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May 19, 2022

Mr. Bradly Hargett, P.E. James J. Benes & Assoc., Inc. 950 Warrenville Road, Suite 101 Lisle, IL 60532

Re: Geotechnical Engineering Services
Retaining Wall
Illinois Route 7 (Southwest Highway) Improvements
Cook County, Illinois
D-91-124-19, Contract 62HB2
Wang No. 411-05-03

This letter report presents the results of Wang Engineering, Inc.'s (Wang) subsurface investigation, laboratory testing, geotechnical evaluations, and recommendations for the proposed retaining wall along Illinois Route 7 (SW Highway) in the Village of Orland Park, Illinois. The proposed retaining wall is part of the roadway improvements along Southwest Highway from the 131<sup>st</sup> Street to 135<sup>th</sup> Street. On the USGS Palos Park *Quadrangle 7.5 Minutes Series* map, the project site is located at NE1/4, Section 34, Tier 37 N, Range 12E of the Third Principal Meridian. A *Site Location Map* is presented as Exhibit 1.

Based on the Cross-Section drawings (Appendix D) and information provided by James J. Benes & Associates, Inc. (JJ Benes), we understand the proposed retaining wall along IL 7 is about 350 feet, between Station 112+50 and 116+00 with approximate offset of 75 feet to the right from proposed roadway centerline. The proposed will have a maximum retained height of 4 feet with front face slope of 1:2.5(V:H). A TSL plan was not yet available at the time of preparation of this report.

#### SUBSURFACE INVESTIGATION

The subsurface investigation consisted of seven retaining wall borings, designated as RW-01 to RW-06, and RW-02HA. The borings were drilled by Wang in February 15 to 23, 2022 The borings were drilled from elevations of 668.65 to 673.12 feet and were advanced to depths of 8 to 25 feet below ground surface (bgs). The as-drilled northings and eastings were acquired with a mapping-grade GPS unit. Stations, offsets, and elevations were provided by JJ Benes. Boring location data are presented in the *Boring Logs* (Appendix A) and the as-drilled boring locations are shown in the *Boring Location Plan* (Exhibit 2).



A track-mounted drilling rig, equipped with hollow stem augers, was used to advance and maintain open boreholes. Soil sampling was performed according to AASHTO T 206, "*Penetration Test and Split Barrel Sampling of Soils*." The soil was sampled at 2.5-foot intervals to the boring termination depths. A jackhammer driven geoprobe sampler was used in HA-series borings to continuously sample the soils. Soil samples collected from each sampling interval were placed in sealed jars and transported to the laboratory for further examination and laboratory testing.

Field boring logs, prepared and maintained by Wang geologists, include lithological descriptions, visual-manual soil (IDH Textural) classifications, results of Rimac and pocket penetrometer unconfined compressive strength tests, and results of Standard Penetration Tests (SPT) recorded as blows per 6 inches of penetration.

Groundwater levels were measured while drilling and upon completion of drilling. The boreholes were backfilled upon completion with soil cuttings and bentonite chips.

### LABORATORY INVESTIGATION

The soil samples were tested in the laboratory for moisture content (AASHTO T265). Selected soil samples were tested for Atterberg limits (AASHTO T 89/90) and particle size (AASHTO T 88) analyses. Field visual descriptions of the soil samples were verified in the laboratory. Laboratory test results are shown in the *Boring Logs* (Appendix A) and in the *Laboratory Test Results* (Appendix B).

## LITHOLOGICAL PROFILE

At the surface, the borings, drilled on the grass area at the top of slope, encountered 4 to 9 inches of silty clay loam topsoil. The borings drilled along the IL 7 roadway and shoulder encountered 14-inch asphalt over 6-inch concrete or 7 to 11 inches of sandy gravel. In descending order, the general lithologic succession encountered beneath the surface includes: 1) man-made ground (fill) and 2) medium stiff to very stiff clay to silty clay loam.

## 1) Man-made ground (fill)

Beneath the pavement, Borings RW-03, RW-05, and RW-06 encountered up to 5 feet of fill. The cohesive fill consists of hard clay loam to silty clay loam with unconfined compressive strength  $(Q_u)$  values of 5.5 to greater than 10.3 tsf and moisture content values of 15 and 16%.

