

Benchmark: B.M. #8-1, R.R. Spike in S.W. side of 18" Elm 100' ± Lt. Sta. 150+00, Elev. 503.71.

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

DESIGNED	DAU KRULL
CHECKED	R. Dohy
DRAWN	R. Dohy
CHECKED	GR

GENERAL NOTES

Reinforcement bars shall conform to the requirements of AASHTO M-31 or M-53 Grade 60.

The proposal for Boring Data.

Reinforcement shall be high strength bolts. Bolts 3/4", open heads 1/2", unless otherwise shown.

Calculated weight of Structural Steel: M23 = 31600 Lbs, M203 = 548,420 Lbs. The total weight of Structural Steel shall be used for shop and field painting of Structural Steel.

Field welding of construction accessories will not be permitted to the uniform flange of girders nor to the top flange for a distance equal to six times the span length each way from the pier supports. Field welding in other areas will be permitted only when approved by the Engineer.

Welding bolts shall be set before bolting cross frames over supports. Slope wall shall be reinforced with welded wire fabric 6x5 inches, weighing 30 lbs. per 100 sq. ft.

Layout of slope walls may be varied in the field to suit ground conditions as directed by the Engineer.

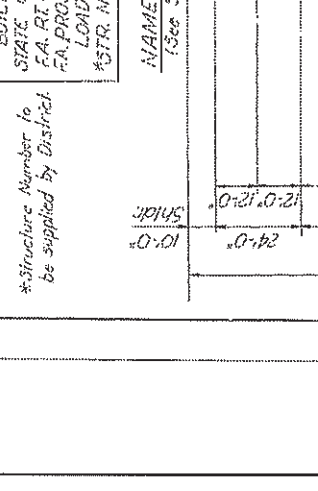
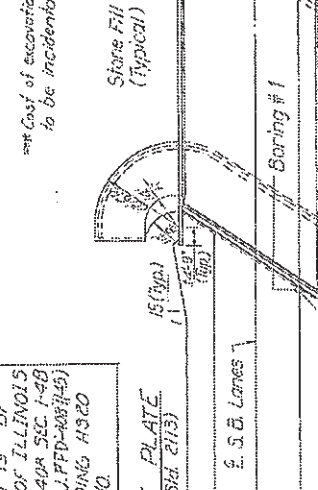
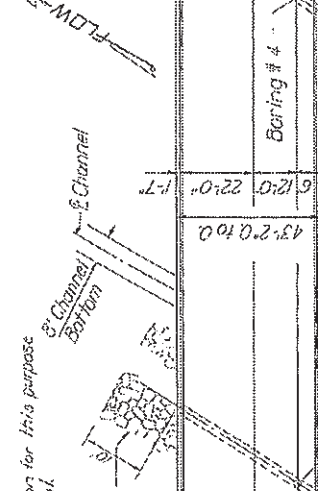
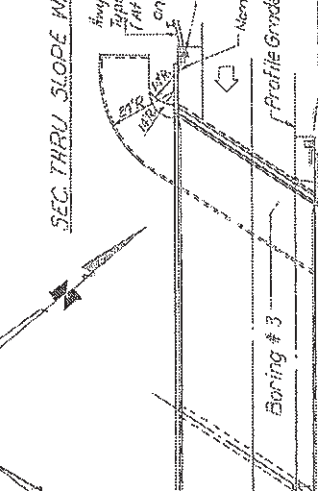
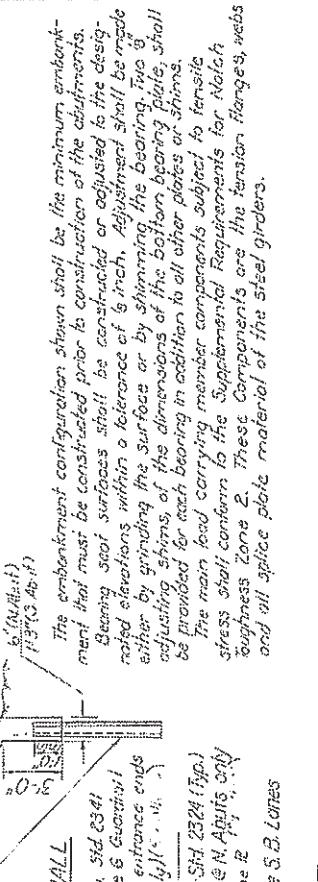
The Contractor shall drive (a.) concrete test piles in a permanent location; One at South Abutment S.B.L., one at North Abutment N.B.L., one at Pier 2 S.B.L. and one at Pier 1 N.B.L., as directed by the Engineer before ordering the remainder of piles.

The embankment configuration shown shall be the minimum embankment that must be constructed prior to construction of the abutments. Existing soil surfaces shall be constructed or adjusted to the design elevations within a tolerance of 1/8 inch. Adjustment shall be made either by grading the surface or by shimming the bearing. The adjusting shims, of the dimensions of the bearing plate, shall be provided for each bearing in addition to all other plates or shims.

The main load carrying member components subject to tensile stress shall conform to the Supplemental Requirements for Hallow Strength Zone 2. These components are the tension flanges, webs and all splice plate material of the steel girders.

