

TOP of BEAM ELEVATIONS TABLE

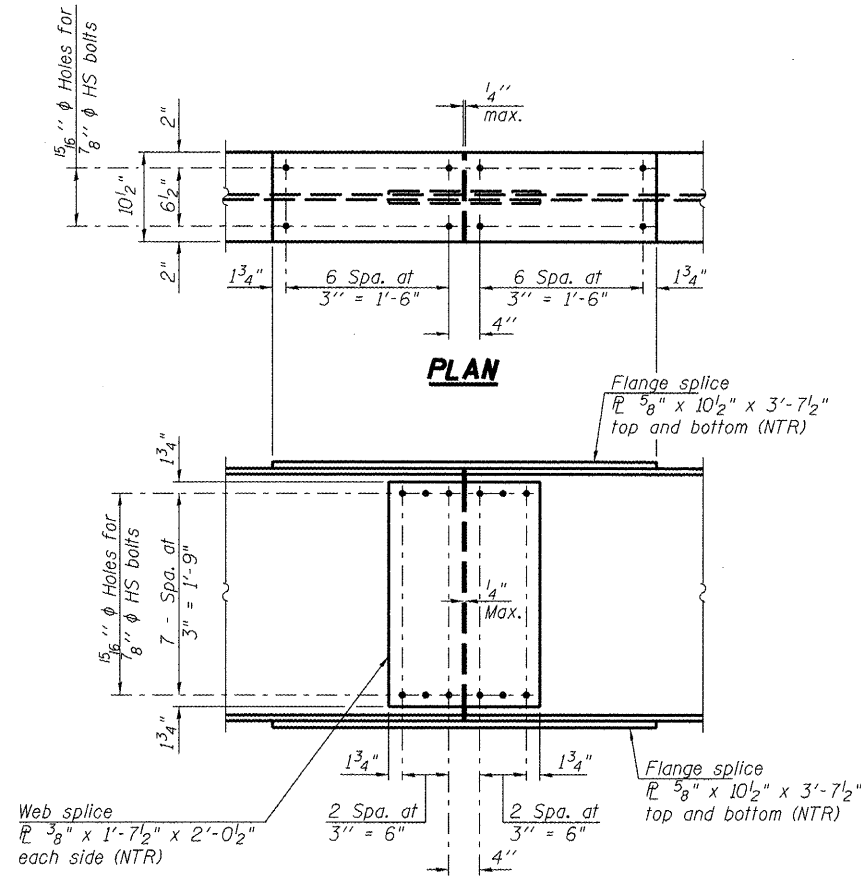
For Fabrication Only

Beam Number	℄ Brg. No. Abut.	℄ Brg. Pier 1	℄ Splice No. 1	℄ Splice No. 2	℄ Brg. Pier 2	℄ Brg. So. Abut.
Beam 1	457.29	457.26	457.26	457.07	457.00	456.78
Beam 2	457.40	457.37	457.36	457.17	457.10	456.88
Beam 3	457.50	457.46	457.44	457.26	457.18	456.96
Beam 4	457.61	457.52	457.49	457.28	457.20	456.98
Beam 5	457.71	457.55	457.49	457.19	457.12	456.90
Beam 6	457.82	457.58	457.49	457.11	457.03	456.81
Beam 7	457.93	457.59	457.48	457.01	456.94	456.72

INTERIOR BEAM MOMENT TABLE			
	0.4 Sp. 1 or 0.6 Sp. 3	Pier 1 or 2	0.5 Sp. 2
I_s	(in ⁴) 3990	3990	3990
$I_c(n)$	(in ⁴) 11273	-	11273
$I_c(3n)$	(in ⁴) 8308	-	8308
$I_c(cr)$	(in ⁴) -	5700	-
S_s	(in ³) 269	269	269
$S_c(n)$	(in ³) 409	-	409
$S_c(3n)$	(in ³) 369	-	369
$S_c(cr)$	(in ³) -	317	-
DC1	(k/')	0.687	0.687
M _{DC1}	(k)	7	195
DC2	(k/')	0.130	0.130
M _{DC2}	(k)	1	37
DW	(k/')	0.260	0.260
M _{DW}	(k)	3	74
$M_k + IM$	(k)	322	445
M_u (Strength I)	(k)	577	1293
$\phi_r M_n, \phi_r M_{nc}$	(k)	2165	1487
f_s DC1	(ksi)	0.3	11.18
f_s DC2	(ksi)	0.04	1.79
f_s DW	(ksi)	0.08	3.59
f_s (L+IM)	(ksi)	12.27	21.89
f_s (Service II)	(ksi)	12.69	38.45
$0.95R_n F_y f$	(ksi)	47.50	47.50
Vr	(k)	18.9	23.5