### 2) Medium stiff to hard silty clay to silty clay loam

Beneath the fill or topsoil, the borings advanced through medium stiff clay, silty clay to silty clay loam interbedded with saturated, loose sand and silt. The unit has  $Q_u$  values of 0.3 to 5.33 tsf and



moisture content values of 15 to 31%. Laboratory testing show liquid limit ( $L_L$ ) values of 33 to 36% and plastic limit ( $P_L$ ) values of 17 and 18%.

The interbedded loose sand and silt unit has thickness up to 5 feet and N values of 3 to 9 blows per foot with moisture content values of 13 to 26%.

### **GROUNDWATER CONDITIONS**

Groundwater was observed while drilling in Borings at elevations of 654 to 663 feet (9.8 to 15.0 feet bgs). The groundwater was measured at depths of 10 and 25 feet bgs (643 and 663 feet) in Borings RW-01 and RW-04.

### ANALYSIS AND RECOMMENDATIONS

Based on the cross-section drawings (Appendix D), the proposed wall will be a cut wall with front slope to 1:2.5(V:H). Generally, non-gravity (flexible) wall types such as a sheet pile or soldier pile type wall would be more suitable. Mechanically Stabilized Earth (MSE) and Reinforced Concrete Cantilever (RCC) would require large open cut excavations into the existing slope, temporary soil retention systems, and may impact the existing right of way. The construction of these wall types would likely also require more backfilling thus longer construction time. We recommend considering flexible walls such as sheet pile wall or soldier pile wall and lagging walls. Soldier piles could be driven or drilled and set.

Drilled soldier pile system should be designed for both lateral earth pressure and lateral deformation. The embedment depth in moment equilibrium for the wall sections should be designed in accordance with the AASHTO LRFD guidelines (AASHTO 2020).

We recommend the lateral earth pressure analysis should be performed for the wall in both shortterm (undrained) and long-term (drained) conditions using the soil parameters in Tables 1 and 2. Elevations provided in tables are based on the average layer elevations across the soil profile and may vary from one boring location to another. The active and passive earth pressure coefficients are provided for straight slope of backfill behind the wall and a 1:2.5 (V:H) in front of the wall.

The design of the wall should ignore 3.0 feet of soil in front of the wall measured from the finished ground surface elevation in providing passive pressure due to excavations required for installation of concrete facing, drainage systems, and frost-heave conditions (IDOT 2012). In developing the design lateral pressure, the pressure due to any existing structures, roadway surcharge loading, and construction equipment surcharge loads should be added to the lateral earth pressure. Drainage behind the wall should be in accordance with IDOT guidelines (IDOT 2012). The water pressure



should be added to the earth pressure if drainage is not provided. The wall design should consider the permanent groundwater at elevation of 663 feet.

Ref Borings: RWB	-01 to KWB-06	, and RWB-	02HA			
Call Description	Unit	Undraine Strength I	ed Shear Properties	Earth Pressure Coefficients		
Average Elevation (feet)	Weight γ (pcf)	Cohesion (psf)	Friction Angle (°)	Active Pressure (straight)	Passive Pressure (1V:2.5H)	
Stiff to Hard CLAY to SILTY CLAY LOAM Surface to EL 666 feet	120	2000	0	1.00		
M Stiff to Stiff CLAY to SILTY CLAY LOAM EL 666 to 661 feet	120	800	0	1.00	1.00	
Loose SAND/SILT EL 661 to 654 feet	53 (submerged)	0	29	0.35	1.87	
Stiff to V Stiff CLAY to SILTY CLAY LOAM EL 654 to 644 feet (EOB)	58 (submerged)	1800	0	1.00	1.00	

#### Table 1: Undrained (Short-term) Geotechnical Parameters for Design of Flexible Wall Ref Borings: RWB-01 to RWB-06, and RWB-02HA

Table 2: Drained (Long-term) Geotechnical Parameters for Design of Flexible Wall Ref Borings: RWB-01 to RWB-06, and RWB-02HA

