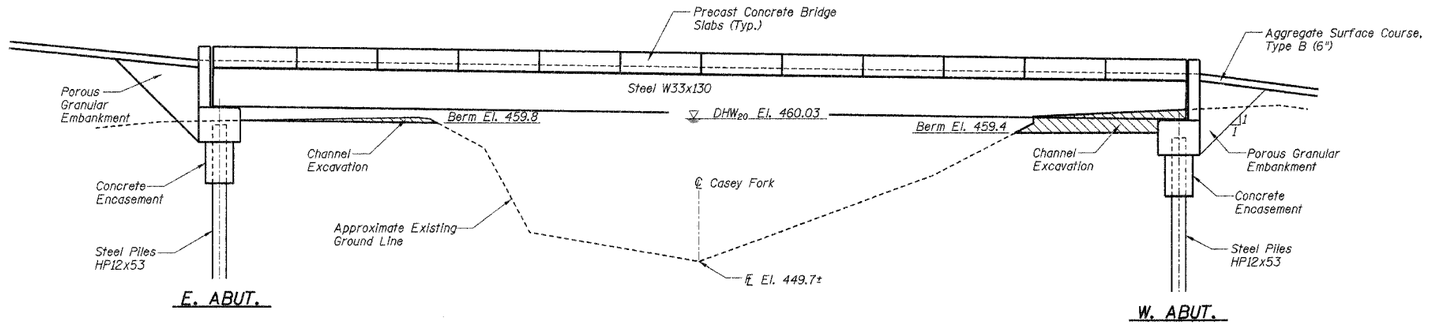
TBM 10-14-98"D" - Double 60d nails in South face of power pole, 34.88' Lt. of Sta. 38+62.96 - Elev. 463.44

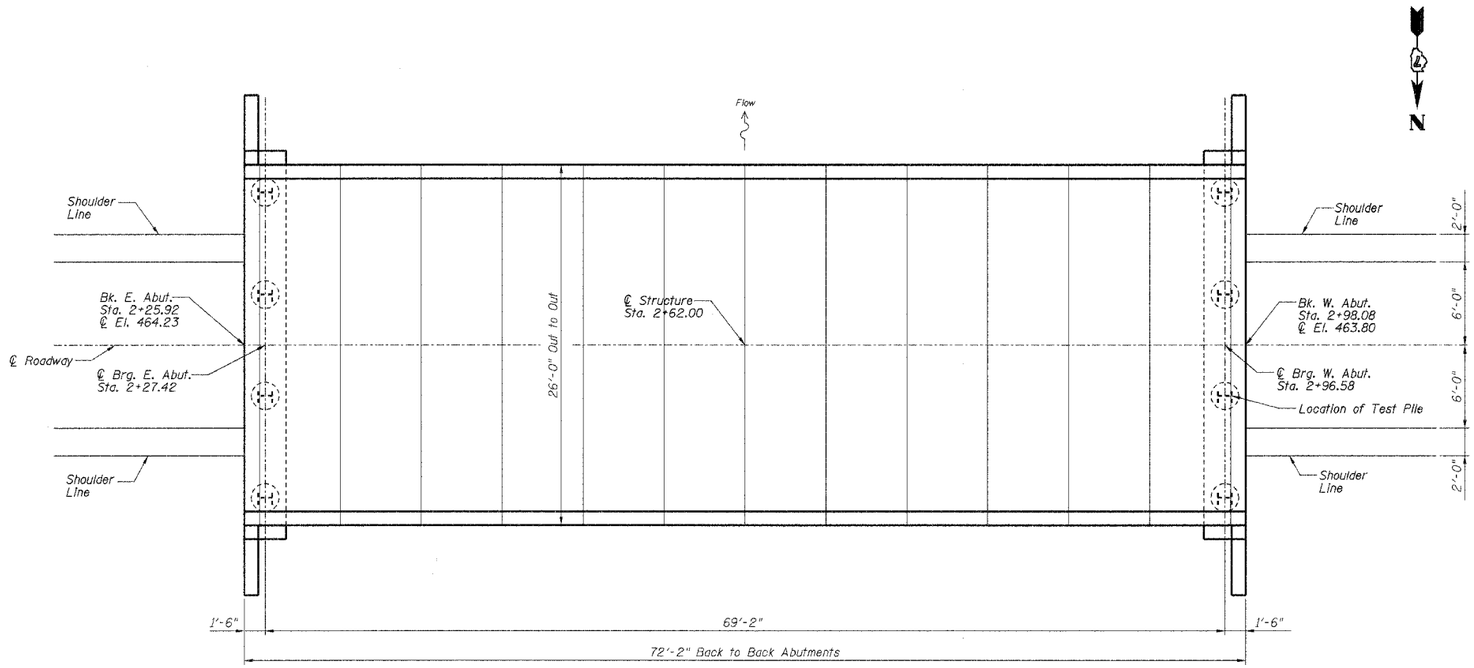
TBM 10-14-98"E" - Double 60d nails in South face of power pole, 21.06' Lt. of Sta. 43+10.83 - Elev. 460.6.44

Proposed Structure: The precast concrete bridge slabs and steel girders were salvaged from a recent Jefferson County Higway Department project.