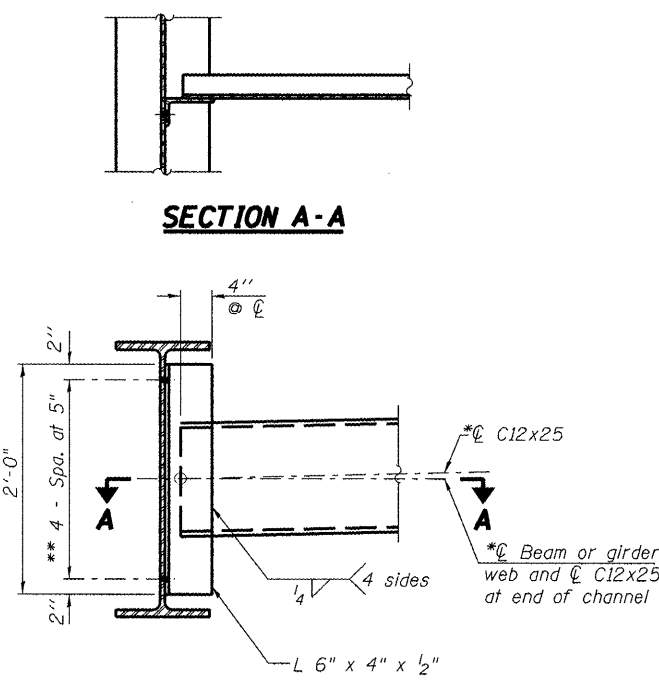
I_s, S_s : Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total-Strength I, and Service II) due to non-composite dead loads (in.⁴ and in.³).
 $I_c(n), S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections, due to short-term composite live loads (in.⁴ and in.³).
 $I_c(3n), S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections, due to long-term composite (superimposed) dead loads (in.⁴ and in.³).
 $I_c(cr), S_c(cr)$: Composite moment of inertia and section modulus of the steel and longitudinal deck reinforcement, used for computing f_s (Total-Strength I and Service II) in cracked sections, due to both short-term composite live loads and long-term composite dead loads (in.⁴ and in.³).
 DC1: Un-factored non-composite dead load (kips/ft.).
 M_{DC1}: Un-factored moment due to non-composite dead load (kip-ft.).
 DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).
 M_{DC2}: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).
 DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).
 M_{DW}: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).
 $M_k + IM$: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).
 M_u (Strength I): Factored design moment (kip-ft.).
 $1.25 (M_{DC1} + M_{DC2}) + 1.5 M_{DW} + 1.75 M_k + IM$
 $\phi_r M_n, \phi_r M_{nc}$: Compact composite positive moment capacity computed according to Article 6.10.7.1 or negative moment capacity computed according to Appendix A6 (kip-ft.).
 f_s DC1: Un-factored stress at edge of flange for controlling steel flange due to vertical non-composite dead loads as calculated below (ksi).
 M_{DC1} / S_{nc}
 f_s DC2: Un-factored stress at edge of flange for controlling steel flange due to vertical composite dead loads as calculated below (ksi).
 $M_{DC2} / S_c(3n)$ or $M_{DC2} / S_c(cr)$ as applicable.
 f_s DW: Un-factored stress at edge of flange for controlling steel flange due to vertical composite future wearing surface loads as calculated below (ksi).
 $M_{DW} / S_c(3n)$ or $M_{DW} / S_c(cr)$ as applicable.
 f_s (L+IM): Un-factored stress at edge of flange for controlling steel flange due to vertical composite live plus impact loads as calculated below (ksi).
 $M_k + IM / S_c(3n)$ or $M_k + IM / S_c(cr)$ as applicable.
 f_s (Service II): Sum of stresses as computed below (ksi).
 $f_{SDC1} + f_{SDC2} + f_{SDW} + 1.3 f_s (L + IM)$
 $0.95R_n F_y f$: Composite stress capacity for Service II loading according to Article 6.10.4.2 (ksi).
 Vr: Maximum factored shear range in composite portion of span computed according to Article 6.10.10.

