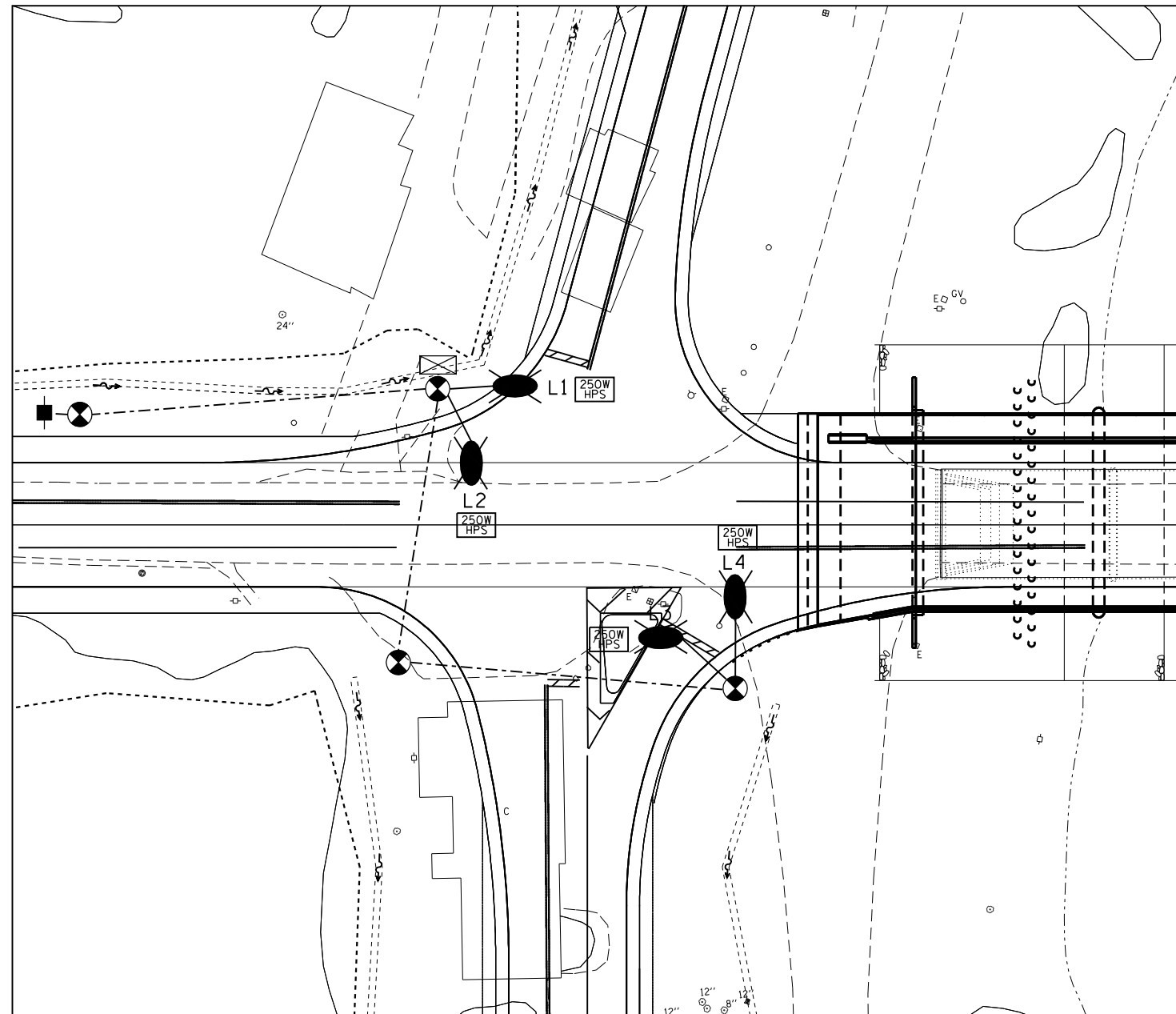


TEMPORARY LIGHTING



TEMPORARY LIGHTING LEGEND

- TEMPORARY LIGHTING/SERVICE CABLE
- ⊗ TEMPORARY WOOD POLE WITH GUY WIRES (40 FT. MH)
- ⊗ TEMPORARY 250W HPS TENON MOUNTED LUMINAIRE
- ⊠ TEMPORARY CONTROLLER CABINET
- TEMPORARY SERVICE INSTALLATION

TEMPORARY OVERHEAD LIGHTING CONSTRUCTION NOTES

- T1. THE CONTRACTOR SHALL FURNISH AND INSTALL EQUIPMENT WITH RESPECT TO THE TEMPORARY TRAFFIC LIGHTING INSTALLATION. THIS SHALL INCLUDE ALL CABLES, AERIAL CABLES, CONDUIT, LIGHTING CONTROLLER, PHOTOCELLS, ELECTRIC SERVICE, WOOD POLES, GUY WIRE, LUMINAIRES, AND ALL OTHER EQUIPMENT REQUIRED FOR THE INSTALLATION.
- T2. UPON REQUEST, THE DEPARTMENT WILL FURNISH FOUR TENON MOUNTED 250W HPS LUMINAIRES FOR USE WITH THE TEMPORARY LIGHTING INSTALLATION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PICKING UP THE ITEMS FROM IDOT AND TRANSPORTING THEM TO THE JOB. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING THE LUMINAIRES FOR THE DURATION OF THE TEMPORARY LIGHTING SYSTEM.
- T3. AERIAL LIGHTING CABLE, WOOD POLES, AND GUY WIRES SHALL BE FURNISHED AND INSTALLED AS REQUIRED AND SHOWN ON THE PLAN SHEETS.
- T4. ALL ELECTRICAL CABLES SHALL MAINTAIN A MINIMUM CLEARANCE ABOVE THE HIGHEST POINT OF THE ROADWAY.
- T5. THE TEMPORARY LUMINAIRES SHALL BE PLACED AS INDICATED ON THE PLANS AND AIMED ON THE ROADWAY TO SATISFACTION OF THE ENGINEER
- T6. THE CONTRACTOR SHALL PROVIDE ALL LABOR, EQUIPMENT, AND MATERIALS REQUIRED TO RELOCATE THE TEMPORARY LIGHTING IN ACCORDANCE WITH THE PROPOSED CONSTRUCTION STAGING.
- T7. THE CONTRACTOR SHALL MAINTAIN THE OPERATION OF THE TEMPORARY LIGHTING SYSTEM THROUGHOUT THE DURATION OF THE PROJECT.
- T8. ALL LABOR, MATERIALS, AND EQUIPMENT REQUIRED TO COMPLY WITH THESE REQUIREMENTS SHALL BE INCLUDED IN THE PRICE FOR TEMPORARY LIGHTING INSTALLATION.

THE CONTRACTOR SHALL COORDINATE THE PLACEMENT OF WOOD POLES, GUY WIRES, AND OTHER TEMPORARY LIGHTING EQUIPMENT WITH THE ENGINEER TO PREVENT CONFLICTS WITH CONSTRUCTION STAGING AND OVERHEAD UTILITIES.

REMOVE EXISTING FLASHING BEACON INSTALLATION COMPLETE - QTY. 1 EACH (INCLUDES ALL ITEMS LISTED BELOW)

THE FOLLOWING ITEMS SHALL BE REMOVED AND DISPOSED OF BY THE CONTRACTOR OFF OF THE RIGHT-OF-WAY. THE SALVAGE VALUE OF THIS EQUIPMENT SHALL BE REFLECTED IN THE UNIT BID PRICE FOR REMOVE EXISTING TRAFFIC SIGNAL EQUIPMENT.

QTY.	ITEM
1.0	WOOD SERVICE POLE
3.0	SIGNAL HEAD, 1-FACE, 1-SECTION POST MOUNTED
3.0	JUNCTION BOXES
1.0	ELECTRIC SERVICE
ALL	ELECTRIC CABLE, CONDUITS, ETC.

THE FOLLOWING ITEMS SHALL BE REMOVED AND DELIVERED TO THE IDOT TRAFFIC BUILDING LOCATED AT 1025 W. DETWEILLER DR., PEORIA:

1.0	FLASHER CONTROLLER
1.0	SOLAR POWERED FLASHER

BILL OF MATERIALS (LIGHTING AND SIGNALS) - SHEET 2 OF 6
FARMINGTON ROAD & KICKAPOO CREEK ROAD

ITEM DESCRIPTION	UNIT	TOTAL QTY.
REMOVE EXISTING FLASHING BEACON INSTALLATION COMPLETE	EACH	1.0
TEMPORARY LIGHTING SYSTEM	LSUM	1.0

SIGNALS AND LIGHTING
SHEET 2 OF 6

FILE NAME = D468185-SHT-LIGHT.dgn

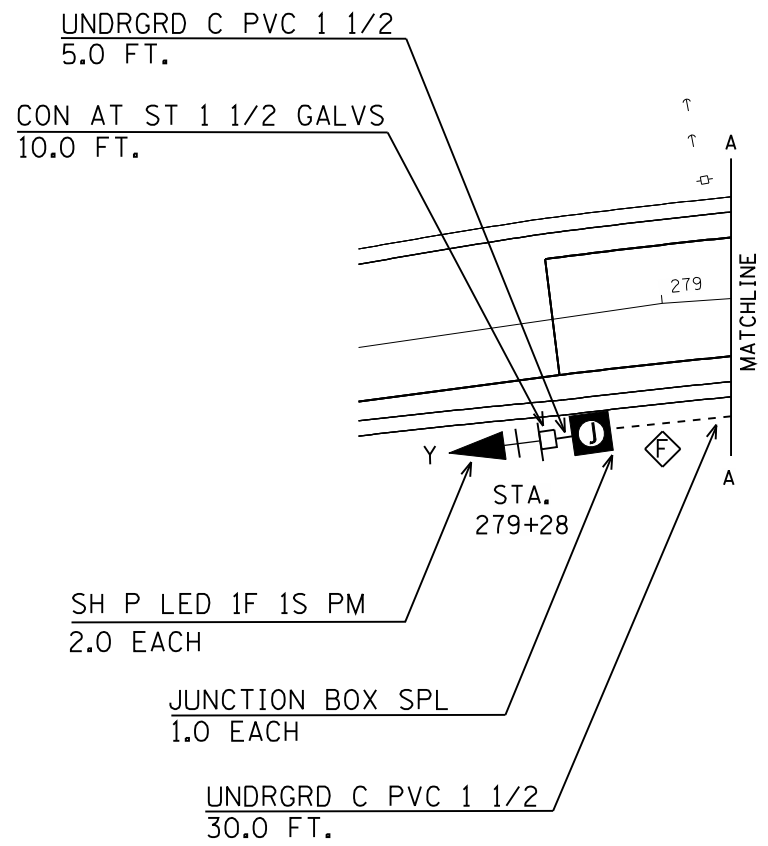
USER NAME = g_jameson	DESIGNED -	REVISED -
	DRAWN -	REVISED -
PLOT SCALE = 38.0568' / IN.	CHECKED -	REVISED -
PLOT DATE = 8/14/2018	DATE -	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

FARMINGTON RD. & KICKAPOO CREEK RD.
REMOVAL OF EXISTING FLASHING BEACONS AND TEMPORARY LIGHTING

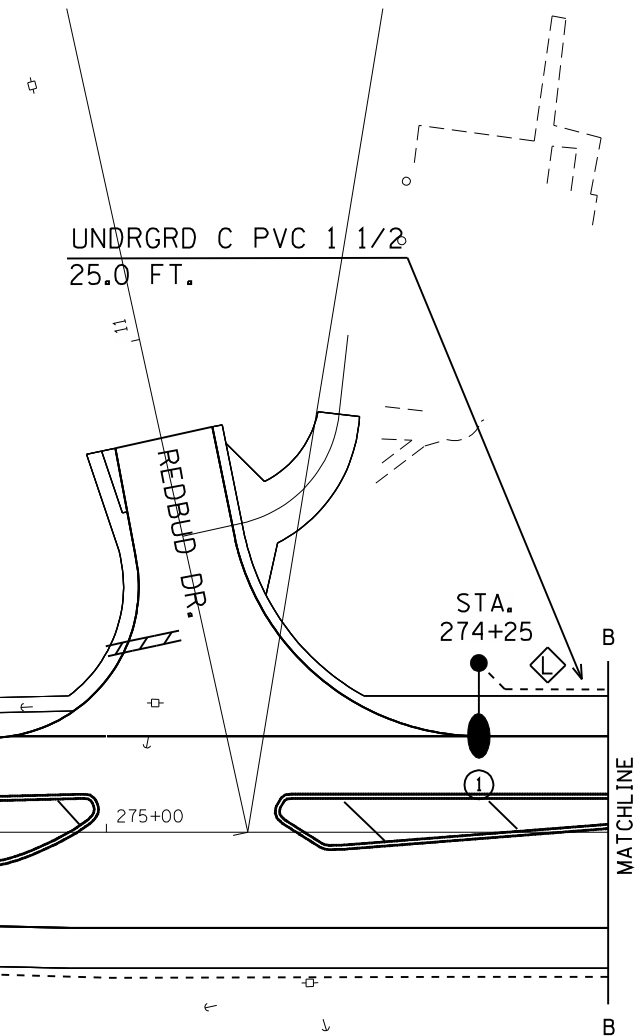
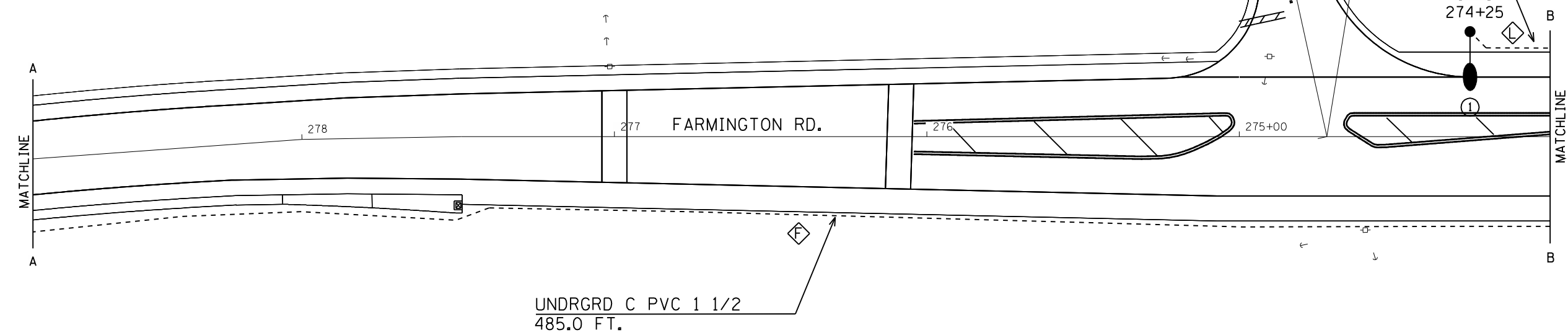
SCALE: STA. TO STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
0TH.	11(N,BR-1,RS-4,W-1)	PEORIA	577	401
CONTRACT NO. 68185				
ILLINOIS FED. AID PROJECT				



**BILL OF MATERIALS (LIGHTING AND SIGNALS) - SHEET 3 OF 6
FARMINGTON ROAD & KICKAPOO CREEK ROAD**

ITEM DESCRIPTION	UNIT	TOTAL QTY.
UNDERGROUND CONDUIT, PVC, 1 1/2" DIA.	FOOT	545.0
CONDUIT ATTACHED TO STRUCTURE, 1 1/2" DIA., GALVANIZED STEEL	FOOT	10.0
ELECTRIC CABLE IN CONDUIT, 600V (XLP-TYPE USE) 1/C NO. 8	FOOT	93.0
LIGHT POLE, GALVANIZED STEEL, 45 FT. M.H., 15 FT. MAST ARM	EACH	1.0
LIGHT POLE FOUNDATION, 30" DIAMETER	FOOT	6.5
BREAKAWAY DEVICE, TRANSFORMER BASE, 15 INCH BOLT CIRCLE	EACH	1.0
ELECTRIC CABLE IN CONDUIT, SIGNAL NO. 14 5C	FOOT	533.0
SIGNAL HEAD, POLYCARBONATE, LED, 1-FACE, 1-SECTION, BRACKET MOUNTED	EACH	2.0
TRAFFIC SIGNAL BACKPLATE, RETROREFLECTIVE	EACH	2.0
LUMINAIRE, LED, HORIZONTAL MOUNT HIGH WATTAGE	EACH	1.0
JUNCTION BOX (SPECIAL)	EACH	1.0



(F) DENOTES #14 3/C WIRE FOR FLASHING BEACON
 (L) DENOTES #8 1/C (X3) FOR OVERHEAD LIGHTING

FILE NAME = D468185-SHT-LIGHT.dgn

USER NAME = gJameson	DESIGNED -	REVISED -
	DRAWN -	REVISED -
PLOT SCALE = 38.0568' / IN.	CHECKED -	REVISED -
PLOT DATE = 8/14/2018	DATE -	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

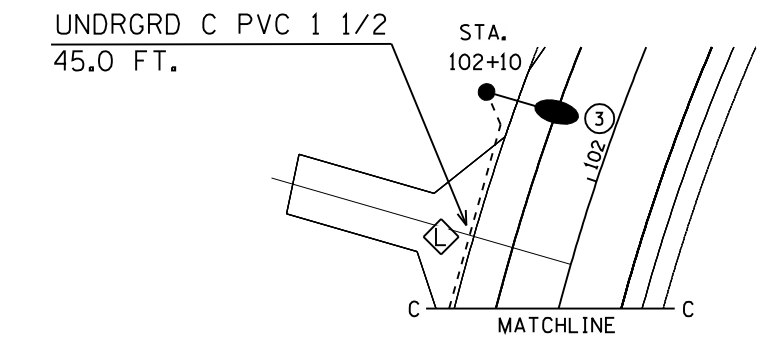
**PROPOSED FLASHING BEACON AND OVERHEAD LIGHTING
FARMINGTON RD. & CREEK RD.**

SCALE: _____ STA. TO STA. _____

SIGNALS AND LIGHTING SHEET 3 OF 6		F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
OTH.	11(N,BR-1,RS-4,W-1)	PEORIA			577	402
					CONTRACT NO. 68185	
ILLINOIS FED. AID PROJECT						

CONDUIT EMBEDDED IN STRUCTURE SHALL BE INSTALLED IN ACCORDANCE WITH HIGHWAY STANDARD 812001

BILL OF MATERIALS (LIGHTING AND SIGNALS) - SHEET 4 OF 6 FARMINGTON ROAD & KICKAPOO CREEK ROAD		
ITEM DESCRIPTION	UNIT	TOTAL QTY.
SERVICE INSTALLATION, TYPE B	EACH	1.0
UNDERGROUND CONDUIT, PVC, 1 1/2" DIA.	FOOT	894.0
UNDERGROUND CONDUIT, PVC, 2" DIA.	FOOT	268.0
CONDUIT ATTACHED TO STRUCTURE, 1 1/2" DIA., GALVANIZED STEEL	FOOT	40.0
CONDUIT EMBEDDED IN STRUCTURE, 2" DIA., PVC	FOOT	135.0
JUNCTION BOX, STAINLESS STEEL, ATTACHED TO STRUCTURE, 18" X 18" X 10"	EACH	1.0
HANDHOLE, PORTLAND CEMENT CONCRETE	EACH	3.0
ELECTRIC CABLE IN CONDUIT, 600V (XLP-TYPE USE) 1/C NO. 8	FOOT	1783.0
LIGHTING CONTROLLER, POLE MOUNTED, 240VOLT, 30AMP	EACH	1.0
LIGHT POLE, GALVANIZED STEEL, 45 FT. M.H., 15 FT. MAST ARM	EACH	4.0
LIGHT POLE FOUNDATION, 30" DIAMETER	FOOT	26.0
BREAKAWAY DEVICE, TRANSFORMER BASE, 15 INCH BOLT CIRCLE	EACH	4.0
ELECTRIC CABLE IN CONDUIT, SIGNAL NO. 14 5C	FOOT	1010.5
SIGNAL HEAD, POLYCARBONATE, LED, 1-FACE, 1-SECTION, BRACKET MOUNTED	EACH	2.0
TRAFFIC SIGNAL BACKPLATE, RETROREFLECTIVE	EACH	2.0
LUMINAIRE, LED, HORIZONTAL MOUNT HIGH WATTAGE	EACH	4.0
JUNCTION BOX (SPECIAL)	EACH	1.0



UNDRGRD C PVC 1 1/2
175.0 FT.

