

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

**PROPOSED
HIGHWAY PLANS**

FAI 74 (I-74)
AT HENDERSON CREEK
SECTION 48-[(25B)BR, BR-1]
PROJECT ACNHPP-0074 (306)
BRIDGE REPLACEMENT
KNOX COUNTY
C-94-083-14

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
74	48-[(25B)BR, BR-1]	KNOX	131	1
			ILLINOIS CONTRACT NO. 68B85	

SHEET NUMBER	DESCRIPTION
1	TITLE SHEET
2	STATE STANDARDS, GENERAL NOTES AND COMMITMENTS
3 - 8	SUMMARY OF QUANTITIES
9	I-74 EASTBOUND EXISTING AND PROPOSED TYPICAL SECTION
10	I-74 WESTBOUND EXISTING AND PROPOSED TYPICAL SECTION
11	ALIGNMENT, TIES AND BENCHMARKS
12	I-74 EASTBOUND ROADWAY PLAN AND PROFILE
13	I-74 WESTBOUND ROADWAY PLAN AND PROFILE
14	SUGGESTED STAGES OF CONSTRUCTION AND TRAFFIC CONTROL NOTES
15	I-74 EASTBOUND EROSION & SEDIMENT CONTROL PLAN
16 - 17	I-74 WESTBOUND EROSION & SEDIMENT CONTROL PLAN
18	I-74 EASTBOUND DRAINAGE PLAN AND PROFILE
19	I-74 WESTBOUND DRAINAGE PLAN AND PROFILE
20	I-74 EASTBOUND PAVEMENT MARKING PLAN
21	I-74 WESTBOUND PAVEMENT MARKING AND SIGNING PLAN
22 - 71	STRUCTURE PLANS
72 - 77	IDOT DISTRICT 4 DETAILS
78 - 94	I-74 EASTBOUND CROSS SECTIONS
95 - 111	I-74 WESTBOUND CROSS SECTIONS
112 - 131	EXISTING PLANS

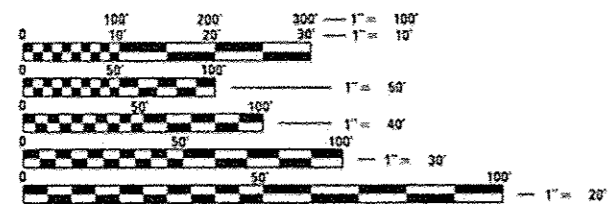
FOR IDOT HIGHWAY STANDARDS, SEE SHEET NO. 2

TRAFFIC DATA

FAI 74
EXISTING TRAFFIC ADT: 13100 (2013)
EXISTING TRUCK ADT: 4700 (2013)
SPEED LIMIT: 70 MPH
SINGLE UNIT: 650
MULTI UNIT: 4050
DESIGN DESIGNATION: INTERSTATE

PROJECT LOCATED IN RIO TOWNSHIP
IN KNOX COUNTY

PROJECT DESCRIPTION:
PROJECT INCLUDES REMOVAL AND REPLACEMENT OF SUPERSTRUCTURE AND
SUBSTRUCTURE OF EXISTING BRIDGES. INTERSTATE ROUTE 74 OVER HENDERSON
CREEK IN RIO TOWNSHIP, KNOX COUNTY.

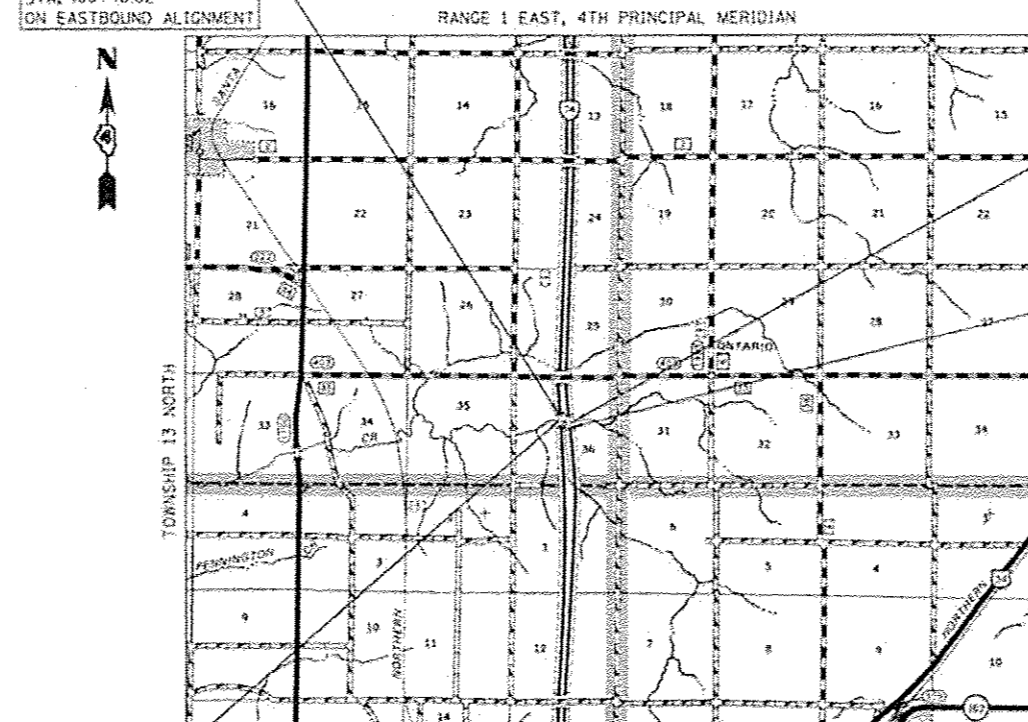


FULL SIZE PLANS HAVE BEEN PREPARED USING STANDARD
ENGINEERING SCALES. REDUCED SIZED PLANS WILL NOT
CONFORM TO STANDARD SCALES. IN MAKING MEASUREMENTS
ON REDUCED PLANS, THE ABOVE SCALES MAY BE USED.

FOR UTILITY INFORMATION CONTACT
J.U.L.I.E.
JOINT UTILITY LOCATION INFORMATION FOR EXCAVATION
1-800-892-0123

PROJECT MANAGER: MICHAEL LEWIS (309)671-3454
PROJECT ENGINEER: MICHAEL MCLUCKIE (309)671-3468
CATALOG NO. 032264-03D
CONTRACT NO. 68B85

PROJECT BEGINS
STA. 409+45.92
ON EASTBOUND ALIGNMENT



STA. 416+04.00
STRUCTURE NO. 048-0003
REMOVE EXISTING 121.5'
THREE SPAN STRUCTURE
PROPOSED S.N. 048-0090
SINGLE SPAN STRUCTURE

STA. 417+20.00
STRUCTURE NO. 048-0004
REMOVE EXISTING 115.0'
THREE SPAN STRUCTURE
PROPOSED S.N. 048-0091
SINGLE SPAN STRUCTURE

PROJECT ENDS
STA. 421+13.76
ON WESTBOUND ALIGNMENT

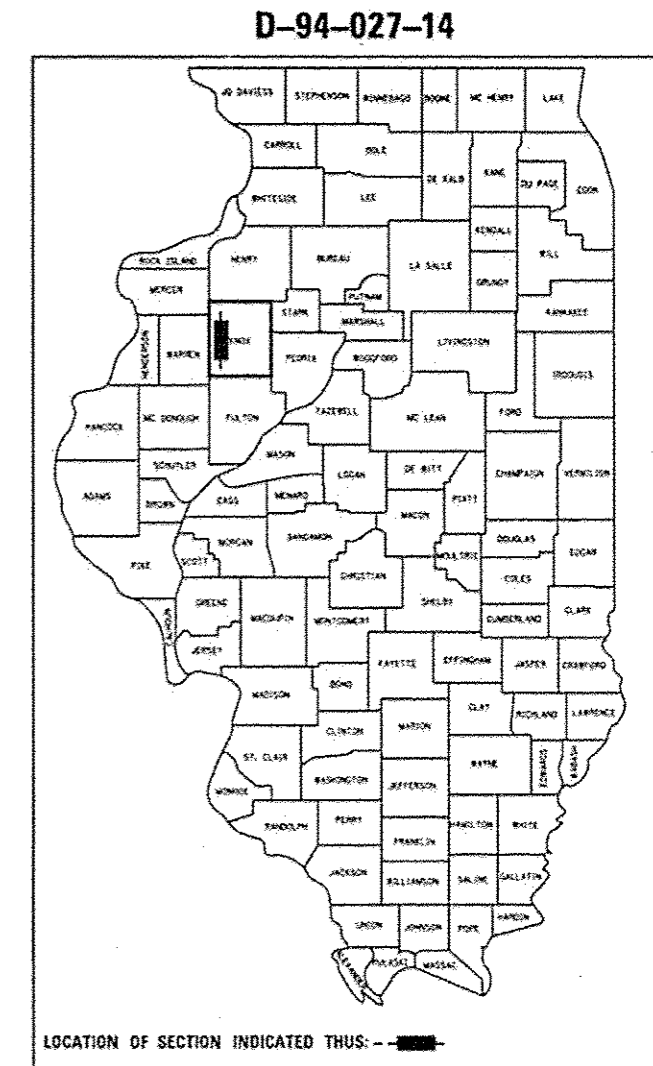


Prady K. Chatterjee 7/29/2014
Expires: 11/30/2014



Prady K. Chatterjee 7/29/2014
Expires: 11/30/2014

GROSS LENGTH = 1167.9 FT. = 0.221 MILE
NET LENGTH = 1167.9 FT. = 0.221 MILE



STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

SUBMITTED Aug 14 20 14
Prady K. Chatterjee
DEPUTY DIRECTOR OF HIGHWAYS, REGION ENGINEER

Oct 17 20 14
John D. Baranzelli, PE, Jr.
acting ENGINEER OF DESIGN AND ENVIRONMENT

Oct 17 20 14
Omer Osman, PE, Jr.
DIRECTOR OF HIGHWAYS, CHIEF ENGINEER

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OF THE STATE OF ILLINOIS



LIST OF STATE STANDARDS

STANDARD NUMBER	DESCRIPTION
000001-06	STANDARD SYMBOLS, ABBREVIATIONS AND PATTERNS
001001-02	AREAS OF REINFORCEMENT BARS
001006	DECIMAL OF AN INCH AND OF A FOOT
280001-07	TEMPORARY EROSION CONTROL SYSTEMS
420001-07	PAVEMENT JOINTS
420401-10	BRIDGE APPROACH PAVEMENT CONNECTOR
421001-02	BAR REINFORCEMENT FOR CRC PAVEMENT
421101-09	24' (7.2 M) CRC PAVEMENT
515001-03	NAME PLATE FOR BRIDGES
542401-01	METAL END SECTION FOR PIPE CULVERTS
601101-01	CONCRETE HEADWALL FOR PIPE DRAIN
606001-05	CONCRETE CURB TYPE B AND COMBINATION CONCRETE CURB AND GUTTER
609006-05	BRIDGE APPROACH PAVEMENT (DRAIN DETAIL)
610001-06	SHOULDER INLET WITH CURB
630001-10	STEEL PLATE BEAM GUARDRAIL
630301-06	SHOULDER WIDENING FOR TYPE 1 (SPECIAL) GUARDRAIL TERMINALS
631011-09	TRAFFIC BARRIER TERMINAL, TYPE 2
631031-12	TRAFFIC BARRIER TERMINAL, TYPE 6
635001-01	DELINEATORS
635006-03	REFLECTOR AND TERMINAL MARKER PLACEMENT
635011-02	REFLECTOR MARKER AND MOUNTING DETAILS
665001-02	WOVEN WIRE FENCE
701101-04	OFF-RD OPERATIONS, MULTILANE, 15' (4.5 m) TO 24" (6 mm) FROM PAVEMENT EDGE
701106-02	OFF-RD OPERATIONS, MULTILANE, MORE THAN 15' (4.5 m) AWAY
701400-07	APPROACH TO LANE CLOSURE, FREEWAY/EXPRESSWAY
701402-09	LANE CLOSURE, FREEWAY/EXPRESSWAY, WITH BARRIER
701406-08	LANE CLOSURE, FREEWAY/EXPRESSWAY, DAY OPERATIONS ONLY
701426-06	LANE CLOSURE, MULTILANE, INTERMITTENT OR MOVING OPER., FOR SPEEDS >= 45 MPH
701428	TRAFFIC CONTROL SETUP AND REMOVAL FREEWAY/EXPRESSWAY
701901-03	TRAFFIC CONTROL DEVICES
704001-07	TEMPORARY CONCRETE BARRIER
720001-01	SIGN PANEL MOUNTING DETAILS
720006-04	SIGN PANEL ERECTION DETAILS
720011-01	METAL POSTS FOR SIGNS, MARKERS & DELINEATORS
780001-04	TYPICAL PAVEMENT MARKINGS

LIST OF DISTRICT FOUR CADD STANDARDS *

STANDARD NUMBER	DESCRIPTION
205001-04	SLOPE STEPS DETAIL
601101-04	SLOPE DRAIN DETAILS FOR BURIED PIPES
601301-04	PIPE ELBOW
601401-04	DETAILS OF SEEPAGE COLLARS FOR BURIED PIPES
630101-04	GUARDRAIL EROSION CONTROL TREATMENTS

* INCLUDED AS SHEETS 72-77

GENERAL NOTES


1. MICROSTATION AND GEOPAK FILES OF THIS PROJECT WILL BE MADE AVAILABLE TO THE CONTRACTOR. IF THERE IS A CONFLICT BETWEEN THE ELECTRONIC FILES AND THE PRINTED AFTER CONTRACT AWARD PLANS AND DOCUMENTS, THE PRINTED CONTRACT PLANS AND DOCUMENTS SHALL TAKE PRECEDENCE OVER THE ELECTRONIC FILES. THE CONTRACTOR SHALL ACCEPT ALL RISK ASSOCIATED WITH USING THE ELECTRONIC FILES AND SHALL HOLD THE DEPARTMENT HARMLESS FOR ANY ERRORS OR OMISSIONS IN THE ELECTRONIC FILES AND THE DATA CONTAINED THEREIN. ERRORS OR DELAYS RESULTING FROM THE USE OF THE ELECTRONIC FILES BY THE CONTRACTOR SHALL NOT RESULT IN AN EXTENSION OF TIME FOR ANY INTERIM OR FINAL COMPLETION DATE OR SHALL NOT BE CONSIDERED CAUSE FOR ADDITIONAL COMPENSATION. THE CONTRACTOR SHALL NOT USE, SHARE, OR DISTRIBUTE THESE ELECTRONIC FILES EXCEPT FOR THE PURPOSE OF CONSTRUCTING THIS CONTRACT. ANY CLAIMS BY THIRD PARTIES DUE TO USE OR ERRORS SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL INCLUDE THIS DISCLAIMER WITH THE TRANSFER OF THESE ELECTRONIC FILES TO ANY OTHER PARTIES AND SHALL INCLUDE APPROPRIATE LANGUAGE BINDING THEM TO SIMILAR RESPONSIBILITIES.
2. NOT USED
3. ALL ELEVATIONS SHOWN ON THE PLANS ARE ESTABLISHED FROM U.S.G.S. MEAN SEA LEVEL DATUM.
4. THE CONTRACTOR WILL SUBMIT TO THE ENGINEER A SATISFACTORY PROGRESS SCHEDULE AND CRITICAL PATH SCHEDULE WHICH SHALL SHOW THE PROPOSED SEQUENCE OF WORK AT THE TIME OF THE PRE-CONSTRUCTION CONFERENCE.
5. AT LOCATIONS WHERE CLEARING IS INDICATED ON THE PLANS BEYOND THE LIMITS OF THE PROPOSED EXCAVATION OR EMBANKMENT, THE CONTRACTOR SHALL RESTORE THE DISTURBED EARTH BY BLADING AND SHAPING TO BLEND WITH THE ADJACENT GROUND. THE CLEARING WILL NOT BE PAID FOR SEPARATELY BUT SHALL BE INCLUDED IN THE COST OF THE EXCAVATION PAY ITEMS IN THE PLANS. PAYMENT FOR RESEEDING OR RESODDING WILL BE AS PROVIDED IN THE PLANS.
6. EARTH EXCAVATION AND BACKFILL FOR THE PROPOSED CURB AND GUTTERS SHALL BE INCLUDED IN THE UNIT COST OF THE VARIOUS ITEMS.
7. THE CONTRACTOR SHALL CONSULT WITH THE ENGINEER IN REGARD TO THE EXACT LENGTH OF THE PIPE DRAINS REQUIRED PRIOR TO ORDERING THESE ITEMS.
8. IN ACCORDANCE WITH SECTION 602 OF THE STANDARD SPECIFICATIONS, THE CONNECTING OF EXISTING DRAIN TILES, PIPE CULVERTS, OR STORM SEWERS TO THE PROPOSED DRAINAGE SYSTEM STRUCTURES WILL NOT BE PAID FOR SEPARATELY BUT SHALL BE CONSIDERED AS INCLUDED IN THE PAY ITEMS PROVIDED.
9. TEN FEET (10 FT.) (3 M) TRANSITIONS SHALL BE USED TO MATCH PROPOSED ITEMS OF WORK TO EXISTING ITEMS IN THE FIELD UNLESS OTHERWISE SHOWN. THE TRANSITION SHALL BE PAID FOR AT THE CONTRACT UNIT PRICE FOR THE PROPOSED ITEM OF WORK SPECIFIED.
10. THE WOVEN WIRE FENCE SHALL BE INSTALLED PRIOR TO THE REMOVAL OF THE EXISTING FARM FENCES. THE CONTRACTOR SHALL PROVIDE A PULL POST AT THE INTERSECTION OF NEW AND EXISTING FENCES. WHEN SO DIRECTED BY THE ENGINEER, THE CONTRACTOR SHALL INSTALL THE WOVEN WIRE FENCE PRIOR TO COMMENCING ANY OTHER WORK IN THE AREA. THE CONTRACTOR SHALL PERFORM ANY CLEARING AND MINOR GRADING AS DIRECTED BY THE ENGINEER TO PROVIDE A SMOOTH GROUND SURFACE FOR THE PROPOSED FENCE.
11. ADD THE FOLLOWING SENTENCE TO THE END OF PARAGRAPH 670.02 (I) AND 670.04 (G): ALL OF THE TELEPHONE LINES PROVIDED SHALL HAVE UNPUBLISHED NUMBERS.
12. SIGN LOCATIONS MAY VARY FROM THE STATIONS SHOWN ON THE PLANS IN ACCORDANCE WITH DIRECTIONS FROM THE ENGINEER AT THE TIME OF CONSTRUCTION. SIGN LOCATIONS MAY BE ADJUSTED IN THE FIELD TO AVOID ANY FOUND UTILITIES. ALL WOOD POST LOCATIONS SHALL BE VERIFIED WITH THE BUREAU OF OPERATIONS, TRAFFIC SECTION, BEFORE INSTALLATION.
13. NOT USED
14. NOT USED

15. ALL WORK NECESSARY FOR THE PLACEMENT OF RIP RAP INCLUDING EXCAVATION AND DEWATERING SHALL BE INCLUDED IN THE COST OF THE RIP RAP. REMOVAL AND DISPOSAL OF REGULATED SUBSTANCES WILL BE PAID SEPARATELY.

16. SLOPE STABILIZATION REQUIRING DROP OFFS GREATER THAN 2 FEET SHALL BE COMPLETED IN LESS THAN 24 HOURS.

COMMITMENTS

NONE

FILE NAME = ...D4-174-HendersonCreek-Combined-sh1282-v	DESIGNED - JCP	REVISED - -	 <p>SEPSTEIN 601 WEST FULTON STREET CHICAGO, ILLINOIS 60607 TEL: 312.454.9100 FAX: 312.552.3217 WEB: WWW.SEPSTEIN.COM</p>	<p>STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION</p>	<p>STATE STANDARDS, GENERAL NOTES AND COMMITMENTS</p>	F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
PLOT TIME = 4:32:42 PM	CHECKED - CAO	REVISED - -				74	48-(25B)BR, BR-1J	KNOX	131	2
PLOT DATE = 7/30/2014	DATE - 08/01/14	REVISED - -				CONTRACT NO. 68B85			ILLINOIS FED. AID PROJECT	
SCALE: N.T.S.						SHEET NO. 1 OF 1 SHEETS		STA. TO STA.		

CODE NO.	ITEM	UNIT	TOTAL QUANTITY	CONSTRUCTION CODE				
				90% FED 10% STATE ROADWAY	90% FED 10% STATE BRIDGE	90% FED 10% STATE BRIDGE	90% FED 10% STATE TRAINEES	100% STATE ROADWAY
				0004	0011	0011	0042	0004
				RURAL	048-0090	048-0091	RURAL	RURAL
20200100	EARTH EXCAVATION	CU YD	1530	1530				
20201200	REMOVAL AND DISPOSAL OF UNSUITABLE MATERIAL	CU YD	60	60				
20400800	FURNISHED EXCAVATION	CU YD	750	750				
20800150	TRENCH BACKFILL	CU YD	89	89				
21101505	TOPSOIL EXCAVATION AND PLACEMENT	CU YD	394	394				
21101615	TOPSOIL FURNISH AND PLACE, 4"	SQ YD	1825	1825				
25000210	SEEDING, CLASS 2A	ACRE	0.75	0.75				
25000400	NITROGEN FERTILIZER NUTRIENT	POUND	64	64				
25000500	PHOSPHORUS FERTILIZER NUTRIENT	POUND	64	64				
25000600	POTASSIUM FERTILIZER NUTRIENT	POUND	64	64				
25000750	MOWING	ACRE	3.5					3.5
25100635	HEAVY DUTY EROSION CONTROL BLANKET	SQ YD	3441	3441				
28000250	TEMPORARY EROSION CONTROL SEEDING	POUND	285	285				
28000305	TEMPORARY DITCH CHECKS	FOOT	128	128				

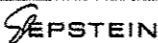
14
 * DENOTES SPECIALTY ITEM

CODE NO.	ITEM	UNIT	TOTAL QUANTITY	CONSTRUCTION CODE					
				90% FED 10% STATE ROADWAY	90% FED 10% STATE BRIDGE	90% FED 10% STATE BRIDGE	90% FED 10% STATE TRAINEEES	100% STATE ROADWAY	
				0004	0011	0011	0042	0004	
				RURAL	048-0090	048-0091	RURAL	RURAL	
28000315	AGGREGATE DITCH CHECKS	TON	23	23					
28000400	PERIMETER EROSION BARRIER	FOOT	2147	2147					
28000500	INLET AND PIPE PROTECTION	EACH	17	17					
28100105	STONE RIPRAP, CLASS A3	SO YD	50	50					
28100107	STONE RIPRAP, CLASS A4	SO YD	3333		1673	1660			
28200200	FILTER FABRIC	SO YD	3463	130	1673	1660			
31101200	SUBBASE GRANULAR MATERIAL, TYPE B 4"	SO YD	657	657					
42001300	PROTECTIVE COAT	SO YD	647	647					
42001420	BRIDGE APPROACH PAVEMENT CONNECTOR (PCC)	SO YD	647	647					
44000100	PAVEMENT REMOVAL	SO YD	623	623					
44000500	COMBINATION CURB AND GUTTER REMOVAL	FOOT	117	117					
50100100	REMOVAL OF EXISTING STRUCTURES	EACH	2		1	1			
50200100	STRUCTURE EXCAVATION	CU YD	1582		804	778			
50300225	CONCRETE STRUCTURES	CU YD	170.2		84.2	86.0			

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 • DENOTES SPECIALTY ITEM


CODE NO.	ITEM	UNIT	TOTAL QUANTITY	CONSTRUCTION CODE				
				90% FED 10% STATE ROADWAY	90% FED 10% STATE BRIDGE	90% FED 10% STATE BRIDGE	90% FED 10% STATE TRAINEES	100% STATE ROADWAY
				0004	0011	0011	0042	0004
				RURAL	048-0090	048-0091	RURAL	RURAL
50300255	CONCRETE SUPERSTRUCTURE	CU YD	694.7		352.2	342.5		
50300260	BRIDGE DECK GROOVING	SQ YD	1457		736	721		
50300300	PROTECTIVE COAT	SQ YD	1751		897	854		
50400745	FURNISHING AND ERECTING PRECAST PRESTRESSED CONCRETE BULB T-BEAMS 72"	FOOT	1234.5		629	605.5		
50800205	REINFORCEMENT BARS, EPOXY COATED	POUND	171880		87610	84270		
50800515	BAR SPLICERS	EACH	1149		579	570		
51201710	FURNISHING STEEL PILES HP12X84	FOOT	1515		765	750		
51202305	DRIVING PILES	FOOT	1515		765	750		
51203710	TEST PILE STEEL HP12X84	EACH	4		2	2		
51500100	NAME PLATES	EACH	2		1	1		
54210182	PIPE ELBOW, 12"	EACH	16	16				
54213447	END SECTIONS 12"	EACH	8	8				
59100100	GEOCOMPOSITE WALL DRAIN	SQ YD	268		132	136		
60100945	PIPE DRAINS 12"	FOOT	386	386				

* DENOTES SPECIALTY ITEM

FILE NAME : \\D4-174-MandanCreek-Combined-sh1005-5000.dwg	DESIGNED - JCP	REVISED -	 <small>46 WEST FLATION STREET CHICAGO, ILLINOIS 60611-2701 TEL: 312.434.8100 FAX: 312.533.2127 WEB: www.sepstein.com</small>	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	SUMMARY OF QUANTITIES				F.A.T. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
PLOT TIME : 1:20:51 PM	CHECKED - GAO	REVISED -							74	48-[[258]BR, BR-1]	KNOX	131	5
PLOT DATE : 8/12/2014	DATE - 08/01/14	REVISED -			SCALE: N/A		SHEET NO. 3 OF 6 SHEETS	STA.	TO STA.	ILLINOIS FED. AID PROJECT		CONTRACT NO. 68B85	

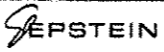
CODE NO.	ITEM	UNIT	TOTAL QUANTITY	CONSTRUCTION CODE						
				90% FED 10% STATE ROADWAY	90% FED 10% STATE BRIDGE	90% FED 10% STATE BRIDGE	90% FED 10% STATE TRAININGS	100% STATE ROADWAY		
				0004	0011	0011	0042	0004		
				RURAL	048-0090	048-0091	RURAL	RURAL		
60603800	COMBINATION CONCRETE CURB AND CUTTER, TYPE B-6.12	FOOT	190	190						
60900515	CONCRETE THRUST BLOCKS	EACH	8	8						
61000225	TYPE F INLET BOX, STANDARD 610001	EACH	4	4						
61000335	TYPE C INLET BOX, STANDARD 610001	EACH	4	4						
• 63000003	STEEL PLATE BEAM GUARDRAIL, TYPE A, 9 FOOT POSTS	FOOT	1437.5	1437.5						
• 63100045	TRAFFIC BARRIER TERMINAL, TYPE 2	EACH	4	4						
• 63100085	TRAFFIC BARRIER TERMINAL, TYPE 6	EACH	8	8						
• 63100167	TRAFFIC BARRIER TERMINAL, TYPE 1 (SPECIAL) TANGENT	EACH	4	4						
63200310	GUARDRAIL REMOVAL	FOOT	1992	1992						
66500105	WOVEN WIRE FENCE, 4'	FOOT	227	227						
66700205	PERMANENT SURVEY MARKERS, TYPE 1	EACH	2	2						
67000400	ENGINEER'S FIELD OFFICE, TYPE A	CAL MO	7	7						
67100100	MOBILIZATION	L SUM	1	1						
70100207	TRAFFIC CONTROL AND PROTECTION, STANDARD 701402	EACH	2	2						

• DENOTES SPECIALTY ITEM

FILE NAME : ...D4-174-HendersonCreek-Combined-sh1006-50	DESIGNED - JCP	REVISED - -	 600 WEST FULTON STREET CHICAGO, ILLINOIS 60612-1077 TEL. 312.454.9400 FAX 312.553.1217 WEB www.sepstein.com	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION		SUMMARY OF QUANTITIES				F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
PLOT TIME : 1:21:21 PM	CHECKED - GAO	REVISED - -								74	48-[125]BR, BR-13	KNOX	131	6
PLOT DATE : 8/12/2014	DATE - 08/01/14	REVISED - -								SCALE: N/A	SHEET NO. 4 OF 6 SHEETS	STA.	TO STA.	CONTRACT NO. 68885

CODE NO.	ITEM	UNIT	TOTAL QUANTITY	CONSTRUCTION CODE					
				90% FED 10% STATE ROADWAY	90% FED 10% STATE BRIDGE	90% FED 10% STATE BRIDGE	90% FED 10% STATE TRAINEES	100% STATE ROADWAY	
				0004	0011	0011	0042	0004	
				RURAL	04B-0090	04B-0091	RURAL	RURAL	
70100700	TRAFFIC CONTROL AND PROTECTION, STANDARD 701406	L SUM	1	1					
70103815	TRAFFIC CONTROL SURVEILLANCE	CAL DA	28	28					
70400100	TEMPORARY CONCRETE BARRIER	FOOT	800	800					
70400200	RELOCATE TEMPORARY CONCRETE BARRIER	FOOT	800	800					
70600305	IMPACT ATTENUATORS, TEMPORARY (FULLY REDIRECTIVE, RESETTABLE), TEST LEVEL 3	EACH	2	2					
70600330	IMPACT ATTENUATORS, RELOCATE (FULLY REDIRECTIVE), TEST LEVEL 3	EACH	2	2					
* 72000200	SIGN PANEL - TYPE 2	SQ FT	15	15					
• 78009004	MODIFIED URETHANE PAVEMENT MARKING - LINE 4"	FOOT	7681	7681					
• 78009006	MODIFIED URETHANE PAVEMENT MARKING - LINE 6"	FOOT	1221	1221					
• 78200420	GUARDRAIL MARKERS, TYPE B	EACH	26	26					
• 78201000	TERMINAL MARKER - DIRECT APPLIED	EACH	4	4					
78300100	PAVEMENT MARKING REMOVAL	SQ FT	2968	2968					
Z0001002	GUARDRAIL AGGREGATE EROSION CONTROL	TON	35	35					
Z0004552	APPROACH SLAB REMOVAL	SQ YD	458	458					


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• DENOTES SPECIALTY ITEM

FILE NAME : \\104-174-HendersonCreek-Combined-sh1807-80	DESIGNED - JCP	REVISED -	 606 WEST PULASKI STREET CHICAGO, ILLINOIS 60641-1209 TEL: 312.354.9100 FAX: 312.359.1212 WWW.SEPSTEIN.COM	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	SUMMARY OF QUANTITIES				F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
PLOT TIME : 11:21:46 PM	CHECKED - GAO	REVISED -			SCALE: N/A	SHEET NO. 5 OF 6 SHEETS	STA.	TO STA.	74	4B-(125B)BR, BR-1	KNOX	131	7
PLOT DATE : 8/12/2014	DATE - 08/01/14	REVISED -							CONTRACT NO. 68885				
											ILLINOIS FED. AID PROJECT		

1/15/14



CODE NO.	ITEM	UNIT	TOTAL QUANTITY	CONSTRUCTION CODE					
				90% FED 10% STATE ROADWAY	90% FED 10% STATE BRIDGE	90% FED 10% STATE BRIDGE	90% FED 10% STATE TRAINEES	100% STATE ROADWAY	
				0004	0011	0011	0042	0004	
				RURAL	048-0090	048-0091	RURAL	RURAL	
Z0013798	CONSTRUCTION LAYOUT	L SUM	1	1					
Z0018002	DRAINAGE SCUPPERS, DS-11	EACH	14		8	6			
Z0022800	FENCE REMOVAL	FOOT	263	263					
Z0030850	TEMPORARY INFORMATION SIGNING	SQ FT	54	54					
Z0046304	PIPE UNDERDRAINS FOR STRUCTURES 4"	FOOT	398		202	196			
Z0064540	SEEPAGE COLLAR	EACH	8	8					
Z0073002	TEMPORARY SOIL RETENTION SYSTEM	SQ FT	396		188	208			
Z0076600	TRAINEES	HOOR	1000				1000		
Z0076604	TRAINEES TRAINING PROGRAM GRADUATE	HOOR	1000				1000		
X0324044	EROSION CONTROL, TEMPORARY PIPE SLOPE DRAIN	EACH	2	2					
X5509900	ABANDON AND FILL EXISTING STORM SEWER	FOOT	205	205					
X5860110	GRANULAR BACKFILL FOR STRUCTURES	CU YD	710.6		318.2	392.4			
X7010410	SPEED DISPLAY TRAILER	CAL MO	7	7					

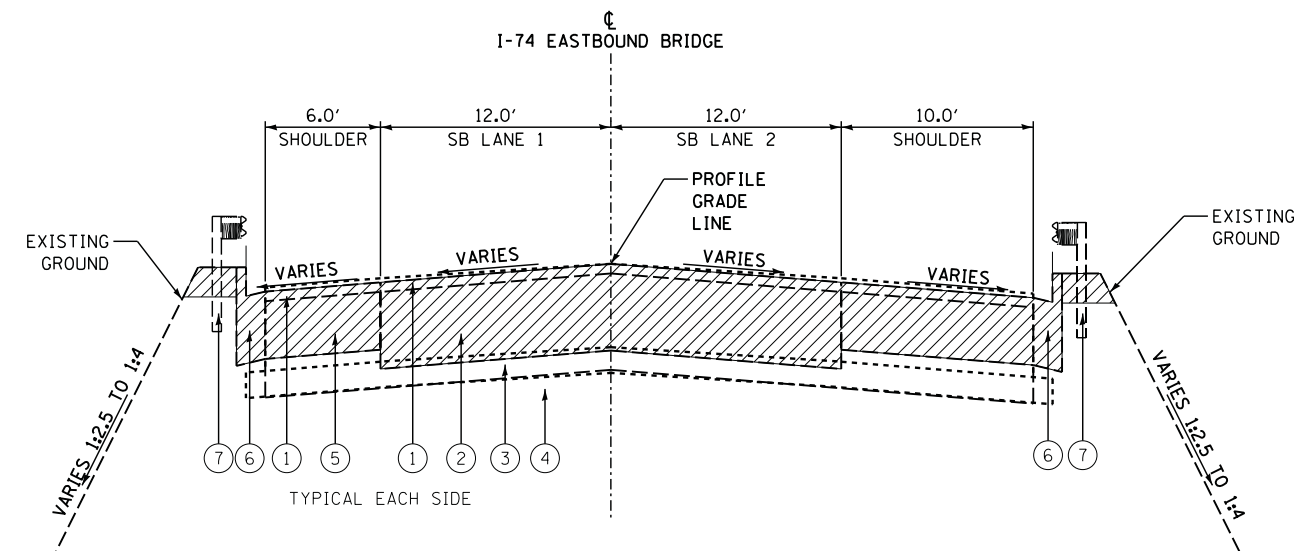
* DENOTES SPECIALTY ITEM

FILE NAME : ...104-174-HendersonCreek-Combined-sh1008-S	DESIGNED - JCP	REVISED -	 <p>100 WEST FULTON STREET CHICAGO, ILLINOIS 60612 TEL: 312.456.9100 FAX: 312.259.1217 WWW.SEPSTEIN.COM</p>	<p align="center">STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION</p>	<p align="center">SUMMARY OF QUANTITIES</p>		F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
PLOT TIME * 1:22:48 PM	CHECKED - GAO	REVISED -					74	48-(1250)BR, BR-1]	KNOX	131	8
PLOT DATE * 8/12/2014	DATE - 08/01/14	REVISED -					CONTRACT NO. 68885				
SCALE: N/A							SHEET NO. 6 OF 6 SHEETS		STA.	TO STA.	
ILLINOIS FED. AID PROJECT											

EXISTING LEGEND:

- ① 1/2" ASPHALT SURFACE
- ② PCC PAVEMENT
- ③ VARIABLE SUBBASE GRANULAR MATERIAL
- ④ SUBGRADE
- ⑤ BITUMINOUS AGGREGATE SHOULDER
- ⑥ TYPE B-6.12 CURB AND GUTTER
- ⑦ GUARDRAIL

-  ROADWAY REMOVAL ITEM (SEE ROADWAY PLANS)
-  STRUCTURE REMOVAL ITEM (SEE STRUCTURE PLANS)



EXISTING TYPICAL SECTION

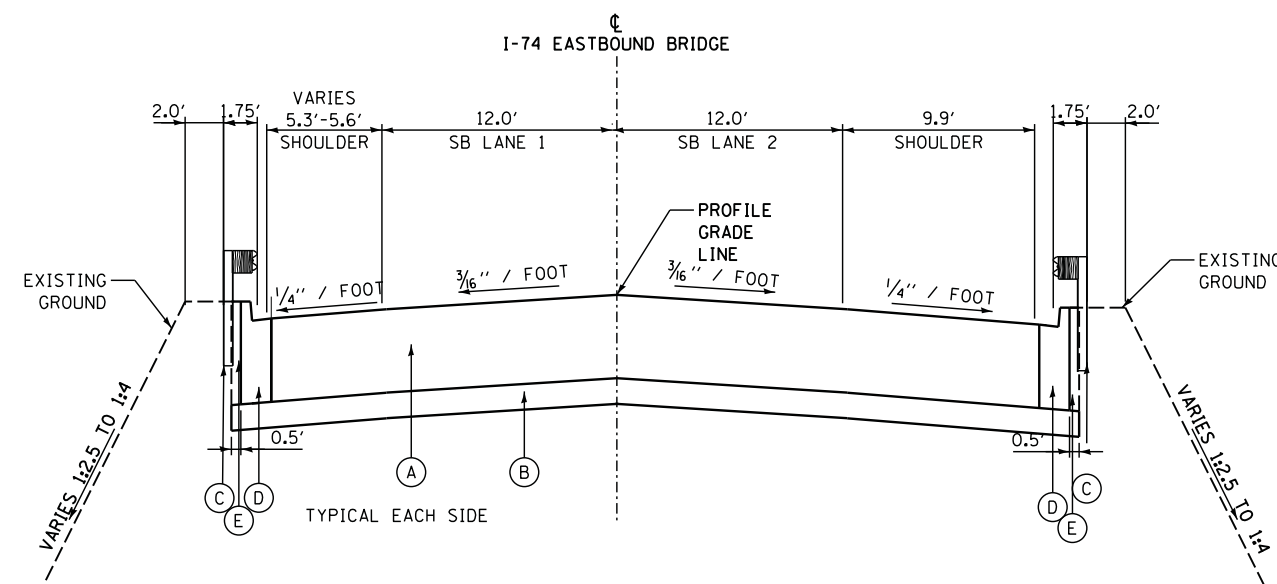
STA. 415+00.00 TO STA. 415+43.50
 STA. 416+65.00 TO STA. 417+08.96
 (BRIDGE OMISSION STA. 415+43.50 TO STA. 416+65.00)

NOTES:

1. TYPE B6.12 CURB AND GUTTER REMOVAL FROM STA. 415+26.40 LT TO STA. 415+42.82 LT FROM STA. 415+29.89 RT TO STA. 415+43.43 RT FROM STA. 416+62.16 LT TO STA. 416+77.04 LT FROM STA. 416+61.86 RT TO STA. 416+75.72 RT
2. SEE DISTRICT 4 SLOPE STEPS DETAIL FOR ADDITIONAL INFORMATION.


PROPOSED LEGEND:

- Ⓐ BRIDGE APPROACH PAVEMENT CONNECTOR (PCC)
- Ⓑ SUBBASE GRANULAR MATERIAL, TYPE B 4"
- Ⓒ STEEL PLATE BEAM GUARDRAIL, TYPE A, 9 FOOT POSTS
- Ⓓ COMBINATION CONCRETE CURB AND GUTTER, TYPE B-6.12
- Ⓔ FURNISHED EXCAVATION





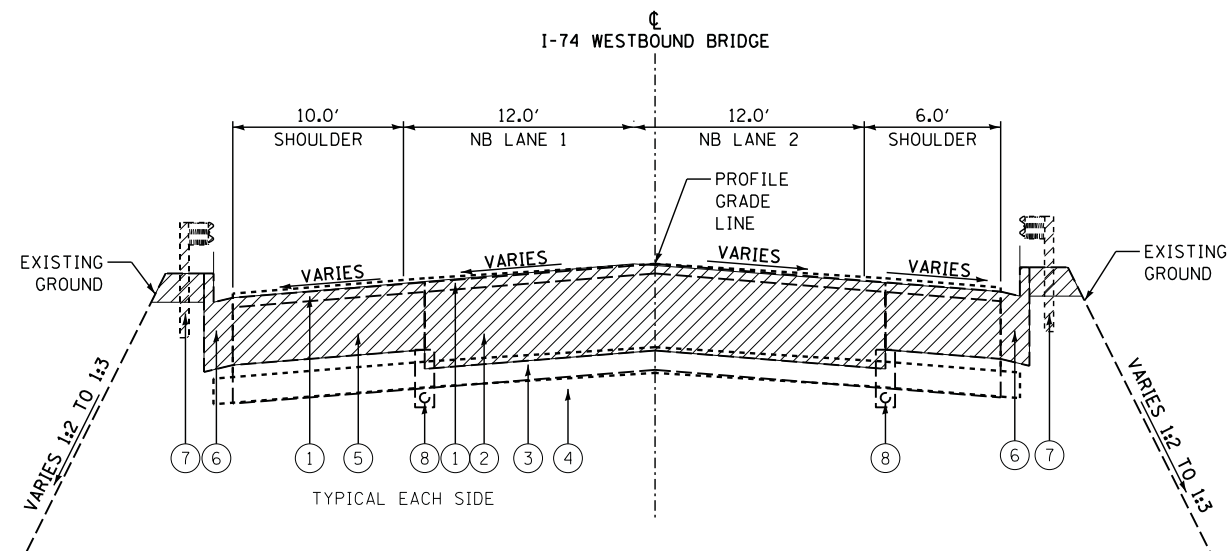
PROPOSED TYPICAL SECTION

STA. 415+00.00 TO STA. 415+21.25
 STA. 416+86.75 TO STA. 417+08.96
 (BRIDGE OMISSION STA. 415+50.25 TO STA. 416+57.75)

FILE NAME =	DESIGNED - JCP	REVISED -	 800 WEST FULTON STREET CHICAGO, ILLINOIS 60611-1259 TEL 312 454 9100 FAX 312 559 1217 WEB www.sepstein.com	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION		I-74 EASTBOUND EXISTING AND PROPOSED TYPICAL SECTION				F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
... \D4-174-HendersonCreek-Combined-sht009-Ex	DRAWN - JCP	REVISED -								74	48-(25B)BR, BR-1J	KNOX	131	9
PLOT TIME = 4:38:49 PM	CHECKED - GAO	REVISED -								CONTRACT NO. 68B85				
PLOT DATE = 7/30/2014	DATE - 08/01/14	REVISED -								ILLINOIS FED. AID PROJECT				
			SCALE: N/A			SHEET NO. 1 OF 1 SHEETS			STA. N/A TO STA. N/A					

EXISTING LEGEND:

- ① 1/2" ASPHALT SURFACE
 - ② PCC PAVEMENT
 - ③ VARIABLE SUBBASE GRANULAR MATERIAL
 - ④ SUBGRADE
 - ⑤ BITUMINOUS AGGREGATE SHOULDER
 - ⑥ TYPE B6.12 CURB AND GUTTER
 - ⑦ GUARDRAIL
 - ⑧ UNDERDRAIN
-  ROADWAY REMOVAL ITEM (SEE ROADWAY PLANS)
 -  STRUCTURE REMOVAL ITEM (SEE STRUCTURE PLANS)



EXISTING TYPICAL SECTION

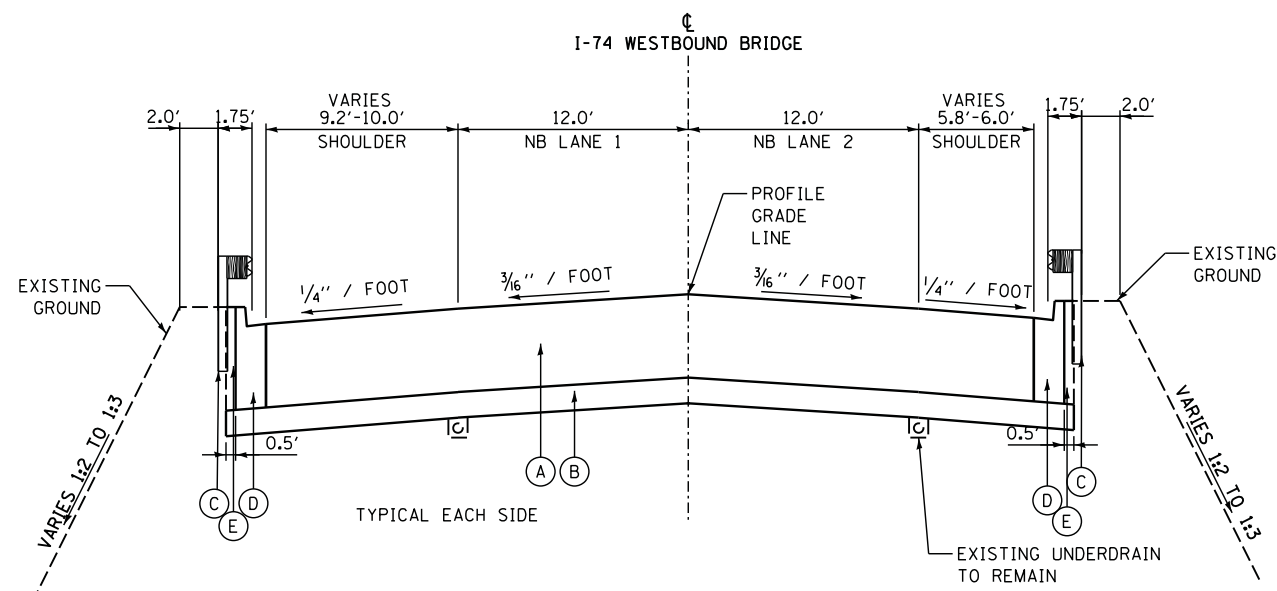
STA. 415+69.56 TO STA. 416+62.52
 STA. 417+77.54 TO STA. 418+24.56
 (BRIDGE OMISSION STA. 416+62.52 TO STA. 417+77.54)

NOTES:

1. TYPE B6.12 CURB AND GUTTER REMOVAL FROM STA. 415+26.40 LT TO STA. 415+42.82 LT FROM STA. 415+29.89 RT TO STA. 415+43.43 RT
2. SEE DISTRICT 4 SLOPE STEPS DETAIL FOR ADDITIONAL INFORMATION.


PROPOSED LEGEND:

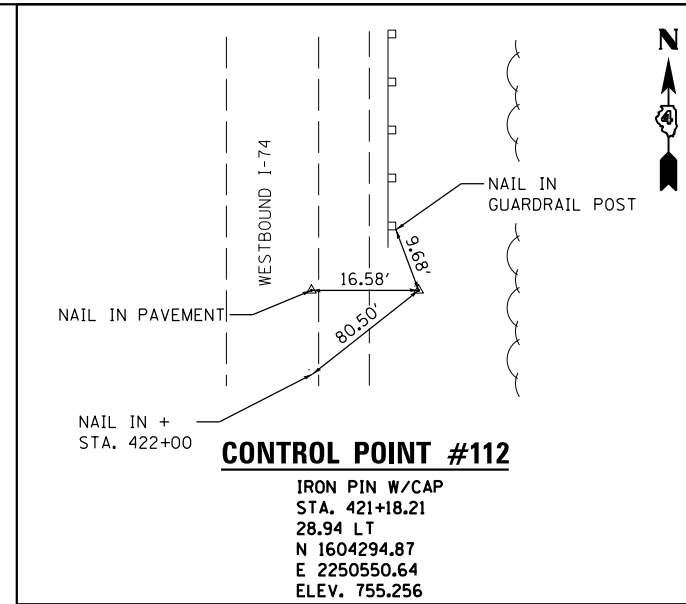
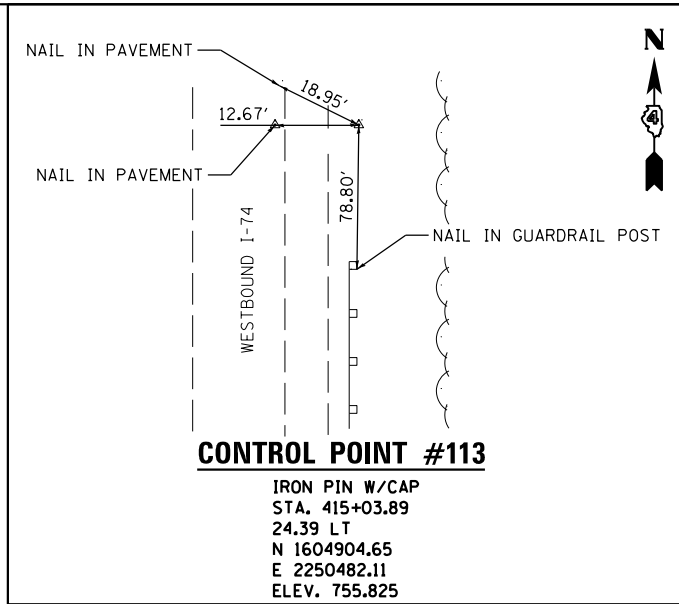
- Ⓐ BRIDGE APPROACH PAVEMENT CONNECTOR (PCC)
- Ⓑ SUBBASE GRANULAR MATERIAL, TYPE B 4"
- Ⓒ STEEL PLATE BEAM GUARDRAIL, TYPE A, 9 FOOT POSTS
- Ⓓ COMBINATION CONCRETE CURB AND GUTTER, TYPE B-6.12
- Ⓔ FURNISHED EXCAVATION



PROPOSED TYPICAL SECTION

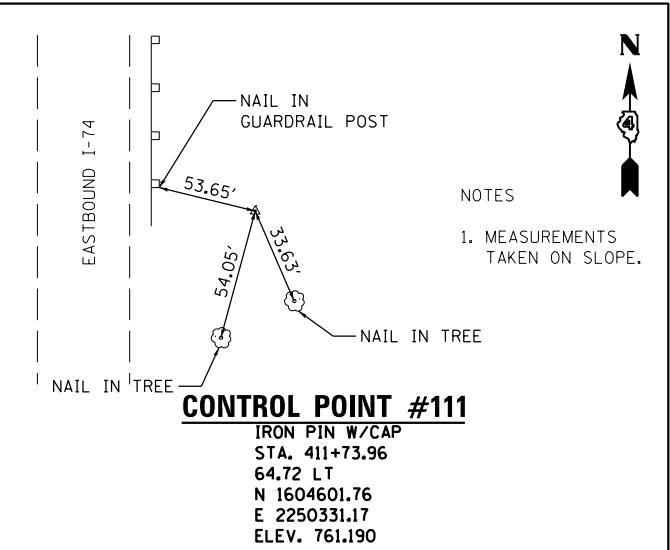
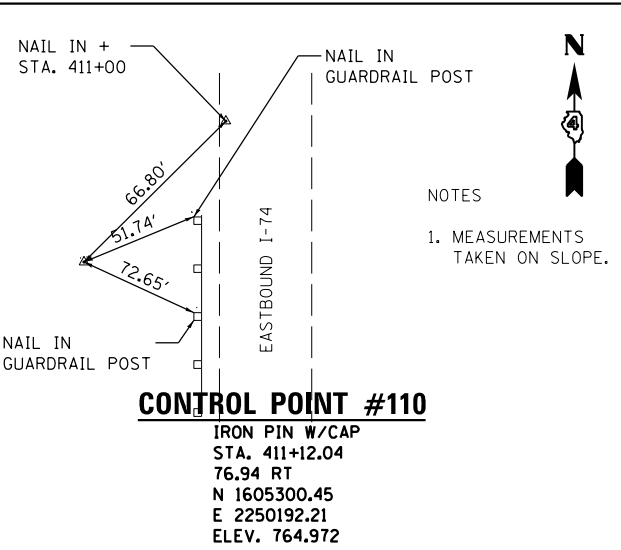
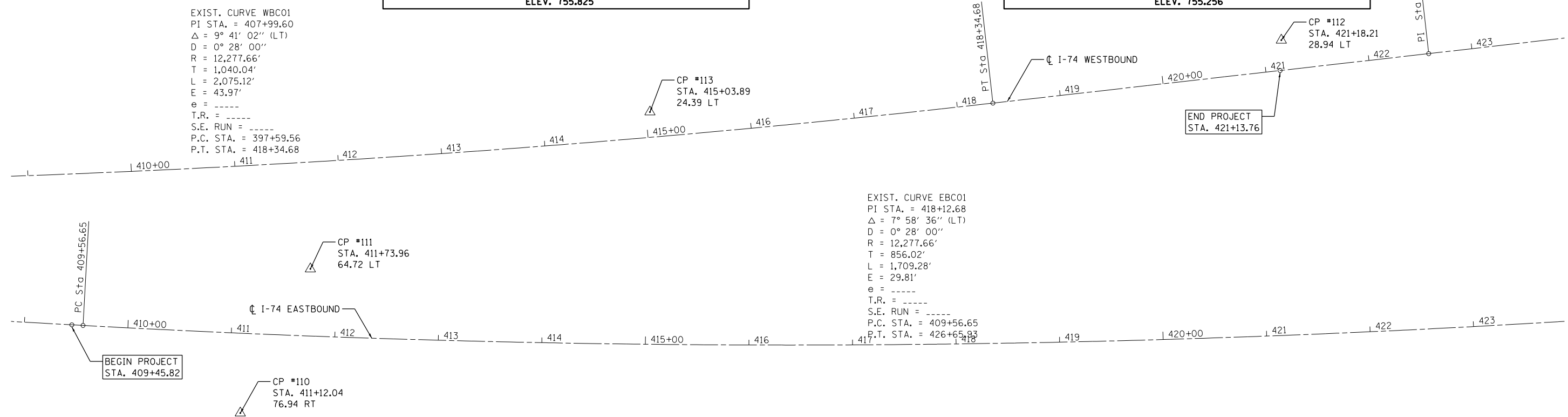
STA. 415+69.56 TO STA. 416+39.18
 STA. 418+00.81 TO STA. 418+24.56
 (BRIDGE OMISSION STA. 416+68.14 TO STA. 417+71.86)

FILE NAME =	DESIGNED - JCP	REVISED -	 800 WEST FULTON STREET CHICAGO, ILLINOIS 60611-1259 TEL 312 454 9100 FAX 312 559 1217 WEB www.sepstein.com	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	I-74 WESTBOUND EXISTING AND PROPOSED TYPICAL SECTION		F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
...\\D4-174-HendersonCreek-Combined-sht010-West	DRAWN - JCP	REVISED -					74	48-(25B)BR, BR-1J	KNOX	131	10
PLOT TIME = 4:38:49 PM	CHECKED - GAO	REVISED -			CONTRACT NO. 68B85						
PLOT DATE = 7/30/2014	DATE - 08/01/14	REVISED -			SCALE: N/A	SHEET NO. 1 OF 1 SHEETS	STA. N/A	TO STA. N/A	ILLINOIS FED. AID PROJECT		



EXIST. CURVE WBC01
 PI STA. = 407+99.60
 $\Delta = 9^\circ 41' 02''$ (LT)
 D = $0^\circ 28' 00''$
 R = 12,277.66'
 T = 1,040.04'
 L = 2,075.12'
 E = 43.97'
 e = -----
 T.R. = -----
 S.E. RUN = -----
 P.C. STA. = 397+59.56
 P.T. STA. = 418+34.68

EXIST. CURVE EBC01
 PI STA. = 418+12.68
 $\Delta = 7^\circ 58' 36''$ (LT)
 D = $0^\circ 28' 00''$
 R = 12,277.66'
 T = 856.02'
 L = 1,709.28'
 E = 29.81'
 e = -----
 T.R. = -----
 S.E. RUN = -----
 P.C. STA. = 409+56.65
 P.T. STA. = 426+65.93

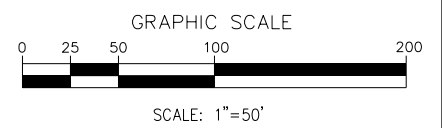


NOTES
 1. MEASUREMENTS
 TAKEN ON SLOPE.

NOTES
 1. MEASUREMENTS
 TAKEN ON SLOPE.


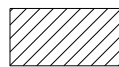

BENCHMARKS

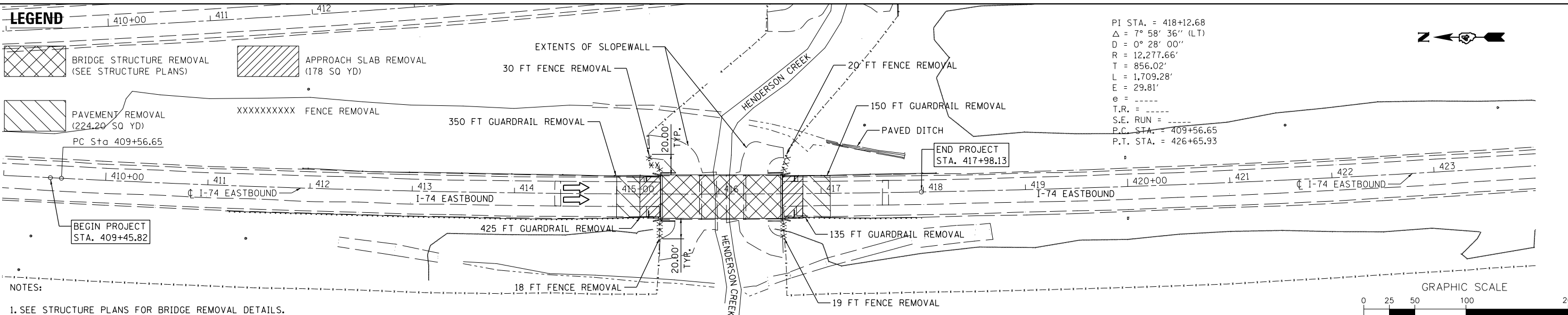
BM #1 CHISELED SQUARE LOCATED ON THE NORTH END OF WEST WING WALL OF SOUTHBOUND HENDERSON CREEK BRIDGE (I-74) ELEV. 758.260
 BM #3 CHISELED SQUARE LOCATED ON THE SOUTH END OF WEST WING WALL OF NORTHBOUND HENDERSON CREEK BRIDGE (I-74) ELEV. 757.278



FILE NAME =	DESIGNED - JCP	REVISED -	 800 WEST FULTON STREET CHICAGO, ILLINOIS 60611-1209 TEL. 312 454 9100 FAX 312 559 1217 WEB www.sepstein.com	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	ALIGNMENT, TIES AND BENCHMARKS			F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
... \D4-174-HendersonCreek-Combined-sht011-ATB.DWG	DRAWN - JCP	REVISED -						74	48-(25B)BR, BR-1J	KNOX	131	11
PLOT TIME = 11:28:52 PM	CHECKED - GAO	REVISED -			SCALE: 1" = 50' SHEET NO. 1 OF 1 SHEETS STA. 415+00.00 TO STA. 418+24.56					CONTRACT NO. 68B85		
PLOT DATE = 8/12/2014	DATE - 08/01/14	REVISED -			ILLINOIS FED. AID PROJECT							

LEGEND

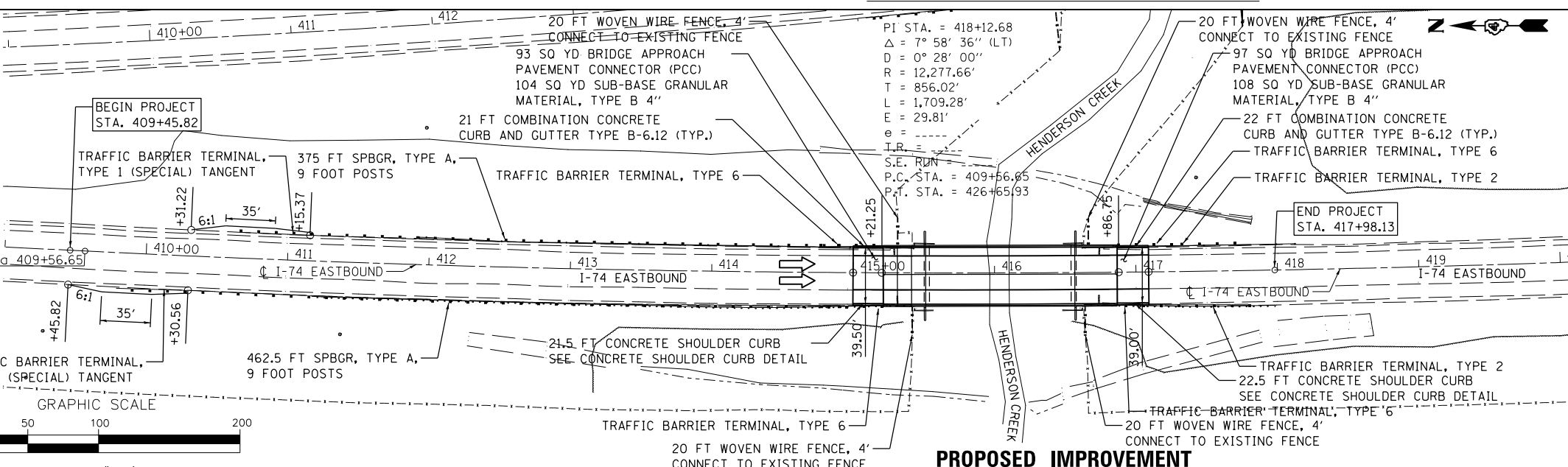
-  BRIDGE STRUCTURE REMOVAL (SEE STRUCTURE PLANS)
-  APPROACH SLAB REMOVAL (178 SQ YD)
-  PAVEMENT REMOVAL (224.20 SQ YD)
- XXXXXXXXXX FENCE REMOVAL



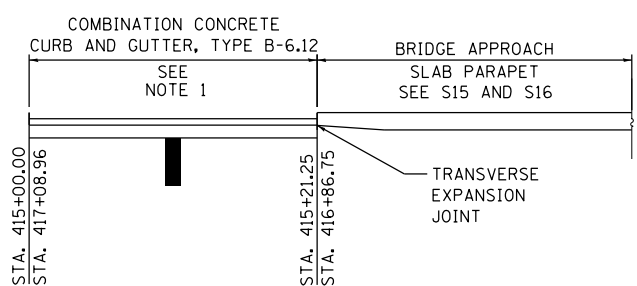
PI STA. = 418+12.68
 $\Delta = 7^\circ 58' 36''$ (LT)
 $D = 0^\circ 28' 00''$
 $R = 12,277.66'$
 $T = 856.02'$
 $L = 1,709.28'$
 $E = 29.81'$
 $e = \text{---}$
 $T.R. = \text{---}$
 $S.E. RUN = \text{---}$
 P.C. STA. = 409+56.65
 P.T. STA. = 426+65.93

NOTES:
 1. SEE STRUCTURE PLANS FOR BRIDGE REMOVAL DETAILS.

EXISTING CONDITION AND DEMOLITION



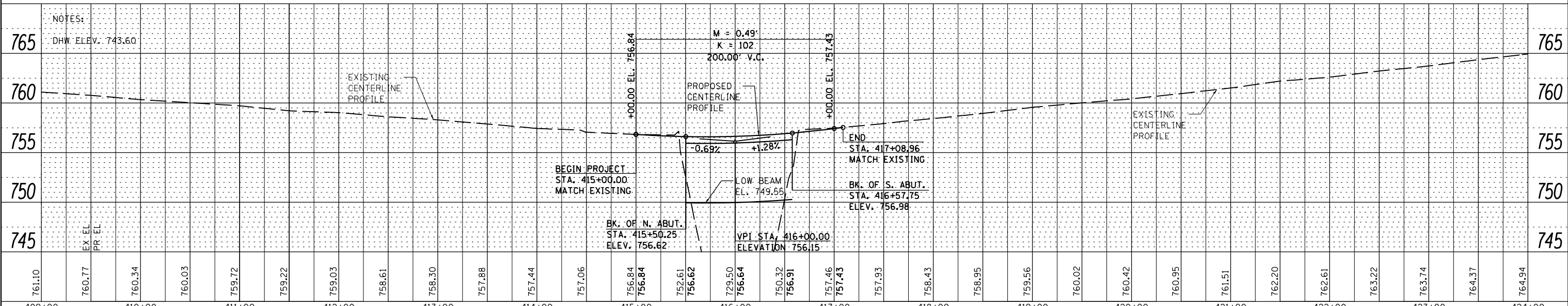
PROPOSED IMPROVEMENT



PLAN VIEW

NOTES:
 1. REFER TO STANDARD DRAWING 606001 FOR COMBINATION CONCRETE CURB AND GUTTER DETAILS.
 2. NORTHEAST CORNER SHOWN. SIMILAR FOR NORTHWEST, SOUTHEAST AND SOUTHWEST CORNERS.


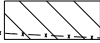

CONCRETE SHOULDER CURB DETAIL




EX. EL.	PR. EL.	409+00	410+00	411+00	412+00	413+00	414+00	415+00	416+00	417+00	418+00	419+00	420+00	421+00	422+00	423+00	424+00																		
761.10	760.77	760.34	760.03	759.72	759.22	759.03	758.61	758.30	757.88	757.44	757.06	756.84	756.84	752.61	756.62	729.50	756.64	750.32	756.91	757.46	757.43	757.93	758.43	758.95	759.56	760.02	760.42	760.95	761.51	762.20	762.61	763.22	763.74	764.37	764.94

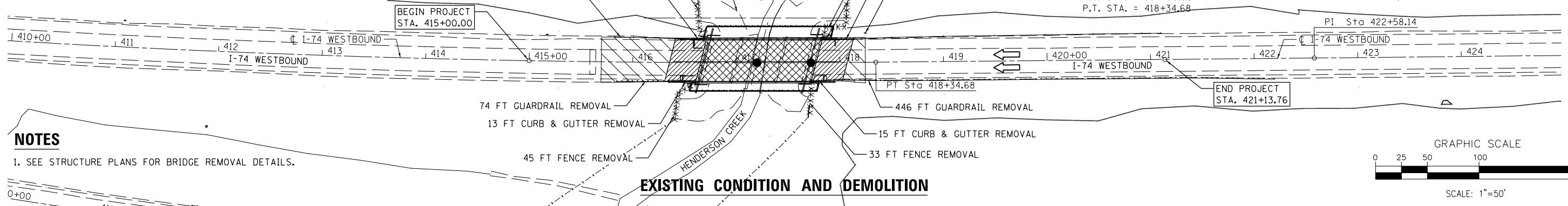
FILE NAME = ...ND4-174-HendersonCreek-Combined-sht012-EBp	DESIGNED - JCP	REVISED -	 800 WEST FULTON STREET CHICAGO, ILLINOIS 60611-1259 TEL 312 454 9100 FAX 312 559 1217 WEB www.stepstein.com	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION			I-74 EASTBOUND ROADWAY PLAN AND PROFILE			F.A.I. R.T.E. 74	SECTION 48-(25B)BR, BR-1J	COUNTY KNOX	TOTAL SHEETS 131	SHEET NO. 12
PLOT TIME = 1:54:52 PM	CHECKED - GAO	REVISED -		SCALE: 1" = 50'			SHEET NO. 1 OF 1 SHEETS			STA. 409+00.00 TO STA. 424+00.00				
PLOT DATE = 8/12/2014	DATE = 08/01/14	REVISED -		CONTRACT NO. 68B85			ILLINOIS FED. AID PROJECT							

LEGEND

-  APPROACH SLAB REMOVAL (280 SQ YD)
-  PAVEMENT REMOVAL (399 SQ YD)
-  BRIDGE STRUCTURE REMOVAL (SEE STRUCTURE PLANS)

XXXXXXXX FENCE REMOVAL
 DIRECTION OF TRAFFIC

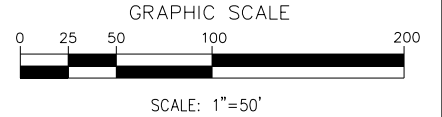
EXIST. CURVE WBC01
 PI STA. = 407+99.60
 $\Delta = 9^\circ 41' 02''$ (LT)
 $D = 0^\circ 28' 00''$
 $R = 12,277.66'$
 $T = 1,040.04'$
 $L = 2,075.12'$
 $E = 43.97'$
 $e = \dots$
 T.R. =
 S.E. RUN =
 P.C. STA. = 397+59.56
 P.T. STA. = 418+34.68



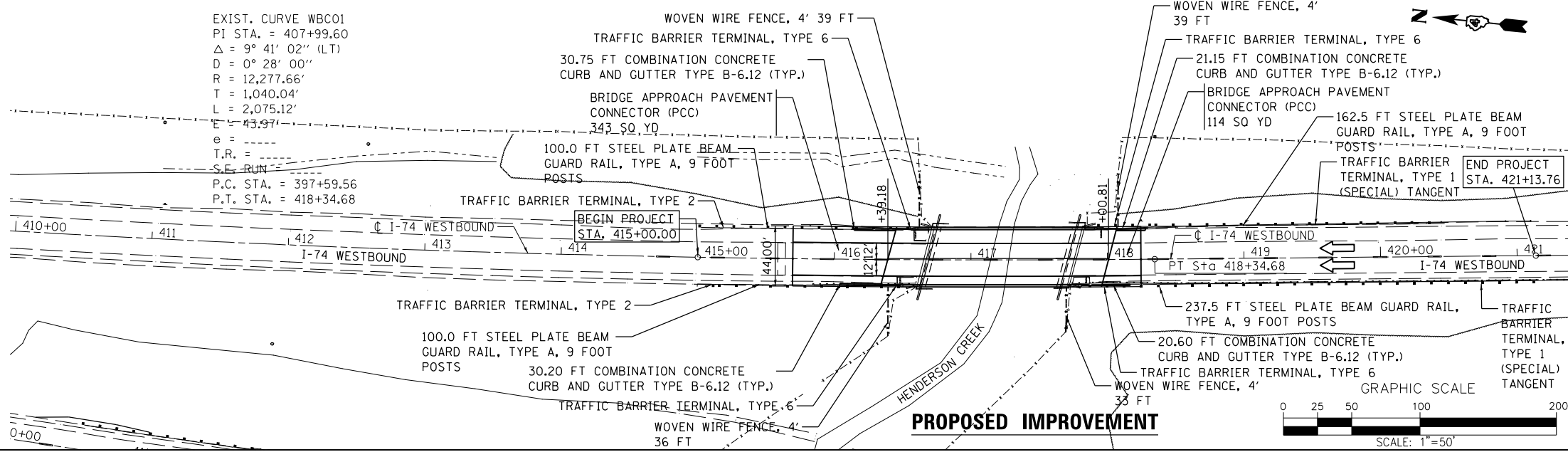
NOTES

1. SEE STRUCTURE PLANS FOR BRIDGE REMOVAL DETAILS.

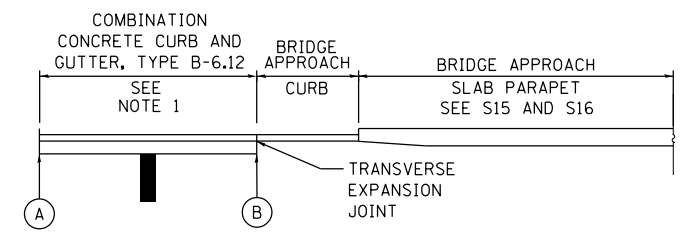
EXISTING CONDITION AND DEMOLITION



DATE	
BY	
PLANNED	
DESIGNED	
CHECKED	
DATE	



PROPOSED IMPROVEMENT



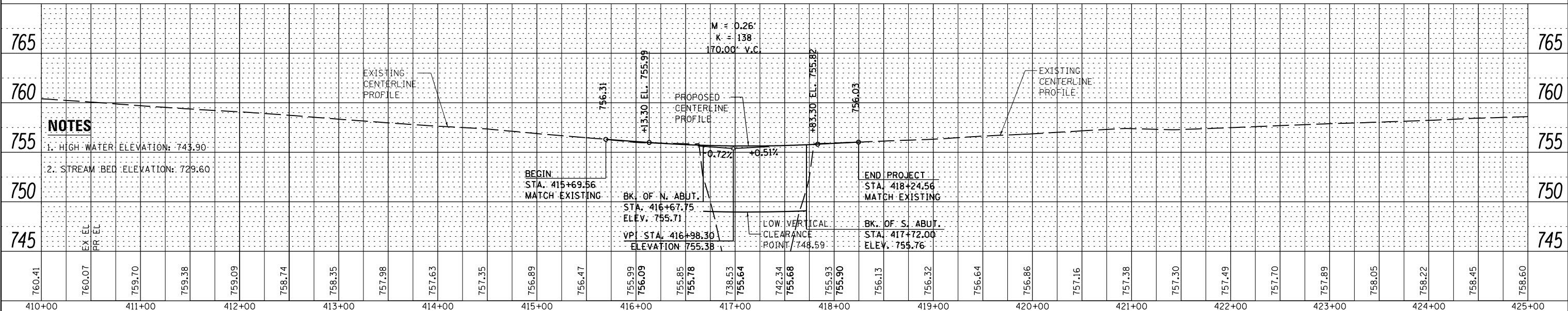
PLAN VIEW

- (A) NE CORNER: STA. 416+14.34
 NW CORNER: STA. 416+04.20
 SE CORNER: STA. 418+22.90
 SW CORNER: STA. 418+11.52
- (B) NE CORNER: STA. 416+45.14
 NW CORNER: STA. 416+34.15
 SE CORNER: STA. 418+06.71
 SW CORNER: STA. 417+95.80

NOTES:
 1. REFER TO STANDARD DRAWING 606001 FOR CONCRETE CURB AND GUTTER DETAILS.
 2. NORTHEAST CORNER SHOWN. SIMILAR FOR NORTHWEST, SOUTHEAST AND SOUTHWEST CORNERS.

CONCRETE SHOULDER CURB DETAIL

DATE	
BY	
PLANNED	
DESIGNED	
CHECKED	
DATE	



NOTES

1. HIGH WATER ELEVATION: 743.90

2. STREAM BED ELEVATION: 729.60

FILE NAME =	DESIGNED - JCP	REVISED -	 STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	I-74 WESTBOUND ROADWAY PLAN AND PROFILE		F.A.I. R.T.E.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
... \D4-174-HendersonCreek-Combined-sh013-WBp	DRAWN - JCP	REVISED -		74	48-(25)BR, BR-1J	KNOX	131	13		
PLOT TIME = 2:22:00 PM	CHECKED - GAO	REVISED -		CONTRACT NO. 68B85						
PLOT DATE = 8/12/2014	DATE - 08/01/14	REVISED -		ILLINOIS FED. AID PROJECT						

SUGGESTED CONSTRUCTION STAGING TRAFFIC CONTROL

THE FOLLOWING SEQUENCE OF TRAFFIC CONTROL IS SUGGESTED. VARIATIONS MAY BE MADE, WITH THE APPROVAL OF THE ENGINEER, IF THE PREVAILING SITE CONDITIONS AT THE TIME OF CONSTRUCTION ALLOW.

STAGE I

USE HIGHWAY STANDARD 701402 TO PLACE TEMPORARY CONCRETE BARRIER TO CLOSE LEFT EASTBOUND LANE OF INTERSTATE 74. REDIRECT TRAFFIC TO USE REMAINING EASTBOUND LANE FOR ONE-WAY TRAFFIC. REMOVE EAST HALF OF EXISTING BRIDGE DECK AND STRUCTURE. CONSTRUCT PROPOSED EAST HALF OF BRIDGE DECK AND STRUCTURE.

USE HIGHWAY STANDARD 701402 TO PLACE TEMPORARY CONCRETE BARRIER TO CLOSE LEFT WESTBOUND LANE OF INTERSTATE 74. REDIRECT TRAFFIC TO USE REMAINING WESTBOUND LANE FOR ONE-WAY TRAFFIC. REMOVE WEST HALF OF EXISTING BRIDGE DECK AND STRUCTURE. CONSTRUCT PROPOSED WEST HALF OF BRIDGE DECK AND STRUCTURE.

STAGE II

USE HIGHWAY STANDARD 701402 TO PLACE TEMPORARY CONCRETE BARRIER TO CLOSE RIGHT EASTBOUND LANE OF INTERSTATE 74. REDIRECT TRAFFIC TO USE REMAINING EASTBOUND LANE FOR ONE-WAY TRAFFIC. REMOVE WEST HALF OF EXISTING SUBSTRUCTURE AND BRIDGE DECK. CONSTRUCT PROPOSED WEST HALF OF SUBSTRUCTURE AND BRIDGE DECK.










USE HIGHWAY STANARD 701402 TO PLACE TEMPORARY CONCRETE BARRIER TO CLOSE RIGHT WESTBOUND LANE OF INTERSTATE 74. REDIRECT TRAFFIC TO USE REMAINING WESTBOUND LANE FOR ONE-WAY TRAFFIC. REMOVE EAST HALF OF EXISTING SUBSTRUCTURE AND BRIDGE DECK. CONSTRUCT PROPOSED EAST HALF OF SUBSTRUCTURE AND BRIDGE DECK.

