

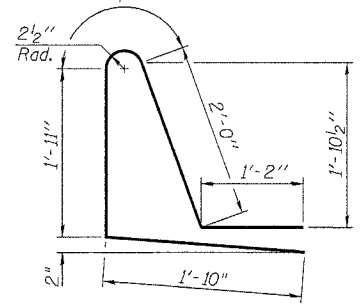


ALUMINUM TUBE

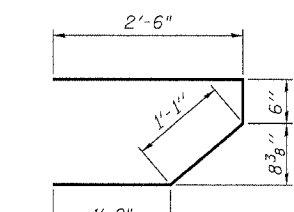
TOP PLAN
(Showing Aluminum Tube)



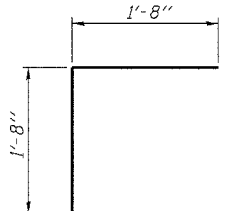
BAR d(E)



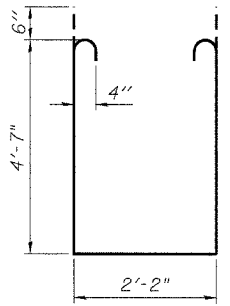
BAR d1(E)



BAR s(E)



BAR v(E)



BAR s1(E)

REVISIONS

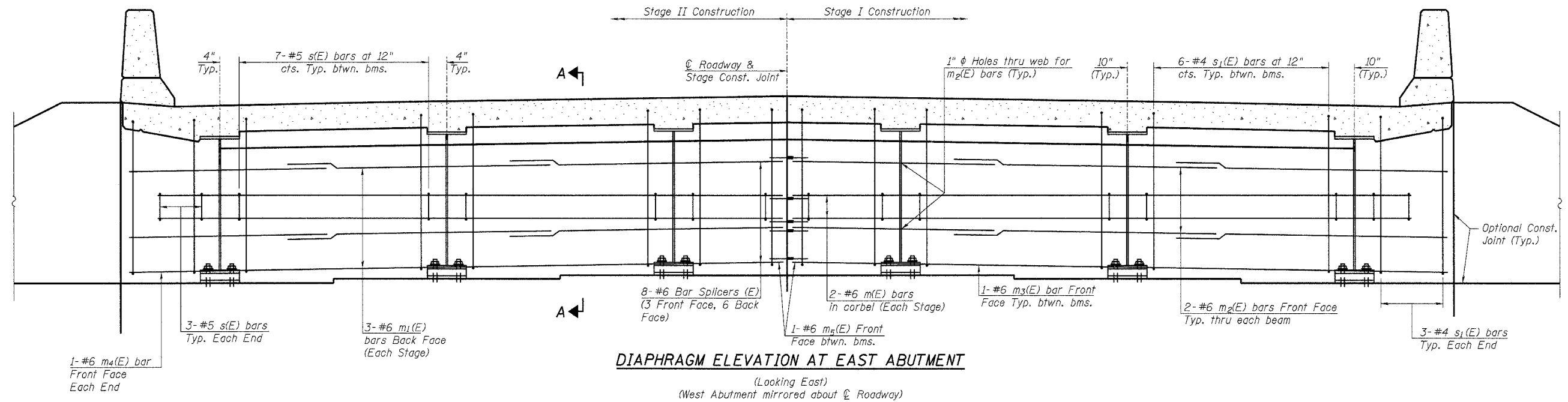
NO.	NAME	DATE

Lin Engineering, Ltd.
Consulting Engineers
Chester, Illinois

Designed By: KKH Checked By: MTH Drawn By: AJF
Date: 3/2007 File: 044-0059.dgn

ILLINOIS DEPARTMENT OF TRANSPORTATION
SUPERSTRUCTURE DETAILS
ILLINOIS ROUTE 146 OVER
BUCK RUN CREEK
F.A.P. ROUTE 885 - SEC. (107A)B-1
JOHNSON COUNTY
STA. 628+25.00
STRUCTURE NO. 044-0059

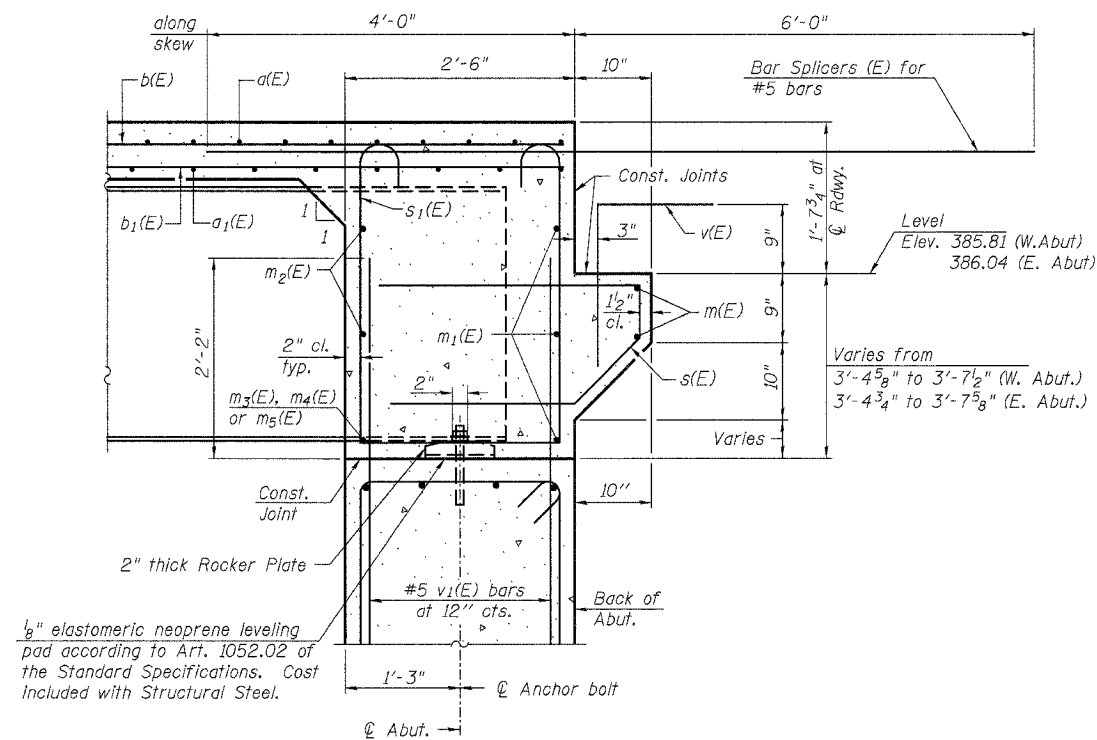
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DIAPHRAGM ELEVATION AT EAST ABUTMENT
(Looking East)
(West Abutment mirrored about ϕ Roadway)

Notes:
Reinforcement bars in diaphragm are billed with superstructure on sheet 7 of 16.
Concrete in diaphragm is Included with Concrete Superstructure on sheet 7 of 16.
For details of bars s(E) & s1(E) see sheet 7 of 16.
The s(E) and s1(E) bars shall be placed parallel to the beams. Spacing for these bars shall be at right angles to the beams.
For location of holes thru web, see sheet 9 of 16.