CON AT ST 1 1/2 GALVS
10.0 FT.

UNDRGRD C PVC 1 1/2
35.0 FT.

CON AT ST 1 1/2 GALVS
10.0 FT.

LT CONT PM 240V 30
1.0 EACH

SERV INSTALL TY B
1.0 EACH

UNDRGRD C PVC 2
3.0 FT.

UNDRGRD C PVC 1 1/2
15.0 FT.

UNDRGRD C PVC 1 1/2
180.0 FT.

UNDRGRD C PVC 1 1/2
5.0 FT.

UNDRGRD C PVC 2
95.0 FT.

UNDRGRD C PVC 1 1/2
188.0 FT.

UNDRGRD C PVC 2
73.0 FT.

UNDRGRD C PVC 1 1/2
5.0 FT.

CON AT ST 1 1/2 GALVS
10.0 FT.

UNDRGRD C PVC 1 1/2
58.0 FT.

UNDRGRD C PVC 1 1/2
65.0 FT.

UNDRGRD C PVC 1 1/2
123.0 FT.

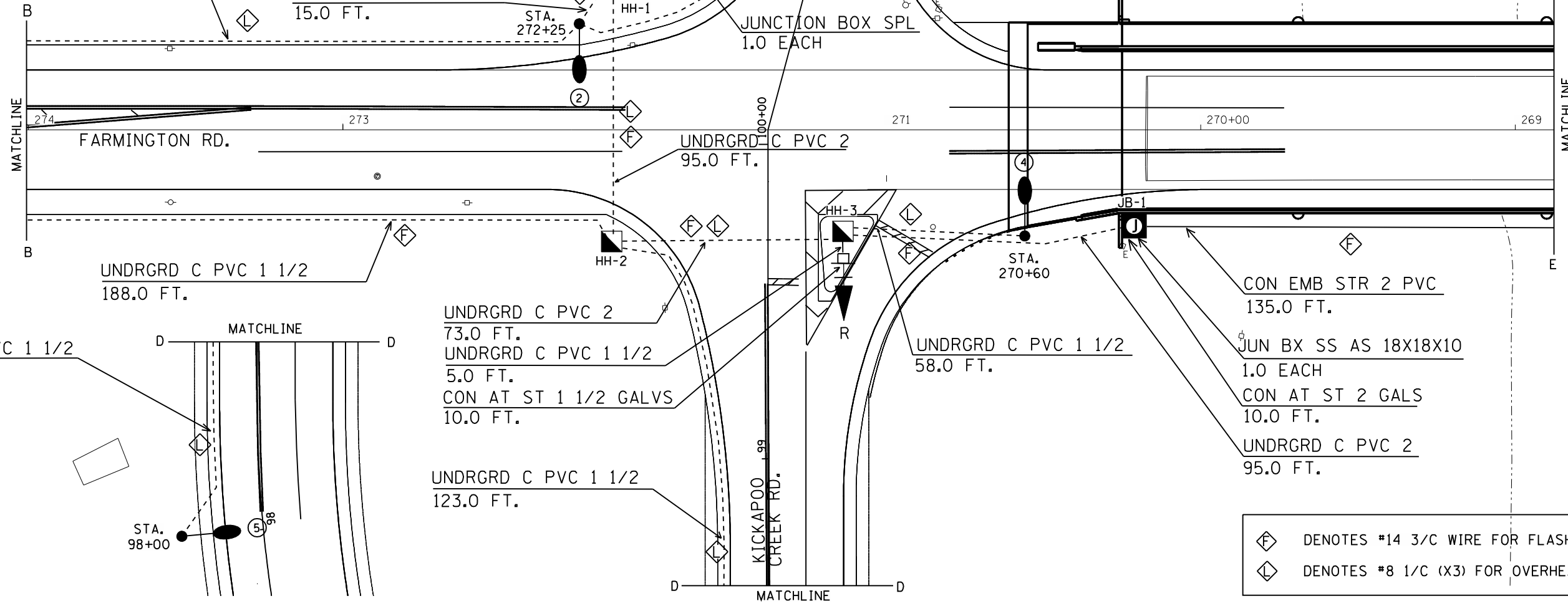
CON EMB STR 2 PVC
135.0 FT.

JUN BX SS AS 18X18X10
1.0 EACH

CON AT ST 2 GALS
10.0 FT.

UNDRGRD C PVC 2
95.0 FT.

- ⬡ DENOTES #14 3/C WIRE FOR FLASHING BEACON
- ⬢ DENOTES #8 1/C (X3) FOR OVERHEAD LIGHTING



FILE NAME = D468185-SHT-LIGHT.dgn

USER NAME = g_jameson	DESIGNED -	REVISED -
	DRAWN -	REVISED -
PLOT SCALE = 38.0568' / IN.	CHECKED -	REVISED -
PLOT DATE = 8/14/2018	DATE -	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

PROPOSED FLASHING BEACON AND OVERHEAD LIGHTING
FARMINGTON RD. & CREEK RD.

SCALE: STA. TO STA.

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
0TH.	11(N, BR-1, RS-4, W-1)	PEORIA	577	403
CONTRACT NO. 68185				

SIGNALS AND LIGHTING
SHEET 4 OF 6

ILLINOIS FED. AID PROJECT

**BILL OF MATERIALS (LIGHTING AND SIGNALS) - SHEET 5 OF 6
FARMINGTON ROAD & KICKAPOO CREEK ROAD**

ITEM DESCRIPTION	UNIT	TOTAL QTY.
UNDERGROUND CONDUIT, PVC, 1 1/2" DIA.	FOOT	500.0
CONDUIT ATTACHED TO STRUCTURE, 1 1/2" DIA., GALVANIZED STEEL	FOOT	20.0
CONDUIT EMBEDDED IN STRUCTURE, 2" DIA., PVC	FOOT	145.0
JUNCTION BOX, STAINLESS STEEL, ATTACHED TO STRUCTURE, 18" X 18" X 10"	EACH	1.0
ELECTRIC CABLE IN CONDUIT, SIGNAL NO. 14 5C	FOOT	664.0
SIGNAL HEAD, POLYCARBONATE, LED, 1-FACE, 1-SECTION, BRACKET MOUNTED	EACH	2.0
TRAFFIC SIGNAL BACKPLATE, RETROREFLECTIVE	EACH	2.0
JUNCTION BOX (SPECIAL)	EACH	2.0

JUNCTION BOX SPL
1.0 EACH

UNDRGRD C PVC 1 1/2
5.0 FT.

STA.
263+16

SH P LED 1F 1S BM
2.0 EACH

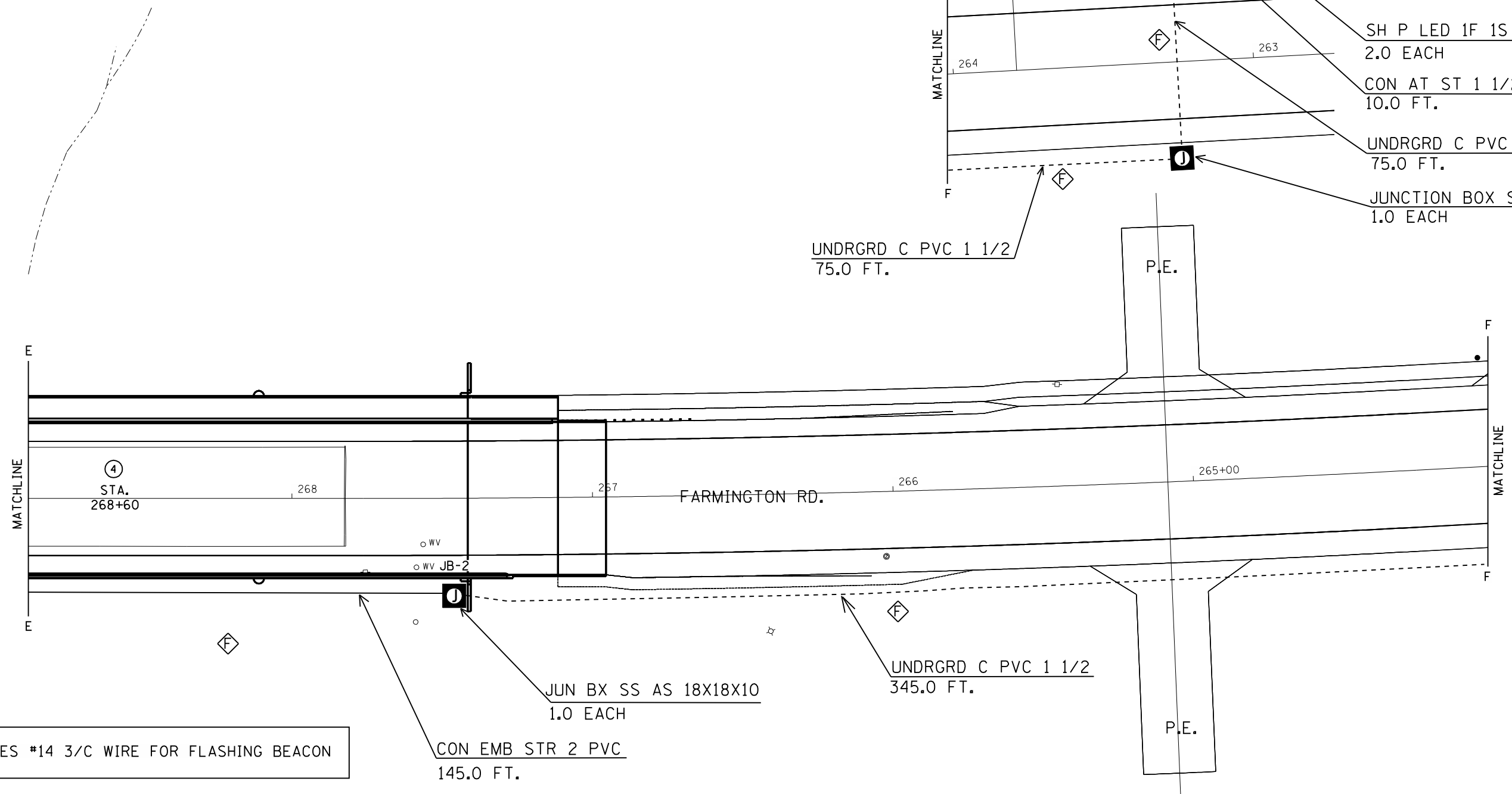
CON AT ST 1 1/2 GALVS
10.0 FT.

UNDRGRD C PVC 1 1/2
75.0 FT.

JUNCTION BOX SPL
1.0 EACH

UNDRGRD C PVC 1 1/2
75.0 FT.

P.E.



⬡ DENOTES #14 3/C WIRE FOR FLASHING BEACON

JUN BX SS AS 18X18X10
1.0 EACH

CON EMB STR 2 PVC
145.0 FT.

UNDRGRD C PVC 1 1/2
345.0 FT.

P.E.

FILE NAME = D468185-SHT-LIGHT.dgn

SIGNALS AND LIGHTING
SHEET 5 OF 6

USER NAME = g_jameson	DESIGNED -	REVISED -
	DRAWN -	REVISED -
PLOT SCALE = 38.0568' / IN.	CHECKED -	REVISED -
PLOT DATE = 8/14/2018	DATE -	REVISED -

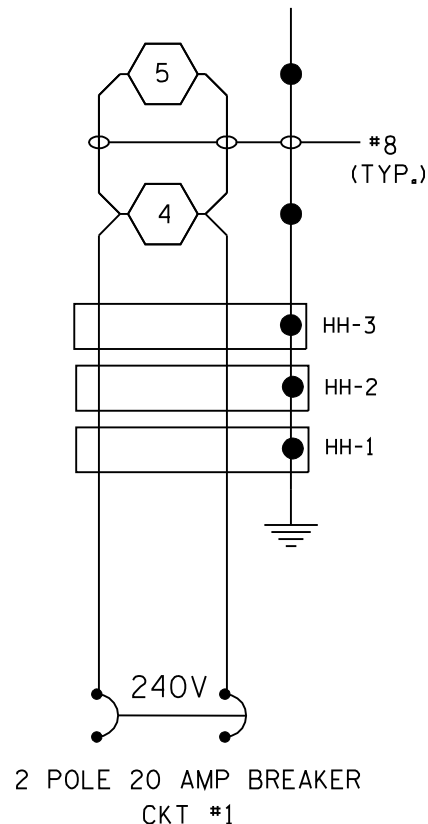
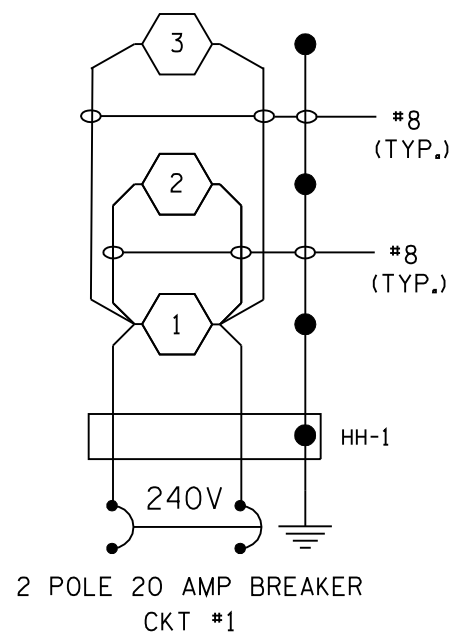
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**PROPOSED FLASHING BEACON AND OVERHEAD LIGHTING
FARMINGTON RD. & CREEK RD.**

SCALE: _____ STA. TO STA. _____

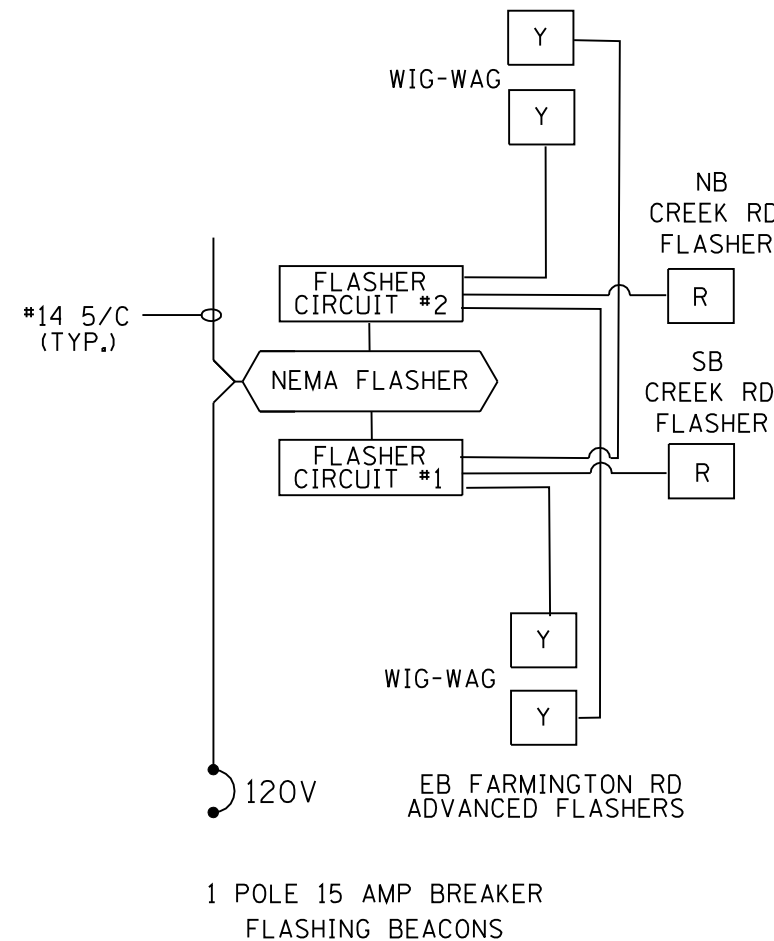
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
0TH.	11(N,BR-1,RS-4,W-1)	PEORIA	577	404
CONTRACT NO. 68185				
ILLINOIS FED. AID PROJECT				

WIRING DIAGRAM
PROPOSED LIGHTING CONTROLLER
WITH FLASHING BEACON CIRCUIT








ALL LIGHT POLES, METALLIC CONDUIT, AND JUNCTION BOXES SHALL BE GROUNDED IN ACCORDANCE WITH NEC REQUIREMENTS

WB FARMINGTON RD
ADVANCED FLASHERS

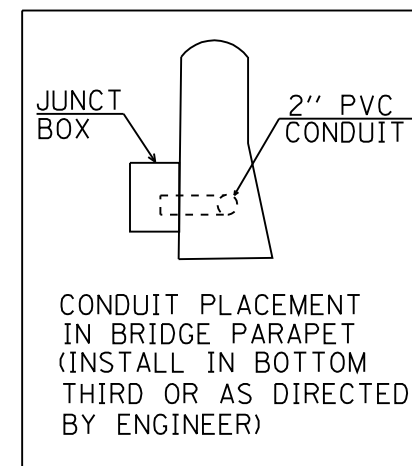
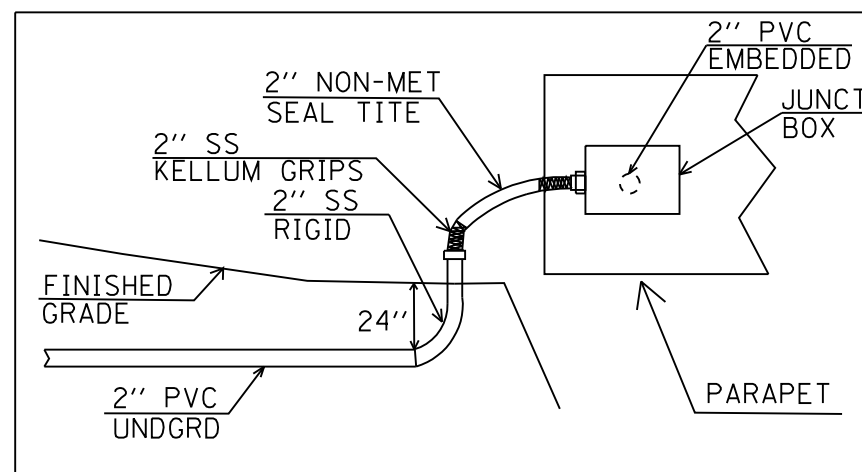


NOTES:

- ALL NECESSARY REVISIONS TO THE WIRING SHOWN ON THIS SHEET SHALL BE MADE AT NO ADDITIONAL COST TO THE DEPARTMENT AND TO THE SATISFACTION OF THE ENGINEER.
- THE LIGHTING CONTROLLER SHALL CONTAIN A NEMA FLASHER SOCKET, TERMINAL BLOCKS, AND ALL OTHER ITEMS REQUIRED FOR FLASHING BEACON OPERATION. THE COST OF THESE ITEMS SHALL BE INCLUDED IN THE COST OF THE LIGHTING CONTROLLER.
- THE LIGHTING CONTROLLER SHALL CONTAIN ONE SPARE 20A BREAKER FOR LIGHTING.