SUGGESTED CONSTRUCTION STAGING TRAFFIC CONTROL NOTES

1. 4 INCH SOLID WHITE LINES SHALL BE USED TO DELINEATE THE TRAFFIC RIGHT EDGE OF THE PAVEMENT.
2. 4 INCH SOLID YELLOW LINES SHALL BE USED TO DELINEATE THE TRAFFIC LEFT EDGE OF THE PAVEMENT.
3. EXISTING SIGNS AND MESSAGES SHALL BE TEMPORARILY COVERED, MODIFIED OR REMOVED AS DIRECTED BY THE ENGINEER.
4. ALL OF THE TRAFFIC CONTROL DEVICES SHALL CONFORM TO THE TRAFFIC CONTROL PLANS OR THE LATEST EDITION OF THE "ILLINOIS MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES FOR STREETS AND HIGHWAYS" AND SHALL BE IN PLACE BEFORE CONSTRUCTION IS STARTED.
5. THE EXACT NUMBER, LOCATION AND SPACING OF ALL SIGNS AND TRAFFIC CONTROL DEVICES MAY BE ADJUSTED TO FIT FIELD CONDITIONS DIRECTED BY THE ENGINEER.
6. TEMPORARY CONCRETE BARRIER AND TEMPORARY IMPACT ATTENUATORS SHALL BE PLACED AS INDICATED IN THE PLANS AND IN ACCORDANCE WITH SECTION 701 OF THE STANDARD SPECIFICATIONS. TEMPORARY CONCRETE BARRIER SHALL BE IN ACCORDANCE WITH SECTION 704 OF THE STANDARD SPECIFICATIONS, AND SHALL BE PLACED WHERE THE TRAVEL LANE IS ADJACENT TO A DROP OF 3 FEET OR GREATER AND AT OTHER LOCATIONS AS DIRECTED BY THE ENGINEER.
7. THE CONTRACTOR SHALL PROVIDE ADVANCE NOTICE CONSTRUCTION SIGNING, SIGNS SHALL BE ERECTED ONE WEEK IN ADVANCE OF THE START OF CONSTRUCTION. SIGNS SHALL BE REMOVED OR COVERED WHEN PROTECTION IS NOT REQUIRED AND RESTORED AS APPROPRIATE.
8. CONSTRUCTION WORK WILL NOT COMMENCE UNTIL ALL SIGNS AND PAVEMENT MARKINGS IN CONFLICT WITH THE STAGED CONSTRUCTION HAVE BEEN REMOVED AND ALL TEMPORARY SIGNS, PAVEMENT MARKINGS AND BARRICADES ARE IN PLACE AND APPROVED BY THE ENGINEER.
9. THE CONTRACTOR SHALL PROVIDE ALL BARRIERS, SIGNS, SUPPORTS, PAVEMENT MARKING MATERIALS AND LABOR NECESSARY FOR THE MAINTENANCE OF TRAFFIC UNLESS NOTED OTHERWISE IN THE SPECIAL PROVISIONS.
10. IMMEDIATELY AFTER THE COMPLETION OF THE WORK, THE CONTRACTOR SHALL RESTORE ALL PERMANENT PAVEMENT MARKINGS, SIGNS AND OTHER TRAFFIC CONTROL DEVICES THAT WERE COVERED, REMOVED, MODIFIED, DAMAGED OR OTHERWISE AFFECTED BY THE CONSTRUCTION.
11. TRAFFIC CONTROL DEVICES AND TEMPORARY CONCRETE BARRIER WALL SHALL BE IN ACCORDANCE WITH I.D.O.T. TRAFFIC CONTROL STANDARD 701901 AND 704001.
12. FOR EACH STAGE OF CONSTRUCTION PROVIDE TRAFFIC CONTROL AS SHOWN ON THE MAINTENANCE OF TRAFFIC PLANS, COORDINATE INSTALLATION OF TEMPORARY PAVEMENT MARKINGS AND OTHER TRAFFIC CONTROL DEVICES WITH THE EXISTING TRAFFIC PATTERNS AT THE ENDS OF THE PROJECT.
13. COST OF ANCHORING TEMPORARY CONCRETE BARRIER TO BRIDGE DECK TO BE INCLUDED IN TRAFIC CONTROL AND PROTECTION, STANDARD 701402 PAY ITEM. SEE SHEET S5 OF S25.

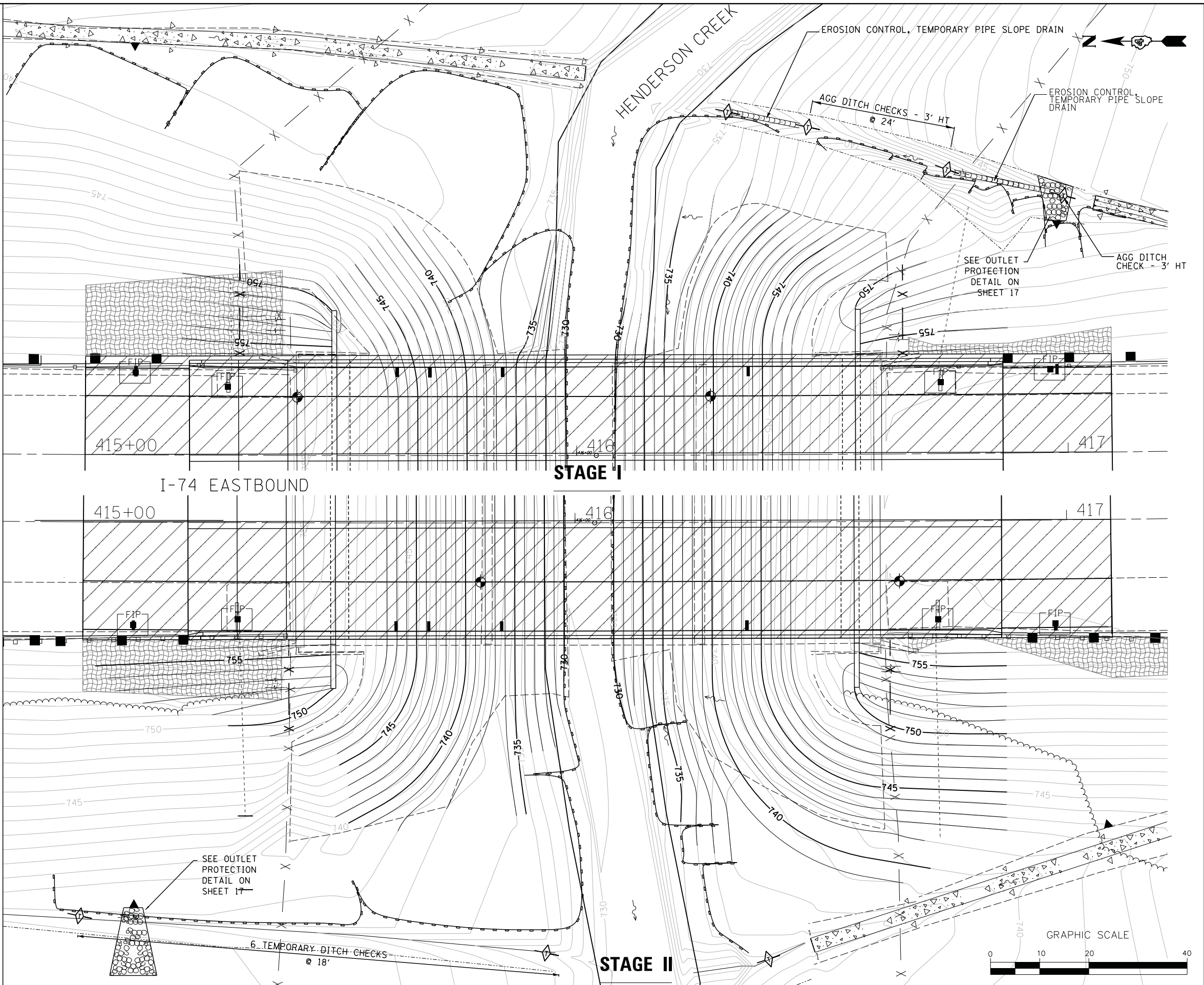
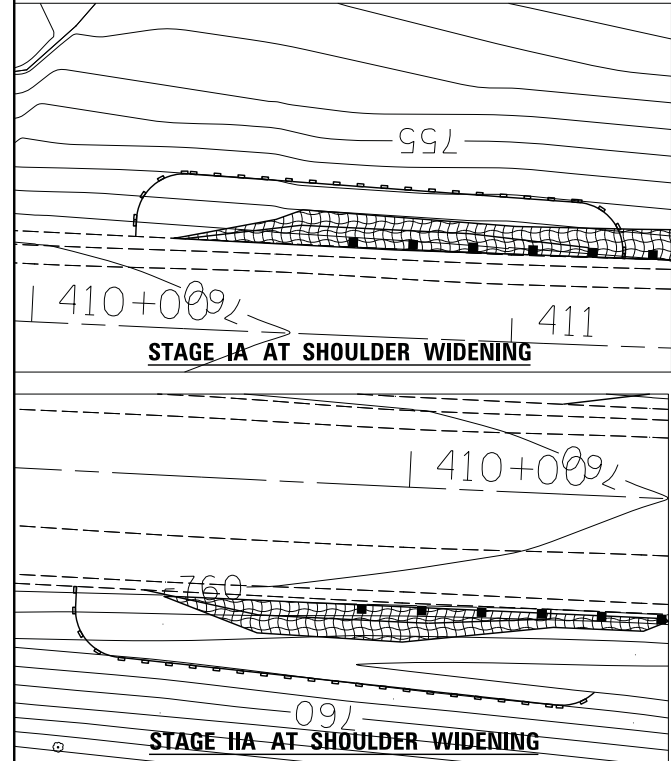
FILE NAME =	DESIGNED - JCP	REVISED - -	 <p>800 WEST FULTON STREET TEL 312 454 9100 CHICAGO, ILLINOIS FAX 312 555 1217 60661-1259 WEB www.sepstein.com</p>	<p align="center">STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION</p>	<p align="center">SUGGESTED STAGES OF CONSTRUCTION AND TRAFFIC CONTROL</p>		F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
... \D4-174-HendersonCreek-Combined-sht014-stp	DRAWN - JCP	REVISED - -					74	48-(25B)BR, BR-1J	KNOX	131	14
PLOT TIME = 4:30:55 PM	CHECKED - GAO	REVISED - -			<p align="center">NOTES</p>		<p align="center">CONTRACT NO. 68B85</p>		<p align="center">ILLINOIS FED. AID PROJECT</p>		
PLOT DATE = 7/30/2014	DATE - 08/01/14	REVISED - -			SCALE: N/A	SHEET NO. 1 OF 1 SHEETS	STA.	TO STA.			

LEGEND

-  PERIMETER EROSION BARRIER
-  WORK ZONE
-  TEMPORARY PIPE SLOPE DRAIN
-  EXISTING PAVED DITCH
-  RIP RAP
-  FABRIC INLET PROTECTION
-  TEMPORARY DITCH CHECKS
-  HEAVY DUTY EROSION CONTROL BLANKET
-  END SECTION, 12" (SEE DRAINAGE SHEET)

NOTES

1. CONTRACTOR SHALL INSTALL PERIMETER EROSION BARRIER PRIOR TO STRIPPING VEGETATION.
2. NO SEDIMENT SHALL BE ALLOWED TO FLOW DOWNSTREAM AT ANY TIME. ALL WATER FROM DEWATERING OPERATIONS SHALL BE FILTERED TO REMOVE SEDIMENT BEFORE IT IS DISCHARGED INTO THE CREEK.
3. HEAVY DUTY EROSION CONTROL BLANKETS SHALL BE USED FOR AREAS ADJACENT TO BODIES OF WATER, FOR LINING DITCHES AND FOR SLOPES 1V:3H OR STEEPER.
4. FOR RIPRAP LIMITS SEE SHEET S1 OF S25 FOR STRUCTURE NO. 048-0090
5. ALL DISTURBED AREAS SHALL BE RESEEDED WITH SEEDING, CLASS 2A.
6. THE EXPECTED DISTURBED WORK AREA FOR THIS PROJECT IS APPROXIMATELY 0.74 ACRES. IF THE WORK AREA DISTURBANCE EXCEEDS 1 ACRE, NOTIFICATION TO THE ILLINOIS ENVIRONMENTAL PROTECTION AGENCY IS REQUIRED.



FILE NAME =	DESIGNED - JCP	REVISED -
...\\04-174-HendersonCreek-Combined-sht015-Ed	DRAWN - JCP	REVISED -
PLOT TIME = 4:38:57 PM	CHECKED - GAO	REVISED -
PLOT DATE = 7/30/2014	DATE - 08/01/14	REVISED -

DESIGNED - JCP	REVISED -
DRAWN - JCP	REVISED -
CHECKED - GAO	REVISED -
DATE - 08/01/14	REVISED -

SEPSTEIN
 800 WEST FULTON STREET
 CHICAGO, ILLINOIS 60611-1259
 TEL 312 454 9100
 FAX 312 455 1217
 WEB www.sepstein.com









**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**I-74 EASTBOUND
 EROSION & SEDIMENT CONTROL PLAN**
 SCALE: 1" = 10'
 SHEET NO. 1 OF 1 SHEETS
 STA. 410+00.00 TO STA. 422+00.00

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
74	48-(125B)BR, BR-1J	KNOX	131	15

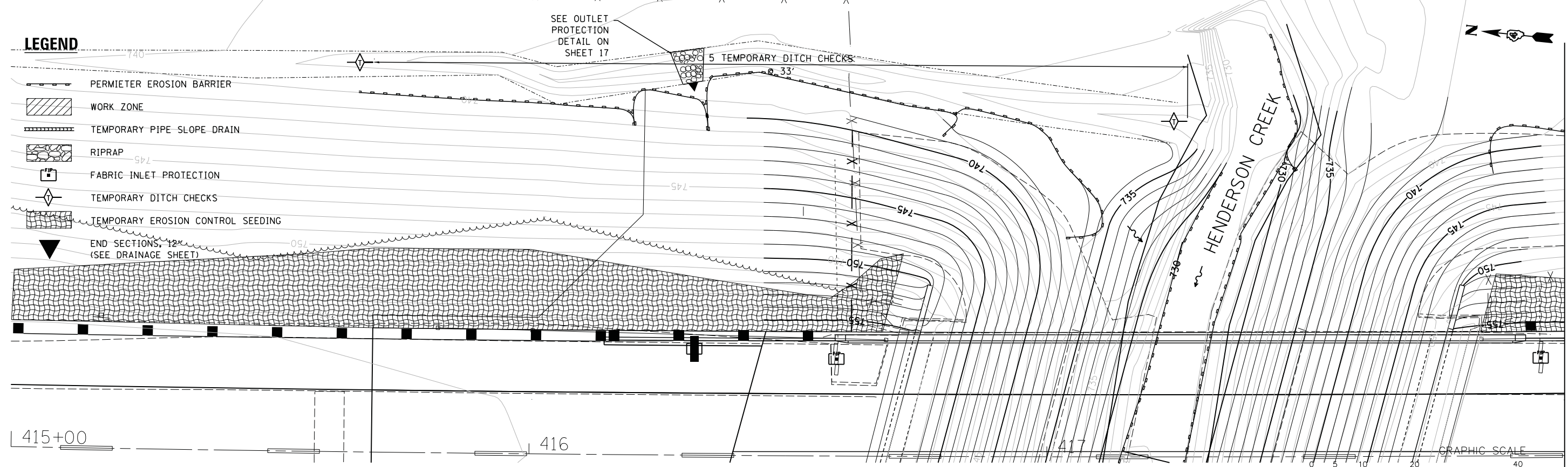
CONTRACT NO. 68B85
 ILLINOIS FED. AID PROJECT

LEGEND

-  PERMIETER EROSION BARRIER
-  WORK ZONE
-  TEMPORARY PIPE SLOPE DRAIN
-  RIPRAP
-  FABRIC INLET PROTECTION
-  TEMPORARY DITCH CHECKS
-  TEMPORARY EROSION CONTROL SEEDING
-  END SECTIONS, 12' (SEE DRAINAGE SHEET)

SEE OUTLET PROTECTION DETAIL ON SHEET 17

5 TEMPORARY DITCH CHECKS @ 33'



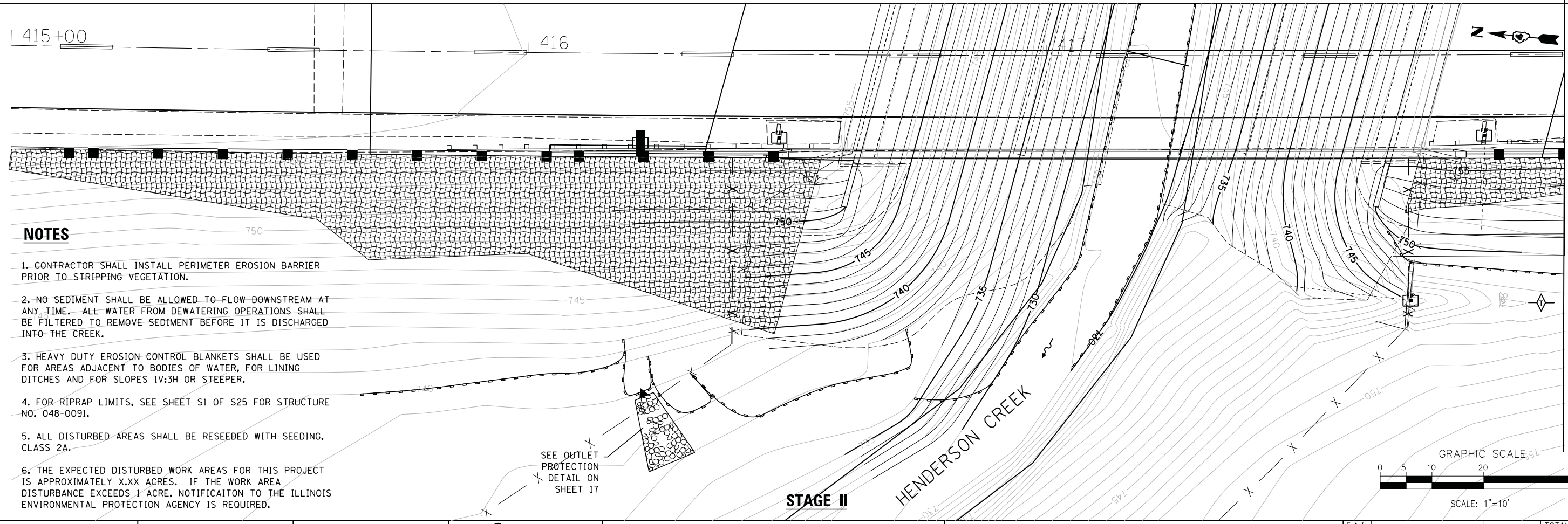
STAGE I

SCALE: 1"=10'

MATCH LINE STA 418+00.00

415+00

416



STAGE II

SCALE: 1"=10'

MATCH LINE STA 418+00.00

NOTES

1. CONTRACTOR SHALL INSTALL PERIMETER EROSION BARRIER PRIOR TO STRIPPING VEGETATION.
2. NO SEDIMENT SHALL BE ALLOWED TO FLOW DOWNSTREAM AT ANY TIME. ALL WATER FROM DEWATERING OPERATIONS SHALL BE FILTERED TO REMOVE SEDIMENT BEFORE IT IS DISCHARGED INTO THE CREEK.
3. HEAVY DUTY EROSION CONTROL BLANKETS SHALL BE USED FOR AREAS ADJACENT TO BODIES OF WATER, FOR LINING DITCHES AND FOR SLOPES 1V:3H OR STEEPER.
4. FOR RIPRAP LIMITS, SEE SHEET S1 OF S25 FOR STRUCTURE NO. 048-0091.
5. ALL DISTURBED AREAS SHALL BE RESEEDED WITH SEEDING, CLASS 2A.
6. THE EXPECTED DISTURBED WORK AREAS FOR THIS PROJECT IS APPROXIMATELY X.XX ACRES. IF THE WORK AREA DISTURBANCE EXCEEDS 1 ACRE, NOTIFICATION TO THE ILLINOIS ENVIRONMENTAL PROTECTION AGENCY IS REQUIRED.

SEE OUTLET PROTECTION DETAIL ON SHEET 17

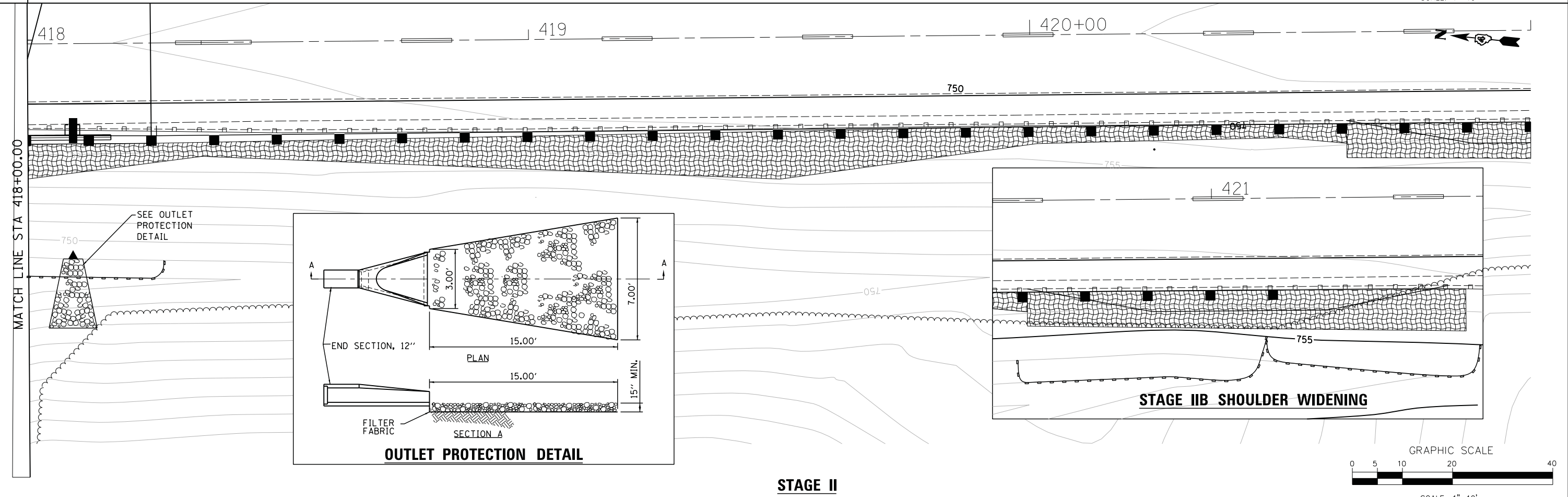
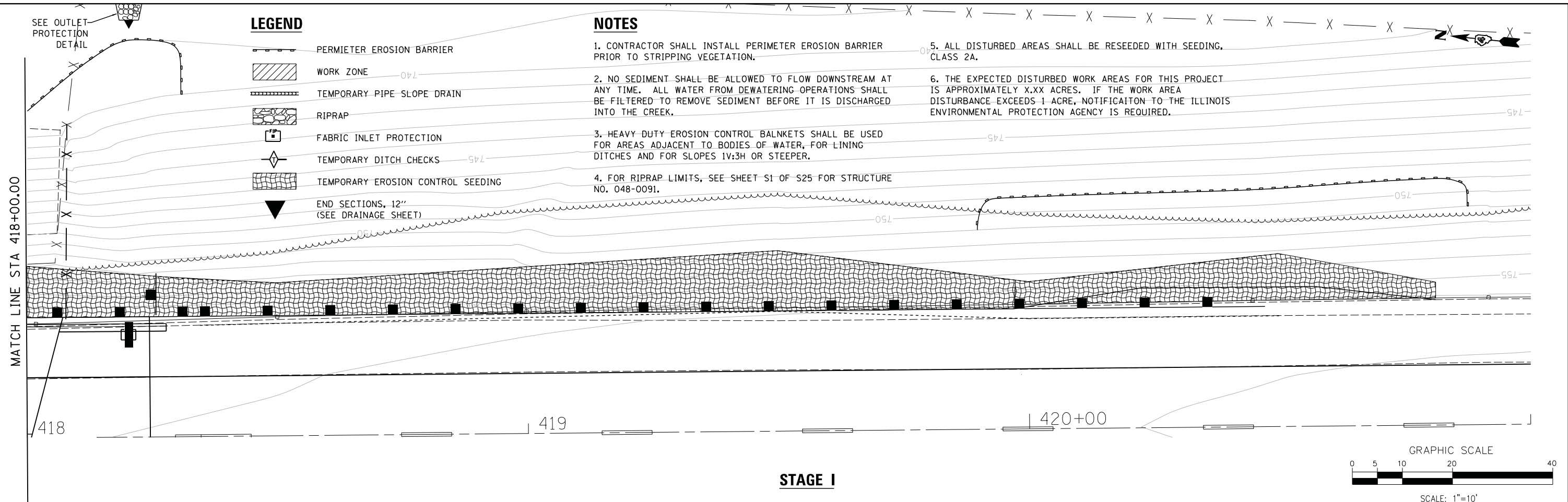
FILE NAME =	DESIGNED - JCP	REVISED -
...\\D4-174-HendersonCreek-Combined-sht016-Web	DRAWN - JCP	REVISED -
PLOT TIME = 4:30:59 PM	CHECKED - GAO	REVISED -
PLOT DATE = 7/30/2014	DATE - 08/01/14	REVISED -

SEPSTEIN
 600 WEST FULTON STREET
 CHICAGO, ILLINOIS 60661-1259
 TEL 312 454 9100
 FAX 312 559 1217
 WEB www.sepstein.com

**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**I-74 WESTBOUND
 EROSION & SEDIMENT CONTROL PLAN**
 SCALE: 1" = 10' SHEET NO. 1 OF 2 SHEETS STA. 415+00.00 TO STA. 418+00.00

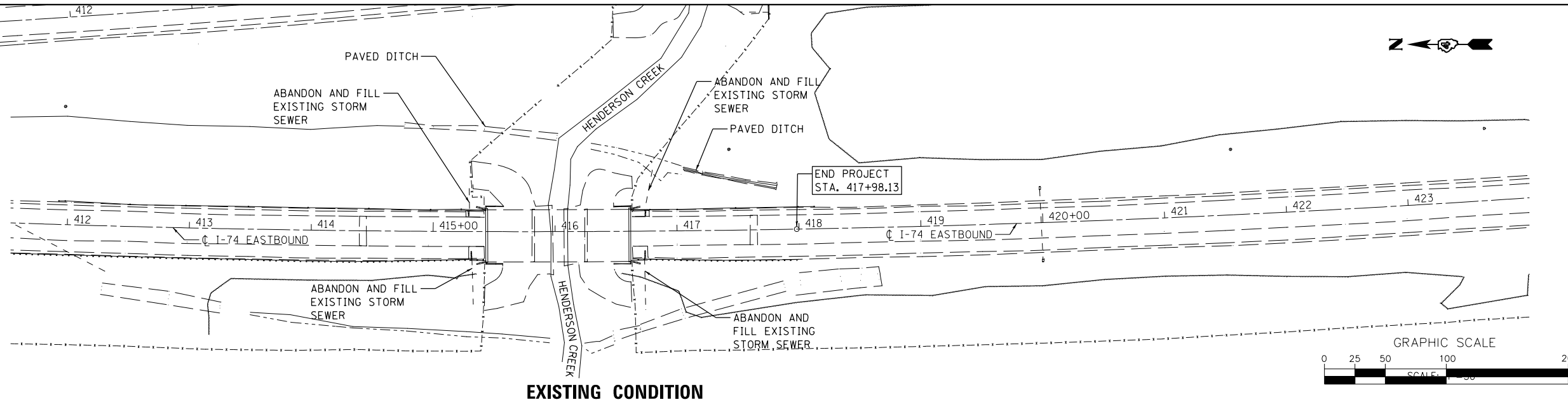
F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
74	48-(25B)BR, BR-1J	KNOX	131	16
CONTRACT NO. 68B85				
ILLINOIS FED. AID PROJECT				



FILE NAME =	DESIGNED - JCP	REVISED -	 800 WEST FULTON STREET CHICAGO, ILLINOIS 60611-1259 TEL 312 454 9100 FAX 312 559 1217 WEB www.sepstein.com	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION		I-74 WESTBOUND EROSION & SEDIMENT CONTROL PLAN		F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
... \D4-174-HendersonCreek-Combined-sht017-Web	DRAWN - JCP	REVISED -						74	48-(25B)BR, BR-1J	KNOX	131	17
PLOT TIME = 4:31:00 PM	CHECKED - GAO	REVISED -						CONTRACT NO. 68B85				
PLOT DATE = 7/30/2014	DATE - 08/01/14	REVISED -						ILLINOIS FED. AID PROJECT				
			SCALE: 1" = 10'			SHEET NO. 2 OF 2 SHEETS		STA. 418+00.00 TO STA. 421+00.00				

NOTES

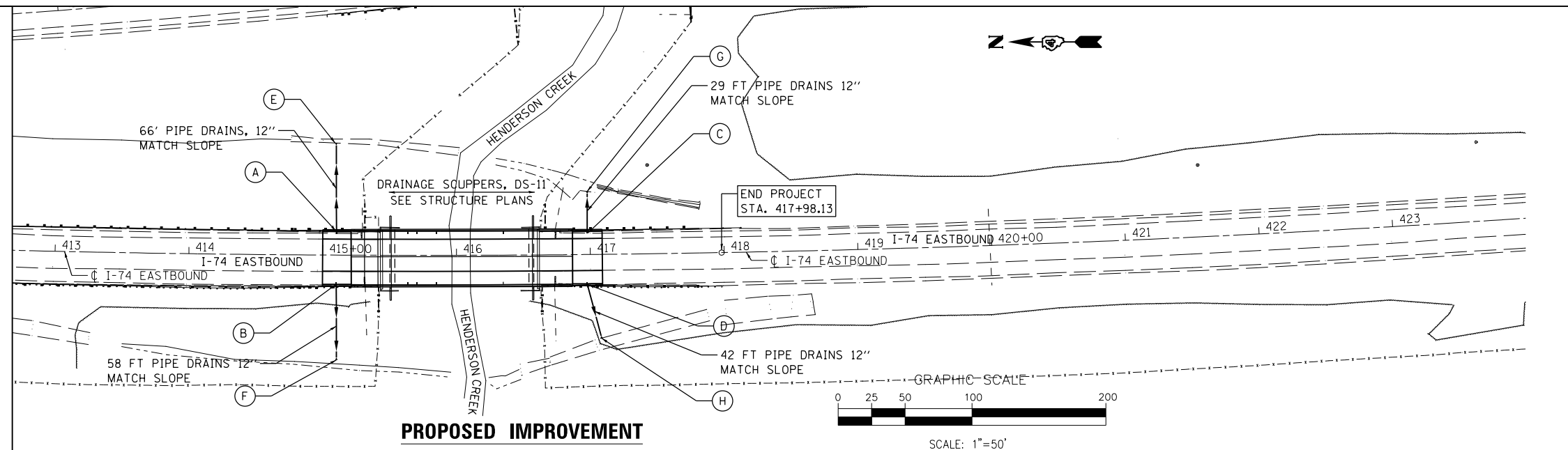
1. ALL 4" PERFORATED PIPE UNDERDRAINS SHALL OUTLET TO DITCHES. SEE STRUCTURE PLANS FOR LOCATION AND DETAILS.
2. CONTRACTOR SHALL VERIFY EXISTING PIPE LOCATIONS AND INVERT ELEVATIONS PRIOR TO CONSTRUCTION.
3. ALL OFFSETS AND RIM ELEVATIONS ARE TO EDGE OF SHOULDER.
4. PROVIDE SEEPAGE COLLAR AND CONCRETE THRUST BLOCK ACCORDING TO DISTRICT FOUR STANDARD 601101-D4 FOR EACH PIPE DRAIN 12" (FOUR LOCATIONS).



EXISTING CONDITION

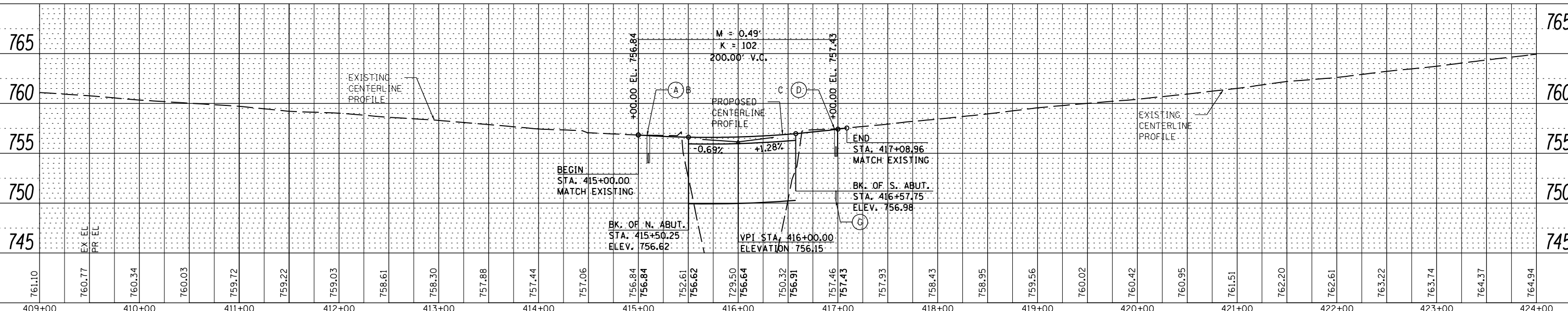
PLAN	SURVEYED	DATE
	PLOTTED	BY
	GRADES CHECKED	
	STRUCTURE NOTATIONS OK'D	
	NO. _____	

- (A) TYPE G INLET BOX, STANDARD 610001
STA. 415+10, 18 LT
RIM EL. 756.47
INV. EL. 753.72
- (B) TYPE F INLET BOX, STANDARD 610001
STA. 415+10, 22 RT
RIM EL. 756.39
INV. EL. 753.64
- (C) TYPE G INLET BOX, STANDARD 610001
STA. 416+98, 18 LT
RIM EL. 757.10
INV. EL. 754.35
- (D) TYPE F INLET BOX, STANDARD 610001
STA. 416+98, 22 RT
RIM EL. 757.02
INV. EL. 754.27
- (E) END SECTIONS 12" OUTLET TO EXISTING DITCH
STA. 415+10, 83 LT
INV. EL. 737.742
- (F) END SECTIONS 12" OUTLET TO TREE LINE
STA. 415+11, 78 RT
INV. EL. 738.260
- (G) END SECTIONS 12" OUTLET TO EXISTING DITCH
STA. 416+98, 45 LT
INV. EL. 750.25
- (H) END SECTIONS 12" OUTLET TO EXISTING DITCH
STA. 417+08, 63 RT
INV. EL. 742.80



PROPOSED IMPROVEMENT

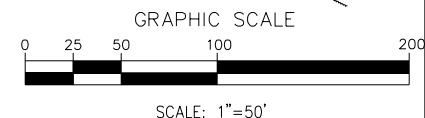
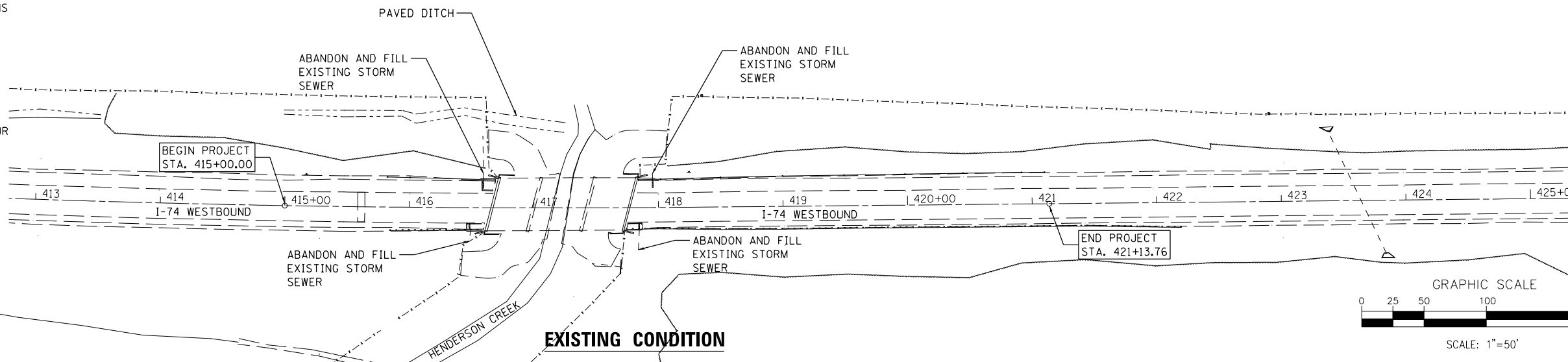
PROFILE	SURVEYED	DATE
	PLOTTED	BY
	GRADES CHECKED	
	STRUCTURE NOTATIONS OK'D	
	NO. _____	



FILE NAME =	DESIGNED - JCP	REVISED -		STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION			I-74 EASTBOUND DRAINAGE PLAN AND PROFILE			F.A.I. R.T.E.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
...\\D4-174-HendersonCreek-Combined-sh018-EB01	DRAWN - JCP	REVISED -								74	48-(25B)BR, BR-1J	KNOX	131	18
PLOT TIME = 2:32:49 PM	CHECKED - GAO	REVISED -								CONTRACT NO. 68B85				
PLOT DATE = 8/12/2014	DATE = 08/01/14	REVISED -								ILLINOIS FED. AID PROJECT				

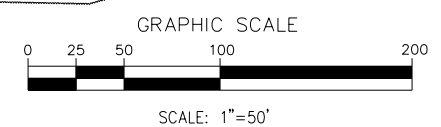
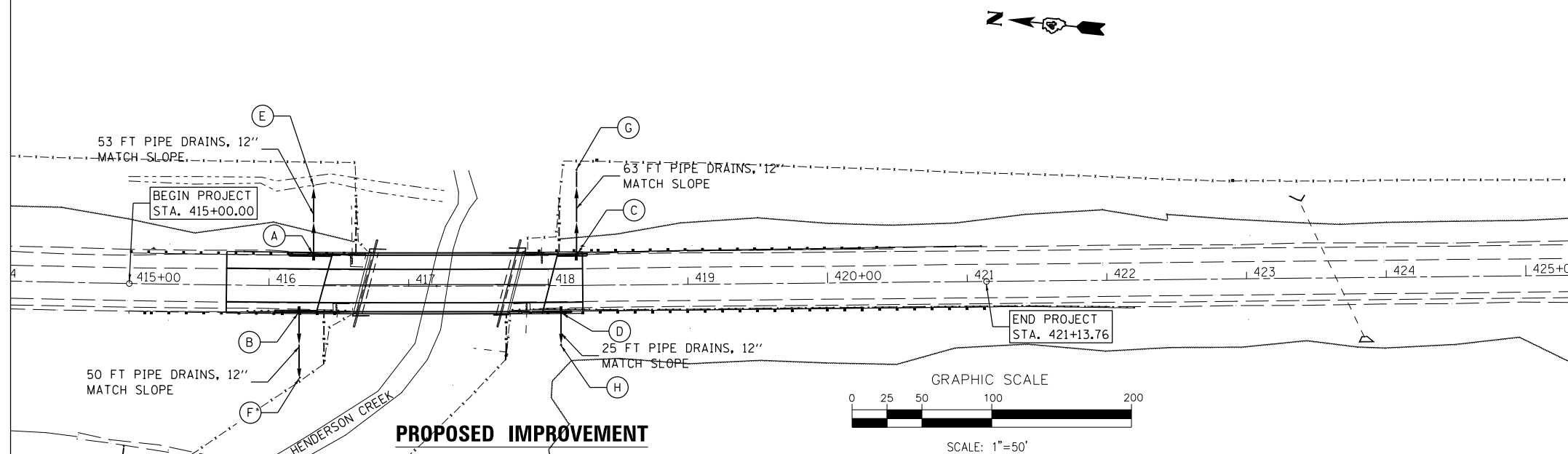
NOTES:

1. ALL PERFORATED PIPE UNDERDRAINS SHALL OUTLET TO DITCHES. SEE STRUCTURE PLANS FOR LOCATION AND DETAILS.
2. CONTRACTOR SHALL VERIFY EXISTING PIPE LOCATIONS AND INVERT ELEVATIONS.
3. ALL OFFSETS ARE TO EDGE OF SHOULDER.
4. PROVIDE SEEPAGE COLLAR AND CONCRETE THRUST BLOCK ACCORDING TO DISTRICT FOUR STANDARD 601101-D4 FOR EACH PIPE DRAIN 12" (FOUR LOCATIONS).

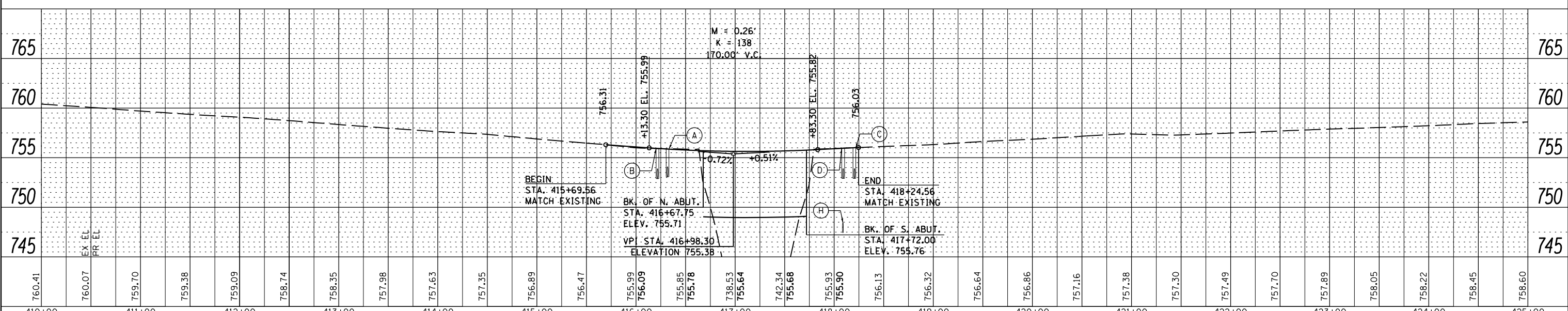


PLAN	SURVEYED	DATE
NOTE BOOK	PLOTTED	BY
NO.	ALIGNED	
	CADD FILE NAME	

- (A) TYPE F INLET BOX, STANDARD 610001
STA. 416+31.84, 20.57 LT
RIM EL. 755.84
INV EL. 753.09
- (B) TYPE G INLET BOX, STANDARD 610001
STA. 416+21.70, 17.26 RT
RIM EL. 755.64
INV EL. 752.89
- (C) TYPE F INLET BOX, STANDARD 610001
STA. 418+20.40, 20.47 LT
RIM EL. 755.66
INV EL. 752.91
- (D) TYPE G INLET BOX, STANDARD 610001
STA. 418+09.02, 17.39 RT
RIM EL. 755.66
INV EL. 752.91
- (E) END SECTIONS, 12" OUTLET TO EXISTING DITCH
STA. 416+31.23, 72 LT
INV EL. 738.56
- (F) END SECTIONS, 12" OUTLET TO TREE LINE
STA. 416+22.07, 65 RT
INV EL. 739.00
- (G) END SECTIONS, 12" OUTLET TO EXISTING DITCH
STA. 418+20.87, 83 LT
INV EL. 738.75
- (H) END SECTIONS, 12" OUTLET TO EXISTING DITCH
STA. 418+08.80, 47 RT
INV EL. 749.00



PROFILE	SURVEYED	DATE
NOTE BOOK	GRADES CHECKED	BY
NO.	STRUCTURE NOTATIONS OK'D	

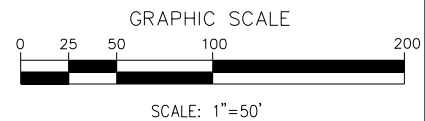
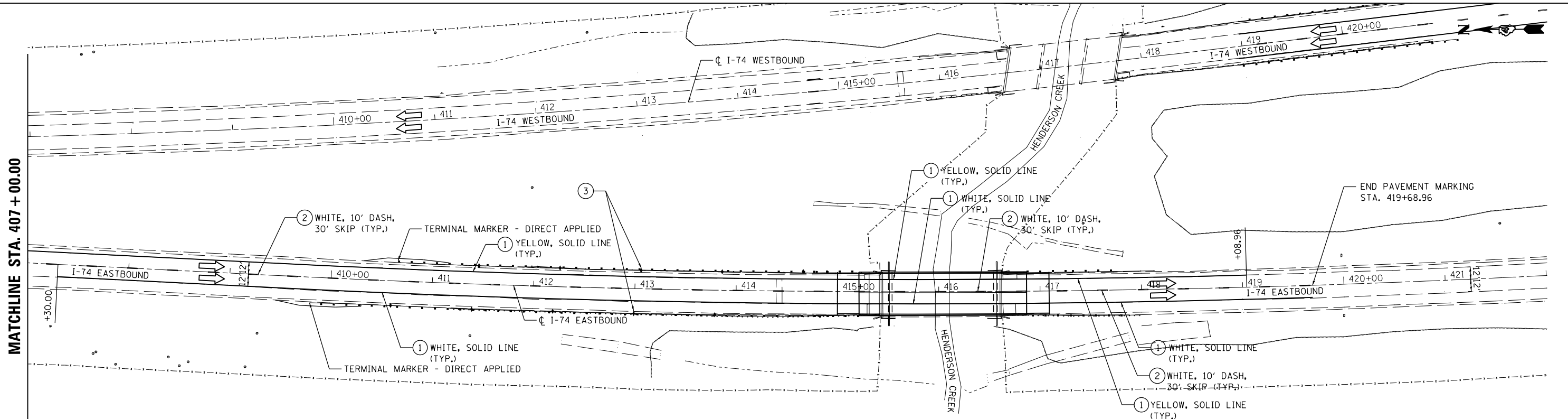
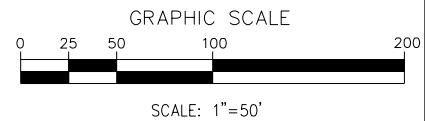
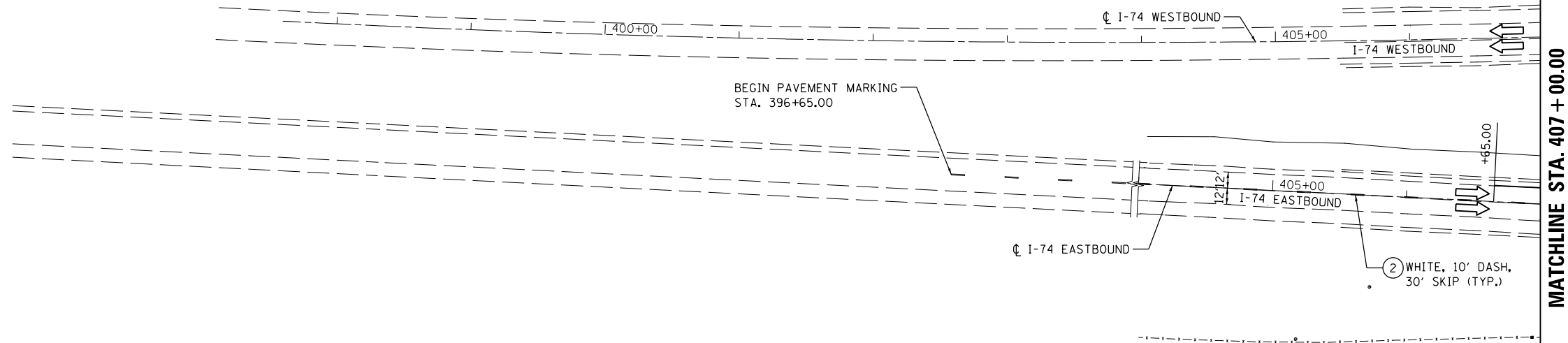


FILE NAME =	DESIGNED - JCP	REVISED -	<p>STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION</p>	I-74 WESTBOUND DRAINAGE PLAN AND PROFILE		F.A.I. R.E.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
...D4-174-HendersonCreek-Combined-sh019-WBo	DRAWN - JCP	REVISED -		74	48-(25B)BR, BR-1J	KNOX	131	19		
PLOT TIME = 2:58:08 PM	CHECKED - GAO	REVISED -		CONTRACT NO. 68B85						
PLOT DATE = 8/12/2014	DATE = 08/01/14	REVISED -		ILLINOIS FED. AID PROJECT						

LEGEND

- ① MODIFIED URETHANE PAVEMENT MARKING - LINE 4"
- ② MODIFIED URETHANE PAVEMENT MARKING, - LINE 6"
- ③ GUARDRAIL MARKERS, TYPE B PER IDOT STANDARD 635001 AND 635006. MARKERS ON RIGHT SHALL BE CRYSTAL AND MARKERS ON LEFT SHALL BE AMBER.

➔ DIRECTION OF TRAFFIC



FILE NAME = ...\\D4-174-HendersonCreek-Combined-sh1020-ES...
 PLOT TIME = 3:03:53 PM
 PLOT DATE = 8/12/2014

DESIGNED - JCP
 DRAWN - JCP
 CHECKED - GAO
 DATE - 08/01/14

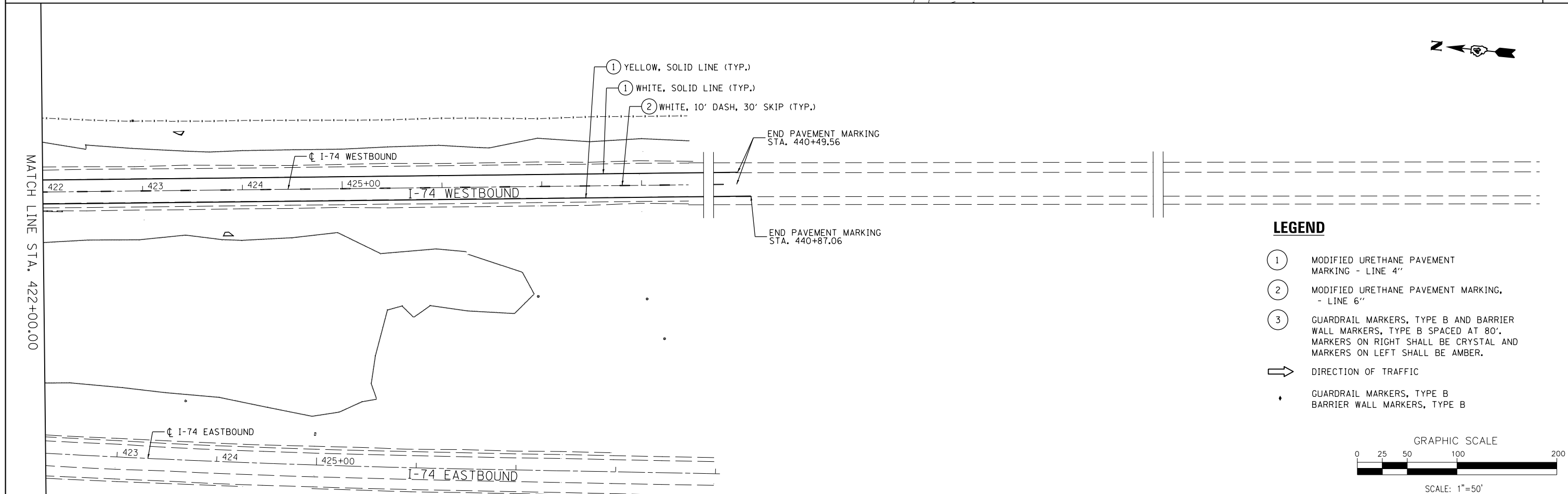
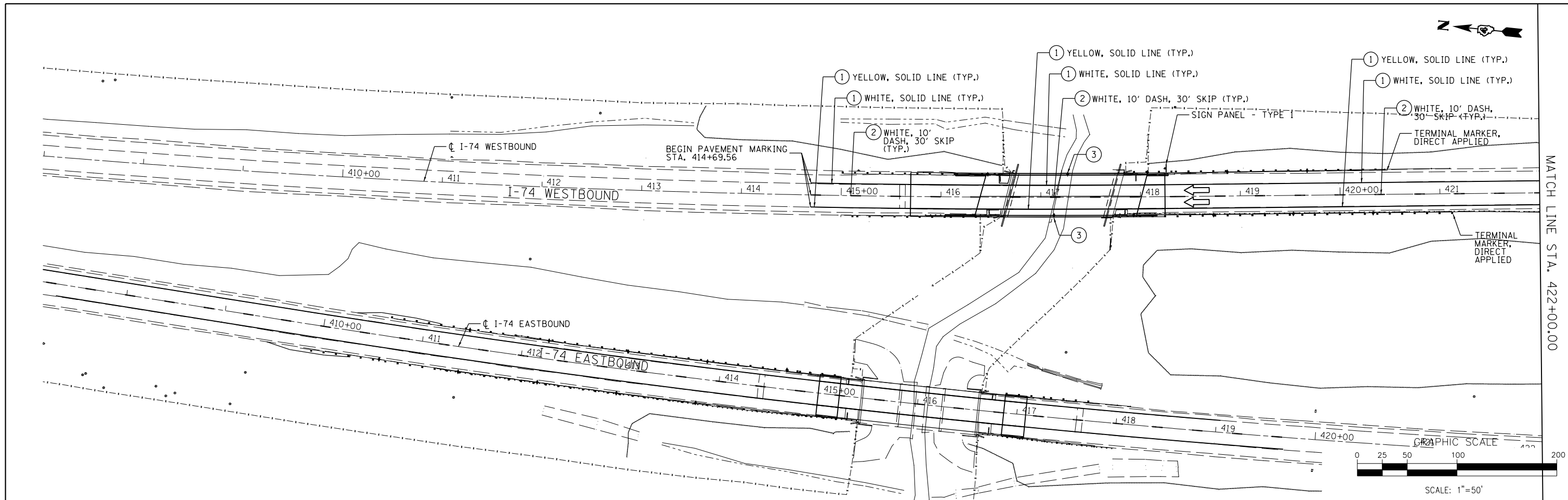
REVISED - -
 REVISED - -
 REVISED - -
 REVISED - -



STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

I-74 EASTBOUND
PAVEMENT MARKING PLAN
 SCALE: 1" = 50 SHEET NO. 1 OF 1 SHEETS STA. 404+00.00 TO STA. 422+00.00

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
74	48-(25B)BR, BR-1J	KNOX	131	20
CONTRACT NO. 68B85				
ILLINOIS FED. AID PROJECT				

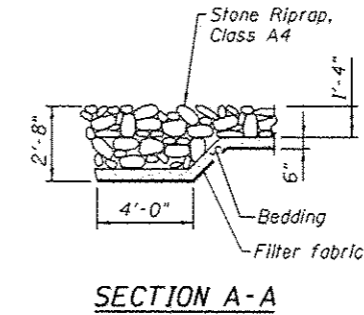
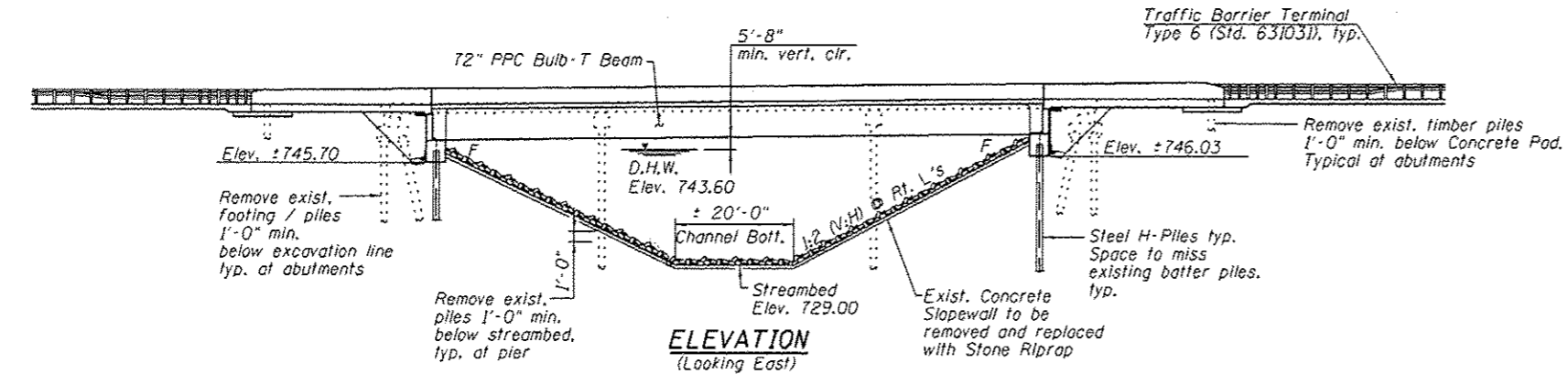


- LEGEND**
- ① MODIFIED URETHANE PAVEMENT MARKING - LINE 4"
 - ② MODIFIED URETHANE PAVEMENT MARKING, - LINE 6"
 - ③ GUARDRAIL MARKERS, TYPE B AND BARRIER WALL MARKERS, TYPE B SPACED AT 80'. MARKERS ON RIGHT SHALL BE CRYSTAL AND MARKERS ON LEFT SHALL BE AMBER.
 - ➔ DIRECTION OF TRAFFIC
 - GUARDRAIL MARKERS, TYPE B BARRIER WALL MARKERS, TYPE B

FILE NAME = ... \D4-174-HendersonCreek-Combined-sht021-WS	DESIGNED - JCP	REVISED -	<p>800 WEST FULTON STREET CHICAGO, ILLINOIS 60611-1209</p> <p>TEL 312 454 9100 FAX 312 555 1217 WEB www.sepstein.com</p>	<p align="center">STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION</p> <p align="center">I-74 WESTBOUND PAVEMENT MARKING AND SIGNING PLAN</p>			F.A.I. RTE. 74	SECTION 48-(25B)BR, BR-1J	COUNTY KNOX	TOTAL SHEETS 131	SHEET NO. 21			
PLOT TIME = 3:04:44 PM	CHECKED - GAO	REVISED -					SCALE: 1" = 50'	SHEET NO. 1	OF 1 SHEETS	STA. 409+00.00	TO STA. 422+00.00	CONTRACT NO. 68B85		
PLOT DATE = 8/12/2014	DATE - 08/01/14	REVISED -					ILLINOIS FED. AID PROJECT							

Bench Mark: BM#1 - Chiseled square located on the north end of the west wingwall EB I-74 Elevation 758.26
 Existing Structure: S.N. 048-0003 built in 1965 as F.A.I. Route 74 (EB), Section 48-25B at Sta. 416+04. Existing structure consists of 3-span continuous reinforced concrete cast in place slab supported by closed concrete abutments and concrete pile bent piers. Structure is 121'-6" long Bk. to Bk. of abutments, width is 44'-0" Out to Out of deck. Structure to be removed and replaced. Traffic to be maintained utilizing stage construction.

No salvage



DESIGN SPECIFICATIONS
 2012 AASHTO LRFD Bridge Design Specifications

LOADING HL-93
 Allow 50#/sq. ft. for future wearing surface.

DESIGN STRESSES
FIELD UNITS

$f'_c = 3,500$ psi
 $f_y = 60,000$ psi (Reinforcement)

PRECAST PRESTRESSED UNITS

$f'_c = 6,000$ psi
 $f'_ci = 5,000$ psi
 $f_{pu} = 270,000$ psi
 $f_{pb1} = 201,960$ psi
 $f_{pb2} = 201,960$ psi

SEISMIC DATA

Seismic Performance Zone (SPZ) = 1
 Design Spectral Acceleration at 1.0 sec. (SD1) = 0.070g
 Design Spectral Acceleration at 0.2 sec. (SDS) = 0.111g
 Soil Site Class = C

DESIGN SCOUR ELEVATION TABLE

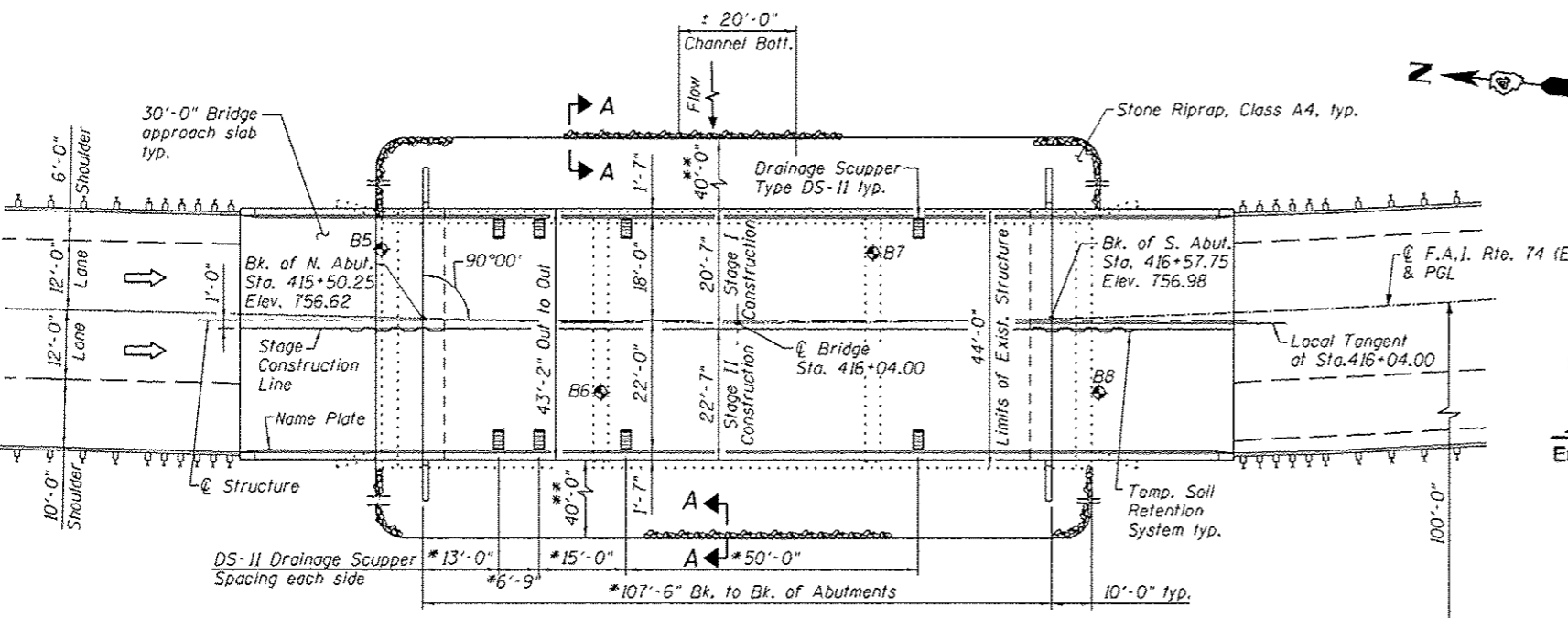
Design Scour Elevation (ft.)	N. Abut.	S. Abut.
	745.68	746.03

WATERWAY INFORMATION

Drainage Area = 9.34 sq. mi. Low Grade Elev. 756.87 ft @ Sta. 415+43.5

Flood	Freq. Yr.	Q C.F.S.	Opening Sq. Ft.		Head - Ft.		Headwater El.		
			Exist.	Prop.	Exist.	Prop.	Exist.	Prop.	
Design	10	1942	530	612	741.5	0.2	0.2	741.7	741.7
Base	50	3168	695	722	743.6	0.3	0.3	743.9	743.9
Overtopping	100	3740	762	768	744.4	0.3	0.3	744.8	744.7
Max. Calc.	500	5135	924	920	746.2	0.4	0.4	746.6	746.5

EWSE = 731.75 (per Survey taken 01-22-2010)



APPROVED
 For Structural Adequacy Only
De Carol Perry
 Engineer of Bridges & Structures

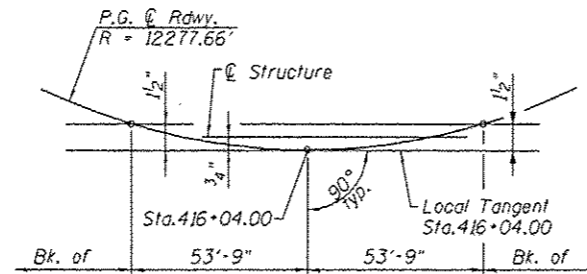
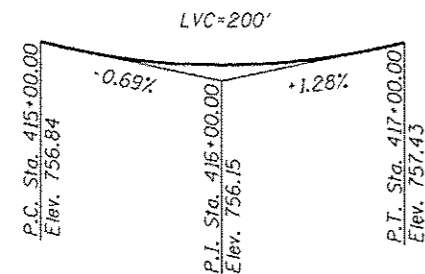
Seal of the State of Illinois Professional Engineer
 Expires: 11/30/2014

STATION 416+04.00
 BUILT 2011 BY
 STATE OF ILLINOIS
 F.A.I. RT. 74 EB SEC. 48-[(25B)BR. BR-1]
 LOADING HL-93
 STR. NO. 048-0090

NAME PLATE
 See Std. 515001

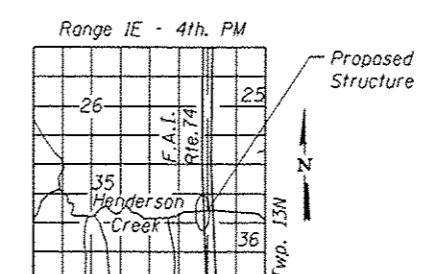
* Measured along local tangent
 ** For riprap limits the Contractor shall match limits of existing slope wall

Note:
 Lane and Shoulder dimensions are measured radially



CURVE DATA

$\Delta = 7^\circ 58' 36''$
 $D = 0^\circ 28' 00''$
 $T = 856.02'$
 $L = 1709.28'$
 $E = 29.81'$
 $R = 12277.66'$
 P.C. = Sta. 409+56.65
 P.T. = Sta. 426+65.93
 P.I. = Sta. 418+12.68



GENERAL PLAN AND ELEVATION
EB I-74 OVER HENDERSON CREEK
 F.A.I. RTE. 74 (EB) - SEC. 48-[(25B)BR. BR-1]

KNOX COUNTY
 STATION 416+04.00
 STRUCTURE NO. 048-0090

FILE NAME: ...0480090-0004-001-GPE.dgn	DESIGNED EV	REVISED -		STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	GENERAL PLAN AND ELEVATION STRUCTURE NO. 048-0090	F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
PLOT TIME: 4:27:42 PM	DRAWN JCP	REVISED -				74	48-[(25B)BR. BR-1]	KNOX	131	22
PLOT DATE: 9/29/2014	CHECKED PC	REVISED -				CONTRACT NO. 68885				
DATE: 05 03 2013	DATE: 05 03 2013	REVISED -				ILLINOIS FED. AID PROJECT				

GENERAL NOTES

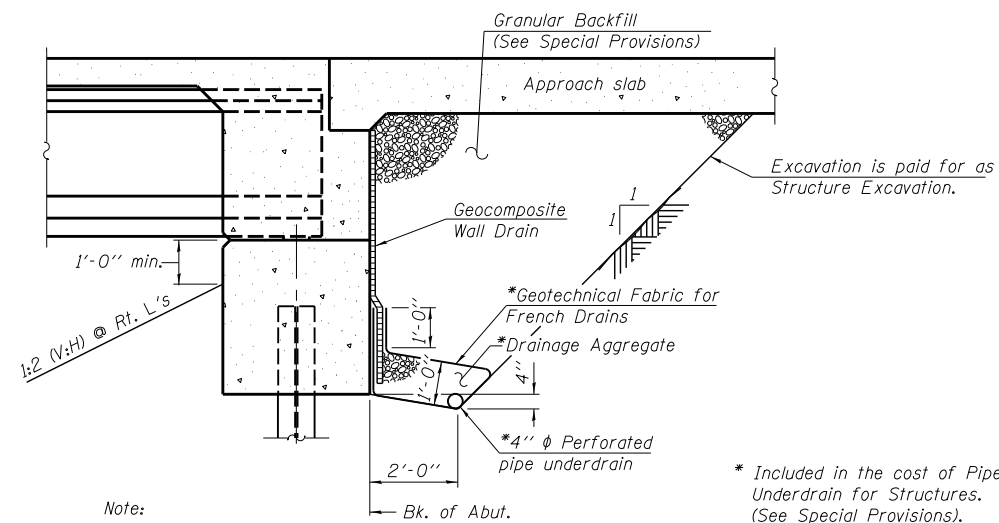
1. Reinforcement bars designated (E) shall be epoxy coated.
2. Layout of slope protection system may be varied to suit ground conditions in the field as directed by the Engineer.
3. Slipforming of the parapets is not allowed.

INDEX OF SHEETS

- S1 General Plan and Elevation
- S2 General Data
- S3 Substructure Layout
- S4 Stage Construction Details and Temp. Soil Retention System
- S5 Temporary Concrete Barrier for Stage Construction
- S6 Top of Deck
- S7 Top of Deck Elevations I
- S8 Top of Deck Elevations II
- S9 Top of Deck Elevations III
- S10 Top of North Approach Slab Elevations
- S11 Top of South Approach Slab Elevations
- S12 Superstructure Plan and Cross Section
- S13 Superstructure Details
- S14 Diaphragm Details
- S15 Bridge Approach Slab
- S16 Bridge Approach Slab Details
- S17 Framing Plan
- S18 72" PPC Bulb T-Beam
- S19 72" PPC Bulb T-Beam Details
- S20 Abutment Plan and Elevation
- S21 Abutment Details
- S22 HP Pile Details
- S23 Drainage Scupper DS-II
- S24 Bar Splicer Assembly and Mechanical Splicer Details
- S25 Boring Logs

TOTAL BILL OF MATERIAL

ITEMS	UNITS	SUPERSTRUCTURE	SUBSTRUCTURE	TOTAL
Stone RipRap, Class A4	SQ YD		1,673	1,673
Filter Fabric	SQ YD		1,673	1,673
Removal of Existing Structures	EACH		1	1
Structure Excavation	CU YD		804	804
Concrete Structures	CU YD		84.2	84.2
Concrete Superstructure	CU YD	352.2		352.2
Bridge Deck Grooving	SQ YD	736		736
Protective Coat	SQ YD	897		897
Furnishing and Erecting Precast Prestressed Concrete Bulb T-Beams 72"	FOOT	629		629
Reinforcement Bars, Epoxy Coated	POUND	71,340	16,270	87,610
Bar Splicers	EACH	479	100	579
Furnishing Steel Piles HP12x84	FOOT		765	765
Driving Piles	FOOT		765	765
Test Pile Steel HP12x84	EACH		2	2
Name Plates	EACH	1		1
Geocomposite Wall Drain	SQ YD		132	132
Granular Backfill for Structures	CU YD		318.2	318.2
Drainage Scuppers, DS-II	EACH	8		8
Pipe Underdrains for Structures 4"	FOOT		202	202
Temporary Soil Retention System	SQ FT		188	188



Note:

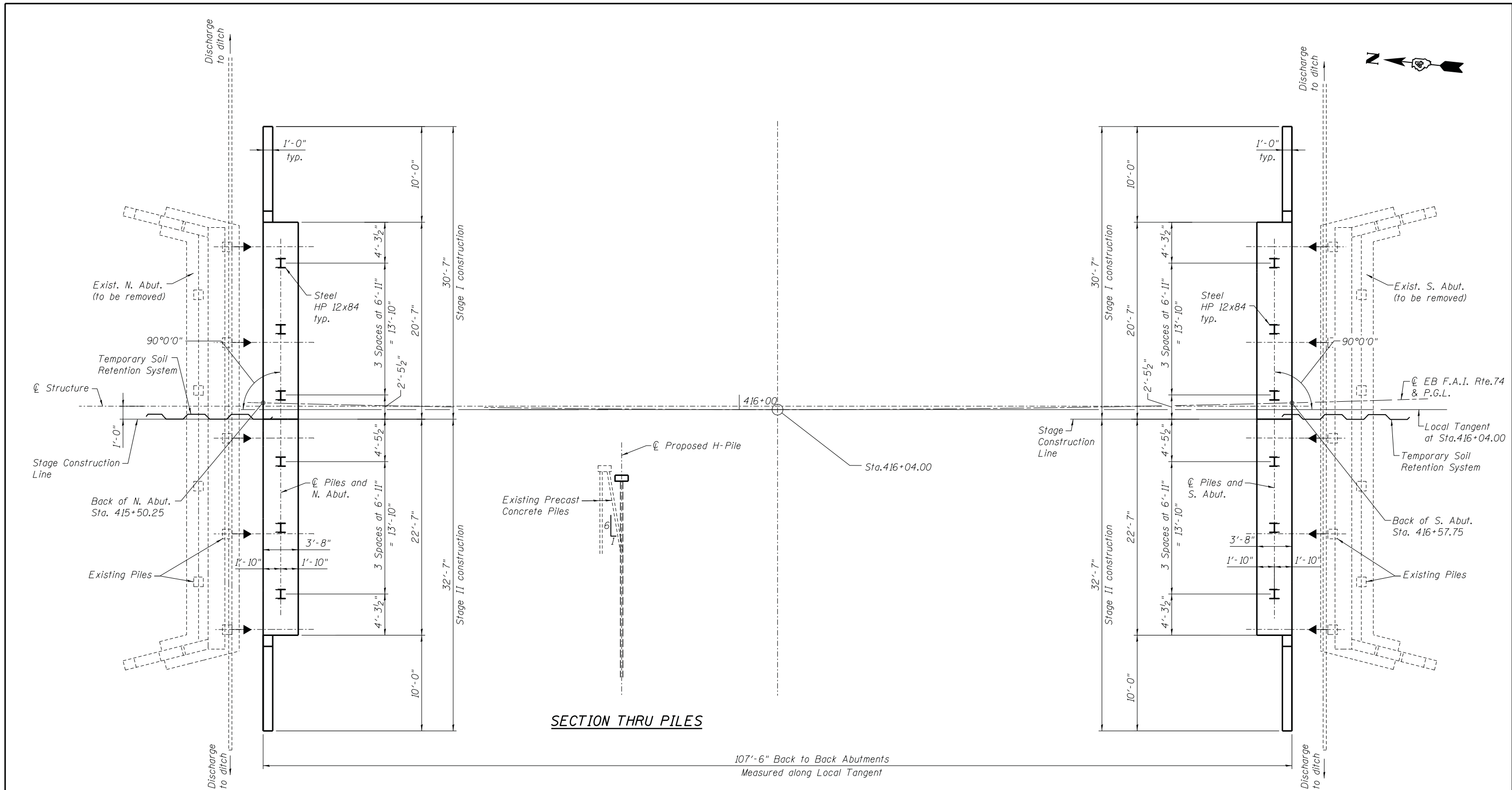
All drainage system components shall extend to 2'-0" from the end of each wingwall except an outlet pipe shall extend until intersecting with the side slopes. The pipes shall drain into concrete headwalls. (See Article 601.05 of the Standard Specifications and Highway Standard 601101).

* Included in the cost of Pipe Underdrain for Structures. (See Special Provisions).

SECTION THRU INTEGRAL ABUTMENT

(Horiz. dim. @ Rt. L's)

FILE NAME = ... \0480090-68084-002-Gen_data.dgn	DESIGNED EV	REVISED -	 800 WEST FULTON STREET CHICAGO, ILLINOIS 60611-1259 TEL 312 454 9100 FAX 312 559 1217 WEB www.sepstein.com	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION		GENERAL DATA STRUCTURE NO. 048-0090		F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
PLOT TIME = 4:31:08 PM	DRAWN JCP	REVISED -						74	48-[125B]BR, BR-1J	KNOX	131	23
PLOT DATE = 7/30/2014	CHECKED PC	REVISED -						CONTRACT NO. 68B85				
	DATE 05 03 2013	REVISED -						SHEET NO. S2 OF S25 SHEETS				
								ILLINOIS FED. AID PROJECT				



SECTION THRU PILES

107'-6" Back to Back Abutments
Measured along Local Tangent

FOUNDATION PLAN

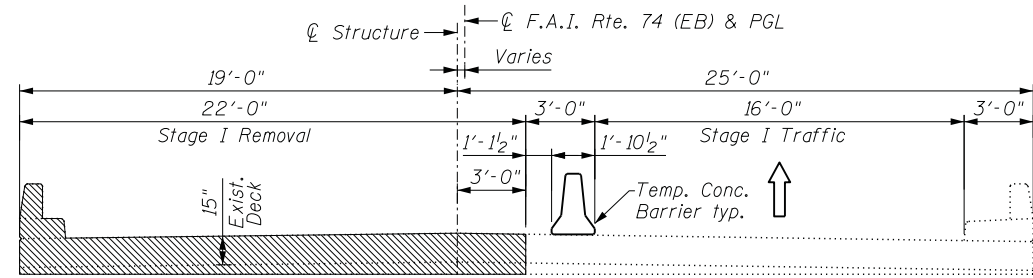
EXISTING PILE DATA

Type - Concrete Piles
Capacity - 25 Ton Min.
Est. Length - 35' Feet
No. Req'd - 22 (1 test pile
at N. Abut. included)

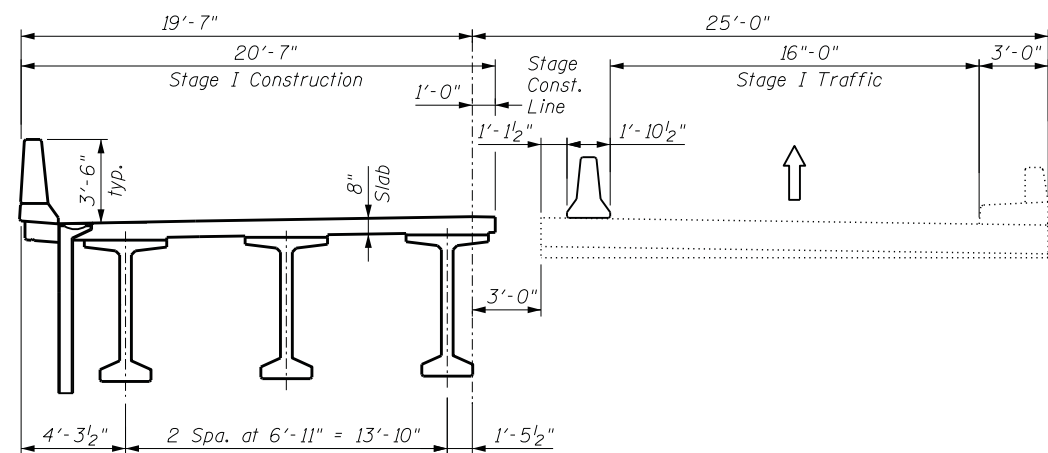
NOTES

1. For Temporary Soil Retention system details see sheet S4 of S25.
2. For Abutment details see sheet S20 of S25.
3. For Steel HP Piles details see sheet S22 of S25.
4. Drive one test pile at each abutment.