MIN. BAR LAP
#6 bar = 2'-9"



SECTION A-A
Dimensions at right angles to abutment, except as shown.

LIN ENGINEERING, LTD.
Consulting Engineers
Chatham, Illinois

Designed By: KJH Checked By: MTH Drawn By: AJF
Date: 3/2007 File: 044-0059.DWG

REVISIONS	
NAME	DATE

ILLINOIS DEPARTMENT OF TRANSPORTATION
CONCRETE END DIAPHRAGMS
ILLINOIS ROUTE 146 OVER
BUCK RUN CREEK
F.A.P. ROUTE 885 - SEC. (107A)B-1
JOHNSON COUNTY
STA. 628+25.00
STRUCTURE NO. 044-0059

	0.5 Span
I_s	(in ⁴) 165.10
$I_c(n)$	(in ⁴) 39246
$I_c(3n)$	(in ⁴) 29261
S_s	(in ³) 667
$S_c(n)$	(in ³) 916
$S_c(3n)$	(in ³) 837
DC1	(k/ft) 0.811
M _{DC1}	(k) 811
DC2	(k/ft) 0.150
M _{DC2}	(k) 150
DW	(k/ft) 0.300
M _{DW}	(k) 300
M _{κ + Imp}	(k) 1364
M _u (Strength I)	(k) 4038
$\phi_r M_n$	(k) 4859
f_s DC1	(ksi) 14.59
f_s DC2	(ksi) 2.15
f_s DW	(ksi) 4.30
f_s 1.3(κ +I)	(ksi) 23.23
f_s (Service II)	(ksi) 44.27
V_f	(k) 31.5

I_s, S_s : Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total-Strength I, and Service II) due to non-composite dead loads (in⁴ and in³).

$I_c(n), S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total-Strength I, and Service II) due to short-term composite live loads (in⁴ and in³).

$I_c(3n), S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total-Strength I, and Service II) due to long-term composite (superimposed) dead loads (in⁴ and in³).

DC1: Un-factored non-composite dead load (kips/ft.).

M_{DC1}: Un-factored moment due to non-composite dead load (kip-ft.).

DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).

M_{DC2}: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).

DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).

M_{DW}: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).

M_{κ + Imp}: Un-factored live load moment plus dynamic load allowance (Impact) (kip-ft.).

M_u (Strength I): Factored design moment (kip-ft.).