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Coll Description	Unit	Drained Strength I	l Shear Properties	Earth Pressure Coefficients			
Average Elevation (feet)	Weight γ (pcf)	Cohesion (psf)	Friction Angle (°)	Active Pressure (straight)	Passive Pressure (1V:2.5H)		
Stiff to Hard CLAY to SILTY CLAY LOAM Surface to EL 666 feet	120	100	30	0.33			
M Stiff to Stiff CLAY to SILTY CLAY LOAM EL 666 to 661 feet	120	0	29	0.35	1.87		
Loose SAND/SILT EL 661 to 654 feet	53 (submerged)	0	29	0.35	1.87		
Stiff to V Stiff CLAY to SILTY CLAY LOAM EL 654 to 644 feet (EOB)	58 (submerged)	100	30	0.33	1.98		

The lateral deformation of the wall should be designed for movement and moment fixity at the base of the pile. The roadway, utilities, and any nearby structures should not be impacted by the lateral movement of the wall. The evaluations should be performed using the recommended soil parameters shown in Table 3, via the p-y curve (COM624) method. Elevations provided in Table 3



is based on the average layer elevations across the profile and may vary from one boring location to another.

Ref Borings:	RWB-01 to $RV$	<b>VB-06</b> , and 1	RWB-02HA		
Soil Description Average Elevation (feet)	Unit Weight γ (pcf)	Undrained Shear Strength c <sub>u</sub> (psf)	Estimated Friction Angle Φ (°)	Estimated Lateral Soil Modulus Parameter k (pci)	Estimated Soil Strain Parameter $\epsilon_{50}$ (%)
Stiff to Hard CLAY to SILTY CLAY LOAM Surface to EL 666 feet	120	2000	0	500	0.5
M Stiff to Stiff CLAY to SILTY CLAY LOAM EL 666 to 661 feet	120	800	0	100	0.7
Loose SAND/SILT EL 661 to 654 feet	53 (submerged)	0	29	30	
Stiff to V Stiff CLAY to SILTY CLAY LOAM EL 654 to 644 feet (EOB)	58 (submerged)	1800	0	500	0.5

Table 3: Recommended Geotechnical Parameters for Lateral Load Analysis for Flexible Walls
Ref Borings: RWB-01 to RWB-06, and RWB-02HA

The global stability of the proposed wall was analyzed based on the soil profile described in Section 3.1 and the information provided in the GPE and cross-section drawing. The stability was analyzed at the critical section near Station 115+00 where the maximum exposed height of 4 feet. The minimum required factor of safety (FOS) is 1.7 in both short-term (undrained) and long-term (drained) conditions (IDOT 2020).

Details of the global stability analysis with critical failure surfaces and results are presented in Appendix C. We estimate the wall will have an adequate FOS of 2.8 (Appendix C-1) in the undrained condition with 4 feet pile embedment. Global stability evaluations were performed to estimate the minimum pile tip elevation required to achieve an FOS of 1.7 in the drained condition as shown in Table 4. It should be noted that typically, the lateral earth pressure and deformation analyses will determine the minimum embedment depth for cantilevered pile wall. Therefore, the designer should perform other analyses including lateral earth pressure and deflection analyses to determine the required design pile embedment.



		Table 4: Re	esults of Global Sta	ability Analys	sis	
Station		Retained	Short-term (U Conditi	ndrained) on	Long-term (Drai	ned) Condition
	Reference Boring(s)	Height (feet)	FOS	Tip Elevation (feet)	FOS	Tip Elevation (feet)
115+00	RWB-02 to RWB-06	4.0	2.80 (Appendix C-1)	667.0	1.75 (Appendix C-2)	662.0

The wall should be constructed according to the current IDOT Standard Specifications for Road and Bridge Construction (2022).

If you have any questions or for further discussion, please contact us at (630) 953-9928.