-  PROPOSED SIGNAL HEAD, 1F 1-SECT, POST MOUNTED (RED)
-  PROPOSED SIGNAL HEAD, 1F 1-SECT, POST MOUNTED (YELLOW)
-  PROPOSED LED LUMINAIRE
-  PROPOSED JUNCTION BOX
-  GROUND ROD

CONDUIT
INSTALLATION
DETAILS



CONDUIT EMBEDDED IN STRUCTURE SHALL BE INSTALLED IN ACCORDANCE WITH HIGHWAY STANDARD 812001

CONDUIT PLACEMENT IN BRIDGE PARAPET (INSTALL IN BOTTOM THIRD OR AS DIRECTED BY ENGINEER)

USER NAME = g_jameson	DESIGNED -	REVISED -
	DRAWN -	REVISED -
PLOT SCALE = 38.0568' / IN.	CHECKED -	REVISED -
PLOT DATE = 8/14/2018	DATE -	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

PROPOSED FLASHER AND OVERHEAD LIGHTING WIRING DIAGRAM
FARMINGTON RD. & CREEK RD.

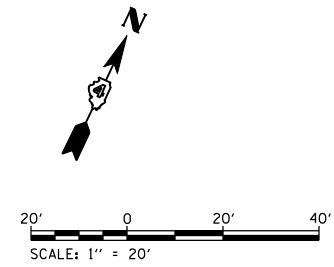
SCALE: _____ STA. _____ TO STA. _____

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
0TH.	11(N,BR-1,RS-4,W-1)	PEORIA	577	405
CONTRACT NO. 68185				
ILLINOIS FED. AID PROJECT				

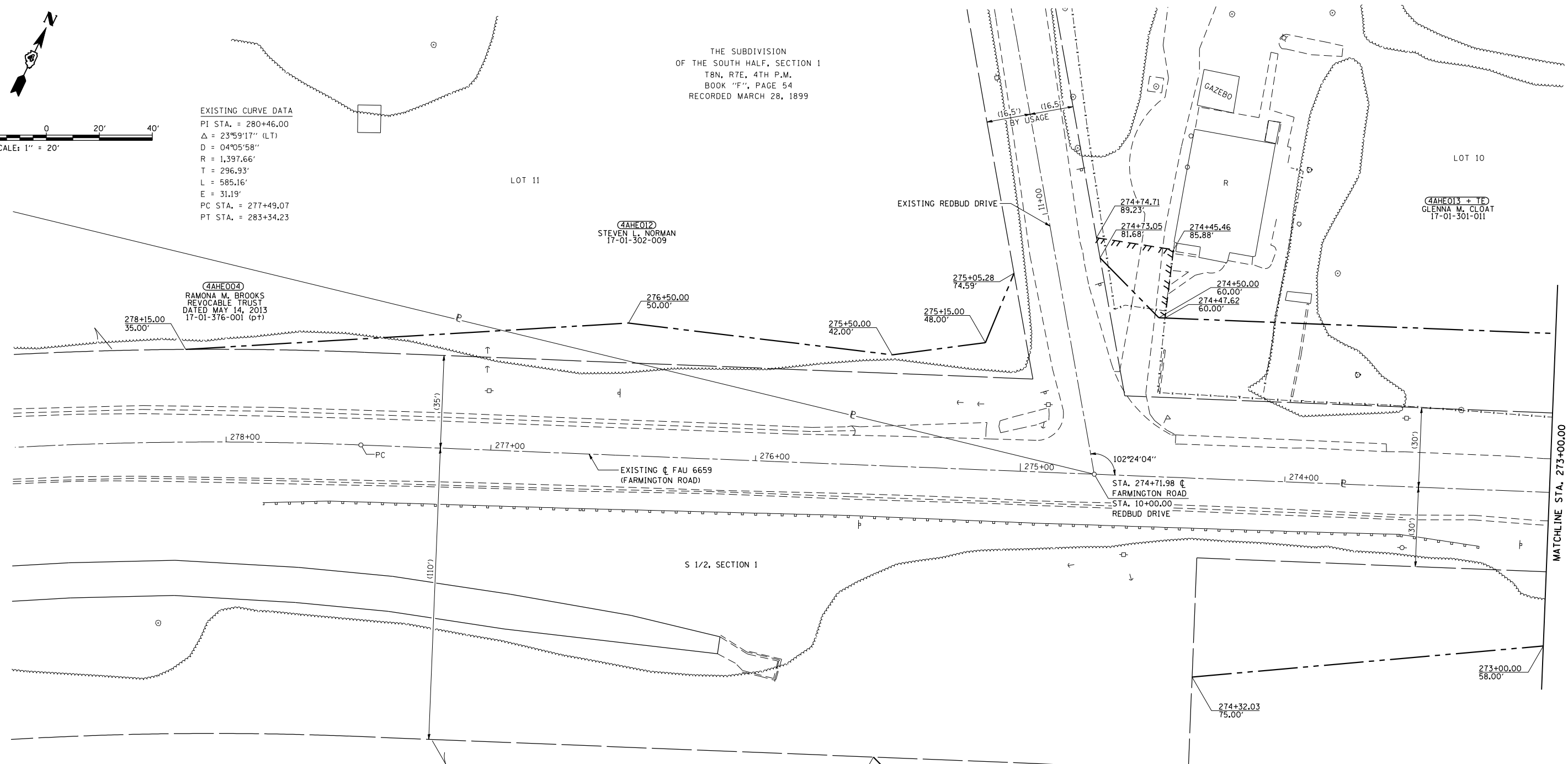
BEARINGS ARE BASED ON CONTROL PROVIDED BY IDOT, DISTRICT 4

SEC 1, T 8 N, R 7 E 4th P.M.

THE SUBDIVISION OF THE SOUTH HALF, SECTION 1 T8N, R7E, 4TH P.M. BOOK "E", PAGE 54 RECORDED MARCH 28, 1899



EXISTING CURVE DATA
 PI STA. = 280+46.00
 $\Delta = 23^{\circ}59'17''$ (LT)
 $D = 04^{\circ}05'58''$
 $R = 1,397.66'$
 $T = 296.93'$
 $L = 585.16'$
 $E = 31.19'$
 PC STA. = 277+49.07
 PT STA. = 283+34.23



(4AHE004)
 RAMONA M. BROOKS
 REVOCABLE TRUST
 DATED MAY 14, 2013
 17-01-376-001 (p+)

(4AHE012)
 STEVEN L. NORMAN
 17-01-302-009

(4AHE013 + TE)
 GLENNA M. CLOAT
 17-01-301-011

(4AHE001 + TE)
 RAMONA M. BROOKS
 REVOCABLE TRUST
 DATED MAY 14, 2013
 17-01-376-001 (p+)

LEGEND

	PROPOSED CENTERLINE
	EXISTING CENTERLINE
	EXISTING RIGHT OF WAY LINE
	PROPOSED RIGHT OF WAY LINE
	PROPOSED TEMPORARY EASEMENT LINE
	PROPOSED PERMANENT EASEMENT LINE
	SECTION LINE
	QUARTER SECTION LINE
	PROPERTY LINE
	EXISTING EASEMENT

FOR OLD ROW SEE:
 PLANFILE 3 SHEETS 43 & 44

8 73

PLAT FILE 527

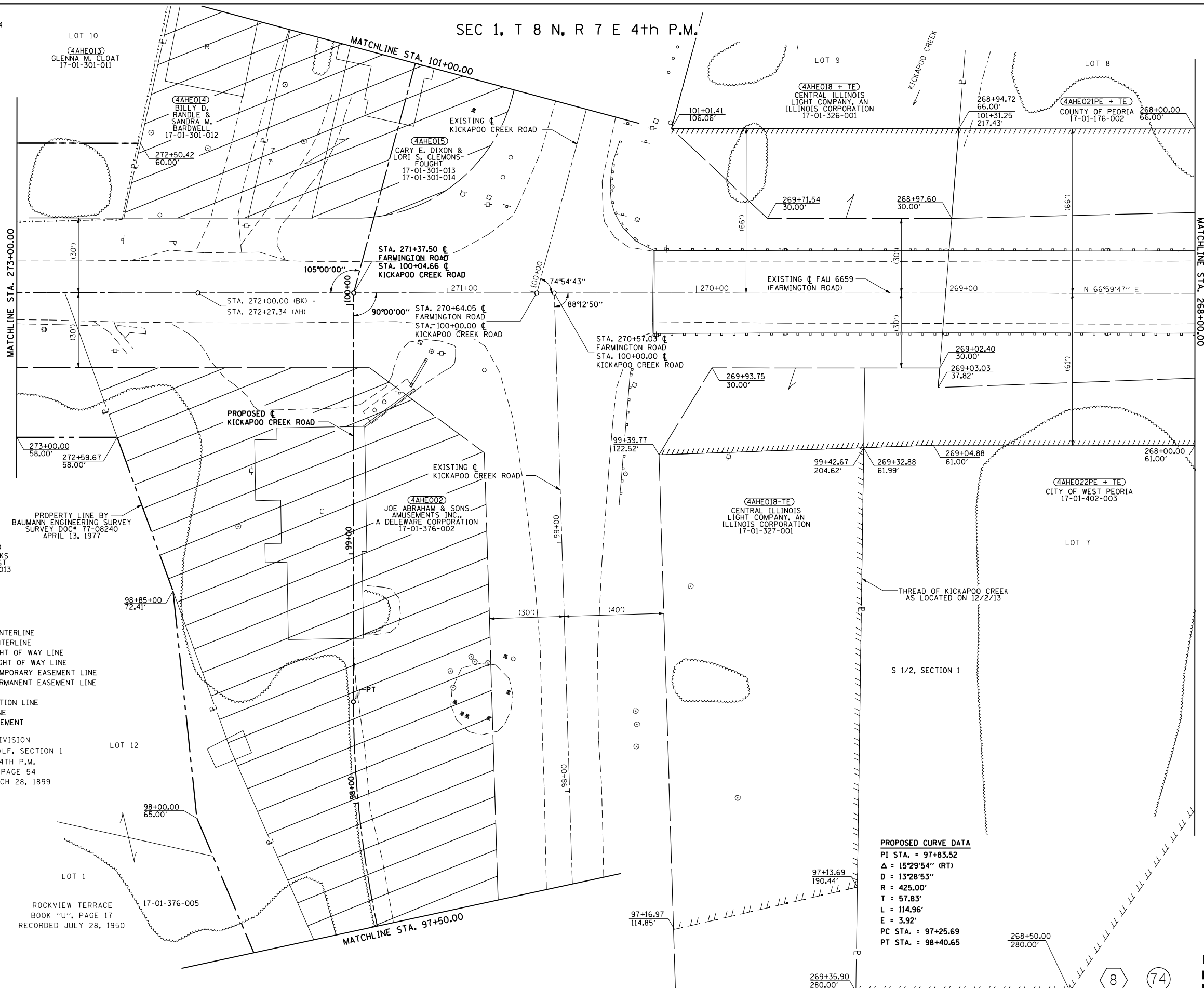
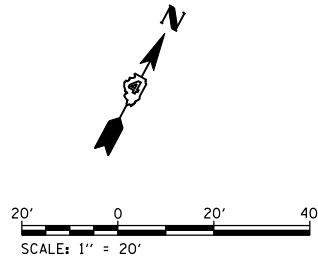
FILE NAME	USER NAME = g_jameson	DESIGNED -	REVISED -
FILEABBREV		DRAWN - DAS	REVISED -
		CHECKED - JWD	REVISED -
		DATE - 04/2017	REVISED -

STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

RIGHT OF WAY PLANS		F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
PROJECT	JOB NO. R-94-007-13	6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	406
SCALE: 1" = 20'	SHEET NO. 01 OF 9 SHEETS	STA. 278+15.00 TO STA. 273+00.00	CATALOG# 032475-00	CONTRACT NO. 68185		
			FED. ROAD DIST. NO.	ILLINOIS FED. AID PROJECT		

BEARINGS ARE BASED ON CONTROL PROVIDED BY IDOT, DISTRICT 4

SEC 1, T 8 N, R 7 E 4th P.M.



4AHE001 + TE
RAMONA M. BROOKS
REVOCABLE TRUST
DATED MAY 14, 2013
17-01-376-001

LEGEND

- PROPOSED CENTERLINE
- EXISTING CENTERLINE
- EXISTING RIGHT OF WAY LINE
- PROPOSED RIGHT OF WAY LINE
- PROPOSED TEMPORARY EASEMENT LINE
- PROPOSED PERMANENT EASEMENT LINE
- SECTION LINE
- QUARTER SECTION LINE
- PROPERTY LINE
- EXISTING EASEMENT

THE SUBDIVISION
OF THE SOUTH HALF, SECTION 1
T8N, R7E, 4TH P.M.,
BOOK "F", PAGE 54
RECORDED MARCH 28, 1899

LOT 1
ROCKVIEW TERRACE
BOOK "U", PAGE 17
RECORDED JULY 28, 1950

PROPOSED CURVE DATA

PI STA. = 97+83.52
Δ = 15°29'54" (RT)
D = 13°28'53"
R = 425.00'
T = 57.83'
L = 114.96'
E = 3.92'
PC STA. = 97+25.69
PT STA. = 98+40.65

**PLAT FILE 527
FOR OLD ROW SEE:
PLATFILE 3 SHEETS 43 & 44**

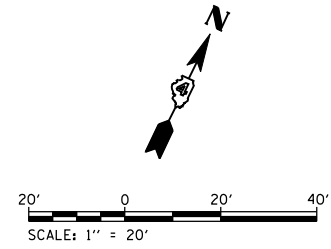
FILE NAME	USER NAME = g_jameson	DESIGNED -	REVISED -
#FILEABBREV#		DRAWN - DAS	REVISED -
		CHECKED - JWD	REVISED -
		DATE - 4/2017	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

RIGHT OF WAY PLANS	
PROJECT	JOB NO. R-94-007-13
SCALE: 1" = 20'	SHEET NO. 02 OF 9 SHEETS
	STA. 273+00.00 TO STA. 268+00.00

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	407
	CATALOG# 032475-00		CONTRACT NO. 68185	
FED. ROAD DIST. NO.	ILLINOIS FED. AID PROJECT			

BEARINGS ARE BASED ON CONTROL PROVIDED BY IDOT, DISTRICT 4

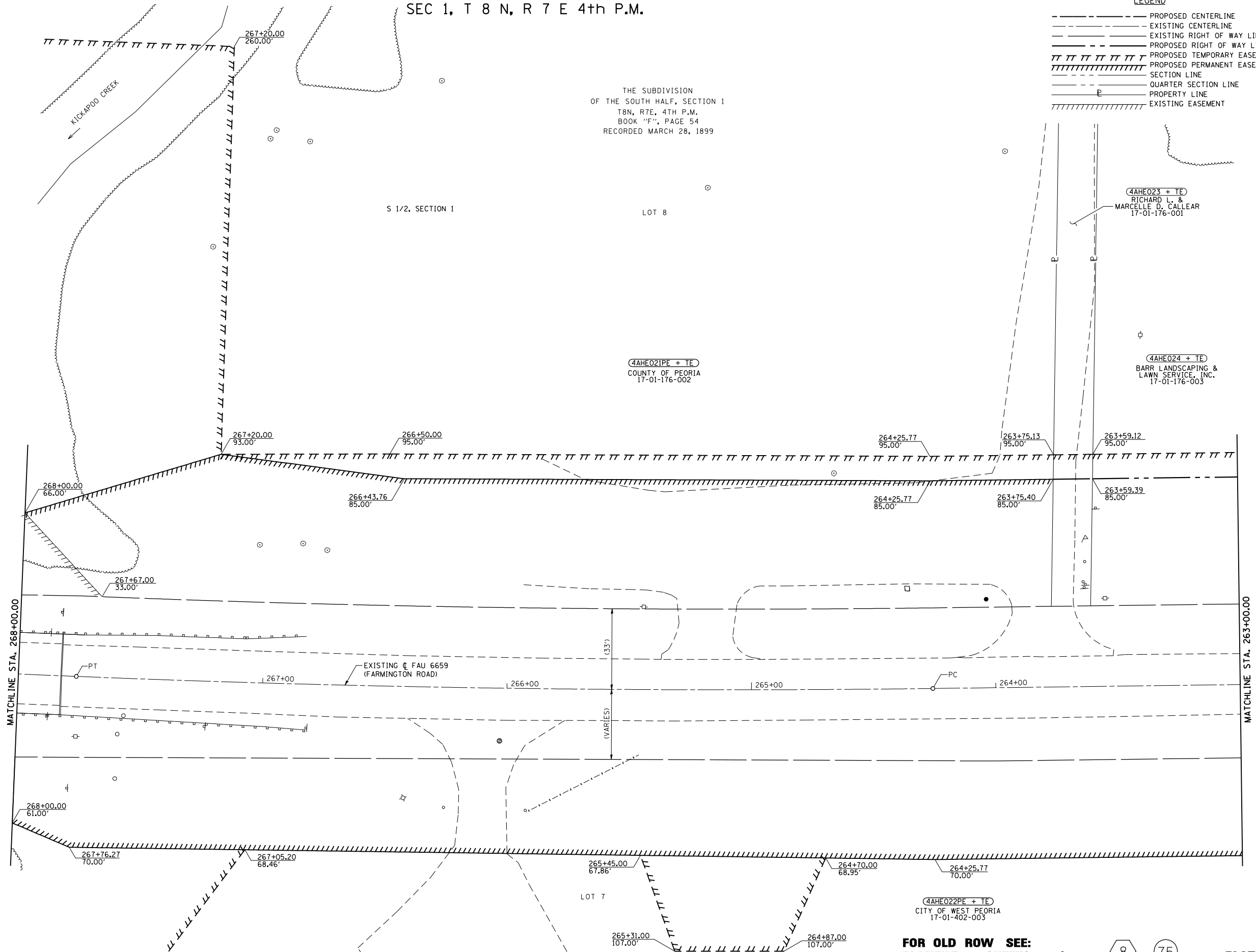


SEC 1, T 8 N, R 7 E 4th P.M.