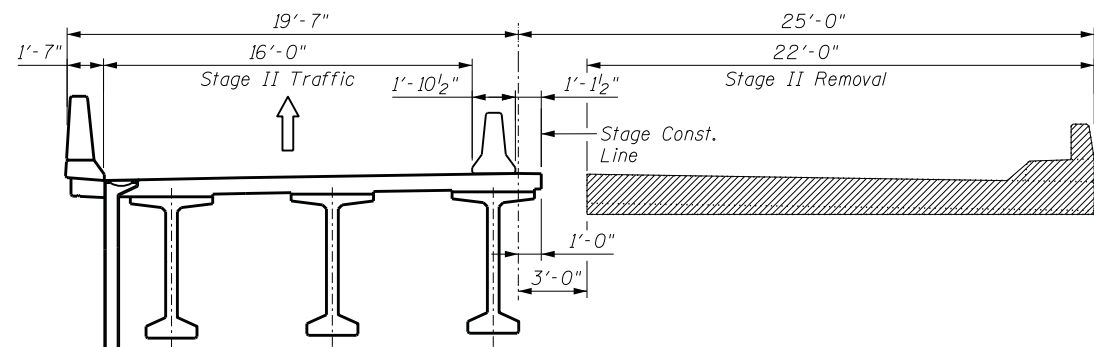
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PLOT TIME = 4:31:10 PM	CHECKED PC	REVISED -				CONTRACT NO. 68B85							
PLOT DATE = 7/30/2014	DATE 05 03 2013	REVISED -				ILLINOIS FED. AID PROJECT							



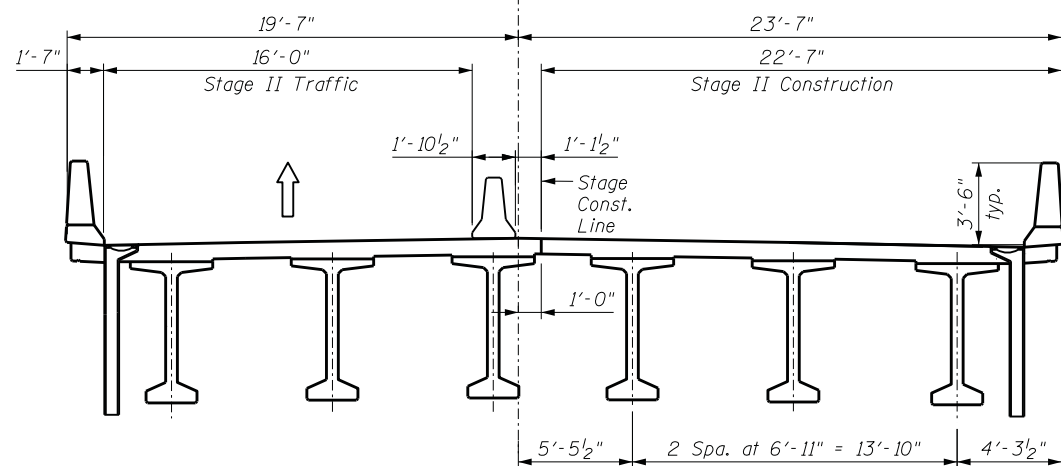
STAGE I REMOVAL



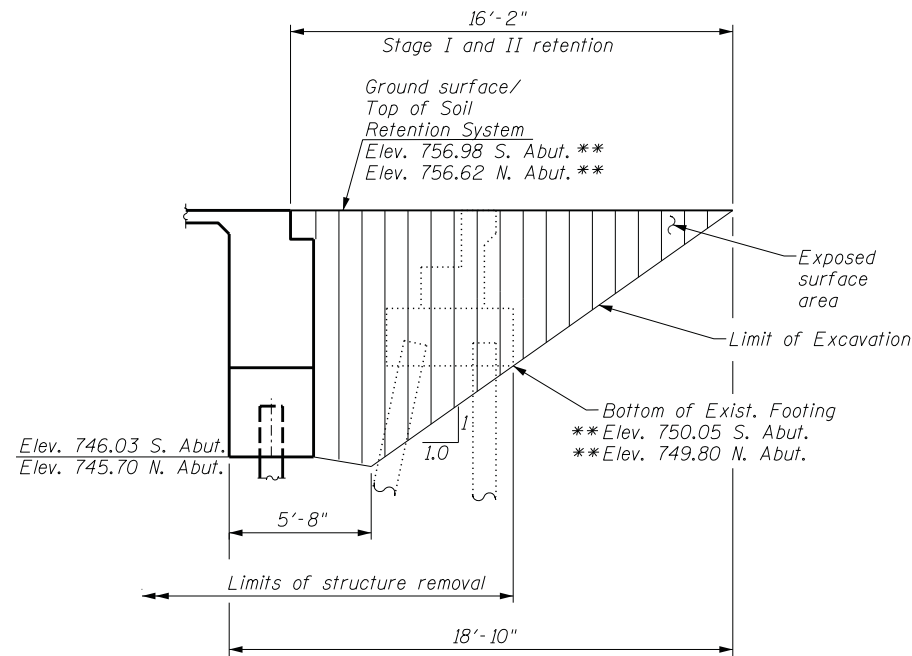
STAGE I CONSTRUCTION



STAGE II REMOVAL



STAGE II CONSTRUCTION



TEMPORARY SOIL RETENTION SYSTEM

**To be verified in field by the Contractor

BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Temporary Soil Retention System	Sq. Ft.	188

NOTES

- All stage cross sections are looking South.
- See sheet S5 of S25 for Temporary Concrete Barrier.
- Backfill shall be placed behind the abutment after the superstructure has been poured and false work removed. See Article 502.10 of the Standard Specifications.
- A cantilevered sheet piling design does not appear feasible and additional members or other retention systems may be necessary. The Contractor shall submit a temporary soil retention system design including plan details and calculations for review and acceptance by the Engineer.
- Hatched area indicates Removal of Existing Structures.

FILE NAME =	DESIGNED <i>EV</i>	REVISED -
...\\0480090-68084-004-Stg.const.dgn	DRAWN <i>JCP</i>	REVISED -
PLOT TIME = 4:31:11 PM	CHECKED <i>PC</i>	REVISED -
PLOT DATE = 7/30/2014	DATE <i>05 03 2013</i>	REVISED -

SEPSTEIN
 800 WEST FULTON STREET
 CHICAGO, ILLINOIS 60611-1259
 TEL 312 454 9100
 FAX 312 555 1217
 WEB www.sepstein.com

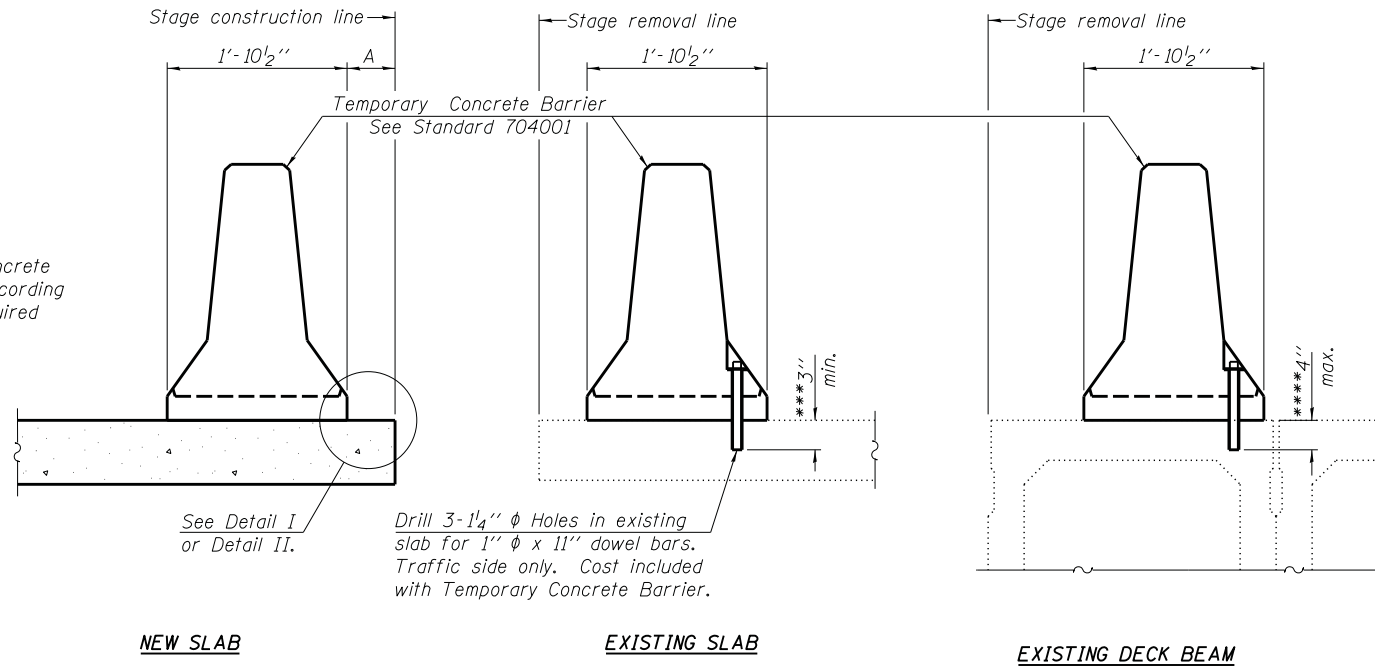
**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**STAGE CONSTRUCTION DETAILS AND TEMP. SOIL RETENTION SYSTEM
 STRUCTURE NO. 048-0090**

SHEET NO. S4 OF S25 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
74	48-[125B]BR, BR-1J	KNOX	131	25
CONTRACT NO. 68B85				
ILLINOIS FED. AID PROJECT				

When "A" is 3'-6" or less, the temporary concrete barrier shall be anchored to the new slab according to Detail I or Detail II. No anchorage is required when "A" is greater than 3'-6".



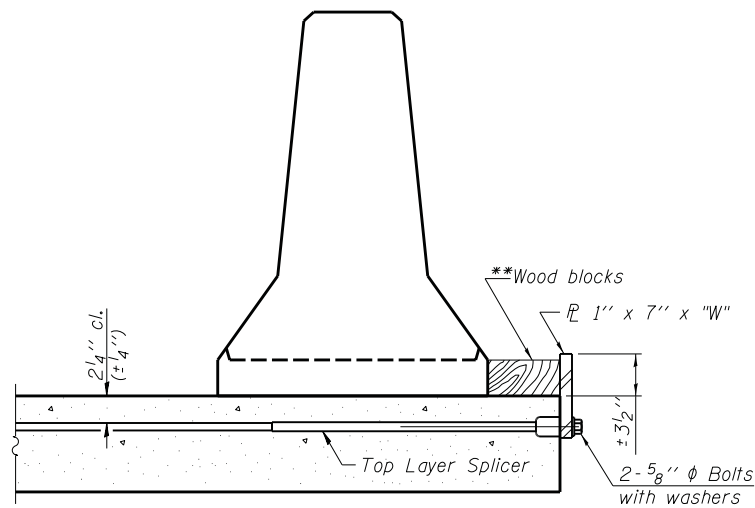
SECTIONS THRU SLAB OR DECK BEAM

NOTES

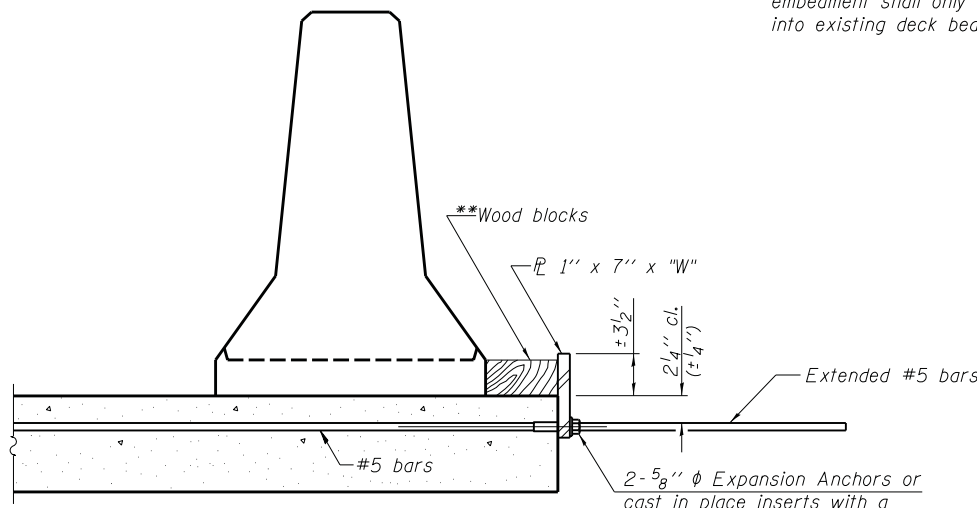
1. Detail I - With Bar Splicer or Couplers: Connect one (1) 1" x 7" x "W" steel PL to the top layer of couplers with 2-5/8" φ bolts screwed to coupler at approximate C of each barrier panel.
2. Detail II - With Extended Reinforcement Bars: Connect one (1) 1" x 7" x "W" steel PL to the concrete slab or concrete wearing surface with 2-5/8" φ Expansion Anchors or cast in place inserts spaced between the top layer of reinforcement at approximate C of each barrier panel.
3. Cost of anchorage is included with Temporary Concrete Barrier.
4. The 1" x 7" x "W" plate shall not be removed until stage II construction forms and all reinforcement bars are in place and the concrete is ready to be placed.

*** Dimension shown is minimum required embedment into concrete. If hot-mix asphalt wearing surface is present, minimum embedment shall be in addition to wearing surface depth.

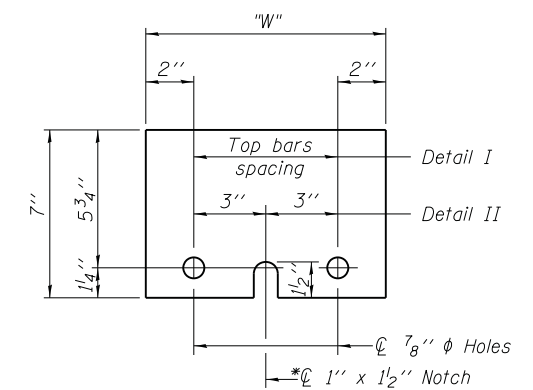
**** If existing deck beam is to remain in place after stage construction, embedment shall only be into wearing surface and not into existing deck beam concrete.



DETAIL I



DETAIL II



STEEL RETAINER PL 1" x 7" x "W"

* Required only with Detail II

** Wood blocks may be omitted when required to provide minimum stage traffic lane width. When the wood blocks are omitted, the concrete barrier shall be in direct contact with the steel retainer plate.

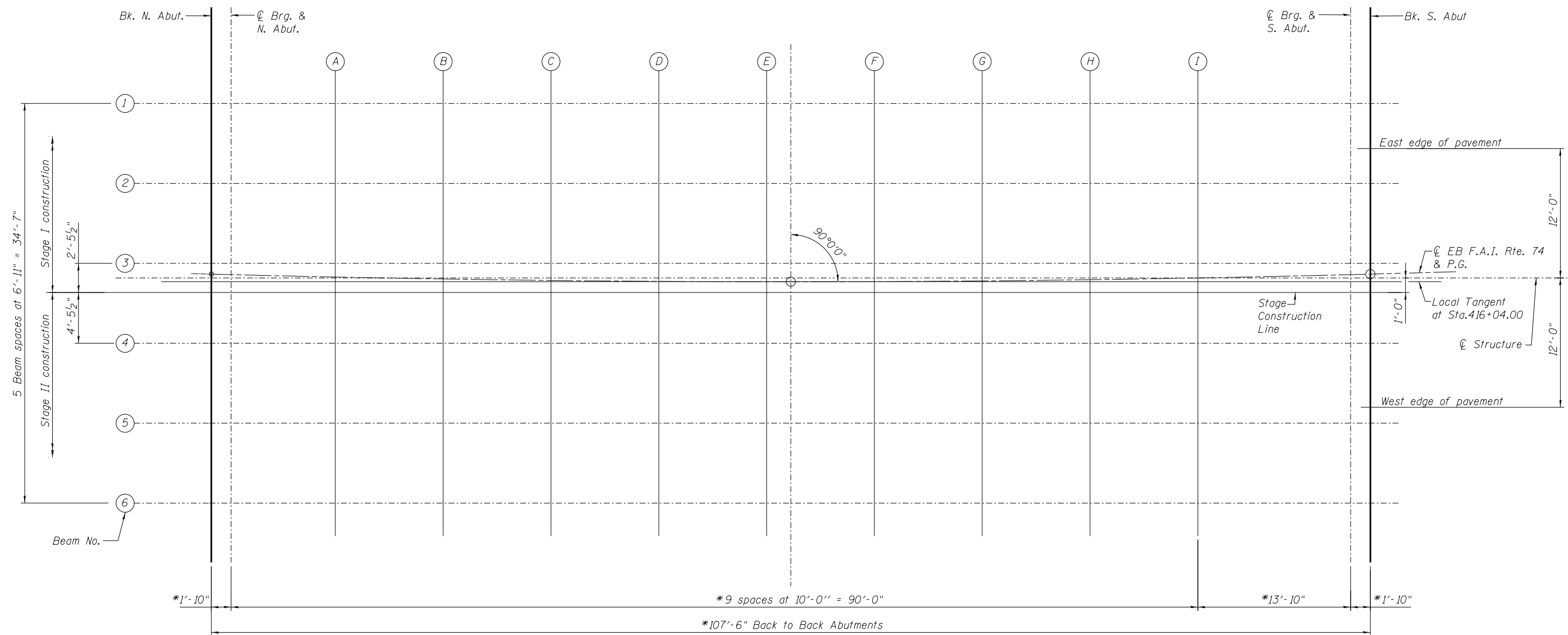
"W" = Top bars spacing + 4"

R-27

7-1-10

FILE NAME = ...\\0480090-68084-005-Temp.conc.barrier.dgn	DESIGNED EV	REVISED -		STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	TEMPORARY CONCRETE BARRIER FOR STAGE CONSTRUCTION STRUCTURE NO. 048-0090	F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
PLOT TIME = 4:31:12 PM	DRAWN JCP	REVISED -				74	48-(25B)BR, BR-1J	KNOX	131	26
PLOT DATE = 7/30/2014	CHECKED PC	REVISED -				CONTRACT NO. 68B85				
	DATE 05 03 2013	REVISED -				ILLINOIS FED. AID PROJECT				

SHEET NO. S5 OF S25 SHEETS




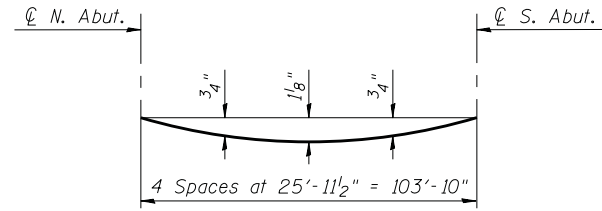
PLAN

* Measured along Local Tangent

NOTES

1. For Top of Slab elevations at South Approach Slab see sheet S11 of S25.
2. For Top of Slab elevations at North Approach Slab see sheet S10 of S25.

FILE NAME = ...\\0480090-68084-006-T05.dgn	DESIGNED <i>EV</i> DRAWN <i>JCP</i>	REVISED - REVISED - REVISED - REVISED -	 <small>800 WEST FULTON STREET CHICAGO, ILLINOIS 60611-1259</small>	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	TOP OF DECK STRUCTURE NO. 048-0090 SHEET NO. S6 OF S25 SHEETS	F.A.I. RTE. 74	SECTION 48-[25B]BR, BR-1J	COUNTY KNOX	TOTAL SHEETS 131	SHEET NO. 27
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PLOT DATE = 7/30/2014	ILLINOIS FED. AID PROJECT									

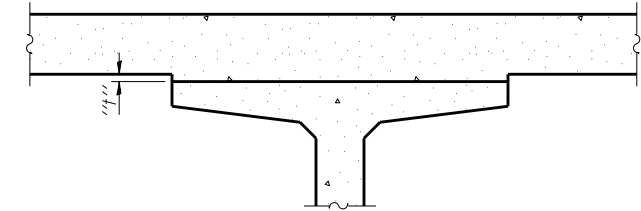


DEAD LOAD DEFLECTION DIAGRAM

(Includes weight of concrete and barrier, excluding beams).

Note:

The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections as shown below and on sheets S8 and S9.



To determine "t": After all precast prestressed beams have been erected, elevations of the top flanges of the beams shall be taken at intervals shown on sheet S6. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflections" shown below and on sheets S8 and S9, minus slab thickness, equals the fillet heights "t" above top flanges of beams.

FILLET HEIGHTS

BEAM 1

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
BK N. ABUT.	415+50.25	-15.23	756.36	756.36
CL N. ABUT.	415+52.08	-15.24	756.36	756.36
A	415+62.08	-15.28	756.35	756.37
B	415+72.08	-15.30	756.34	756.39
C	415+82.08	-15.33	756.35	756.42
D	415+92.08	-15.34	756.37	756.44
E	416+02.08	-15.35	756.39	756.48
F	416+12.08	-15.34	756.43	756.51
G	416+22.08	-15.33	756.48	756.55
H	416+32.08	-15.31	756.53	756.59
I	416+42.08	-15.29	756.60	756.63
CL S. ABUT	416+55.92	-15.24	756.71	756.71
BK S. ABUT	416+57.75	-15.23	756.72	756.72

EAST EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
BK N. ABUT.	415+50.25	-11.94	756.43	756.43
CL N. ABUT.	415+52.08	-11.94	756.43	756.43
A	415+62.08	-11.98	756.42	756.44
B	415+72.08	-12.01	756.41	756.46
C	415+82.08	-12.04	756.42	756.49
D	415+92.08	-12.05	756.49	756.56
E	416+02.08	-12.05	756.83	756.92
F	416+12.08	-12.05	756.50	756.58
G	416+22.08	-12.04	756.55	756.62
H	416+32.08	-12.02	756.60	756.66
I	416+42.08	-12.00	756.67	756.70
CL S. ABUT	416+55.92	-11.94	756.78	756.78
BK S. ABUT	416+57.75	-11.94	756.79	756.79

BEAM 2

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
BK N. ABUT.	415+50.25	-8.31	756.49	756.49
CL N. ABUT.	415+52.08	-8.32	756.48	756.48
A	415+62.08	-8.36	756.47	756.50
B	415+72.08	-8.39	756.47	756.52
C	415+82.08	-8.41	756.48	756.54
D	415+92.08	-8.42	756.49	756.57
E	416+02.08	-8.43	756.52	756.61
F	416+12.08	-8.43	756.56	756.64
G	416+22.08	-8.42	756.60	756.67
H	416+32.08	-8.40	756.66	756.72
I	416+42.08	-8.37	756.72	756.76
CL S. ABUT	416+55.92	-8.32	756.83	756.83
BK S. ABUT	416+57.75	-8.31	756.85	756.85

BEAM 3

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
BK N. ABUT.	415+50.25	-1.40	756.60	756.60
CL N. ABUT.	415+52.08	-1.40	756.59	756.59
A	415+62.08	-1.44	756.58	756.60
B	415+72.08	-1.47	756.58	756.62
C	415+82.08	-1.49	756.58	756.65
D	415+92.08	-1.51	756.60	756.68
E	416+02.08	-1.51	756.63	756.71
F	416+12.08	-1.51	756.66	756.74
G	416+22.08	-1.50	756.71	756.78
H	416+32.08	-1.48	756.77	756.82
I	416+42.08	-1.45	756.83	756.87
CL S. ABUT	416+55.92	-1.40	756.94	756.94
BK S. ABUT	416+57.75	-1.40	756.96	756.96

☉ ROADWAY, PROFILE GRADE LINE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
BK N. ABUT.	415+50.25	0.00	756.62	756.62
CL N. ABUT.	415+52.08	0.00	756.61	756.61
A	415+62.08	0.00	756.60	756.63
B	415+72.08	0.00	756.60	756.65
C	415+82.08	0.00	756.61	756.67
D	415+92.08	0.00	756.62	756.70
E	416+02.08	0.00	756.65	756.74
F	416+12.08	0.00	756.69	756.77
G	416+22.08	0.00	756.73	756.80
H	416+32.08	0.00	756.79	756.85
I	416+42.08	0.00	756.85	756.89
CL S. ABUT	416+55.92	0.00	756.96	756.96
BK S. ABUT	416+57.75	0.00	756.98	756.98

STAGE CONSTRUCTION LINE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
BK N. ABUT.	415+50.25	1.06	756.60	756.60
CL N. ABUT.	415+52.08	1.05	756.60	756.60
A	415+62.08	1.02	756.58	756.61
B	415+72.08	0.99	756.58	756.63
C	415+82.08	0.96	756.59	756.66
D	415+92.08	0.95	756.61	756.68
E	416+02.08	0.95	756.63	756.72
F	416+12.08	0.95	756.67	756.75
G	416+22.08	0.96	756.72	756.79
H	416+32.08	0.98	756.77	756.83
I	416+42.08	1.00	756.84	756.87
CL S. ABUT	416+55.92	1.05	756.95	756.95
BK S. ABUT	416+57.75	1.06	756.96	756.96

BEAM 4

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
BK N. ABUT.	415+50.25	5.52	756.53	756.53
CL N. ABUT.	415+52.08	5.51	756.53	756.53
A	415+62.08	5.47	756.52	756.54
B	415+72.08	5.44	756.51	756.56
C	415+82.08	5.42	756.52	756.59
D	415+92.08	5.41	756.54	756.61
E	416+02.08	5.40	756.56	756.65
F	416+12.08	5.41	756.60	756.68
G	416+22.08	5.42	756.65	756.72
H	416+32.08	5.44	756.70	756.76
I	416+42.08	5.46	756.77	756.80
CL S. ABUT	416+55.92	5.51	756.88	756.88
BK S. ABUT	416+57.75	5.52	756.89	756.89

WEST EDGE OF PAVEMENT


Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
BK N. ABUT.	415+50.25	12.06	756.43	756.43
CL N. ABUT.	415+52.08	12.05	756.43	756.43
A	415+62.08	12.02	756.41	756.44
B	415+72.08	11.99	756.41	756.46
C	415+82.08	11.96	756.42	756.48
D	415+92.08	11.95	756.43	756.51
E	416+02.08	11.95	756.46	756.55
F	416+12.08	11.95	756.50	756.58
G	416+22.08	11.96	756.54	756.61
H	416+32.08	11.98	756.60	756.66
I	416+42.08	12.00	756.67	756.70
CL S. ABUT	416+55.92	12.05	756.77	756.77
BK S. ABUT	416+57.75	12.06	756.79	756.79

BEAM 5

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
BK N. ABUT.	415+50.25	12.44	756.42	756.42
CL N. ABUT.	415+52.08	12.43	756.42	756.42
A	415+62.08	12.39	756.40	756.43
B	415+72.08	12.36	756.40	756.45
C	415+82.08	12.34	756.41	756.48
D	415+92.08	12.33	756.43	756.50
E	416+02.08	12.32	756.45	756.54
F	416+12.08	12.32	756.49	756.57
G	416+22.08	12.33	756.53	756.61
H	416+32.08	12.35	756.59	756.65
I	416+42.08	12.38	756.66	756.69
CL S. ABUT	416+55.92	12.43	756.76	756.76
BK S. ABUT	416+57.75	12.44	756.78	756.78

BEAM 6

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
BK N. ABUT.	415+50.25	19.35	756.28	756.28
CL N. ABUT.	415+52.08	19.34	756.27	756.27
A	415+62.08	19.31	756.26	756.28
B	415+72.08	19.28	756.26	756.31
C	415+82.08	19.25	756.26	756.33
D	415+92.08	19.24	756.28	756.36
E	416+02.08	19.24	756.31	756.39
F	416+12.08	19.24	756.34	756.43
G	416+22.08	19.25	756.39	756.46
H	416+32.08	19.27	756.45	756.50
I	416+42.08	19.29	756.51	756.55
CL S. ABUT	416+55.92	19.34	756.62	756.62
BK S. ABUT	416+57.75	19.35	756.64	756.64

FILE NAME = ...\\0480090-68084-009-TOS.Elevs3.dgn	DESIGNED <i>EV</i> DRAWN <i>JCP</i>	REVISED - REVISED -	 800 WEST FULTON STREET CHICAGO, ILLINOIS 60611-2599 TEL 312 454 9100 FAX 312 555 1217 WEB www.sepstein.com	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	TOP OF DECK ELEVATIONS III STRUCTURE NO. 048-0090	F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
PLOT TIME = 4:31:16 PM	CHECKED <i>PC</i>	REVISED -				74	48-[125B]BR, BR-1J	KNOX	131	30
PLOT DATE = 7/30/2014	DATE <i>05 03 2013</i>	REVISED -				CONTRACT NO. 68B85				
SHEET NO. S9 OF S25 SHEETS						ILLINOIS FED. AID PROJECT				

EAST EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations
N. End of N. Appr. Slab	415+21.25	-17.78	756.41
A1	415+31.25	-17.84	756.36
A2	415+41.25	-17.90	756.33
S. End of N. Appr. Slab	415+51.25	-17.95	756.30

EAST EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
N. End of N. Appr. Slab	415+21.25	-11.78	756.53
A1	415+31.25	-11.84	756.49
A2	415+41.25	-11.90	756.45
S. End of N. Appr. Slab	415+51.25	-11.95	756.43

☉ ROADWAY, PROFILE GRADE LINE

Location	Station	Offset	Theoretical Grade Elevations
N. End of N. Appr. Slab	415+21.25	0.00	756.72
A1	415+31.25	0.00	756.67
A2	415+41.25	0.00	756.64
S. End of N. Appr. Slab	415+51.25	0.00	756.62

STAGE CONSTRUCTION LINE

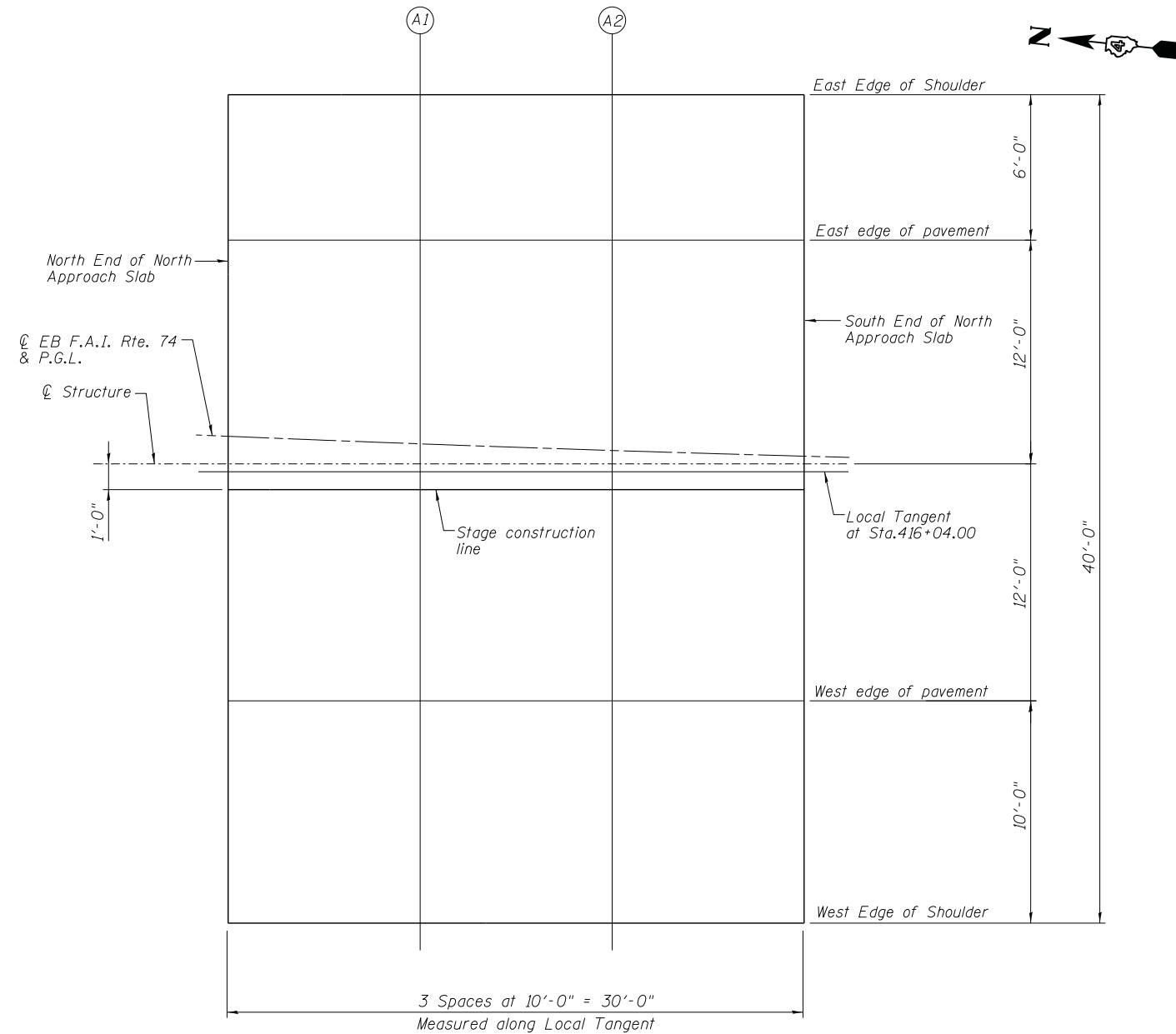
Location	Station	Offset	Theoretical Grade Elevations
N. End of N. Appr. Slab	415+21.25	1.22	756.70
A1	415+31.25	1.16	756.66
A2	415+41.25	1.10	756.63
S. End of N. Appr. Slab	415+51.25	1.05	756.60

WEST EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
N. End of N. Appr. Slab	415+21.25	12.22	756.53
A1	415+31.25	12.16	756.49
A2	415+41.25	12.10	756.46
S. End of N. Appr. Slab	415+51.25	12.05	756.43

WEST EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations
N. End of N. Appr. Slab	415+21.25	22.22	756.32
A1	415+31.25	22.16	756.28
A2	415+41.25	22.10	756.24
S. End of N. Appr. Slab	415+51.25	22.05	756.22



PLAN

EAST EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations
N. End of S. Appr. Slab	416+56.75	-17.95	756.66
A3	416+66.75	-17.90	756.75
A4	416+76.75	-17.84	756.85
S. End of S. Appr. Slab	416+86.75	-17.78	756.96

EAST EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
N. End of S. Appr. Slab	416+56.75	-11.95	756.78
A3	416+66.75	-11.90	756.87
A4	416+76.75	-11.84	756.97
S. End of S. Appr. Slab	416+86.75	-11.78	757.08

☉ ROADWAY, PROFILE GRADE LINE

Location	Station	Offset	Theoretical Grade Elevations
N. End of S. Appr. Slab	416+56.75	0.00	756.97
A3	416+66.75	0.00	757.06
A4	416+76.75	0.00	757.16
S. End of S. Appr. Slab	416+86.75	0.00	757.27

STAGE CONSTRUCTION LINE

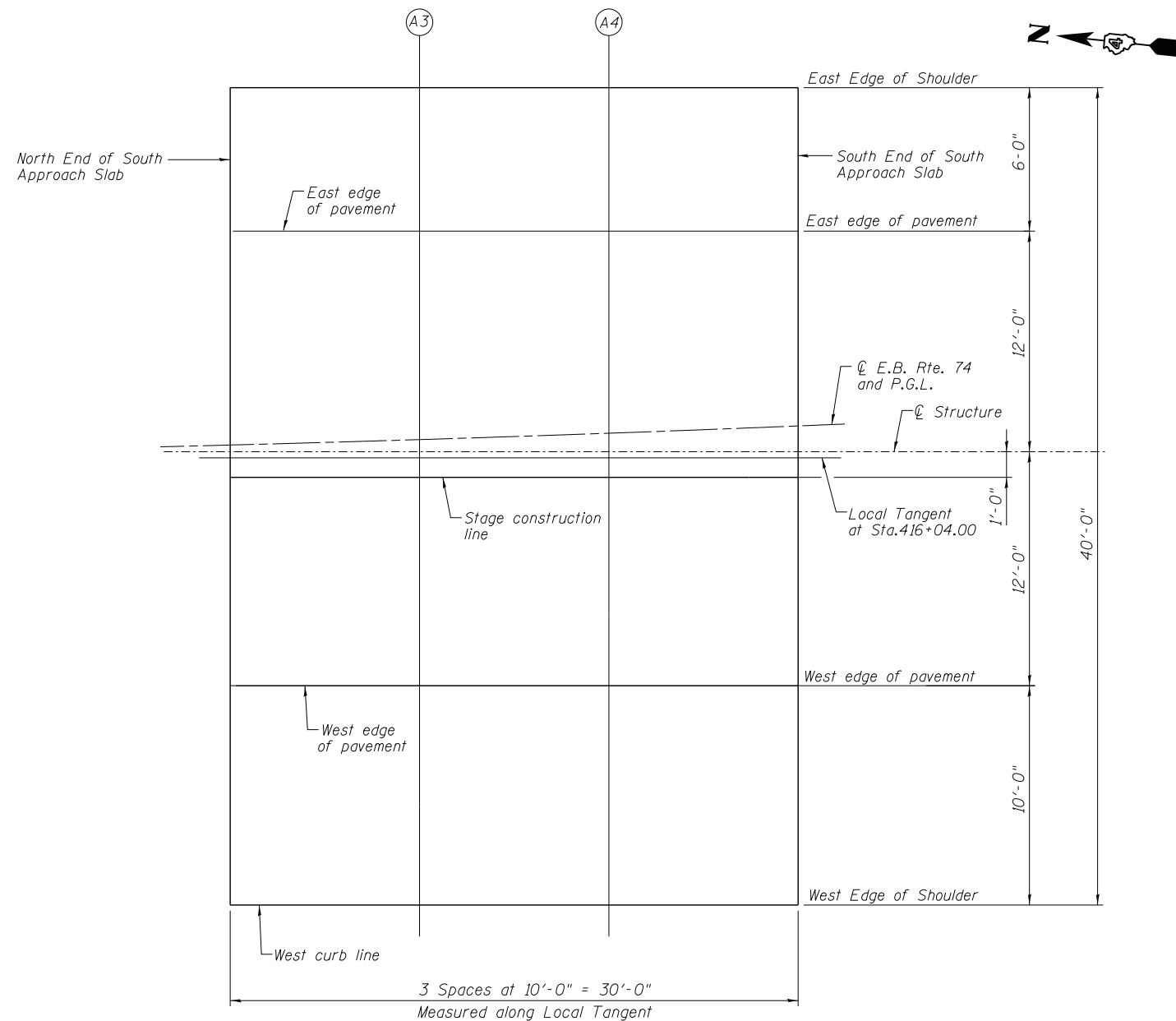
Location	Station	Offset	Theoretical Grade Elevations
N. End of S. Appr. Slab	416+56.75	1.05	756.95
A3	416+66.75	1.10	757.04
A4	416+76.75	1.16	757.14
S. End of S. Appr. Slab	416+86.75	1.22	757.25

WEST EDGE OF PAVEMENT

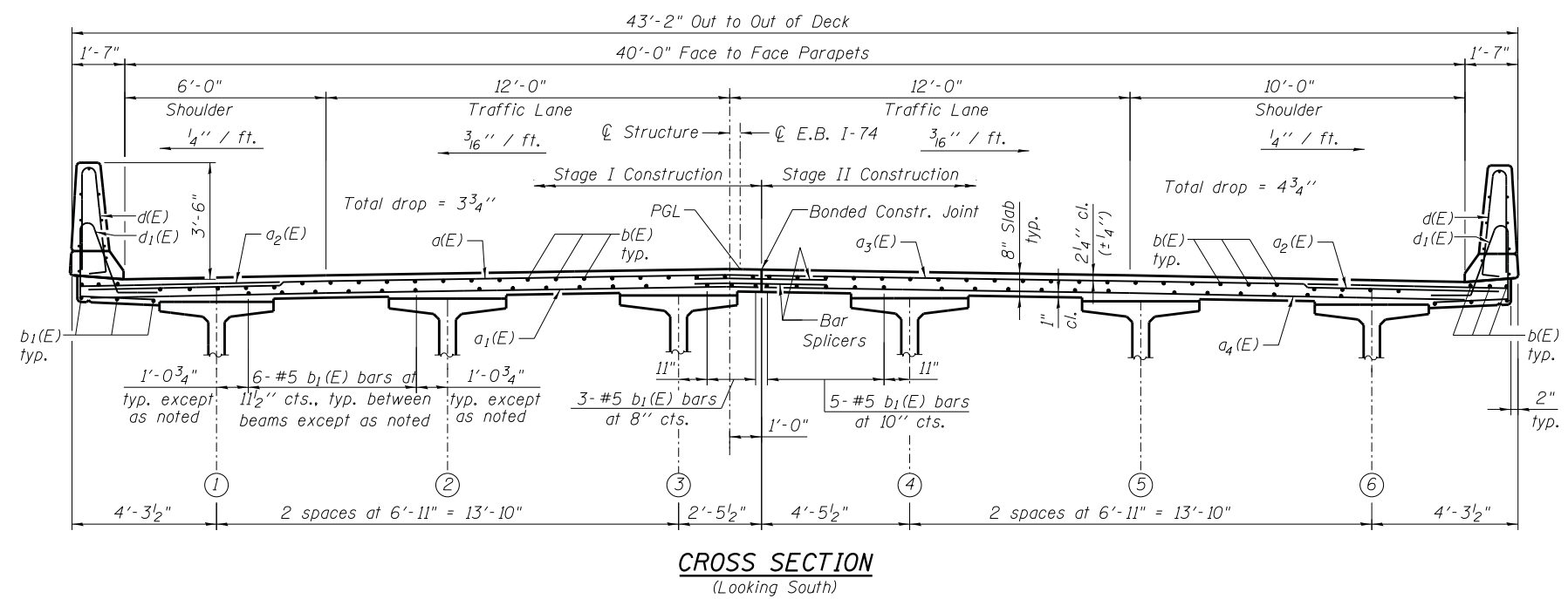
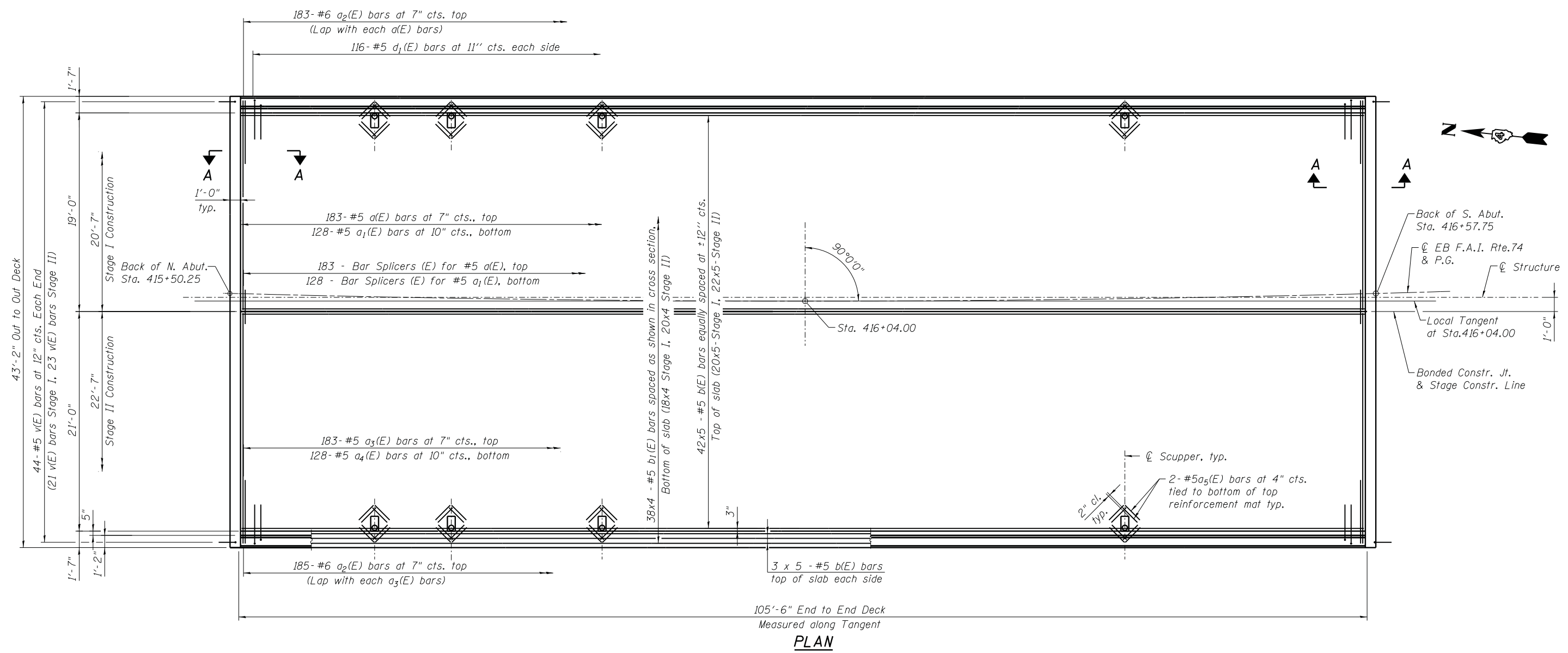
Location	Station	Offset	Theoretical Grade Elevations
N. End of S. Appr. Slab	416+56.75	12.05	756.78
A3	416+66.75	12.10	756.87
A4	416+76.75	12.16	756.97
S. End of S. Appr. Slab	416+86.75	12.22	757.08

WEST EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations
N. End of S. Appr. Slab	416+56.75	22.05	756.57
A3	416+66.75	22.10	756.66
A4	416+76.75	22.16	756.77
S. End of S. Appr. Slab	416+86.75	22.22	756.88



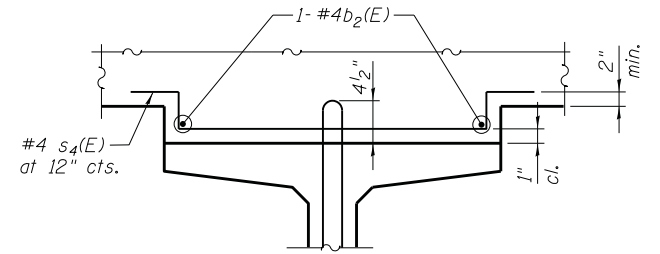
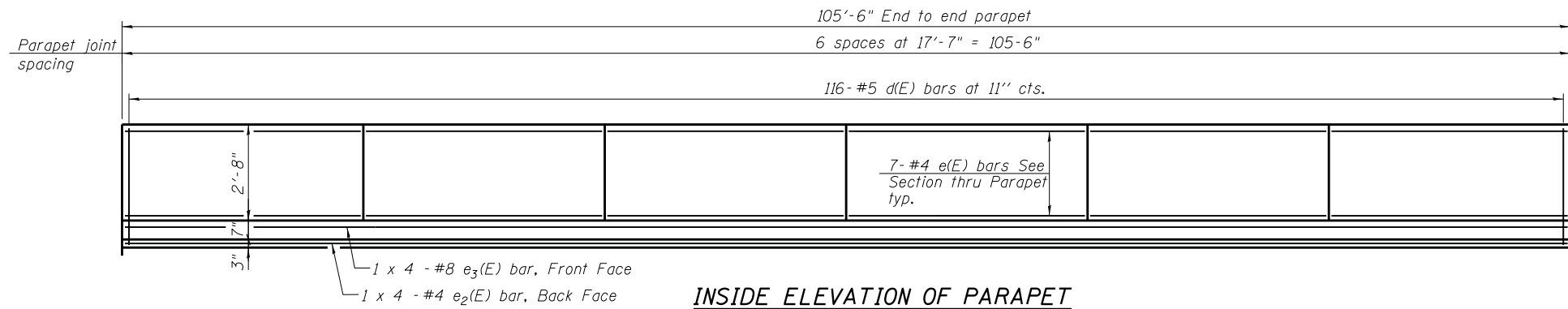
PLAN



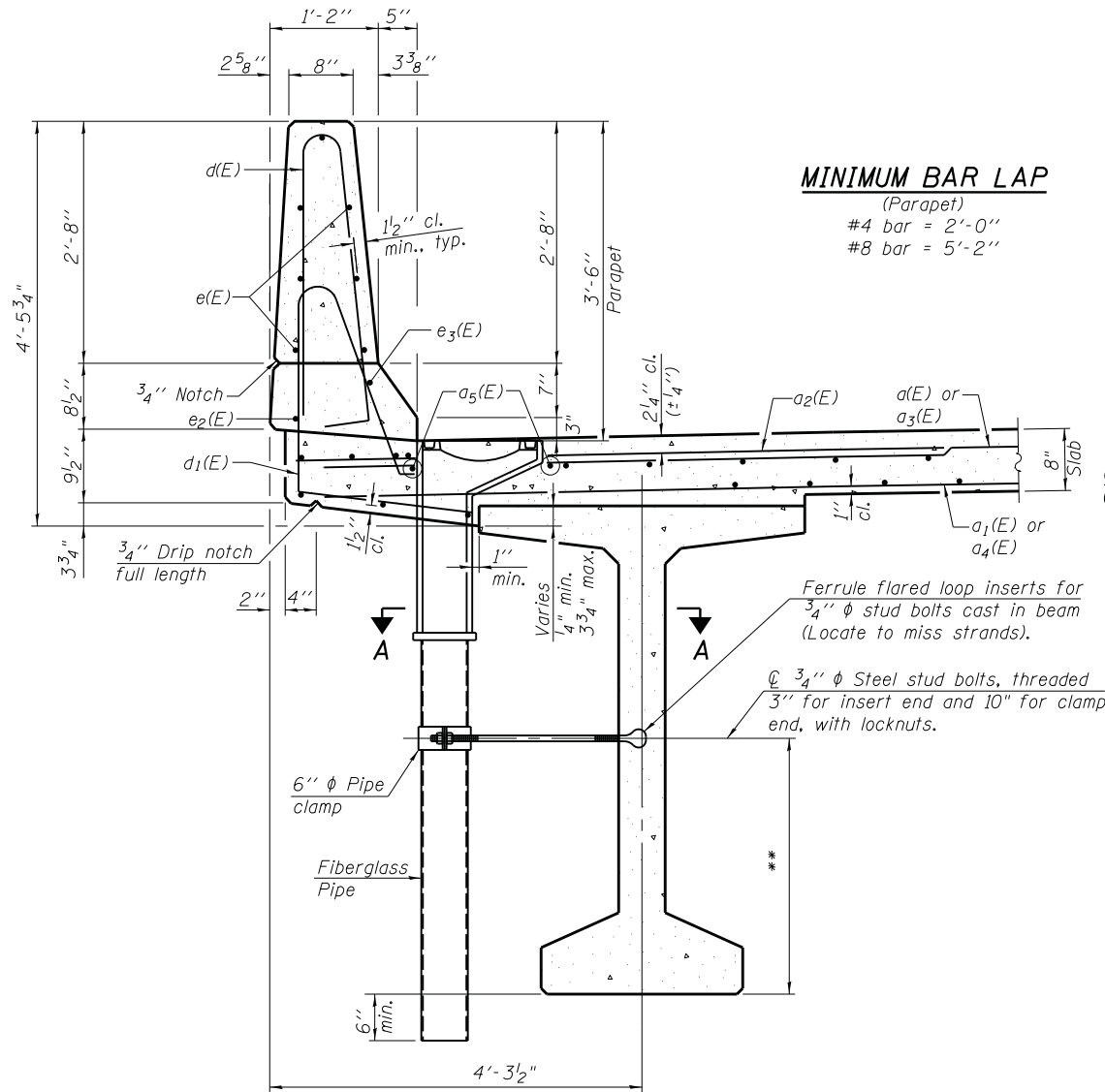
MINIMUM BAR LAP
#5 bar = 2'-7"

- NOTES**
1. For Parapet reinforcement, superstructure details and Bill of Material, see sheet S13 of S25.
 2. For Section A-A and diaphragm details see sheet S14 of S25.
 3. For Bar Splicer (E) details, see sheet S24 of S25.
 4. Bars indicated thus 20 x 3-#5 etc. indicates 20 lines of bars with 3 lengths per line.
 5. Cut longitudinal reinforcement to clear drainage scuppers.
 6. For drainage scupper locations see sheet S1.

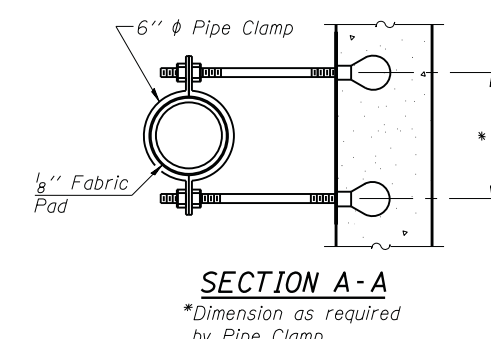
FILE NAME = ...\\0480090-68084-012-Super.dgn	DESIGNED <i>EV</i>	REVISED -	<p>800 WEST FULTON STREET CHICAGO, ILLINOIS 60611-1259</p> <p>TEL. 312.454.9100 FAX 312.559.1217 WEB www.sepstein.com</p>	<p align="center">STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION</p>	<p align="center">SUPERSTRUCTURE PLAN AND CROSS SECTION STRUCTURE NO. 048-0090</p>	F.A.I. RTE. 74	SECTION 48-[125B]BR, BR-1J	COUNTY KNOX	TOTAL SHEETS 131	SHEET NO. 33			
PLOT TIME = 4:31:20 PM	DRAWN <i>JCP</i>	REVISED -				SHEET NO. S12 OF S25 SHEETS			CONTRACT NO. 68B85				
PLOT DATE = 7/30/2014	CHECKED <i>PC</i>	REVISED -				ILLINOIS FED. AID PROJECT							
	DATE <i>05 03 2013</i>	REVISED -											



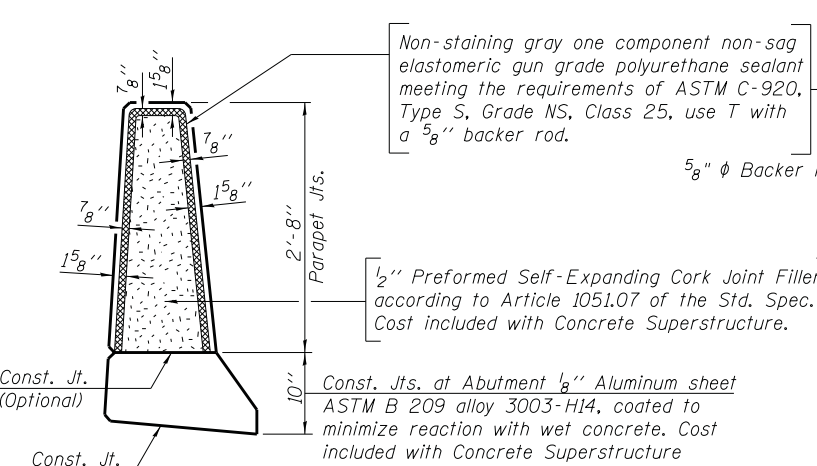
FILLET REINFORCEMENT DETAIL
Bars b₂(E) and s₄(E) should be placed in locations where the fillet exceeds 2" minimum.



SECTION THRU PARAPET
**For insert locations see sheet S18 of S25.

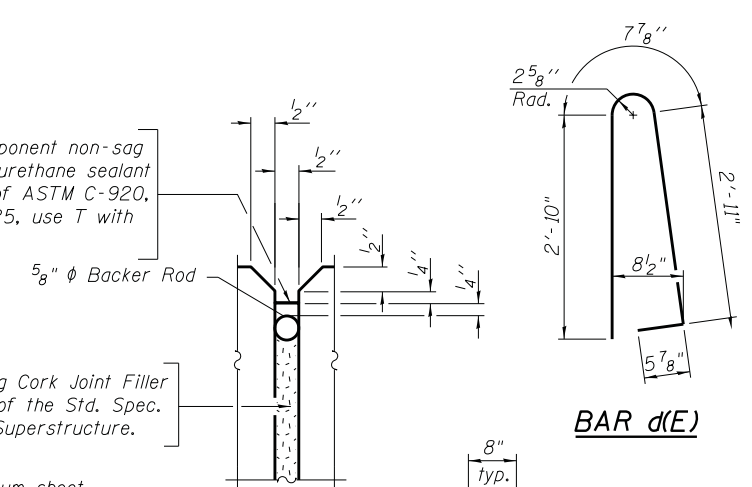


MINIMUM BAR LAP
(Parapet)
#4 bar = 2'-0"
#8 bar = 5'-2"

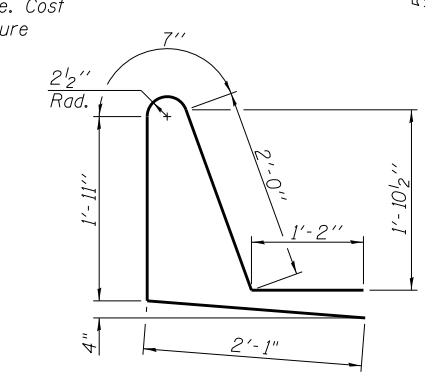


PARAPET JOINT DETAILS

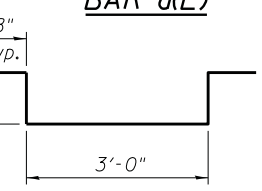
- Notes:
1. Fiberglass pipe shall conform to ASTM D2996, with short-time rupture strength hoop tensile stress of 30,000 p.s.i. minimum.
 2. The clamping device and inserts shall be galvanized according to AASHTO M 232.
 3. Cost of fiberglass pipe and clamping device and inserts included with Drainage Scupper, DS-11.



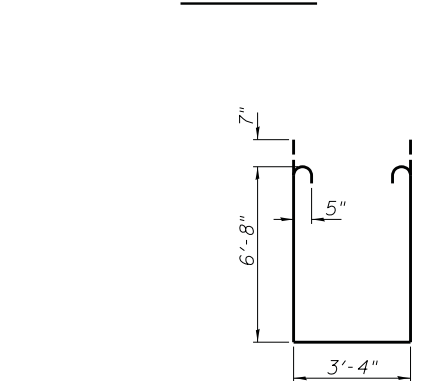
BAR d(E)



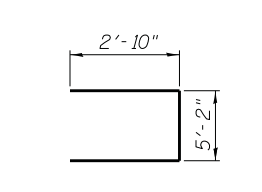
BAR d1(E)



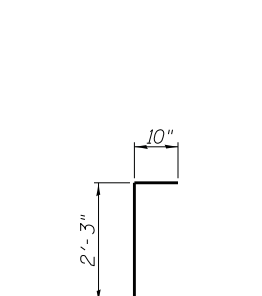
BAR s4(E)



BAR s1(E)



BAR s(E)

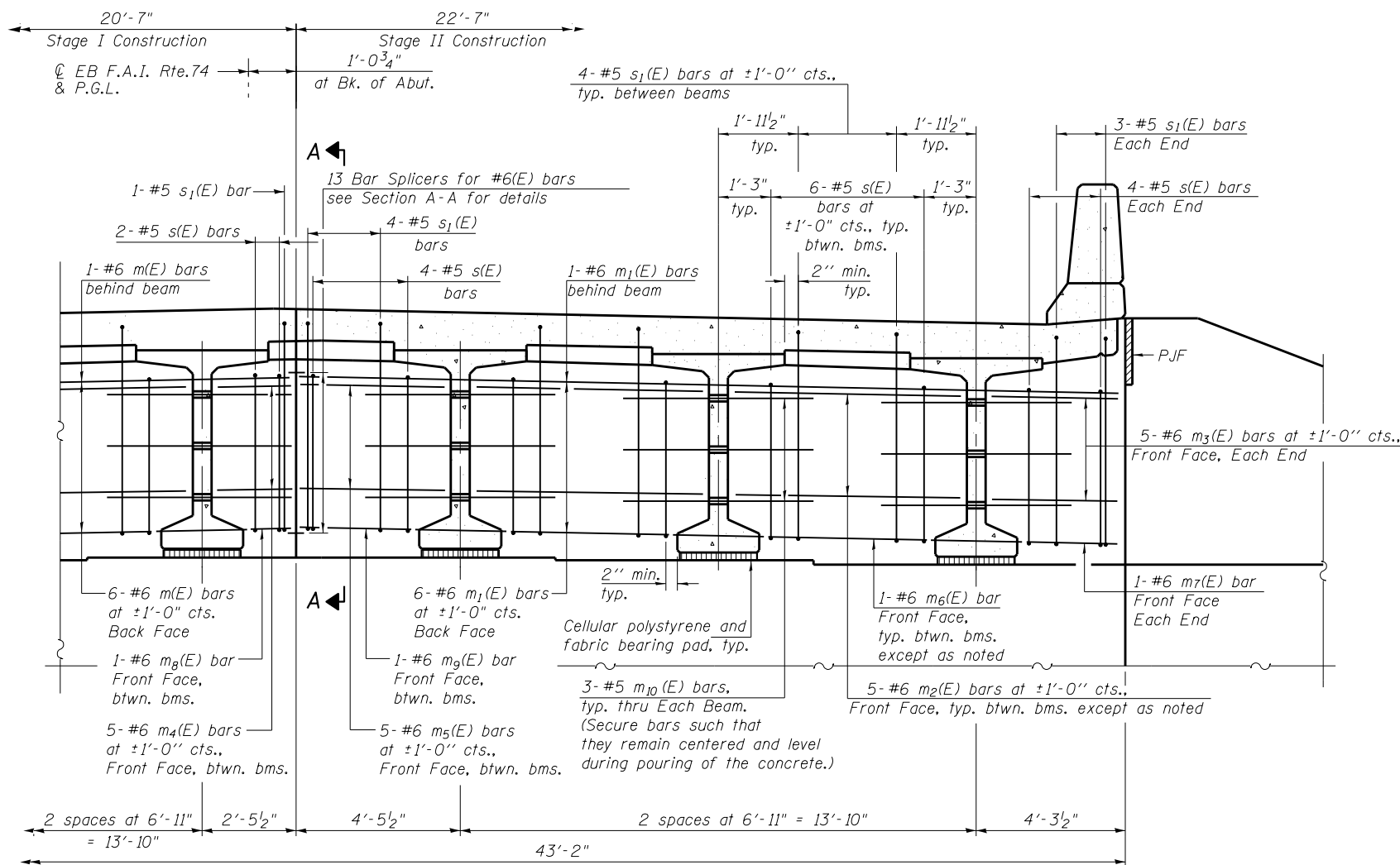


BAR v(E)

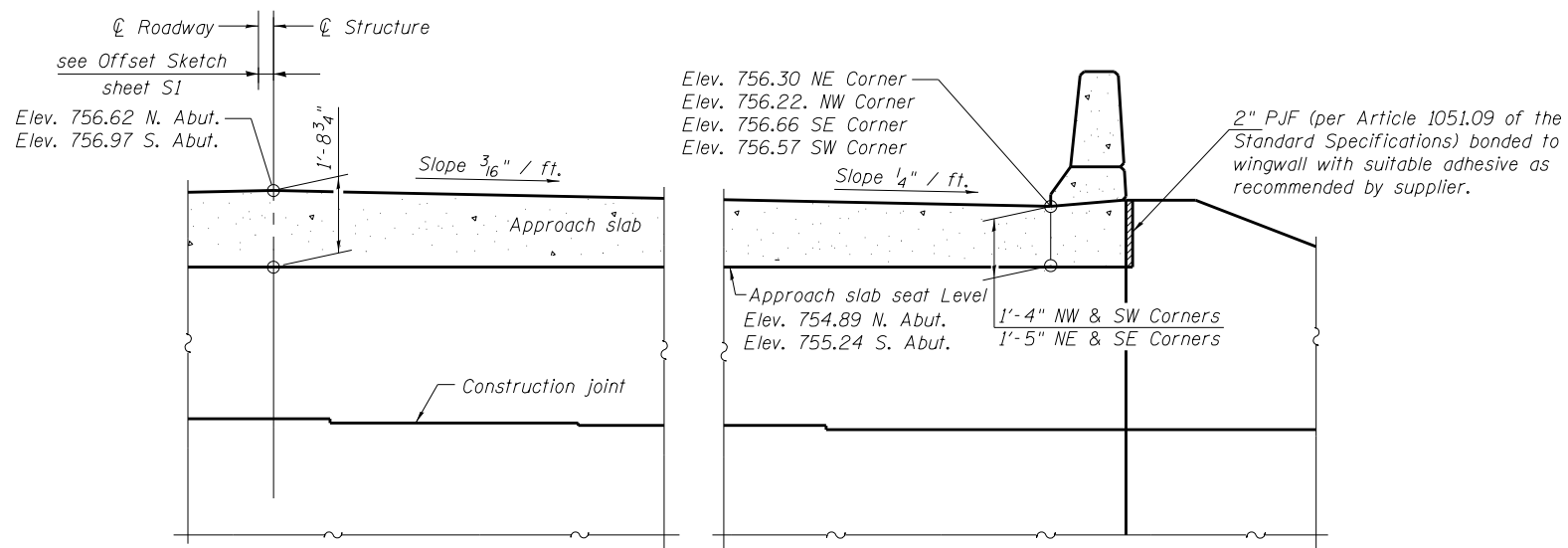
SUPERSTRUCTURE BILL OF MATERIAL

Bar	No.	Size	Length	Shape
d(E)	183	#5	20' - 1"	U
a1(E)	128	#5	20' - 2"	U
a2(E)	366	#6	6' - 6"	U
a3(E)	183	#5	22' - 2"	U
a4(E)	128	#5	22' - 1"	U
a5(E)	64	#5	1' - 6"	U
b(E)	240	#5	23' - 7"	U
b1(E)	152	#5	28' - 9"	U
b2(E)	24	#4	27' - 0"	U
d(E)	232	#5	6' - 11"	U
d1(E)	232	#5	7' - 9"	U
e(E)	84	#4	17' - 3"	U
e2(E)	8	#4	28' - 0"	U
e3(E)	8	#8	30' - 4"	U
m(E)	14	#6	20' - 3"	U
m1(E)	14	#6	22' - 3"	U
m2(E)	40	#6	6' - 3"	U
m3(E)	20	#6	3' - 9"	U
m4(E)	10	#6	1' - 10"	U
m5(E)	10	#6	3' - 11"	U
m6(E)	8	#6	4' - 6"	U
m7(E)	4	#6	2' - 11"	U
m8(E)	2	#6	1' - 1"	U
m9(E)	2	#6	3' - 1"	U
m10(E)	36	#5	4' - 0"	U
s(E)	76	#5	10' - 10"	U
s1(E)	54	#5	17' - 10"	U
s4(E)	348	#4	5' - 3"	U
v(E)	88	#5	3' - 1"	U
Concrete Superstructure			Cu. Yd.	213.7
Bridge Deck Grooving			Sq. Yd.	469
Protective Coat			Sq. Yd.	571
Reinforcement Bars, Epoxy Coated			Pound	38,660

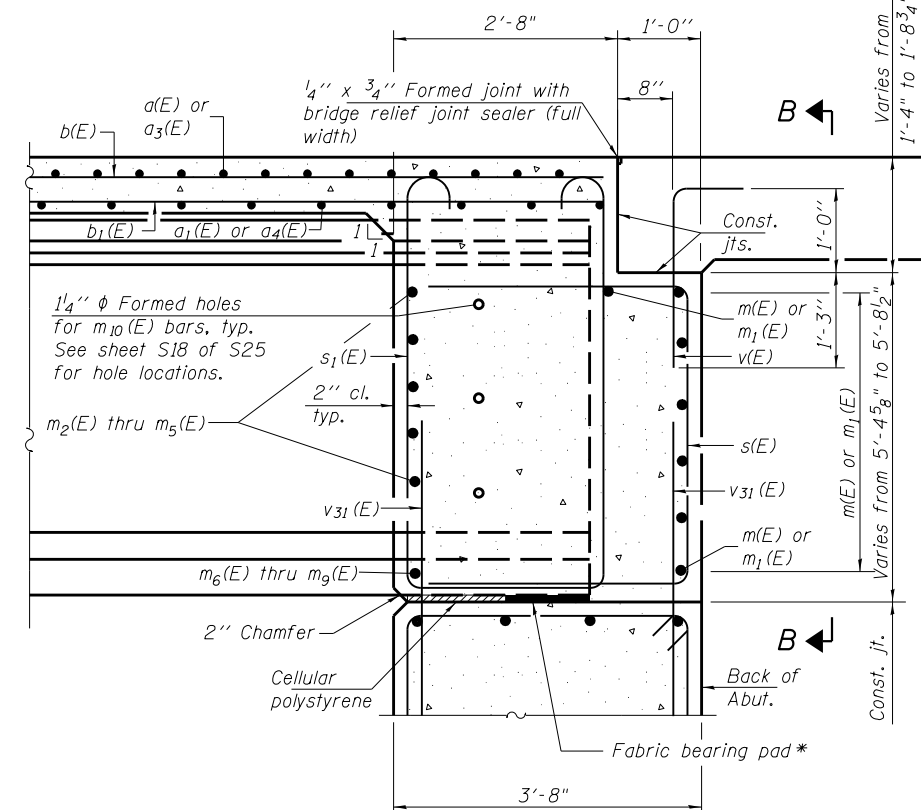
Bars indicated thus 1 x 4 - #8 etc. indicates 1 line of bars with 4 lengths per line.



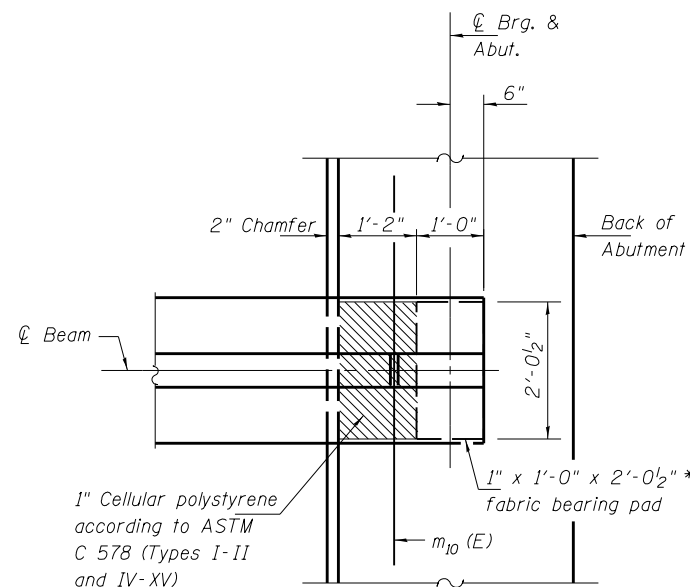
DIAPHRAGM ELEVATION AT ABUTMENT



SECTION B-B



SECTION A-A



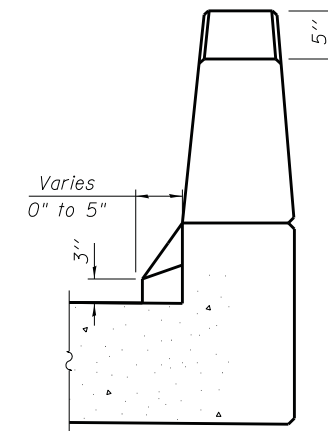
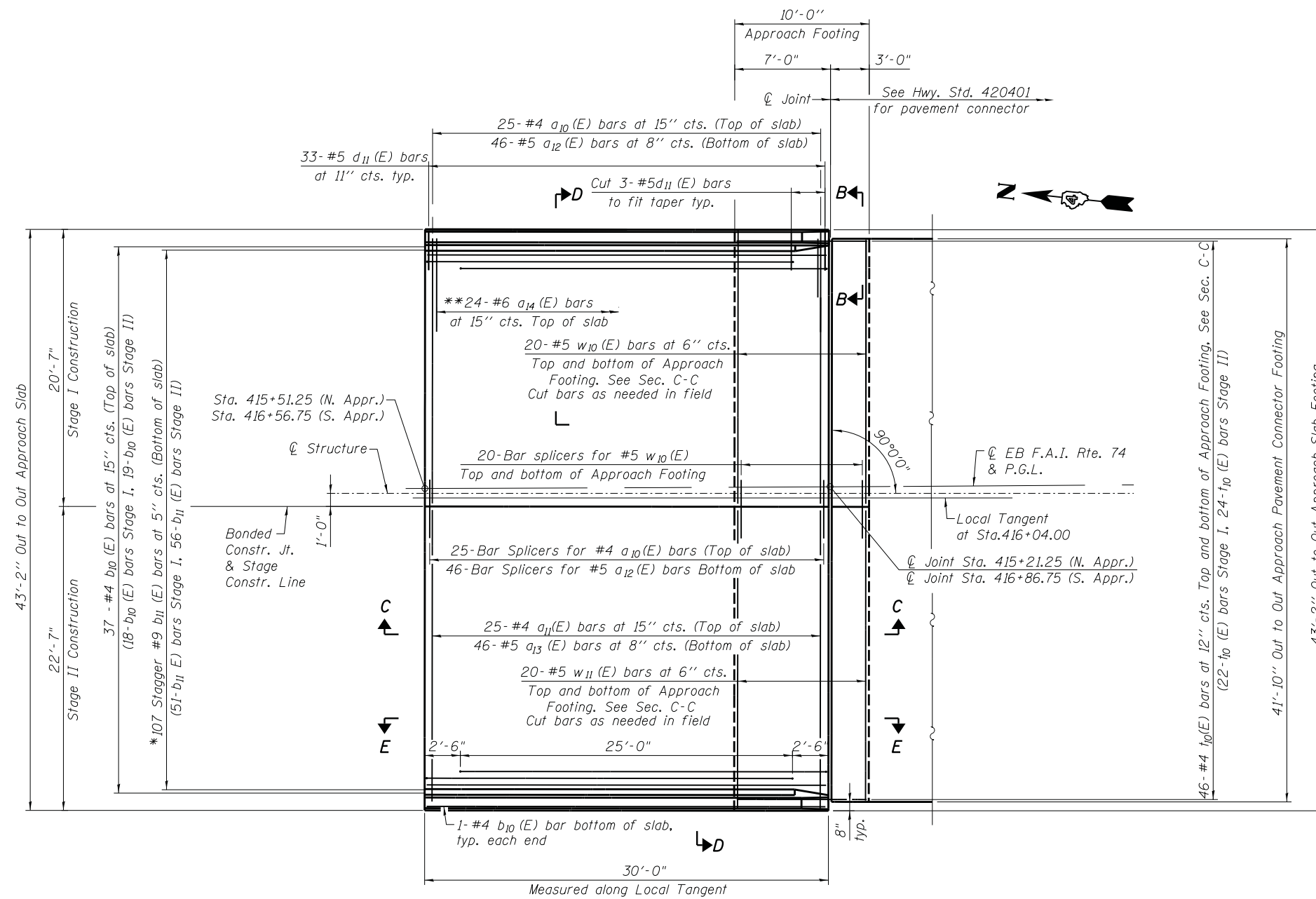
PARTIAL PLAN AT ABUTMENT
(Showing bottom flange of beam)

*Cost of fabric bearing pad is included with Concrete Superstructure

NOTES

1. Reinforcement bars in diaphragm are billed with superstructure on sheet S13 of S25.
2. Concrete in diaphragm is included with Concrete Superstructure on sheet S13 of S25.
3. For details of bars s(E), s1(E) and v(E) see sheet S13 of S25.
4. The approach slab seat shall have a constant slope determined from the control points shown.
5. Cost of cellular polystyrene is included with Concrete Superstructure.