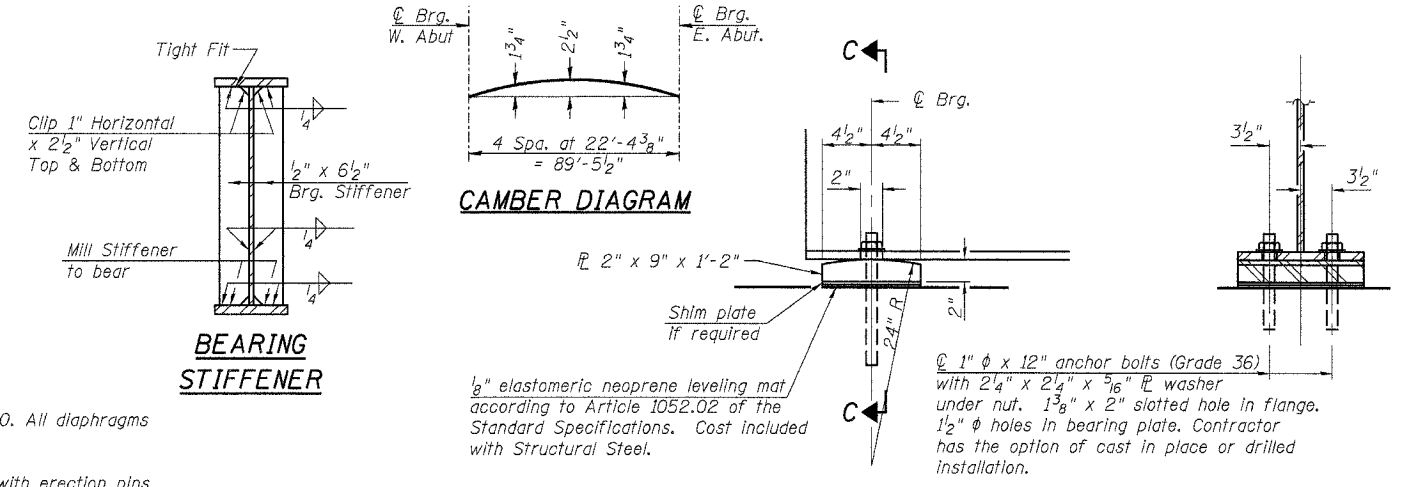
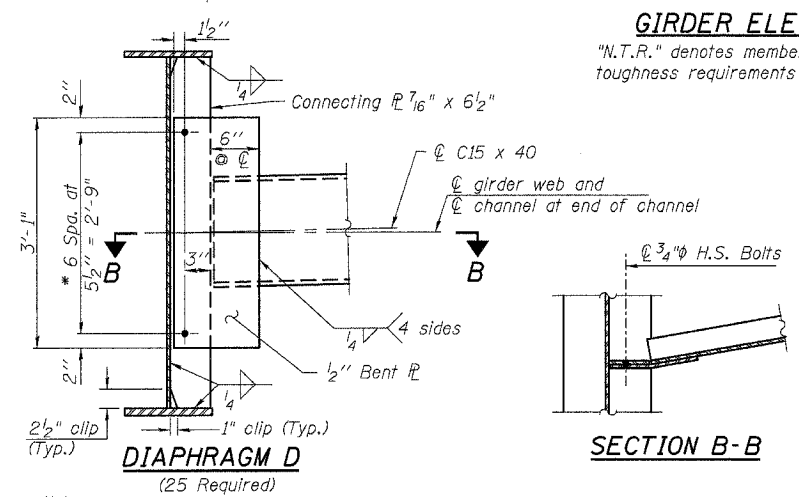
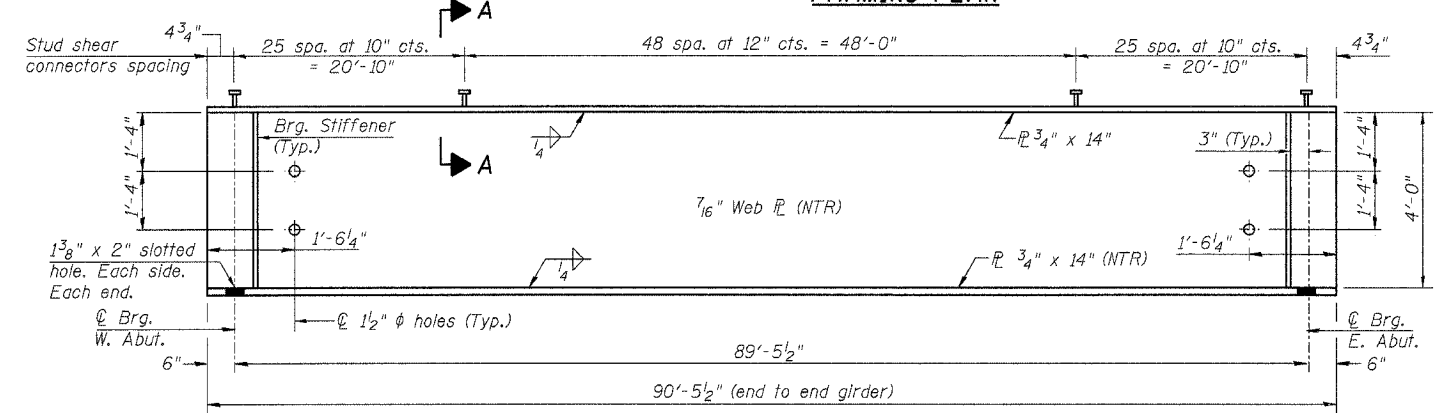
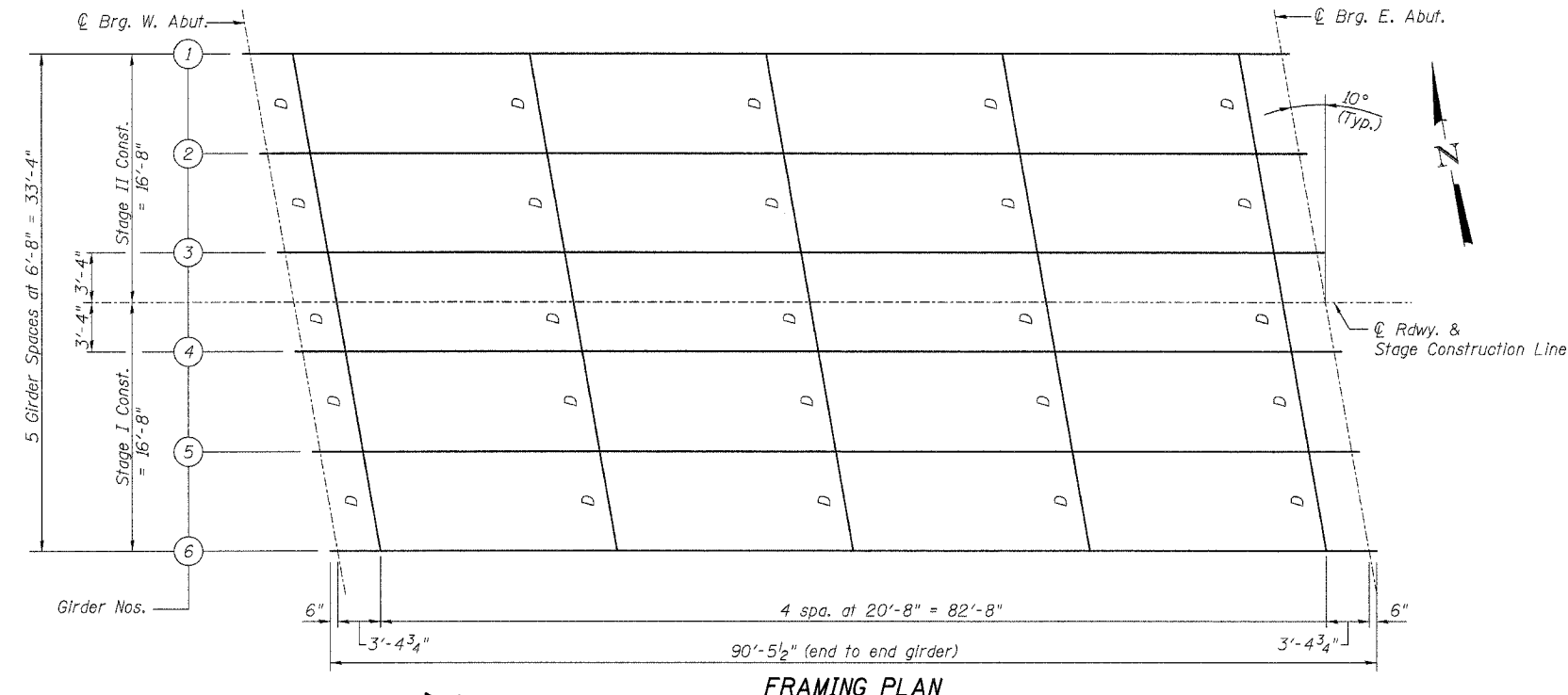
$\phi_r M_n$: Compact composite positive moment capacity computed according to Article 6.10.7.1 (kip-ft.).

f_s (Service II): Sum of stresses as computed from the moments below (ksi).

M_{DC1} + M_{DC2} + M_{DW} + 1.3 M_{κ + Imp}

V_f : Factored shear range computed according to Article 6.10.10.

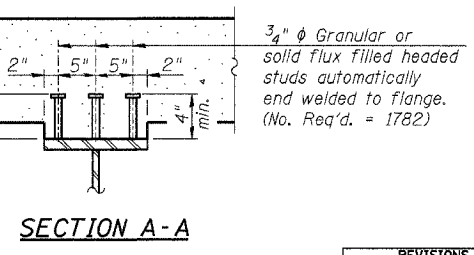
	Abut.
R _{DC1}	(k) 36.3
R _{DC2}	(k) 6.7
R _{DW}	(k) 13.4
R _{κ + Imp}	(k) 85.2
R _{Total}	(k) 141.6



- NOTES:**
- All girders and bearing stiffeners shall be AASHTO M270 Grade 50. All diaphragms and connecting plates shall be AASHTO M270 Grade 36.
 - All diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted.
 - Load carrying components designated "NTR" shall conform to the Supplemental Requirements for Notch Toughness, Zone 2.
 - Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. ASTM A307 Grade C anchor bolts may be used in lieu of ASTM F1554 Grade 36 (F_y=36ksi). The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.
 - Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.