THE SUBDIVISION
OF THE SOUTH HALF, SECTION 1
T8N, R7E, 4TH P.M.
BOOK "F", PAGE 54
RECORDED MARCH 28, 1899

LEGEND

- PROPOSED CENTERLINE
- EXISTING CENTERLINE
- EXISTING RIGHT OF WAY LINE
- PROPOSED RIGHT OF WAY LINE
- ||||| PROPOSED TEMPORARY EASEMENT LINE
- ||||| PROPOSED PERMANENT EASEMENT LINE
- SECTION LINE
- QUARTER SECTION LINE
- PROPERTY LINE
- ||||| EXISTING EASEMENT



EXISTING CURVE DATA
PI STA. = 266+01.06
 $\Delta = 03^{\circ}04'56''$ (RT)
D = $00^{\circ}52'46''$
R = 6,515.60'
T = 175.29'
L = 350.50'
E = 2.36'
PC STA. = 264+25.77
PT STA. = 267+76.27

FOR OLD ROW SEE:
PLANFILE 3 SHEETS 43 & 44

8 75

PLAT FILE 527

FILE NAME	USER NAME = g_jameson	DESIGNED -	REVISED -
FILELABBRV		DRAWN - DAS	REVISED -
	PLOT SCALE = 40.0000' / IN.	CHECKED - JWD	REVISED -
	PLOT DATE = 8/14/2018	DATE - 04/2017	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

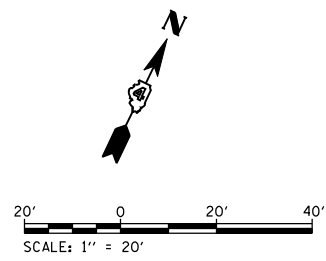
RIGHT OF WAY PLANS		F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
PROJECT	JOB NO. R-94-007-13	6659	11(N,BR-1,RS-4,W-1)	PEORIA	577	408
SCALE: 1" = 20'	SHEET NO. 03 OF 9 SHEETS	CATALOG# 032475-00		CONTRACT NO. 68185		
		FED. ROAD DIST. NO.		ILLINOIS FED. AID PROJECT		

BEARINGS ARE BASED ON CONTROL PROVIDED BY IDOT, DISTRICT 4

SEC 1, T 8 N, R 7 E 4th P.M.

LEGEND

- PROPOSED CENTERLINE
- EXISTING CENTERLINE
- EXISTING RIGHT OF WAY LINE
- PROPOSED RIGHT OF WAY LINE
- ||||| PROPOSED TEMPORARY EASEMENT LINE
- ||||| PROPOSED PERMANENT EASEMENT LINE
- SECTION LINE
- QUARTER SECTION LINE
- PROPERTY LINE
- ||||| EXISTING EASEMENT

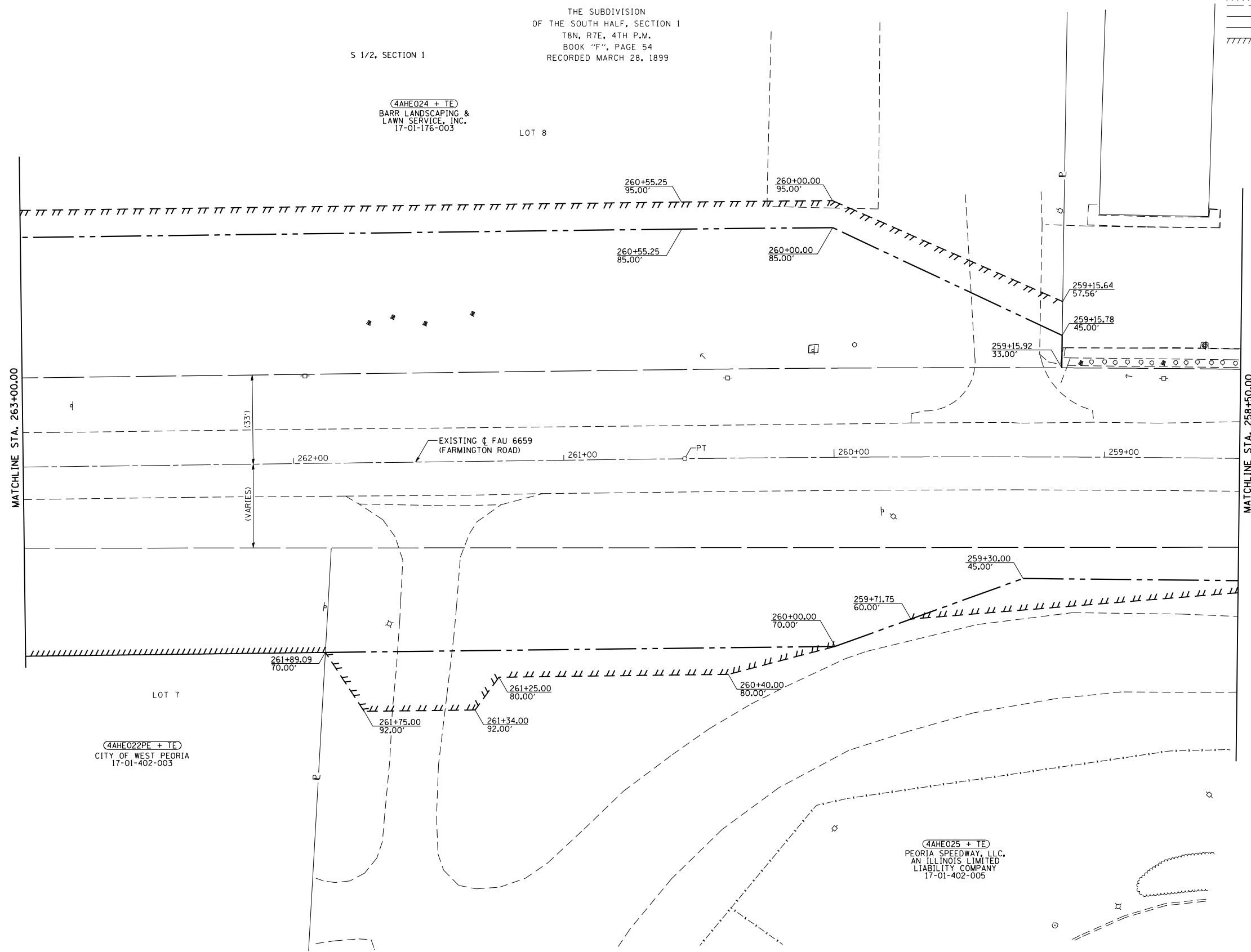


THE SUBDIVISION
OF THE SOUTH HALF, SECTION 1
T8N, R7E, 4TH P.M.
BOOK "F", PAGE 54
RECORDED MARCH 28, 1899

S 1/2, SECTION 1

(4AHE024 + TE)
BARR LANDSCAPING &
LAWN SERVICE, INC.
17-01-176-003

LOT 8



(4AHE022PE + TE)
CITY OF WEST PEORIA
17-01-402-003

(4AHE025 + TE)
PEORIA SPEEDWAY, LLC,
AN ILLINOIS LIMITED
LIABILITY COMPANY
17-01-402-005

EXISTING CURVE DATA
PI STA. = 257+90.22
Δ = 03°36'55" (LT)
D = 00°40'55"
R = 8,403.54'
T = 265.21'
L = 530.24'
E = 4.18'
PC STA. = 255+25.01
PT STA. = 260+55.25

FOR OLD ROW SEE:
PLANFILE 3 SHEETS 43 & 44

8 76

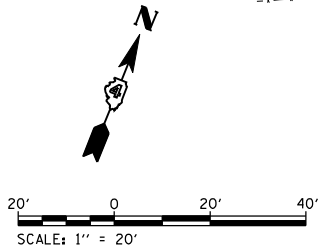
PLAT FILE 527

FILE NAME	USER NAME = g_jameson	DESIGNED -	REVISED -
FILE ABBREV		DRAWN - DAS	REVISED -
	PLOT SCALE = 40.0000' / IN.	CHECKED - JWD	REVISED -
	PLOT DATE = 8/14/2018	DATE - 04/2017	REVISED -

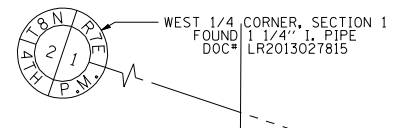
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

RIGHT OF WAY PLANS		F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
PROJECT	JOB NO. R-94-007-13	6659	11(N,BR-1,RS-4,W-1)	PEORIA	577	409
SCALE: 1" = 20'	SHEET NO. 04 OF 9 SHEETS	CATALOG# 032475-00		CONTRACT NO. 68185		
STA. 263+00.00 TO STA. 258+50.00		FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT				

BEARINGS ARE BASED ON CONTROL PROVIDED BY IDOT, DISTRICT 4



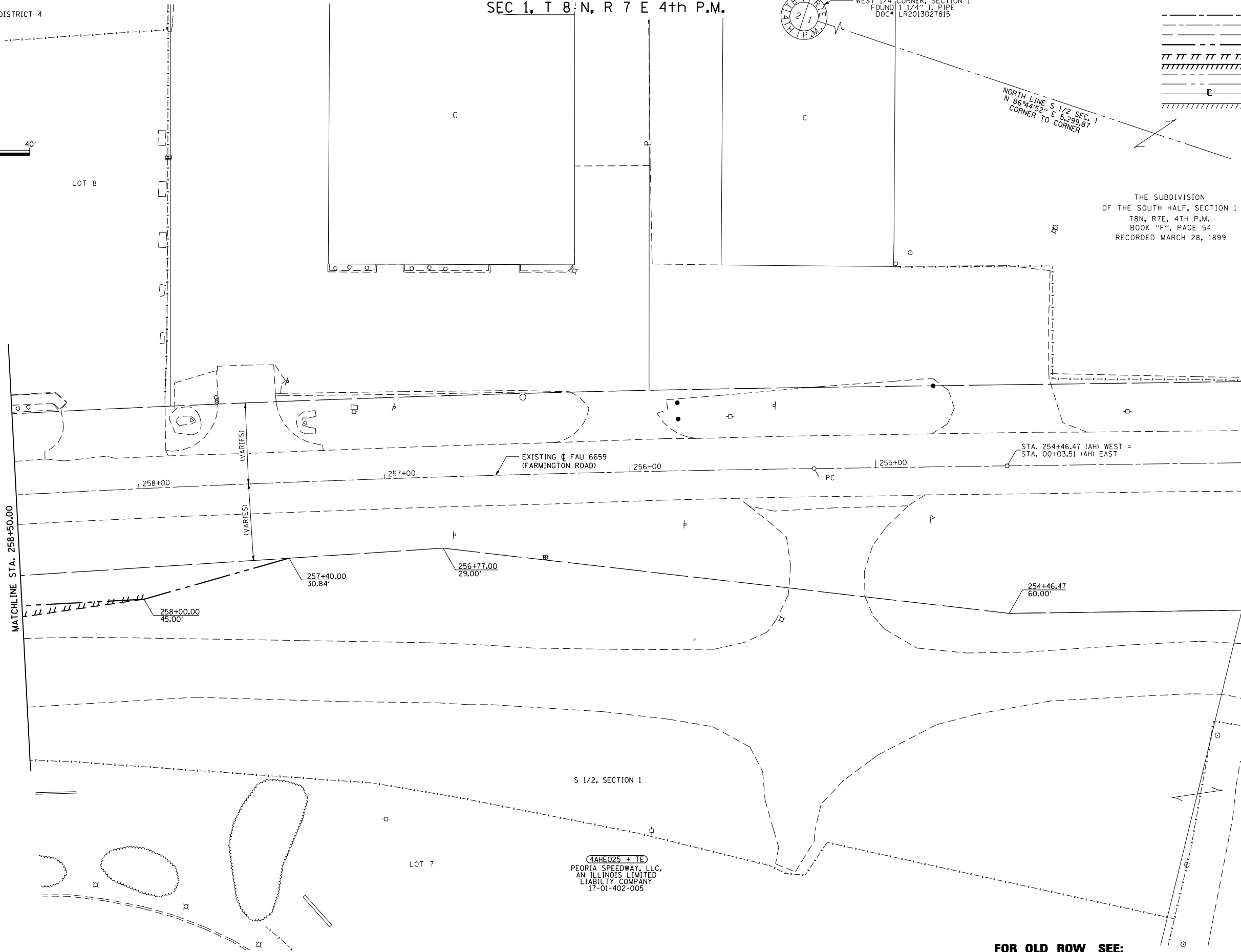
SEC 1, T 8 N, R 7 E 4th P.M.



LEGEND

- PROPOSED CENTERLINE
- EXISTING CENTERLINE
- EXISTING RIGHT OF WAY LINE
- PROPOSED RIGHT OF WAY LINE
- ||||| PROPOSED TEMPORARY EASEMENT LINE
- ||||| PROPOSED PERMANENT EASEMENT LINE
- SECTION LINE
- QUARTER SECTION LINE
- PROPERTY LINE
- ||||| EXISTING EASEMENT

THE SUBDIVISION
OF THE SOUTH HALF, SECTION 1
T8N, R7E, 4TH P.M.
BOOK "F", PAGE 54
RECORDED MARCH 28, 1899



EXISTING CURVE DATA
PI STA. = 257+90.22
 $\Delta = 03^{\circ}36'55''$ (LT)
D = $00^{\circ}40'55''$
R = 8,403.54'
T = 265.21'
L = 530.24'
E = 4.18'
PC STA. = 255+25.01
PT STA. = 260+55.25

(4AHE025 + TE)
PEORIA SPEEDWAY, LLC,
AN ILLINOIS LIMITED
LIABILITY COMPANY
17-01-402-005

FOR OLD ROW SEE:
PLANFILE 3 SHEETS 43 & 44

8 77 PLAT FILE 527

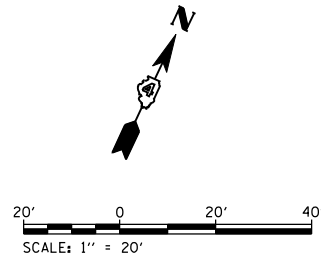
FILE NAME *FILEABBREV*	USER NAME = g_jameson	DESIGNED -	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	RIGHT OF WAY PLANS		F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	PLOT SCALE = 40.0000' / IN.	DRAWN - DAS	REVISED -		PROJECT	JOB NO. R-94-007-13	6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	410
PLOT DATE = 8/14/2018	CHECKED - JWD	DATE - 04/2017	REVISED -	SCALE: 1" = 20'	SHEET NO. 05 OF 9 SHEETS	STA. 258+50.00 TO STA. 257+40.00	CATALOG# 032475-00		CONTRACT NO. 68185		
							FED. ROAD DIST. NO.		ILLINOIS FED. AID PROJECT		

BEARINGS ARE BASED ON CONTROL PROVIDED BY IDOT, DISTRICT 4

SEC 1, T 8 N, R 7 E 4th P.M.

LEGEND

- PROPOSED CENTERLINE
- EXISTING CENTERLINE
- EXISTING RIGHT OF WAY LINE
- PROPOSED RIGHT OF WAY LINE
- ||||| PROPOSED TEMPORARY EASEMENT LINE
- ||||| PROPOSED PERMANENT EASEMENT LINE
- SECTION LINE
- QUARTER SECTION LINE
- PROPERTY LINE
- ||||| EXISTING EASEMENT



NIRVANA GARDENS A SUBDIVISION OF LOTS 1, 2 AND 6 IN NIRVANA SUBDIVISION AND TRACT C BOOK 13, PAGE 80 RECORDED AUG 15, 2015

4AHE007 HINDU TEMPLE OF CENTRAL ILLINOIS, AN ILLINOIS NOT-FOR-PROFIT CORPORATION 17-01-351-015

4AHE008E GREGORY S. DAVIS 17-01-376-008

4AHE009TE MICHAEL O. & LINDA F. EMERY 17-01-376-029

GLEN VIEW TERRACE, A SUBDIVISION OF PART OF THE SOUTHWEST QUARTER OF SECTION 1 T8N, R7E, 4TH P.M. BOOK "U", PAGE 84 DOC# 500800 RECORDED SEPTEMBER 5, 1952

4AHE006 THOMAS & MARGARET ELMORE 17-01-376-007

THE SUBDIVISION OF THE SOUTH HALF, SECTION 1 T8N, R7E, 4TH P.M. RECORDED MARCH 28, 1899 BOOK "F", PAGE 54

PROPOSED CURVE DATA

PI STA. = 94+37.05
 $\Delta = 32^{\circ}04'20''$ (LT)
 D = 0891.06'
 R = 700.00'
 T = 201.20'
 L = 391.83'
 E = 28.34'
 PC STA. = 92+35.85
 PT STA. = 96+27.69

EXISTING CURVE DATA

PI STA. = 88+87.75	PI STA. = 94+45.67
$\Delta = 15^{\circ}01'28''$ (RT)	$\Delta = 18^{\circ}21'36''$ (LT)
D = 0396.14'	D = 0424.47'
R = 1,519.61'	R = 1,298.34'
T = 200.39'	T = 209.82'
L = 398.48'	L = 416.04'
E = 13.16'	E = 16.84'
PC STA. = 86+87.36	PC STA. = 92+35.85
PT STA. = 90+85.84	PT STA. = 96+51.89

FOR OLD ROW SEE:
 PLANFILE 3 SHEETS 43 & 44
 PLANFILE 35 SHEET 10

8 78

PLAT FILE 527

FILE NAME	USER NAME = g_jameson	DESIGNED -	REVISED -
#FILEABBREV#		DRAWN - DAS	REVISED -
	PLOT SCALE = 40.0000' / IN.	CHECKED - JWD	REVISED -
	PLOT DATE = 8/14/2018	DATE - 04/2017	REVISED -

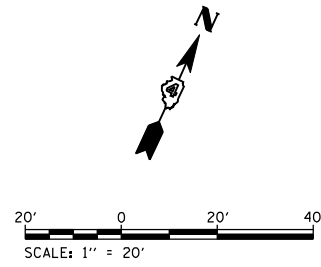
STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

RIGHT OF WAY PLANS

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N,BR-1,RS-4,W-1)	PEORIA	577	411
PROJECT		JOB NO. R-94-007-13		
SHEET NO. 6 OF 9 SHEETS		STA. 91+60.00 TO STA. 94+00.00		
SCALE: 1" = 20'		CATALOG# 032475-00 CONTRACT NO. 68185		
		FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT		

BEARINGS ARE BASED ON CONTROL PROVIDED BY IDOT, DISTRICT 4

SEC 1, T 8 N, R 7 E 4th P.M.