Sincerely,

## WANG ENGINEERING, INC.

Met W. Seyhun, P.E. Senior Project Manager Nesam S. Balakumaran, P.Eng. Project Geotechnical Engineer









## **APPENDIX A**

wangeng Telephor Fax:	wang Engineering g@wangeng.com	Client Project Locatior	<b>IL</b>	BC Ji . Route	DRI WE ames 7 fro	NG I Job J. B m 13 Cool	i L No enes 31st k Co	OG   : 411-0 s & Ass Street ounty,	RW-01 05-03 sociates to 135th Street L	Datum: N Elevation North: 18 East: 111 Station: 1 Offset: 67	AVD 8 : 673.1 14539. 8229.7 11+71 '.52' R	8 2 ft 34 ft '5 ft .72 T	Page	1 of 1
Profile Elevation	SOIL AND ROCK DESCRIPTION	Depth ( <b>ft</b> )	Sample Type	Sample No. SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROC DESCRIPTION	Depth X	Sample Type	Sample No. SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
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        <u>   </u> <u>663.3</u> G	ray coarse SAND; wet to	- 		5 U 5 S H	1.25 P	20								
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Profile	SOIL AND ROCK	Depth (ft)	Sample Typ	Sample No	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%	Profile	Elevation (ft)	SOIL AND ROC DESCRIPTION	K <sup>Depth</sup>	Sample Typ	Sample No SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%
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## **APPENDIX B**