FILE NAME = ... \0480090-68084-014-Diaphragm.dgn	DESIGNED EV DRAWN JCP	REVISED - REVISED -	 800 WEST FULTON STREET CHICAGO, ILLINOIS 60611-1209 TEL. 312 454 9100 FAX 312 555 1217 WEB www.sepstein.com	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	DIAPHRAGM DETAILS STRUCTURE NO. 048-0090 SHEET NO. S14 OF S25 SHEETS	F.A.I. RTE. 74	SECTION 48-(25B)BR, BR-1J	COUNTY KNOX	TOTAL SHEETS 131	SHEET NO. 35
PLOT TIME = 9:31:50 AM	CHECKED PC	REVISED -				CONTRACT NO. 68B85				
PLOT DATE = 10/1/2014	DATE 05 03 2013	REVISED -				ILLINOIS FED. AID PROJECT				



VIEW B-B

PLAN

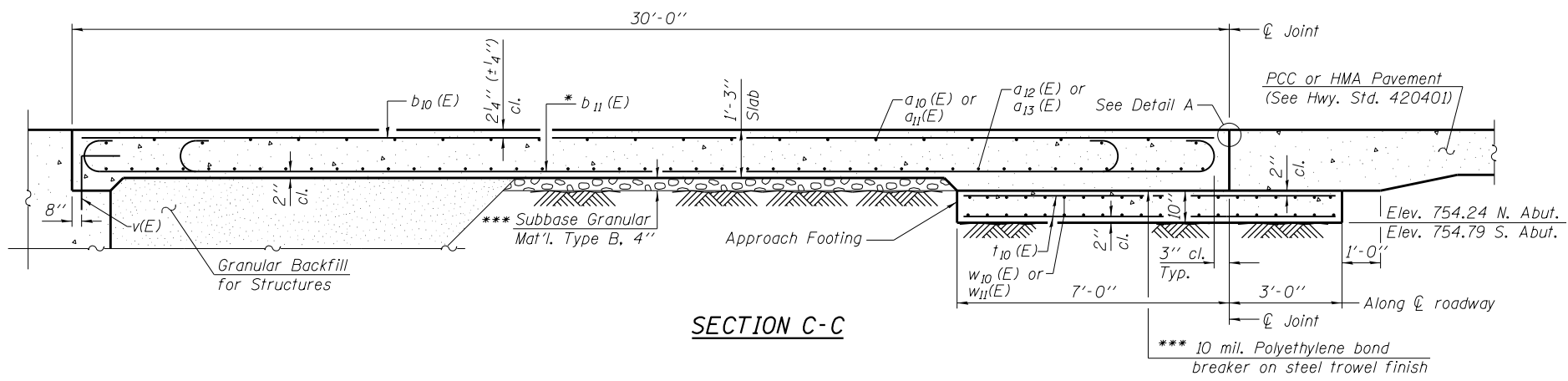
South Approach slab is shown,
North Approach slab is similar

- * Tilt #9 b₁₁(E) bars as required to maintain clearance.
- ** Space between a₁₀(E) and a₁₁(E) bars, typ. ea. parapet.

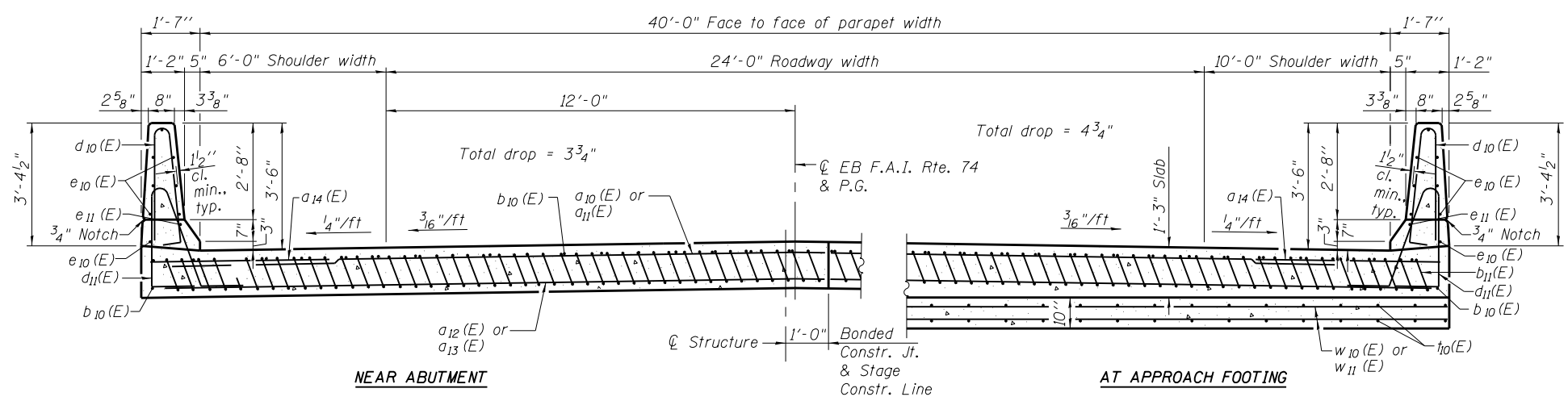
NOTES

1. For Sections C-C & D-D, View E-E, bar bending diagrams and Bill of Materials see sheet S16 of S25.
2. For Bar Splicer (E) details see sheet S24 of S25.
3. a₁₀(E) thru a₁₃(E) bar spacings measured along \bar{C} Rdwy.

FILE NAME = ... \0480090-68084-015-ApprSlab.dgn	DESIGNED <i>EV</i> DRAWN <i>JCP</i>	REVISED - REVISED - REVISED - REVISED -	 800 WEST FULTON STREET CHICAGO, ILLINOIS 60611-1259 TEL 312 454 9100 FAX 312 555 1217 WEB www.sepstein.com	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	BRIDGE APPROACH SLAB STRUCTURE NO. 048-0090	F.A.I. RTE. 74	SECTION 48-[125B]BR, BR-1J	COUNTY KNOX	TOTAL SHEETS 131	SHEET NO. 36
PLOT TIME = 4:31:25 PM	CHECKED <i>PC</i>	DATE 05 03 2013				CONTRACT NO. 68B85				
PLOT DATE = 7/30/2014	DATE	REVISED -				SHEET NO. S15 OF S25 SHEETS				
						ILLINOIS FED. AID PROJECT				

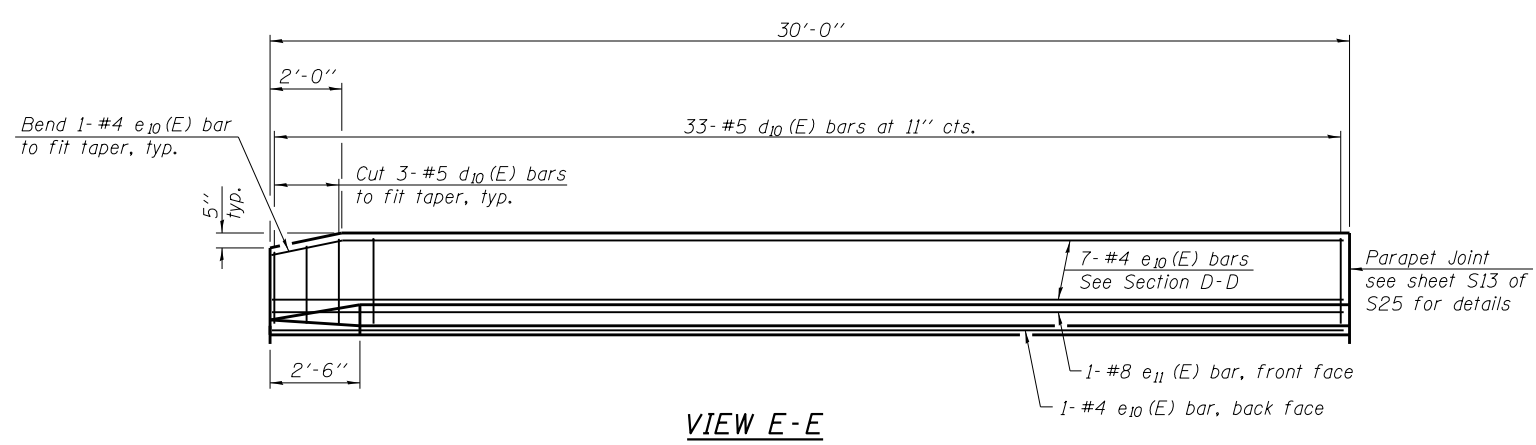


SECTION C-C

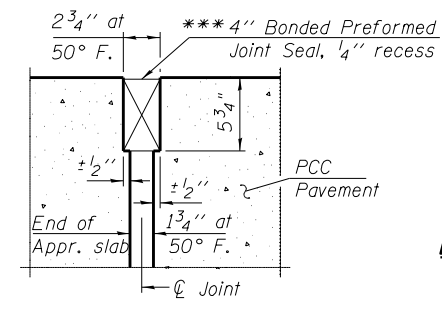


SECTION D-D

(See Plan for dimensions not shown)

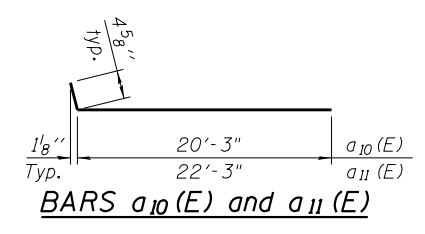


VIEW E-E

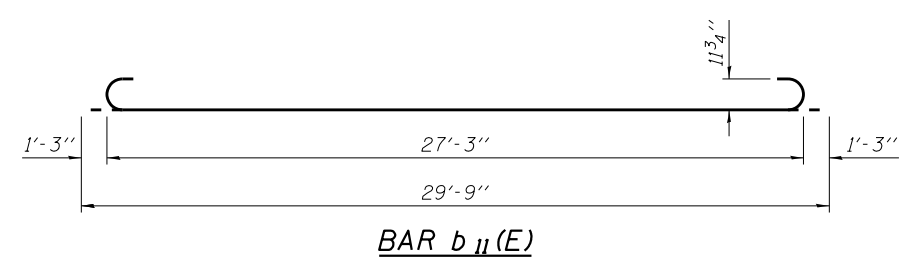


RIGID PAVEMENT

DETAIL A



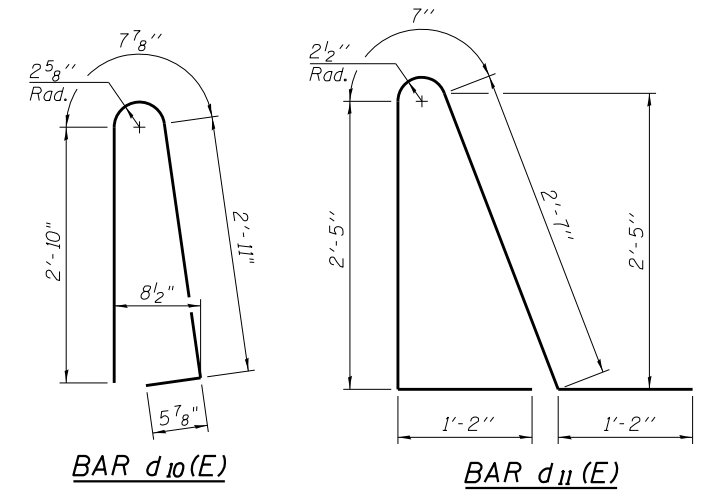
BARS a10(E) and a11(E)



BAR b11(E)

NOTES

1. Approach slab and parapet concrete shall be paid for as Concrete Superstructure.
2. Approach footing concrete shall be paid for as Concrete Structures.
3. Reinforcement shall be paid for as Reinforcement Bars, Epoxy Coated.
4. For v(E) bar details, see sheet S13 of S25.
5. The approach footing maximum applied service bearing pressure (Qmax) = 2.0 ksf.
6. For bar splicer details, see sheet S24 of S25.
7. Cost of excavation for approach footing included with Concrete Structures.
8. For Granular Backfill for Structures and drainage treatment details, see sheet S2 of S25.
9. For additional parapet details, see sheet S13 of S25.



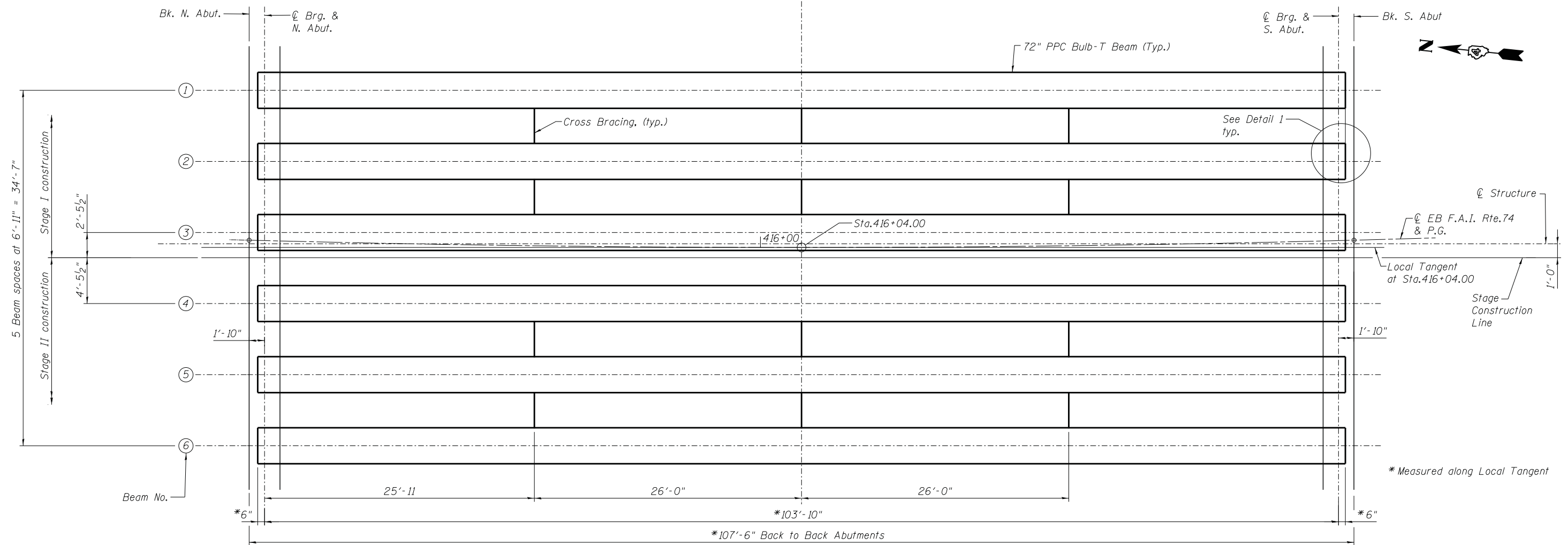
BAR d10(E)

BAR d11(E)

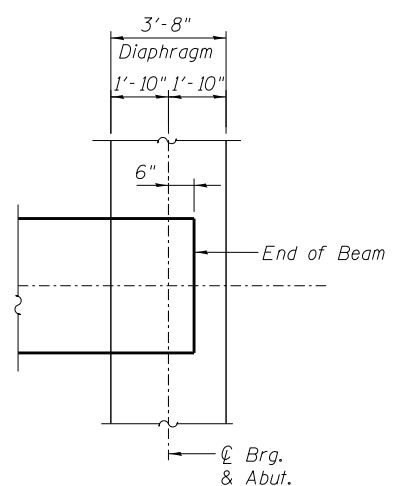
- * Tilt #9 b11(E) bars as required to maintain clearance.
- *** Cost included with Concrete Superstructure.

TWO APPROACH SLABS
BILL OF MATERIAL

Bar	No.	Size	Length	Shape	
a10(E)	50	#4	20' - 8"	—	
a11(E)	50	#4	22' - 8"	—	
a2(E)	92	#5	20' - 4"	—	
a3(E)	92	#5	22' - 4"	—	
a4(E)	96	#6	6' - 6"	—	
b10(E)	78	#4	29' - 8"	—	
b11(E)	214	#9	29' - 9"	—	
d10(E)	132	#5	6' - 11"	—	
d11(E)	132	#5	7' - 11"	—	
e10(E)	32	#4	29' - 8"	—	
e11(E)	4	#8	29' - 8"	—	
t10(E)	184	#4	9' - 8"	—	
w10(E)	80	#5	20' - 4"	—	
w11(E)	80	#5	22' - 4"	—	
Concrete Superstructure				Cu. Yd.	138.5
Concrete Structures				Cu. Yd.	26.6
Bridge Deck Grooving				Sq. Yd.	267
Protective Coat				Sq. Yd.	326
Reinforcement Bars, Epoxy Coated				Pound	37,430



PLAN



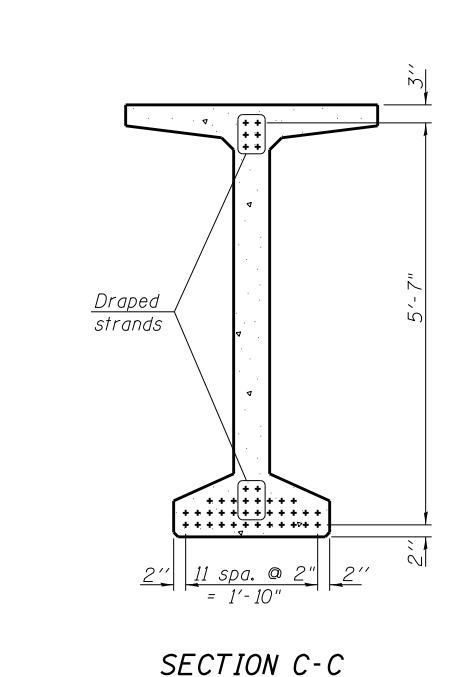
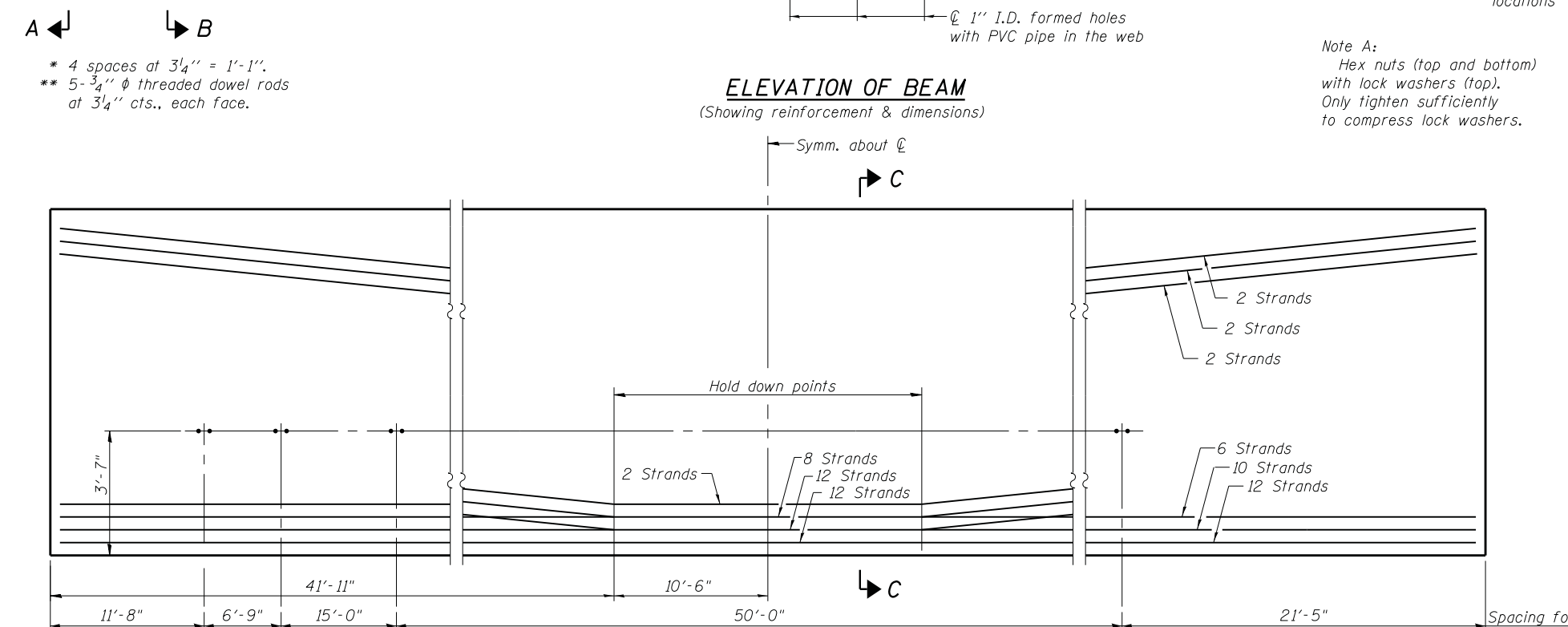
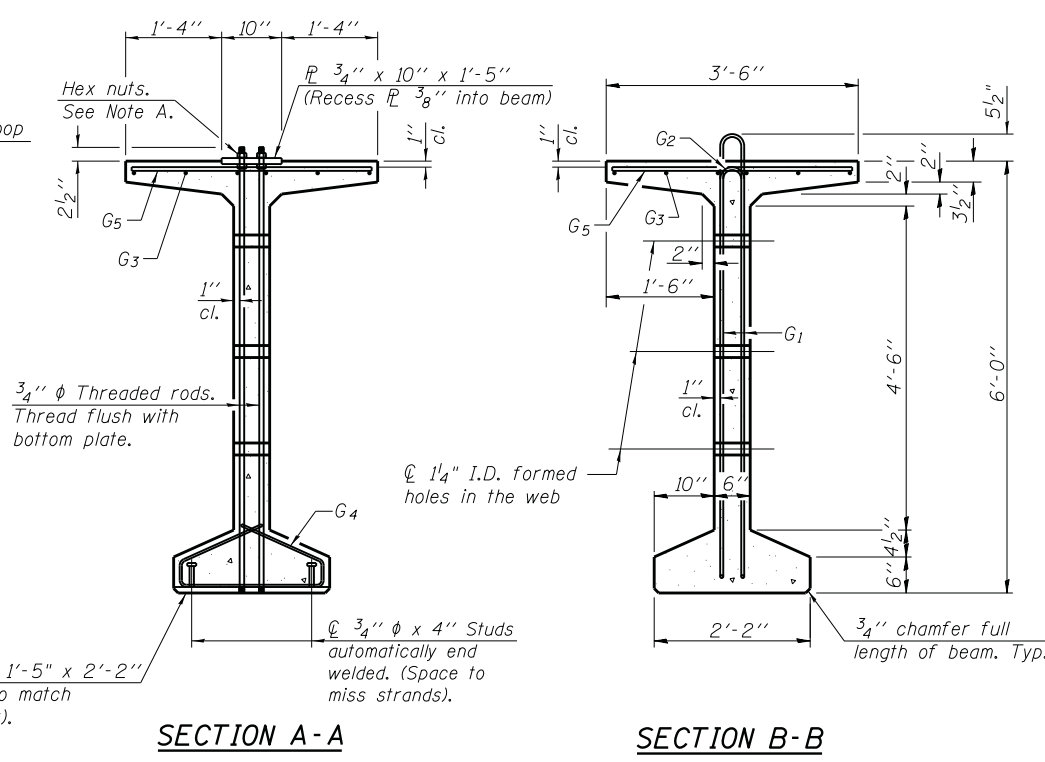
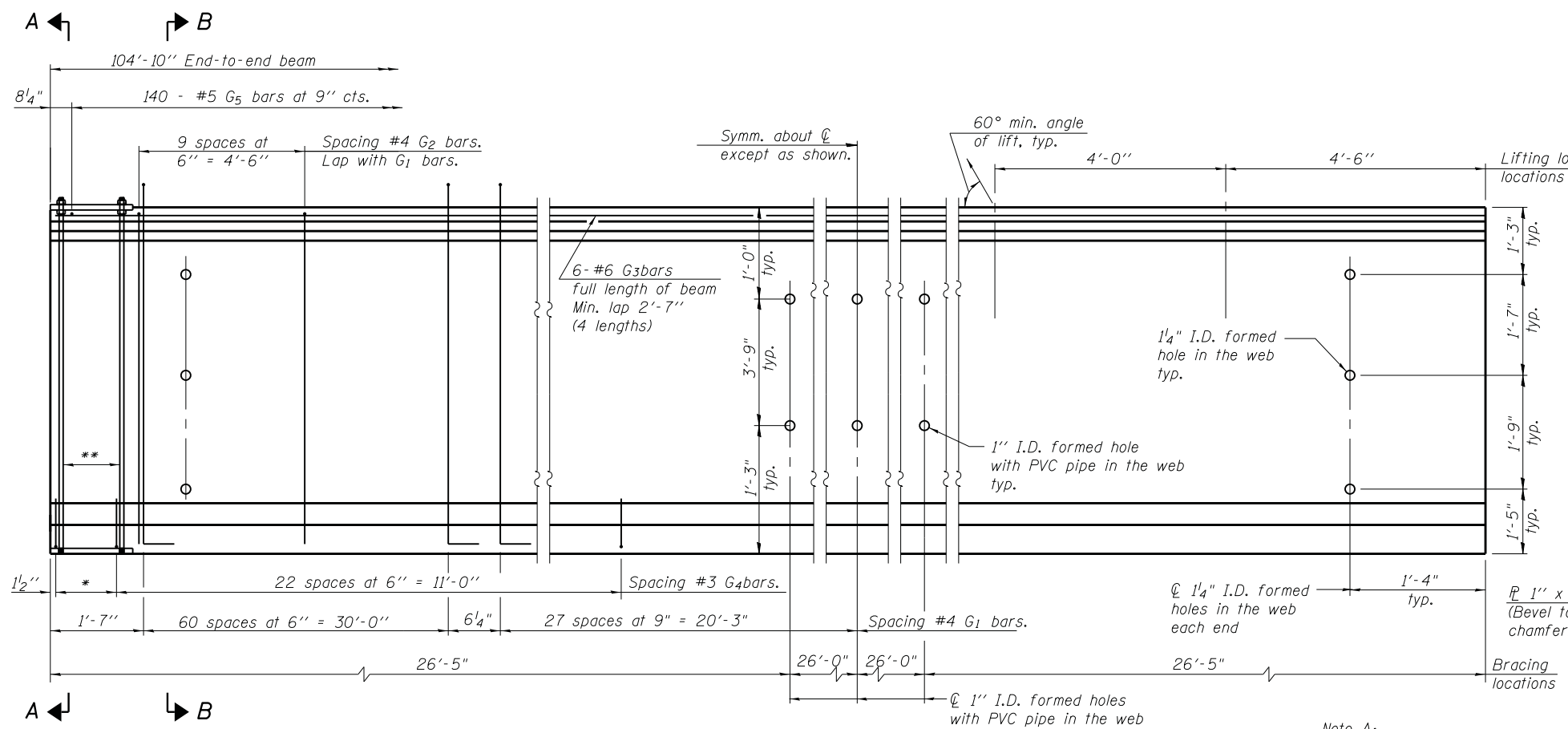
DETAIL 1

INTERIOR BEAM MOMENT TABLE		
0.5 Sp. 1		
<i>I</i>	(in ⁴)	545894
<i>I'</i>	(in ⁴)	1043629
<i>S_b</i>	(in ³)	14915
<i>S_b'</i>	(in ³)	19847
<i>S_t</i>	(in ³)	15421
<i>S_t'</i>	(in ³)	53747
<i>DC1</i>	(k/')	1.534
<i>M_{DC1}</i>	(k)	2068.1
<i>DC2</i>	(k/')	0.173
<i>M_{DC2}</i>	(k)	233.6
<i>DW</i>	(k/')	0.333
<i>M_{DW}</i>	(k)	449.3
<i>M_L + IM</i>	(k)	1869

INTERIOR BEAM REACTION TABLE		
Abut.		
<i>R_{DC1}</i>	(k)	79.7 **
<i>R_{DC2}</i>	(k)	9.0
<i>R_{DW}</i>	(k)	17.3
<i>R_L + IM</i>	(k)	89.0
<i>R_{Total}</i>	(k)	195.0

I: Non-composite moment of inertia of beam section (in.⁴).
I': Composite moment of inertia of beam section (in.⁴).
S_b: Non-composite section modulus for the bottom fiber of the prestressed beam (in.³).
S_b': Composite section modulus for the bottom fiber of the prestressed beam (in.³).
S_t: Non-composite section modulus for the top fiber of the prestressed beam (in.³).
S_t': Composite section modulus for the top fiber of the prestressed beam (in.³).
DC1: Un-factored non-composite dead load (kips/ft.).
M_{DC1}: Un-factored moment due to non-composite dead load (kip-ft.).
DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).
M_{DC2}: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).
DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).
M_{DW}: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).
M_L + IM: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).

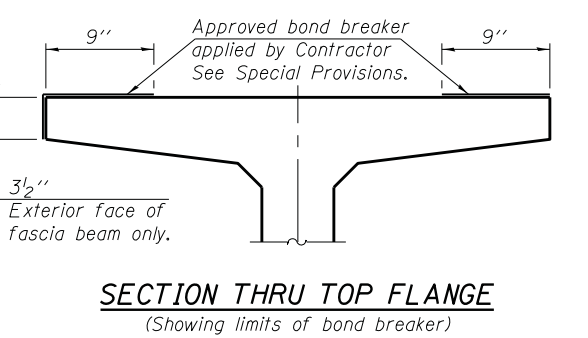
** End Diaphragm Load is excluded



*****BAR LIST
ONE BEAM ONLY**

Bar	No.	Size	Length	Shape
G ₁	178	#4	13'-7"	∩L
G ₂	20	#4	11'-8"	∩
G ₃	24	#6	28'-6"	—
G ₄	54	#3	4'-11"	∩
G ₅	139	#5	3'-4"	—

- ***For information only
- NOTES**
- See sheet S19 of S25 for additional details and Bill of Material.
 - Required release strength, *f'ci*, shall be 5,000 psi.
 - Apply approved bond breaker as shown in Section thru Top Flange full length of beam. See Special Provisions.

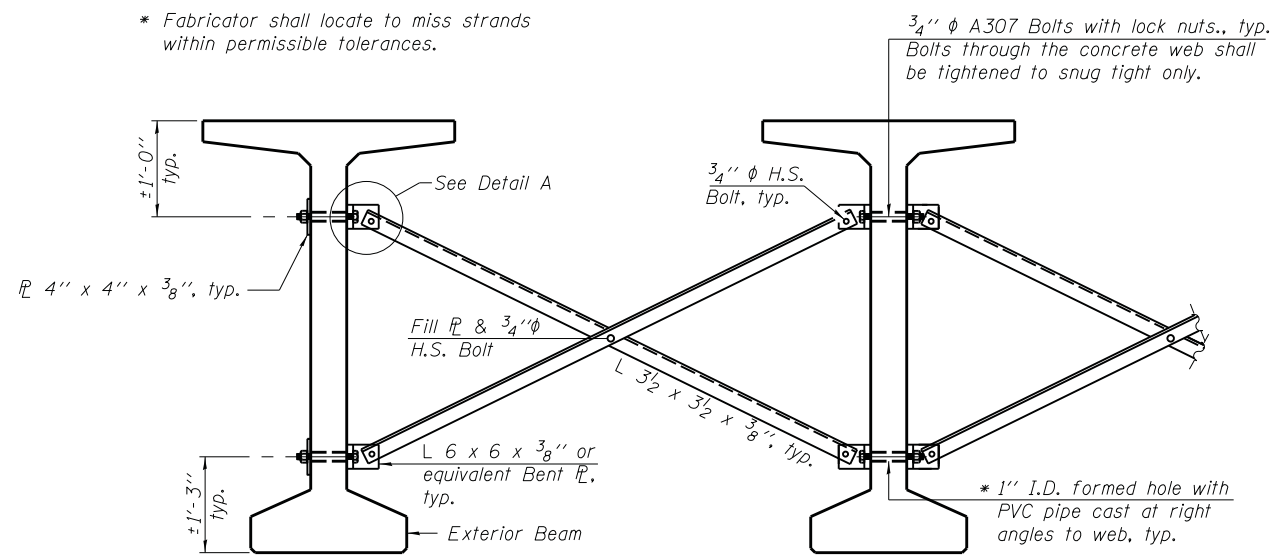


Note A:
Hex nuts (top and bottom) with lock washers (top). Only tighten sufficiently to compress lock washers.



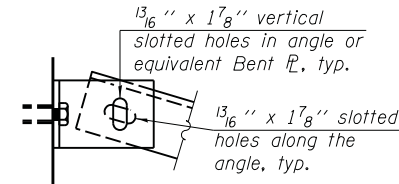
NOTES

1. Inserts for $\frac{3}{4}$ " ϕ threaded dowel rods, when specified, are to be two strut, ferrule type for interior beams and single ferrule, flared loop type for exterior beams.
2. Prestressing steel shall be uncoated high strength, low relaxation 7-wire strand, Grade 270. The nominal diameter shall be $\frac{1}{2}$ " and the nominal cross-sectional area shall be 0.153 sq. in.
3. Reinforcement bars shall conform to ASTM A 706, Grade 60.
4. A minimum $2\frac{1}{2}$ " ϕ lifting pin shall be used to engage the lifting loops during handling.
5. The top and bottom plates shall be AASHTO M270 Grade 50.
6. The bottom plates and studs shall be galvanized according to AASHTO M111. Top plates and threaded rods need not be galvanized.
7. Threaded rods shall be ASTM F 1554 Grade 55.

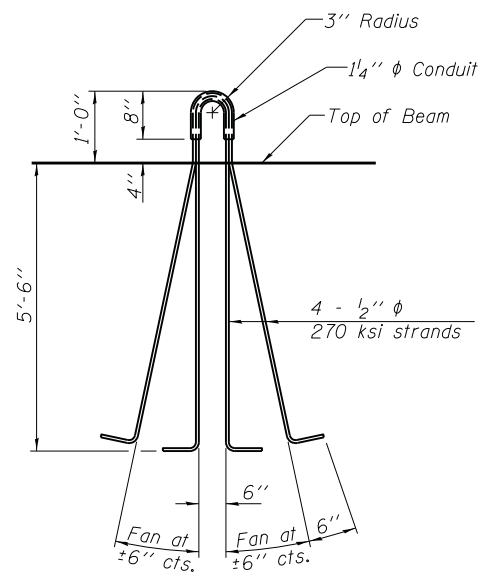


NOTES

1. All material for bracing shall be hot dip galvanized according to AASHTO M111 unless otherwise noted.
2. Two hardened washers are required for each set of oversized holes.
3. All holes shall be $\frac{15}{16}$ " ϕ unless otherwise noted.
4. $\frac{5}{16}$ " x 3" x 3" plate washers are required over all slotted holes.
5. All bolts shall be galvanized according to AASHTO M232.
6. Bracing shall be installed as beams are erected and tightened as soon as possible during erection.
7. Permanent bracing shall not be paid for separately, but shall be included in the cost of Furnishing and Erecting Precast Prestressed Concrete Bulb T-Beams.

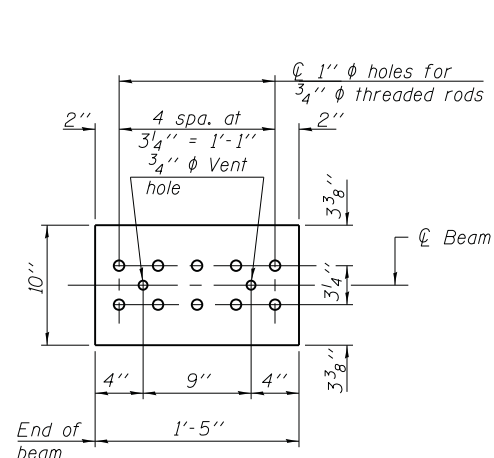


DETAIL A

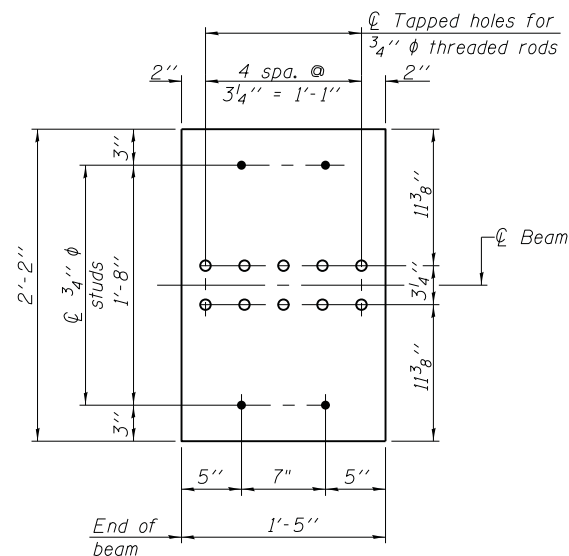


LIFTING LOOP DETAIL

PERMANENT BRACING DETAILS FOR BULB-T BEAMS

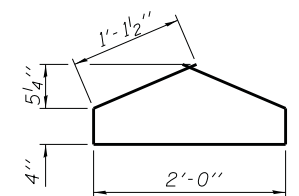
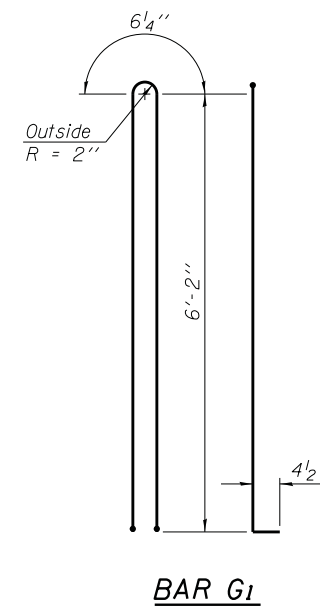


TOP PLATE



BOTTOM PLATE

See bearing details for pintle hole locations when required.



BILL OF MATERIAL

Item	Unit	Total
Furnishing and Erecting Precast Prestressed Concrete Bulb T-Beams, 72"	Ft.	629

FILE NAME = ... \0480090-68084-019-PPC.T.beam Details.dgn	DESIGNED EV	REVISED -
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PLOT DATE = 7/30/2014	CHECKED PC	REVISED -
	DATE 05 03 2013	REVISED -

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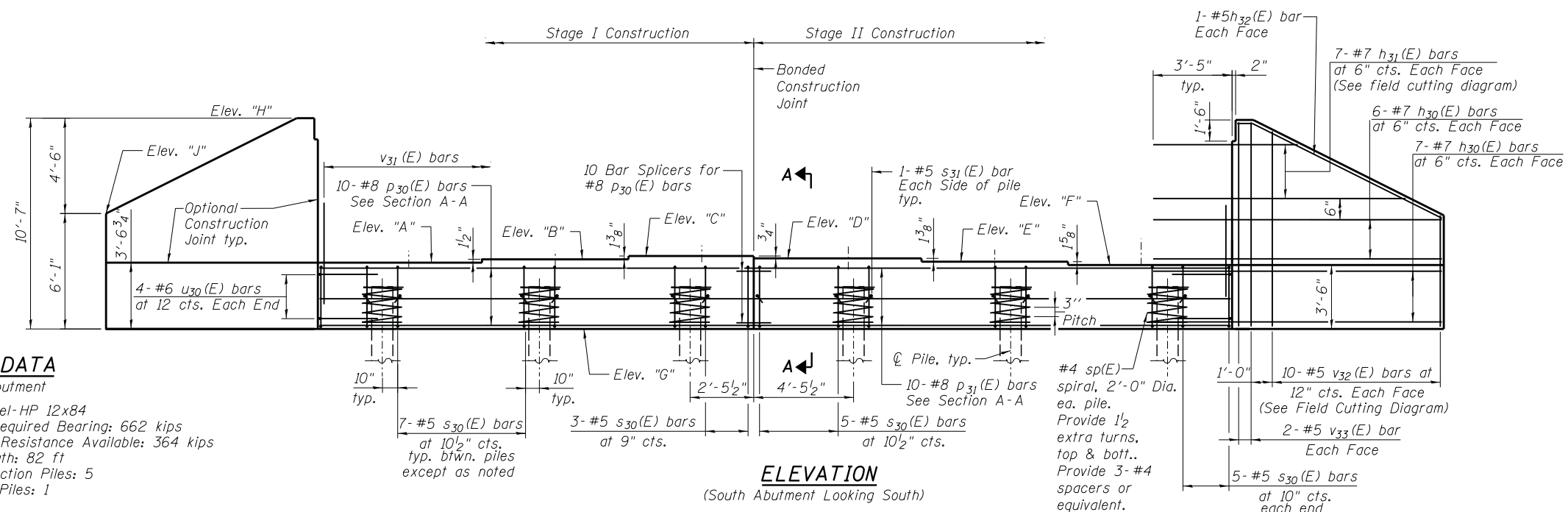
**72" PPC BULB T-BEAM DETAILS
STRUCTURE NO. 048-0090**

SHEET NO. S19 OF S25 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
74	48-(125B)BR, BR-1J	KNOX	131	40
CONTRACT NO. 68B85				
ILLINOIS FED. AID PROJECT				

NOTES

1. Pour steps monolithically with cap.



PILE DATA

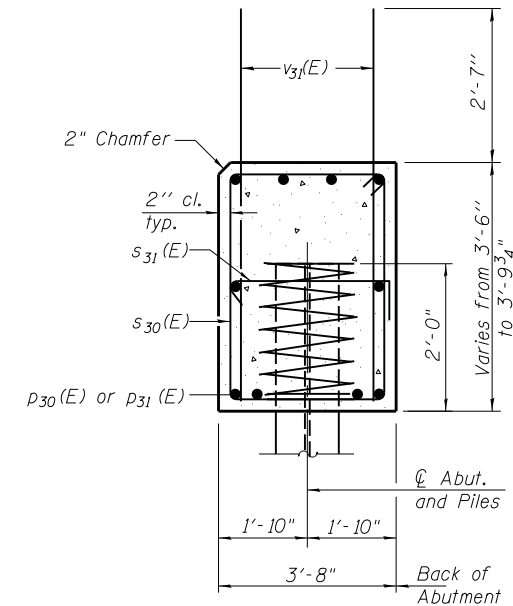
North Abutment

Type: Steel-HP 12x84
 Nominal Required Bearing: 662 kips
 Factored Resistance Available: 364 kips
 Est. Length: 82 ft
 No. Production Piles: 5
 No. Test Piles: 1

PILE DATA

South Abutment

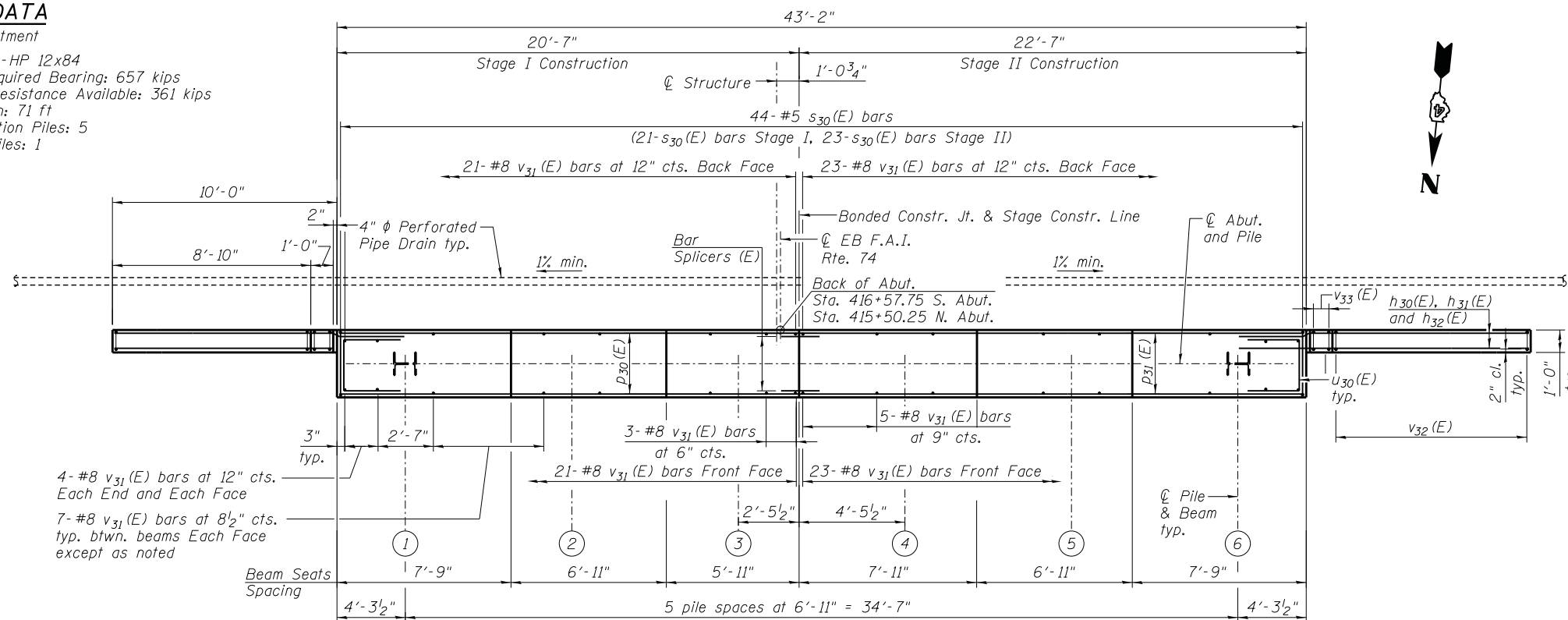
Type: Steel-HP 12x84
 Nominal Required Bearing: 657 kips
 Factored Resistance Available: 361 kips
 Est. Length: 71 ft
 No. Production Piles: 5
 No. Test Piles: 1



SECTION A-A

ELEVATION

(South Abutment Looking South)



PLAN

South Abutment show.
 North Abutment is similar

	N. Abut.	S. Abut.
Elev. "A"	749.26	749.61
Elev. "B"	749.39	749.73
Elev. "C"	749.50	749.84
Elev. "D"	749.43	749.78
Elev. "E"	749.32	749.67
Elev. "F"	749.18	749.53
Elev. "G"	745.70	746.03
Elev. "H"	756.28	756.61
Elev. "J"	751.78	752.11

NOTES

1. For Section thru Abutment, drainage details, Bar Bending Diagrams and Bill of Materials see sheet S21 of S25.
2. Backfill shall be placed behind the abutment after the superstructure has been poured and falsework removed. See Article 502.10 of the Standard Specifications.
3. For Bar Splicer (E) details see sheet S24 of S25.
4. For details of piles see sheet S22 of S25.

BILL OF MATERIAL

North Abutment

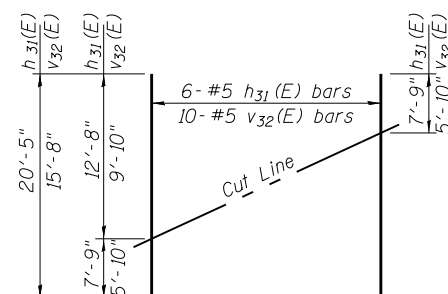
Bar	No.	Size	Length	Shape
h30(E)	52	#7	13' - 5"	—
h31(E)	12	#7	20' - 5"	—
h32(E)	4	#5	10' - 5"	—
p30(E)	10	#8	20' - 3"	—
p31(E)	10	#8	22' - 3"	—
s30(E)	46	#5	13' - 11"	□
s31(E)	12	#5	4' - 4"	┌
* sp(E)	12	#4	2' - 0"	W W W W
u30(E)	8	#6	11' - 0"	┌
v31(E)	88	#8	6' - 0"	—
v32(E)	20	#5	15' - 8"	—
v33(E)	4	#5	10' - 3"	—
Structure Excavation	Cu. Yd.	402		
Concrete Structures	Cu. Yd.	28.8		
Reinforcement Bars, Epoxy Coated	Pound	5,760		
Furnishing Steel Piles HP12x84	Foot	410		
Driving Piles	Foot	410		
Test Pile Steel HP12x84	Each	1		
Geocomposite Wall Drain	Sq. Yd.	66		
Granular Backfill for Structures	Cu. Yd.	159.1		
Pipe Underdrain for Structures, 4" dia.	Foot	100		

BILL OF MATERIAL

South Abutment

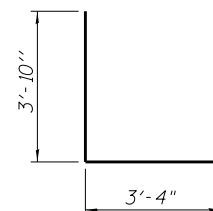
Bar	No.	Size	Length	Shape
h30(E)	52	#7	13' - 5"	—
h31(E)	12	#7	20' - 5"	—
h32(E)	4	#5	10' - 5"	—
p30(E)	10	#8	20' - 3"	—
p31(E)	10	#8	22' - 3"	—
s30(E)	46	#5	13' - 11"	□
s31(E)	12	#5	4' - 4"	┌
* sp(E)	12	#4	2' - 0"	W W W W
u30(E)	8	#6	11' - 0"	┌
v31(E)	88	#8	6' - 0"	—
v32(E)	20	#5	15' - 8"	—
v33(E)	4	#5	10' - 3"	—
Structure Excavation	Cu. Yd.	402		
Concrete Structures	Cu. Yd.	28.8		
Reinforcement Bars, Epoxy Coated	Pound	5,760		
Furnishing Steel Piles HP12x84	Foot	355		
Driving Piles	Foot	355		
Test Pile Steel HP12x84	Each	1		
Geocomposite Wall Drain	Sq. Yd.	66		
Granular Backfill for Structures	Cu. Yd.	159.1		
Pipe Underdrain for Structures, 4" dia.	Foot	102		

* Length is height of spiral.

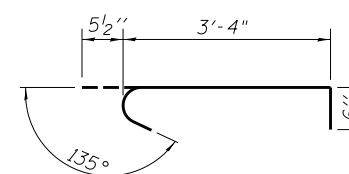


FIELD CUTTING DIAGRAM

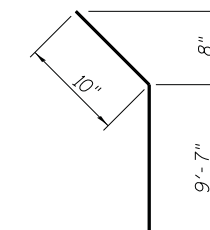
Order h31(E) and v32(E) full length. Cut as shown and use remainder of bars in opposite face.



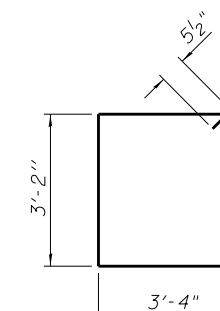
BAR u30(E)



BAR s31(E)



BAR h32(E)



BAR s30(E)

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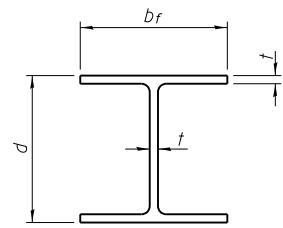
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**ABUTMENT DETAILS
 STRUCTURE NO. 048-0090**

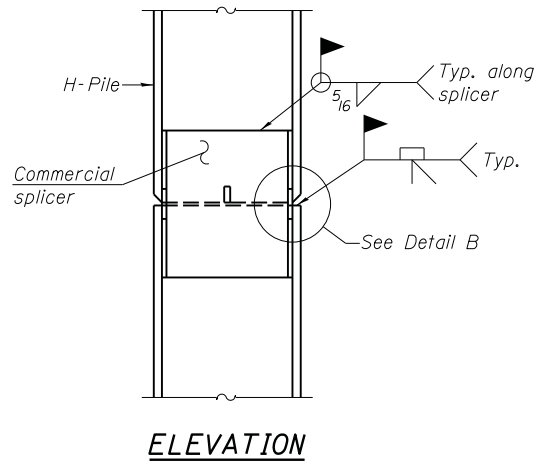
SHEET NO. S21 OF S25 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
74	48-[125B]BR, BR-1J	KNOX	131	42
			CONTRACT NO. 68B85	
ILLINOIS FED. AID PROJECT				

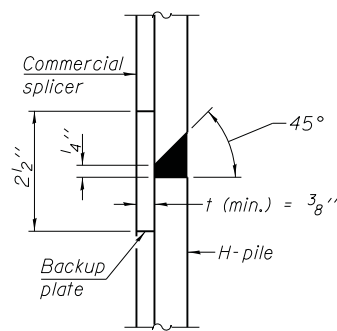


STEEL PILE TABLE

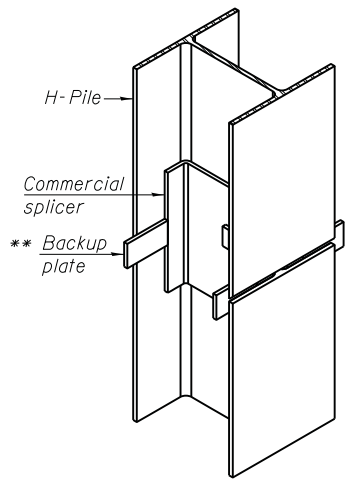
Designation	Depth d	Flange width br	Web and Flange thickness t	Encasement diameter A
HP 14x117	14 1/4"	14 7/8"	13/16"	30"
x102	14"	14 3/4"	1/16"	30"
x89	13 7/8"	14 3/4"	5/8"	30"
x73	13 5/8"	14 5/8"	1/2"	30"
HP 12x84	12 1/4"	12 1/4"	1/16"	24"
x74	12 1/8"	12 1/4"	5/8"	24"
x63	12"	12 1/8"	1/2"	24"
x53	11 3/4"	12"	7/16"	24"
HP 10x57	10"	10 1/4"	9/16"	24"
x42	9 3/4"	10 1/8"	7/16"	24"
HP 8x36	8"	8 1/8"	7/16"	18"



ELEVATION

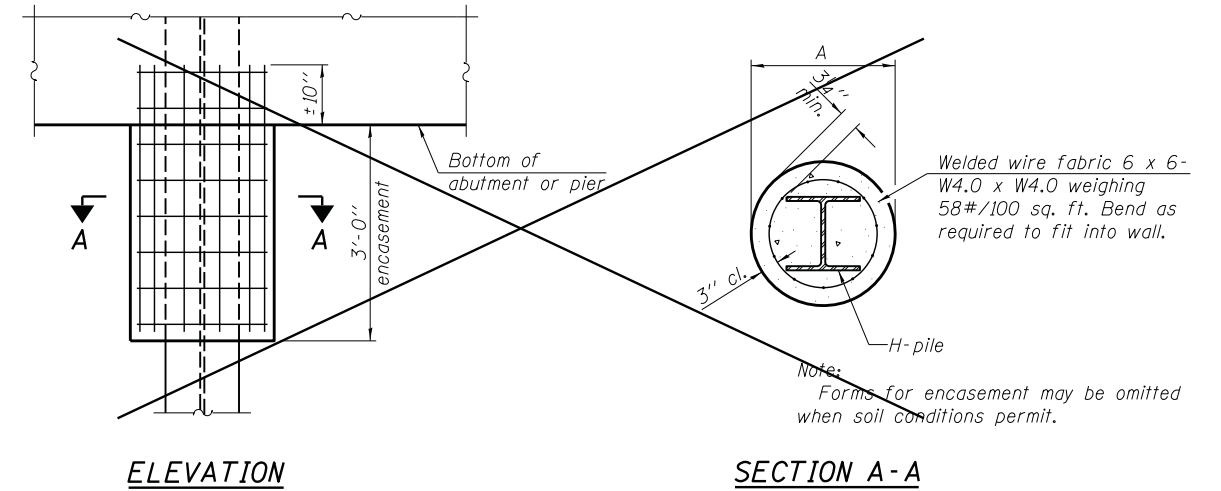


DETAIL "B"



ISOMETRIC VIEW

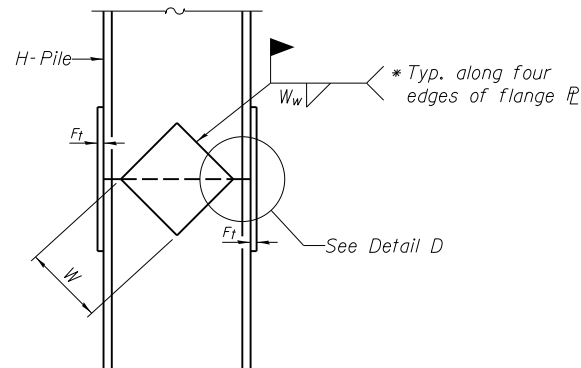
WELDED COMMERCIAL SPLICE



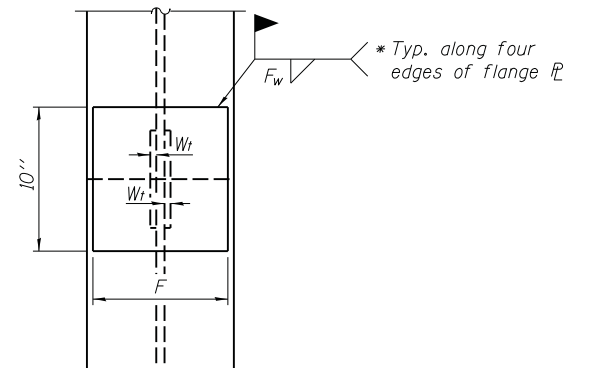
ELEVATION

SECTION A-A

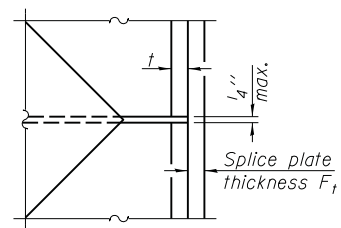
PILE ENCASEMENT



ELEVATION



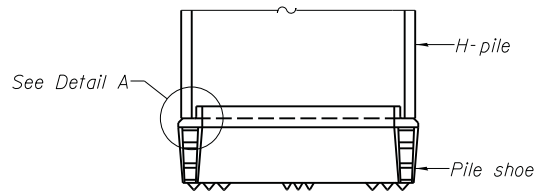
END VIEW



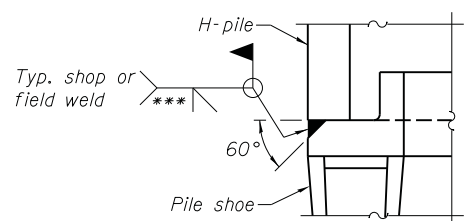
DETAIL D

WELDED PLATE FIELD SPLICE

Designation	F	F _t	F _w	W	W _t	W _w
HP 14x117	12 1/2"	1"	7/8"	7 3/4"	5 1/2"	1/2"
x102	12 1/2"	7/8"	3/4"	7 3/4"	5 1/2"	1/2"
x89	12 1/2"	3/4"	1/16"	7 3/4"	5 1/2"	1/2"
x73	12 1/2"	5/8"	9/16"	7 3/4"	5 1/2"	1/2"
HP 12x84	10"	7/8"	1/16"	6 1/2"	5 1/2"	1/2"
x74	10"	7/8"	1/16"	6 1/2"	5 1/2"	1/2"
x63	10"	5/8"	1/2"	6 1/2"	1/2"	3/8"
x53	10"	5/8"	1/2"	6 1/2"	1/2"	3/8"
HP 10x57	8"	3/4"	9/16"	5 1/4"	1/2"	3/8"
x42	8"	5/8"	9/16"	5 1/4"	1/2"	3/8"
HP 8x36	7"	5/8"	7/16"	4 1/4"	1/2"	3/8"

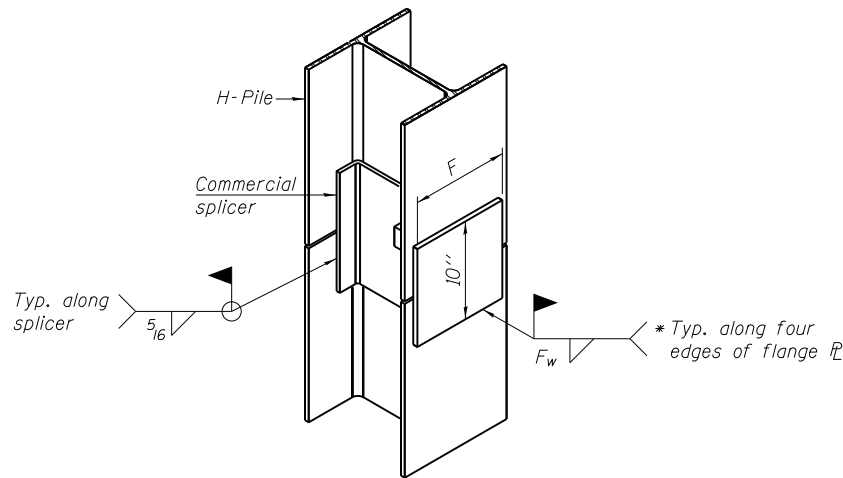


ELEVATION



DETAIL A

H-PILE SHOE ATTACHMENT



ISOMETRIC VIEW

WELDED COMMERCIAL SPLICE ALTERNATE

- * Interrupt welds 1/4" from end of web and/or each flange.
- ** Remove portions of backup plates that extend outside the flanges.
- *** Weld size per pile shoe manufacturer (5/16" min.).

NOTES

1. The steel H-piles shall be according to AASHTO M270 Grade 50.

F-HP 1-27-12

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PLOT DATE = 7/30/2014	DATE 05 03 2013	REVISED -

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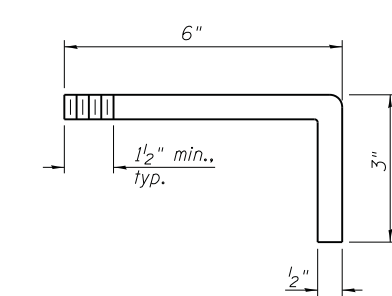
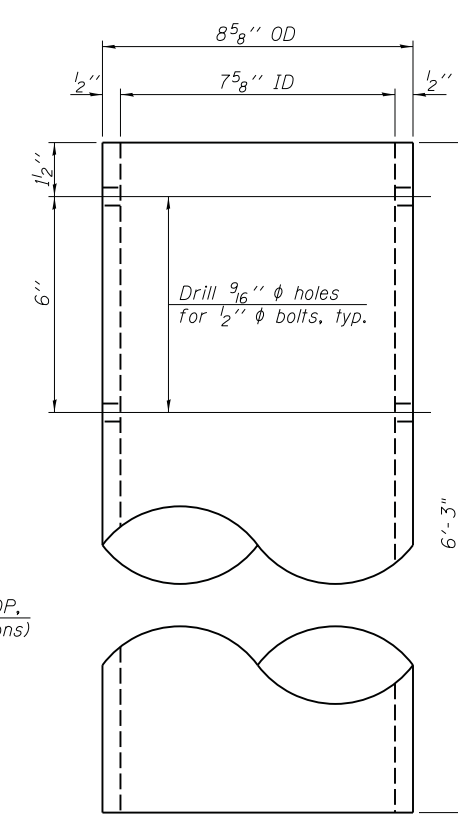
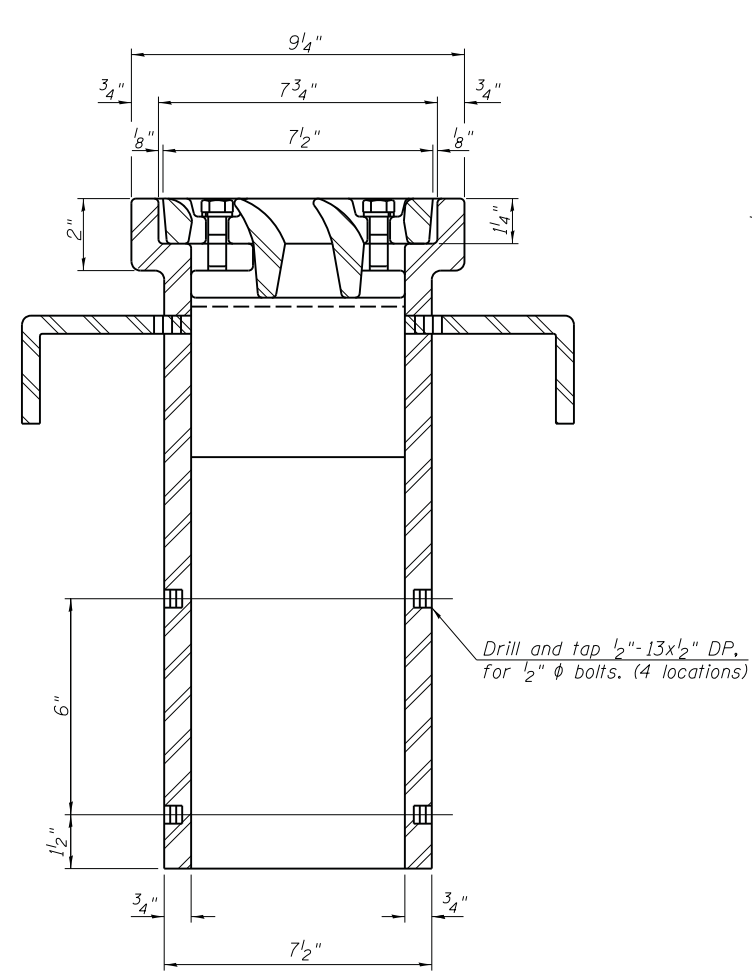
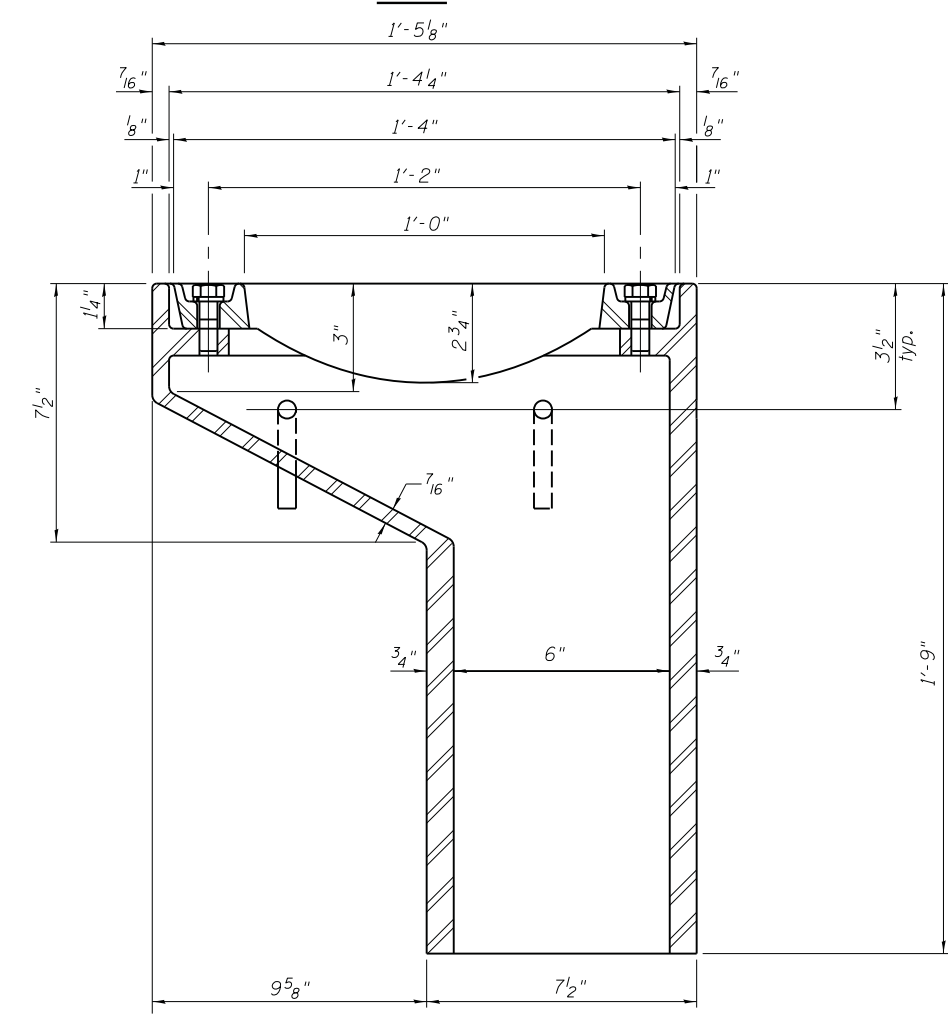
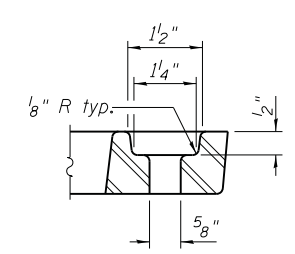
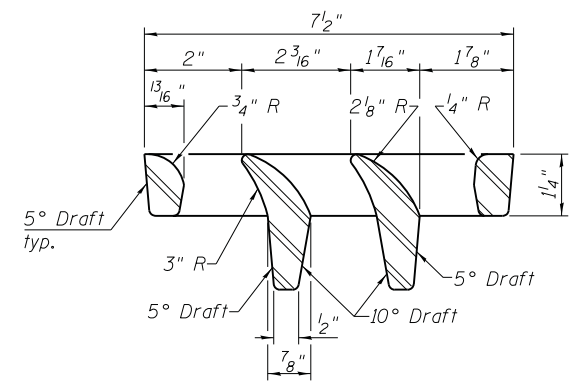
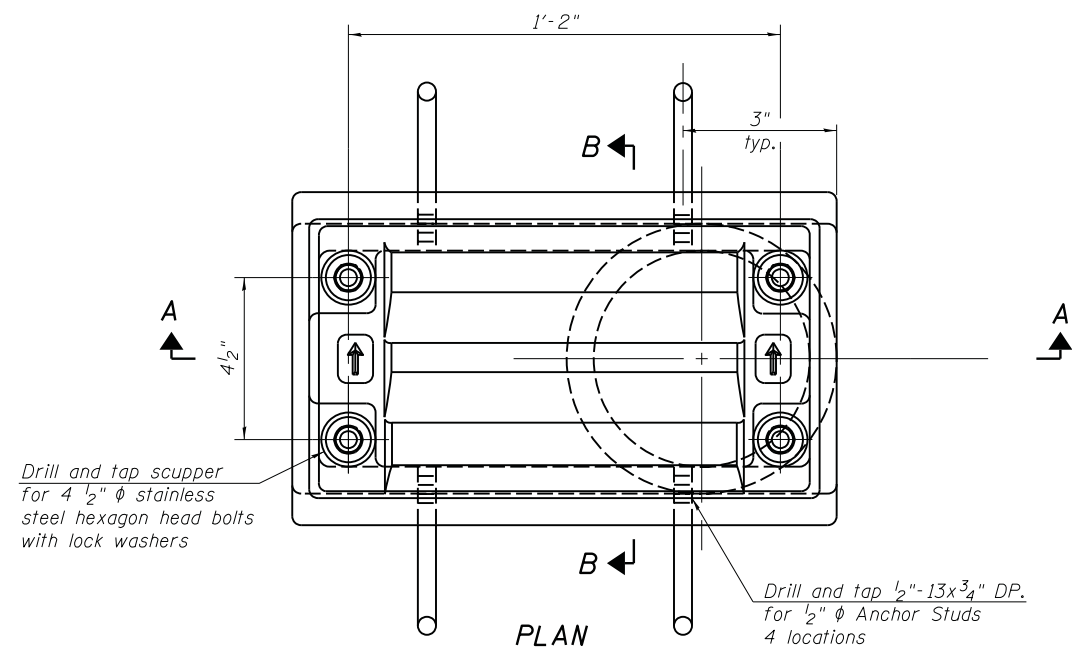
**HP PILE DETAILS
 STRUCTURE NO. 048-0090**

SHEET NO. S22 OF S25 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
74	48-(125B)BR, BR-11	KNOX	131	43
CONTRACT NO. 68B85				
ILLINOIS FED. AID PROJECT				

NOTES

1. All cast iron parts shall be gray iron conforming to the requirements of AASHTO M 105, Class 35B.
2. Bolts, anchor studs, washers and nuts shall conform to the requirements of ASTM A 307 and shall be galvanized according to AASHTO M 232.
3. Downspouts located on the exterior side of a painted steel fascia beam shall be painted with the finish coat specified for the exterior side of the fascia beam.
4. As an alternate, bolts, anchor studs, washers and nuts may be stainless steel according to Article 1006.29(d) of the Standard Specifications.
5. Structural steel weldments of equal sections and of the same configuration may be substituted for the cast iron scupper frame. Fillet or full penetration welds shall be used for the weldments. Details shall be submitted to the Engineer for approval. Structural steel weldments shall not be substituted for the cast iron scupper grate. Structural steel frames and downspouts shall be galvanized according to AASHTO M111.
6. The Contractor shall take appropriate measures to assure that Protective Coat is not applied to the scupper.
7. Cost of the Grate, Frame, Downspout, Anchor Studs, Bolts, Washers and Nuts including complete installation of the scupper shall be paid for at the contract unit price each for Drainage Scupper, DS-11.
8. Alternate fiberglass downspout conforming to ASTM D 2996 with a short-time rupture strength hoop tensile stress of 30,000 psi min. may be used in lieu of the cast iron or steel equivalent.



BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Drainage Scupper, DS-11	Each	8

DS-11

7-1-10

FILE NAME =	DESIGNED EV	REVISED -
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PLOT DATE = 7/30/2014	DATE 05 03 2013	REVISED -

SEPSTEIN

800 WEST FULTON STREET
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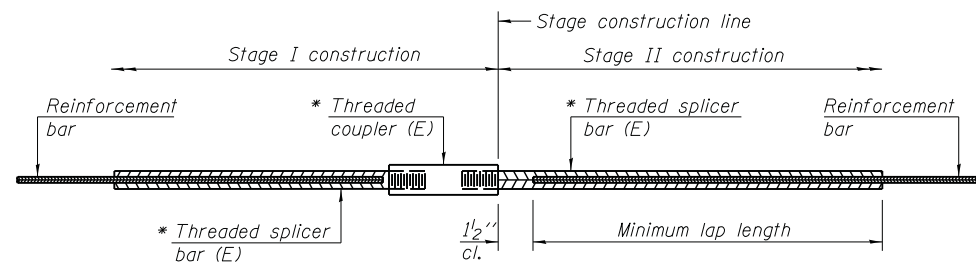
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**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**DRAINAGE SCUPPER DS-11
STRUCTURE NO. 048-0090**

SHEET NO. S23 OF S25 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
74	48-(125B)BR, BR-11	KNOX	131	44
CONTRACT NO. 68B85				
ILLINOIS FED. AID PROJECT				



STANDARD BAR SPLICER ASSEMBLY

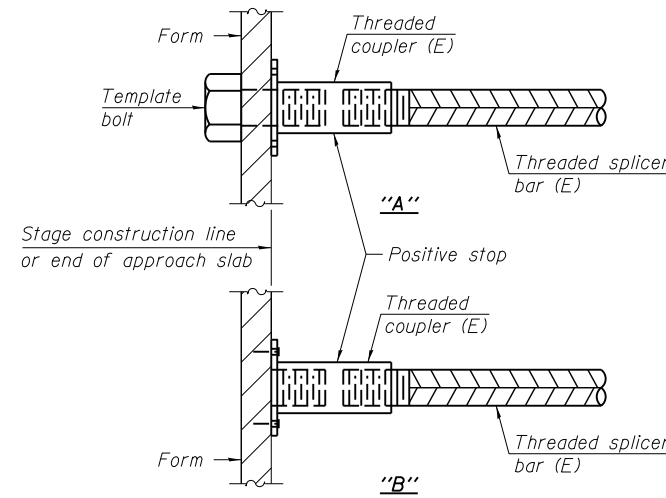
Minimum Lap Lengths						
Bar size to be spliced	Table 1	Table 2	Table 3	Table 4	Table 5	Table 6
3, 4	1'-5"	1'-11"	2'-1"	2'-4"	2'-7"	2'-11"
5	1'-9"	2'-5"	2'-7"	2'-11"	3'-3"	3'-8"
6	2'-1"	2'-11"	3'-1"	3'-6"	3'-10"	4'-5"
7	2'-9"	3'-10"	4'-2"	4'-8"	5'-2"	5'-10"
8	3'-8"	5'-1"	5'-5"	6'-2"	6'-9"	7'-8"
9	4'-7"	6'-5"	6'-10"	7'-9"	8'-7"	9'-8"

- Table 1: Black bar, 0.8 Class C
- Table 2: Black bar, Top bar lap, 0.8 Class C
- Table 3: Epoxy bar, 0.8 Class C
- Table 4: Epoxy bar, Top bar lap, 0.8 Class C
- Table 5: Epoxy bar, Class C
- Table 6: Epoxy bar, Top bar top, Class C

Threaded splicer bar length = min. lap length + 1/2" + thread length

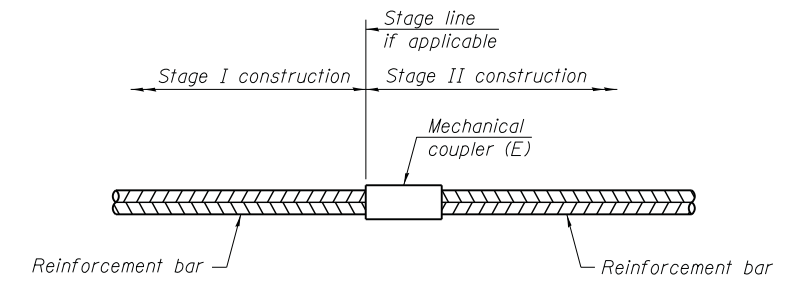
* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

Location	Bar size	No. assemblies required	Table for minimum lap length
Deck	#5	311	3
Diaphragm	#6	26	3
Abutments	#8	20	3
Approach Slab	#5	172	3
Approach Slab	#4	50	3



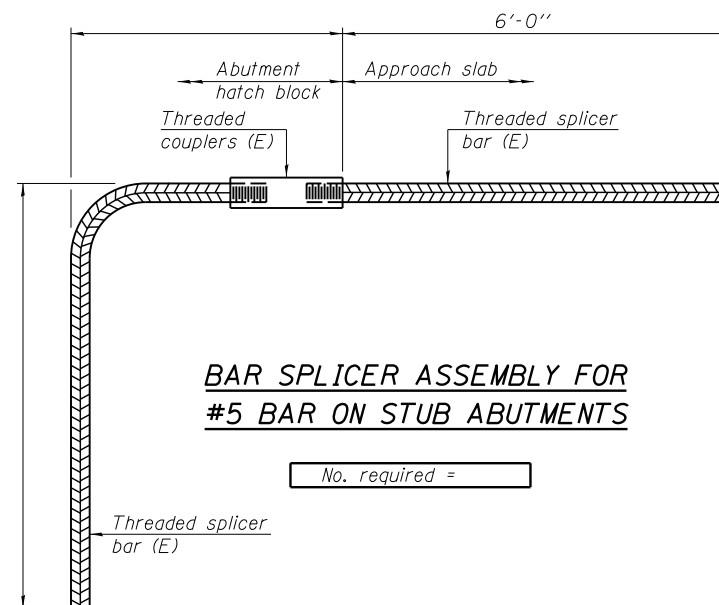
INSTALLATION AND SETTING METHODS

"A" : Set bar splicer assembly by means of a template bolt.
 "B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms.
 (E) : Indicates epoxy coating.



STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required



BAR SPLICER ASSEMBLY FOR #5 BAR ON STUB ABUTMENTS

NOTES

1. Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.
2. All reinforcement shall be lapped and tied to the splicer bars.
3. Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications.
4. See approved list of bar splicer assemblies and mechanical splicers for alternatives.

BSD-1

8-31-12

FILE NAME =	DESIGNED <i>EV</i>	REVISED -
...\\0480090-68084-024-Bar_splacers.dgn	DRAWN <i>JCP</i>	REVISED -
PLOT TIME = 4:31:38 PM	CHECKED <i>PC</i>	REVISED -
PLOT DATE = 7/30/2014	DATE <i>05 03 2013</i>	REVISED -

SEPSTEIN
 800 WEST FULTON STREET
 CHICAGO, ILLINOIS 60611-1259
 TEL 312 454 9100
 FAX 312 559 1217
 WEB www.sepstein.com

**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**BAR SPLICER ASSEMBLY AND MECHANICAL SPLICER DETAILS
 STRUCTURE NO. 048-0090**

SHEET NO. S24 OF S25 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
74	48-[25B]BR, BR-1J	KNOX	131	45
CONTRACT NO. 68B85				
ILLINOIS FED. AID PROJECT				

BRIDGE FOUNDATION BORING LOG

PROJECT I-74-2()38 BRIDGE FAI Route 74 Date 1-3-62
 ROUTE FAI 74 over Henderson Creek Bored By Alvin E. Moine
 SEC. 48-25B STA. 117+20 Left Lane Checked By _____
 COUNTY Knox

Elevation	Z	t/sf	w (%)	Surface Water El.	Elevation	Z	t/sf	w (%)
737.80				-	737.8			
Ground Surface						23	2.5 E	-
Soft Dark Brown Silty CLAY LOAM								
	4	0.5 B	35		-25	34	2.5 S	12
733.8					711.3			
Medium Black Silty CLAY								
	7	1.0 E	34			59	-	-
731.3								
Stiff Black Silty CLAY								
	13	1.5 E	-		-30	34	-	-
728.8								
Medium Brown CLAY LOAM								
	5	0.7 B	26			78	-	-
724.8					709.8			
Loose Brown Sandy LOAM								
	4	-	25					
722.85								
Stiff Light Grayish Brown CLAY								
	5	1.2 B	28			93	2.9 S	-
718.8								
Very Stiff Light Gray CLAY								
	9	1.5 E	-		-40	100/5"	-	-
	16	2.3 S	19			50/3"	-	-
					694.8			
					-45			

V - Standard Penetration Test - Blows per foot to drive 2" O.D. Split Spoon Sampler 12" with 40# hammer falling 30".
 Qu - Unconfined Compressive Strength - t/sf
 Type failure:
 B - Bulge Failure
 S - Shear Failure
 E - Estimated Value
 w - Water Content - percentage of oven dry weight-%.

BRIDGE FOUNDATION BORING LOG

PROJECT I-74-2()38 BRIDGE FAI Route 74 Date 1-8-62
 ROUTE FAI 74 over Henderson Creek Bored By Alvin E. Moine
 SEC. 48-25B STA. 117+20 Left Lane Checked By _____
 COUNTY Knox

Elevation	Z	t/sf	w (%)	Surface Water El.	Elevation	Z	t/sf	w (%)
710.70				-	710.70			
Ground Surface								
Stiff Brown Shaley CLAY								
	5	1.5 E	27		-25	100/5"	5.2	-
					715.2			
End of Boring								
	6	1.5 E	-					
734.2								
Very Stiff Brownish Gray CLAY								
	33	1.8 S	30		-30			
729.2								
Hard Light Gray Shaley CLAY								
	98	1.8 S	9		-35			
724.2								
Hard Light Gray Shaley CLAY								
	50/5"	-	-		-40			
	50/4"	-	-					
	100/5"	-	-		-20			
					-45			

V - Standard Penetration Test - Blows per foot to drive 2" O.D. Split Spoon Sampler 12" with 140# hammer falling 30".
 Qu - Unconfined Compressive Strength - t/sf
 Type failure:
 B - Bulge Failure
 S - Shear Failure
 E - Estimated Value
 w - Water Content - percentage of oven dry weight-%.

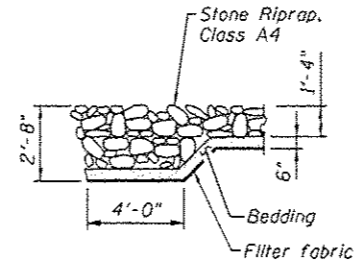
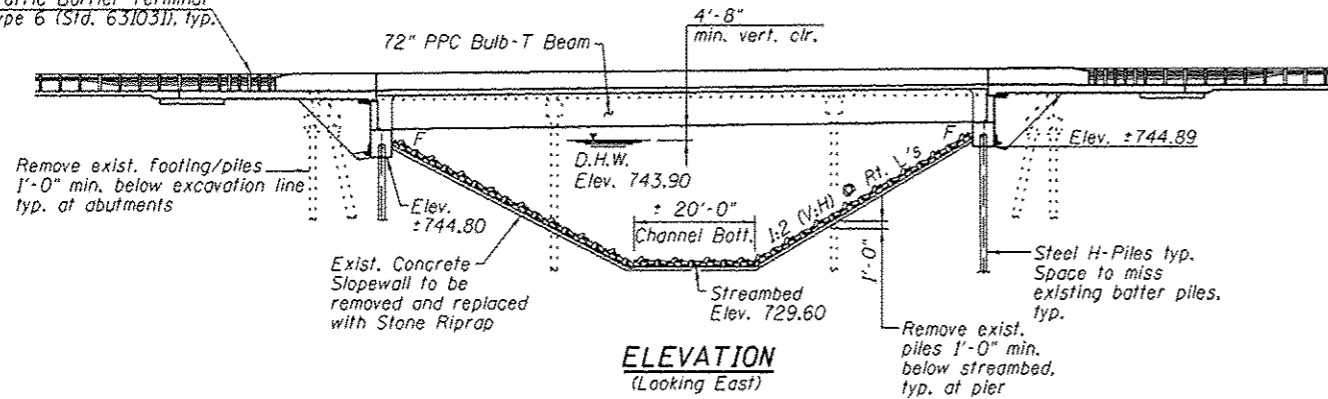
NOTES

1. The structure boring for the project location completed during the year 1961-1962, furnished by Illinois Department of Transportation, has been included as Exhibit B in the Structure Geotechnical Report for the current project.

Bench Mark: BM#3 - Chiseled square located on the south end of the west wingwall WB I-74 Elevation 757.28
 Existing Structure: S.N. 048-0004 built in 1965 as F.A.I. Route 74 (WB), Section 48-25B at Sta. 417+20. Existing structure consists of 3-span continuous reinforced concrete cast in place slab supported by closed concrete abutments and concrete pile bent piers. Structure is 115'-0" long Bk. to Bk. of abutments, width is 44'-0" Out to Out of deck. Structure to be removed and replaced. Traffic to be maintained utilizing stage construction.

No salvage

Traffic Barrier Terminal
 Type 6 (Std. 631031), typ.



DESIGN SPECIFICATIONS
 2012 AASHTO LRFD Bridge Design Specifications

LOADING HL-93
 Allow 50#/sq. ft. for future wearing surface.

DESIGN STRESSES
FIELD UNITS

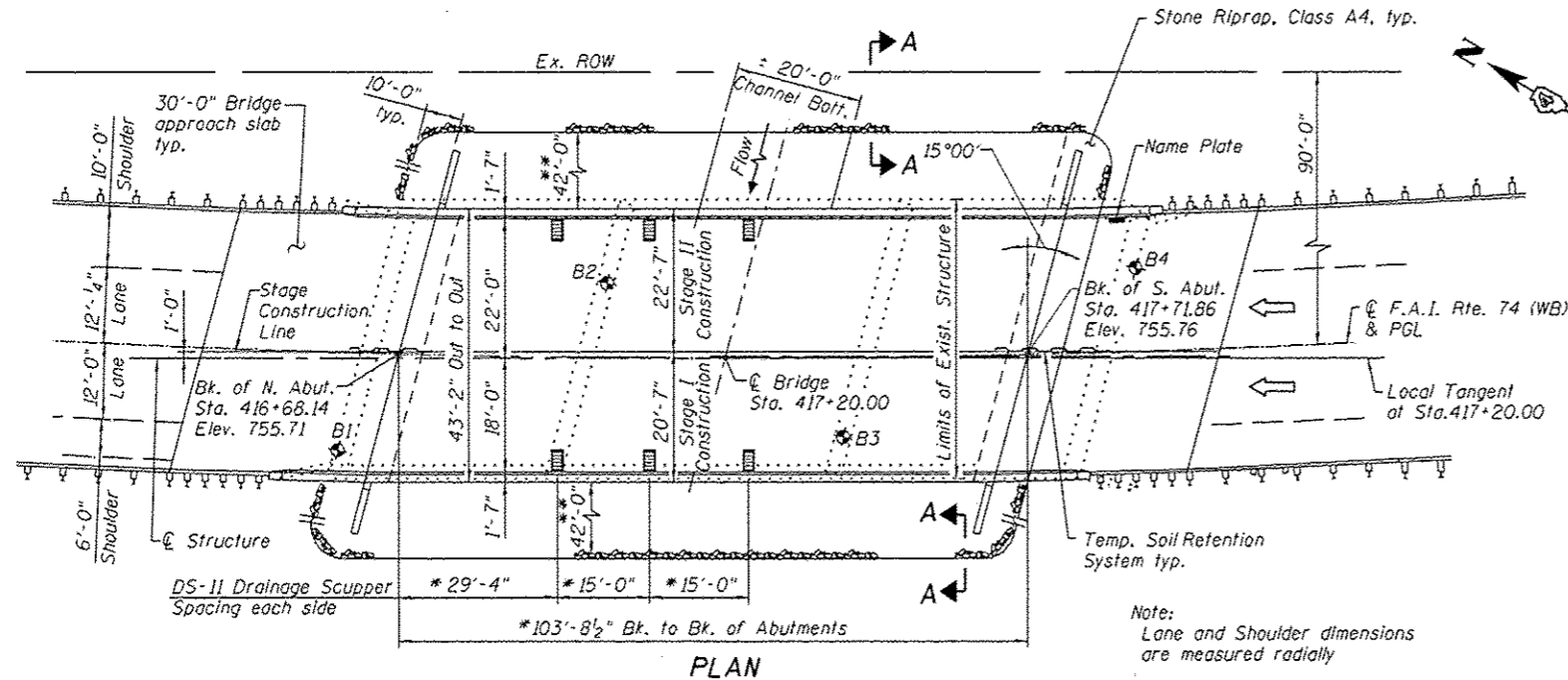
$f'_c = 3,500$ psi
 $f_y = 60,000$ psi (Reinforcement)

PRECAST PRESTRESSED UNITS

$f'_c = 6,000$ psi
 $f'_ci = 5,000$ psi
 $f_{pu} = 270,000$ psi
 (1/2" low lax strands)
 $f_{pbt} = 201,960$ psi
 (1/2" low lax strands)

SEISMIC DATA

Seismic Performance Zone (SPZ) = 1
 Design Spectral Acceleration at 1.0 sec. (SD1) = 0.070g
 Design Spectral Acceleration at 0.2 sec. (SDS) = 0.111g
 Soil Site Class = C



APPROVED
 For Structural Adequacy Only
St. Carl Kruger
 Engineer of Bridges & Structures

DESIGN SCOUR ELEVATION TABLE

Design Scour Elevation (ft.)	N. Abut.	S. Abut.
	744.80	744.89

WATERWAY INFORMATION

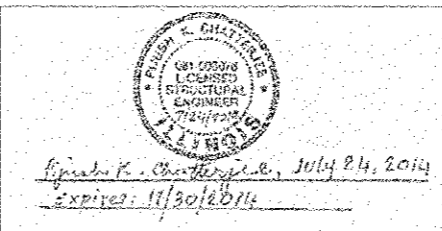
Drainage Area = 9.34 sq. mi. Low Grade Elev. 755.95 ft @ Sta. 417+77.5

Flood	Freq. Yr.	C.F.S.	Opening Sq. Ft.		Nat. Head - Ft.		Headwater El.		
			Exist.	Prop.	H.W.E.	Exist.	Prop.	Exist.	Prop.
Design	10	1942	434	524	741.7	0.3	0.1	742.0	741.8
Base	50	3168	582	680	743.9	0.6	0.3	744.4	744.2
Overtopping	100	3740	642	742	744.7	0.7	0.4	745.4	745.1
Max. Calc.	500	5135	786	889	746.5	1.0	0.6	747.5	747.1

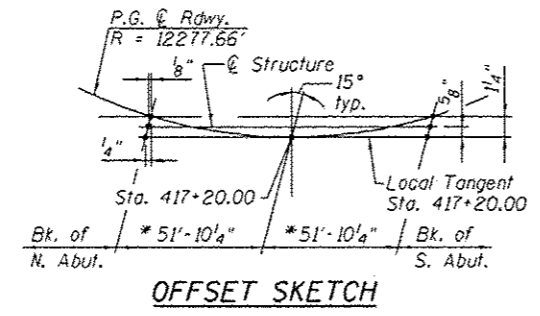
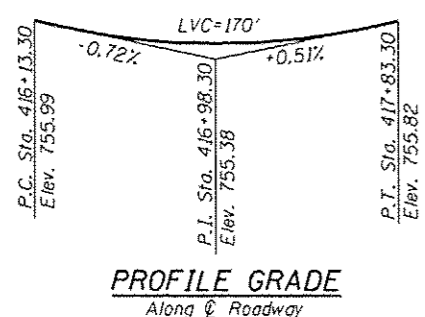
EWSE = 732.18 (per Survey taken 01-22-2010)

STATION 416+04.00
 BUILT 2011 BY
 STATE OF ILLINOIS
 F.A.I. RT.74 WB SEC. 48-[(25B)BR, BR-1]
 LOADING HL-93
 STR. NO. 048-0091

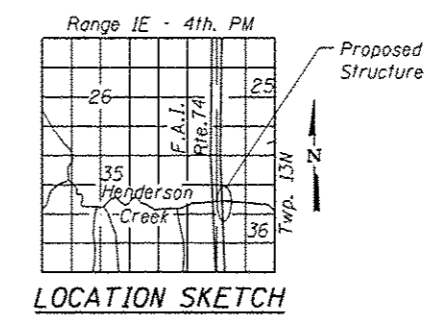
NAME PLATE
 See Std. 515001



Note:
 Lane and Shoulder dimensions are measured radially
 *Measured along local tangent
 **For riprap limits the Contractor shall match limits of existing slope wall



CURVE DATA
 $\Delta = 9^{\circ}-41'-02''$
 $D = 0^{\circ}-28'-00''$
 $T = 1040.04'$
 $L = 2075.12'$
 $E = 43.97'$
 $R = 12277.66'$
 P.C. = Sta. 397+59.56
 P.T. = Sta. 418+34.68
 P.I. = Sta. 407+99.60



GENERAL PLAN AND ELEVATION
WB I-74 OVER HENDERSON CREEK
F.A.I. RTE. 74 (WB) - SEC. 48-[(25B)BR, BR-1]
KNOX COUNTY
STATION 417+20.00
STRUCTURE NO. 048-0091

FILE NAME: ...N0480091-00004-001-CPE.dgn	DESIGNED EV	REVISED -		STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	GENERAL PLAN AND ELEVATION STRUCTURE NO. 048-0091	F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
PLOT TIME: 4:29:11 PM	DRAWN JCP	REVISED -				74	48-[(25B)BR, BR-1]	KNOX	131	47
PLOT DATE: 9/29/2014	CHECKED PC	REVISED -				CONTRACT NO. 68885				
DATE: 05 03 2013	DATE: 05 03 2013	REVISED -				ILLINOIS FED. AID PROJECT				

GENERAL NOTES

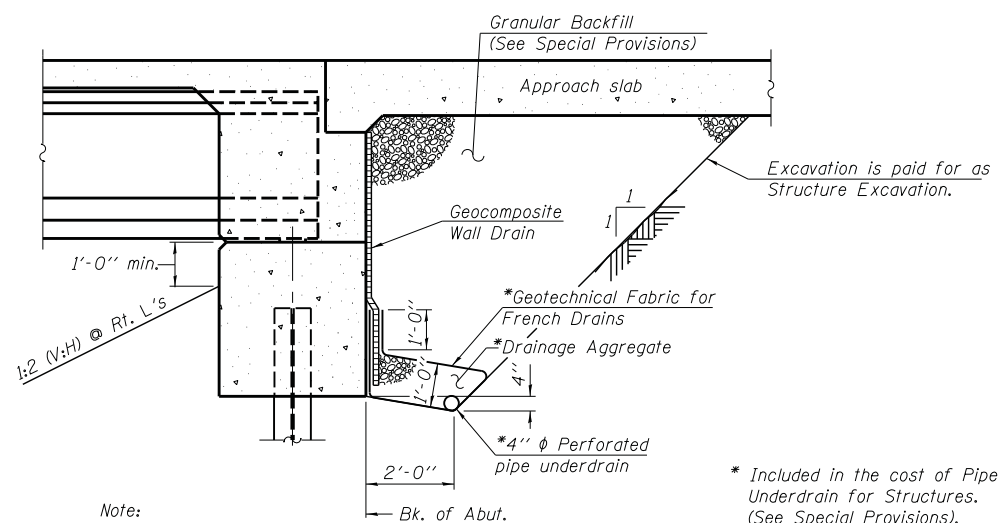
1. Reinforcement bars designated (E) shall be epoxy coated.
2. Layout of slope protection system may be varied to suit ground conditions in the field as directed by the Engineer.
3. Slipforming of the parapets is not allowed.

INDEX OF SHEETS

- S1 General Plan and Elevation
- S2 General Data
- S3 Substructure Layout
- S4 Stage Construction Details and Temp. Soil Retention System
- S5 Temporary Concrete Barrier for Stage Construction
- S6 Top of Deck
- S7 Top of Deck Elevations I
- S8 Top of Deck Elevations II
- S9 Top of Deck Elevations III
- S10 Top of North Approach Slab Elevations
- S11 Top of South Approach Slab Elevations
- S12 Superstructure Plan and Cross Section
- S13 Superstructure Details
- S14 Diaphragm Details
- S15 Bridge Approach Slab
- S16 Bridge Approach Slab Details
- S17 Framing Plan
- S18 72" PPC Bulb T-Beam
- S19 72" PPC Bulb T-Beam Details
- S20 Abutment Plan and Elevation
- S21 Abutment Details
- S22 HP Pile Details
- S23 Drainage Scupper DS-II
- S24 Bar Splicer Assembly and Mechanical Splicer Details
- S25 Boring Logs

TOTAL BILL OF MATERIAL

ITEMS	UNITS	SUPERSTRUCTURE	SUBSTRUCTURE	TOTAL
Stone RipRap, Class A4	SQ YD		1,660	1,660
Filter Fabric	SQ YD		1,660	1,660
Removal of Existing Structures	EACH			1
Structure Excavation	CU YD		778	778
Concrete Structures	CU YD	26.7	59.3	86.0
Concrete Superstructure	CU YD	342.5		342.5
Bridge Deck Grooving	SQ YD	721		721
Protective Coat	SQ YD	854		854
Furnishing and Erecting Precast Prestressed Concrete Bulb T-Beams 72"	FOOT	605.5		605.5
Reinforcement Bars, Epoxy Coated	POUND	67,200	17,070	84,270
Bar Splicers	EACH	470	100	570
Furnishing Steel Piles HP12x84	FOOT		750	750
Driving Piles	FOOT		750	750
Test Pile Steel HP12x84	EACH		2	2
Name Plates	EACH	1		1
Geocomposite Wall Drain	SQ YD		136	136
Granular Backfill for Structures	CU YD		392.4	392.4
Drainage Scuppers, DS-II	EACH	6		6
Pipe Underdrains for Structures 4"	FOOT		196	196
Temporary Soil Retention System	SQ FT		208	208



Note:

All drainage system components shall extend to 2'-0" from the end of each wingwall except an outlet pipe shall extend until intersecting with the side slopes. The pipes shall drain into concrete headwalls. (See Article 601.05 of the Standard Specifications and Highway Standard 60110).

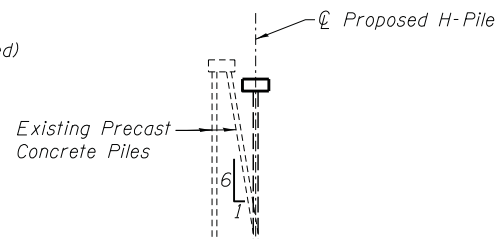
* Included in the cost of Pipe Underdrain for Structures. (See Special Provisions).

SECTION THRU INTEGRAL ABUTMENT
(Horiz. dim. @ Rt. L's)

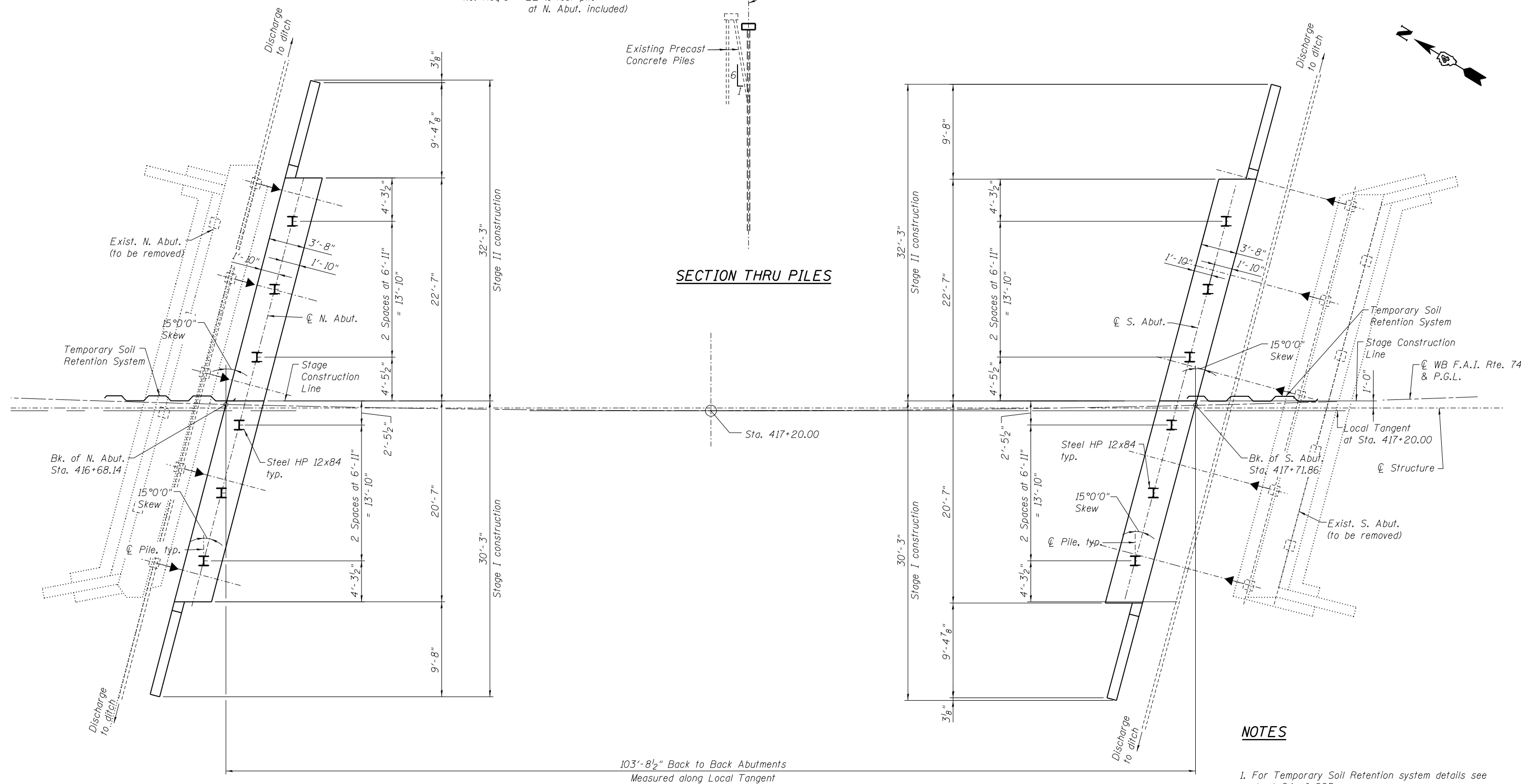
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PLOT DATE = 7/30/2014	CHECKED PC	REVISED -				CONTRACT NO. 68B85				
	DATE 05 03 2013	REVISED -				SHEET NO. S2 OF S25 SHEETS				
						ILLINOIS FED. AID PROJECT				

EXISTING PILE DATA

Type - Concrete Piles
 Capacity - 25 Ton Min.
 Est. Length - 35' Feet
 No. Req'd - 22 (1 test pile at N. Abut. included)



SECTION THRU PILES



103'-8 1/2" Back to Back Abutments
 Measured along Local Tangent

FOUNDATION PLAN

NOTES

1. For Temporary Soil Retention system details see sheet S4 of S25.
2. For Abutment details see sheet S20 of S25.
3. For Steel HP Piles details see sheet S22 of S25.
4. Drive one test pile at each abutment.

FILE NAME = ...\\0480091-68084-003-Footing Plan.dgn	DESIGNED <i>EV</i>	REVISED -
PLOT TIME = 4:31:48 PM	DRAWN <i>JCP</i>	REVISED -
PLOT DATE = 7/30/2014	CHECKED <i>PC</i>	REVISED -
	DATE <i>05 03 2013</i>	REVISED -

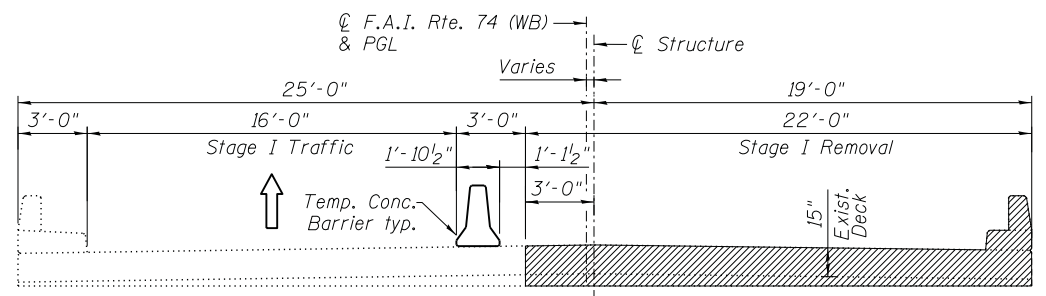
SEPSTEIN
 800 WEST FULTON STREET
 CHICAGO, ILLINOIS 60611-1259
 TEL 312 454 9100
 FAX 312 559 1217
 WEB www.sepstein.com

**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

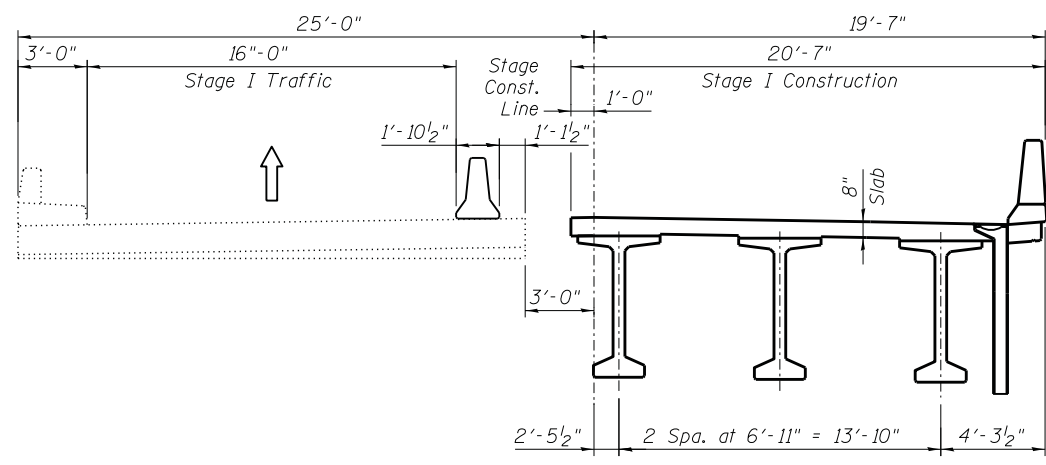
**SUBSTRUCTURE LAYOUT
 STRUCTURE NO. 048-0091**

SHEET NO. S3 OF S25 SHEETS

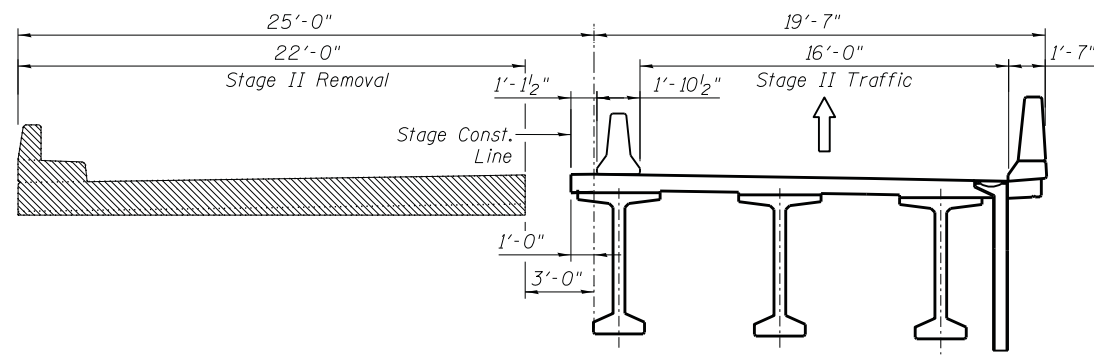
F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
74	48-(25B)BR, BR-11	KNOX	131	49
CONTRACT NO. 68B85				
ILLINOIS FED. AID PROJECT				



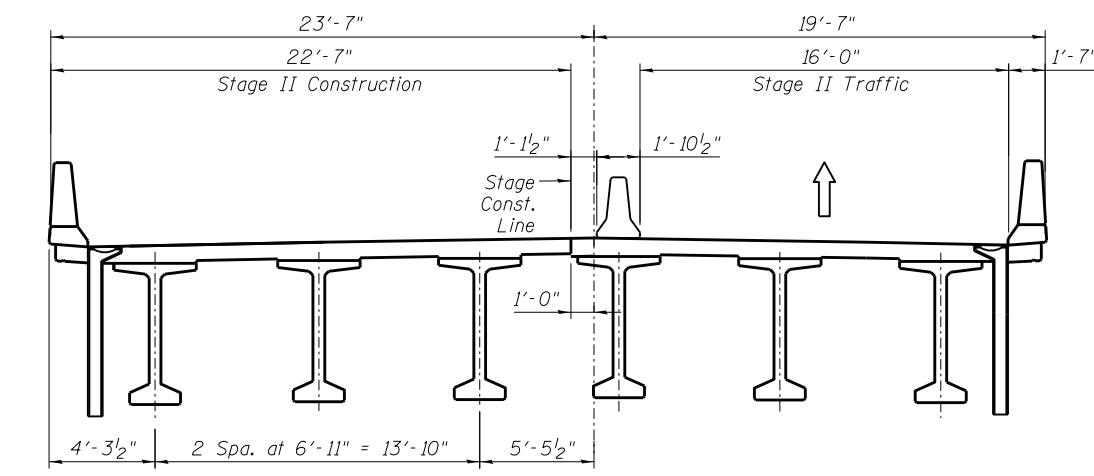
STAGE I REMOVAL



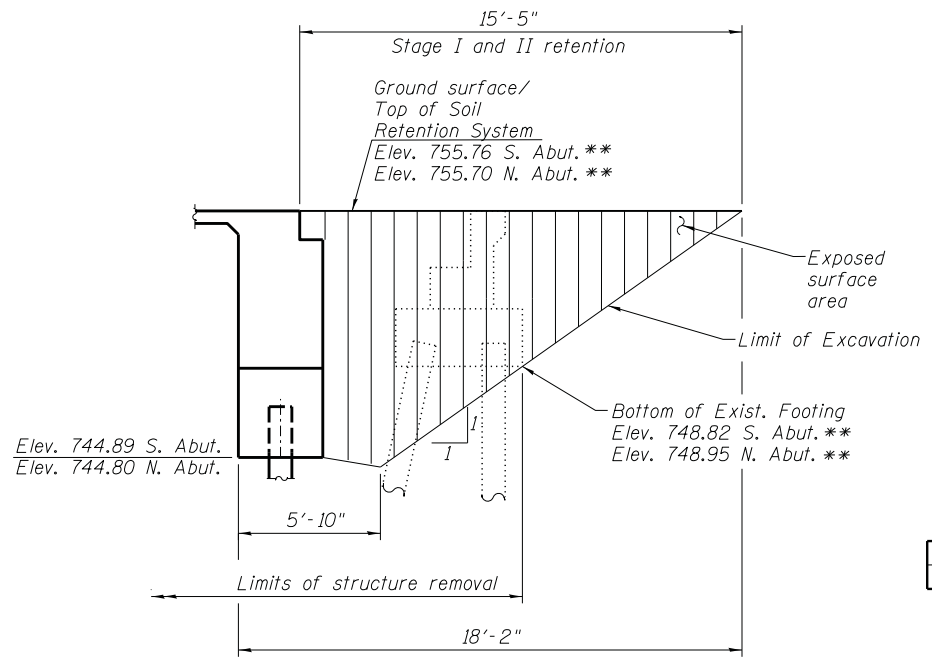
STAGE I CONSTRUCTION



STAGE II REMOVAL



STAGE II CONSTRUCTION



TEMPORARY SOIL RETENTION SYSTEM

**To be verified in field by the Contractor

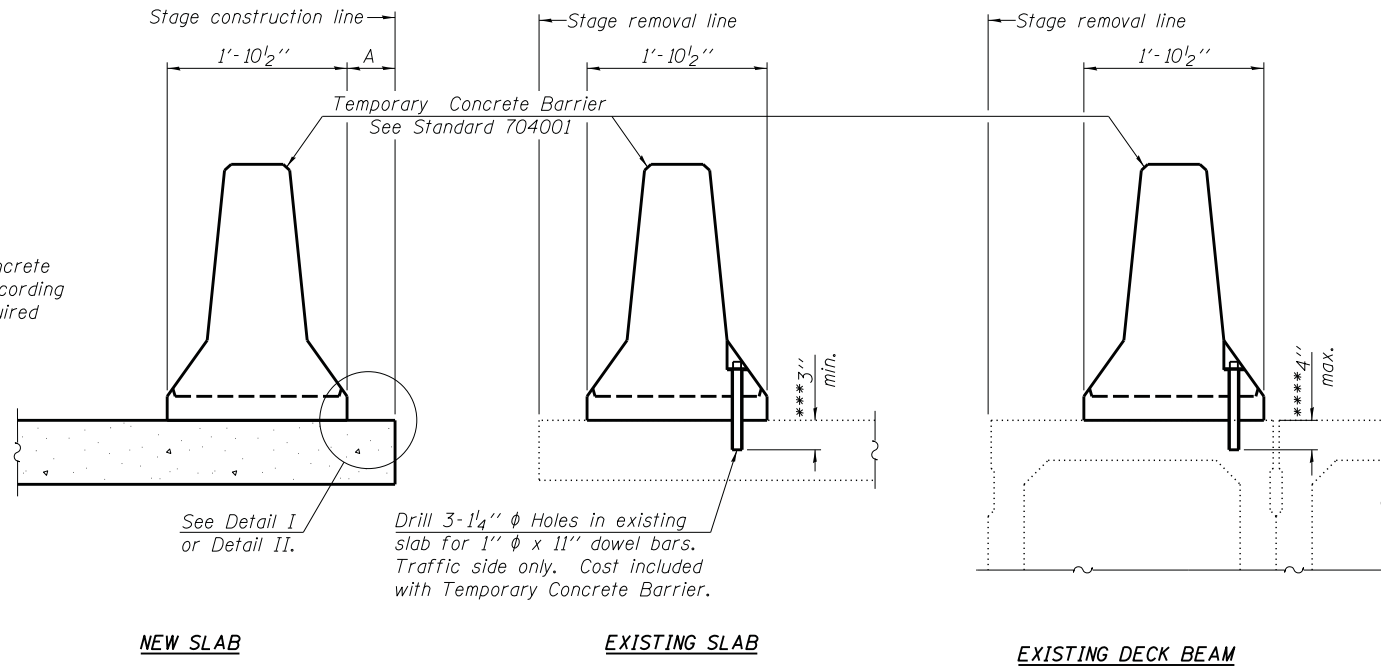
BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Temporary Soil Retention System	Sq. Ft.	208

NOTES

1. All stage cross sections are looking South.
2. See sheet S5 of S25 for Temporary Concrete Barrier.
3. Backfill shall be placed behind the abutment after the superstructure has been poured and false work removed. See Article 502.10 of the Standard Specifications.
4. A cantilevered sheet piling design does not appear feasible and additional members or other retention systems may be necessary. The Contractor shall submit a temporary soil retention system design including plan details and calculations for review and acceptance by the Engineer.
5. Hatched area indicates Removal of Existing Structures.

When "A" is 3'-6" or less, the temporary concrete barrier shall be anchored to the new slab according to Detail I or Detail II. No anchorage is required when "A" is greater than 3'-6".



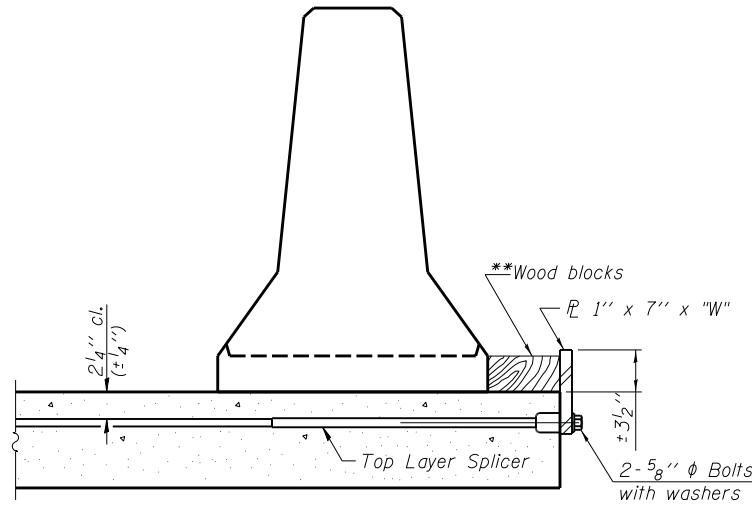
SECTIONS THRU SLAB OR DECK BEAM

NOTES

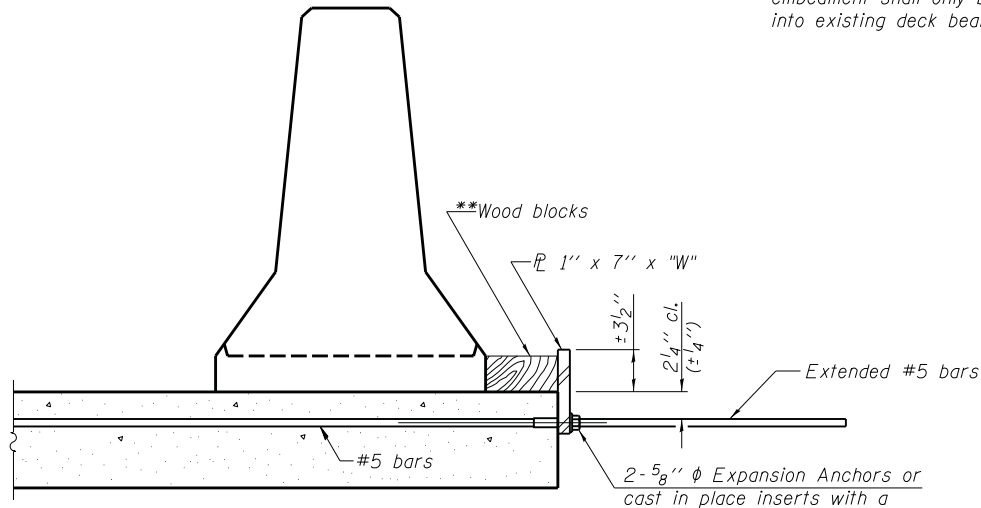
1. Detail I - With Bar Splicer or Couplers:
Connect one (1) 1" x 7" x "W" steel \bar{L} to the top layer of couplers with 2- $\frac{5}{8}$ " ϕ bolts screwed to coupler at approximate \bar{C} of each barrier panel.
2. Detail II - With Extended Reinforcement Bars:
Connect one (1) 1" x 7" x "W" steel \bar{L} to the concrete slab or concrete wearing surface with 2- $\frac{5}{8}$ " ϕ Expansion Anchors or cast in place inserts spaced between the top layer of reinforcement at approximate \bar{C} of each barrier panel.
3. Cost of anchorage is included with Temporary Concrete Barrier.
4. The 1" x 7" x "W" plate shall not be removed until stage II construction forms and all reinforcement bars are in place and the concrete is ready to be placed.

*** Dimension shown is minimum required embedment into concrete.
If hot-mix asphalt wearing surface is present, minimum embedment shall be in addition to wearing surface depth.

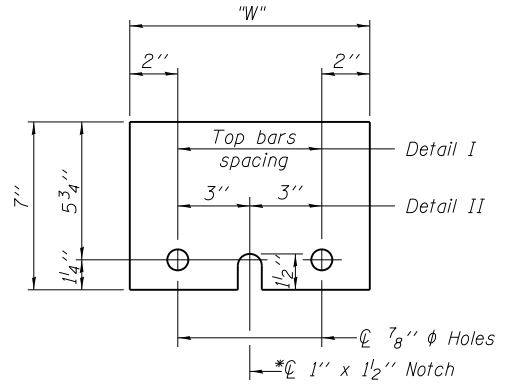
**** If existing deck beam is to remain in place after stage construction, embedment shall only be into wearing surface and not into existing deck beam concrete.



DETAIL I



DETAIL II



STEEL RETAINER 1" x 7" x "W"

* Required only with Detail II

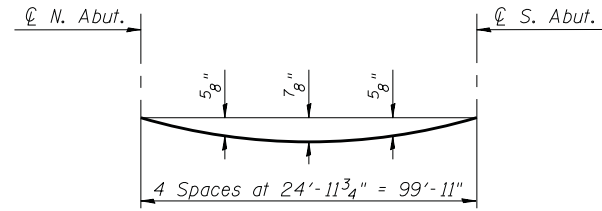
** Wood blocks may be omitted when required to provide minimum stage traffic lane width. When the wood blocks are omitted, the concrete barrier shall be in direct contact with the steel retainer plate.

"W" = Top bars spacing + 4"

R-27

7-1-10

FILE NAME = ... \0480091-68084-005-Temp.conc.barrier.dgn	DESIGNED EV	REVISED -	 800 WEST FULTON STREET CHICAGO, ILLINOIS 60611-1259 TEL 312 454 9100 FAX 312 555 1217 WEB www.sepstein.com	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	TEMPORARY CONCRETE BARRIER FOR STAGE CONSTRUCTION STRUCTURE NO. 048-0091	F.A.I. R.T.E.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
PLOT TIME = 4:31:50 PM	DRAWN JCP	REVISED -				74	48-(125B)BR, BR-1J	KNOX	131	51
PLOT DATE = 7/30/2014	CHECKED PC	REVISED -				CONTRACT NO. 68B85				
	DATE 05 03 2013	REVISED -				SHEET NO. S5 OF S25 SHEETS				
						ILLINOIS FED. AID PROJECT				

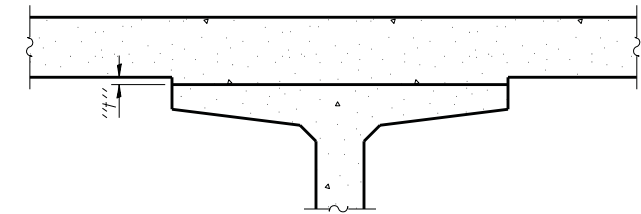


DEAD LOAD DEFLECTION DIAGRAM

(Includes weight of concrete and barrier, excluding beams).

Note:

The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections as shown below and on sheets S8 and S9.



To determine "t": After all precast prestressed beams have been erected, elevations of the top flanges of the beams shall be taken at intervals shown on sheet S6. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflections" shown below and on sheets S8 and S9, minus slab thickness, equals the fillet heights "t" above top flanges of beams.

FILLET HEIGHTS

BEAM 1

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
BK N. ABUT.	416+73.30	19.26	755.35	755.35
CL N. ABUT.	416+75.20	19.27	755.35	755.35
A	416+85.20	19.30	755.32	755.34
B	416+95.20	19.32	755.30	755.35
C	417+05.20	19.34	755.30	755.35
D	417+15.20	19.35	755.29	755.36
E	417+25.20	19.35	755.30	755.38
F	417+35.20	19.34	755.31	755.38
G	417+45.20	19.32	755.33	755.39
H	417+55.20	19.30	755.36	755.40
I	417+65.20	19.27	755.39	755.42
CL BRG. S. ABUT	417+75.10	19.22	755.44	755.44
BK S. ABUT	417+77.00	19.22	755.44	755.44

BEAM 2

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
BK N. ABUT.	416+71.45	12.34	755.50	755.50
CL N. ABUT.	416+73.34	12.34	755.50	755.50
A	416+83.34	12.38	755.47	755.49
B	416+93.34	12.40	755.45	755.49
C	417+03.34	12.42	755.44	755.50
D	417+13.34	12.43	755.44	755.50
E	417+23.34	12.43	755.44	755.52
F	417+33.34	12.42	755.45	755.52
G	417+43.34	12.41	755.47	755.53
H	417+53.34	12.39	755.50	755.54
I	417+63.34	12.36	755.53	755.55
CL BRG. S. ABUT	417+73.25	12.32	755.57	755.57
BK S. ABUT	417+75.15	12.31	755.58	755.58

EAST EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
BK N. ABUT.	416+71.34	11.96	755.51	755.51
CL N. ABUT.	416+73.25	11.97	755.50	755.50
A	416+83.25	12.00	755.48	755.50
B	416+93.25	12.03	755.46	755.50
C	417+03.25	12.04	755.45	755.51
D	417+13.25	12.05	755.45	755.51
E	417+23.25	12.06	755.45	755.53
F	417+33.25	12.05	755.46	755.53
G	417+43.25	12.03	755.48	755.54
H	417+53.25	12.01	755.51	755.55
I	417+63.25	11.98	755.54	755.56
CL BRG. S. ABUT	417+73.15	11.94	755.58	755.58
BK S. ABUT	417+75.04	11.93	755.59	755.59

BEAM 3

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
BK N. ABUT.	416+69.59	5.41	755.62	755.62
CL N. ABUT.	416+71.49	5.42	755.61	755.61
A	416+81.49	5.46	755.58	755.61
B	416+91.49	5.48	755.56	755.61
C	417+01.49	5.50	755.55	755.61
D	417+11.49	5.51	755.55	755.62
E	417+21.49	5.52	755.55	755.63
F	417+31.49	5.51	755.56	755.63
G	417+41.49	5.50	755.58	755.64
H	417+51.49	5.48	755.60	755.65
I	417+61.49	5.45	755.63	755.66
CL BRG. S. ABUT	417+71.40	5.41	755.67	755.67
BK S. ABUT	417+73.30	5.40	755.68	755.68

STAGE CONSTRUCTION LINE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
BK N. ABUT.	416+68.39	0.95	755.69	755.69
CL N. ABUT.	416+70.30	0.95	755.68	755.68
A	416+80.30	0.99	755.66	755.68
B	416+90.30	1.02	755.64	755.68
C	417+00.30	1.04	755.62	755.68
D	417+10.30	1.05	755.62	755.69
E	417+20.30	1.06	755.62	755.70
F	417+30.30	1.05	755.63	755.70
G	417+40.30	1.04	755.65	755.70
H	417+50.30	1.02	755.67	755.71
I	417+60.30	0.99	755.70	755.72
CL BRG. S. ABUT	417+70.20	0.95	755.74	755.74
BK S. ABUT	417+72.10	0.95	755.75	755.75

☉ ROADWAY, PROFILE GRADE LINE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
BK N. ABUT.	416+68.14	0.00	755.71	755.71
CL N. ABUT.	416+70.04	0.00	755.70	755.70
A	416+80.04	0.00	755.67	755.69
B	416+90.04	0.00	755.65	755.70
C	417+00.04	0.00	755.64	755.70
D	417+10.04	0.00	755.63	755.70
E	417+20.04	0.00	755.64	755.71
F	417+30.04	0.00	755.64	755.71
G	417+40.04	0.00	755.66	755.72
H	417+50.04	0.00	755.68	755.73
I	417+60.04	0.00	755.71	755.74
CL BRG. S. ABUT	417+69.95	0.00	755.75	755.75
BK S. ABUT	417+71.85	0.00	755.76	755.76

BEAM 4

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
BK N. ABUT.	416+67.74	-1.51	755.68	755.68
CL N. ABUT.	416+69.64	-1.50	755.68	755.68
A	416+79.64	-1.47	755.65	755.67
B	416+89.64	-1.44	755.63	755.67
C	416+99.64	-1.42	755.62	755.68
D	417+09.64	-1.41	755.61	755.68
E	417+19.64	-1.40	755.61	755.69
F	417+29.64	-1.41	755.62	755.69
G	417+39.64	-1.42	755.64	755.70
H	417+49.64	-1.44	755.66	755.70
I	417+59.64	-1.47	755.69	755.71
CL BRG. S. ABUT	417+69.55	-1.50	755.73	755.73
BK S. ABUT	417+71.44	-1.51	755.73	755.73

WEST EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
BK N. ABUT.	416+65.88	-8.44	755.58	755.58
CL N. ABUT.	416+67.78	-8.43	755.58	755.58
A	416+77.78	-8.39	755.55	755.57
B	416+87.78	-8.36	755.53	755.57
C	416+97.78	-8.34	755.51	755.57
D	417+07.78	-8.32	755.50	755.57
E	417+17.78	-8.32	755.50	755.58
F	417+27.78	-8.32	755.51	755.58
G	417+37.78	-8.33	755.53	755.58
H	417+47.78	-8.35	755.55	755.59
I	417+57.78	-8.38	755.58	755.60
CL BRG. S. ABUT	417+67.70	-8.41	755.61	755.61
BK S. ABUT	417+69.59	-8.42	755.62	755.62

BEAM 5

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
BK N. ABUT.	416+64.91	-12.07	755.53	755.53
CL N. ABUT.	416+66.81	-12.06	755.52	755.52
A	416+76.81	-12.02	755.49	755.51
B	416+86.81	-11.99	755.47	755.51
C	416+96.81	-11.96	755.46	755.51
D	417+06.81	-11.95	755.45	755.52
E	417+16.81	-11.94	755.45	755.52
F	417+26.81	-11.95	755.45	755.52
G	417+36.81	-11.96	755.47	755.53
H	417+46.81	-11.97	755.49	755.53
I	417+56.81	-12.00	755.52	755.54
CL BRG. S. ABUT	417+66.73	-12.03	755.55	755.55
BK S. ABUT	417+68.62	-12.04	755.62	755.62

BEAM 6

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection and Grinding
BK N. ABUT.	416+64.02	-15.36	755.46	755.46
CL N. ABUT.	416+65.92	-15.35	755.46	755.46
A	416+75.92	-15.31	755.43	755.45
B	416+85.92	-15.28	755.40	755.45
C	416+95.92	-15.26	755.39	755.45
D	417+05.92	-15.24	755.38	755.45
E	417+15.92	-15.24	755.38	755.46
F	417+25.92	-15.24	755.38	755.45
G	417+35.92	-15.25	755.40	755.46
H	417+45.92	-15.26	755.42	755.46
I	417+55.92	-15.29	755.44	755.47
CL BRG. S. ABUT	417+65.85	-15.32	755.48	755.48
BK S. ABUT	417+67.74	-15.33	755.49	755.49

FILE NAME = ...\\0480091-68084-009-TOS.Elevs3.dgn	DESIGNED <i>EV</i> DRAWN <i>JCP</i>	REVISED - REVISED -	 800 WEST FULTON STREET CHICAGO, ILLINOIS 60611-1299 TEL 312 454 9100 FAX 312 555 1217 WEB www.sepstein.com	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	TOP OF DECK ELEVATIONS III STRUCTURE NO. 048-0091	F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
PLOT TIME = 4:31:55 PM	CHECKED <i>PC</i>	REVISED -				74	48-[125B]BR, BR-1J	KNOX	131	55
PLOT DATE = 7/30/2014	DATE <i>05 03 2013</i>	REVISED -				SHEET NO. S9 OF S25 SHEETS			ILLINOIS FED. AID PROJECT	

EAST EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations
N. End of N. Appr. Slab	416+45.06	21.16	755.42
A1	416+55.06	21.21	755.37
A2	416+65.06	21.93	755.32
S. End of N. Appr. Slab	416+75.06	21.97	755.29

EAST EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
N. End of N. Appr. Slab	416+42.38	11.81	755.63
A1	416+52.38	11.87	755.58
A2	416+62.38	11.92	755.54
S. End of N. Appr. Slab	416+72.38	11.96	755.51

STAGE CONSTRUCTION LINE

Location	Station	Offset	Theoretical Grade Elevations
N. End of N. Appr. Slab	416+39.43	0.85	755.82
A1	416+49.43	0.90	755.77
A2	416+59.43	0.94	755.72
S. End of N. Appr. Slab	416+69.43	0.95	755.69

WB F.A.I. Rte. 74 & P.G.L.

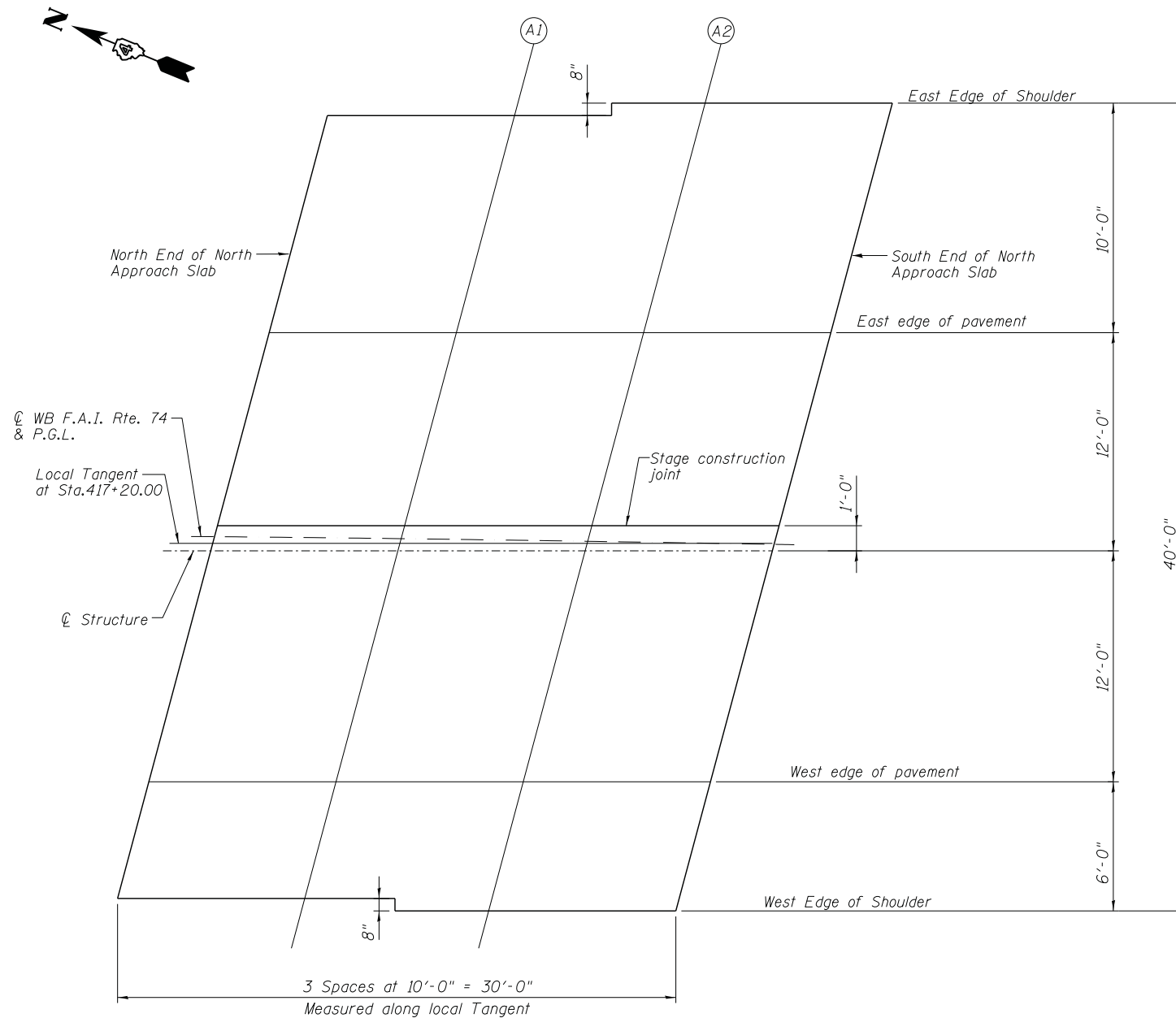
Location	Station	Offset	Theoretical Grade Elevations
N. End of N. Appr. Slab	416+39.18	0.00	755.83
A1	416+49.18	0.00	755.78
A2	416+59.18	0.00	755.73
S. End of N. Appr. Slab	416+69.18	0.00	755.70

WEST EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
N. End of N. Appr. Slab	416+35.94	-12.23	755.66
A1	416+45.94	-12.17	755.61
A2	416+55.94	-12.12	755.57
S. End of N. Appr. Slab	416+65.94	-12.07	755.53

WEST EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations
N. End of N. Appr. Slab	416+34.46	-17.58	755.56
A1	416+44.46	-17.51	755.51
A2	416+54.32	-18.12	755.45
S. End of N. Appr. Slab	416+64.32	-18.07	755.41



PLAN

EAST EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations
N. End of S. Appr. Slab	417+76.69	21.92	755.39
A3	417+86.69	21.87	755.44
A4	417+96.69	21.15	755.50
S. End of S. Appr. Slab	418+06.69	21.08	755.56

EAST EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
N. End of S. Appr. Slab	417+74.01	11.93	755.58
A3	417+84.01	11.89	755.63
A4	417+94.01	11.83	755.68
S. End of S. Appr. Slab	418+04.01	11.76	755.74

STAGE CONSTRUCTION JOINT

Location	Station	Offset	Theoretical Grade Elevations
N. End of S. Appr. Slab	417+71.07	0.95	755.74
A3	417+81.07	0.90	755.79
A4	417+91.07	0.85	755.84
S. End of S. Appr. Slab	418+01.07	0.78	755.89

☉ ROADWAY, PROFILE GRADE LINE

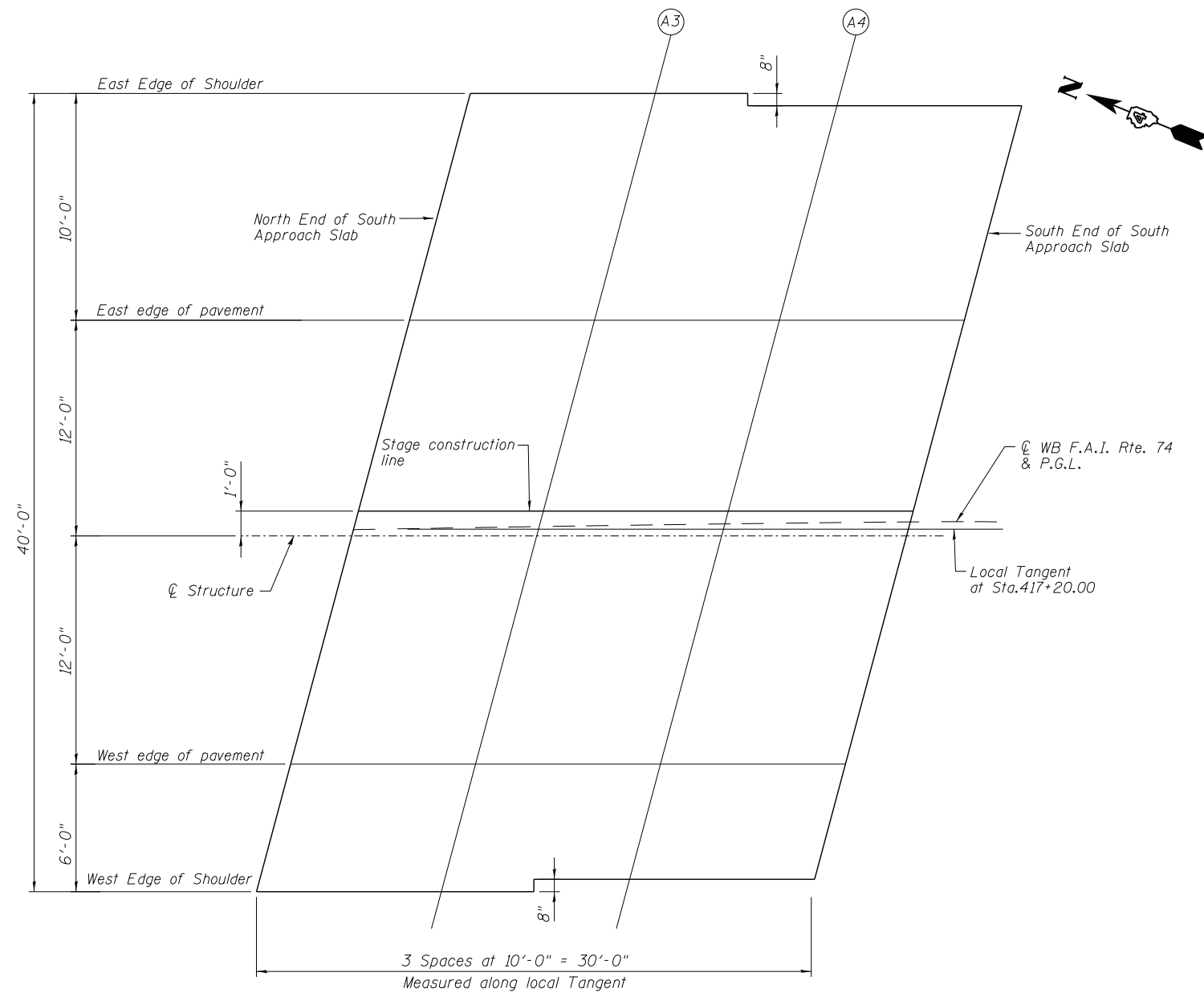
Location	Station	Offset	Theoretical Grade Elevations
N. End of S. Appr. Slab	417+70.81	0.00	755.75
A3	417+80.81	0.00	755.80
A4	417+90.81	0.00	755.85
S. End of S. Appr. Slab	418+00.81	0.00	755.90

WEST EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
N. End of S. Appr. Slab	417+67.59	-12.04	755.56
A3	417+77.59	-12.08	755.60
A4	417+87.59	-12.13	755.65
S. End of S. Appr. Slab	417+97.59	-12.19	755.70

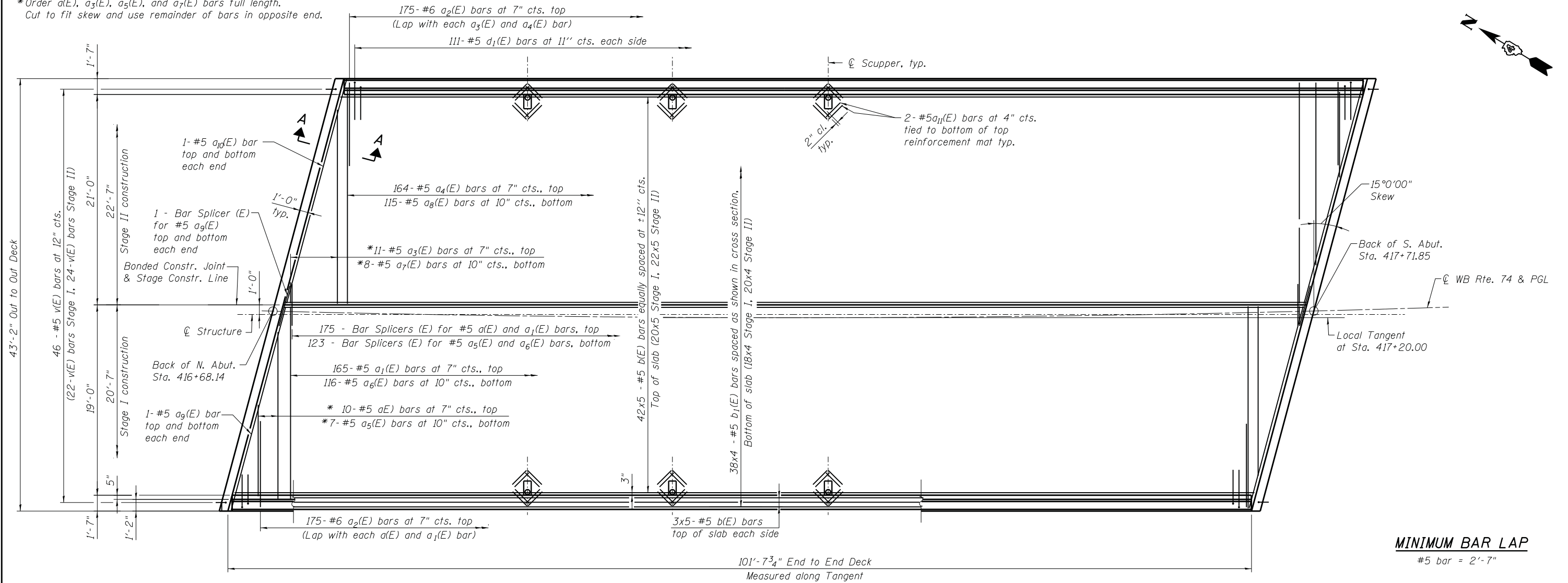
WEST EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations
N. End of S. Appr. Slab	417+65.98	-18.03	755.42
A3	417+75.98	-18.08	755.47
A4	417+85.98	-17.46	755.52
S. End of S. Appr. Slab	417+95.98	-17.52	755.57



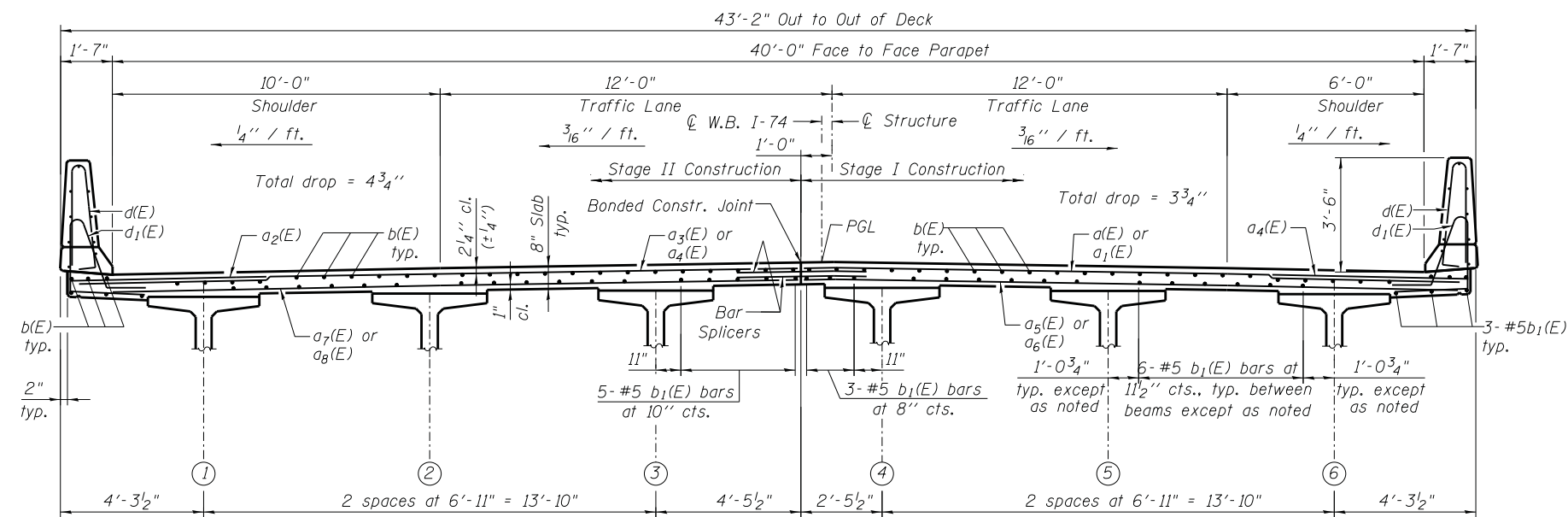
PLAN

*Order a(E), a₃(E), a₅(E), and a₇(E) bars full length.
Cut to fit skew and use remainder of bars in opposite end.



PLAN

MINIMUM BAR LAP
#5 bar = 2'-7"

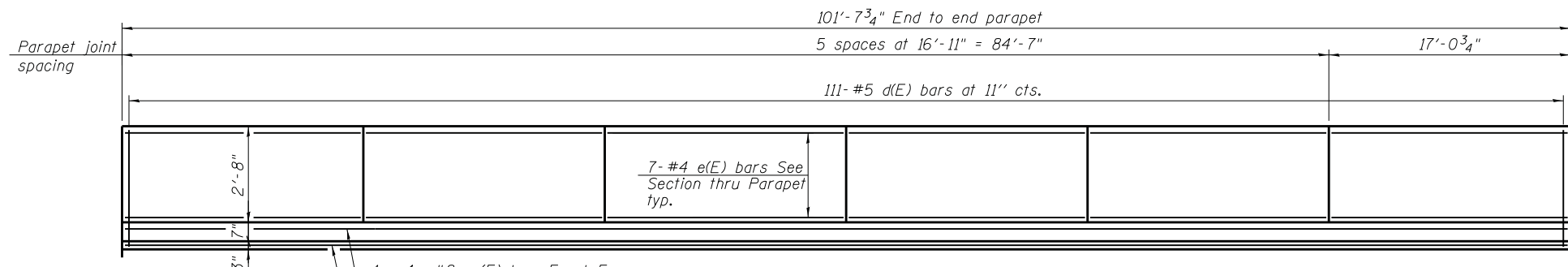


CROSS SECTION
(Looking South)

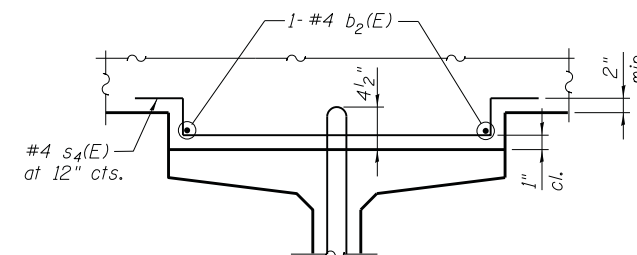
NOTES

1. For Parapet reinforcement, superstructure details and Bill of Material, see sheet S13 of S25.
2. For Section A-A and diaphragm details see sheet S14 of S25.
3. For Bar Splicer (E) details, see sheet S24 of S25.
4. Bars indicated thus 20 x 3-#5 etc. indicates 20 lines of bars with 3 lengths per line.
5. For drainage scuppers location see sheet S1.
6. Cut longitudinal reinforcement to clear drainage scuppers.