LEGEND

	PROPOSED CENTERLINE
	EXISTING CENTERLINE
	EXISTING RIGHT OF WAY LINE
	PROPOSED RIGHT OF WAY LINE
	PROPOSED TEMPORARY EASEMENT LINE
	PROPOSED PERMANENT EASEMENT LINE
	SECTION LINE
	QUARTER SECTION LINE
	PROPERTY LINE
	EXISTING EASEMENT

THE SUBDIVISION OF THE SOUTH HALF, SECTION 1 T8N, R7E, 4TH P.M. BOOK "F", PAGE 54 RECORDED MARCH 28, 1899

PROPOSED CURVE DATA

PI STA. = 94+37.05	PI STA. = 97+83.52
$\Delta = 32^{\circ}04'20''$ (LT)	$\Delta = 15^{\circ}29'54''$ (RT)
D = 08'11.06"	D = 13'28'53"
R = 700.00'	R = 425.00'
T = 201.20'	T = 57.83'
L = 391.83'	L = 114.96'
E = 28.34'	E = 3.92'
PC STA. = 92+35.85	PC STA. = 97+25.69
PT STA. = 96+27.69	PT STA. = 98+40.65

EXISTING CURVE DATA

PI STA. = 94+45.67
$\Delta = 18^{\circ}21'36''$ (LT)
D = 04'24.47"
R = 1,298.34'
T = 209.82'
L = 416.04'
E = 16.84'
PC STA. = 92+35.85
PT STA. = 96+51.89



FILE NAME #FILEABBREV#	USER NAME = g_jameson	DESIGNED -	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	RIGHT OF WAY PLANS		F.A.U. RTE. = 6659	SECTION = 11(N, BR-1, RS-4, W-1)	COUNTY = PEORIA	TOTAL SHEETS = 577	SHEET NO. = 412
	PLOT SCALE = 40.0000' / IN.	CHECKED - JWD	REVISED -		PROJECT =	JOB NO. R-94-007-13	CATALOG# 032475-00	CONTRACT NO. 68185	FED. ROAD DIST. NO. =	ILLINOIS FED. AID PROJECT =	
PLOT DATE = 8/14/2018	DATE = 04/2017	REVISED -	REVISED -	SCALE: 1" = 20'	SHEET NO. 7 OF 9 SHEETS	STA. 94+00.00 TO STA. 97+50.00					

SEC 1, T 8 N, R 7 E 4th P.M.

BEARINGS ARE BASED ON CONTROL PROVIDED BY IDOT, DISTRICT 4

LEGEND

- PROPOSED CENTERLINE
- - - - - EXISTING CENTERLINE
- ===== EXISTING RIGHT OF WAY LINE
- PROPOSED RIGHT OF WAY LINE
- ||||| PROPOSED TEMPORARY EASEMENT LINE
- /////// PROPOSED PERMANENT EASEMENT LINE
- SECTION LINE
- QUARTER SECTION LINE
- PROPERTY LINE
- /////// EXISTING EASEMENT

THE SUBDIVISION OF THE SOUTH HALF, SECTION 1 T8N, R7E, 4TH P.M. BOOK "F", PAGE 54 RECORDED MARCH 28, 1899

LOT 10
S 1/2, SECTION 1

4AHE016
ZACHARY J. & BETH A. LEDBETTER
17-01-301-010

4AHE014
BILLY D. RANDLE & SANDRA M. BARDWELL
17-01-301-012

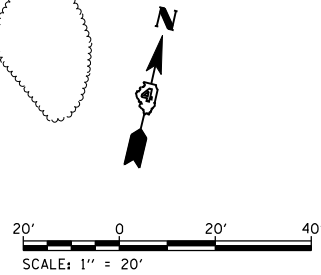
4AHE015
CARY E. DIXON & LORI S. CLEMONS-FOUGHT
17-01-301-013
17-01-301-014

4AHE013
GLENN M. CLOAT
17-01-301-011

4AHE018E
CENTRAL ILLINOIS LIGHT COMPANY, AN ILLINOIS CORPORATION
17-01-326-001

4AHE021 + TE
COUNTY OF PEORIA
17-01-176-002

PROPOSED CURVE DATA		EXISTING CURVE DATA	
PI STA. = 105+36.65	PI STA. = 102+18.69	PI STA. = 104+84.70	
$\Delta = 28^{\circ}15'49''$ (LT)	$\Delta = 11^{\circ}49'48''$ (RT)	$\Delta = 16^{\circ}31'18''$ (LT)	
D = 08^{\circ}11'06''	D = 13^{\circ}28'53''	D = 03^{\circ}32'44''	
R = 700.00'	R = 425.00'	R = 1,615.99'	
T = 176.24'	T = 44.03'	T = 234.62'	
L = 345.31'	L = 87.75'	L = 465.98'	
E = 21.85'	E = 2.27'	E = 16.94'	
PC STA. = 103+60.41	PC STA. = 101+74.66	PC STA. = 102+50.08	
PT STA. = 107+05.72	PT STA. = 102+62.41	PT STA. = 107+16.06	



MATCHLINE STA. 104+50.00

MATCHLINE STA. 101+00.00

PROPOSED KICKAPOO CREEK ROAD

EXISTING KICKAPOO CREEK ROAD

THREAD OF KICKAPOO CREEK AS OF 12/2/13

KICKAPOO CREEK

LOT 9

LOT 8

FILE NAME	USER NAME = gjameson	DESIGNED -	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	RIGHT OF WAY PLANS		F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
*FILE ABBREVS		DRAWN - DAS	REVISED -		PROJECT	JOB NO. R-94-007-13	6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	413
		CHECKED - JWD	REVISED -		SCALE: 1" = 20'	SHEET NO. 8 OF 9 SHEETS	CATALOG# 032475-00		CONTRACT NO. 68185		
		DATE - 04/2017	REVISED -			STA. 101+00.00 TO STA. 104+50.00	ILLINOIS FED. AID PROJECT				

8 80

PLAT FILE 527

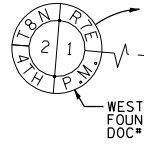
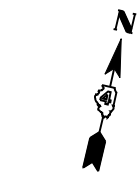
BEARINGS ARE BASED ON CONTROL PROVIDED BY IDOT, DISTRICT 4

SEC 1, T 8 N, R 7 E 4th P.M.

J.C. FLANAGAN'S SUBDIVISION OF THE NORTH HALF OF SECTION 1 T8N, R7E, 4TH P.M. BOOK "B", PAGE 149

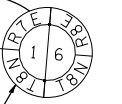
LEGEND

- PROPOSED CENTERLINE
- - - EXISTING CENTERLINE
- EXISTING RIGHT OF WAY LINE
- PROPOSED RIGHT OF WAY LINE
- ||||| PROPOSED TEMPORARY EASEMENT LINE
- ||||| PROPOSED PERMANENT EASEMENT LINE
- SECTION LINE
- QUARTER SECTION LINE
- PROPERTY LINE
- ||||| EXISTING EASEMENT



WEST 1/4 CORNER, SECTION 1 FOUND 1 1/4" I. PIPE DOC# LR2013027815

SCALE: 1" = 20'



EAST 1/4 CORNER, SECTION 1 SET MAG NAIL DOC# LR2013027815

THE SUBDIVISION OF THE SOUTH HALF, SECTION 1 T8N, R7E, 4TH P.M. BOOK "F", PAGE 54 RECORDED MARCH 28, 1899

PROPOSED CURVE DATA
 PI STA. = 105+36.65
 Δ = 28°15'49" (LT)
 D = 08°11'06"
 R = 700.00'
 T = 176.24'
 L = 345.31'
 E = 21.85'
 PC STA. = 103+60.41
 PT STA. = 107+05.72

EXISTING CURVE DATA
 PI STA. = 104+84.70
 Δ = 16°31'18" (LT)
 D = 03°32'44"
 R = 1,615.99'
 T = 234.62'
 L = 465.98'
 E = 16.94'
 PC STA. = 102+50.08
 PT STA. = 107+16.06

106+71.82
423.75'

4AHE016
ZACHARY J. & BETH A. LEDBETTER
17-01-301-010

MATCHLINE STA. 104+50.00

4AHE018-PE + TE
CENTRAL ILLINOIS LIGHT COMPANY, AN ILLINOIS CORPORATION
17-01-326-001

4AHE021 + TE
COUNTY OF PEORIA
17-01-176-002

FILE NAME #FILEABBREV#	USER NAME = g_jameson	DESIGNED -	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	RIGHT OF WAY PLANS		F.A.U. RTE. = 6659	SECTION = 11(N, BR-1, RS-4, W-1)	COUNTY = PEORIA	TOTAL SHEETS = 577	SHEET NO. = 414
	PLOT SCALE = 40.0000' / IN.	DRAWN - DAS	REVISED -		PROJECT = SHEET NO. 9 OF 9 SHEETS	JOB NO. = R-94-007-13	CATALOG# = 032475-00	CONTRACT NO. = 68185	FED. ROAD DIST. NO. =	ILLINOIS FED. AID PROJECT =	
PLOT DATE = 8/14/2018	CHECKED - JWD	DATE = 04/2017	REVISED -	SCALE: 1" = 20'	STA. 107+54.72 TO STA. 104+50.00						

LAST SAVED DATE: 8/16/2018

Benchmark: Chiseled "□" North end of East abutment of S.N. 072-0063, Elev. 478.42.

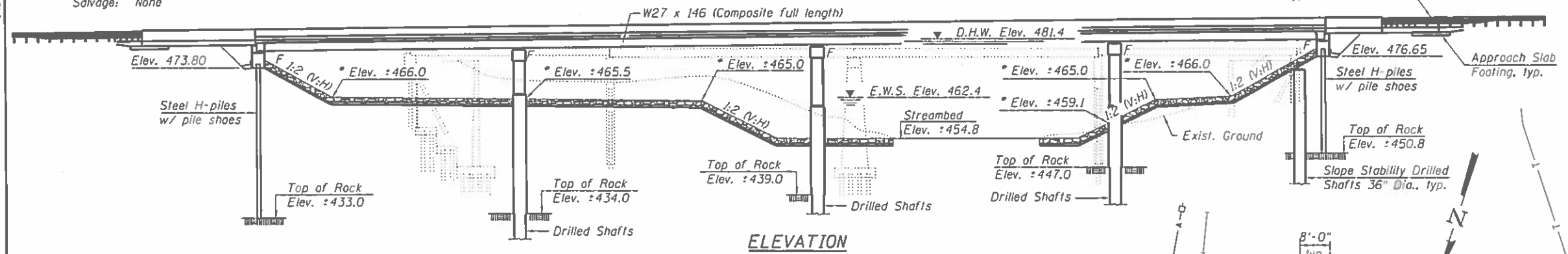
Existing Structure: S.N. 072-0063. Originally built in 1922 as S.B.I. Route 8, Section IIB as a two-span through truss structure. In 1972, the bridge was widened and reconstructed as S.B.I. Route 8, Section IIBR. The existing trusses were removed, the existing closed abutments were abandoned and new stub abutments on steel H-piles were added, two new pile bent piers on H-piles were added and the existing pier was widened. The resulting four-span superstructure consists of 11 - 21' & 27' deep PPC Deck Beams, 238'-0" Bk. to Bk. abutments, 33'-0" out to out. Structure to be removed and replaced. Traffic to be maintained utilizing stage construction.

Salvage: None

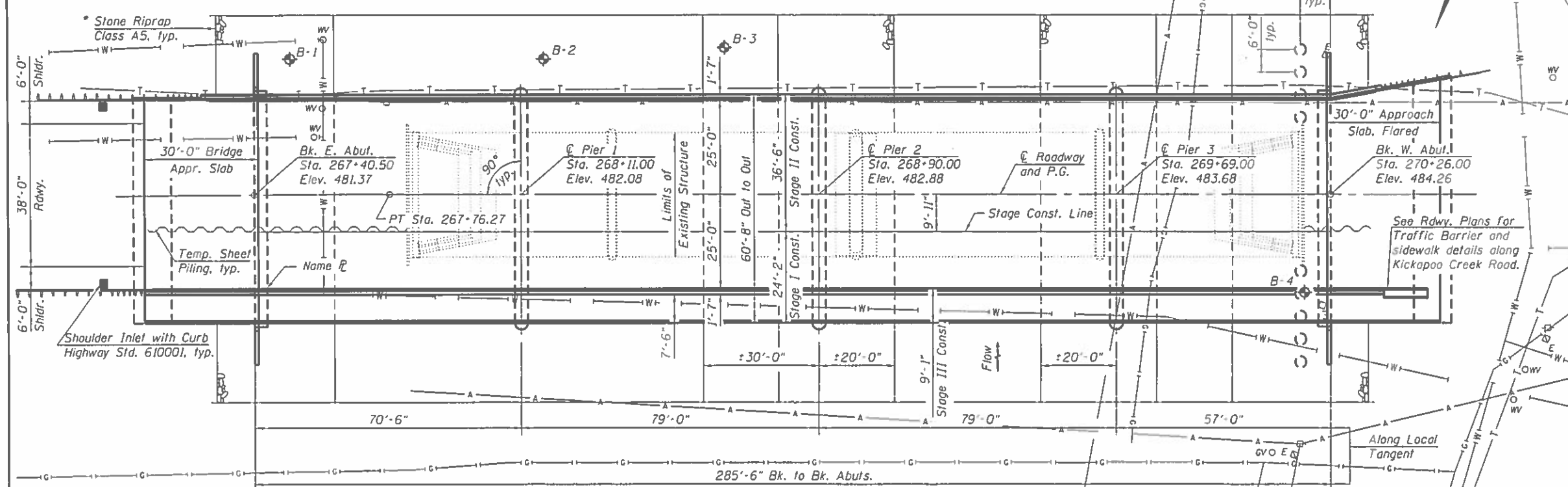
Note:

- 1. All utilities shown to be relocated by others. See Roadway Plans.
- For the Grading Plan, Riprap Layout, and Quantities not shown on sheet 2, see Roadway Plans.

Traffic Barrier Terminal, Type 6
Std. 631031, typ. 3 corners



ELEVATION



PLAN

DESIGN SCOUR ELEVATION TABLE

E. Abut.	Pier 1	Pier 2	Pier 3	W. Abut.
473.80	450.50	439.80	446.30	476.65

WATERWAY INFORMATION TABLE

Flood	Freq. Yr.	C.F.S.	Opening Sq. Ft.		Nat. Head - Ft.		Headwater El.		
			Exist.	Prop.	H.W.E.	Exist.	Prop.	Exist.	Prop.
Overtopping	<5						475.30	475.30	
Overtopping	10	17,560	2,577	3,550	477.00	0.40	0.20	477.40	477.20
Design	50	29,360	2,577	4,020	481.40	0.20	0.10	481.60	481.50
Base	100	35,600	2,577	4,020	483.20	0.10	0.10	483.30	483.30
Max. Calc.	500	51,560	2,577	4,020	485.40	0.10	0.10	485.50	485.50

STATION 268+90.00
BUILT 20 BY
STATE OF ILLINOIS
F.A.U. RT. 6659
SEC. 11KN, BR-1, RS-4, W-1)
LOADING HL-93
STRUCTURE NO. 072-0245

NAME PLATE
See Std. 515001

- LEGEND**
- Designates Soil Boring Location
 - Aerial Power Line
 - Gas Pipe
 - Telephone Cable
 - Water Pipe
 - Power Pole
 - Water Meter Valve Box
 - Gas Meter Valve Box
 - Electrical Junction Box

LOADING HL-93
Allow 50#/sq. ft. for future wearing surface.

DESIGN SPECIFICATIONS
2012 AASHTO LRFD Bridge
Design Specifications, 6th Edition

DESIGN STRESSES

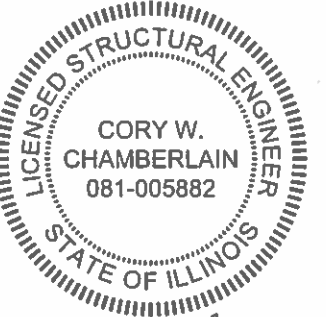
- FIELD UNITS**
- f'c = 5,000 psi (Superstructure Concrete)
 - f'c = 3,500 psi (Substructure Concrete)
 - f'c = 6,000 psi (Precast)
 - fy = 60,000 psi (Reinforcement)
 - fy = 50,000 psi (M270 Grade 50)

SEISMIC DATA

Seismic Performance Zone (SPZ) = 1
Design Spectral Acceleration at 1.0 sec. (S_{D1}) = 0.079g
Design Spectral Acceleration at 0.2 sec. (S_{D5}) = 0.131g
Soil Site Class C

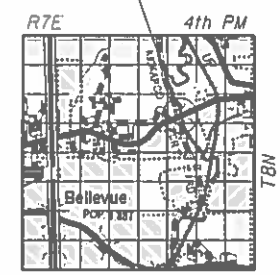
APPROVED
For Structural Adequacy Only

Cory W. Chamberlain
Engineer of Bridges & Structures



Cory W. Chamberlain 8-13-2018
Expires: 11/30/2018
Applies to sheets 1-39 and 41-52.