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# **APPENDIX C**

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# **APPENDIX D**

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LINOIS ANSPORTATION SCALE: 1"=5' VERT. SHEET OF SHEETS S	670 665 675 670			STAGE-1: STAGE-3: STAGE-3: STAGE-3:	CUT (SF) 0.00 4.66 155.76	FILL (SF) 0:00 94.60 8:54 8:54 5:4 0 125	50 (46.1' RT)		670 665 115+50.00 670 670
LINOIS ANSPORTATION SCALE: 1"=5' VERT. SHEET OF SHEETS STA. 114	670			STAGE-1: STAGE-2: STAGE-3: STAGE-3: STAGE-1: STAGE-1:	CUT (SF) 0.00 4.66 155.76 2.00 %	FILL (SF) 0.00 94.60 8.54 54 FILL (SF) 0.00	(662.57)(46.1°RT)		670 115 + 50.00 675 675 675 675 675
LINOIS ANSPORTATION SCALE: 1"=5' VERT. SHEET OF SHEETS STA. 114+00.0	670		MOR: X7 1.50 %	STAGE-1: STAGE-2: STAGE-3: STAGE-3: STAGE-3: STAGE-3:	CUT (SF) 0.00 4.66 155.76 25.76 2.00 % -2.00 %	FILL (SF) 0.00 94.60 8.54 90 725 FILL (SF) 0.00 98.51 10.71	662.57 (46.1 ° RT)		670 115+50.00 675 675 675 675 675
LINOIS ANSPORTATION SCALE: 1"=10' HORIZ. SCALE: 1"=5' VERT. SHEET OF SHEETS STA. 114+00.00 T	670		$M_{R} = 0.00 \text{ M}$	STAGE-1: STAGE-3: STAGE-3: STAGE-3: STAGE-3: STAGE-3:	CUT (SF) 0.00 4.66 155.76 2.55.76 0.00 2.58 158.90	F.ELL (SF) 0.00 94.60 8.54 5.54 5.54 5.54 5.54 5.54 5.54 5.54	662.57 (46.1' RT)		670 665 115 + 50.00 675 670 670 665 115 + 00.00
LINOIS ANSPORTATION SCALE: 1"=5' VERT. SHEET OF SHEETS STA. 114+00.00 TO STA	665			STAGE-1: STAGE-2: STAGE-3: STAGE-3: STAGE-3: STAGE-2: STAGE-2: STAGE-2: STAGE-3:	CUT (SF) 0,00 4,66 155.76 255.76 0,00 2,58 158.90	FILL (SF) 0.00 94.60 8.54 5 5 5 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	(662.57) (46.1 'RT)		670 665 115 + 50.00 670 670 665 115 + 00.00
LINOIS ANSPORTATION SCALE: 1"=5" VERT. SHEET OF SHEETS STA. 114+00.00 TO STA.116+(	670			STAGE - 1: STAGE - 2: STAGE - 2: STAGE - 3: STAGE - 3: STAGE - 3:	CUT (SF) 0.00 4.66 155.76 2.00 % CUT (SF) 0.00 2.58 158.90	FILL (SF) 0.00 94.60 8.54 75 75 75 75 75 75 75 75 75 75 75 75 75	(662.57) (46.1 'RT)		670 665 115 + 50.00 670 670 665 115 + 00.00
LINOIS ANSPORTATION SCALE: 1"=5' VERT. SHEET OF SHEETS STA. 114+00.00 TO STA. 116+00.00	670		72 PR 5.9	STAGE - 1: STAGE - 2: STAGE - 3: STAGE - 3: STAGE - 3: STAGE - 2: STAGE - 2: STAGE - 3:	CUT (SF) 0.00 4.66 155.76 2.200 %	FILL (SF) 0.00 94.60 8.54 8.54 50 425 50 425 51 10.71	(662:57) (46.1°RT)		665 115+50.00 670 670 665 115+00.00
LINOIS ANSPORTATION SCALE: 1"=5' VERT. SHEET OF SHEETS STA. 114+00.00 TO STA. 116+00.00	665			STAGE-1: STAGE-2: STAGE-3: STAGE-3: STAGE-3: STAGE-2: STAGE-2: STAGE-3:	CUT (SF) 0.00 4.66 155.76 2.200 %	FILL (SF) 0.00 94.60 8.54 50 425 50 425 50 425 50 425 50 425 50 425 50 425 50 425 50 425 51 10.71	(662: 57/46:1'RT)		665 115 + 50.00 675 675 675 675 675
LINOIS ANSPORTATION SCALE: 1''=5' VERT. SHEET OF SHEETS STA. 114+00.00 TO STA. 116+00.00	670			STAGE-1: STAGE-2: STAGE-3: STAGE-3: STAGE-3:	CUT (SF) 0.00 4.66 155.76 2.200 % 0.00 2.58 158.90	FILL (SF) 0.00 94.60 8.54 8.54 5 5 7 7 9 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	(6622.57 (46.3* RT)		670 665 115 + 50.00 670 670 665 115 + 00.00
LINOIS CROSS SECTIONS – IL RTE 7 ANSPORTATION 1''=10' HORIZ. SCALE: 1''=5' VERT. SHEET OF SHEETS STA. 114+00.00 TO STA.116+00.00	670			STAGE-12: STAGE-3: STAGE-3: STAGE-3: STAGE-2: STAGE-2: STAGE-3:	CUT (SF) 0.00 4.66 155.76 2.00 % 6 2.58 158.90	FILL (SF) 0.00 94.60 8.54 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	662.57 (46.1' NT)		670 115 + 50.00 675 675 675 675