FILE NAME = ... \0480091-68084-012-Super.dgn	DESIGNED EV	REVISED -	 800 WEST FULTON STREET CHICAGO, ILLINOIS 60611-1259 TEL. 312.454.9100 FAX 312.555.1217 WEB www.sepstein.com	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION		SUPERSTRUCTURE PLAN AND CROSS SECTION STRUCTURE NO. 048-0091		F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
PLOT TIME = 4:31:59 PM	DRAWN JCP	REVISED -						74	48-(125B)BR, BR-1J	KNOX	131	58
PLOT DATE = 7/30/2014	CHECKED PC	REVISED -						CONTRACT NO. 68B85				
	DATE 05 03 2013	REVISED -						ILLINOIS FED. AID PROJECT				

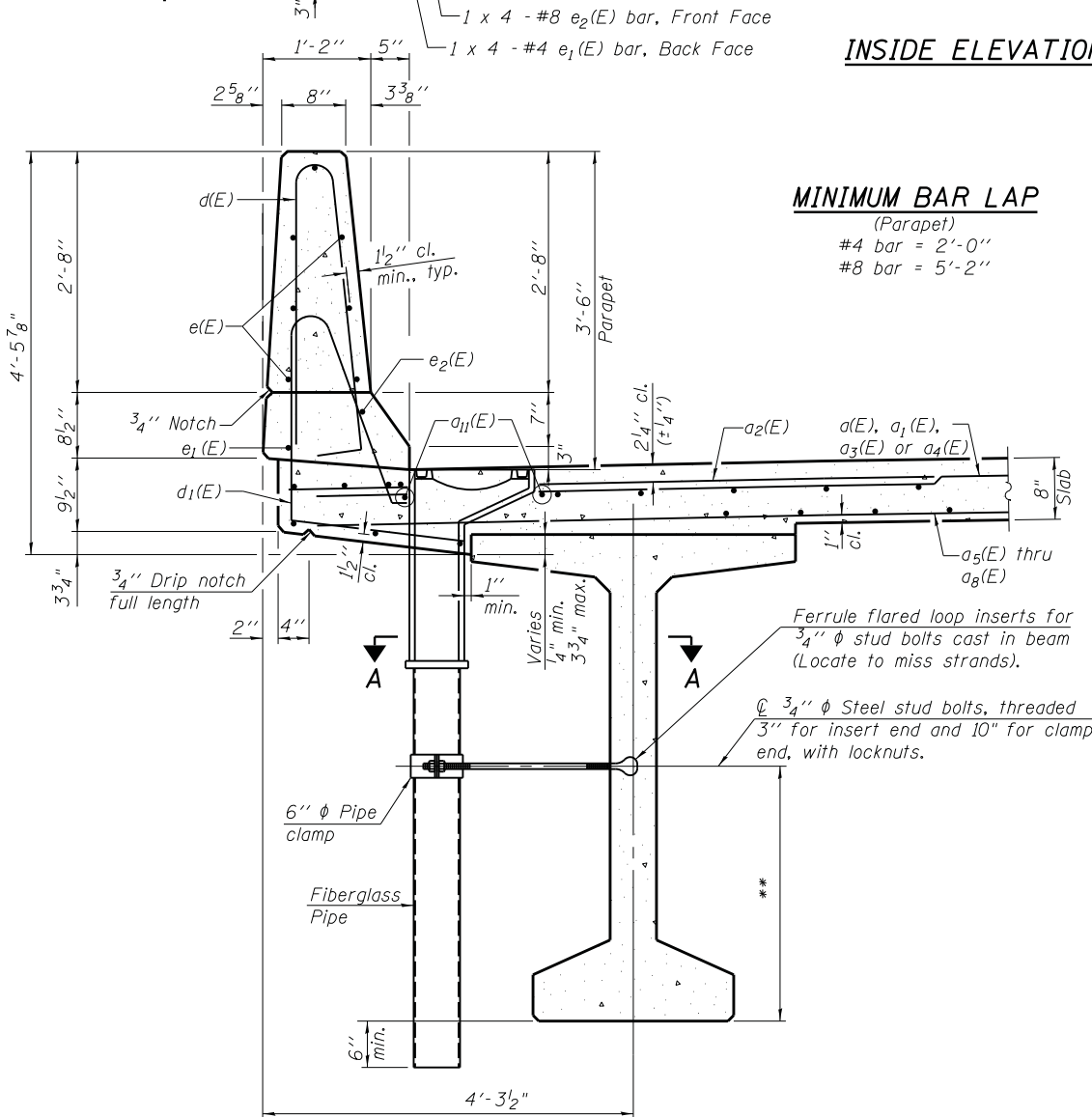


INSIDE ELEVATION OF PARAPET



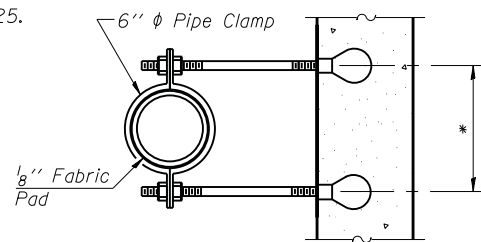
FILLET REINFORCEMENT DETAIL

Bars $b_2(E)$ and $s_4(E)$ should be placed in locations where the fillet exceeds $2\frac{1}{2}$ "



SECTION THRU PARAPET

**For insert locations see sheet S18 of S25.



SECTION A-A

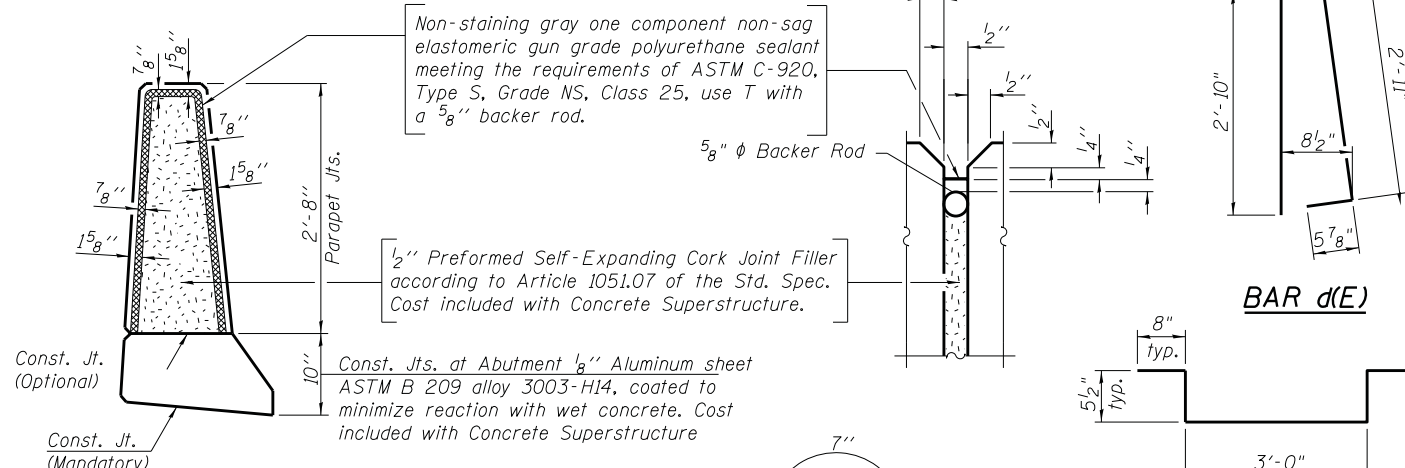
*Dimension as required by Pipe Clamp

MINIMUM BAR LAP

(Parapet)

#4 bar = 2'-0"

#8 bar = 5'-2"



PARAPET JOINT DETAILS

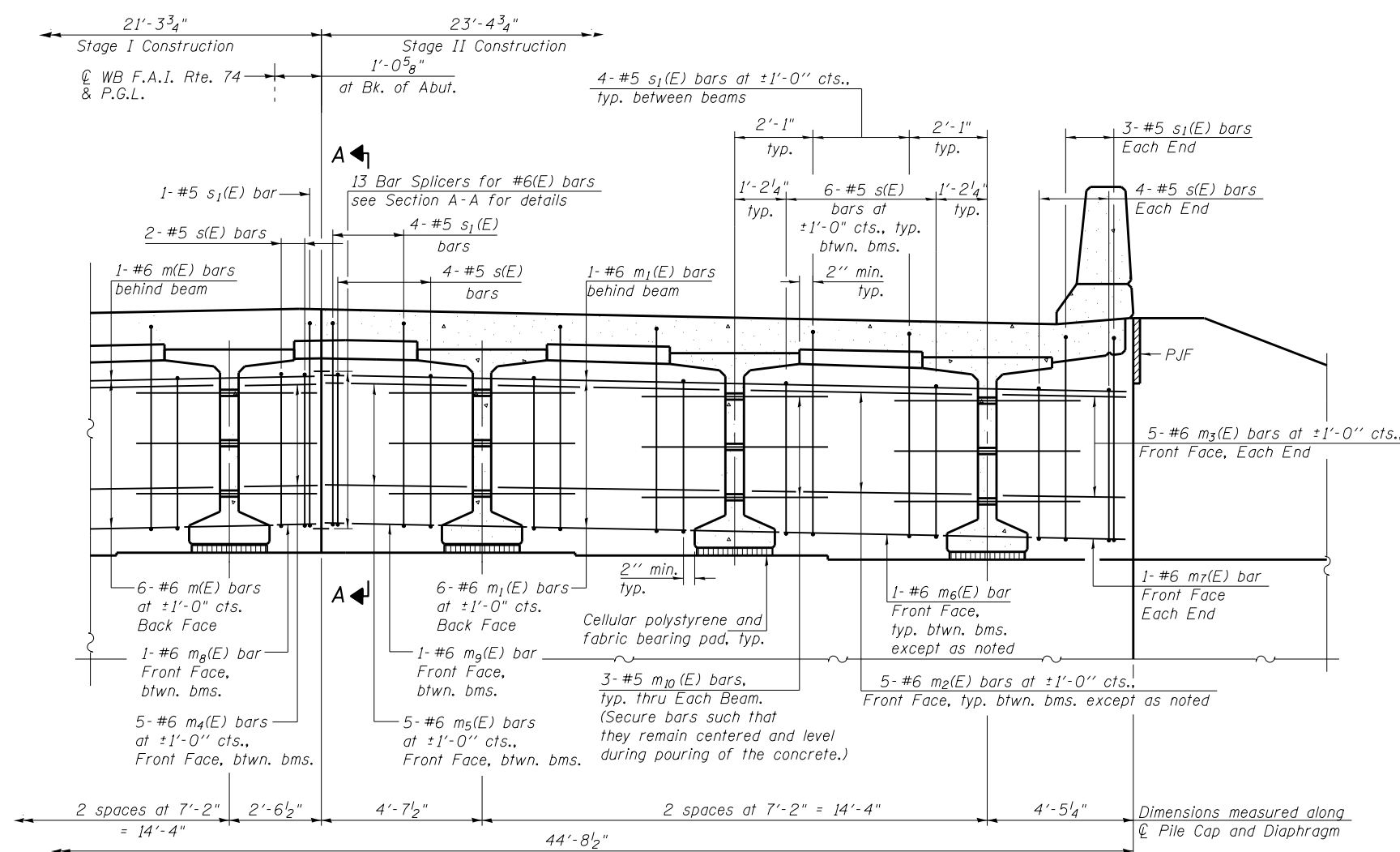
Notes:

1. Fiberglass pipe shall conform to ASTM D2996, with short-time rupture strength hoop tensile stress of 30,000 p.s.i. minimum.
2. The clamping device and inserts shall be galvanized according to AASHTO M 232.
3. Cost of fiberglass pipe and clamping device and inserts included with Drainage Scupper, DS-11.

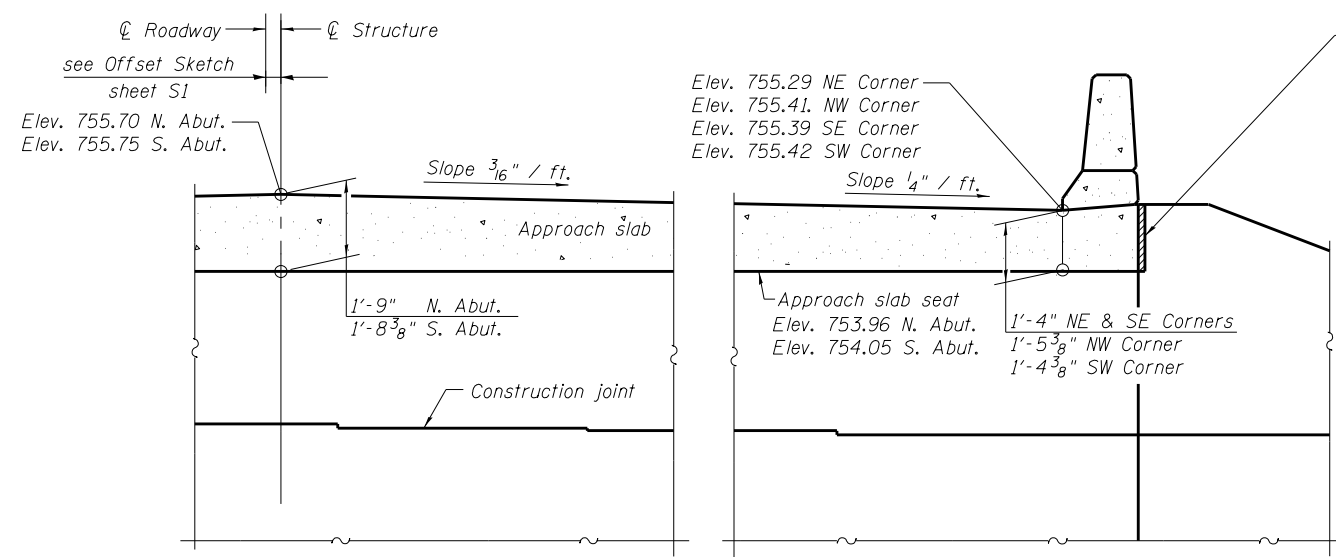
SUPERSTRUCTURE BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a(E)	10	#5	20' - 10"	—
a1(E)	165	#5	20' - 2"	—
a2(E)	350	#6	6' - 6"	—
a3(E)	11	#5	22' - 10"	—
a4(E)	164	#5	22' - 2"	—
a5(E)	7	#5	20' - 9"	—
a6(E)	116	#5	20' - 1"	—
a7(E)	8	#5	22' - 10"	—
a8(E)	115	#5	22' - 3"	—
a9(E)	4	#5	21' - 0"	—
a10(E)	4	#5	23' - 1"	—
a11(E)	48	#5	1' - 6"	—
b(E)	240	#5	22' - 9"	—
b1(E)	152	#5	27' - 9"	—
b2(E)	24	#4	30' - 0"	—
d(E)	222	#5	6' - 11"	—
d1(E)	222	#5	7' - 9"	—
e(E)	84	#4	16' - 8"	—
e1(E)	8	#4	27' - 5"	—
e2(E)	8	#8	29' - 10"	—
m(E)	14	#6	21' - 0"	—
m1(E)	14	#6	23' - 1"	—
m2(E)	40	#6	6' - 4"	—
m3(E)	20	#6	3' - 10"	—
m4(E)	10	#6	1' - 11"	—
m5(E)	10	#6	4' - 0"	—
m6(E)	8	#6	4' - 8"	—
m7(E)	4	#6	3' - 0"	—
m8(E)	2	#6	1' - 1"	—
m9(E)	2	#6	3' - 2"	—
m10(E)	36	#5	4' - 0"	—
s(E)	76	#5	10' - 9"	—
s1(E)	54	#5	17' - 10"	—
s4(E)	372	#4	5' - 3"	—
v(E)	88	#5	3' - 1"	—
Concrete Superstructure		Cu. Yd.		211.4
Bridge Deck Grooving		Sq. Yd.		452
Protective Coat		Sq. Yd.		550
Reinforcement Bars, Epoxy Coated		Pound		37,750

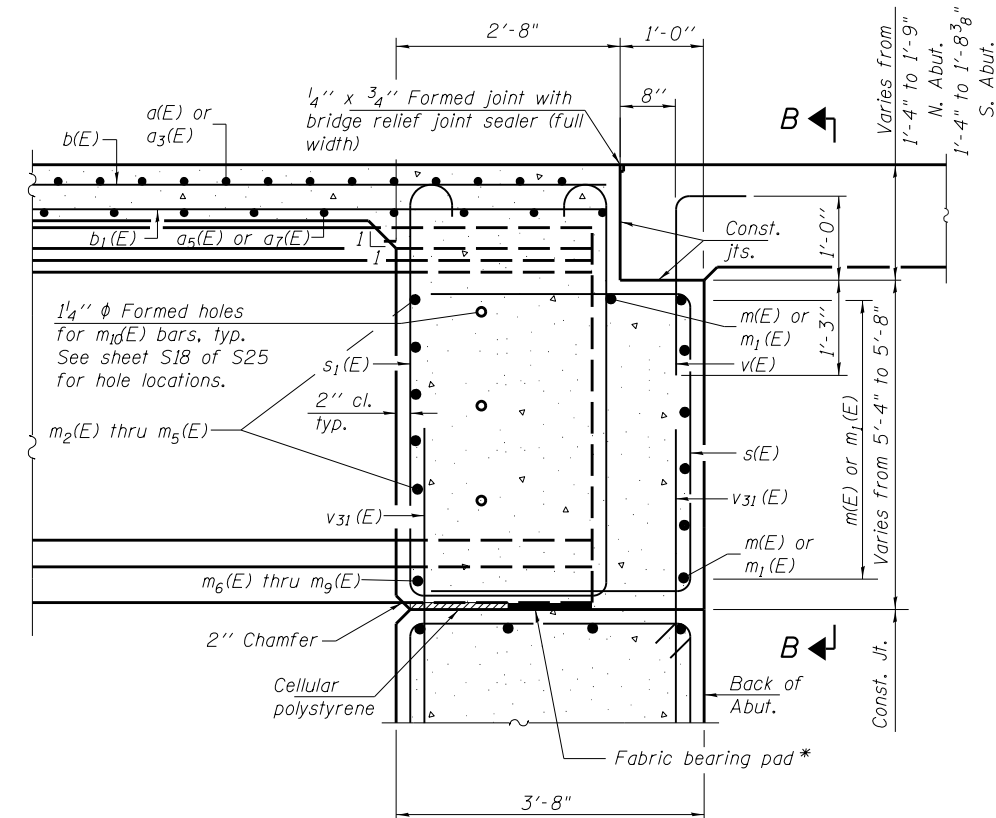
Bars indicated thus 1 x 4 - #8 etc. indicates 1 line of bars with 4 lengths per line.



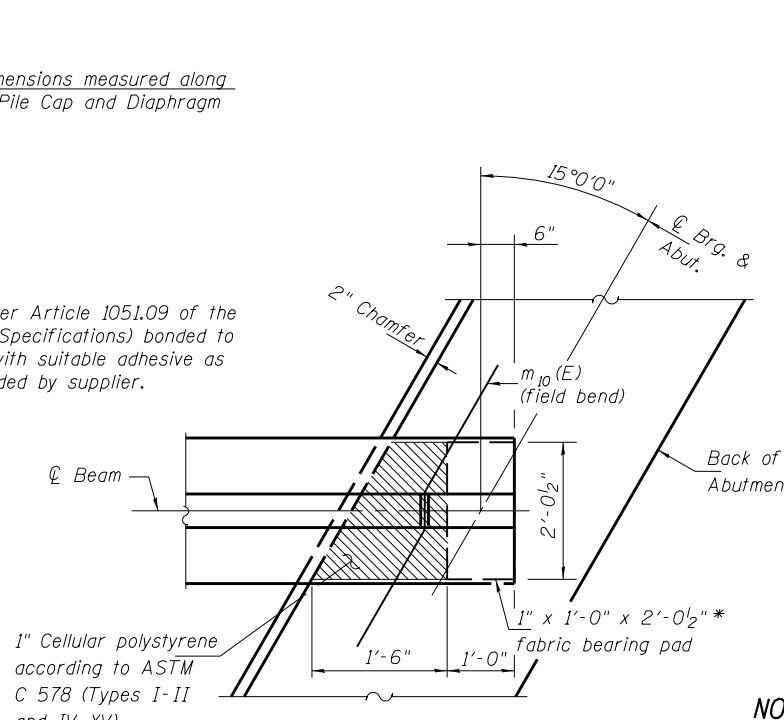
DIAPHRAGM ELEVATION AT ABUTMENT



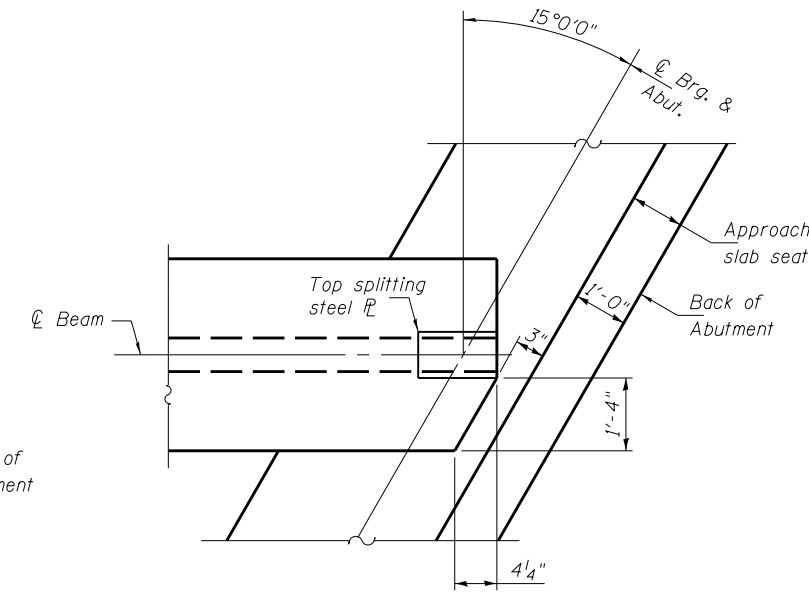
SECTION B-B



SECTION A-A



PARTIAL PLAN AT ABUTMENT
(Showing bottom flange of beam)



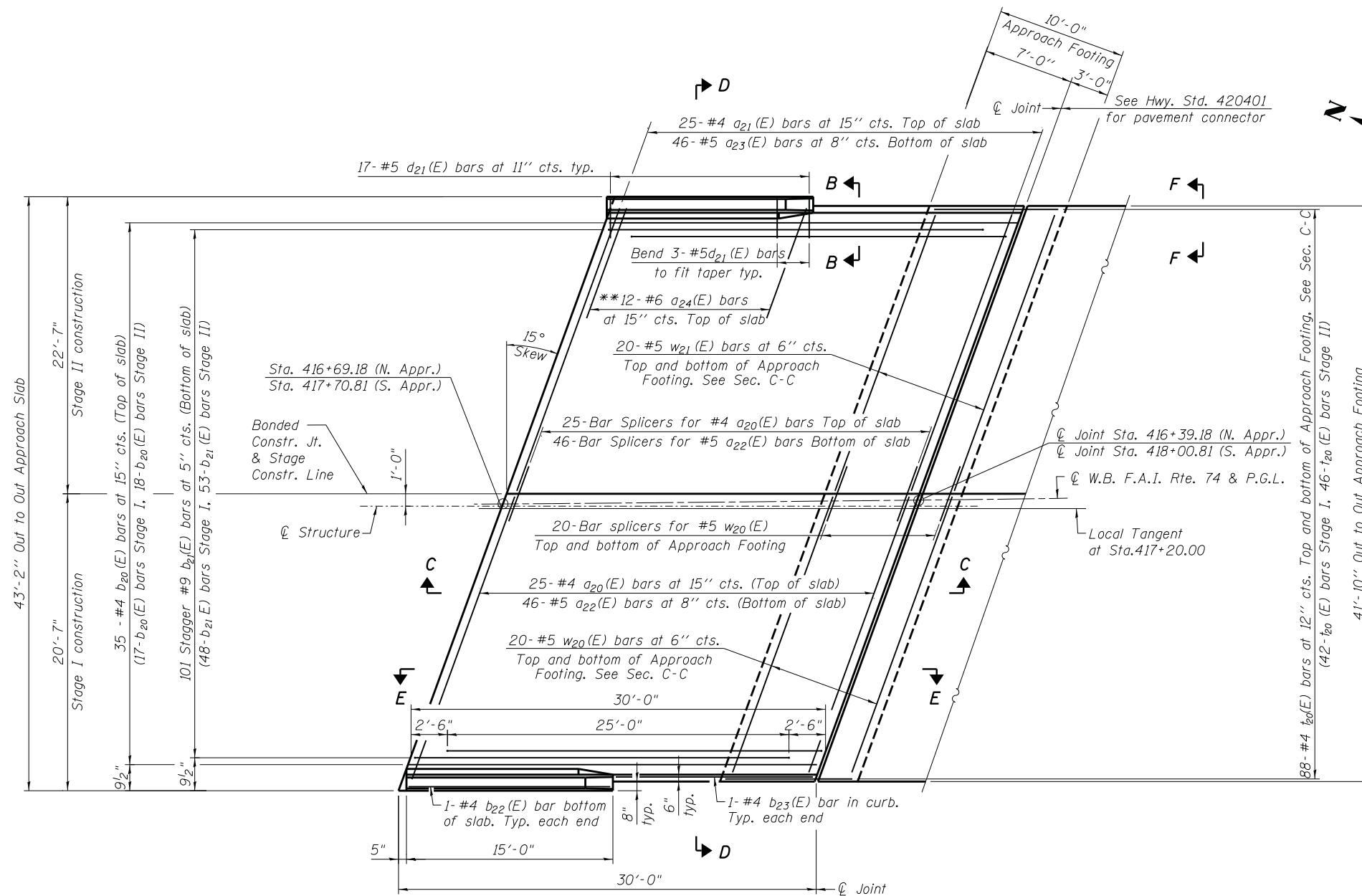
TOP FLANGE PLAN
(Showing top flange of beam)

NOTES

1. Reinforcement bars in diaphragm are billed with superstructure on sheet S13 of S25.
2. Concrete in diaphragm is included with Concrete Superstructure on sheet S13 of S25.
3. For details of bars s(E), s1(E) and v(E) see sheet S13 of S25.
4. The approach slab seat shall have a constant slope determined from the control points shown.
5. Cost of cellular polystyrene is included with Concrete Superstructure.

*Cost of fabric bearing pad is included with Concrete Superstructure

FILE NAME = ... \0480091-68084-014-Diaphragm.dgn	DESIGNED EV	REVISED -	 800 WEST FULTON STREET CHICAGO, ILLINOIS 60611-1209 TEL 312 454 9100 FAX 312 555 1217 WEB www.sepstein.com	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	DIAPHRAGM DETAILS STRUCTURE NO. 048-0091	F.A.I. R.T.E. 74	SECTION 48-(25)BR, BR-1J	COUNTY KNOX	TOTAL SHEETS 131	SHEET NO. 60
PLOT TIME = 9:32:31 AM	DRAWN JCP	REVISED -				CONTRACT NO. 68B85				
PLOT DATE = 10/1/2014	CHECKED PC	REVISED -				ILLINOIS FED. AID PROJECT				
	DATE 05 03 2013	REVISED -				SHEET NO. S14 OF S25 SHEETS				



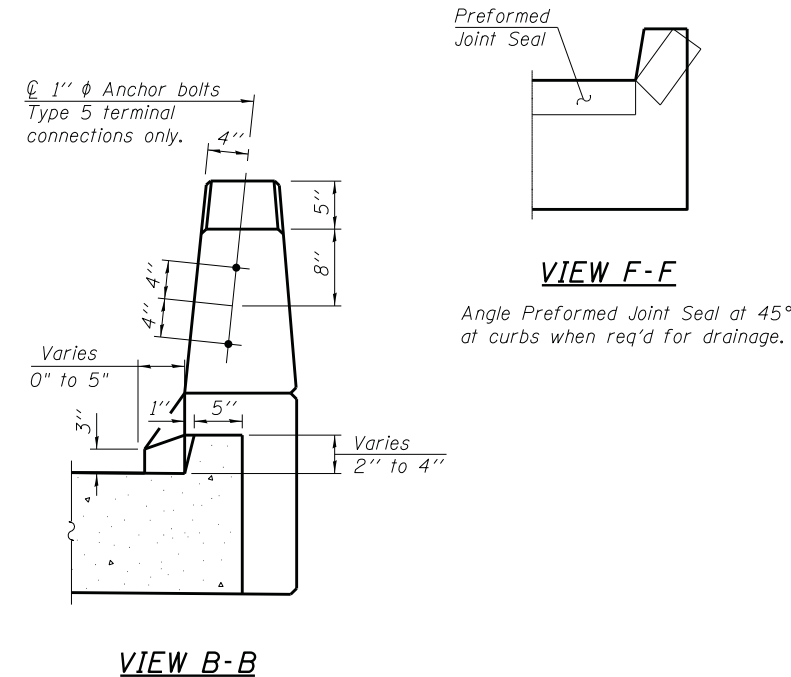
PLAN

South Approach slab is shown,
North Approach slab is similar

- * Tilt #9 b₂₁(E) bars as required to maintain clearance.
- ** Space between a₂₀(E) and a₂₁(E) bars, typ. ea. parapet.

NOTES

1. For Sections C-C & D-D, View E-E, bar bending diagrams and Bill of Materials see sheet S16 of S25.
2. For Bar Splicer (E) details see sheet S24 of S25.
3. a₂₀(E) thru a₂₃(E) bar spacings measured along \perp Rdwy.



BA-L 12-12-12

FILE NAME =	DESIGNED EV	REVISED -
... \0480091-68084-015-ApprSlab.dgn	DRAWN JCP	REVISED -
PLOT TIME = 4:32:03 PM	CHECKED PC	REVISED -
PLOT DATE = 7/30/2014	DATE 05 03 2013	REVISED -

SEPSTEIN
 800 WEST FULTON STREET
 CHICAGO, ILLINOIS 60611-1259
 TEL 312 454 9100
 FAX 312 555 1217
 WEB www.sepstein.com

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

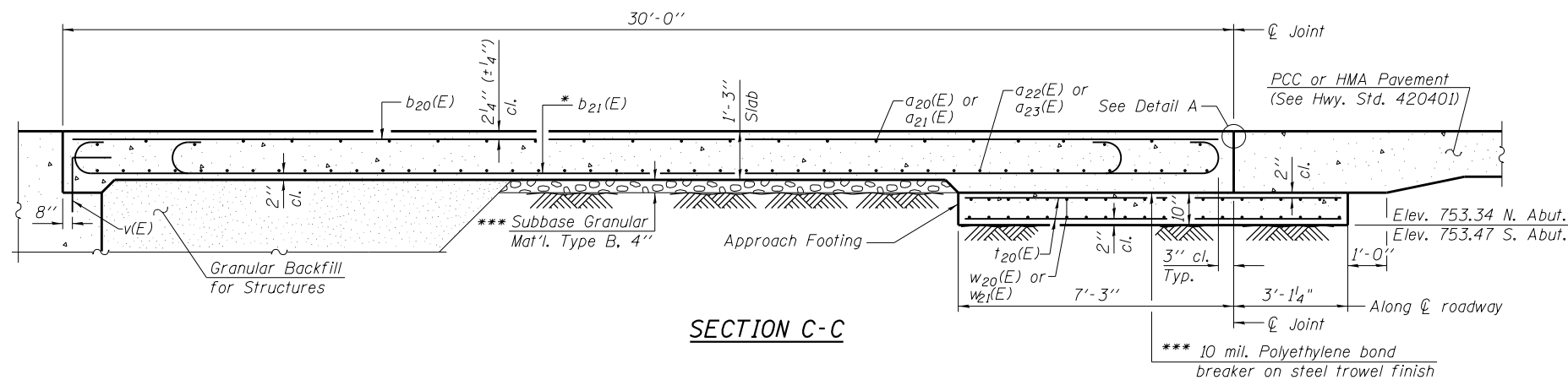
**BRIDGE APPROACH SLAB
STRUCTURE NO. 048-0091**

SHEET NO. S15 OF S25 SHEETS

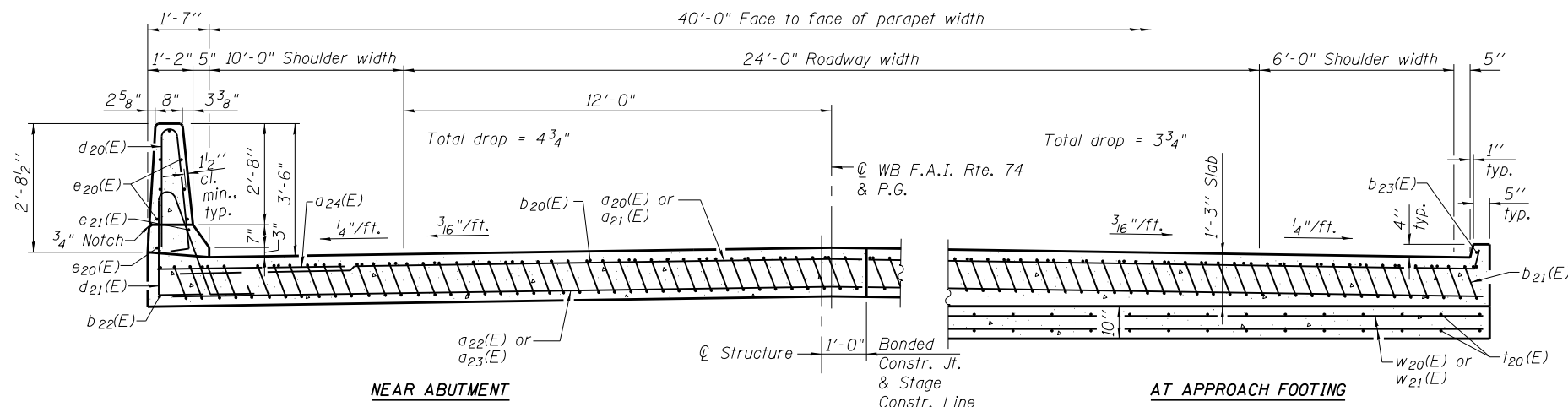
F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
74	48-[25B]BR, BR-1J	KNOX	131	61
CONTRACT NO. 68B85				
ILLINOIS FED. AID PROJECT				

NOTES

1. Approach slab and parapet concrete shall be paid for as Concrete Superstructure.
2. Approach footing concrete shall be paid for as Concrete Structures.
3. Reinforcement shall be paid for as Reinforcement Bars, Epoxy Coated.
4. For v(E) bar details, see sheet S13 of S25.
5. The approach footing maximum applied service bearing pressure (Qmax) = 2.0 ksf.
6. For bar splicer details, see sheet S24 of S25.
7. Cost of excavation for approach footing included with Concrete Structures.
8. For Granular Backfill for Structures and drainage treatment details, see sheet S2 of S25.
9. For additional parapet details, see sheet S13 of S25.

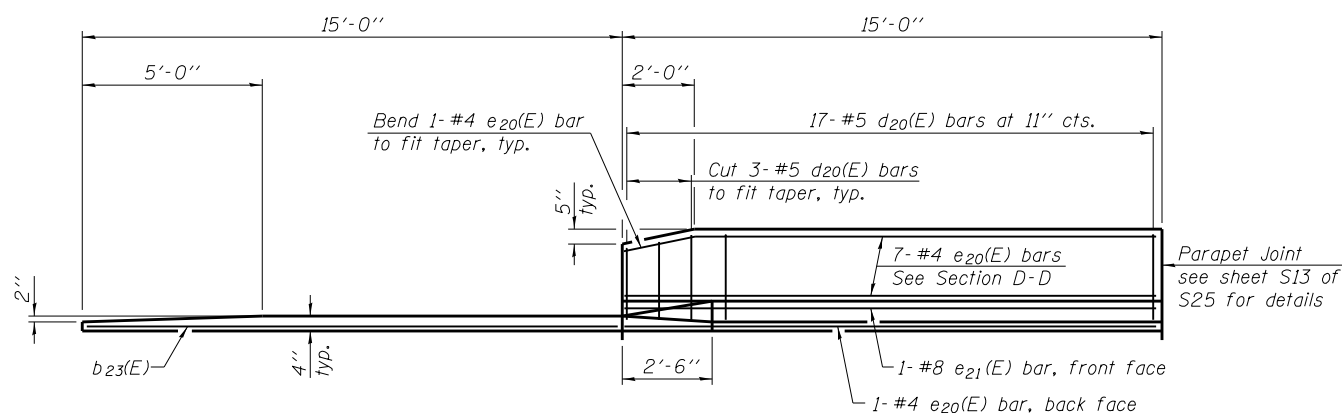


SECTION C-C

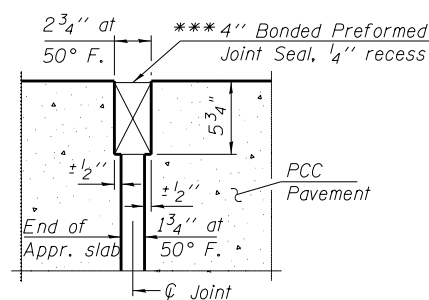


SECTION D-D

(See Plan for dimensions not shown)

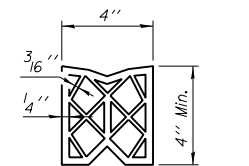


VIEW E-E

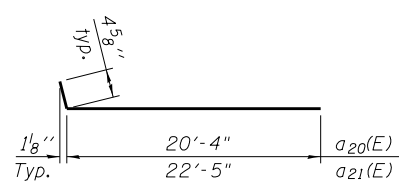


RIGID PAVEMENT

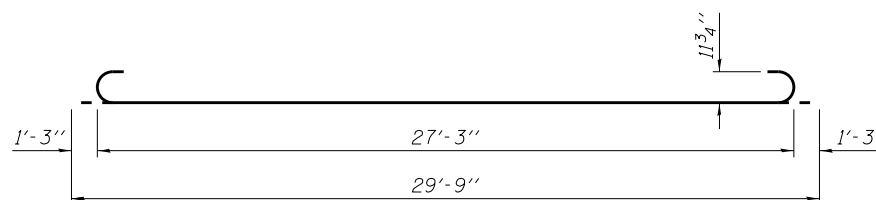
DETAIL A



BONDED PREFORMED JOINT SEAL



BARS a20(E) and a21(E)



BAR b21(E)

**TWO APPROACH SLABS
BILL OF MATERIAL**

Bar	No.	Size	Length	Shape
a20(E)	50	#4	20' - 9"	—
a21(E)	50	#4	22' - 10"	—
a22(E)	92	#5	20' - 4"	—
a23(E)	92	#5	22' - 5"	—
a24(E)	48	#6	6' - 6"	—
b20(E)	70	#4	29' - 8"	—
b21(E)	202	#9	29' - 9"	—
b22(E)	4	#4	14' - 8"	—
b23(E)	4	#4	14' - 9"	—
d20(E)	68	#5	6' - 11"	—
d21(E)	68	#5	7' - 11"	—
e20(E)	32	#4	14' - 8"	—
e21(E)	4	#8	14' - 8"	—
t20(E)	176	#4	9' - 8"	—
w20(E)	80	#5	20' - 4"	—
w21(E)	80	#5	22' - 5"	—
Concrete Superstructure		Cu. Yd.	131.1	
Concrete Structures		Cu. Yd.	26.7	
Bridge Deck Grooving		Sq. Yd.	269	
Protective Coat		Sq. Yd.	304	
Reinforcement Bars, Epoxy Coated		Pound	34,160	

FILE NAME =	DESIGNED <i>EV</i>	REVISED -
... \0480091-68084-016-ApprSlabDetails.dgn	DRAWN <i>JCP</i>	REVISED -
PLOT TIME = 9:33:35 AM	CHECKED <i>PC</i>	REVISED -
PLOT DATE = 10/1/2014	DATE <i>05 03 2013</i>	REVISED -

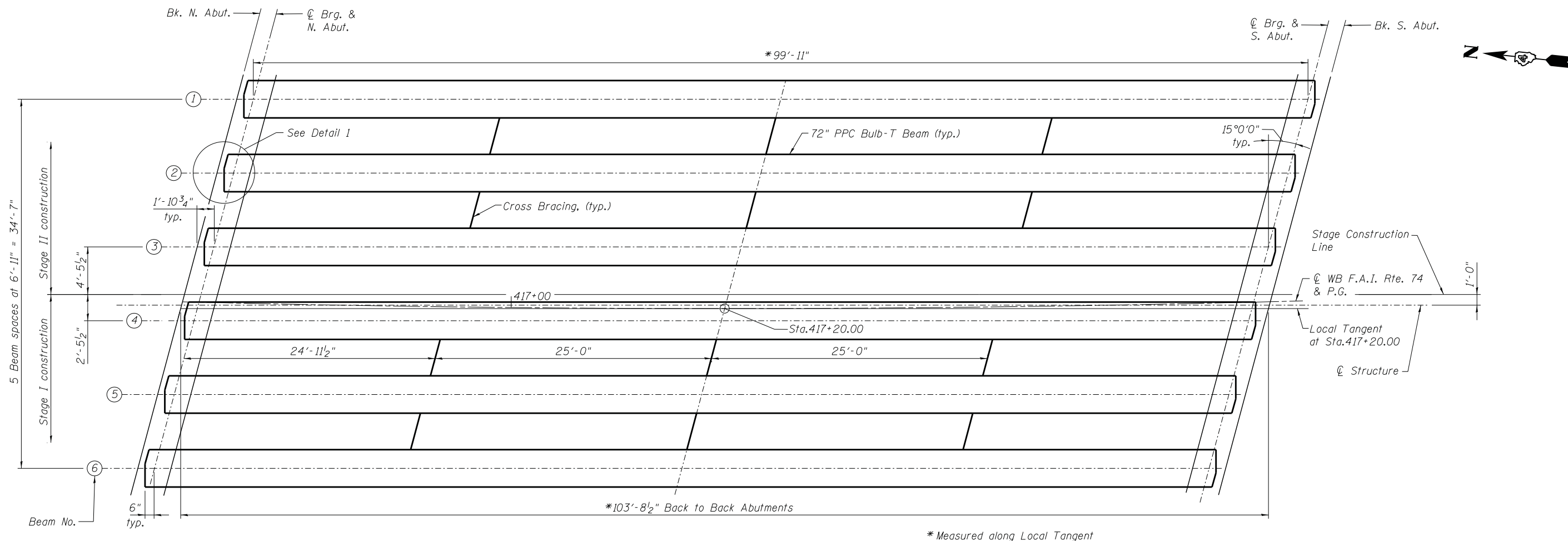
SEPSTEIN
600 WEST FULTON STREET
CHICAGO, ILLINOIS 60611-1209
TEL 312 454 9100
FAX 312 555 1217
WEB www.sepstein.com

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

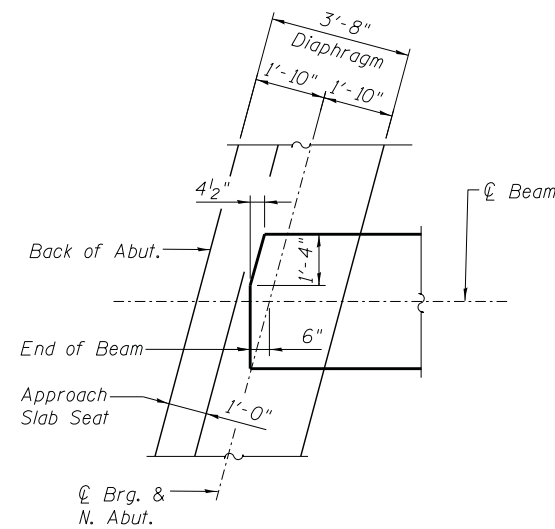
**BRIDGE APPROACH SLAB DETAILS
STRUCTURE NO. 048-0091**

SHEET NO. S16 OF S25 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
74	48-(25B)BR, BR-1J	KNOX	131	62
CONTRACT NO. 68B85				
ILLINOIS FED. AID PROJECT				



PLAN



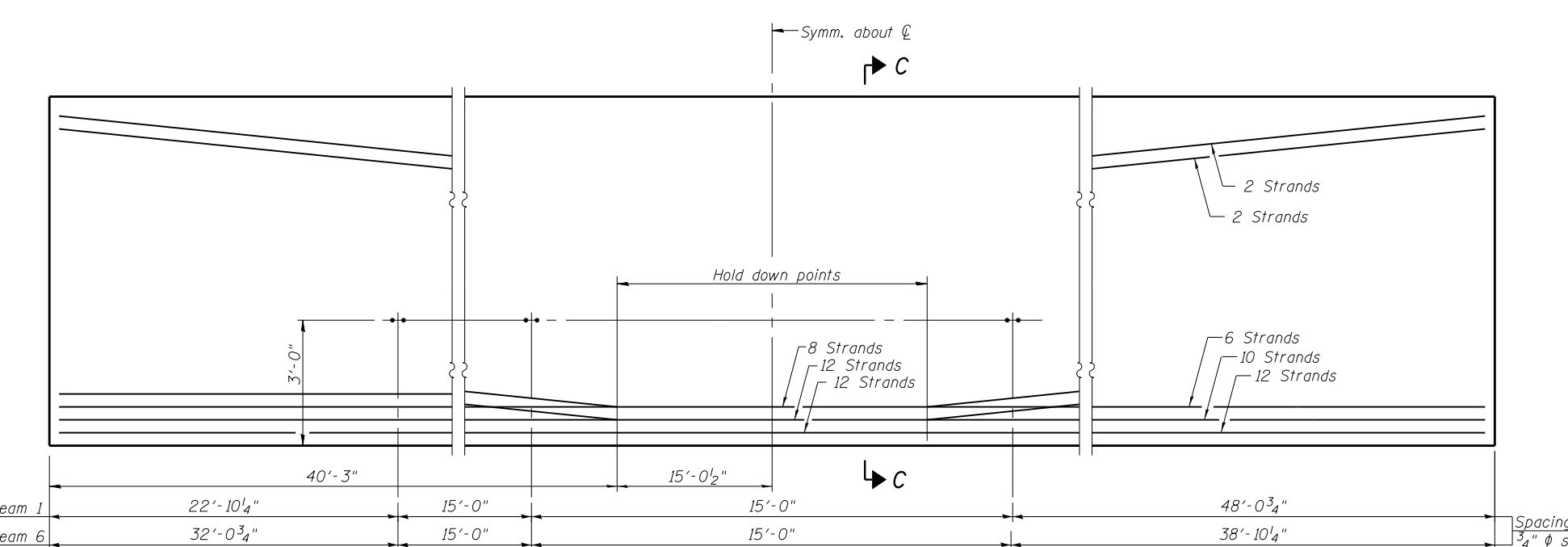
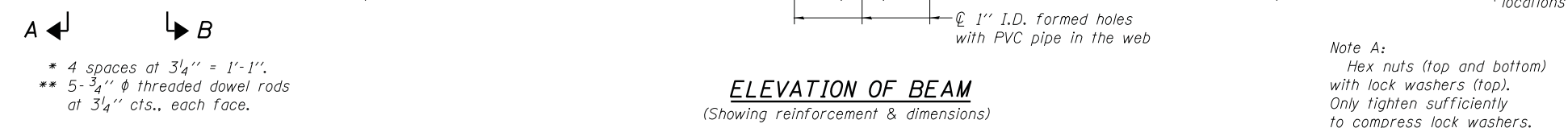
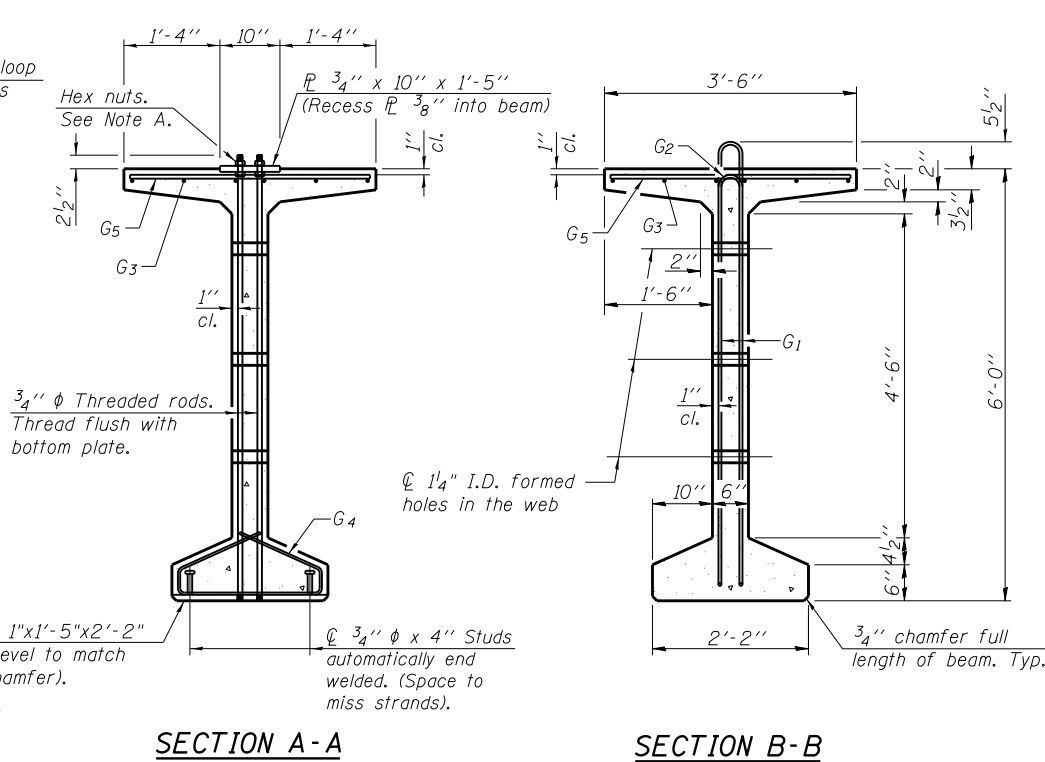
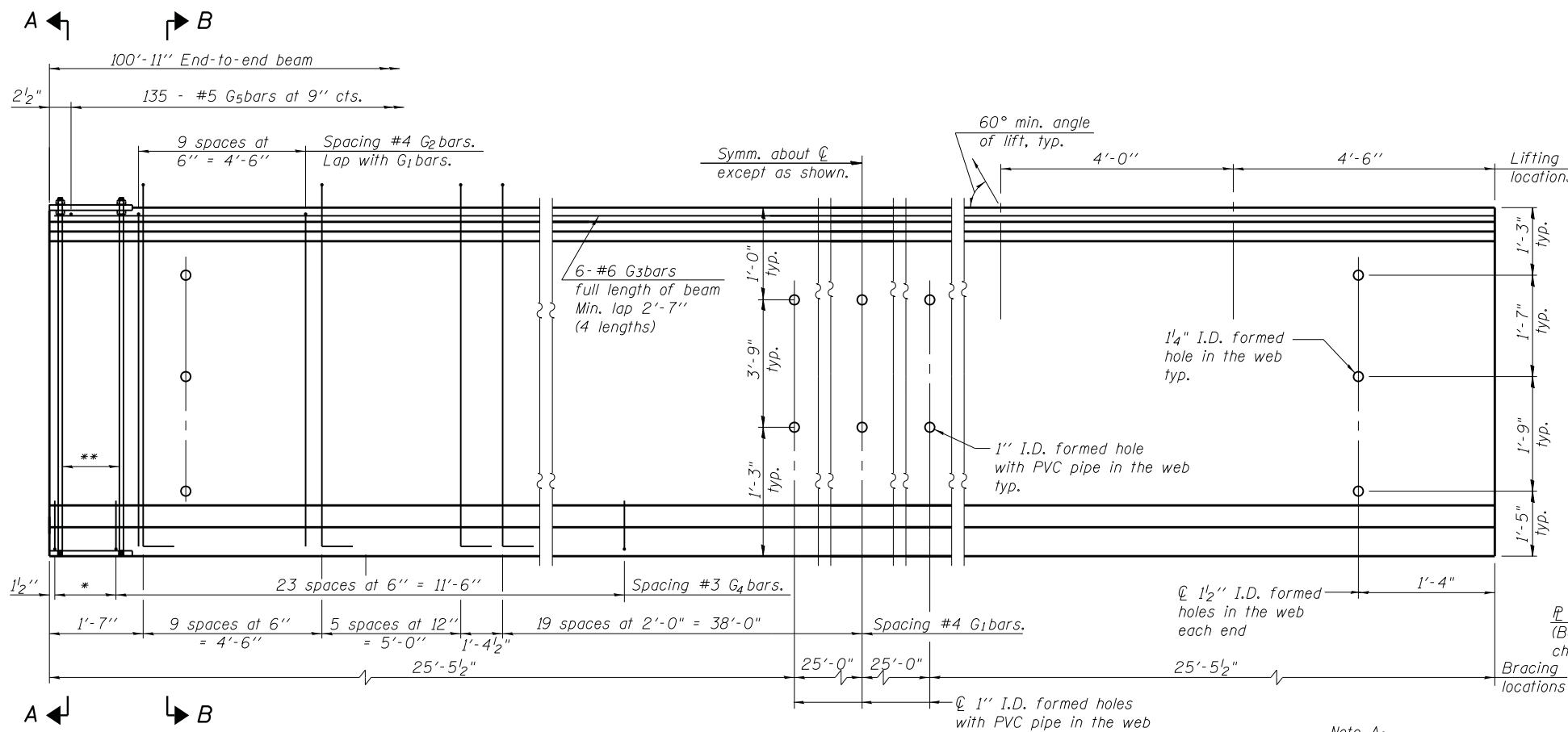
DETAIL 1
Clip Top Flange only

INTERIOR BEAM MOMENT TABLE		
0.5 Sp. 1		
I	(in ⁴)	545894
I'	(in ⁴)	1042047
S _b	(in ³)	14915
S _b '	(in ³)	19851
S _t	(in ³)	15421
S _t '	(in ³)	53421
DC1	(k/')	1.526
M _{DC1}	(k)	1905.2
DC2	(k/')	0.173
M _{DC2}	(k)	216.4
DW	(k/')	0.333
M _{DW}	(k)	416.2
M _{L + IM}	(k)	1,782

INTERIOR BEAM REACTION TABLE		
Abut.		
R _{DC1}	(k)	76.3**
R _{DC2}	(k)	8.7
R _{DW}	(k)	16.7
R _{L + IM}	(k)	87.6
R _{Total}	(k)	189.3

I: Non-composite moment of inertia of beam section (in.⁴).
 I': Composite moment of inertia of beam section (in.⁴).
 S_b: Non-composite section modulus for the bottom fiber of the prestressed beam (in.³).
 S_b': Composite section modulus for the bottom fiber of the prestressed beam (in.³).
 S_t: Non-composite section modulus for the top fiber of the prestressed beam (in.³).
 S_t': Composite section modulus for the top fiber of the prestressed beam (in.³).
 DC1: Un-factored non-composite dead load (kips/ft.).
 M_{DC1}: Un-factored moment due to non-composite dead load (kip-ft.).
 DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).
 M_{DC2}: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).
 DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).
 M_{DW}: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).
 M_{L + IM}: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).

** End Diaphragm Load is excluded



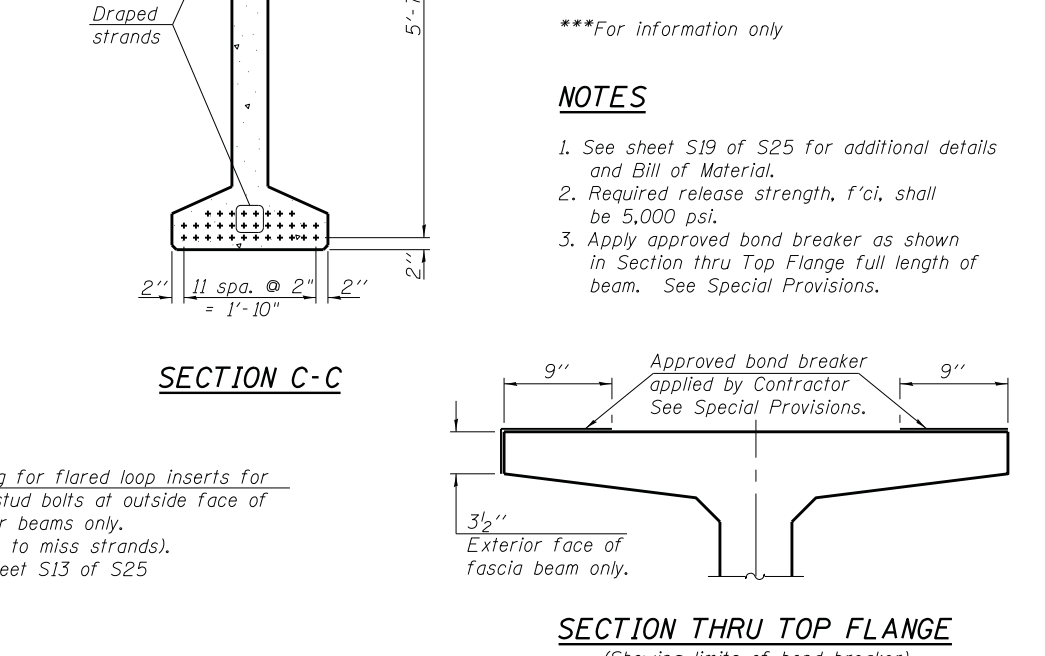
SECTION A-A

SECTION B-B

*****BAR LIST ONE BEAM ONLY**

Bar	No.	Size	Length	Shape
G ₁	69	#4	13'-7"	∩L
G ₂	20	#4	11'-8"	∩
G ₃	24	#6	27'-3"	—
G ₄	56	#3	4'-11"	∩
G ₅	135	#5	3'-4"	—

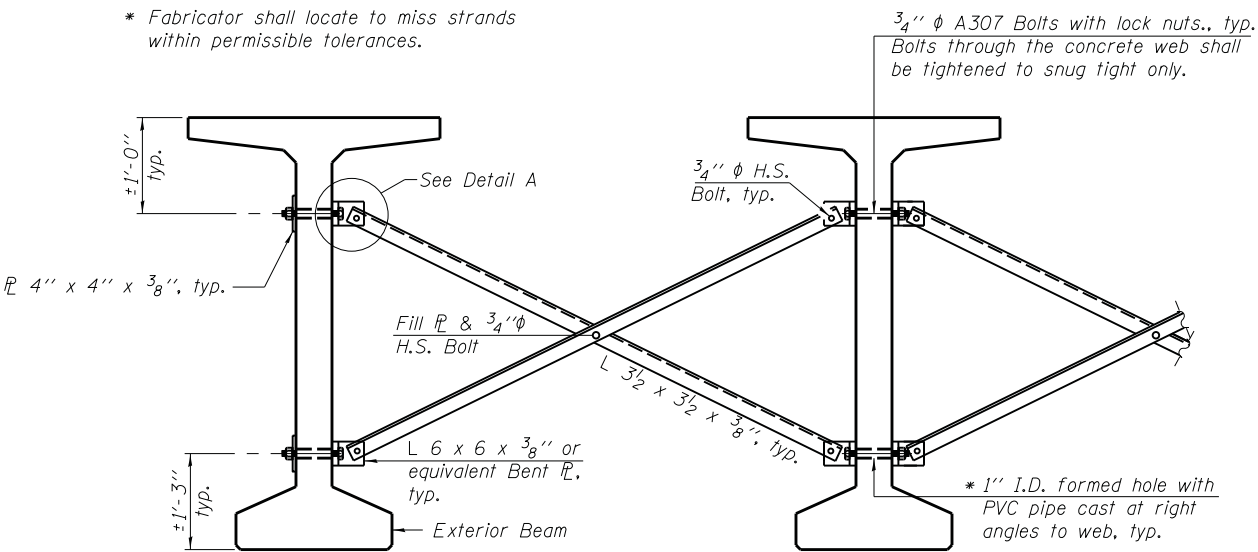
***For information only



ELEVATION OF BEAM (Showing prestressing steel)

SECTION C-C

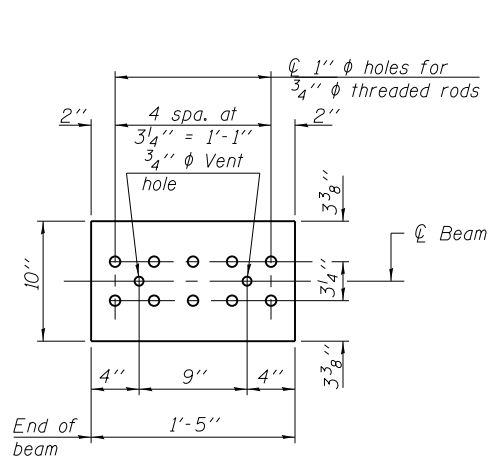
SECTION THRU TOP FLANGE (Showing limits of bond breaker)



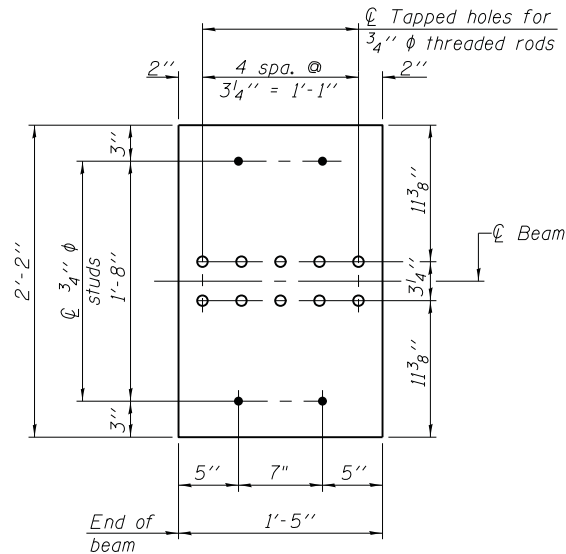
Notes:

All material for bracing shall be hot dip galvanized according to AASHTO M111 unless otherwise noted.
 Two hardened washers are required for each set of oversized holes.
 All holes shall be 15/16" φ unless otherwise noted.
 5/16" x 3" x 3" plate washers are required over all slotted holes.
 All bolts shall be galvanized according to AASHTO M232.
 Bracing shall be installed as beams are erected and tightened as soon as possible during erection.
 Permanent bracing shall not be paid for separately, but shall be included in the cost of Furnishing and Erecting Precast Prestressed Concrete Bulb T-Beams.

PERMANENT BRACING DETAILS FOR BULB-T BEAMS



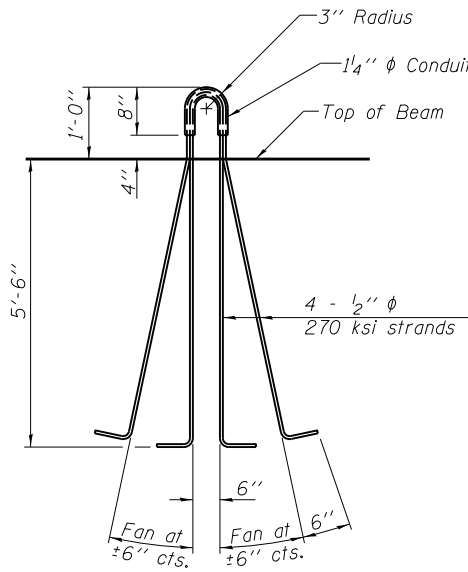
TOP PLATE



BOTTOM PLATE

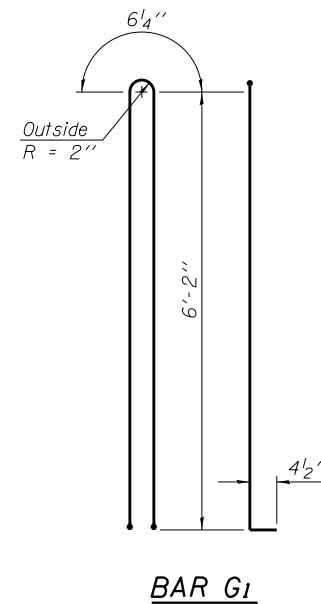
See bearing details for pintle hole locations when required.

LIFTING LOOP DETAIL

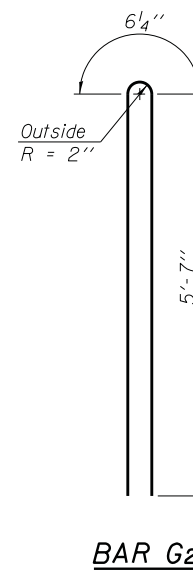


NOTES

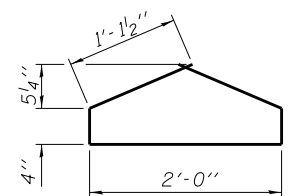
1. Inserts for 3/4" φ threaded dowel rods, when specified, are to be two strut, ferrule type for interior beams and single ferrule, flared loop type for exterior beams.
2. Prestressing steel shall be uncoated high strength, low relaxation 7-wire strand, Grade 270. The nominal diameter shall be 1/2" and the nominal cross-sectional area shall be 0.153 sq. in.
3. Reinforcement bars shall conform to ASTM A 706, Grade 60.
4. A minimum 2 1/2" φ lifting pin shall be used to engage the lifting loops during handling.
5. The top and bottom plates shall be AASHTO M270 Grade 50.
6. The bottom plates and studs shall be galvanized according to AASHTO M111. Top plates and threaded rods need not be galvanized.
7. Threaded rods shall be ASTM F 1554 Grade 55.



BAR G1



BAR G2



BAR G4

BILL OF MATERIAL

Item	Unit	Total
Furnishing and Erecting Precast Prestressed Concrete Bulb T-Beams, 72"	Ft.	605.5

FILE NAME = ... \0480091-68084-019-PPC.T.beam Details.dgn
 PLOT TIME = 4:32:09 PM
 PLOT DATE = 7/30/2014

DESIGNED EV
 DRAWN JCP
 CHECKED PC
 DATE 05 03 2013

REVISED -
 REVISED -
 REVISED -
 REVISED -

SEPSTEIN
 800 WEST FULTON STREET
 CHICAGO, ILLINOIS 60611-1259
 TEL 312 454 9100
 FAX 312 555 1217
 WEB www.sepstein.com

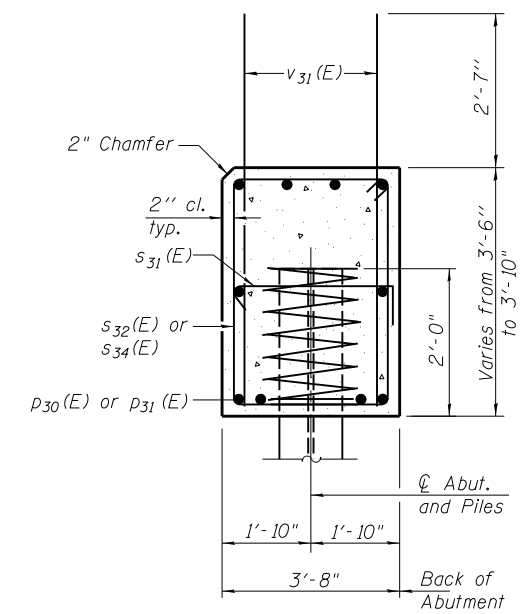
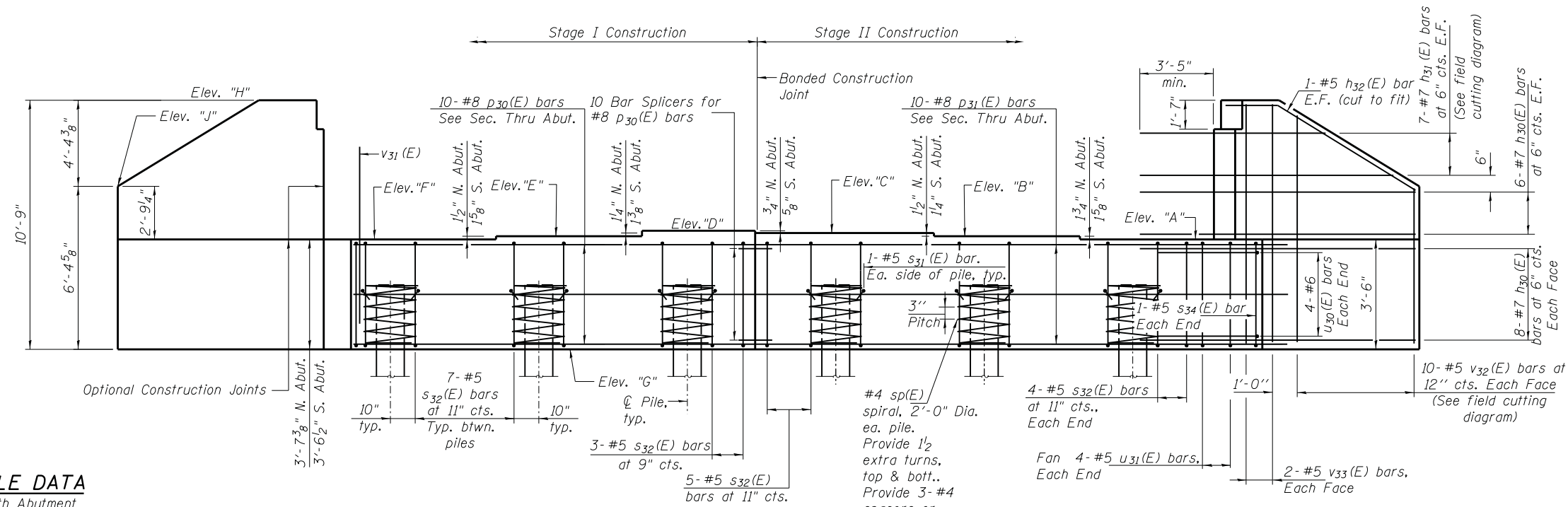
**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**72" PPC BULB T-BEAM DETAILS
 STRUCTURE NO. 048-0091**

SHEET NO. S19 OF S25 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
74	48-(125B)BR, BR-1J	KNOX	131	65
CONTRACT NO. 68B85				
ILLINOIS FED. AID PROJECT				

Notes:
Pour steps monolithically with cap.

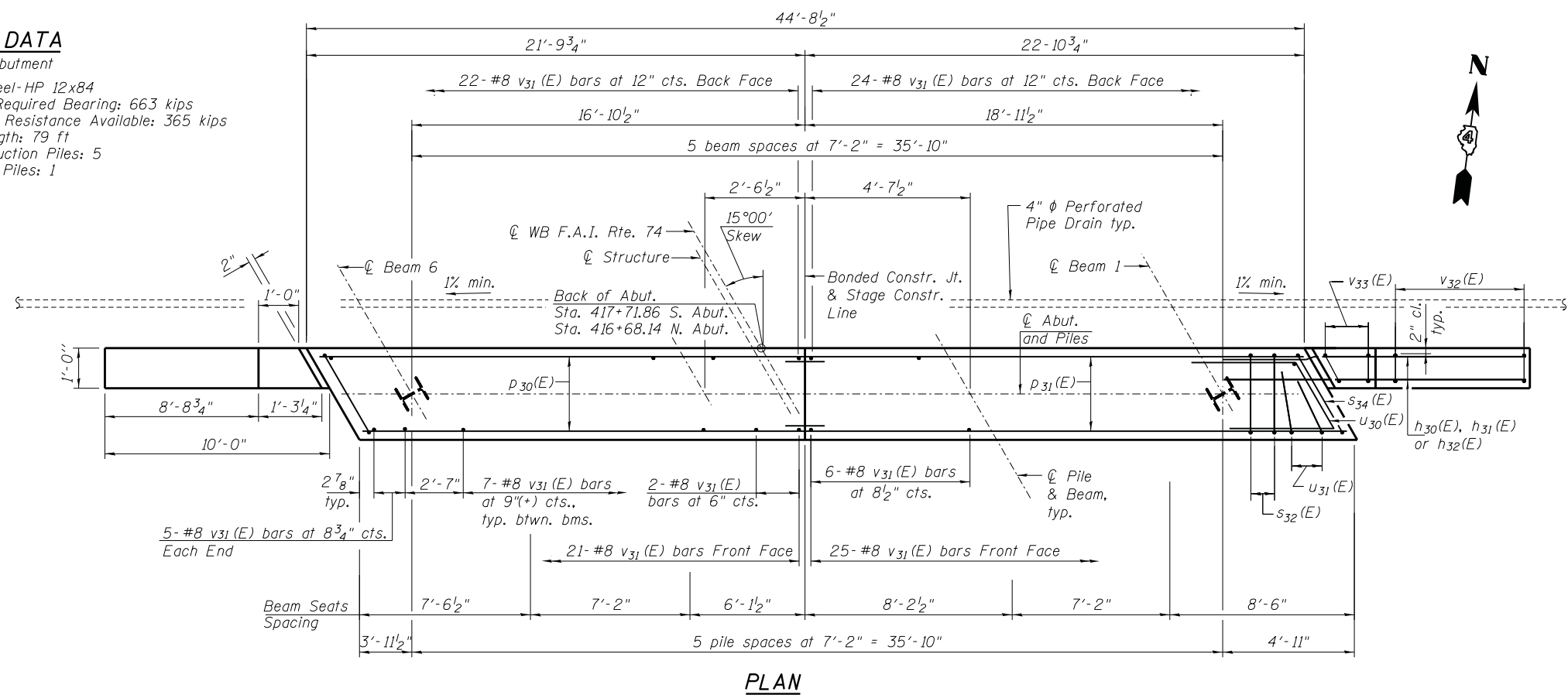


PILE DATA

North Abutment
Type: Steel-HP 12x84
Nominal Required Bearing: 658 kips
Factored Resistance Available: 362 kips
Est. Length: 71 ft
No. Production Piles: 5
No. Test Piles: 1

PILE DATA

South Abutment
Type: Steel-HP 12x84
Nominal Required Bearing: 663 kips
Factored Resistance Available: 365 kips
Est. Length: 79 ft
No. Production Piles: 5
No. Test Piles: 1



	N. Abut.	S. Abut.
Elev. "A"	748.30	748.39
Elev. "B"	748.45	748.53
Elev. "C"	748.57	748.63
Elev. "D"	748.63	748.68
Elev. "E"	748.53	748.57
Elev. "F"	748.41	748.43
Elev. "G"	744.80	744.89
Elev. "H"	755.55	755.64
Elev. "J"	751.19	751.28

NOTES:

- For Section thru Abutment, drainage details, Bar Bending Diagrams and Bill of Materials see sheet S21 of S25.
- Backfill shall be placed behind the abutment after the superstructure has been poured and falsework removed. See Article 502.10 of the Standard Specifications.
- For Bar Splicer (E) details see sheet S24 of S25.
- For details of piles see sheet S22 of S25.

BILL OF MATERIAL

North Abutment

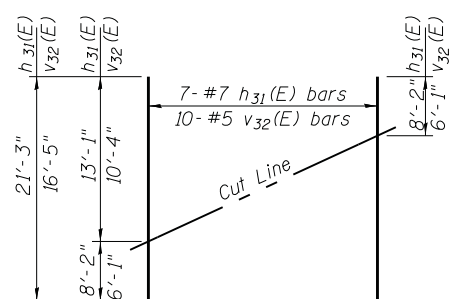
Bar	No.	Size	Length	Shape
h ₃₀ (E)	56	#7	13' - 5"	▬
h ₃₁ (E)	14	#7	21' - 3"	▬
h ₃₂ (E)	4	#5	10' - 5"	▬
p ₃₀ (E)	10	#8	20' - 6"	▬
p ₃₁ (E)	10	#8	22' - 7"	▬
s ₃₁ (E)	12	#5	4' - 4"	⌋
s ₃₂ (E)	44	#5	13' - 11"	⌋
s ₃₄ (E)	2	#5	14' - 1"	⌋
* sp(E)	12	#4	2' - 0"	▩
u ₃₀ (E)	8	#6	11' - 1"	⌋
u ₃₁ (E)	8	#5	9' - 8"	⌋
v ₃₁ (E)	92	#8	5' - 11"	▬
v ₃₂ (E)	20	#5	16' - 5"	▬
v ₃₃ (E)	8	#5	10' - 5"	▬
Structure Excavation	Cu. Yd.		389	
Concrete Structures	Cu. Yd.		29.7	
Reinforcement Bars, Epoxy Coated	Pound		6,180	
Furnishing Steel Piles HP12x84	Foot		355	
Driving Piles	Foot		355	
Test Pile Steel HP12x84	Each		1	
Geocomposite Wall Drain	Sq. Yd.		68	
Granular Backfill for Structures	Cu. Yd.		196.2	
Pipe Underdrain for Structures, 4" dia.	Foot		99	

BILL OF MATERIAL

South Abutment

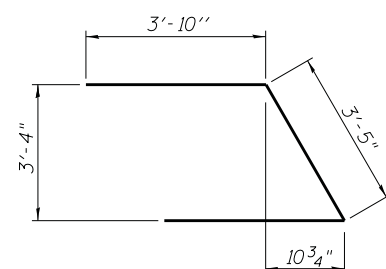
Bar	No.	Size	Length	Shape
h ₃₀ (E)	56	#7	13' - 5"	▬
h ₃₁ (E)	14	#7	21' - 3"	▬
h ₃₂ (E)	4	#5	10' - 5"	▬
p ₃₀ (E)	10	#8	20' - 6"	▬
p ₃₁ (E)	10	#8	22' - 7"	▬
s ₃₁ (E)	12	#5	4' - 4"	⌋
s ₃₂ (E)	44	#5	13' - 11"	⌋
s ₃₄ (E)	2	#5	14' - 1"	⌋
* sp(E)	12	#4	2' - 0"	▩
u ₃₀ (E)	8	#6	11' - 1"	⌋
u ₃₁ (E)	8	#5	9' - 8"	⌋
v ₃₁ (E)	92	#8	5' - 11"	▬
v ₃₂ (E)	20	#5	16' - 5"	▬
v ₃₃ (E)	8	#5	10' - 5"	▬
Structure Excavation	Cu. Yd.		389	
Concrete Structures	Cu. Yd.		29.7	
Reinforcement Bars, Epoxy Coated	Pound		6,180	
Furnishing Steel Piles HP12x84	Foot		395	
Driving Piles	Foot		395	
Test Pile Steel HP12x84	Each		1	
Geocomposite Wall Drain	Sq. Yd.		68	
Granular Backfill for Structures	Cu. Yd.		196.2	
Pipe Underdrain for Structures, 4" dia.	Foot		97	

*Length is height of spiral.

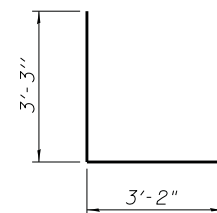


FIELD CUTTING DIAGRAM

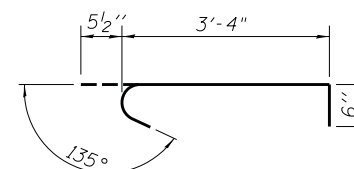
Order h₃₁(E) and v₃₂(E) full length. Cut as shown and use remainder of bars in opposite face.



