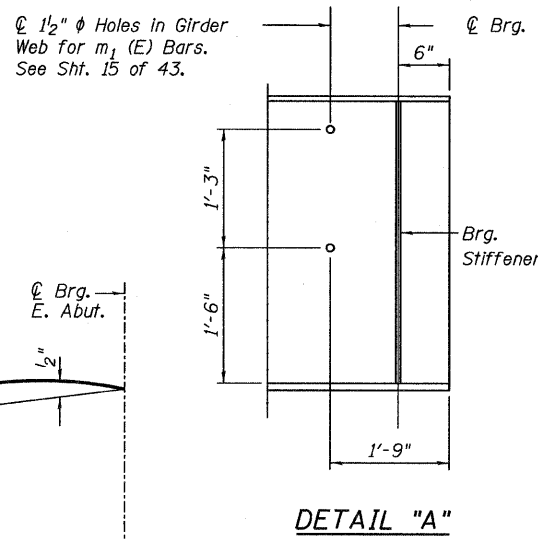
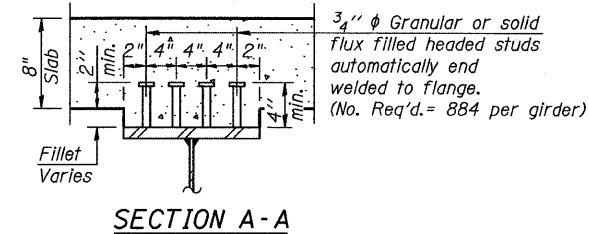


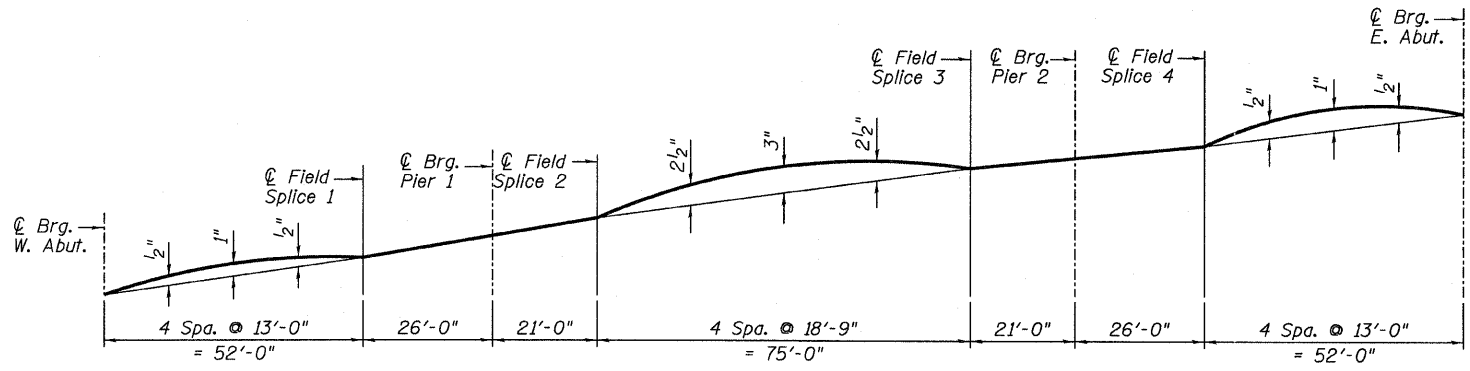
	0.4 Sp. 1 or 0.6 Sp. 3	Pier 1 or 2	0.5 Sp. 2
I_s	(in ⁴) 14,253	18,828	11,464
$I_c(n)$	(in ⁴) 34,167	-	27,370
$I_c(3n)$	(in ⁴) 25,278	-	20,628
S_s	(in ³) 799	966	607
$S_c(n)$	(in ³) 1,048	-	815
$S_c(3n)$	(in ³) 969	-	752
Z	(in ³) -	-	-
ρ	(k/ft) 1.023	1.564	0.996
$M\phi$	(k) 300	1,640	579
$s\phi$	(k/ft) .500	-	.500
$M_s\phi$	(k) 164	-	353
M_L	(k) 684	653	825
M_{IM}	(k) 168	147	170
$\phi_s [M_L + i]$	(k) 1,420	1,333	1,658
M_a	(k) 2,449	3,865	3,367
M_u	(k) 4,952	-	4,030
$f_s \phi$ non-comp	(ksi) 4.5	20.4	11.4
$f_s \phi$ (comp)	(ksi) 2.0	-	5.6
$f_s \phi_3 [M_L + M_I]$	(ksi) 16.3	16.6	24.4
f_s (Overload)	(ksi) 22.8	37.0	41.5
f_s (Total)	(ksi) -	48.0	-
VR	(k) 63.6	-	53.4



* Compact section
 ** Braced non-compact and partially braced section

	Abut.	Pier
$R\phi$	(k) 39.5	170.0
R_L	(k) 43.8	69.6
R_I	(k) 10.7	15.6
R_{Total}	(k) 94.0	255.2

I_s, S_s : Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total and Overload) due to non-composite dead loads (in⁴ and in³).
 $I_c(n), S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total and Overload) due to short-term composite live loads (in⁴ and in³).
 $I_c(3n), S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total and Overload) due to long-term composite (superimposed) dead loads (in⁴ and in³).
 Z : Plastic Section Modulus of the steel section in non-composite areas (in³).
 ρ : Un-factored non-composite dead load (kips/ft.).
 $M\phi$: Un-factored moment due to non-composite dead load (kip-ft.).
 $s\phi$: Un-factored long-term composite (superimposed) dead load (kips/ft.).
 $M_s\phi$: Un-factored moment due to long-term composite (superimposed) dead load (kip-ft.).
 M_L : Un-factored live load moment (kip-ft.).
 M_I : Un-factored moment due to impact (kip-ft.).
 M_a : Factored design moment (kip-ft.).
 $1.3 [M\phi + M_s\phi + \frac{1}{3} (M_L + M_I)]$
 M_u : Compact composite moment capacity according to AASHTO LFD 10.50.1.1 or compact non-composite moment capacity according to AASHTO LFD 10.48.1 (kip-ft.).
 f_s (Overload): Sum of stresses as computed from the moments below (ksi).
 $M\phi + M_s\phi + \frac{1}{3} (M_L + M_I)$
 f_s (Total): Sum of stresses as computed from the moments below on non-compact section (ksi).
 $1.3 [M\phi + M_s\phi + \frac{1}{3} (M_L + M_I)]$
 VR : Maximum $L +$ impact shear range within the composite portion of the span for stud shear connector design (kips).



Note:
 Load carrying components designated "N.T.R." shall conform to the Supplemental Requirements for Notch Toughness, Zone 2.