TOP OF WEB ELEVATIONS
(For Fabrication Only)

Location	Girder 1	Girder 2	Girder 3	Girder 4	Girder 5	Girder 6
⊙ Brg. W. Abut.	386.42	386.55	386.66	386.66	386.56	386.44
⊙ Brg. E. Abut.	386.65	386.78	386.88	386.89	386.79	386.66



REVISIONS

NAME	DATE

REVISIONS

NAME	DATE

DESIGNED BY: KWH
CHECKED BY: MTH
DATE: 3/2007

FILE: 044-0059.DGN
DRAWN BY: AUF

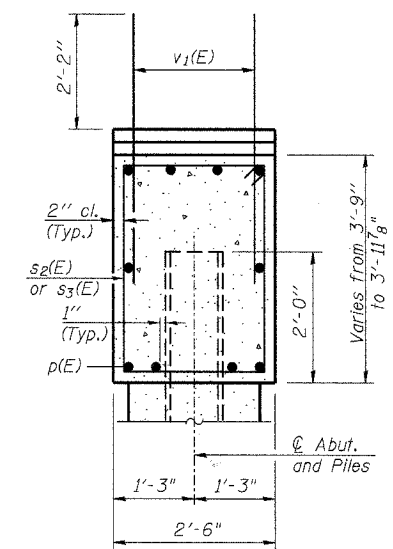
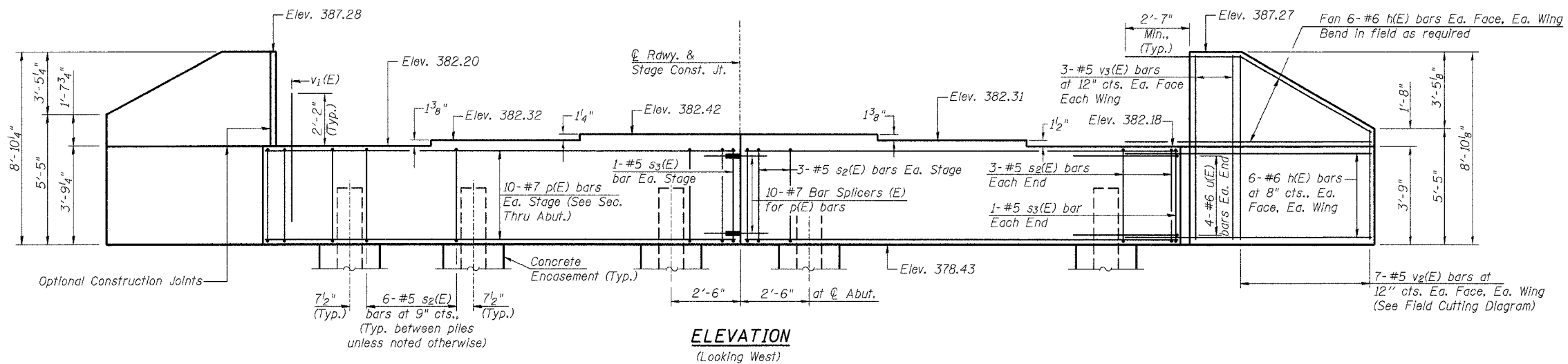
ILLINOIS DEPARTMENT OF TRANSPORTATION
FRAMING PLAN & STEEL DETAILS
 ILLINOIS ROUTE 146 OVER
 BUCK RUN CREEK
 F.A.P. ROUTE 885 - SEC. (107A)B-1
 JOHNSON COUNTY
 STA. 628+25.00
 STRUCTURE NO. 044-0059

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Notes:
 Pour steps monolithically with cap.
 Space reinforcement in cap to miss Anchor Bolts.

ROUTE NO. F.A.P. 885	SECTION (107A) B-1	COUNTY Johnson	TOTAL SHEETS 38	SHEET NO. 32	SHEET NO. 10 16 SHEETS
FED. ROAD DIST. NO. 7		ILLINOIS		FED. AID PROJECT-	

Contract # 98776

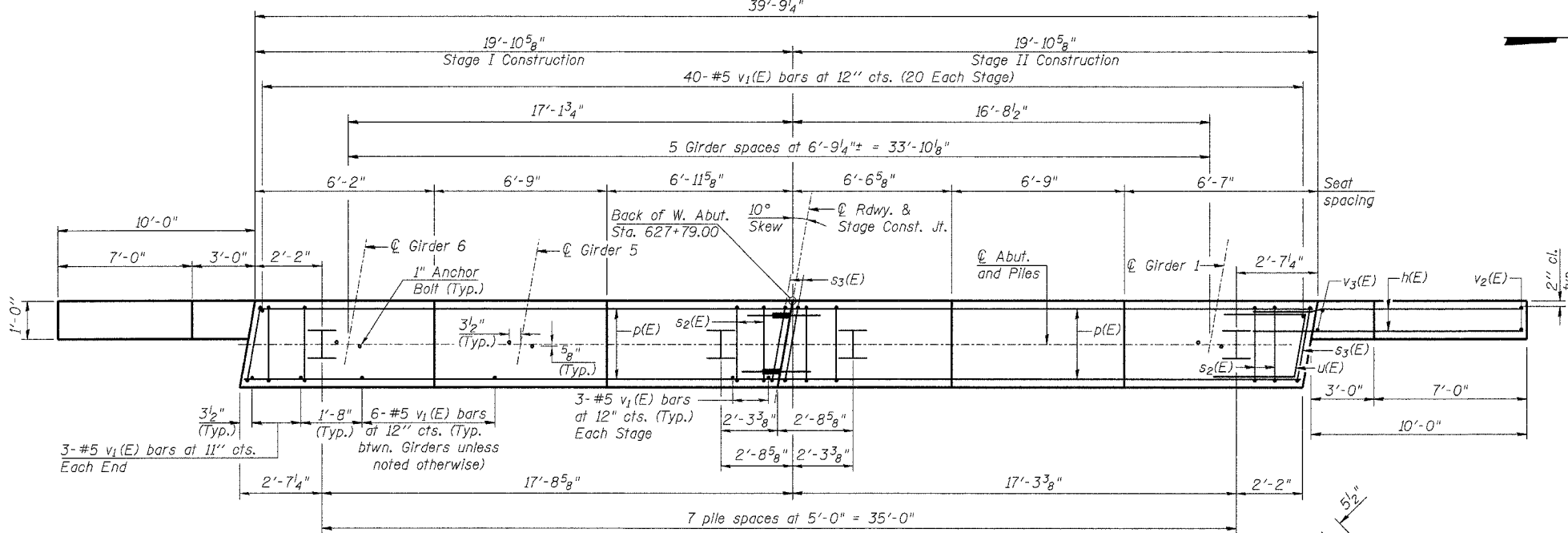


SEC. THRU ABUT.
 (Dimensions at Rt. L's)