LINOIS CROSS SECTIONS – IL RTE 7 ANSPORTATION SCALE: 1''=5' VERT. SHEET OF SHEETS STA. 114+00.00 TO STA. 116+00.00 TO STA. 116+00.00	670			STAGE-1 STAGE-3: STAGE-3: STAGE-3: STAGE-3: STAGE-3: STAGE-3:	CUT (SF) 0.00 4.66 155.76 2.00 % 5. 0.00 2.58 158.90 158.90	FILL (SF) 0.00 94.60 8.54 54 FILL (SF) 0.00 98.51 10.71			670 665 115 + 50.00 670 665 115 + 00.00 665
LINOIS CROSS SECTIONS – IL RTE 7 ANSPORTATION SCALE: 1''=10' HORIZ. SCALE: 1''=5' VERT. SHEET OF SHEETS STA. 114+00.00 TO STA. 116+00.00 [ILLINOIS]	670			STAGE-1: STAGE-2: STAGE-3: STAGE-3: STAGE-3: STAGE-3: STAGE-2: STAGE-2: STAGE-2: STAGE-3:	CUT (SF) 0.00 4.66 155.76 2.60 % 5.6 2.58 158.90 2.58 158.90	FILL (SF) 0.00 94.60 8.54 5.4 5.4 5.4 5.4 5.4 5.4 5.4 5.4 5.4	(662.57)(46.1'RT)		665 <b>115 + 50.00</b> 670 675 675
LINOIS ANSPORTATION SCALE: 1'=5' VERT. SHEET OF SHEETS STA. 114+00.00 TO STA. 116+00.00 [ILLINOIS FED.	6.70			STAGE-1: STAGE-2: STAGE-3: STAGE-3: STAGE-3: STAGE-3: STAGE-4: STA	CUT (SF) 0.00 4.66 155.76 N N N N N N N N N N N N N N N N N N N	FILL (SF) 0:00 94:60 8:54 5:555 5:54 5:54 5:54 5:54 5:54 5:54 5:54 5:54 5:54 5:54	60 km		670 665 115 + 50.00 675 665 115 + 00.00 675 675
LINOIS ANSPORTATION SCALE: 1"=5' VERT. SHEET OF SHEETS STA. 114+00.00 TO STA. 116+00.00 ILLINOIS FED. AID PRC	6.65			STAGE-1: STAGE-2: STAGE-3: STAGE-3: STAGE-3: STAGE-3: STAGE-2: STAGE-3:	CUT (SF) 0.00 4.66 155.76 No 2.58 158.90 2.58 158.90	EILL (SE) 0.00 94.60 8.54 5.55 5.54 5.54 5.54 5.54 5.54 5.54 5.54 5.54 5.54 5.54	2.65 (a60: kT)		670 665 115 + 50.00 675 675 675 675 675 675 675
LINOIS CROSS SECTIONS – IL RTE 7 ANSPORTATION SCALE: 1"=10' HORIZ. SCALE: 1"=5' VERT. SHEET OF SHEETS STA. 114+00.00 TO STA.116+00.00 ILLINOIS FED. AID PROJECT	670 665 675 675 675 675			STAGE-1: STAGE-3: STAGE-3: STAGE-3: STAGE-3: STAGE-2: STAGE-2: STAGE-2:	CUT (SF) 0.00 4.66 155.76 2.58 158.90 2.58 158.90	FILL (SF) 0.00 94.60 8.54 5 10.71 10.71 10.71 10.71 10.71 10.71 10.71 10.71 10.71 10.71 10.71	(662.55) (46.1' RT)		670 665 115 + 50.00 670 670 670 670 670 670
LINOIS CROSS SECTIONS – IL RTE 7 ANSPORTATION SCALE: 1''=10' HORIZ. SCALE: 1''=5' VERT. SHEET OF SHEETS STA. 114+00.00 TO STA. 116+00.00 [ILLINOIS FED. AID PROJECT VERT.]	6.70 6.65 6.75 6.75 6.75			STAGE - 1: STAGE - 3: STAGE - 3: STAGE - 3: STAGE - 3: STAGE - 1: STAGE - 1: STAGE - 2: STAGE - 3: STAGE - 3:	CUT (SF) 0.00 4.66 155.76       	FILL (SE) 0.00 94.60 8.54 5 7 5 7 5 7 5 7 5 7 5 7 7 7 7 7 7 7 7 7 7 7 7 7	(662.55) (46.1' M)		670 665 115 + 50.00 670 665 115 + 00.00 665 115 + 00.00 670 670 670
LINOIS CROSS SECTIONS – IL RTE 7 ANSPORTATION SCALE: 1''=10' HORIZ. CROSS SECTIONS – IL RTE 7 SCALE: 1''=5' VERT. SHEET OF SHEETS STA. 114+00.00 TO STA. 116+00.00 [ILLINOIS FED. AID PROJECT]	670 665 675 675 675			STAGE - 1: STAGE - 3: STAGE - 3: STAGE - 3: STAGE - 3: STAGE - 1: STAGE - 2: STAGE - 3: STAGE - 3:	CUT (SF) 0,00 4,66 155.76 0,00 2,58 158.90 158.90 158.90 158.90 158.90	FILL (SF) 0.00 94.60 8.54 FILL (SF) 0.00 93.51 10.71 10.71 FILL (SF) 0.00 93.51 10.71	6662.65 (a60'RT)		6.70 6.65 115 + 50.00 6.70 6.70 6.70 6.75 115 + 00.00 6.75 6.70 6.70 6.70 6.70 6.70 6.70 6.70