TOTAL BILL OF MATERIAL

Item	Unit	Super	Sub	Total
Name Plates	Each	2	2	4
Protective Coat	Sq. Yd.	3444	48	3492
Class X Concrete	Cu. Yd.	828.0	332.6	1160.6
Structural Steel	Long Sum	1	1	2
Steel Sheet Piling	Each	5500	5500	11000
Reinforcement Bars (epoxy Coated)	Pounds	86,610	48,350	134,960
Concrete Piles	Lm. Ft.	137,490	137,490	274,980
Test Piles Concrete	Each	4	4	8
Slope Wall (6")	Sq. Yd.	470	470	940
Neutral Expansion Joint (4")	Lm. Ft.	100	100	200
Neutral Expansion Joint (2")	Lm. Ft.	100	100	200
Confined Excavation	Cu. Yd.	684	684	1368
Confined Pier (South Bound Structure)	Each	1	1	2
Confined Pier (North Bound Structure)	Each	1	1	2
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Confined Pier (North Bound Structure)	Each	1	1	2
Steel Sheet Piling	Sq. Ft.	5630	5630	11260
Stone Fill	Tons	1170	1170	2340
Steel Roof Concrete	Cu. Yd.	169.4	169.4	338.8



DESIGN STRESSES

$f_c = 3,500$ p.s.i.
 $f_y = 60,000$ p.s.i. (Reinforcement)
 $f_y = 36,000$ p.s.i. (Structural)
 $f_y = 36,000$ p.s.i. (Steel Sheet Piling)

LOADING HS 20-44

Design Specifications: 1977 AASHTO and 1918 Interim Specifications as applicable.

Allow 25 # 1 sq. ft. for future wearing surface.

WATERWAY INFORMATION

Drainage Area: 51.4 Sq. Miles
 Design Discharge (50 year): 9200 c.f.s.

Required Opening: 157.7 Sq. Ft.
 Proposed Opening: 1570 Sq. Ft.
 Created Head for Design Flood: 0.43'
 100 Year Discharge: 11,500 c.f.s.
 Created Head for 100 Year Flood: 0.57'
 100 Year High Water Elev.: 505.4

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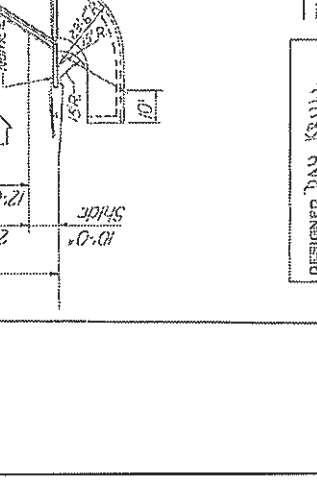
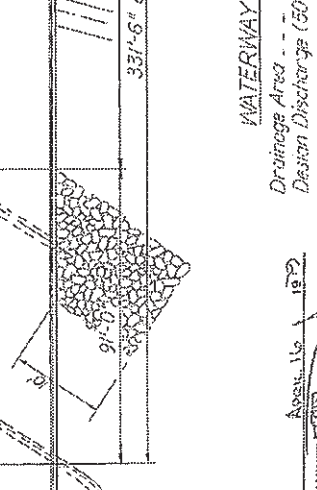
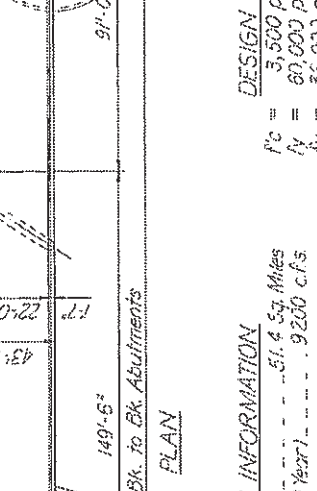
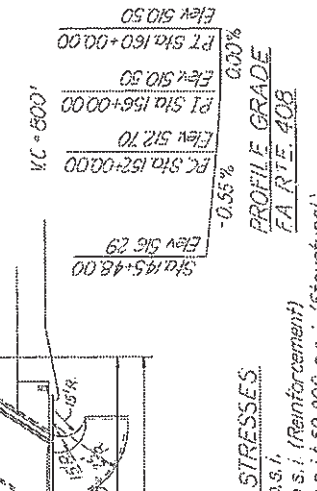
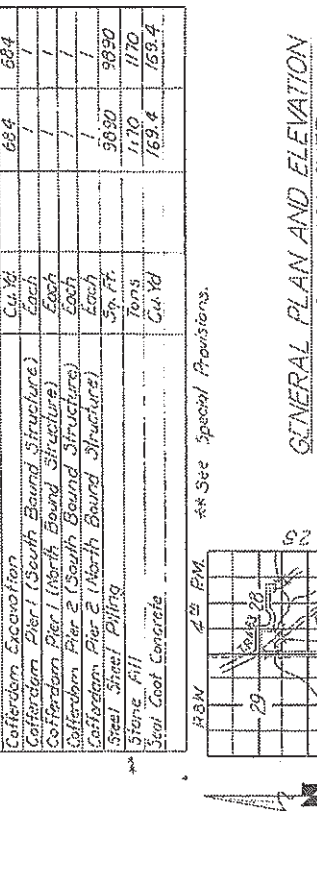
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 STATION 149+60.00 (E CHANNEL)

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EXISTING SN: 001-005556
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 DATE -

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