BILL OF MATERIAL
 (West Abutment)

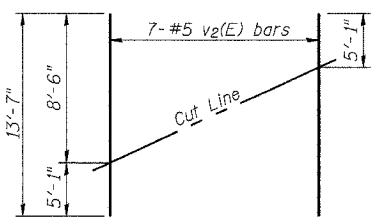
Bar	No.	Size	Length	Shape
h(E)	48	#6	12'-6"	—
p(E)	20	#7	19'-7"	—
s2(E)	48	#5	12'-1"	□
s3(E)	4	#5	12'-3"	□
u(E)	8	#6	7'-3"	┌
v1(E)	76	#5	4'-4"	—
v2(E)	14	#5	13'-7"	—
v3(E)	12	#5	8'-6"	—
Structure Excavation		Cu. Yd.	131	
Concrete Structures		Cu. Yd.	14.9	
Reinforcement Bars, Epoxy Coated		Pound	3100	
Furnishing Steel Piles HP12x53		Foot	400	
Driving Piles		Foot	400	
Pile Shoes		Each	8	
Concrete Encasement		Cu. Yd.	2.8	
Anchor Bolts, 1"		Each	12	

For details of Bar Splacers, see sheet 13 of 16.
 For details of piles and Concrete Encasement, see sheet 14 of 16.
 For details of Integral Abutment Bearing, see sheet 9 of 16.
 For drainage details, see Section Thru Integral Abutment on sheet 2 of 16.



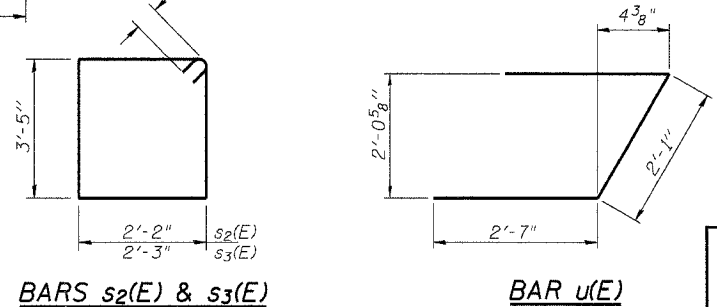
PLAN

PILE DATA
 Type: Steel HP12x53 with pile shoes
 Nominal Required Bearing: 419 kips
 Factored Resistance Available: 209 kips
 Est. Length: 50 ft
 No. Production Piles: 8



FIELD CUTTING DIAGRAM

Order v2(E) full length. Cut as shown and use remainder of bars in opposite face.



BARS s2(E) & s3(E)

BAR u(E)

REVISIONS

NO.	NAME	DATE

LIN ENGINEERING, LTD.
 Consulting Engineers
 Chatham, Illinois

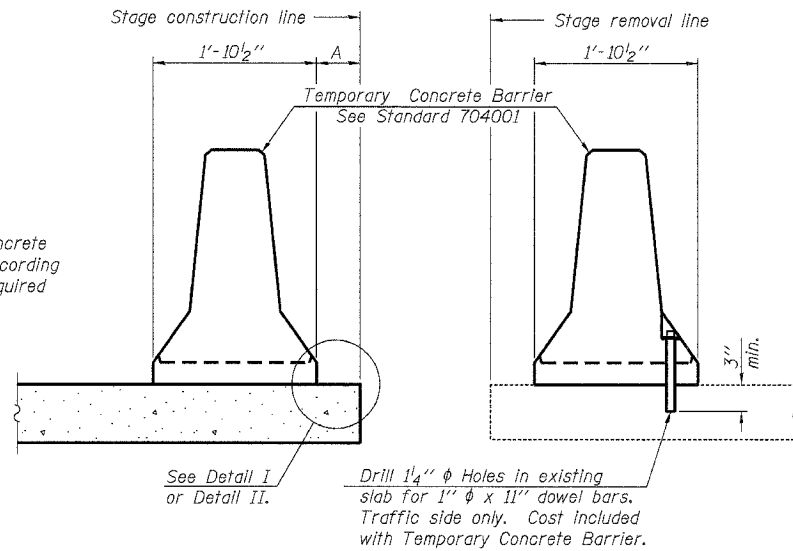
Designed By: KKH Checked By: MTH Drawn By: AJF
 Date: 3/2007 File: 044-005L.DWG

ILLINOIS DEPARTMENT OF TRANSPORTATION
WEST ABUTMENT
 ILLINOIS ROUTE 146 OVER
 BUCK RUN CREEK
 F.A.P. ROUTE 885 - SEC. (107A)B-1
 JOHNSON COUNTY
 STA. 628+25.00
 STRUCTURE NO. 044-0059

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ROUTE NO. F.A.P. 885	SECTION (107A) B-1	COUNTY Johnson	TOTAL SHEETS 38	SHEET NO. 34	SHEET NO. 12 16 SHEETS
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT-		

Contract # 98776



When "A" is 3'-6" or less, the temporary concrete barrier shall be anchored to the new slab according to Detail I or Detail II. No anchorage is required when "A" is greater than 3'-6".