INTERIOR BEAM REACTION TABLE		
	Abuts.	Pier 1 or 2
R _{DC1}	(k) 5.4	44.0
R _{DC2}	(k) 1.0	8.3
R _{DW}	(k) 2.0	16.7
$R_k + IMP$	(k) 50.5	88.1
R _{Total}	(k) 58.9	157.1



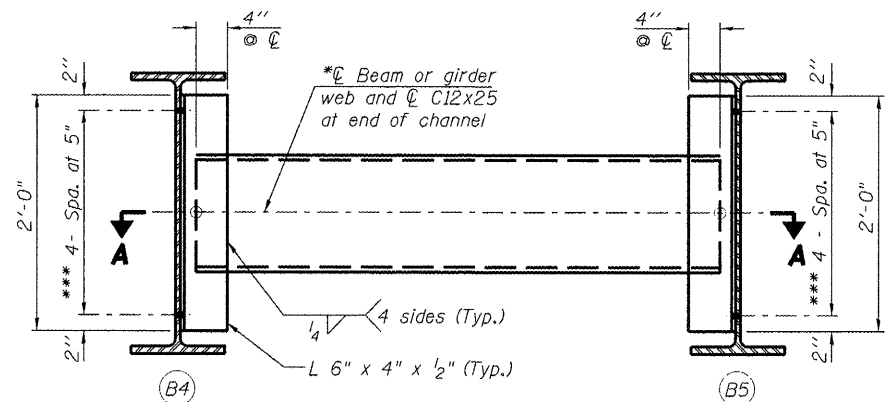
FIELD SPLICE DETAIL
(14 Required)

Note: Load carrying components designated "NTR" shall conform to the Impact Testing Requirements, Zone 2.



INTERIOR DIAPHRAGM D1
(35 Required)

Note:
 Two hardened washers required for each set of oversized holes.
 *Alternate channels C12x30 are permitted to facilitate material acquisition. Calculated weight of structural steel is based on the lighter section.
 The alternate, if utilized, shall be provided at no additional cost to the Department.
 **3/4" φ HS bolts, 15/16" φ holes



INTERIOR DIAPHRAGM D2
(7 Required)

Note:
 *** 3/4" φ H.S. bolts, 13/16" φ holes in Beam 5 web and 13/16" x 1 7/8" vertically slotted holes in connection angle at Beam 5 end of diaphragm assembly.
 3/4" φ H.S. bolts, 15/16" φ holes in all connection parts at Beam 4 end of diaphragm assembly. Other notes on Diaphragm D1 pertain and Section A-A Similar.

Note:
 All connecting angles and diaphragms shall conform to the requirements of AASHTO M270 Grade 36, and all beams, splice plates and bearing plates shall conform to the requirements of AASHTO M270 Grade 50.

FILE NAME = 0760028-78134.dgn	USER NAME = huff00028	DESIGNED - BDC	REVISED -
		CHECKED - MNM	REVISED -
		DRAWN - DAB	REVISED -
		CHECKED - TEH	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

STRUCTURAL STEEL DETAILS
STRUCTURE NO. 076-0028
 SHEET NO. 20 OF 28 SHEETS

F.A.P. RTE. 132	SECTION 1038-1	COUNTY Pope	TOTAL SHEETS 52	SHEET NO. 44
CONTRACT NO. 78134			ILLINOIS FED. AID PROJECT	