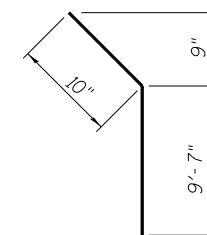
BAR u₃₀(E)



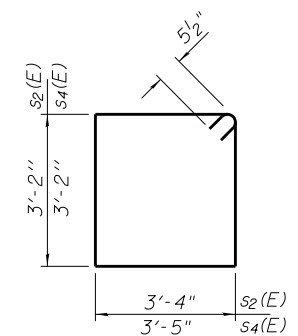
BAR u₃₁(E)



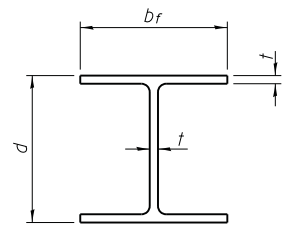
BAR s₃₀(E)



BAR h₃₂(E)

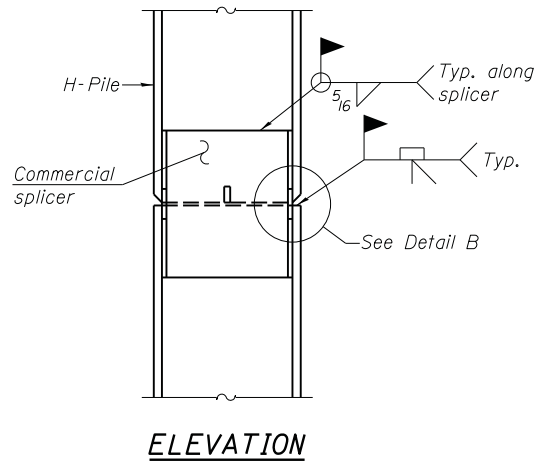


BARS s₃₁(E) and s₃₂(E)

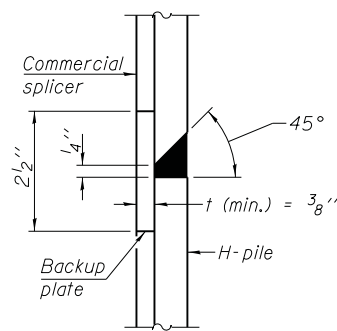


STEEL PILE TABLE

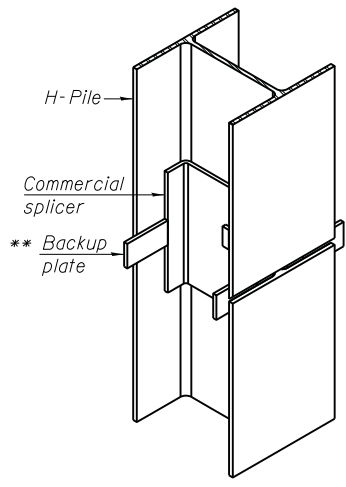
Designation	Depth d	Flange width br	Web and Flange thickness t	Encasement diameter A
HP 14x117	14 1/4"	14 7/8"	13/16"	30"
x102	14"	14 3/4"	1/16"	30"
x89	13 7/8"	14 3/4"	5/8"	30"
x73	13 5/8"	14 5/8"	1/2"	30"
HP 12x84	12 1/4"	12 1/4"	1/16"	24"
x74	12 1/8"	12 1/4"	5/8"	24"
x63	12"	12 1/8"	1/2"	24"
x53	11 3/4"	12"	7/16"	24"
HP 10x57	10"	10 1/4"	9/16"	24"
x42	9 3/4"	10 1/8"	7/16"	24"
HP 8x36	8"	8 1/8"	7/16"	18"



ELEVATION

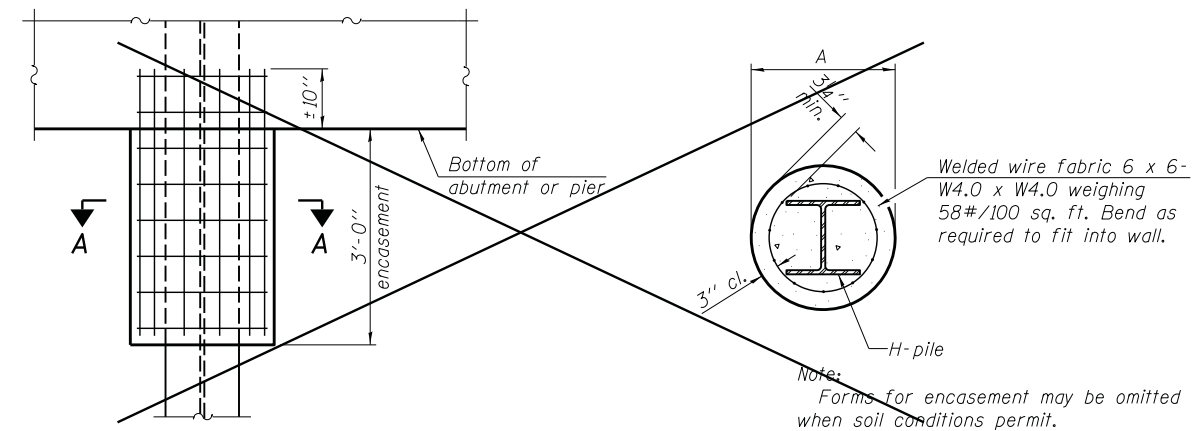


DETAIL "B"



ISOMETRIC VIEW

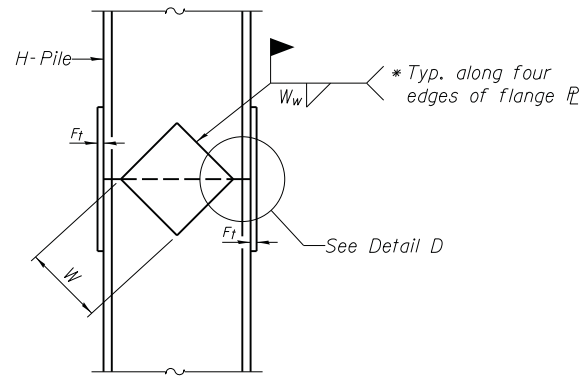
WELDED COMMERCIAL SPLICE



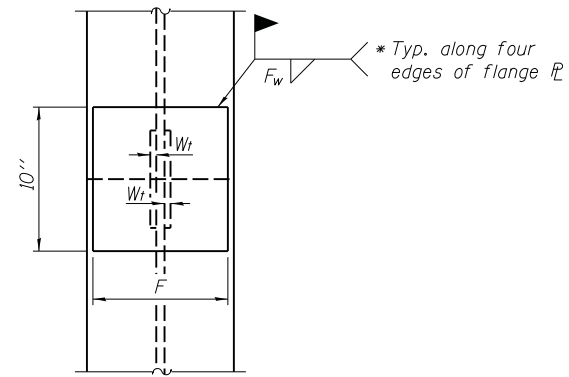
ELEVATION

SECTION A-A

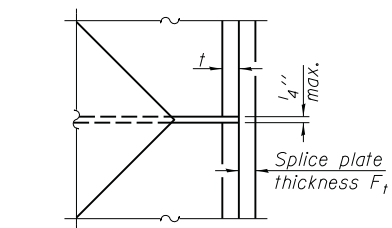
PILE ENCASEMENT



ELEVATION



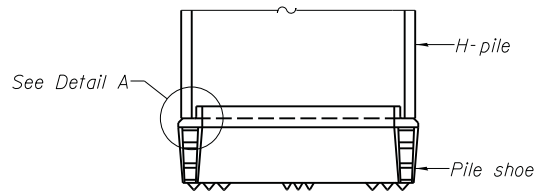
END VIEW



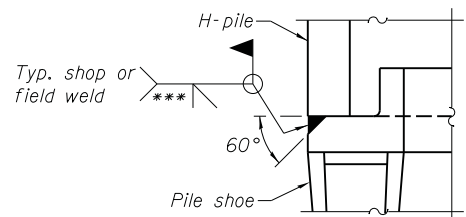
DETAIL D

WELDED PLATE FIELD SPLICE

Designation	F	F _t	F _w	W	W _t	W _w
HP 14x117	12 1/2"	1"	7/8"	7 3/4"	5 8"	1/2"
x102	12 1/2"	7/8"	3/4"	7 3/4"	5 8"	1/2"
x89	12 1/2"	3/4"	1/16"	7 3/4"	5 8"	1/2"
x73	12 1/2"	5/8"	9/16"	7 3/4"	5 8"	1/2"
HP 12x84	10"	7/8"	1/16"	6 1/2"	5 8"	1/2"
x74	10"	7/8"	1/16"	6 1/2"	5 8"	1/2"
x63	10"	5/8"	1/2"	6 1/2"	1/2"	3/8"
x53	10"	5/8"	1/2"	6 1/2"	1/2"	3/8"
HP 10x57	8"	3/4"	9/16"	5 1/4"	1/2"	3/8"
x42	8"	5/8"	9/16"	5 1/4"	1/2"	3/8"
HP 8x36	7"	5/8"	7/16"	4 1/4"	1/2"	3/8"

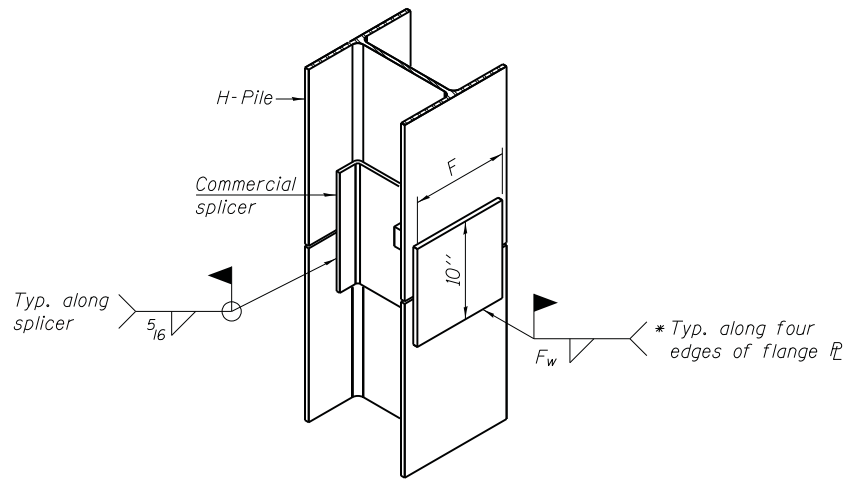


ELEVATION



DETAIL A

H-PILE SHOE ATTACHMENT



ISOMETRIC VIEW

WELDED COMMERCIAL SPLICE ALTERNATE

- * Interrupt welds 1/4" from end of web and/or each flange.
- ** Remove portions of backup plates that extend outside the flanges.
- *** Weld size per pile shoe manufacturer (5/16" min.).

Note:
The steel H-piles shall be according to AASHTO M270 Grade 50.

F-HP

1-27-12

FILE NAME =	DESIGNED EV	REVISED -
...\\0480091-68084-022-Pile.details.dgn	DRAWN JCP	REVISED -
PLOT TIME = 4:32:13 PM	CHECKED PC	REVISED -
PLOT DATE = 7/30/2014	DATE 05 03 2013	REVISED -

SEPSTEIN
800 WEST FULTON STREET
CHICAGO, ILLINOIS 60611-1259
TEL 312 454 9100
FAX 312 555 1217
WEB www.sepstein.com

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

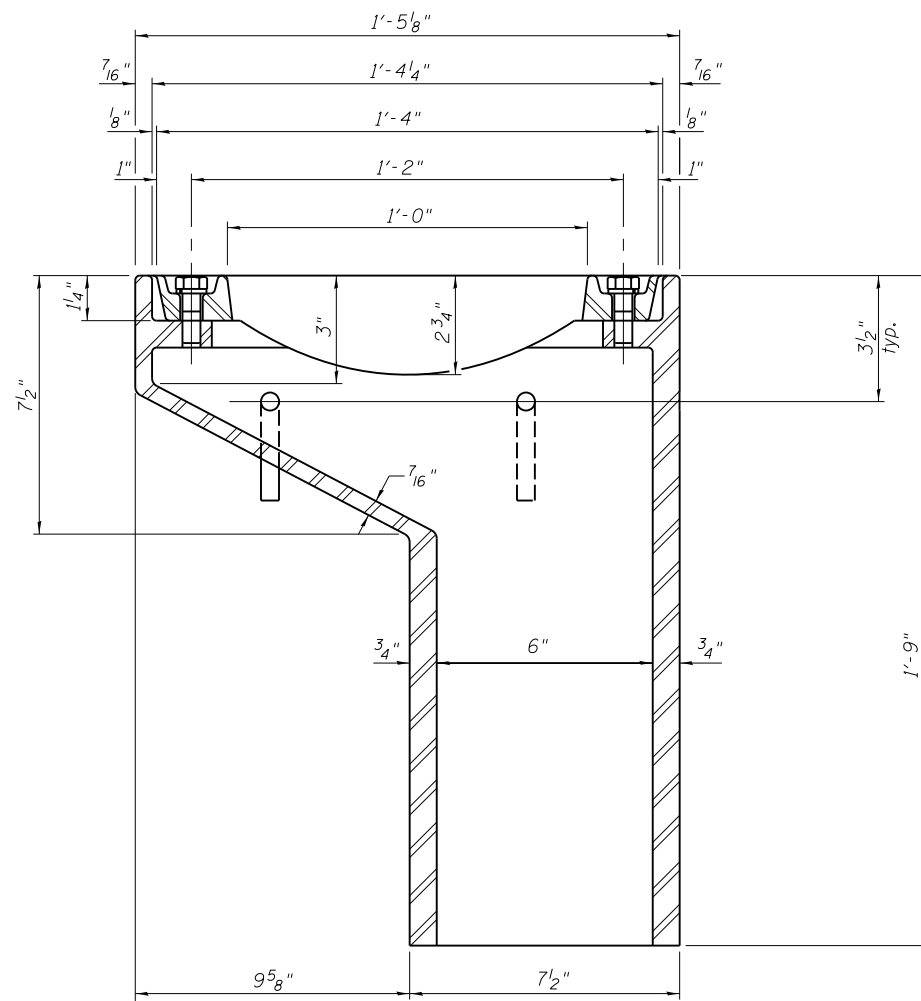
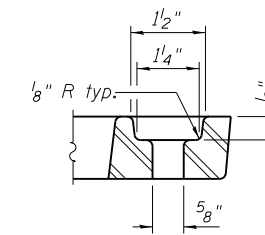
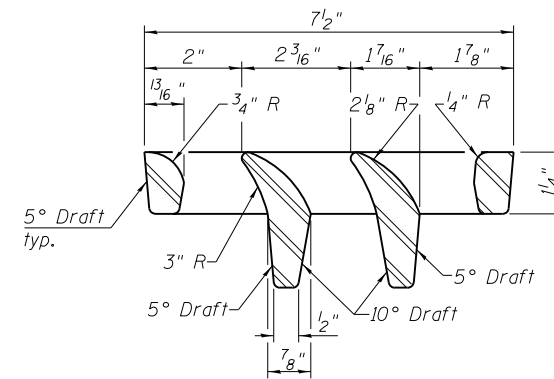
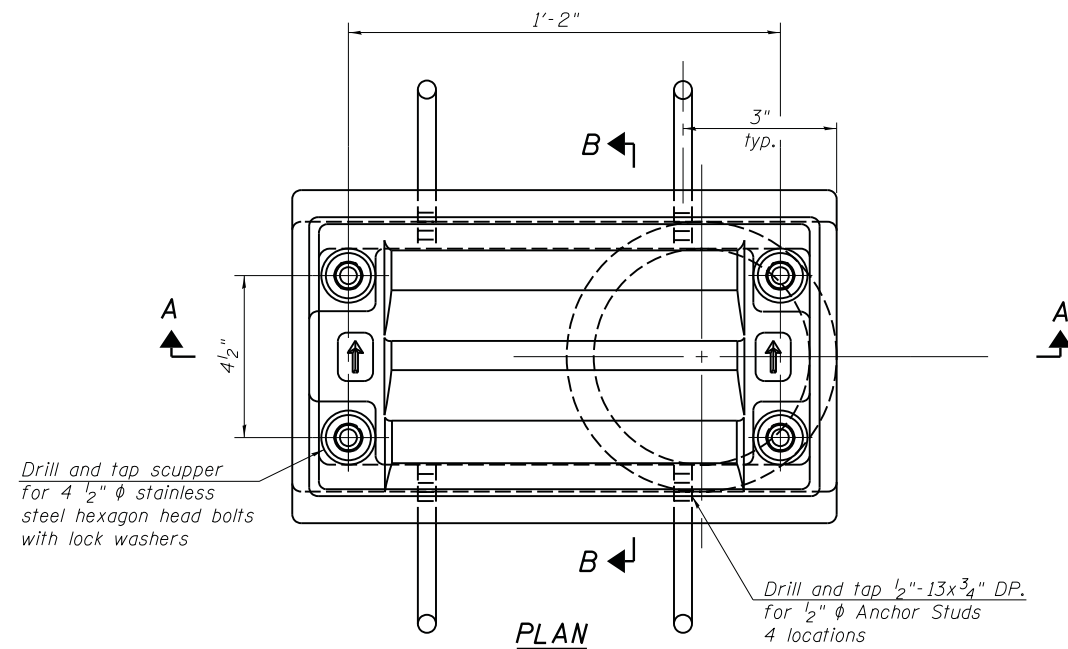
**HP PILE DETAILS
STRUCTURE NO. 048-0091**

SHEET NO. S22 OF S25 SHEETS

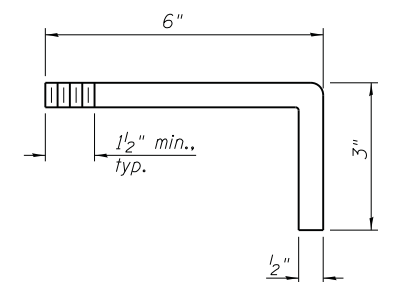
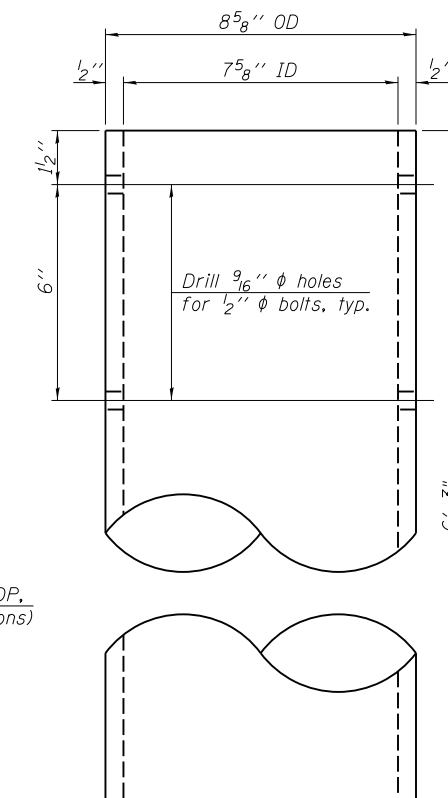
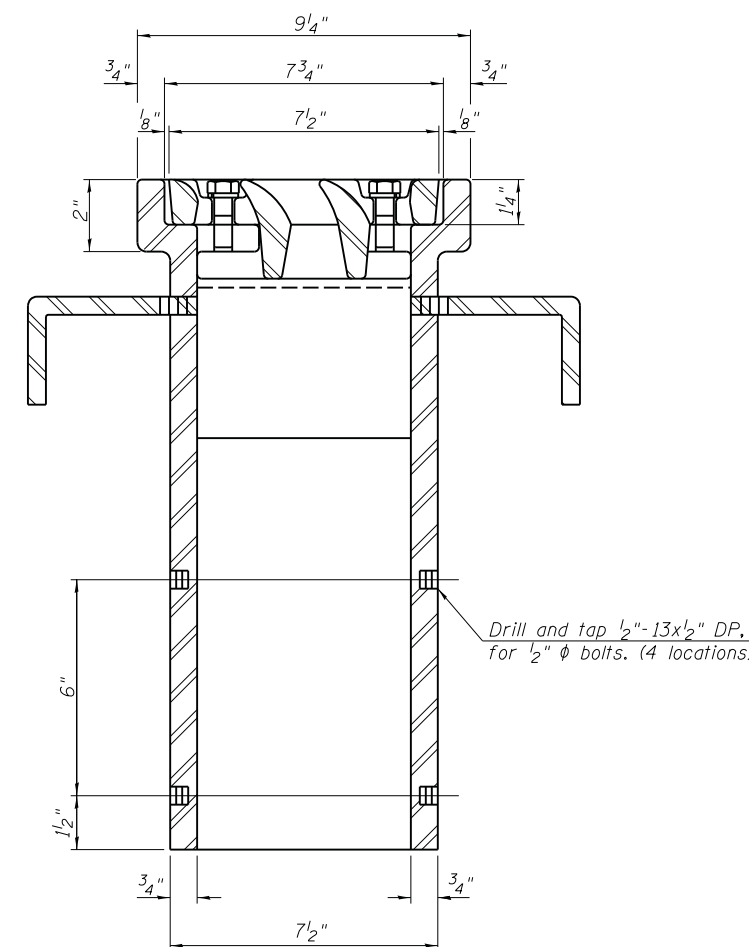
F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
74	48-(125B)BR, BR-1J	KNOX	131	68
CONTRACT NO. 68B85				
ILLINOIS FED. AID PROJECT				

NOTES

1. All cast iron parts shall be gray iron conforming to the requirements of AASHTO M 105, Class 35B.
2. Bolts, anchor studs, washers and nuts shall conform to the requirements of ASTM A 307 and shall be galvanized according to AASHTO M 232.
3. Downspouts located on the exterior side of a painted steel fascia beam shall be painted with the finish coat specified for the exterior side of the fascia beam.
4. As an alternate, bolts, anchor studs, washers and nuts may be stainless steel according to Article 1006.29(d) of the Standard Specifications.
5. Structural steel weldments of equal sections and of the same configuration may be substituted for the cast iron scupper frame. Fillet or full penetration welds shall be used for the weldments. Details shall be submitted to the Engineer for approval. Structural steel weldments shall not be substituted for the cast iron scupper grate. Structural steel frames and downspouts shall be galvanized according to AASHTO M111.
6. The Contractor shall take appropriate measures to assure that Protective Coat is not applied to the scupper.
7. Cost of the Grate, Frame, Downspout, Anchor Studs, Bolts, Washers and Nuts including complete installation of the scupper shall be paid for at the contract unit price each for Drainage Scupper, DS-11.
8. Alternate fiberglass downspout conforming to ASTM D 2996 with a short-time rupture strength hoop tensile stress of 30,000 psi min. may be used in lieu of the cast iron or steel equivalent.



SECTION A-A
See sheet S13 of S25 for scupper location relative to parapet.



ANCHOR STUD DETAIL

DOWNSPOUT

BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Drainage Scupper, DS-11	Each	6

DS-11

7-1-10

FILE NAME =	DESIGNED EV	REVISED -
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PLOT TIME = 4:32:14 PM	CHECKED PC	REVISED -
PLOT DATE = 7/30/2014	DATE 05 03 2013	REVISED -

SEPSTEIN

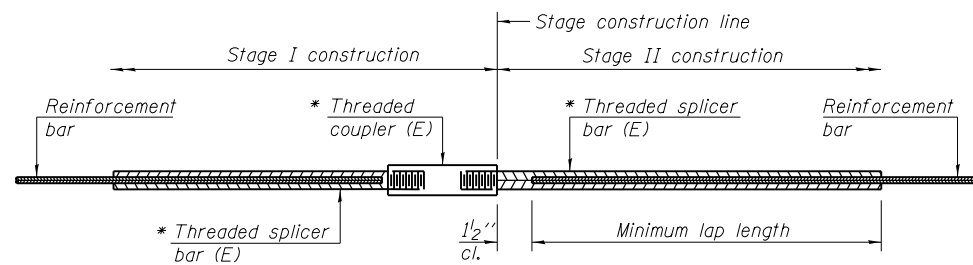
800 WEST FULTON STREET TEL 312 454 9100
CHICAGO, ILLINOIS FAX 312 555 1217
60661-1259 WEB www.sepstein.com

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**DRAINAGE SCUPPER DS-11
STRUCTURE NO. 048-0091**

SHEET NO. S23 OF S25 SHEETS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
74	48-(125B)BR, BR-11	KNOX	131	69
				CONTRACT NO. 68B85
ILLINOIS FED. AID PROJECT				



STANDARD BAR SPLICER ASSEMBLY

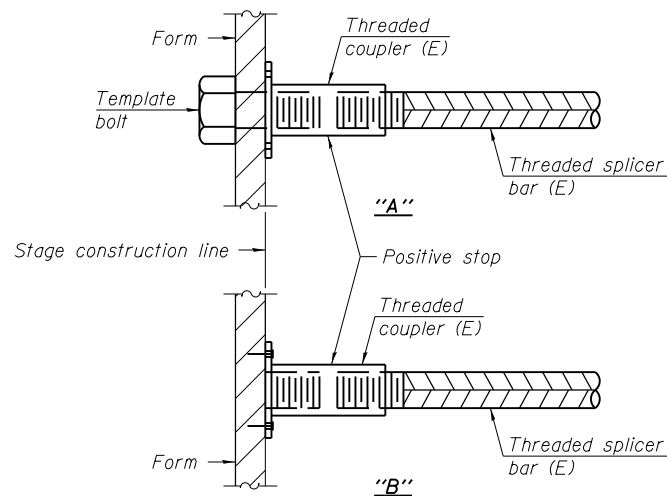
Minimum Lap Lengths					
Bar size to be spliced	Table 1	Table 2	Table 3	Table 4	Table 5
3, 4	1'-5"	1'-11"	2'-1"	2'-4"	2'-3"
5	1'-9"	2'-5"	2'-7"	2'-11"	2'-10"
6	2'-1"	2'-11"	3'-1"	3'-6"	3'-4"
7	2'-9"	3'-10"	4'-2"	4'-8"	4'-6"
8	3'-8"	5'-1"	5'-5"	6'-2"	5'-10"
9	4'-7"	6'-5"	6'-10"	7'-9"	7'-5"

- Table 1: Black bar, 0.8 Class C
- Table 2: Black bar, Top bar lap, 0.8 Class C
- Table 3: Epoxy bar, 0.8 Class C
- Table 4: Epoxy bar, Top bar lap, 0.8 Class C
- Table 5: Epoxy bar, Top bar lap, Class B

Threaded splicer bar length = min. lap length + 1 1/2" + thread length

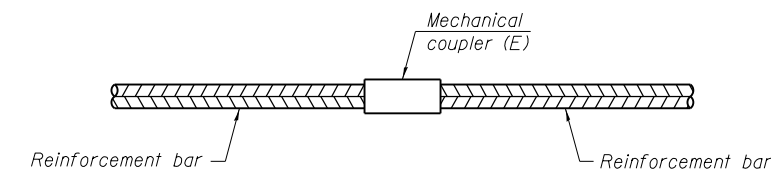
* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

Location	Bar size	No. assemblies required	Table for minimum lap length
Deck	#5	302	3
Diaphragm	#6	26	3
Abutments	#8	20	3
Approach Slab	#5	172	3
Approach Slab	#4	50	3



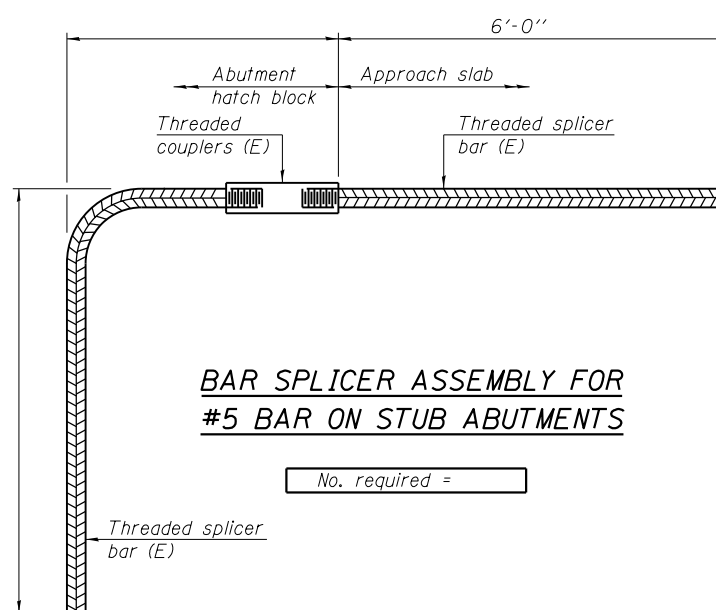
INSTALLATION AND SETTING METHODS

"A" : Set bar splicer assembly by means of a template bolt.
 "B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms.
 (E) : Indicates epoxy coating.



STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required



BAR SPLICER ASSEMBLY FOR #5 BAR ON STUB ABUTMENTS

No. required =

NOTES

1. Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.
2. All reinforcement shall be lapped and tied to the splicer bars.
3. Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications.
4. See approved list of bar splicer assemblies and mechanical splicers for alternatives.

BRIDGE FOUNDATION BORING LOG

PROJECT I-74-2(-)38 BRIDGE FAI Route 74 Date 12-29-61
 ROUTE FAI 74 over Henderson Creek Bored By Alvin E. Moine
 SEC. 18-25B STA. 117+20 Left Lane Checked By _____
 COUNTY Knox

Elevation	Z	t/sf	w (%)	Surface Water El.	Elevation	Z	t/sf	w (%)
737.60					737.60	100/6"	-	12
					713.6			
	7	0.5 E	30		-25	50/3"	-	-
733.6								
	-5	4 0.7 B	37		-100	5"	-	-
					-30	50/1"	-	-
	-10	5	-		-50	1"	-	-
726.1					-35	100/5"	-	-
	23	-	-					
					702.1			
723.6								
	-15	31 2.9 S	14					
					-40			
718.6								
	-20	95 3.3 S	-					
					-45			

V - Standard Penetration Test - Blows per foot to drive 2" J.D. Split Spoon Sampler 12" with 40# hammer falling 30".
 Qu - Unconfined Compressive Strength - t/sf
 Type failure:
 B - Bulge Failure
 S - Shear Failure
 E - Estimated Value
 w - Water Content - percentage of oven dry weight-%.

BRIDGE FOUNDATION BORING LOG

PROJECT I-74-2(-)38 BRIDGE FAI Route 74 Date 1-17-62
 ROUTE FAI 74 over Henderson Creek Bored By Alvin E. Moine
 SEC. 18-25B STA. 117+20 Left Lane Checked By _____
 COUNTY Knox

Elevation	Z	t/sf	w (%)	Surface Water El.	Elevation	Z	t/sf	w (%)
737.0					737.0			
					713.7			
					-25	100/8"	2.8 S	-
	5	-	-					
					-5	100	2.0 S	-
	4	-	37					
					731.2			
					-30	50/3"	2.0 E	19
	3	-	-					
					-10	3 0.8 B	-	-
					726.2			
					-35			
	13	0.9 S	15					
					723.7			
					-15	92 3.0 E	10	-
					721.2			
					-40			
					-20	30 3.1 S	-	-
					-45			

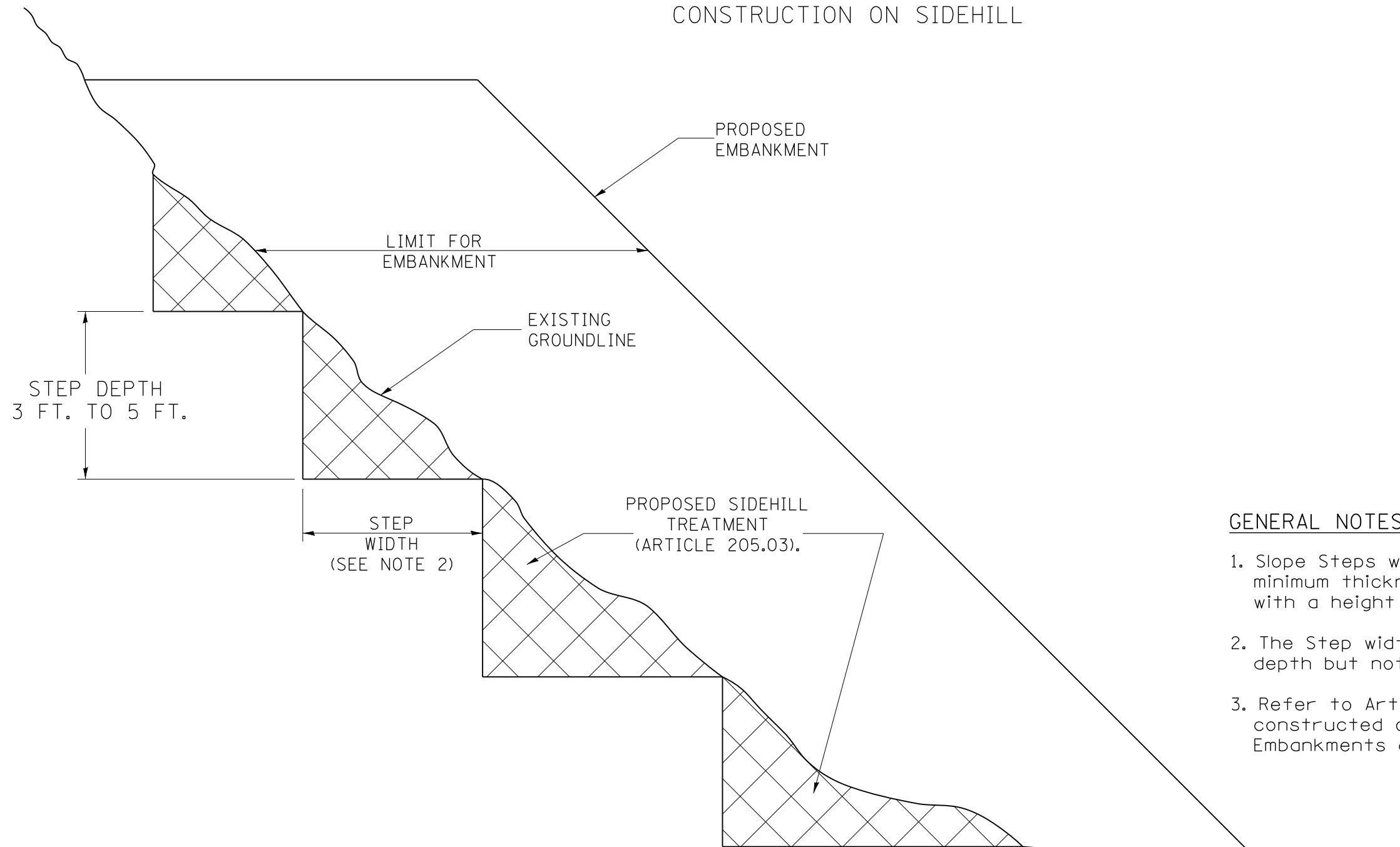
V - Standard Penetration Test - Blows per foot to drive 2" J.D. Split Spoon Sampler 12" with 40# hammer falling 30".
 Qu - Unconfined Compressive Strength - t/sf
 Type failure:
 B - Bulge Failure
 S - Shear Failure
 E - Estimated Value
 w - Water Content - percentage of oven dry weight-%.

NOTES

1. The structure boring for the project location completed during the year 1961-1962, furnished by Illinois Department of Transportation, has been included as Exhibit B in the Structure Geotechnical Report for the current project.

SLOPE STEPS DETAIL

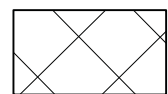
TYPICAL CROSS-SECTION EMBANKMENT CONSTRUCTION ON SIDEHILL



GENERAL NOTES:

1. Slope Steps will be required for all 12(300) minimum thickness "silver fills" and on a fills with a height of 10'(3.0m).
2. The Step width shall be twice the Step depth but not less than 6 feet.
3. Refer to Article 205.03 for Embankment to be constructed on Hillside or Slopes, or if existing Embankments are to be widened.

REPLACEMENT MATERIAL:



STANDARD EMBANKMENT
(IN ACCORDANCE WITH
205 OF THE STANDARD SPECIFICATION).

All dimensions are in inches (millimeters)
unless otherwise noted.

1-1-97	RENUM. L-5.03, NEW REVISION BOX, REVISED TITLE	T.P.			
	BOX, REVISED GENERAL NOTES.				
10-16-06	REVISED TO 2007 SPEC.	M.A.			

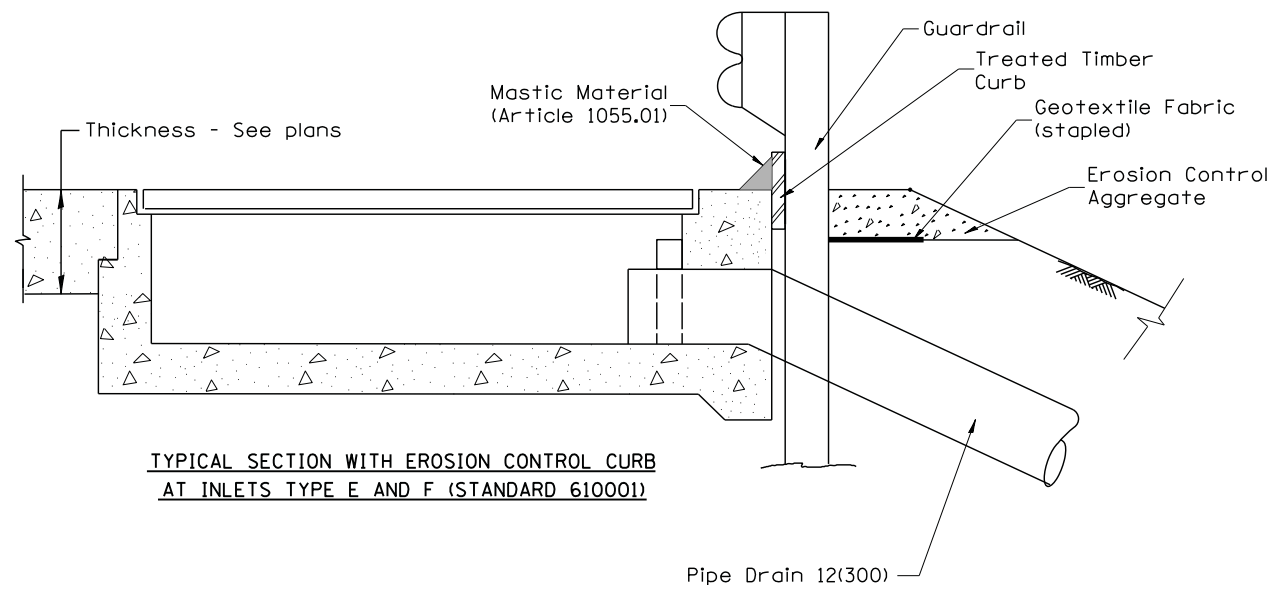
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

SLOPE STEPS DETAIL

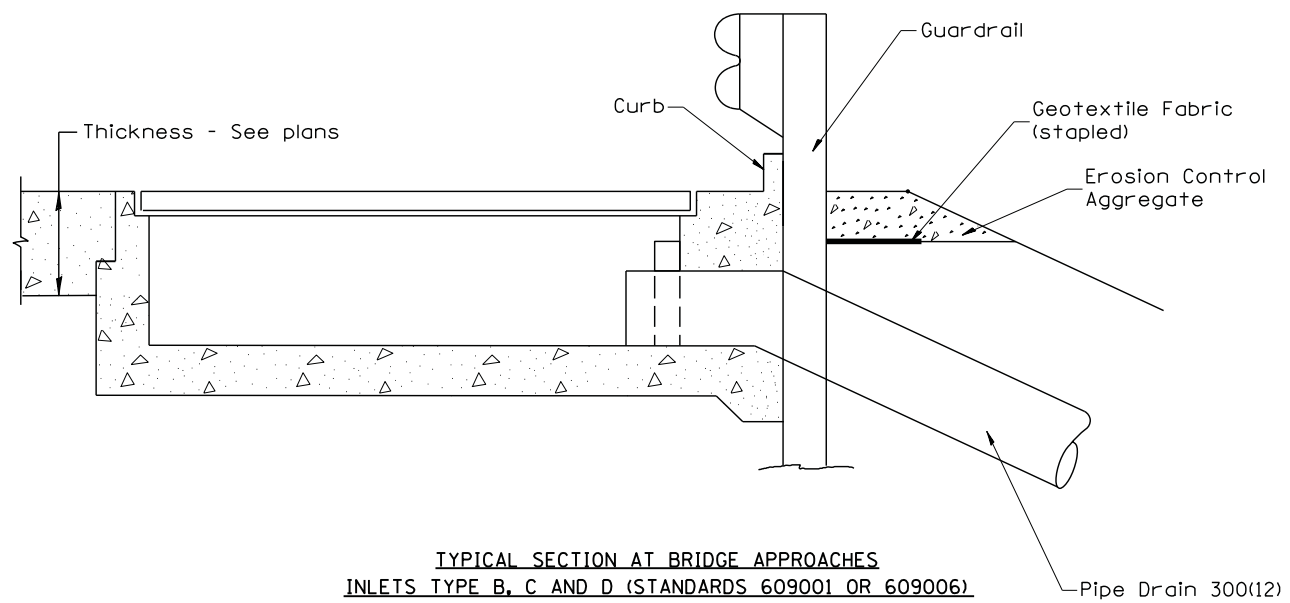
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CADD STD. 205001-D4

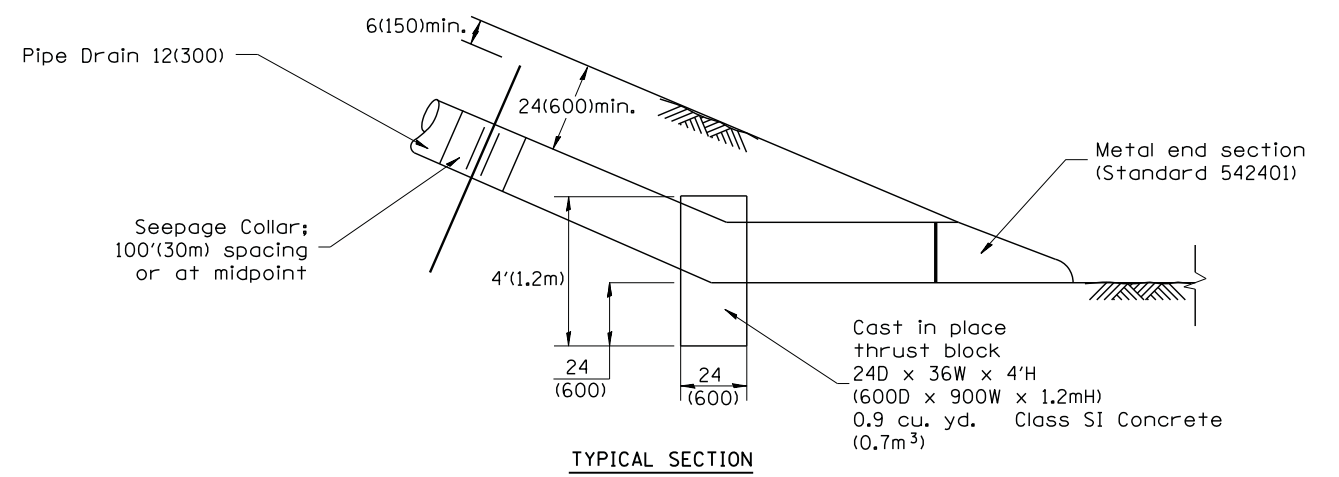
F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
74	48-[25B]BR, BR-1J	KNOX	131	72
CONTRACT NO. 68B85				
FED. ROAD DIST. NO.		ILLINOIS FED. AID PROJECT		



TYPICAL SECTION WITH EROSION CONTROL CURB AT INLETS TYPE E AND F (STANDARD 610001)



TYPICAL SECTION AT BRIDGE APPROACHES INLETS TYPE B, C AND D (STANDARDS 609001 OR 609006)



TYPICAL SECTION

GENERAL NOTES

1. The material for Pipe Drains shall be bituminous coated corrugated steel culvert pipe or bituminous coated corrugated aluminum alloy pipe in accordance with Article 601.02(b) or 601.02(e).
2. An approved mastic material (Article 1055.01) shall be applied to the inside of the connecting bands.

All dimensions are in inches (millimeters) unless otherwise noted.

QUANTITIES	
CALC. BY: _____	DATE: _____
CHECKED BY: _____	DATE: _____
QUANTITY CALCULATIONS ARE ON FILE AT THE DISTRICT 4 OFFICE; BUREAU OF PROJECT IMPLEMENTATION; DOCUMENTATION SECTION	

01-01-97	RENUM. H-1.04, NEW REVISION BOX, REVISED TITLE BOX, REVISED DESIGNER NOTES, ADDED QUANTITY	T.P.	
10-16-06	REVISION TO 2007 SPEC.	M.A.	

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SLOPE DRAIN DETAILS FOR BURIED PIPES

NOT TO SCALE

CADD STD. 601101-D4

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
74	48-[125B]BR, BR-1J	KNOX	131	73
CONTRACT NO. 68B85				
FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT				

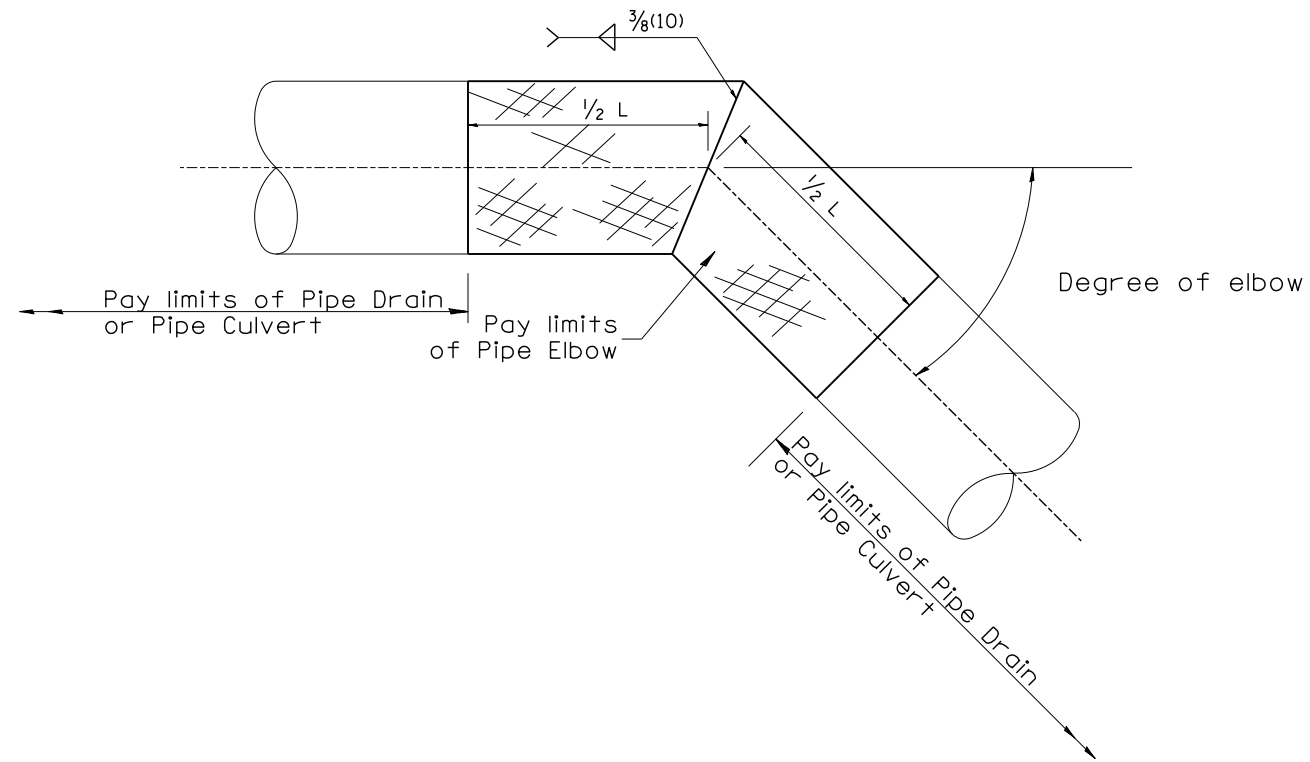
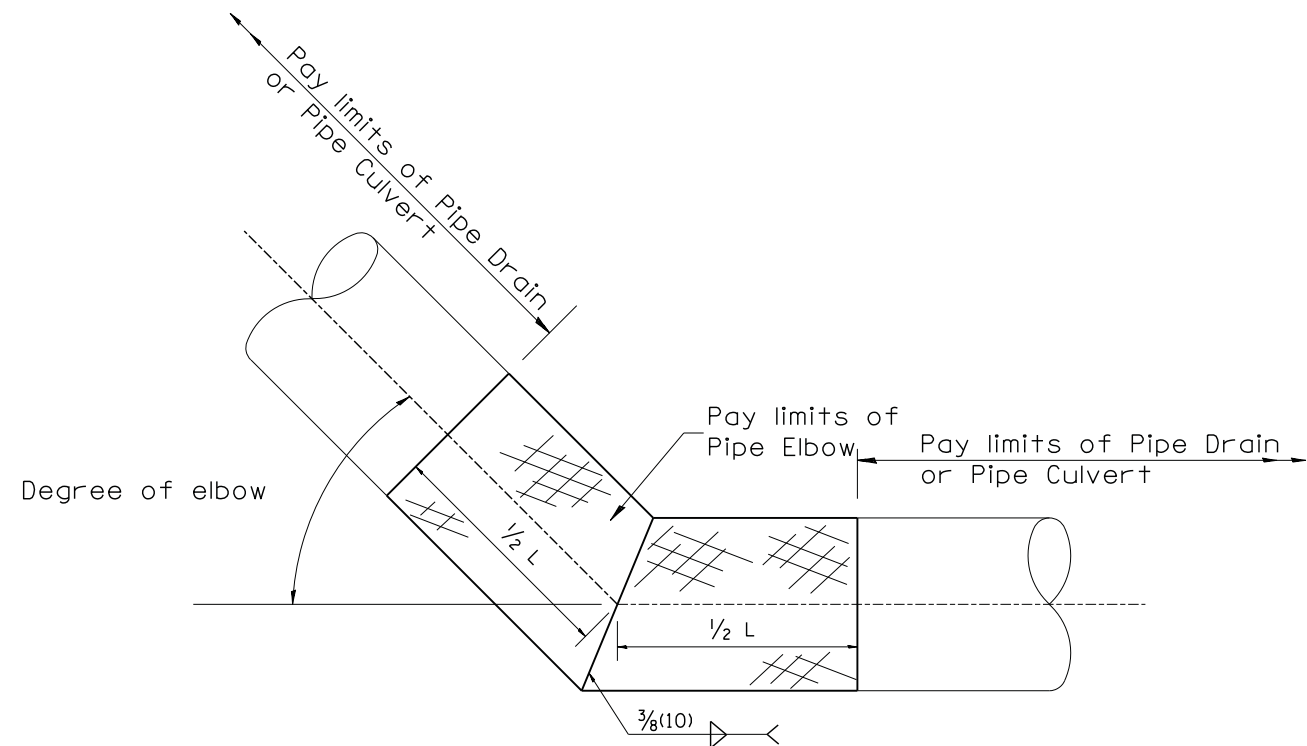


TABLE A ELBOW DESIGN CONTROLS		
PIPE DIAMETER	L = Pay limits of Pipe Elbow and minimum length of pipe required for fabrication	
	DEGREE OF ELBOW ≤ 45°	DEGREE OF ELBOW ≥ 46°
12(300)	24(600)	4'(1.22M)
15(375)	24(600)	4'(1.22M)
18(450)	24(600)	4'(1.22M)
21(525)	24(600)	4'(1.22M)
24(600)	4'(1.22M)	4'(1.22M)
30(750)	4'(1.22M)	6'(1.83M)
36(900)	4'(1.22M)	6'(1.83M)

TABLE B ELBOW DESIGN CONTROLS	
EARTH SLOPE (V:H)	DEGREE OF ELBOW *
1:6	9°
1:4	14°
1:3	18°
1:2	26°
1:1 1/2	33°

* Approximate - based upon 0.5% inlet and outlet flowlines.



All dimensions are in inches (millimeters) unless otherwise noted.

01-01-97	RENUM. J-11.05, NEW REVISION BOX,	T.P.
	REVISED TITLE BOX	
10-16-06	REVISED TO 2007 SPEC.	M.A.

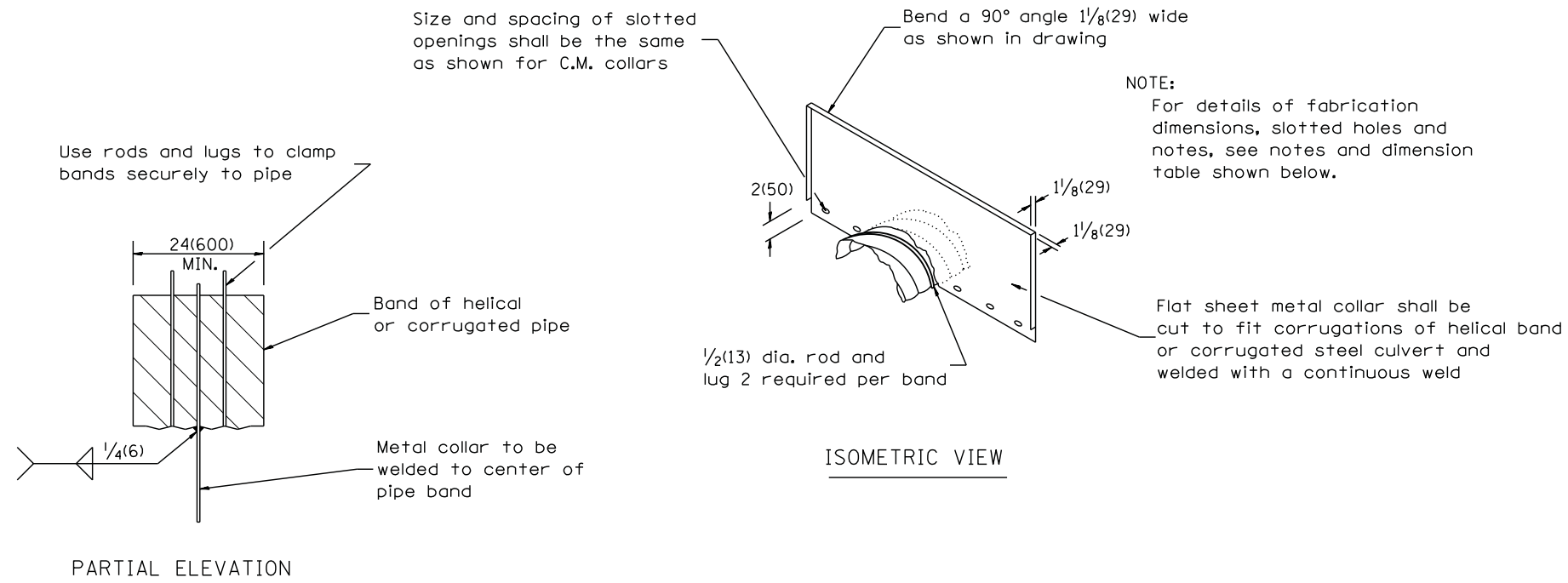
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

NOT TO SCALE

PIPE ELBOW

CADD STD. 601301-D4

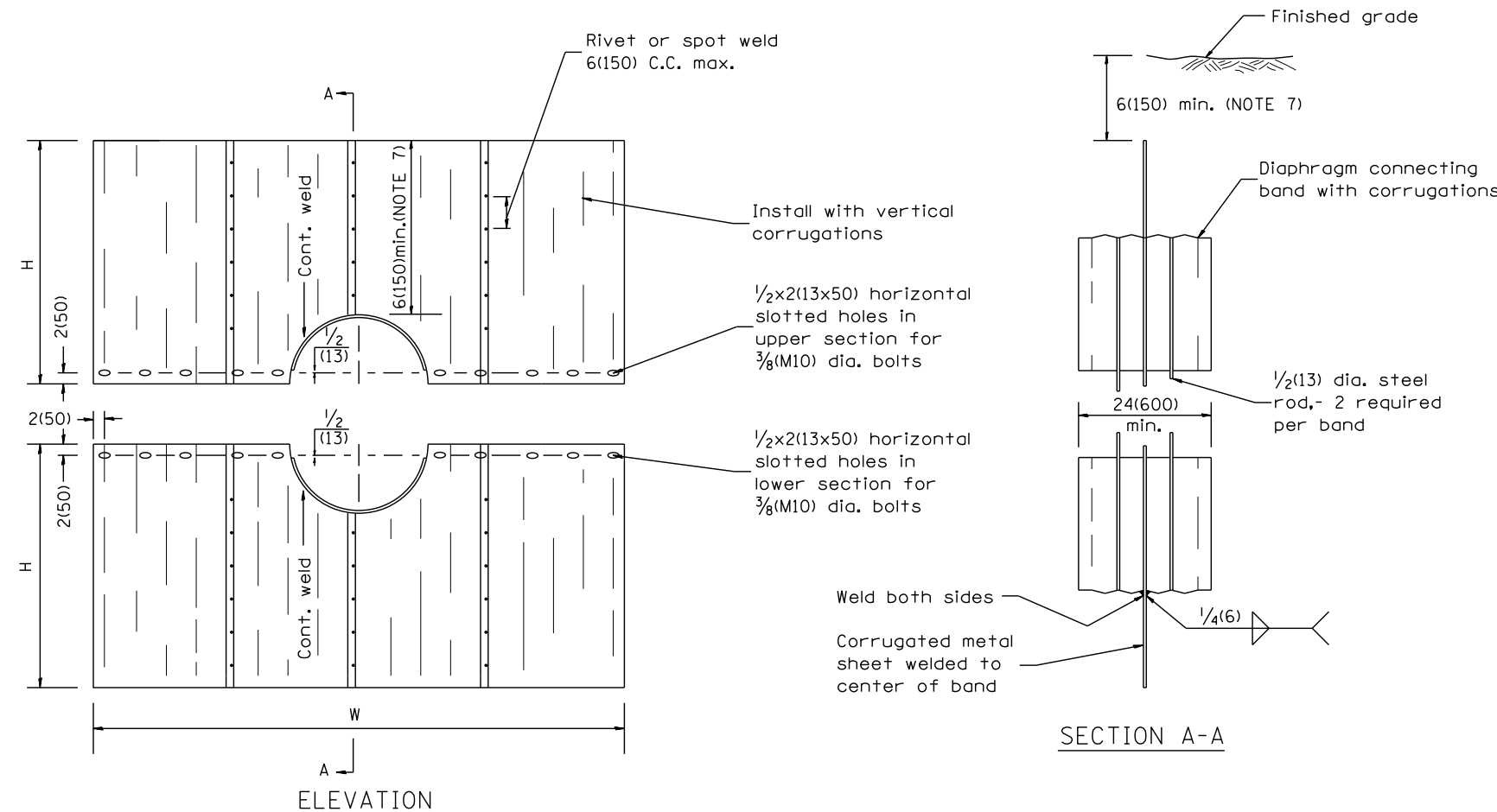
F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
74	48-[25B]BR, BR-1J	KNOX	131	74
CONTRACT NO. 68B85				
FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT				



DETAILS OF CORRUGATED PIPE COLLAR

NOTES FOR COLLARS:

1. Materials and coatings for all collars shall be the same as that specified for the pipe.
2. Collars shall be shop fabricated, assembled and marked by painting to identify matching half sections of each collar.
3. The laps between the half sections and between the pipe and connecting bands shall be caulked with fiberized asphalt mastic at the time of installation.
4. All tank lugs, rods, and nuts shall be galvanized steel. Where aluminum collars are used, The rods and lugs shall be separated from the aluminum bands. By at least two (2) layers of 2(50) wide plastic tape with a total thickness of 2 1/4 mils or more.
5. The collars shall be welded to the connecting bands as shown on the drawings, all welds shall be treated as specified for class I, II, and III welds, miscellaneous. (Refer to AWS Standard Specifications)
6. Bands shall be fabricated from material having the same class of corrugations as the pipe to which it is to be attached.
7. Upper half of sheet may be cut shorter to provide 6(150) min. earth cover.



SEEPAGE COLLAR DIMENSION TABLE

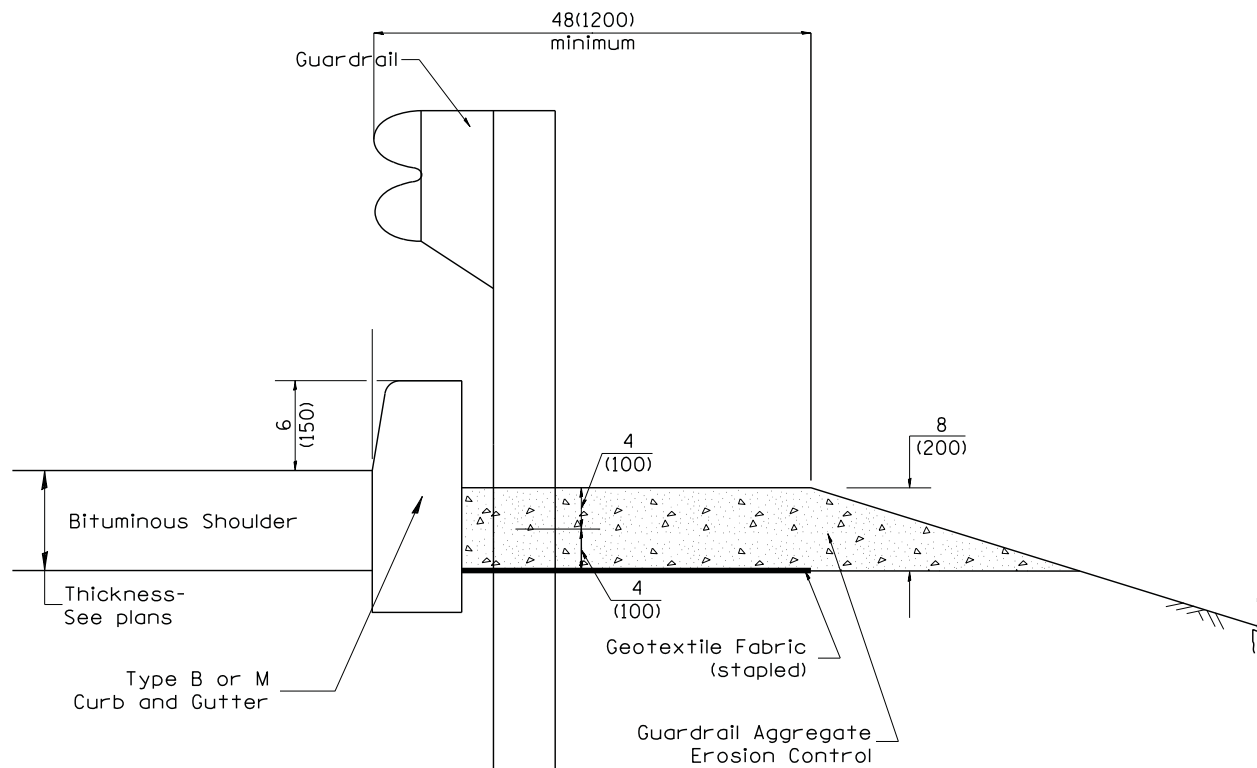
PIPE DIAMETER	NOMINAL COLLAR SIZE	FABRICATIONS DIMENSIONS	
		W(WIDTH)	H(HEIGHT)
12(300) 15(375), 18(450) 21(525), 24(600)	8'x6' (2.4m x 1.8m)	8'-0" (2.44m)	38(966)
27(675) 30(750)	8'x7' (2.4m x 2.1m)	8'-0" (2.44m)	3'-8" (1.12m)
36(900), 42(1050) 48(1200)	10'x7' (3.0m x 2.1m)	10'-0" (3.05m)	3'-8" (1.12m)

Collar dimensions shown may be increased to allow fabrication from standard size sheets.

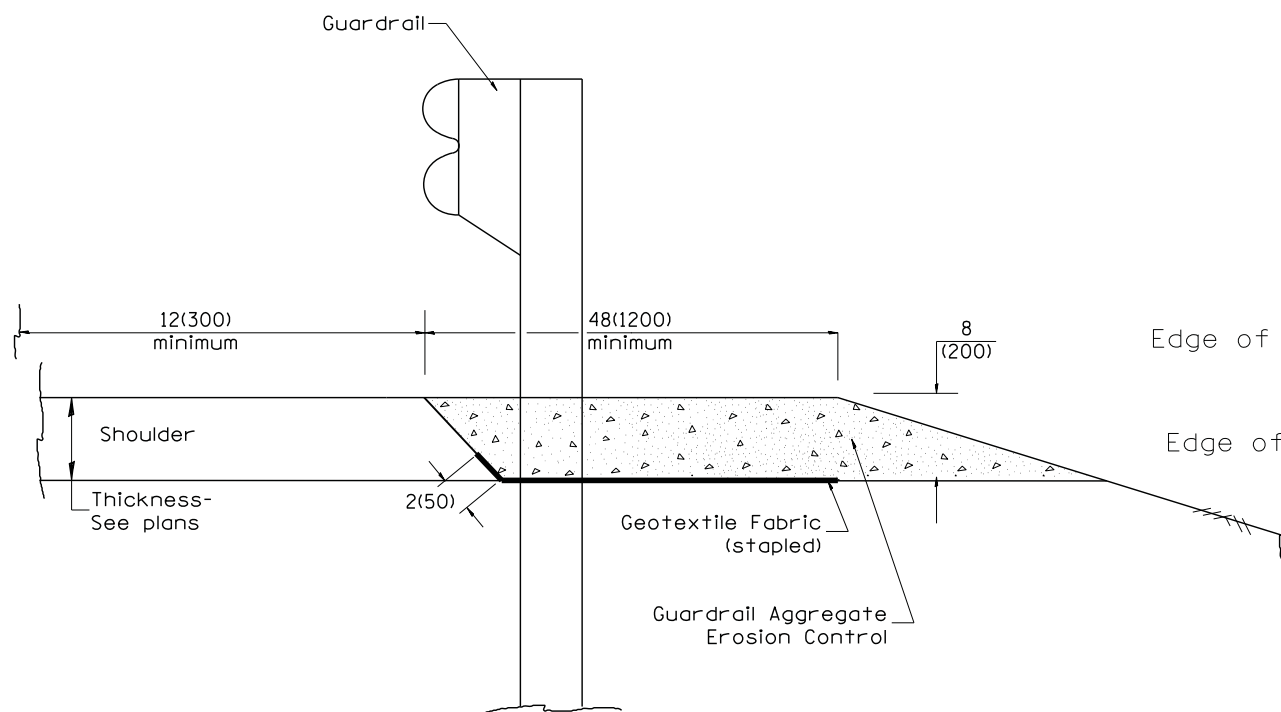
DETAILS OF SEEPAGE COLLAR

SEEPAGE COLLAR SPACING
Less than 24(600) pipe: 100' (30m) spacing or midpoint
Equal to or greater than 24(600) pipe: 80' (24m) spacing or midpoint

All dimensions are in inches (millimeters) unless otherwise noted.



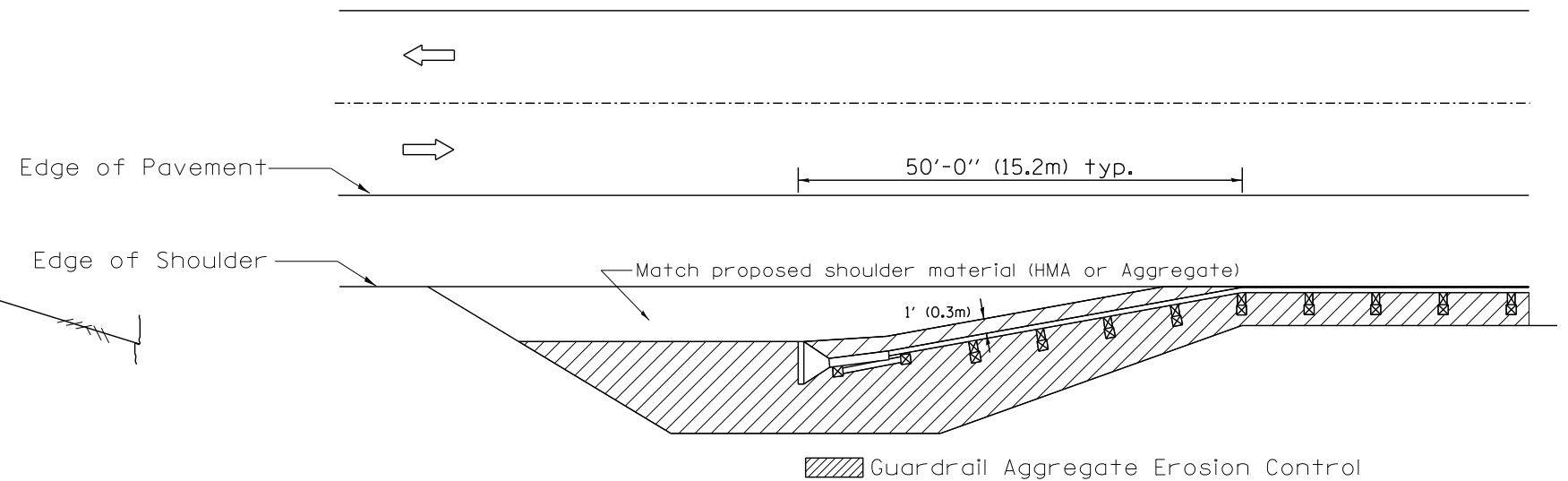
TYPICAL SECTION WITH EROSION CONTROL CURB



TYPICAL SECTION WITHOUT EROSION CONTROL CURB

GENERAL NOTES: GUARDRAIL AGGREGATE EROSION CONTROL

1. This work shall consist of grading as needed, furnishing and installing geotextile fabric and staples, and furnishing, placing and shaping crushed aggregate around and behind Steel Plate Beam Guardrail posts in accordance with Plan Details.
2. Before placing the aggregate and the Geotextile Fabric, weeds and grass shall be removed from the area to be covered.
3. After the area has been prepared, and in a dry condition, the Geotextile fabric shall be placed with a 12(300) minimum overlap. A knife cut for guardrail post installation is necessary.
4. The aggregate shall be deposited, compacted and shaped by either mechanical or hand methods, in a manner reasonably true to line and grade.
5. The Contractor shall have the option of placing the guardrail before or after the Geotextile Fabric and Aggregate are in place. If the guardrail is placed after the Geotextile Fabric and Aggregate, then any voids must be filled and the aggregate returned to line and grade.
6. Materials shall meet the following requirements:
 - A. The crushed aggregate shall be CA1 gradation in accordance with Article 1004.01(c) of the Standard Specifications.
 - B. The Geotextile Fabric shall be nonwoven fabric in accordance with Article 1080.02 of the Standard Specifications.



All dimensions are in inches (millimeters) unless otherwise noted.

01-01-97	RENUM. C-22.01, NEW REVISION BOX	T.P.	3-7-11	Added Detail showing plan view	R.D.
03-01-97	CORRECT STD. NUMBERS IN NOTES PG. 2	J.A.	8-10-12	Revised curb "B" and aggregate	R.D.
11-03-00	CORRECTION TO NOTES	M.A.			
10-16-06	REVISED TO 2007 SPEC.	M.A.			

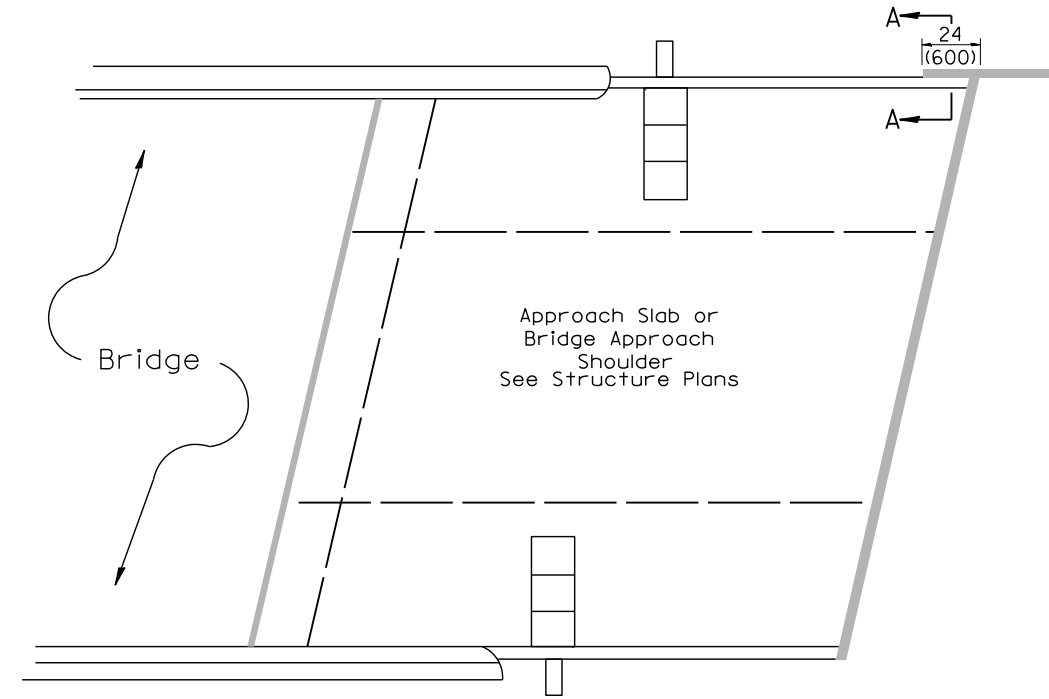
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

GUARDRAIL EROSION CONTROL TREATMENTS

NOT TO SCALE

SHT. 1 OF 2
CADD STD. 630101-D4

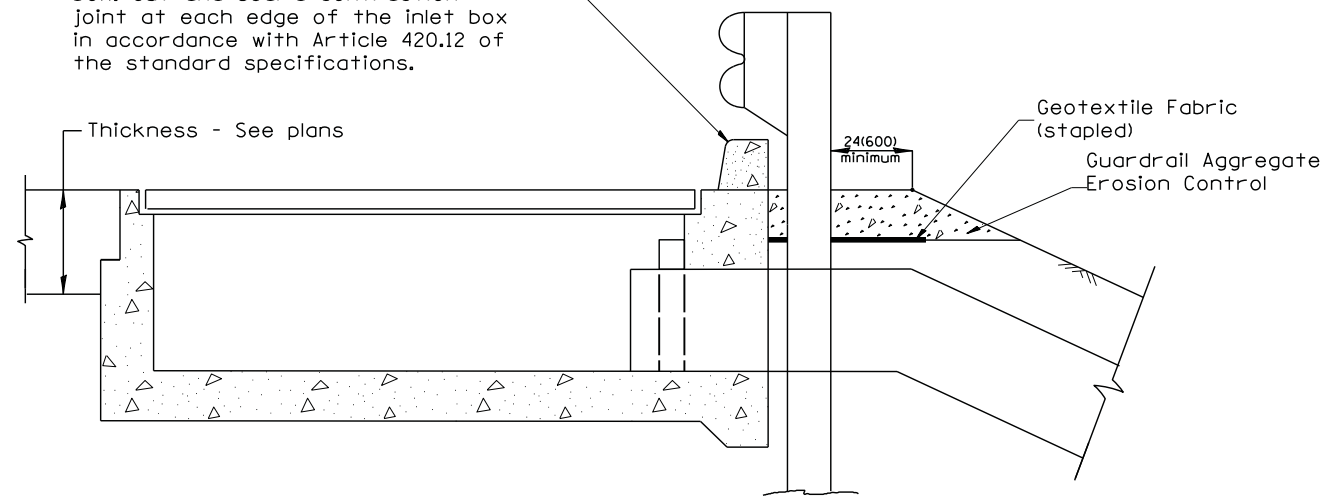
F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
74	48-[125B/BR, BR-1]	KNOX	131	76
FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT			CONTRACT NO. 68B85	



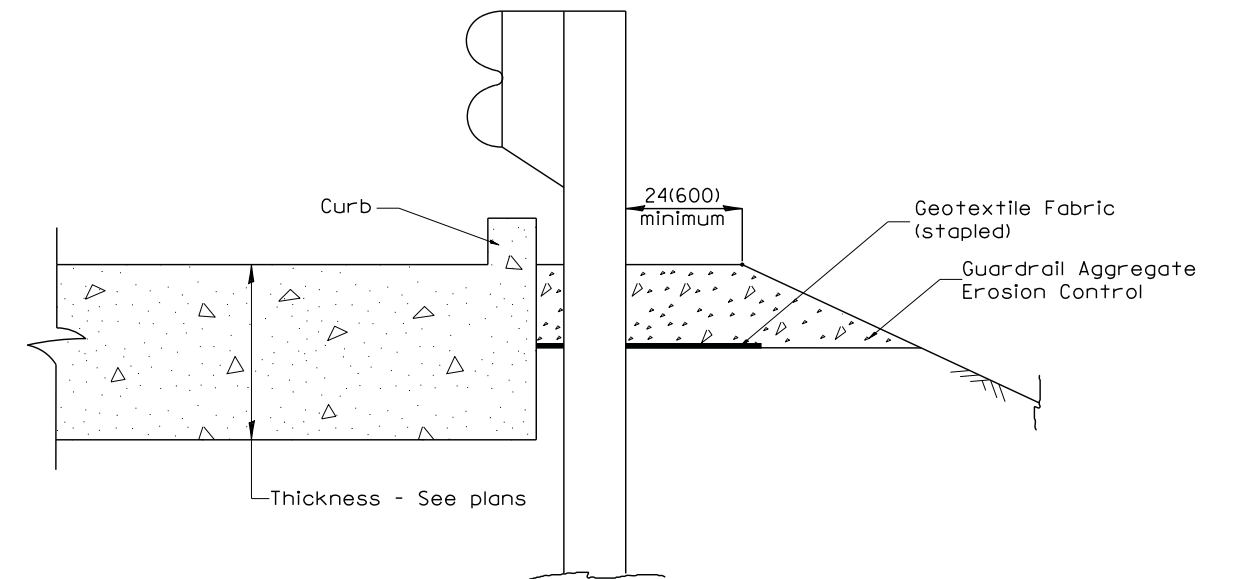
PLAN VIEW

APPROACH SLAB OR BRIDGE APPROACH SHOULDER
(STANDARD 609001 or 609006)

Type B or M curb.
Install an 8' long #4 epoxy coated reinforcement bar across the inlet box. Cut and seal a contraction joint at each edge of the inlet box in accordance with Article 420.12 of the standard specifications.



TYPICAL SECTION AT INLETS
TYPE E & F (STANDARD 610001)



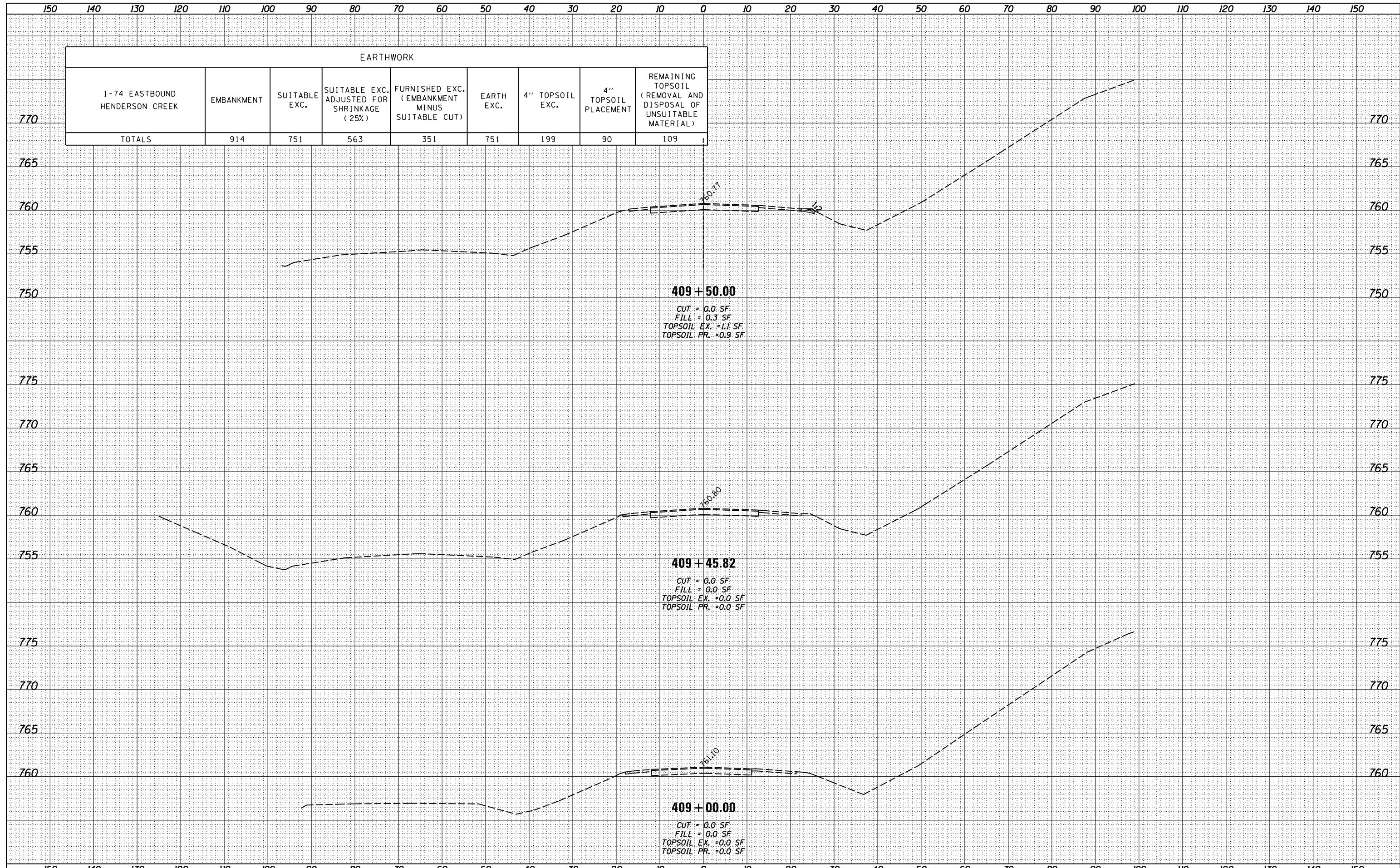
SECTION A-A
TYPICAL SECTION WITH BRIDGE APPROACH CURB

All dimensions are in inches (millimeters) unless otherwise noted.

				STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION		GUARDRAIL EROSION CONTROL TREATMENTS				SHT. 2 OF 2 CADD STD. 630101-D4	
F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.							
74	48-[25B/BR, BR-1]	KNOX	131	77							
				CONTRACT NO. 68B85							
FED. ROAD DIST. NO.		ILLINOIS		FED. AID PROJECT							

DATE	
BY	
SURVEYED	
PLOTTED	
TEMPLATE	
AREAS	
CHECKED	
FINL SURVEY	
NOTE BOOK	
NO.	

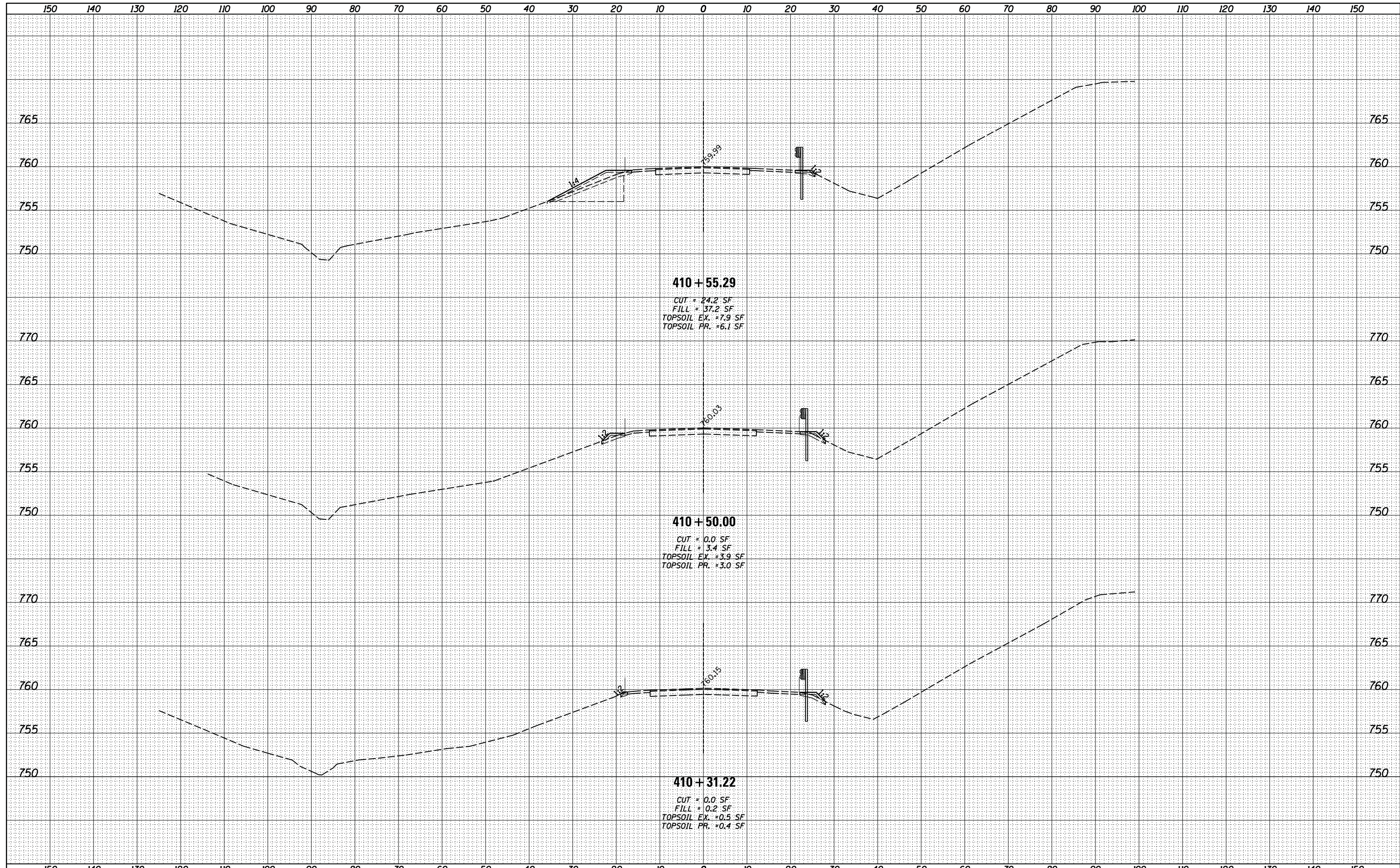
DATE	
BY	
SURVEYED	
PLOTTED	
TEMPLATE	
AREAS	
CHECKED	
ORIGINAL SURVEY	
NOTE BOOK	
NO.	



EARTHWORK								
I-74 EASTBOUND HENDERSON CREEK	EMBANKMENT	SUITABLE EXC.	SUITABLE EXC. ADJUSTED FOR SHRINKAGE (25%)	FURNISHED EXC. (EMBANKMENT MINUS SUITABLE CUT)	EARTH EXC.	4" TOPSOIL EXC.	4" TOPSOIL PLACEMENT	REMAINING TOPSOIL (REMOVAL AND DISPOSAL OF UNSUITABLE MATERIAL)
TOTALS	914	751	563	351	751	199	90	109

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410 + 55.29
 CUT = 24.2 SF
 FILL = 37.2 SF
 TOPSOIL EX. = 7.9 SF
 TOPSOIL PR. = 6.1 SF

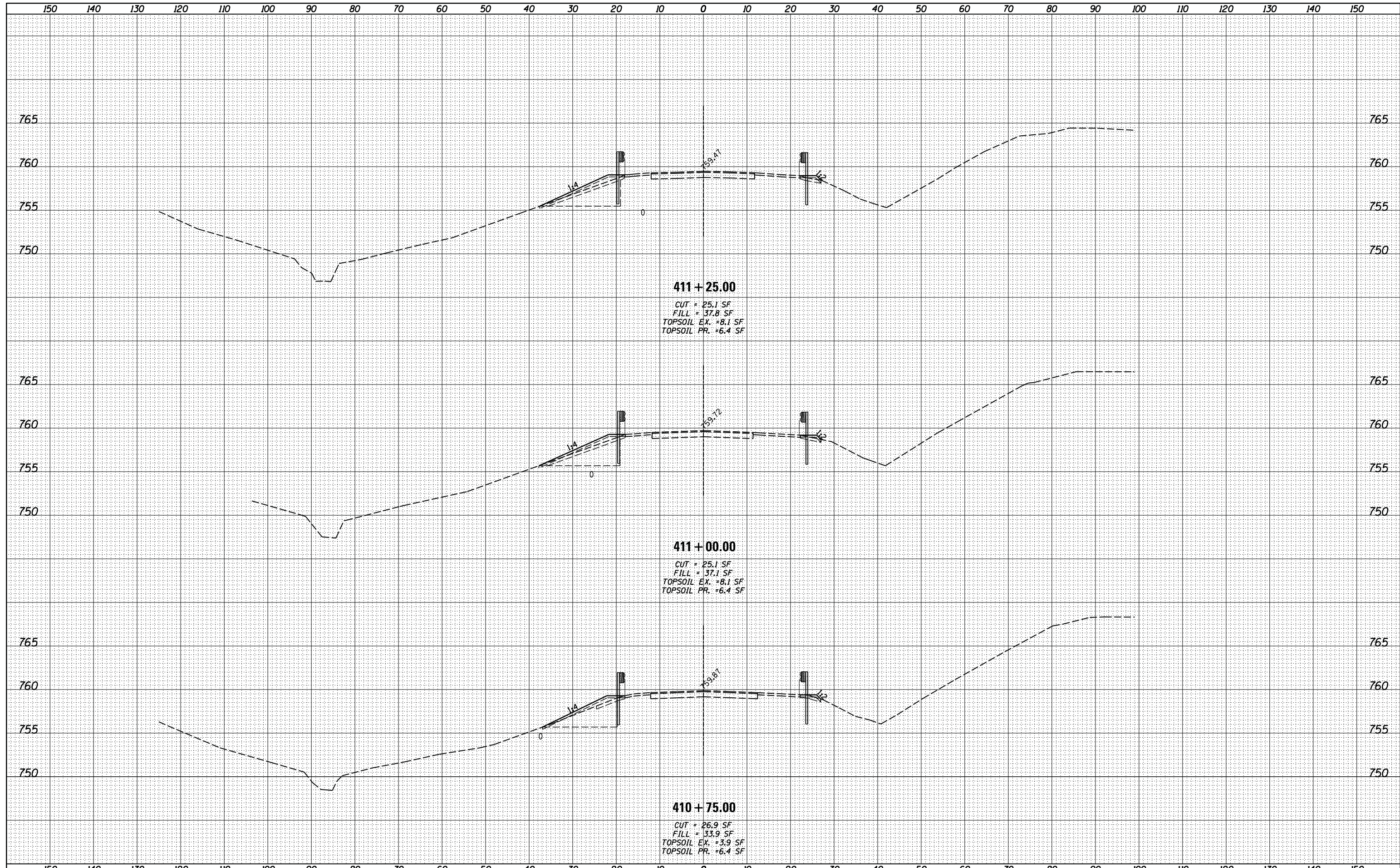
410 + 50.00
 CUT = 0.0 SF
 FILL = 3.4 SF
 TOPSOIL EX. = 3.9 SF
 TOPSOIL PR. = 3.0 SF

410 + 31.22
 CUT = 0.0 SF
 FILL = 0.2 SF
 TOPSOIL EX. = 0.5 SF
 TOPSOIL PR. = 0.4 SF

FILE NAME =	USER NAME = *USER*	DESIGNED - JCP	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	I-74 EASTBOUND CROSS SECTIONS			F.A.I. RTÉ.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
FILEL		DRAWN - TLM	REVISED -		74	48-[125B]BR, BR-1J	KNOX	131	80			
PLOT SCALE = *SCALE*		CHECKED - GAO	REVISED -		CONTRACT NO. 68B85							
MODELNAME	PLOT DATE = 7/30/2014	DATE - 02/22/2013	REVISED -		ILLINOIS FED. AID PROJECT							
				SCALE: N/A	SHEET 3	OF 17 SHEETS	STA. 410+31.22	TO STA. 410+55.29				

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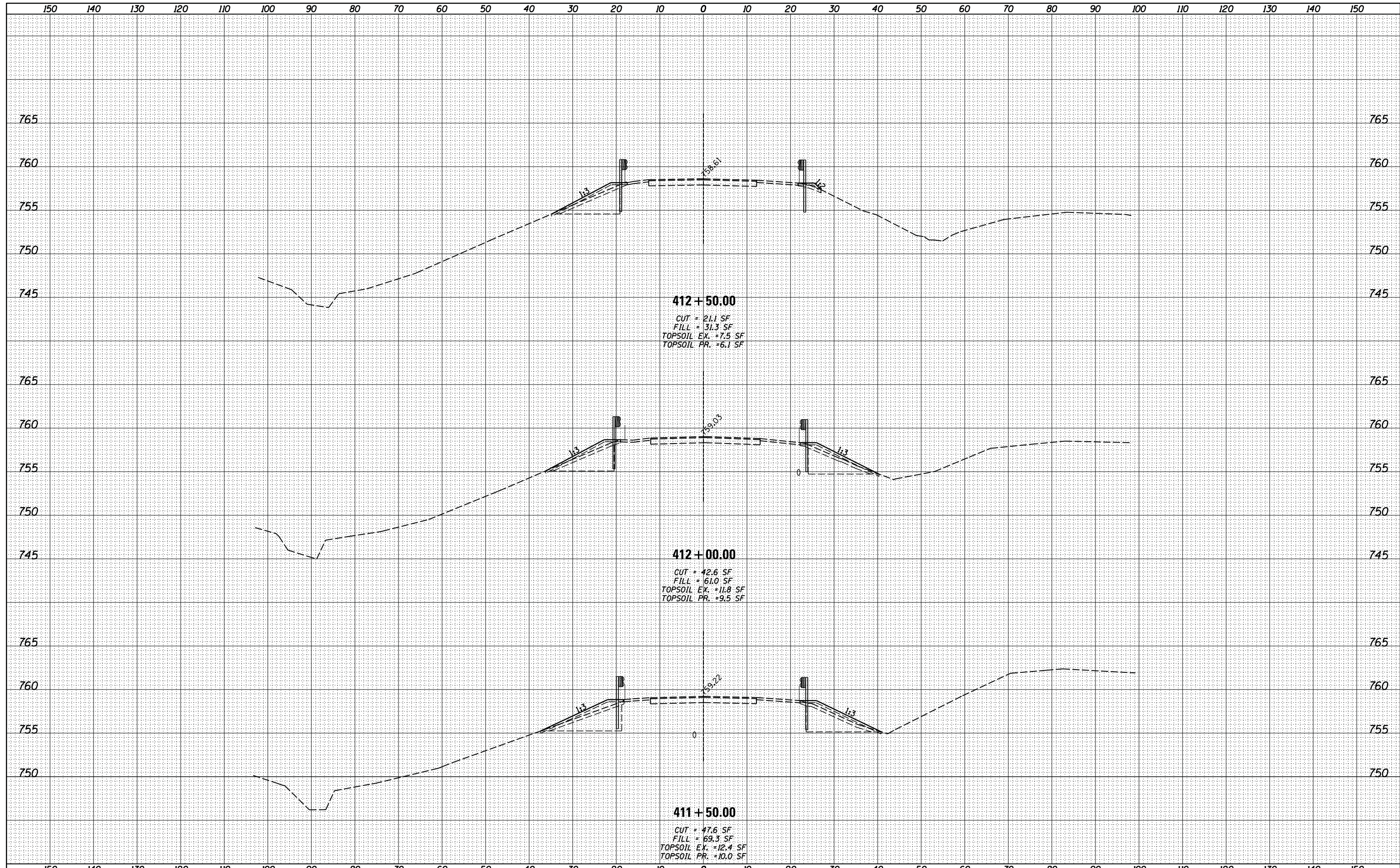
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FILE NAME =	USER NAME = *USERS*	DESIGNED - JCP	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	I-74 EASTBOUND CROSS SECTIONS			F.A.I. RTE. 74	SECTION 48-[25B]BR, BR-1J	COUNTY KNOX	TOTAL SHEETS 131	SHEET NO. 81
FILEL		DRAWN - TLM	REVISED -		SCALE: N/A	SHEET 4	OF 17 SHEETS	STA. 410+75.00	TO STA. 411+25.00	CONTRACT NO. 68B85		
		CHECKED - GAO	REVISED -		ILLINOIS FED. AID PROJECT							
MODELNAME	PLOT DATE = 7/30/2014	DATE - 02/22/2013	REVISED -									

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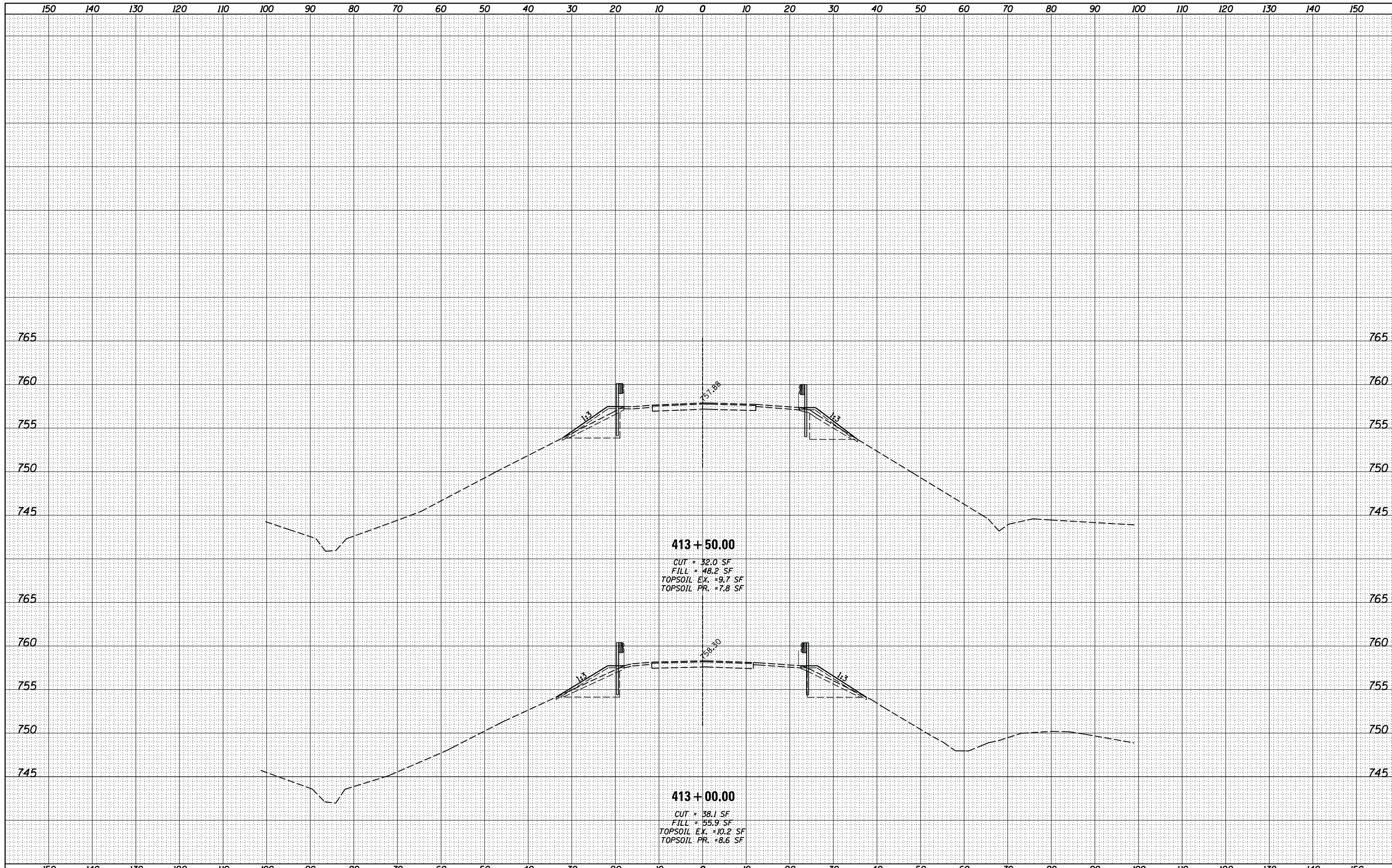
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FILE NAME =	USER NAME = *USERS*	DESIGNED - JCP	REVISSED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	I-74 EASTBOUND CROSS SECTIONS		F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
FILEL		DRAWN - TLM	REVISSED -				74	48-[25B]BR, BR-1J	KNOX	131	82	
		CHECKED - GAO	REVISSED -		SCALE: N/A		SHEET 5 OF 17 SHEETS		STA. 411+50.00 TO STA. 412+50.00		CONTRACT NO. 68B85	
MODELNAME	PLOT DATE = 7/30/2014	DATE - 02/22/2013	REVISSED -		ILLINOIS FED. AID PROJECT							

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NOTE BOOK
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413 + 50.00
 CUT = 32.0 SF
 FILL = 48.2 SF
 TOPSOIL EX. = 9.7 SF
 TOPSOIL PR. = 7.8 SF

413 + 00.00
 CUT = 38.1 SF
 FILL = 55.9 SF
 TOPSOIL EX. = 10.2 SF
 TOPSOIL PR. = 8.6 SF

FILE NAME =
 FILEL
 MODELNAME

USER NAME = *USERS*
 PLOT SCALE = *SCALE*
 PLOT DATE = 7/30/2014

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 DRAWN - TLM
 CHECKED - GAO
 DATE - 02/22/2013

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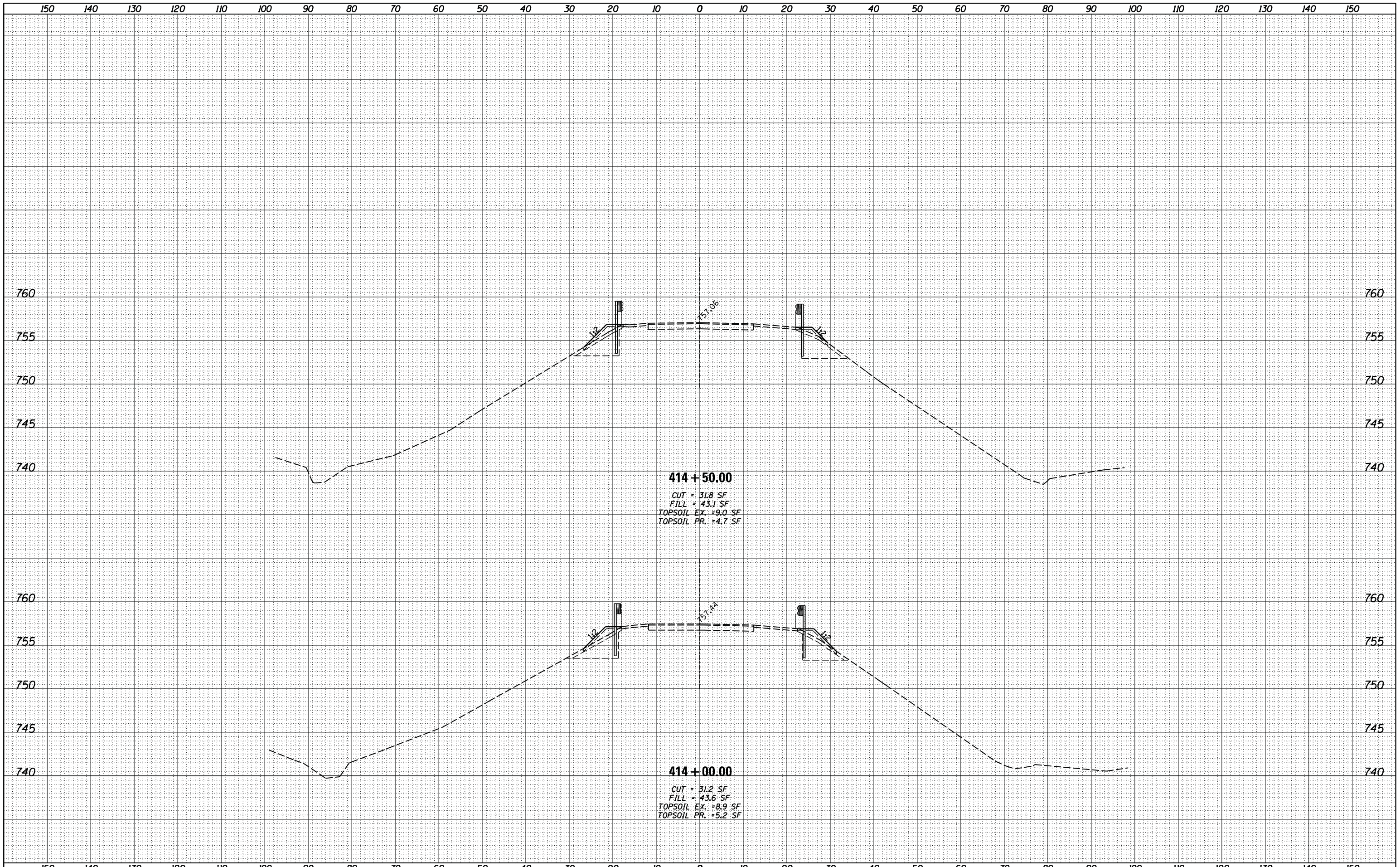
STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

I-74 EASTBOUND CROSS SECTIONS
 SCALE: N/A SHEET 6 OF 17 SHEETS STA. 413+00.00 TO STA. 413+50.00

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
74	48-[25B]BR, BR-1J	KNOX	131	83
				CONTRACT NO. 68B85
ILLINOIS FED. AID PROJECT				

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ORIGINAL SURVEY	
NOTE BOOK	
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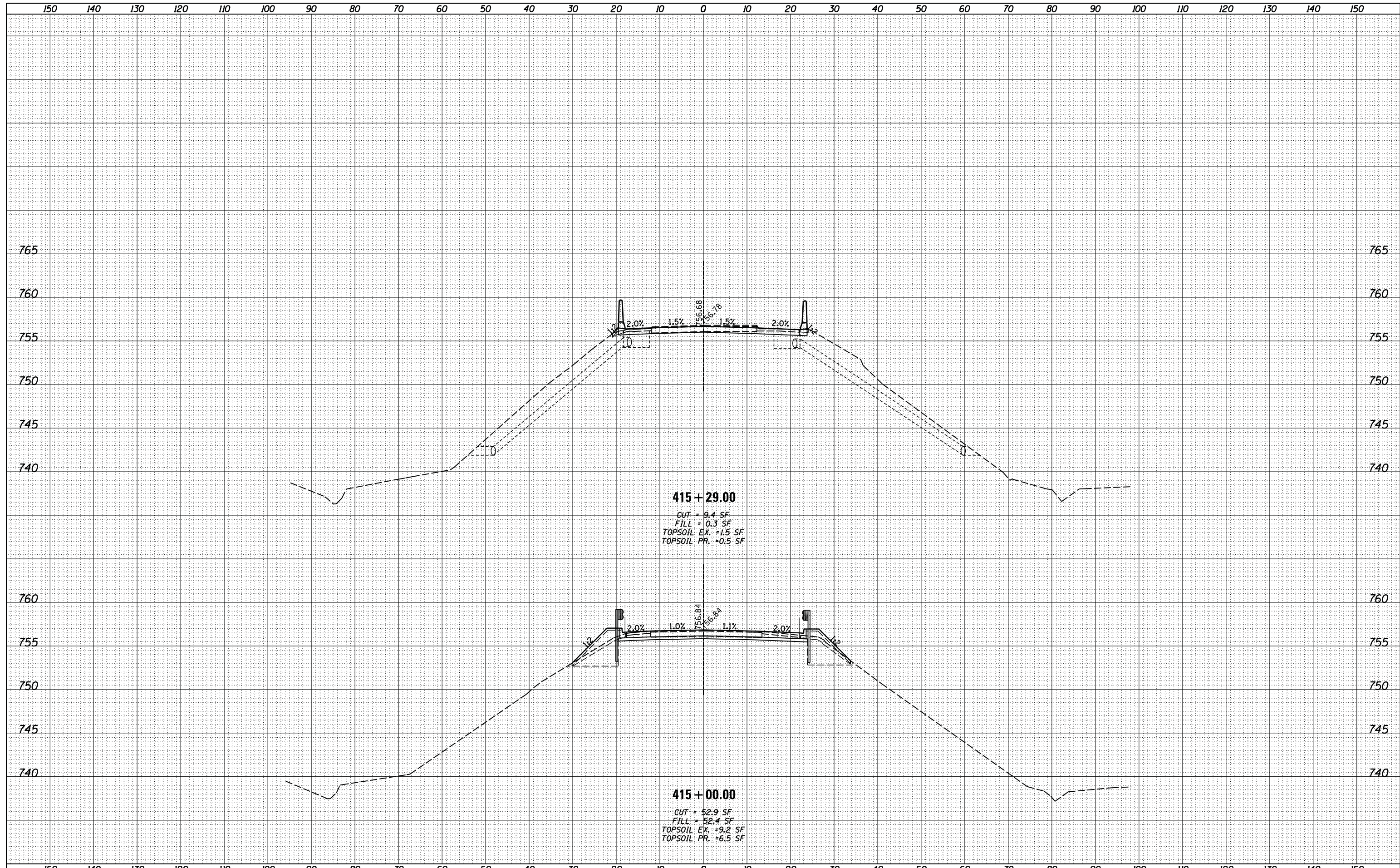
414 + 50.00
CUT = 31.8 SF
FILL = 43.1 SF
TOPSOIL EX. = 9.0 SF
TOPSOIL PR. = 4.7 SF

414 + 00.00
CUT = 31.2 SF
FILL = 43.6 SF
TOPSOIL EX. = 8.9 SF
TOPSOIL PR. = 5.2 SF

FILE NAME	USER NAME	DESIGNED	REVISED	<p align="center">STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION</p> <p align="center">I-74 EASTBOUND CROSS SECTIONS</p>		F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.					
FILEL		DRAWN	REVISED			74	48-[25B]BR, BR-1J	KNOX	131	84					
MODELNAME		CHECKED	REVISED			CONTRACT NO. 68B85									
	PLOT DATE	DATE	REVISED	SCALE:	N/A	SHEET	7	OF	17	SHEETS	STA. 414+00.00	TO	STA. 414+50.00	ILLINOIS FED. AID PROJECT	

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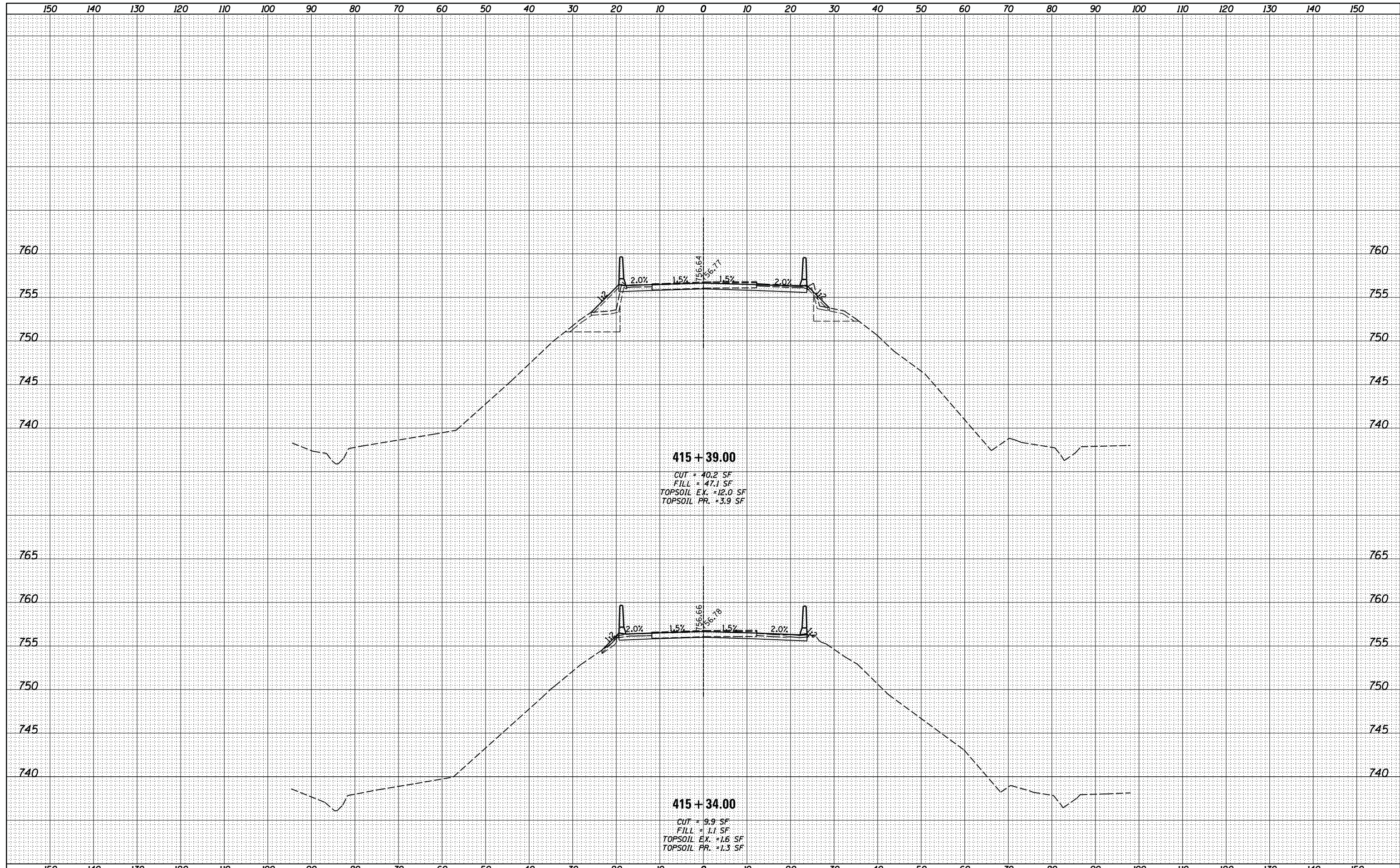
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FILE NAME =	USER NAME = *USERS*	DESIGNED - JCP	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	I-74 EASTBOUND CROSS SECTIONS			F.A.I. RTE. 74	SECTION 48-[125B]BR, BR-1J	COUNTY KNOX	TOTAL SHEETS 131	SHEET NO. 85
FILEL		DRAWN - TLM	REVISED -		SCALE: N/A	SHEET 8	OF 17 SHEETS	STA. 415+00.00	TO STA. 415+29.00	CONTRACT NO. 68B85		
		CHECKED - GAO	REVISED -		ILLINOIS FED. AID PROJECT							
MODELNAME	PLOT DATE = 7/30/2014	DATE - 02/22/2013	REVISED -									

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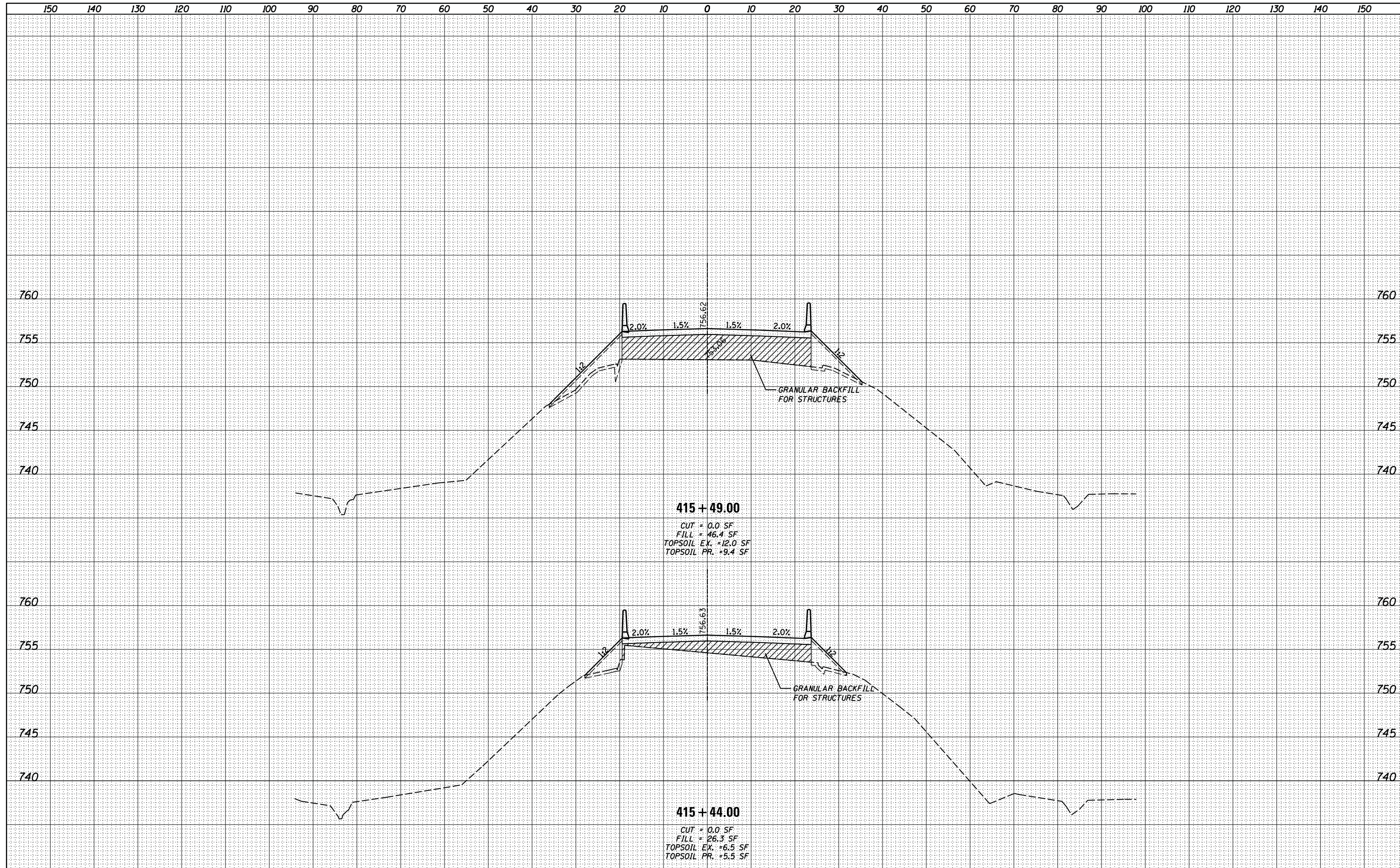
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FILE NAME =	USER NAME = *USERS*	DESIGNED - JCP	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	I-74 EASTBOUND CROSS SECTIONS				F.A.I. RTE. 74	SECTION 48-[125B]BR, BR-1J	COUNTY KNOX	TOTAL SHEETS 131	SHEET NO. 86
FILEL		DRAWN - TLM	REVISED -		SCALE: N/A SHEET 9 OF 17 SHEETS STA. 415+34.00 TO STA. 415+39.00				CONTRACT NO. 68B85				
		CHECKED - GAO	REVISED -		ILLINOIS FED. AID PROJECT								
MODELNAME	PLOT DATE = 7/30/2014	DATE - 02/22/2013	REVISED -										

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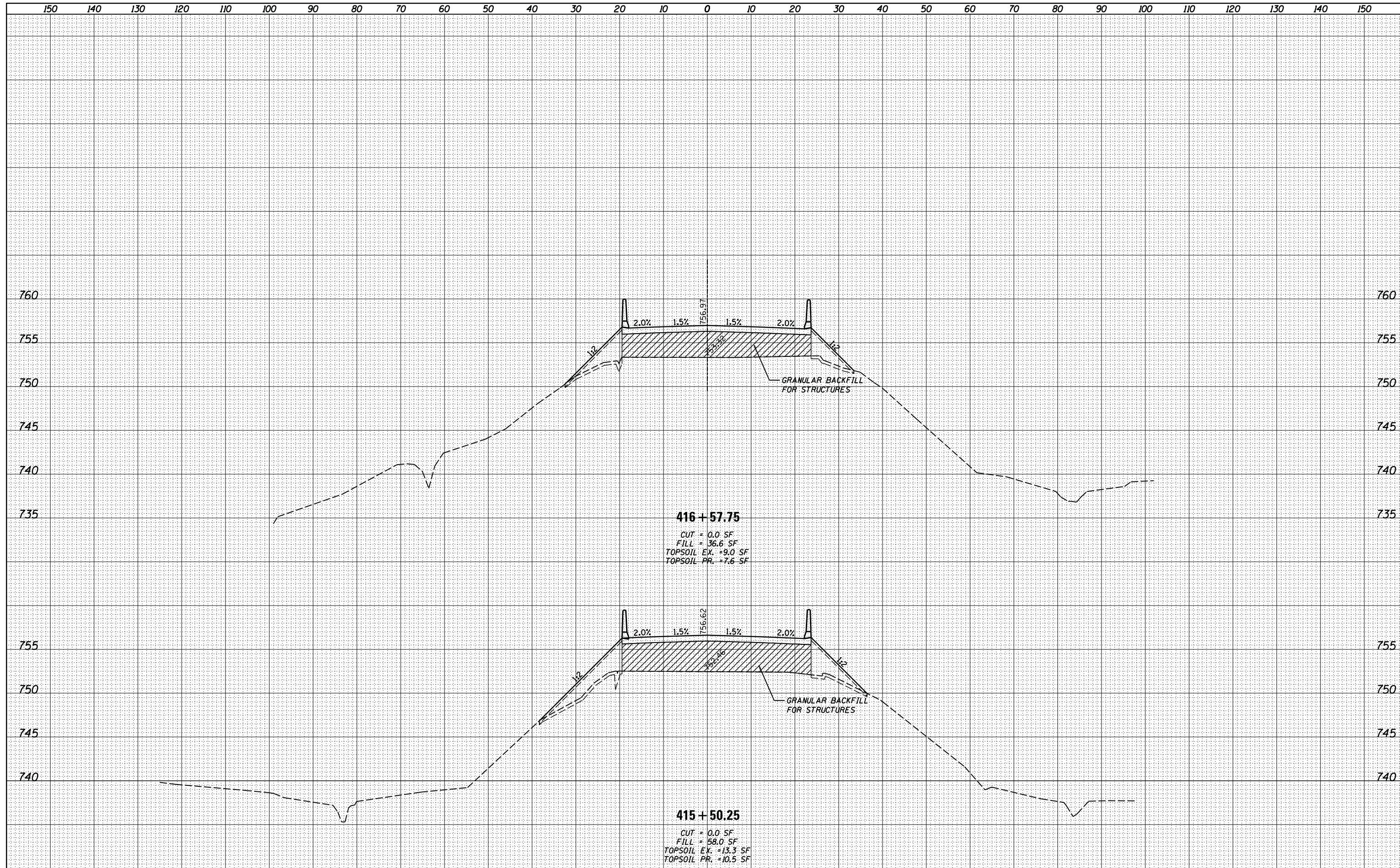
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FILE NAME	USER NAME = *USERS*	DESIGNED - JCP	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	I-74 EASTBOUND CROSS SECTIONS			F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
FILEL		DRAWN - TLM	REVISED -					74	48-[25B]BR, BR-1J	KNOX	131	88	
PLOT SCALE = *SCALE*		CHECKED - GAO	REVISED -		SCALE: N/A			SHEET 11 OF 17 SHEETS			STA. 415+44.00 TO STA. 415+49.00	CONTRACT NO. 68B85	
MODELNAME	PLOT DATE = 7/30/2014	DATE - 02/22/2013	REVISED -		ILLINOIS FED. AID PROJECT								

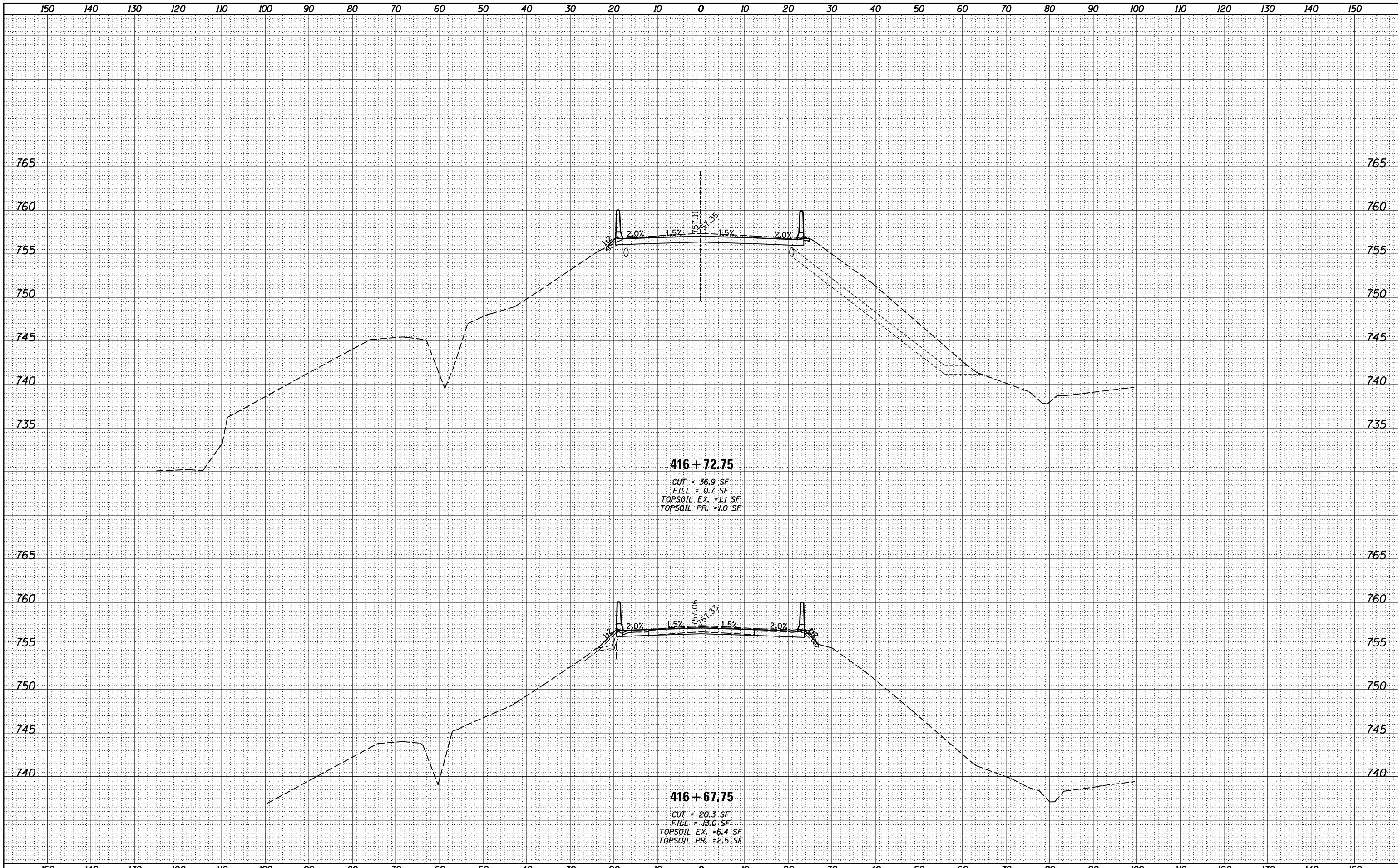
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416 + 57.75
 CUT = 0.0 SF
 FILL = 36.6 SF
 TOPSOIL EX. = 9.0 SF
 TOPSOIL PR. = 7.6 SF

415 + 50.25
 CUT = 0.0 SF
 FILL = 58.0 SF
 TOPSOIL EX. = 13.3 SF
 TOPSOIL PR. = 10.5 SF



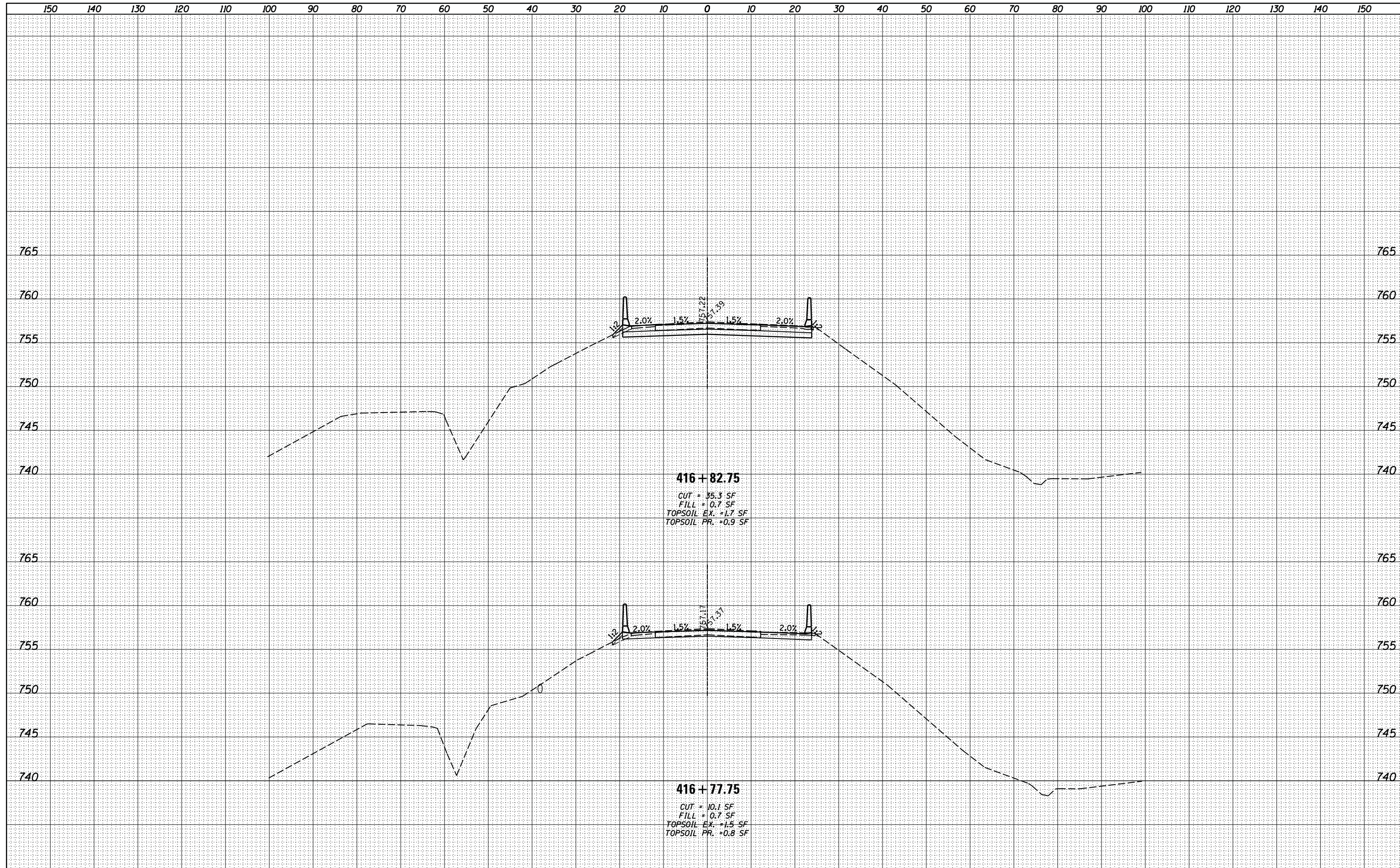
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FINAL SURVEY	SURVEYED
NOTE BOOK	PLOTTED
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FILE NAME =	USER NAME = *USERS*	DESIGNED - JCP	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	I-74 EASTBOUND CROSS SECTIONS	F.A.I. RTE. =	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
FILEL		DRAWN - TLM	REVISED -			74	48-[25B]BR, BR-1J	KNOX	131	91	
		CHECKED - GAO	REVISED -			CONTRACT NO. 68B85					
MODELNAME	PLOT DATE = 7/30/2014	DATE - 02/22/2013	REVISED -			SCALE: N/A	SHEET 14 OF 17 SHEETS	STA. 416+67.75 TO STA. 416+72.75	ILLINOIS FED. AID PROJECT		

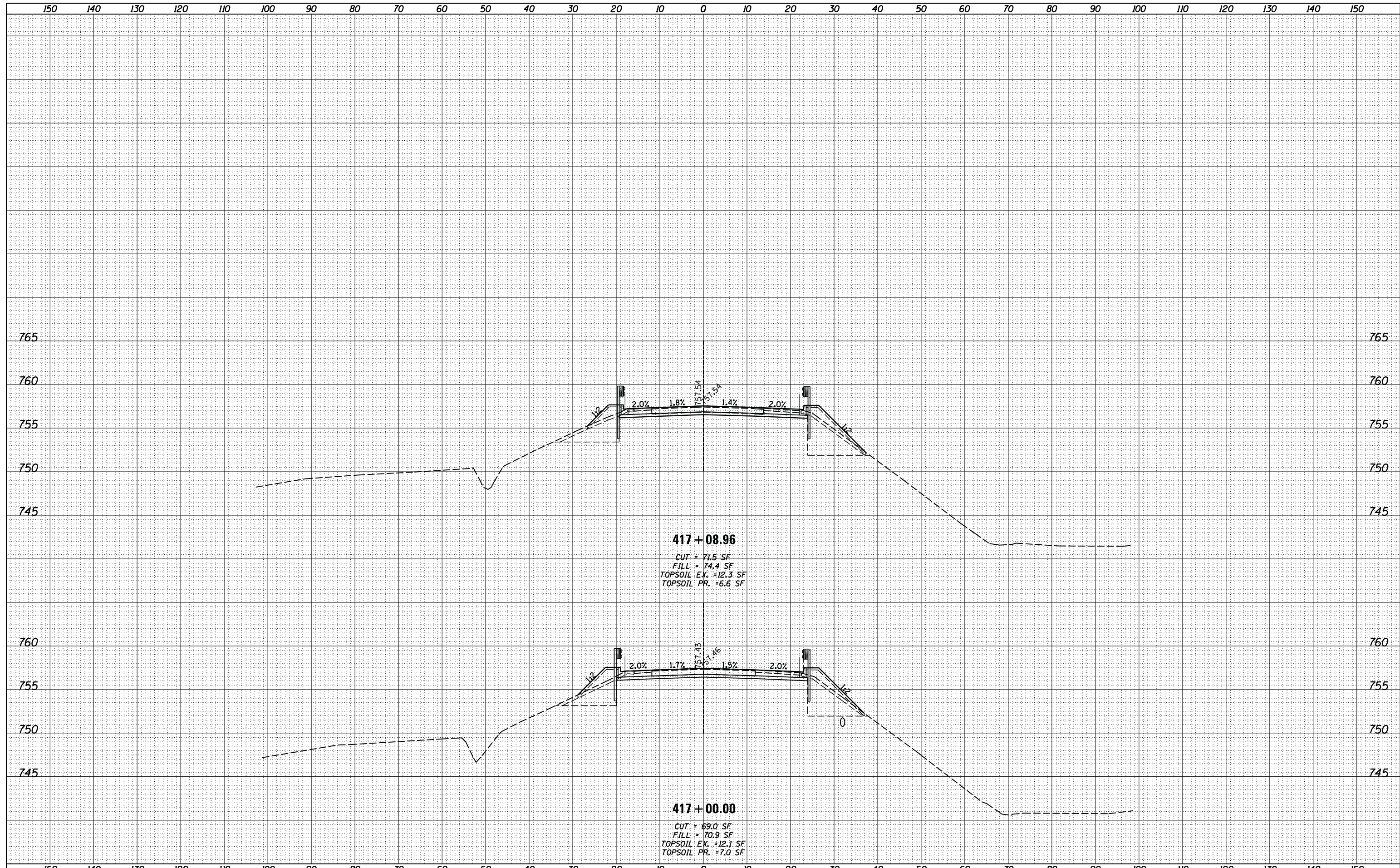
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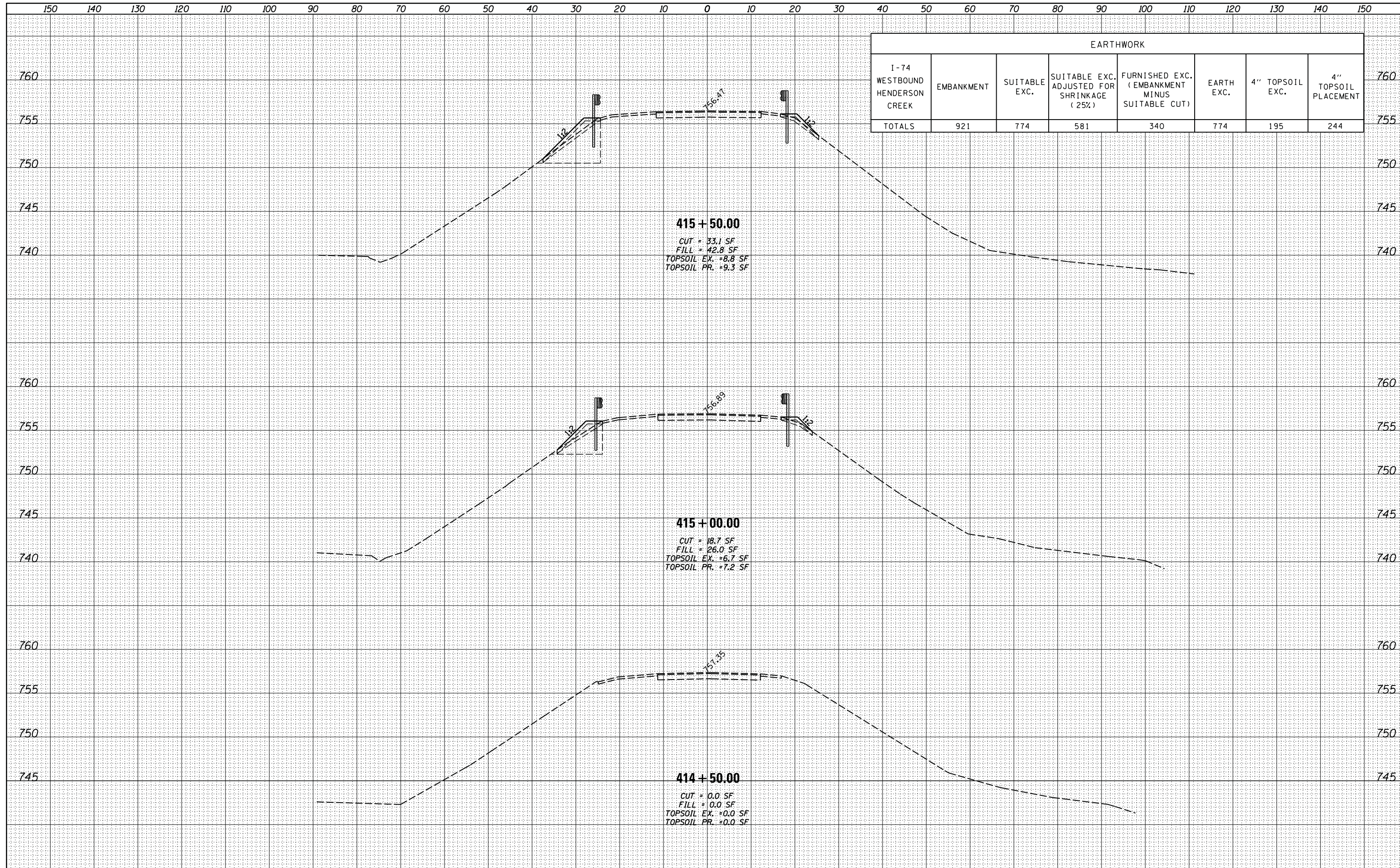


FILE NAME =	USER NAME = *USERS*	DESIGNED - JCP	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	I-74 EASTBOUND CROSS SECTIONS			F.A.I. RTE. 74	SECTION 48-[25B]BR, BR-1J	COUNTY KNOX	TOTAL SHEETS 131	SHEET NO. 93
FILEL	PLOT SCALE = *SCALE*	DRAWN - TLM	REVISED -		SCALE: N/A	SHEET 16	OF 17 SHEETS	STA. 417+00.00	TO STA. 417+08.96	CONTRACT NO. 68B85		
MODELNAME	PLOT DATE = 7/30/2014	CHECKED - GAO	REVISED -		ILLINOIS FED. AID PROJECT							
		DATE - 02/22/2013	REVISED -									

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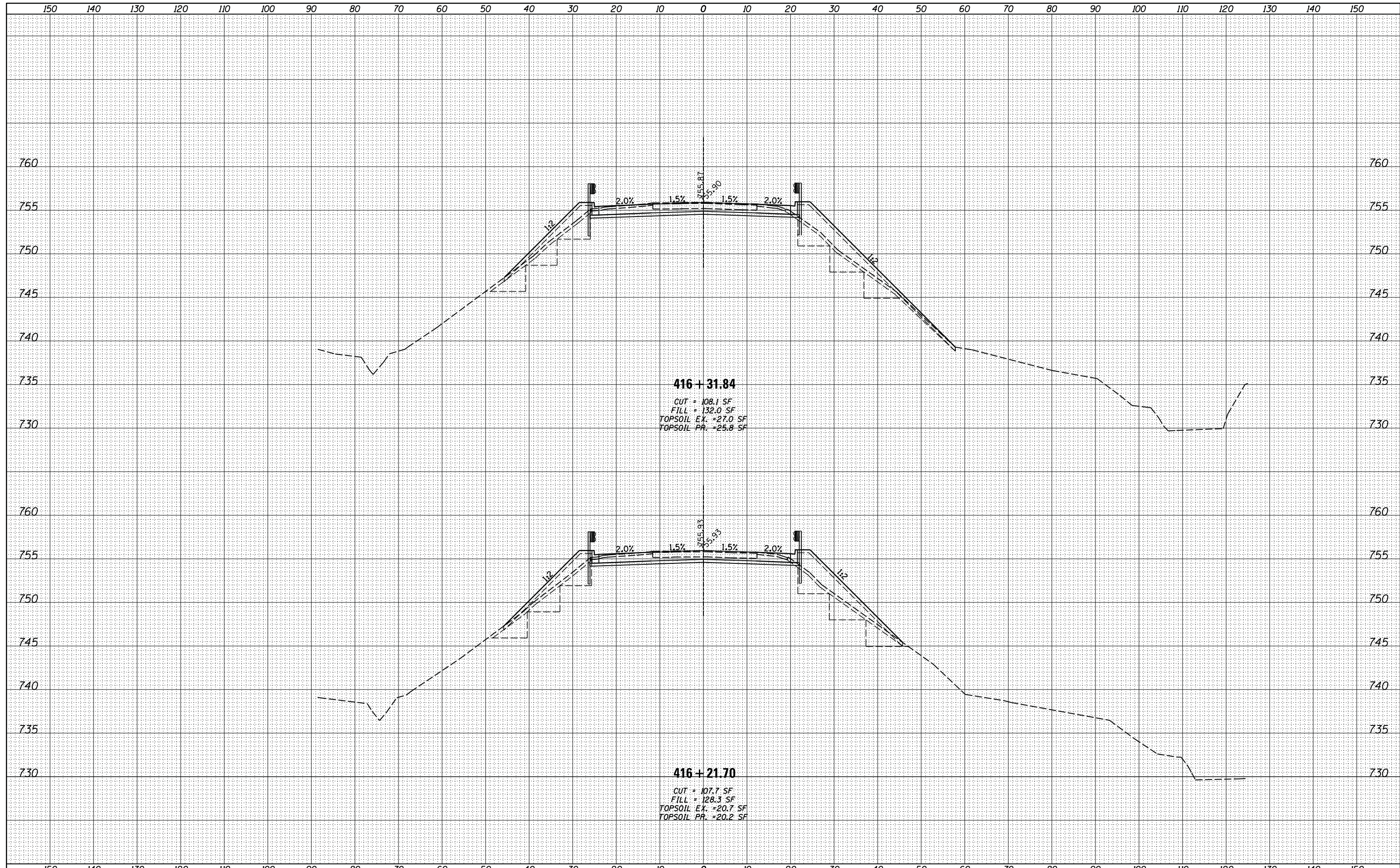
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EARTHWORK							
1-74 WESTBOUND HENDERSON CREEK	EMBANKMENT	SUITABLE EXC.	SUITABLE EXC. ADJUSTED FOR SHRINKAGE (25%)	FURNISHED EXC. (EMBANKMENT MINUS SUITABLE CUT)	EARTH EXC.	4" TOPSOIL EXC.	4" TOPSOIL PLACEMENT
TOTALS	921	774	581	340	774	195	244



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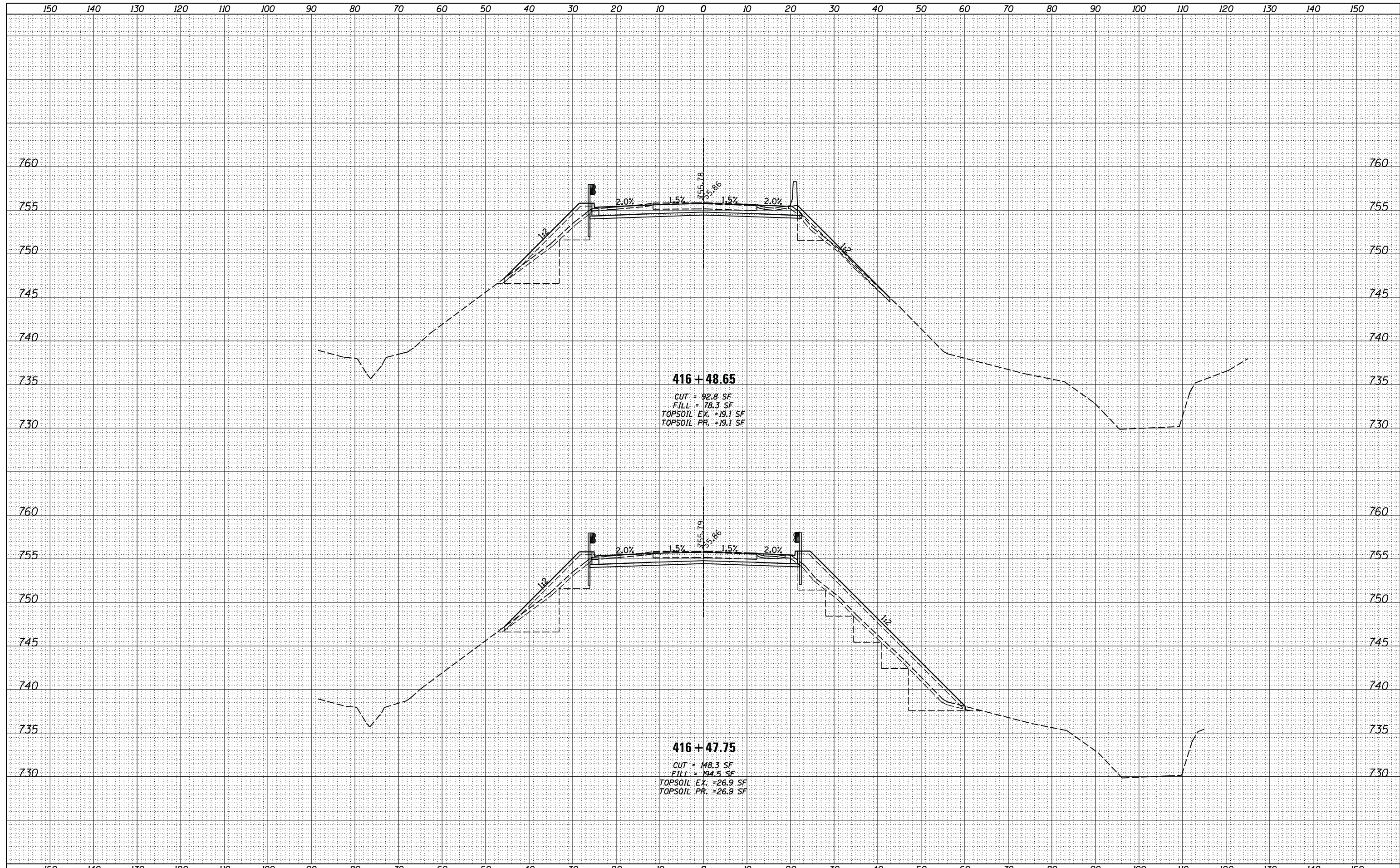
416 + 31.84
 CUT = 108.1 SF
 FILL = 132.0 SF
 TOPSOIL EX. = 27.0 SF
 TOPSOIL PR. = 25.8 SF

416 + 21.70
 CUT = 107.7 SF
 FILL = 128.3 SF
 TOPSOIL EX. = 20.7 SF
 TOPSOIL PR. = 20.2 SF

FILE NAME =	USER NAME = *USERS*	DESIGNED - JCP	REVISIED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	I-74 WESTBOUND CROSS SECTIONS			F.A.I. RTÉ.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
FILEL		DRAWN - TLM	REVISIED -					74	48-[125B]BR, BR-1J	KNOX	131	97
PLOT SCALE = *SCALE*		CHECKED - GAO	REVISIED -		SCALE: N/A SHEET 3 OF 17 SHEETS STA. 416+21.70 TO STA. 416+31.84			CONTRACT NO. 68B85				
MODELNAME	PLOT DATE = 7/30/2014	DATE - 02/22/2013	REVISIED -		ILLINOIS FED. AID PROJECT							

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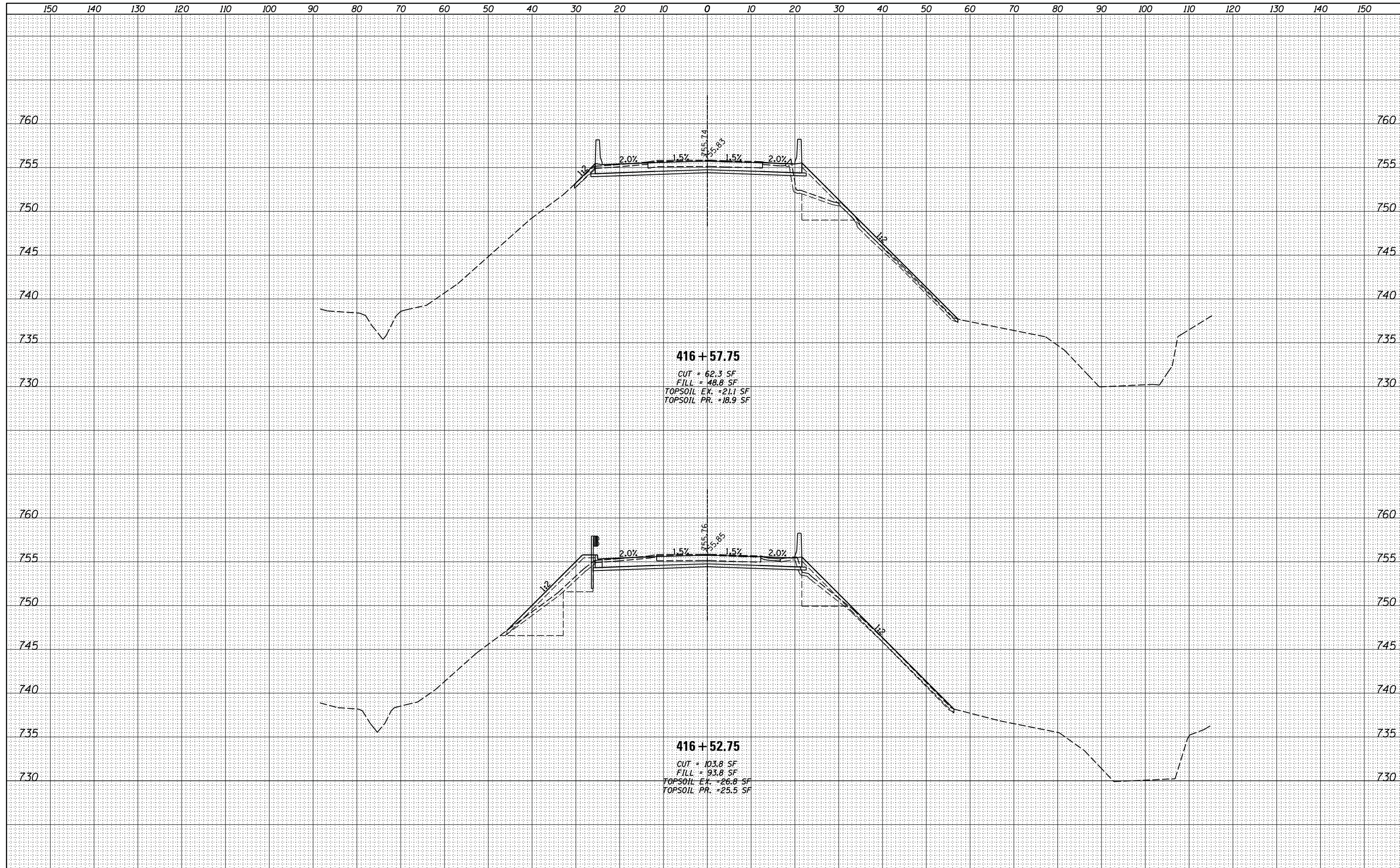
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FILE NAME =	USER NAME = *USER*	DESIGNED - JCP	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	I-74 WESTBOUND CROSS SECTIONS			F.A.I. RTE. 74	SECTION 48-[125B]BR, BR-1J	COUNTY KNOX	TOTAL SHEETS 131	SHEET NO. 98
FILEL		DRAWN - TLM	REVISED -		SCALE: N/A	SHEET 4	OF 17 SHEETS	STA. 416+47.75	TO STA. 416+48.65	CONTRACT NO. 68B85		
		CHECKED - GAO	REVISED -		ILLINOIS FED. AID PROJECT							
MODELNAME	PLOT DATE = 7/30/2014	DATE - 02/22/2013	REVISED -									

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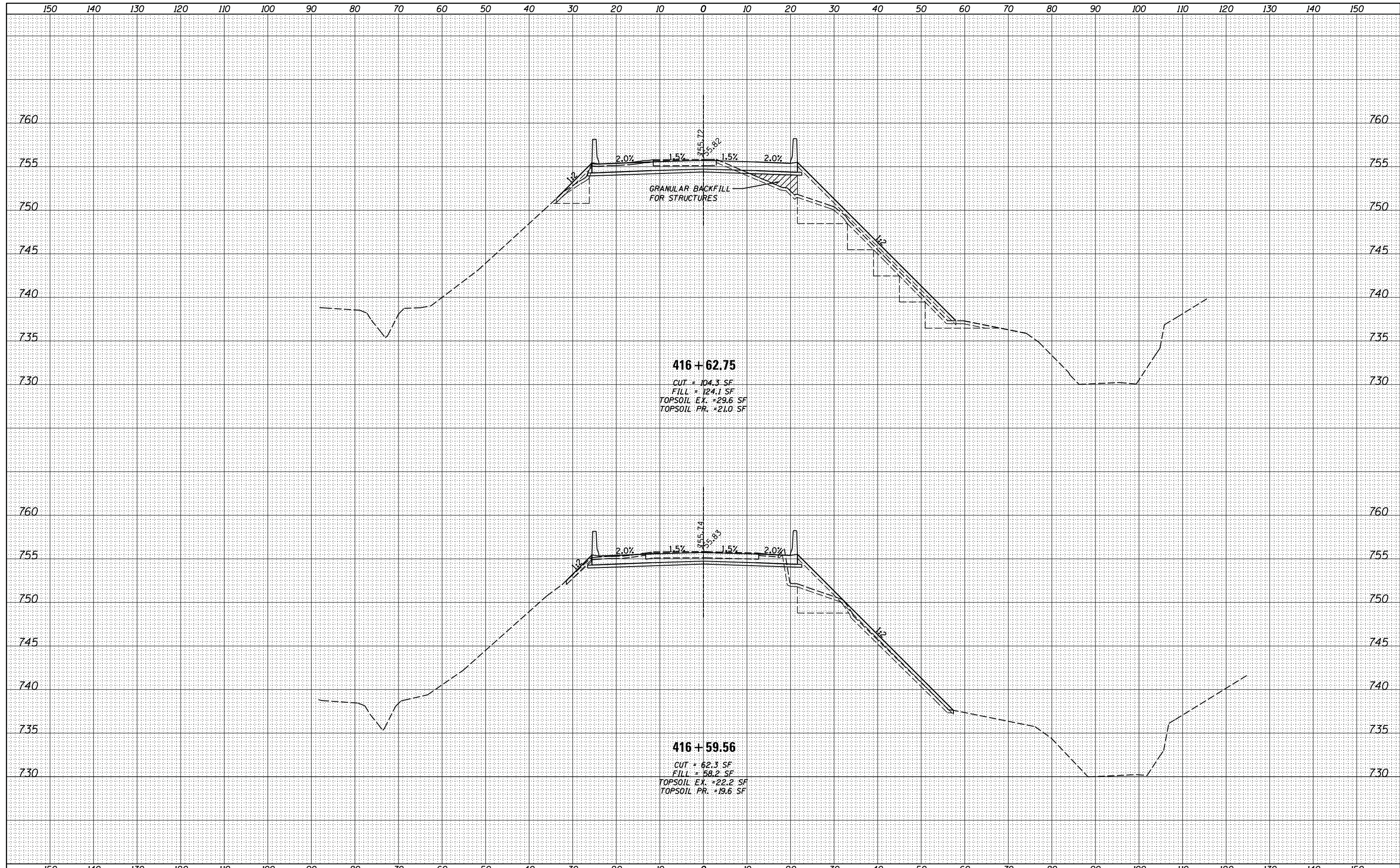
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FILE NAME =	USER NAME = *USERS*	DESIGNED - JCP	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	I-74 WESTBOUND CROSS SECTIONS			F.A.I. RTE. 74	SECTION 48-[125B]BR, BR-1J	COUNTY KNOX	TOTAL SHEETS 131	SHEET NO. 99
FILEL		DRAWN - TLM	REVISED -		SCALE: N/A	SHEET 5	OF 17 SHEETS	STA. 416+52.75	TO STA. 416+57.75	CONTRACT NO. 68B85		
		CHECKED - GAO	REVISED -		ILLINOIS FED. AID PROJECT							
MODELNAME	PLOT DATE = 7/30/2014	DATE - 02/22/2013	REVISED -									

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416 + 62.75
 CUT = 104.3 SF
 FILL = 124.1 SF
 TOPSOIL EX. = 29.6 SF
 TOPSOIL PR. = 21.0 SF

416 + 59.56
 CUT = 62.3 SF
 FILL = 58.2 SF
 TOPSOIL EX. = 22.2 SF
 TOPSOIL PR. = 19.6 SF

FILE NAME	USER NAME = *USER*	DESIGNED - JCP	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	I-74 WESTBOUND CROSS SECTIONS	F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
FILEL		DRAWN - TLM	REVISED -			74	48-[125B]BR, BR-1J	KNOX	131	100	
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MODELNAME	PLOT DATE = 7/30/2014	DATE - 02/22/2013	REVISED -			SCALE: N/A	SHEET 6 OF 17 SHEETS	STA. 416+59.56 TO STA. 416+62.75	ILLINOIS FED. AID PROJECT		