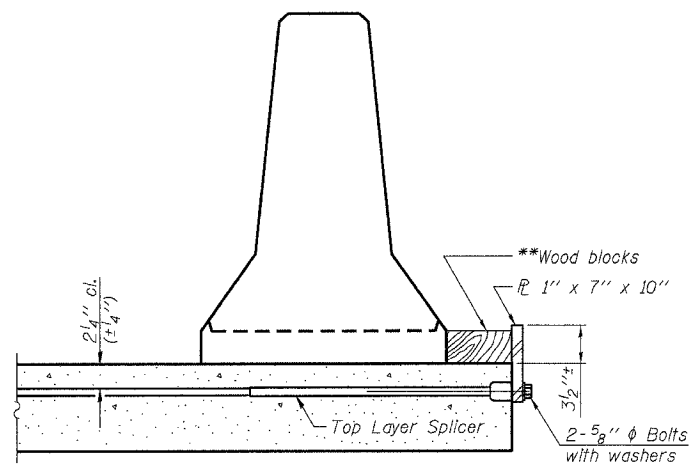
NEW SLAB

EXISTING SLAB

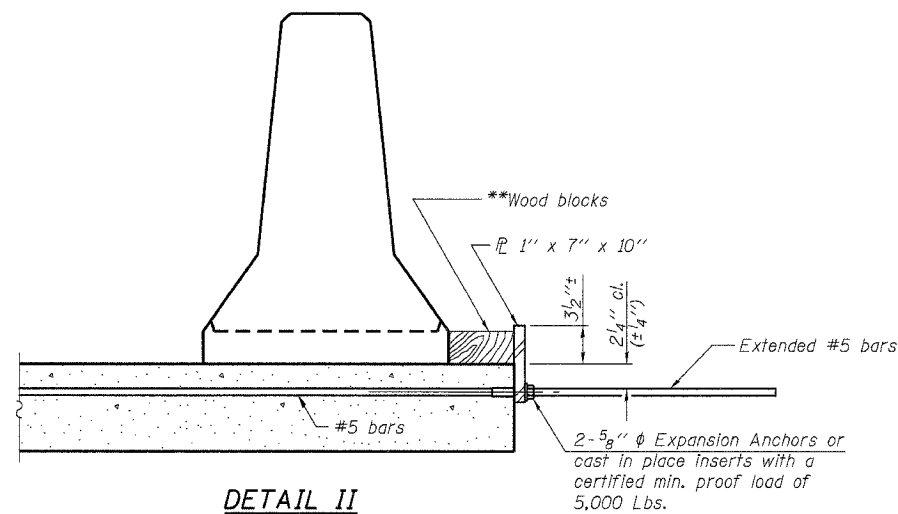
SECTIONS THRU SLAB

NOTES

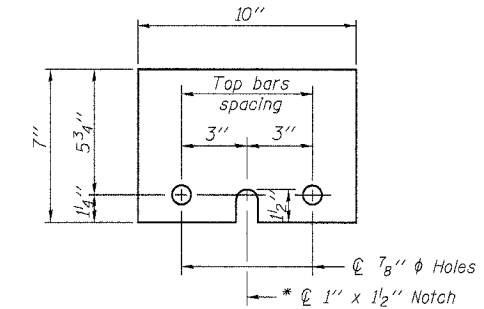
- Detail I - With Bar Splicer or Couplers:
Connect one (1) 1"x7"x10" steel \bar{P} to the top layer of couplers with 2- $\frac{5}{8}$ " ϕ bolts screwed to coupler at approximate \bar{C} of each barrier panel.
- Detail II - With Extended Reinforcement Bars:
Connect one (1) 1"x7"x10" steel \bar{P} to the concrete slab with 2- $\frac{5}{8}$ " ϕ Expansion Anchors or cast in place inserts spaced between the top layer of reinforcement at approximate \bar{C} of each barrier panel.
- Cost of anchorage is included with Temporary Concrete Barrier. The 1" x 7" x 10" plate shall not be removed until stage II construction forms and all reinforcement bars are in place and the concrete is ready to be placed.



DETAIL I



DETAIL II



STEEL RETAINER \bar{P} 1" x 7" x 10"

* Required only with Detail II

** Wood blocks may be omitted when required to provide minimum stage traffic lane width. When the wood blocks are omitted, the concrete barrier shall be in direct contact with the steel retainer plate.


REVISIONS	
NAME	DATE

Lin Engineering, Ltd.
Consulting Engineers
Chatham, Illinois

Designed By: KKH Checked By: MTH Drawn By: AUF
Date: 3/2007 File: 044-0059.DWG

ILLINOIS DEPARTMENT OF TRANSPORTATION
TEMPORARY CONCRETE BARRIER
ILLINOIS ROUTE 146 OVER
BUCK RUN CREEK
F.A.P. ROUTE 885 - SEC. (107A)B-1
JOHNSON COUNTY
STA. 628+25.00
STRUCTURE NO. 044-0059

The diameter of this part is equal or larger than the diameter of bar spliced.



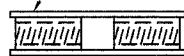
The diameter of this part is the same as the diameter of the bar spliced.

ROLLED THREAD DOWEL BAR



**** ONE PIECE**

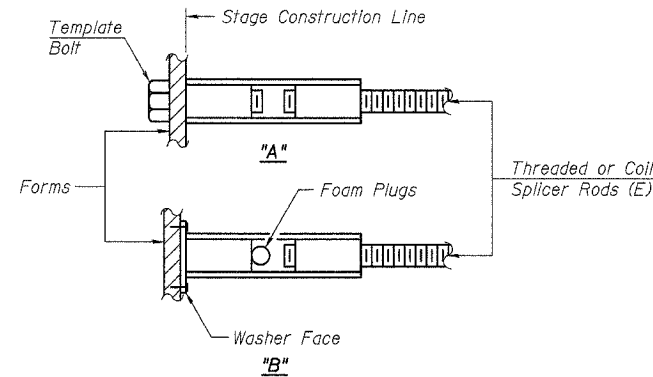
Wire Connector



WELDED SECTIONS

BAR SPLICER ASSEMBLY ALTERNATIVES

** Heavy Hex Nuts conforming to ASTM A 563, Grade C, D or DH may be used.



INSTALLATION AND SETTING METHODS

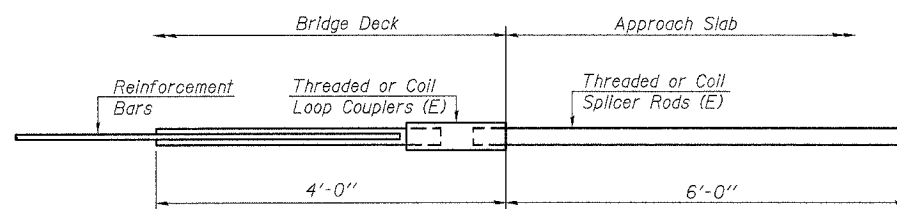
"A" : Set bar splicer assembly by means of a template bolt.
 "B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms.
 (E) : Indicates epoxy coating.