PROJECT LOCATION



LOCATION SKETCH

GENERAL PLAN AND ELEVATION
FARMINGTON ROAD OVER KICKAPOO CREEK
FAU 6659 - SECTION 11KN, BR-1, RS-4, W-1)
PEORIA COUNTY
STATION 268+90.00
STRUCTURE NO. 072-0245

FILE NAME: L:\pds\100\CADD\CADD_Sheets\0720245-68185.dgn



USER NAME = dhaberling	DESIGNED = TJZ/SBC	REVISED
FILE NAME = 0720245-68185.dgn	CHECKED = CWC	REVISED
PLOT SCALE = @2" = 1' / in.	DRAWN = DLH	REVISED
PLOT DATE = 8/16/2018	CHECKED = SBC	REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SHEET NO. 1 OF 52 SHEETS

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11KN, BR-1, RS-4, W-1)	PEORIA	577	415
ILLINOIS FED. AID PROJECT			CONTRACT NO. 68185	

GENERAL NOTES

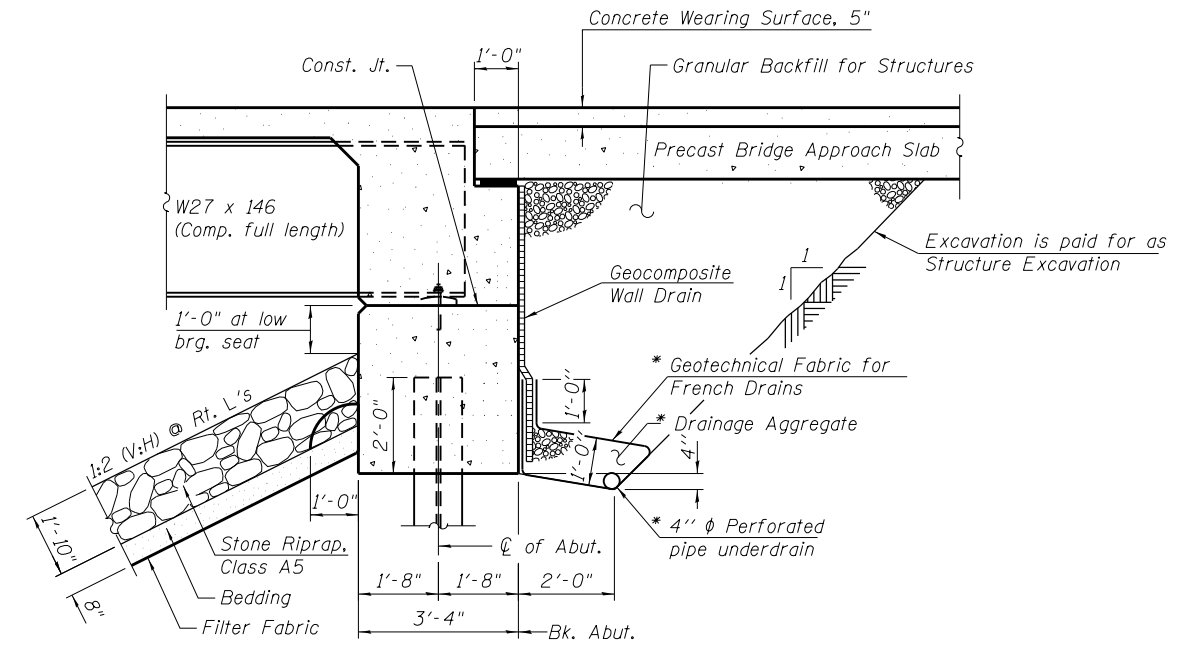
- Fasteners shall be ASTM A325 Type 1, high strength galvanized bolts. Bolts $\frac{7}{8}$ " ϕ , holes $\frac{5}{16}$ " ϕ , unless otherwise noted. See Special Provisions for "Hot Dipped Galvanizing for Structural Steel".
- Calculated weight of Structural Steel = 566,630 lbs.
- No field welding is permitted except as specified in the contract documents.
- Reinforcement bars designated (E) shall be epoxy coated.
- Bearing seat surfaces shall be constructed or adjusted to the designated elevations within a tolerance of $\frac{1}{8}$ inch (0.01 ft.). Adjustment shall be made either by grinding the surface or by shimming the bearings.
- Layout of the slope protection system may be varied to suit ground conditions in the field as directed by the Engineer.
- The embankment configuration shown shall be the minimum that must be placed and compacted prior to construction of the abutments.
- All new structural steel shall be galvanized. The fascia and underside of the exterior beams and their associated splice plates shall be painted. The color of the final finish coat of paint shall be Blue, Munsell No. 10B 3/6. See Special Provisions for "Hot Dip Galvanizing for Structural Steel".
- Current ratings on file for existing structure.
 Inventory: HS 15.4
 Operating: HS 27.6
 Live Load Restrictions: Legal Loads Only

 Inventory and Operating Ratings and Live Load Restrictions are provided for information only. Inventory and Operating Ratings are based on HS Loading configuration. Live Load Restrictions are based on Illinois legal loads and configurations. The Ratings and Live Load Restrictions are not necessarily representative of capacities to support the Contractor's equipment.
- Slipforming of the parapets is not allowed.
- The finishing machine rails shall be placed on the top of the flange of the exterior beams within the deck pour. Beam blocks shall be placed between beams at all tie locations in each bay for the full width of the deck pour.

TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Removal of Existing Structures, No. 1	Each		1	1
Structure Excavation	Cu. Yd.		278	278
Concrete Structures	Cu. Yd.		424.1	424.1
Concrete Superstructure	Cu. Yd.	557.7		557.7
Bridge Deck Grooving	Sq. Yd.	2,132		2,132
Protective Coat	Sq. Yd.	2,648		2,648
Furnishing and Erecting Structural Steel	L. Sum	1		1
Furnishing and Erecting Structural Steel	Pound	8,510		8,510
Stud Shear Connectors	Each	11,884		11,884
Reinforcement Bars	Pound		65,790	65,790
Reinforcement Bars, Epoxy Coated	Pound	139,500	110,350	249,850
Bar Splicers	Each	843	1,038	1,881
Mechanical Splicers	Each		252	252
Parapet Railing	Foot	325		325
Furnishing Steel Piles HP12 x 53	Foot		825	825
Driving Piles	Foot		825	825
Test Pile Steel HP12 x 53	Each		2	2
Pile Shoes	Each		24	24
Name Plates	Each	1		1
Permanent Casing	Foot		284	284
Drilled Shaft in Soil	Cu. Yd.		289.5	289.5
Drilled Shaft in Rock	Cu. Yd.		145.7	145.7
Preformed Joint Strip Seal	Foot		119	119
Anchor Bolts, 1"	Each		120	120
Temporary Sheet Piling	Sq. Ft.		1,972	1,972
Geocomposite Wall Drain	Sq. Yd.		113	113
Concrete Wearing Surface, 5"	Sq. Yd.	410.6		410.6
Precast Bridge Approach Slab	Sq. Ft.	3,675		3,675
Pedestrian Rail (Special)	Foot	340		340
Granular Backfill for Structures	Cu. Yd.		201	201
* Asbestos Bearing Pad Removal	Each		44	44
Pipe Underdrains for Structures 4"	Foot		244	244
Steel Railing (Special)	Foot		286	286

** The quantity for Asbestos Bearing Pad Removal has been estimated from the original 1971 plans. The actual quantity may be less due to emergency beam replacement recently completed.



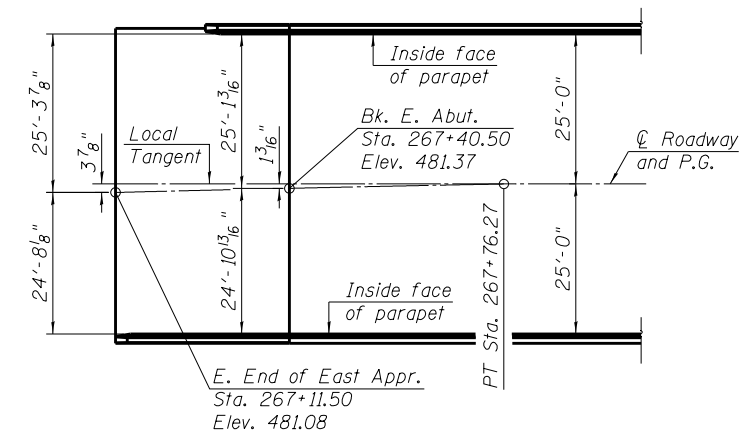
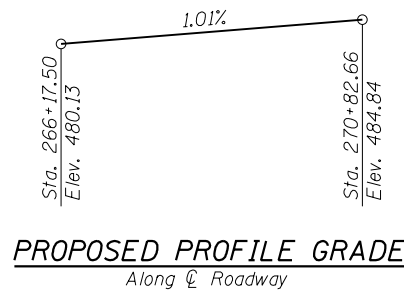
SECTION THRU INTEGRAL ABUTMENT
(Horiz. dim. @ Rt. L's)

* Included in the cost of Pipe Underdrains for Structures. (See Special Provisions)

Note:
All drainage system components shall extend to 2'-0" from the end of each wingwall except an outlet pipe shall extend until intersecting with the side slopes. The pipes shall drain into concrete headwalls. (See Article 601.05 of the Standard Specifications and Highway Standard 601101).

CURVE DATA

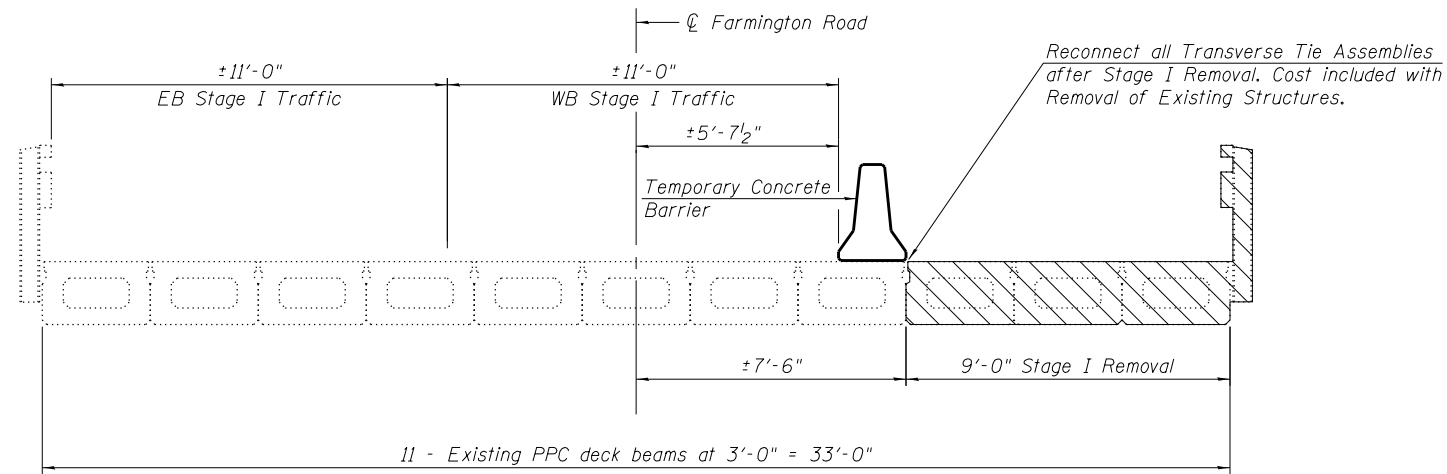
$\Delta = 3^\circ 04' 56''$ (RT)
 $D = 0^\circ 52' 46''$
 $R = 6,515.60'$
 $T = 175.29'$
 $L = 350.50'$
 $E = 2.36'$
 P.C. STA. = 264+25.77
 P.I. STA. = 266+01.06
 P.T. STA. = 267+76.27
 S.E. - None



OFFSET SKETCH

USER NAME = *OPERATOR*	DESIGNED - TJZ	REVISED
	CHECKED - SBC	REVISED
PLOT SCALE = 0.2" = 1' / 1/4"	DRAWN - DLH	REVISED
PLOT DATE = 8/16/2018	CHECKED - SBC	REVISED

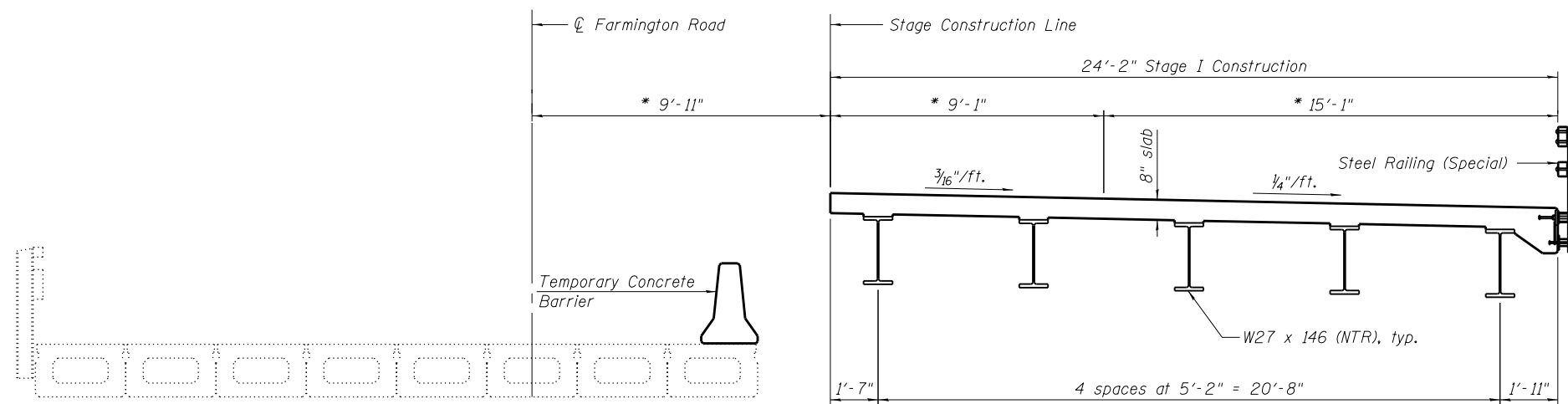
F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	416
CONTRACT NO. 68185				



STAGE I REMOVAL

Notes:

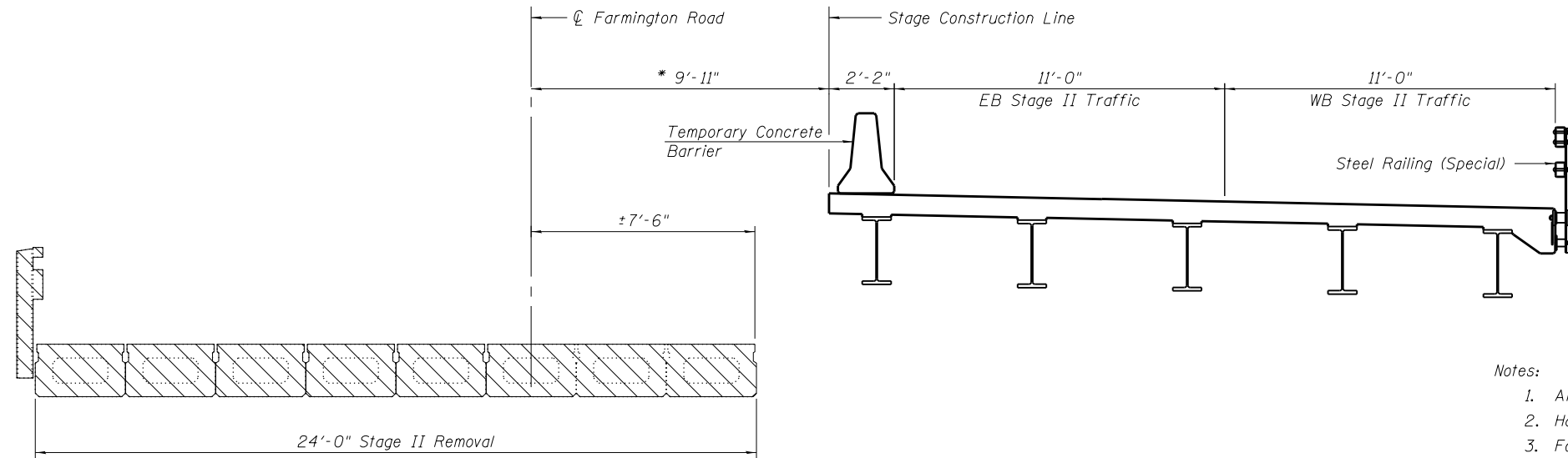
1. All Cross Sections are looking West.
 2. Hatched area indicates Removal of Existing Structures.
 3. For details of Temporary Concrete Barrier, see sheet 7.
 4. See Roadway Plans for Quantities of Temporary Concrete Barrier.
 5. For details of Steel Railing (Special), see sheet 8.
- * Dimension varies from Sta. 267+11.50 to Sta. 267+76.27. See offset sketch on sheet 2.



STAGE I CONSTRUCTION

USER NAME = *OPERATOR*	DESIGNED - TJZ	REVISED
	CHECKED - SBC	REVISED
PLOT SCALE = 0.2" / 1'	DRAWN - DLH	REVISED
PLOT DATE = 8/16/2018	CHECKED - SBC	REVISED

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N,BR-1,RS-4,W-1)	PEORIA	577	417
CONTRACT NO. 68185				

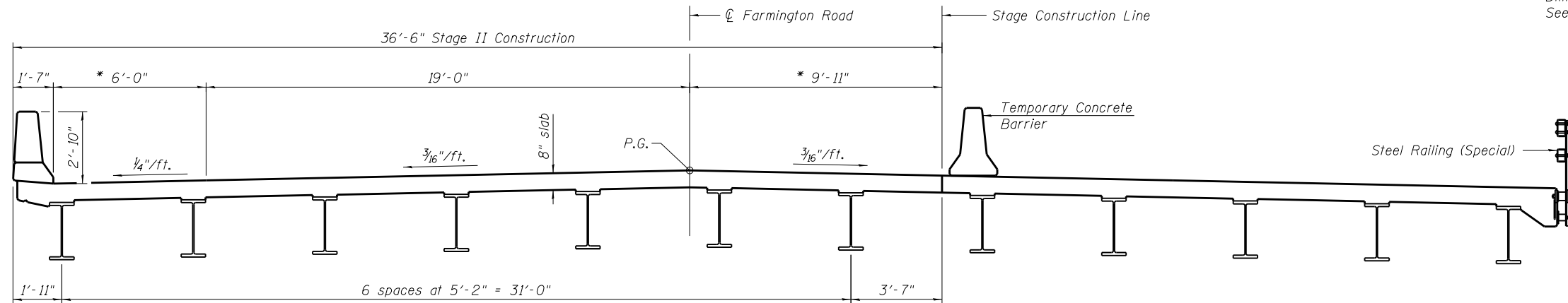


STAGE II REMOVAL

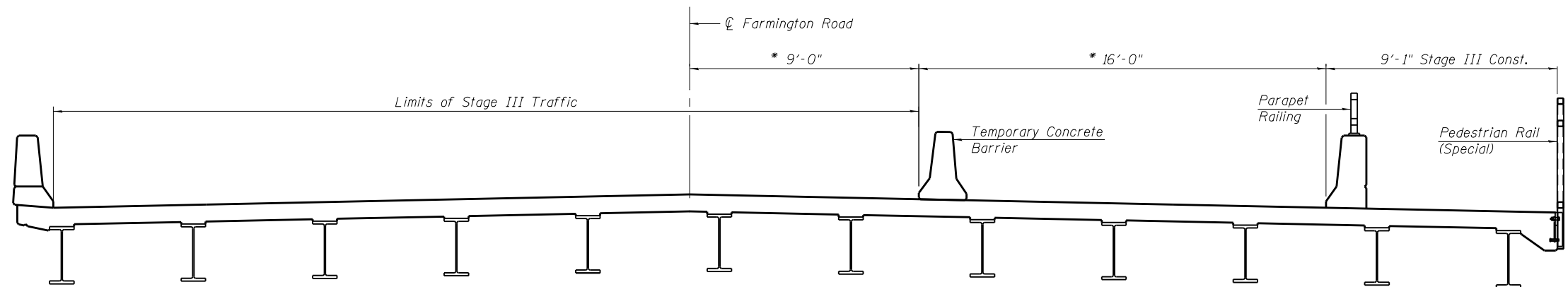
Notes:

1. All Cross Sections are looking West.
2. Hatched area indicates Removal of Existing Structures.
3. For details of Temporary Concrete Barrier, see sheet 7.
4. See Roadway Plans for Quantities of Temporary Concrete Barrier.
5. For details of Steel Railing (Special), see sheet 8.
6. For details of Parapet Railing and Pedestrian Rail (Special), see sheets 27 and 28.

* Dimension varies from Sta. 267+11.50 to Sta. 267+76.27. See offset sketch on sheet 2.