ROUTE	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
TR 227	98-11120-00-BR		JEFFERSON	35	31
FED. ROAD DIST. NO. 7	ILLINOIS		FED. AID PROJECT		
			CONTRACT NO. 95437		



ELEVATION
(No Scale)



PLAN
(No Scale)

BILL OF MATERIAL (BRIDGE ONLY)

ITEM	UNIT	SUPER	SUB	TOTAL
Channel Excavation	Cu Yd	-	24	24
Porous Granular Embankment	Ton	-	60	60
Concrete Structures	Cu Yd	-	23.0	23.0
Setting Precast Concrete Bridge Slabs	L Sum	1	-	1
Furnishing and Erecting Structural Steel	L Sum	-	1	1
Reinforcement Bars	Pound	-	3220	3220
Furnishing Steel Piles HP 12x53	Foot	-	346	346
Driving Steel Piles	Foot	-	346	346
Test Pile Steel HP12x53	Each	-	1	1

GENERAL NOTES

See Section 502 of the Standard Specifications for Structure Excavation.

The Contractor shall drive one (1) Steel HP12x53 Test Pile in a permanent location at the West Abutment as directed by the Engineer before ordering the remainder of the piles.

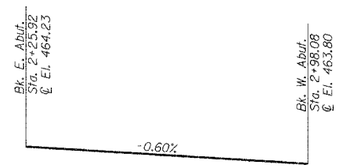
Reinforcement bars shall conform to the requirements of AASHTO M-31 or M-53 grade 60.

Channel excavation shall be excavated as shown within the limits of the proposed bridge, then tapered to the ends of the wingwalls as directed by the Engineer. If the Engineer deems the material satisfactory, it may be used to construct the roadway embankment.

See Specifications for Soil Borings.

Do not scale these drawings.

In addition to all other requirements of section 512 of the Standard Specifications, splices for Steel H-piles shall develop the full capacity of the steel's cross sectional area of the pile for tension, shear and bending forces. One approved method of achieving this requirement is full penetration butt welding of the entire cross section. Other types of splices meeting the full capacity requirement may be allowed subject to the approval of the Engineer. Any proposal by the Contractor to use an alternate splice method must include adequate documentation demonstrating that the full tension, shear and bending capacities will be met. Appropriate welder qualifications will be required for the positions and processes used in splicing all piles. Nondestructive testing of completed welds will be limited to visual inspection.



GRADE ACROSS STRUCTURE

WATERWAY DATA
See Sheet 24 of 35

DESIGN STRESSES
FIELD UNITS
f'_c = 3,500 psi
f_y = 60,000 psi

DESIGN SPECIFICATIONS
AASHTO - 2002 17th Edition

DESIGN LOADING HS 15-44
No allowance for future wearing surface.

SEISMIC DESIGN
Seismic Performance Category (SPC) = B
Bedrock Acceleration Coefficient (A) = 0.10g
Site Coefficient (S) = 1.5

I certify that to the best of my information, knowledge, and belief, this bridge is structurally adequate for the design loading shown on plans. The design is an economical one for the structure and complies with requirements of the current AASHTO Standard Specifications for Highway Bridges.

GARY L. HAHN 81-4853 LICENSED STRUCTURAL ENGINEER STATE OF ILLINOIS

Gary L. Hahn 5/24/05

GARY L. HAHN
CENTRALIA, ILLINOIS
ILLINOIS LICENSED STRUCTURAL
ENGINEER NO. 81-4853
EXPIRES NOV. 30, 2006

GENERAL PLAN AND ELEVATION
PROPOSED FIELD ROAD BRIDGE
OVER CASEY FORK
STA. 2+62.00
SECTION 98-11120-00-BR
JEFFERSON COUNTY, ILLINOIS

Sheet 31 of 35
Job No. 52303

05/24/2005

RHUTASEL and ASSOCIATES, INC.
CONSULTING ENGINEERS • LAND SURVEYORS
CENTRALIA, ILLINOIS FREEBURG, ILLINOIS