NOTES

Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars.
 Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length.
 All reinforcement bars shall be lapped and tied to the splicer rods or dowel bars.
 Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars.
 Other systems of similar design may be submitted to the Engineer for approval. Approval shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

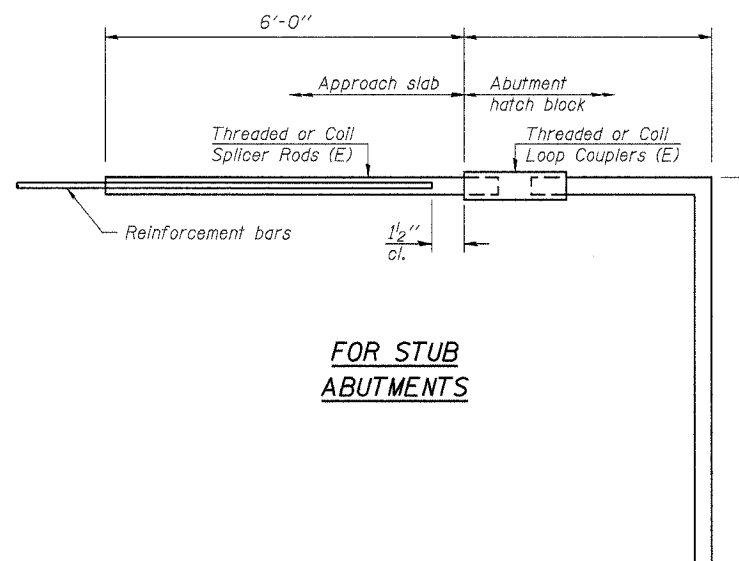
- ① Minimum Capacity (Tension in kips) = $1.25 \times f_y \times A_t$
 - ② Minimum *Pull-out Strength (Tension in kips) = $0.66 \times f_y \times A_t$
- Where f_y = Yield strength of lapped reinforcement bars in ksi.
 A_t = Tensile stress area of lapped reinforcement bars.
 * = 28 day concrete

Bar Size to be Spliced	Splicer Rod or Dowel Bar Length	Strength Requirements	
		Min. Capacity kips - tension	Min. Pull-Out Strength kips - tension
#4	1'-8"	14.7	7.9
#5	2'-0"	23.0	12.3
#6	2'-7"	33.1	17.4
#7	3'-5"	45.1	23.8
#8	4'-6"	58.9	31.3
#9	5'-9"	75.0	39.6
#10	7'-3"	95.0	50.3
#11	9'-0"	117.4	61.8



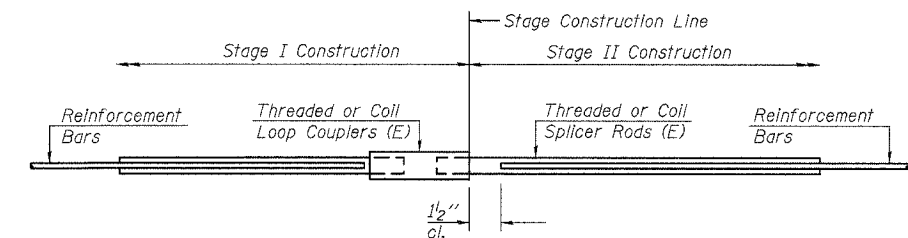
FOR INTEGRAL OR SEMI-INTEGRAL ABUTMENTS

Bar Splicer for #5 bar
Min. Capacity = 23.0 kips - tension
Min. Pull-out Strength = 12.3 kips - tension
No. Required = 72



FOR STUB ABUTMENTS

Bar Splicer for #5 bar
Min. Capacity = 23.0 kips - tension
Min. Pull-out Strength = 12.3 kips - tension
No. Required =



STANDARD

Bar Size	No. Assemblies Required	Location
#5	313	Deck
#6	16	Diaphragms
#7	20	Abutments

ILLINOIS DEPARTMENT OF TRANSPORTATION
BAR SPLICER ASSEMBLY DETAILS
 ILLINOIS ROUTE 146 OVER
 BUCK RUN CREEK
 F.A.P. ROUTE 885 - SEC. (107A)B-1
 JOHNSON COUNTY
 STA. 628+25.00
 STRUCTURE NO. 044-0059

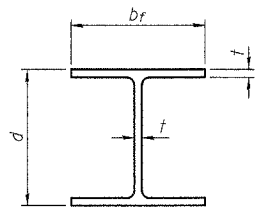
REVISIONS

NAME	DATE

LIN ENGINEERING, LTD.
 Consulting Engineers
 Chatham, Illinois

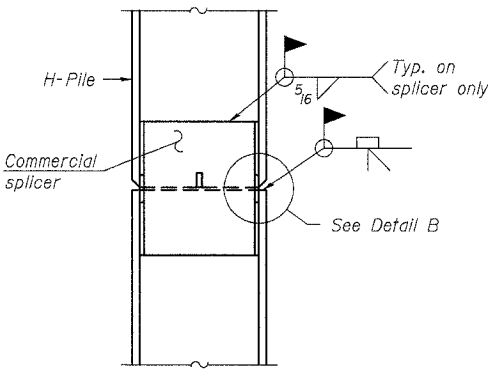
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 Date: 3/2007 File: 044-0059.DWG

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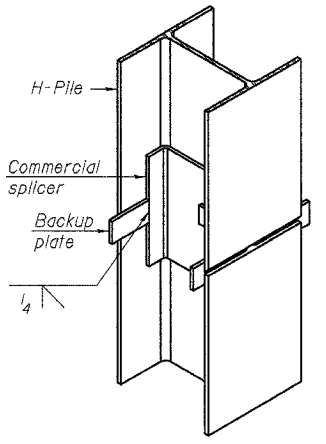


STEEL PILE TABLE

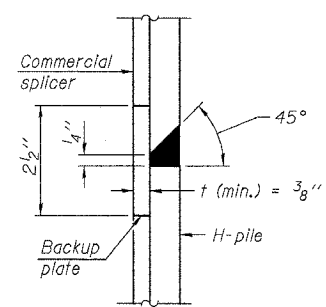
Designation	Depth d	Flange width bf	Web and Flange thickness t	Encasement diameter A
HP 14x117	14 1/4"	14 7/8"	13/16"	30"
x102	14"	14 3/4"	1/16"	30"
x89	13 7/8"	14 3/4"	5/8"	30"
x73	13 5/8"	14 5/8"	1/2"	30"
HP 12x84	12 1/4"	12 1/4"	1/16"	24"
x74	12 1/8"	12 1/4"	5/8"	24"
x63	12"	12 1/8"	1/2"	24"
x53	11 3/4"	12"	7/16"	24"
HP 10x57	10"	10 1/4"	9/16"	24"
x42	9 3/4"	10 1/8"	7/16"	24"
HP 8x36	8"	8 1/8"	7/16"	18"