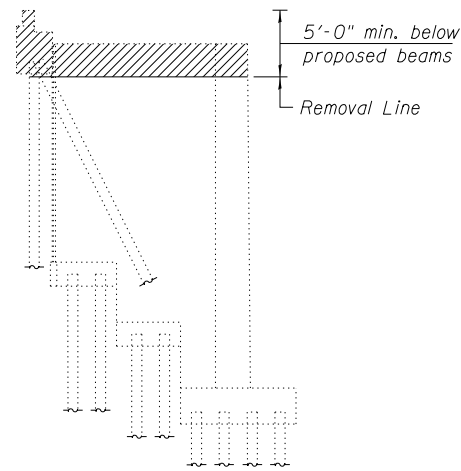
STAGE II CONSTRUCTION



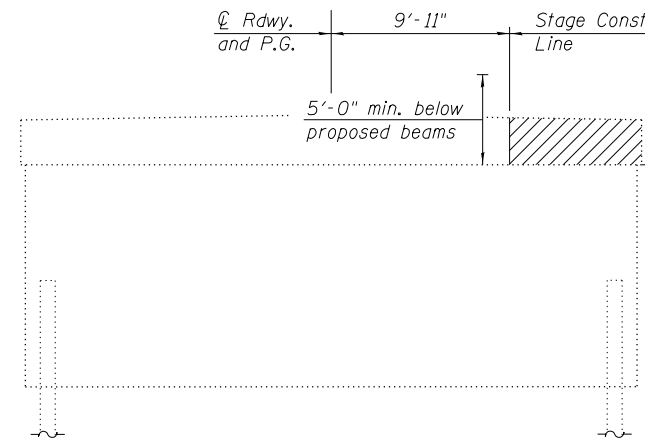
STAGE III CONSTRUCTION

USER NAME = *OPERATOR*	DESIGNED - TJZ	REVISED
	CHECKED - SBC	REVISED
PLOT SCALE = 0.2' / 1" =	DRAWN - DLH	REVISED
PLOT DATE = 8/16/2018	CHECKED - SBC	REVISED

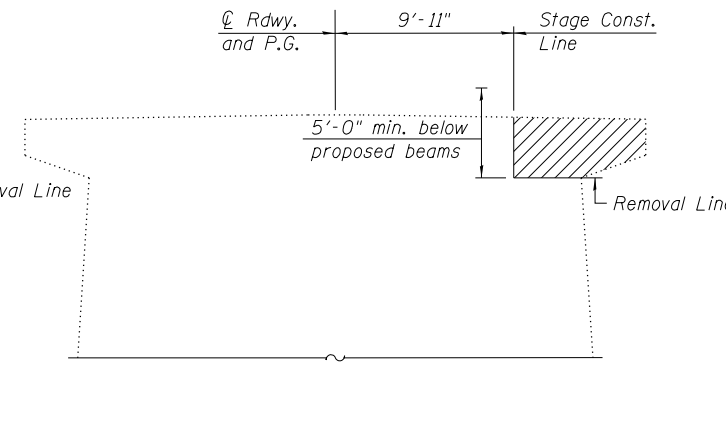
F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N,BR-1,RS-4,W-1)	PEORIA	577	418
CONTRACT NO. 68185				



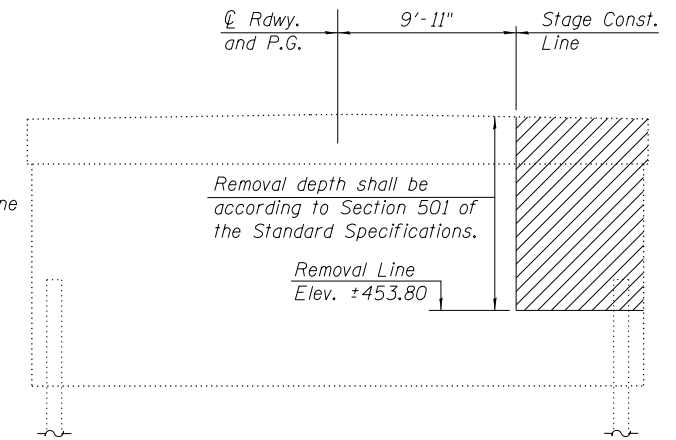
EAST ABUTMENT STAGE I REMOVAL ELEVATION
(Looking South)



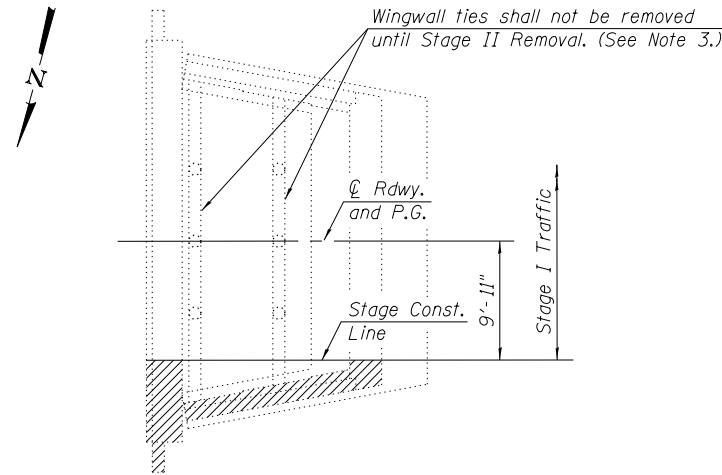
PIER 1 STAGE I REMOVAL
(Looking West)



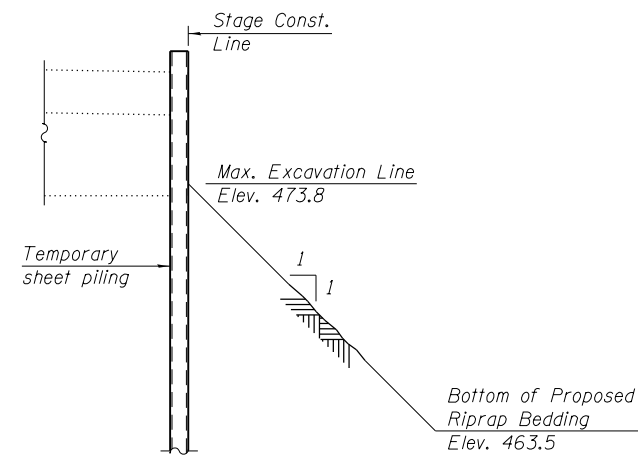
PIER 2 STAGE I REMOVAL
(Looking West)



PIER 3 STAGE I REMOVAL
(Looking West)



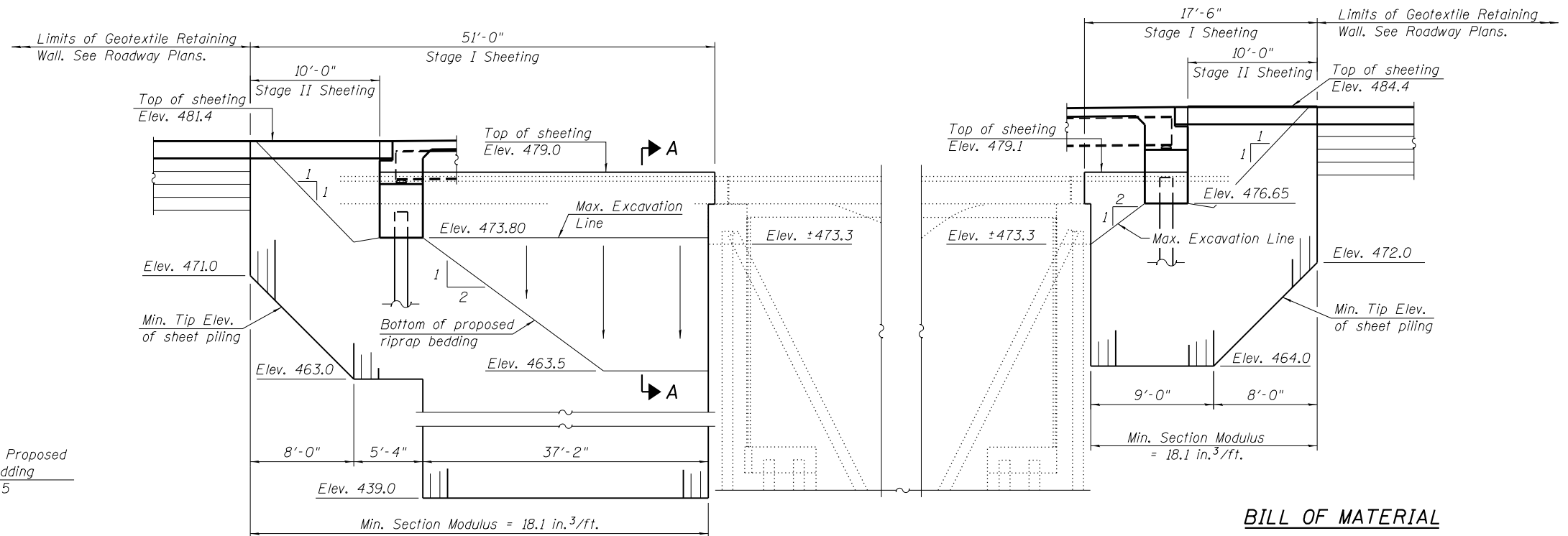
EAST ABUTMENT STAGE I REMOVAL PLAN



SECTION A-A
(Showing Stage I Grading at East Abutment for clarity.)

Notes:

1. Hatched area indicates concrete removal. Cost included in Removal of Existing Structures. All remaining portions of the existing substructure to be removed during Stage II removal operations.
2. If the Contractor chooses to alter the temporary cantilevered sheet piling design requirements shown on the plans, a design submittal including plan details and calculations will be required for review and acceptance by the Engineer.
3. The existing reinforced concrete wingwall ties shall not be removed during Stage I Removal operations. If the existing wingwall ties are encountered within the designated removal limits shown, the Contractor shall terminate removal operations at the top of the existing wingwall ties.
4. Due to the close proximity of the existing piers to proposed piers 2 and 3, the Contractor shall take care not to damage or compromise the proposed piers during Stage II removal operations.

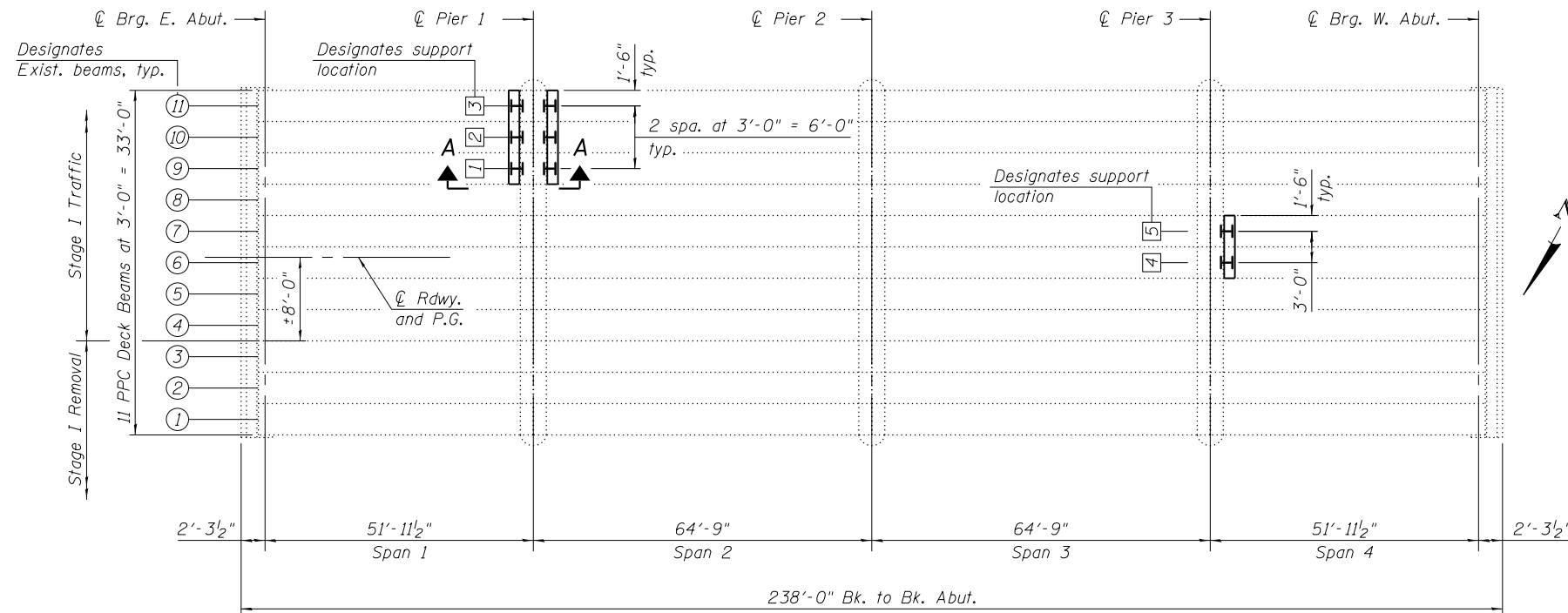


TEMPORARY SHEET PILING
(Looking South)

BILL OF MATERIAL

Item	Unit	Total
Temporary Sheet Piling	Sq. Ft.	1,972

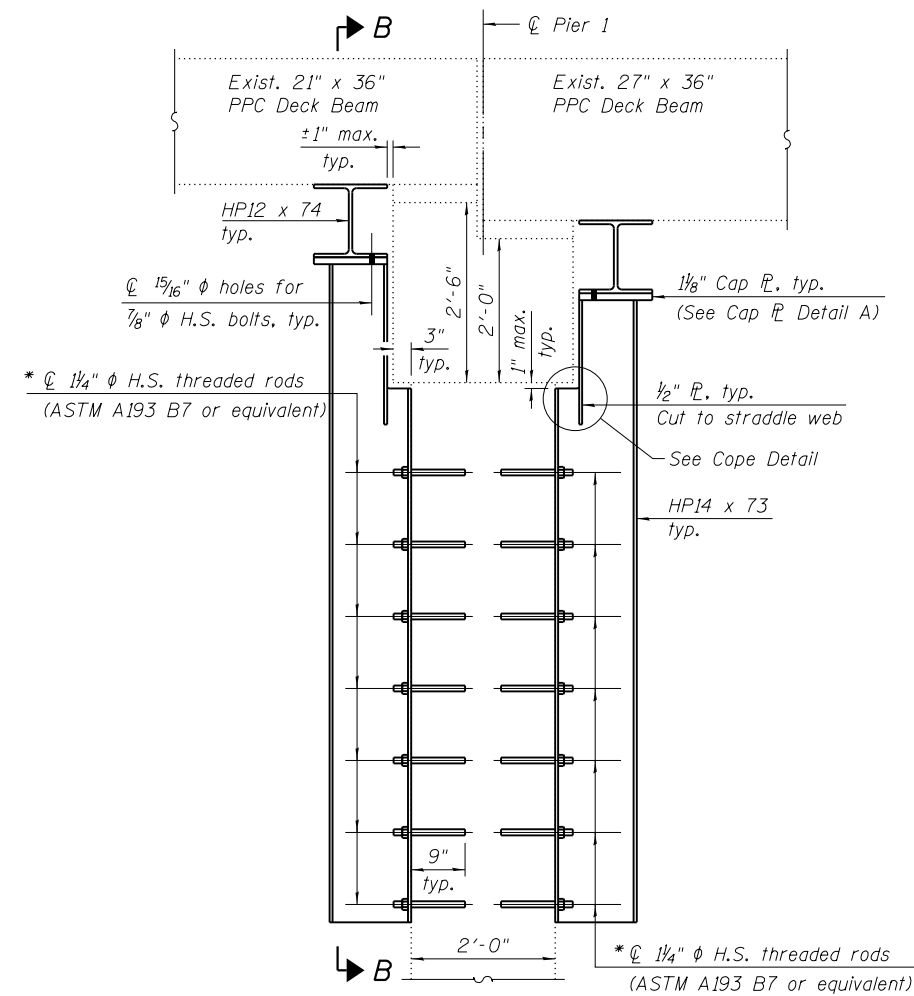
LAST SAVED DATE: 8/15/2018



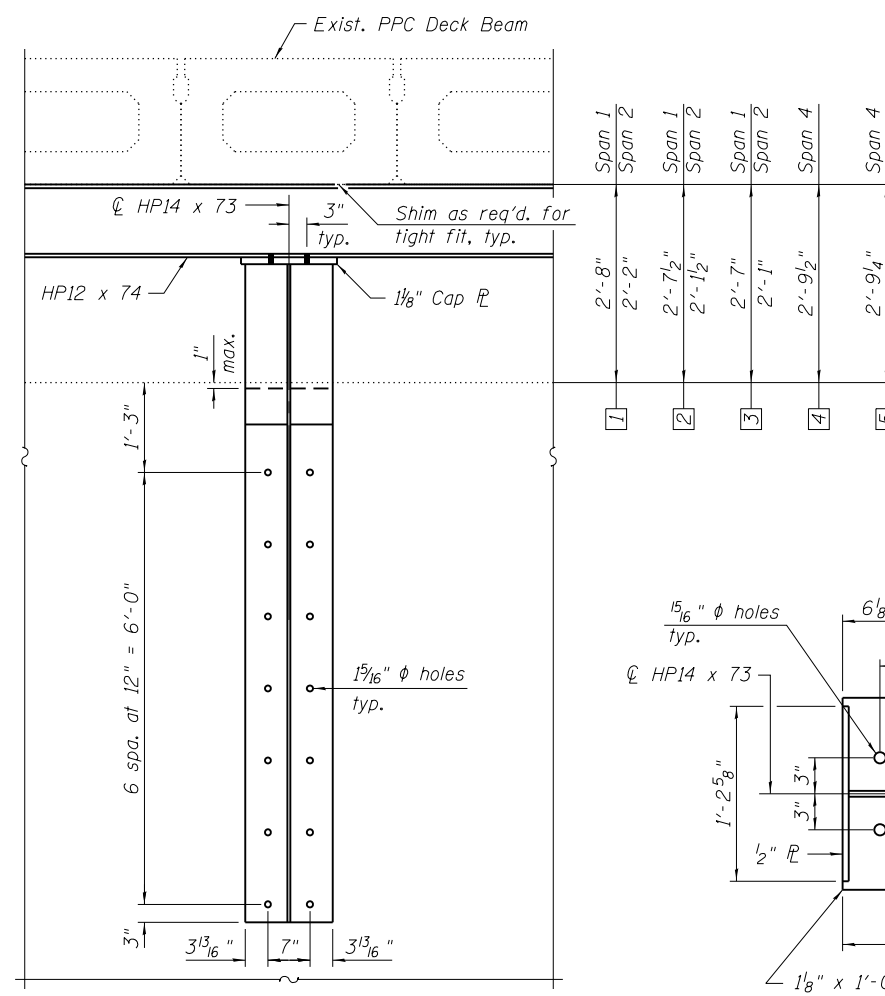
Notes:

1. All structural steel shall conform to AASHTO Classification M270 Gr. 50, unless otherwise noted.
2. Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.
3. The contractor is advised that the existing PPC Deck Beams are in a deteriorated condition with reduced load carrying capacity. It is the contractors responsibility to account for the condition of the beams when developing construction procedures.
4. If the contractor's procedure for placement of structural steel supports involves placement of cranes or other heavy equipment on the bridge, a detailed procedure shall be submitted to the Engineer for approval. The procedure shall include calculations, prepared and sealed by an Illinois Licensed Structural Engineer, verifying that the equipment and procedure used will not overstress the existing beams. To distribute load to multiple beams and protect the existing surface, in all cases a double layer mat of heavy timbers shall be used at all times under crane tracks or wheels and any outriggers in the down position. If necessary, shims shall be used under the crane mat to ensure uniform contact with the underlying beams.
5. Any dewatering required for the installation of the support columns shall be included in the cost for Furnishing and Erecting Structural Steel, Pounds.
6. Areas with low clearance may require additional excavation or added measures to properly install the temporary support columns at Pier 1. The Contractor shall field measure and determine a procedure for placing the columns. The procedure shall be submitted to the Engineer for approval. Cost shall be considered as included with Contract unit price for Furnishing and Erecting Structural Steel, Pounds.