ELEVATION

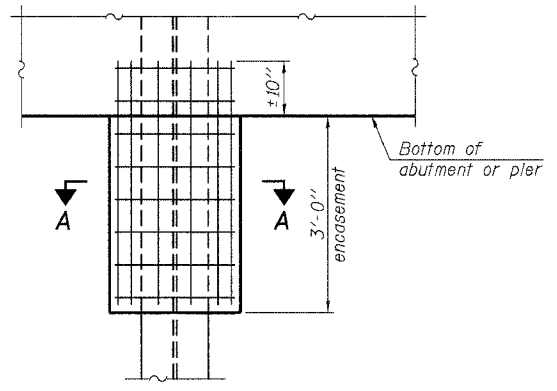


ISOMETRIC VIEW



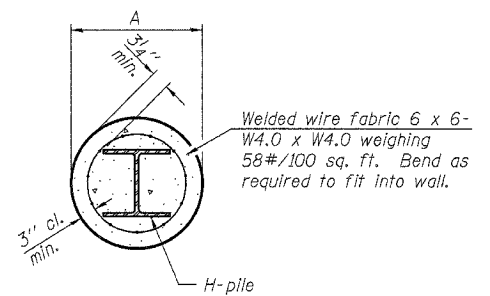
DETAIL "B"

WELDED COMMERCIAL SPLICE



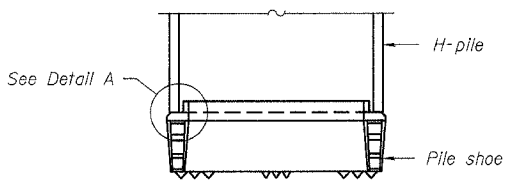
ELEVATION

PILE ENCASEMENT

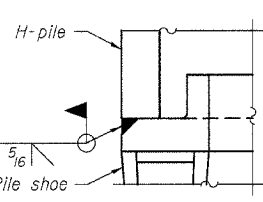


SECTION A-A

Note:
Forms for encasement may be omitted when soil conditions permit.

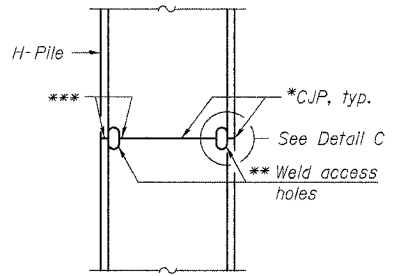


ELEVATION

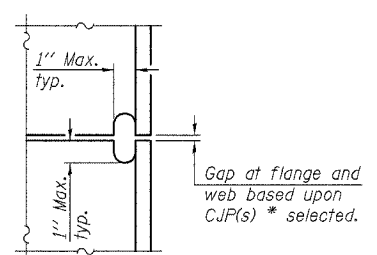


DETAIL A

H-PILE SHOE ATTACHMENT

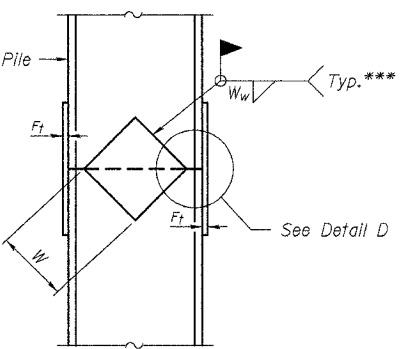


ELEVATION

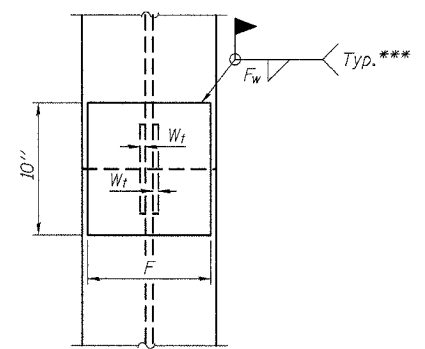


DETAIL C

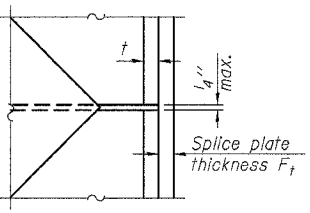
COMPLETE PENETRATION WELD SPLICE



ELEVATION



END VIEW



DETAIL D

WELDED PLATE FIELD SPLICE

Note:
The steel H-piles shall be according to AASHTO M270 Grade 50.

Designation	F	Ft	Fw	W	Wt	Ww
HP 14x117	12 1/2"	1"	7/8"	7 3/4"	5 3/8"	1/2"
x102	12 1/2"	7/8"	3/4"	7 3/4"	5 3/8"	1/2"
x89	12 1/2"	3/4"	1/16"	7 3/4"	5 3/8"	1/2"
x73	12 1/2"	5/8"	9/16"	7 3/4"	5 3/8"	1/2"
HP 12x84	10"	7/8"	1/16"	6 1/2"	5 3/8"	1/2"
x74	10"	7/8"	1/16"	6 1/2"	5 3/8"	1/2"
x63	10"	5/8"	1/2"	6 1/2"	1/2"	3/8"
x53	10"	5/8"	1/2"	6 1/2"	1/2"	3/8"
HP 10x57	8"	3/4"	9/16"	5 1/4"	1/2"	3/8"
x42	8"	5/8"	9/16"	5 1/4"	1/2"	3/8"
HP 8x36	7"	5/8"	7/16"	4 1/4"	1/2"	3/8"

- * Use joint conforming to Figure 3.4 in AWS D1.1, Structure Welding Code - Steel.
- ** Preparation per Fig. 5.2 in AWS D1.1, Structure Welding Code - Steel.
- *** Interrupt welds 1/4" from end of each pile.

REVISIONS

NAME	DATE

LIN ENGINEERING, LTD.
Consulting Engineers
Chatham, Illinois

Designed By: KKH Checked By: MTH Drawn By: AUF
Date: 3/2007 File: 044-0058.DGN

ILLINOIS DEPARTMENT OF TRANSPORTATION
STEEL PILE DETAILS
ILLINOIS ROUTE 146 OVER
BUCK RUN CREEK
F.A.P. ROUTE 885 - SEC. (107A)B-1
JOHNSON COUNTY
STA. 628+25.00
STRUCTURE NO. 044-0059

8/2/2007 1:50:45 PM ...044-0059.dgn