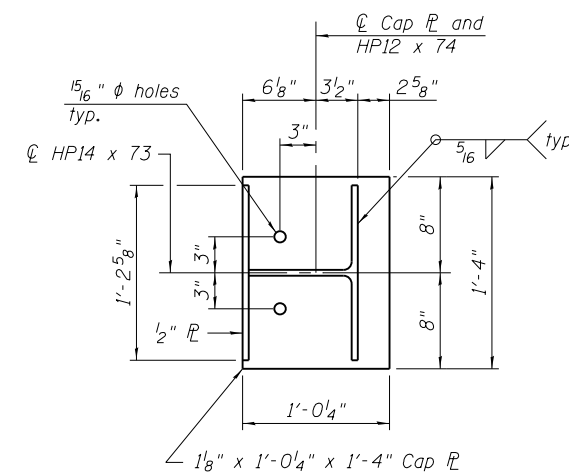
PLAN



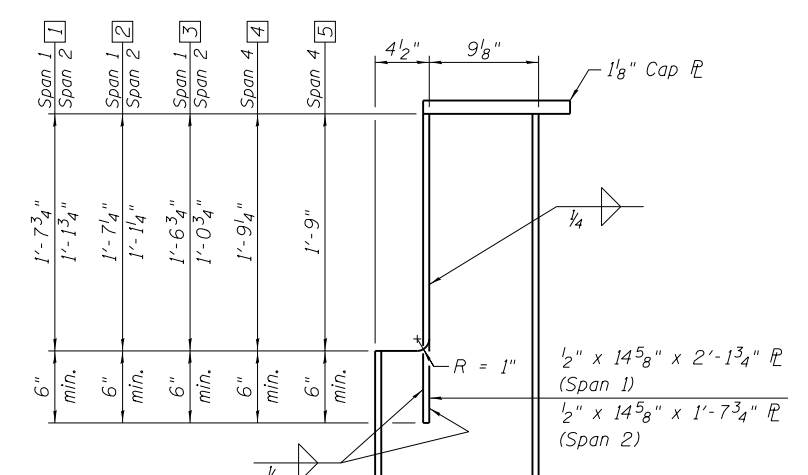
SECTION A-A



SECTION B-B



CAP PLATE DETAIL A



COPE DETAIL

* Drill and grout 1/4" ϕ threaded rods in 1/2" ϕ holes in concrete pier according to Article 584 of the Standard Specifications. Cost included with Furnishing and Erecting Structural Steel, Pounds.

BILL OF MATERIAL

ITEM	UNIT	TOTAL
Furnishing and Erecting Structural Steel	Pound	8,510

design firm no. 184001036



USER NAME = *OPERATOR*	DESIGNED - SBC	REVISED
PLOT SCALE = 0x2" = 1'-0"	CHECKED - TJZ	REVISED
PLOT DATE = 8/16/2018	DRAWN - DLH	REVISED
	CHECKED - SBC	REVISED

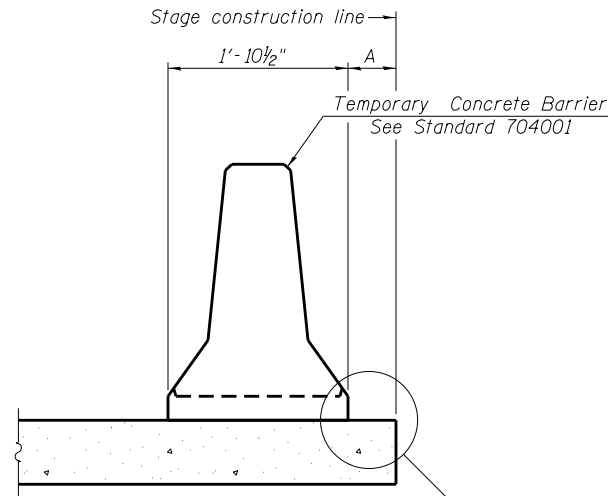
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TEMPORARY BEAM SUPPORT DETAILS
STRUCTURE NO. 072-0245

SHEET NO. 6 OF 52 SHEETS

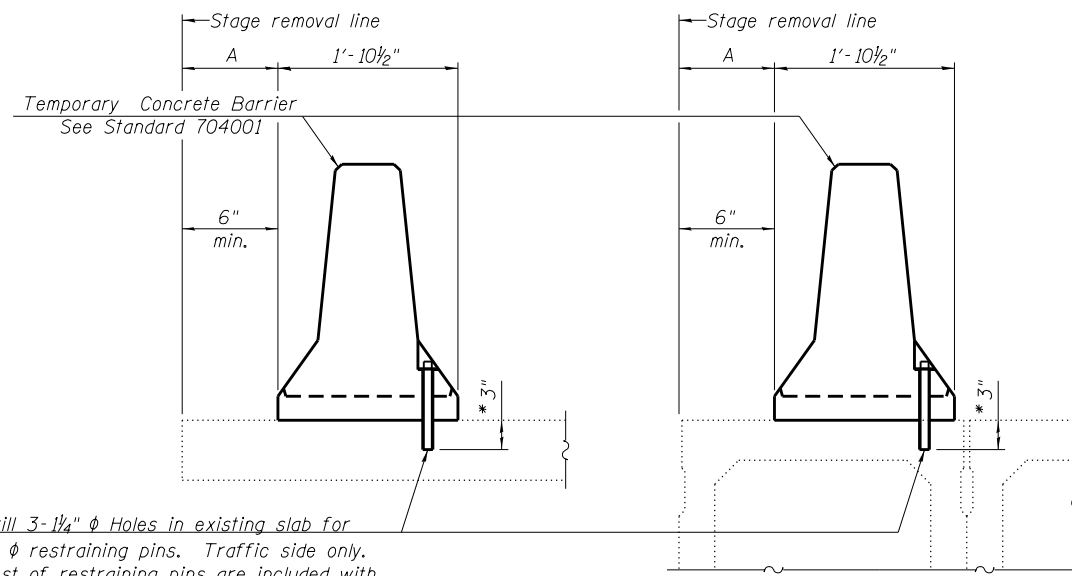
F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N,BR-1,RS-4,W-1)	PEORIA	577	420
CONTRACT NO. 68185				

ILLINOIS FED. AID PROJECT



When "A" is 3'-1" or less, the temporary concrete barrier shall be restrained to the new slab according to Detail I, II or III. No restraint is required when "A" is greater than 3'-1".

NEW SLAB OR NEW DECK BEAM

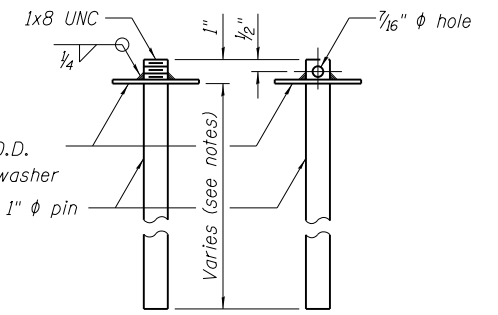


Drill 3-1/4" ϕ Holes in existing slab for 1" ϕ restraining pins. Traffic side only. Cost of restraining pins are included with Temporary Concrete Barrier. No restraint is required when "A" is greater than 3'-1".

EXISTING SLAB

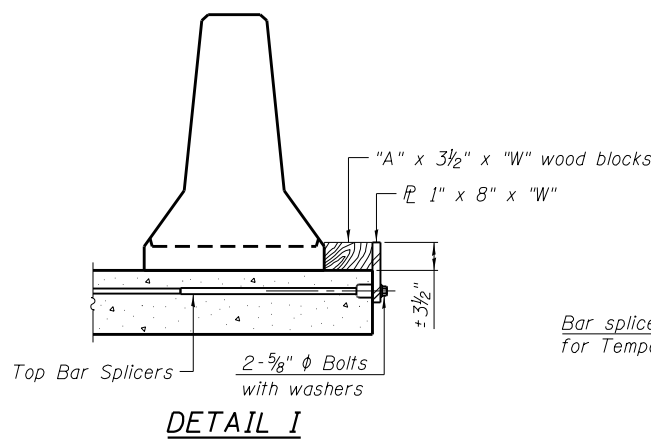
EXISTING DECK BEAM

SECTIONS THRU SLAB OR DECK BEAM

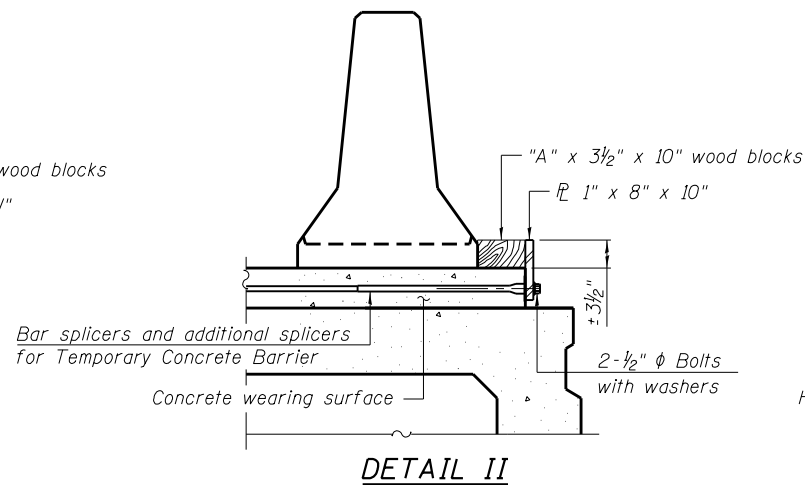


US Std. 1 1/16" I.D. x 2 1/2" O.D. x approx. 8 gauge thick washer

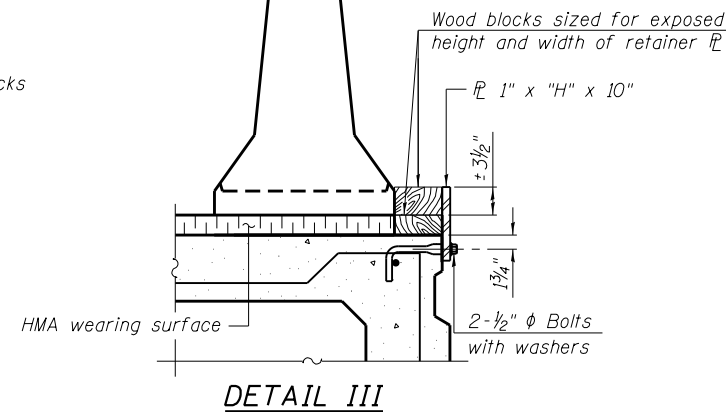
RESTRAINING PIN



DETAIL I



DETAIL II

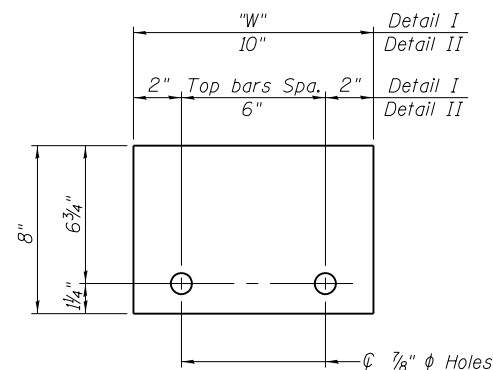


DETAIL III

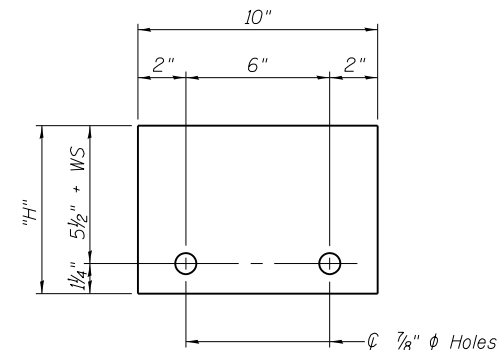
BAR SPLICER FOR #4 BAR - DETAIL III

Notes:

1. Cost of retainer assembly is included with Temporary Concrete Barrier.
2. A retainer assembly shall be located at the approximate ϕ of each temporary concrete barrier.
3. The retainer plate shall not be removed until the concrete on the adjacent stage is ready to be poured. For Detail III applications the retainer plate shall not be removed until just prior to placing the adjacent beam.
4. When omitted and the barrier shall be placed in direct contact with the steel retainer plate. For deck beam applications the minimum required 'A' distance is 6" to accommodate the shear key clamping device.
5. For pay item Temporary Concrete Barrier, see Roadway Plans.



STEEL RETAINER 1" x 8" x "W"
(Detail I and II)



STEEL RETAINER 1" x "H" x 10"
(Detail III)

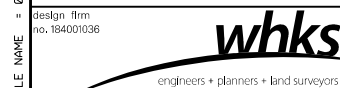
Detail I - Installation for a new bridge deck or bridge slab.

Detail II - Installation for a new deck beam with an initial concrete wearing surface. Additional bar splicers shall be provided at 6'-0" centers and paired with the bar splicers of the concrete wearing surface reinforcement to accommodate the installation of the retainer assemblies. The cost of the additional bar splicers is included with the concrete wearing surface.

Detail III - Installation for a new deck beam with no initial wearing surface or with an initial hot-mix asphalt (HMA) wearing surface present. The deck beam directly beneath the temporary concrete barrier shall be fabricated with bar splicer inserts in the side of the beam, as detailed, to accommodate the installation of the retainer assemblies. A pair of bar splicers, 6" apart, shall be placed at 6'-0" centers along the length of the beam. The cost of the bar splicers is included with the deck beam.

R-27

2-17-2017



USER NAME = *OPERATOR*	DESIGNED - TJZ	REVISED
PLOT SCALE = 0:2' = 1" / 16"	CHECKED - SBC	REVISED
PLOT DATE = 8/16/2018	DRAWN - DLH	REVISED
	CHECKED - SBC	REVISED

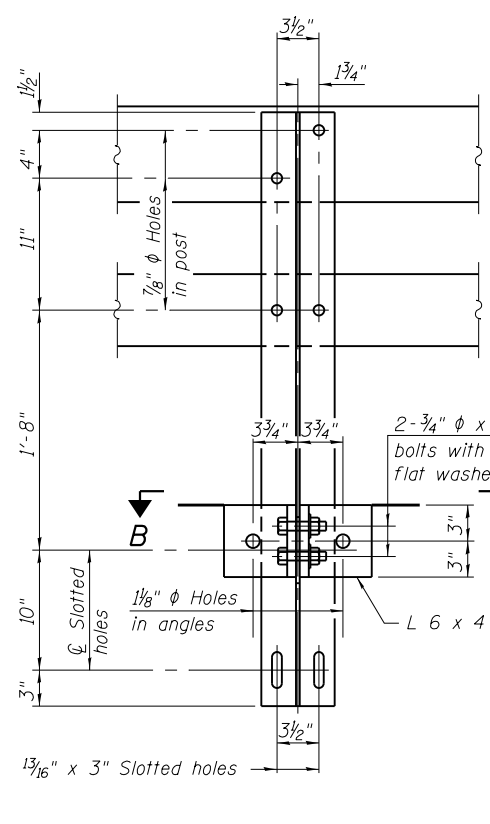
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**TEMPORARY CONCRETE BARRIER FOR STAGE CONSTRUCTION
STRUCTURE NO. 072-0245**

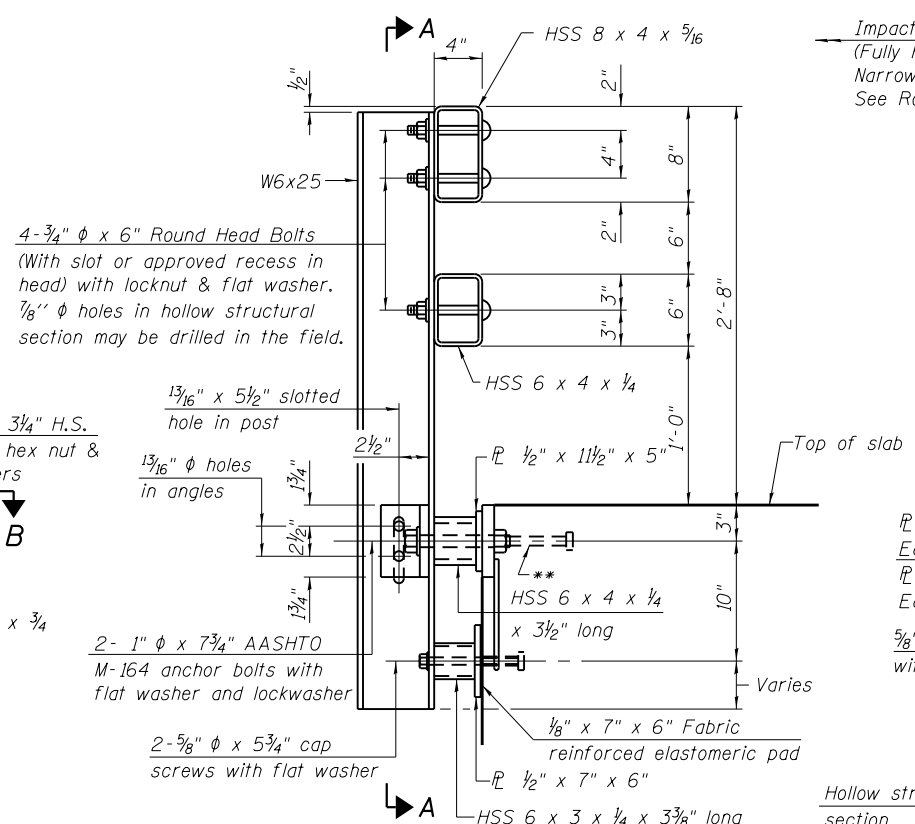
SHEET NO. 7 OF 52 SHEETS

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	421
CONTRACT NO. 68185				

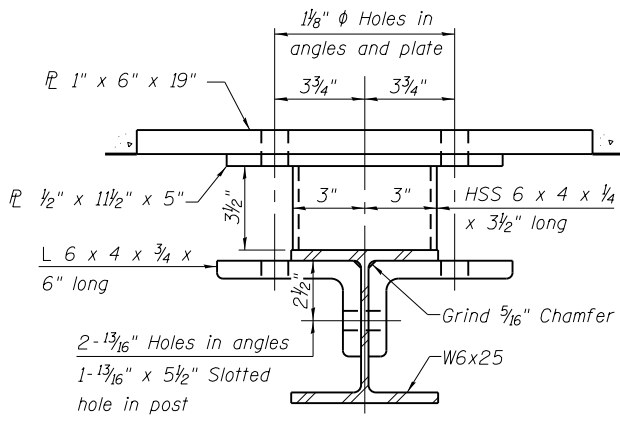
ILLINOIS FED. AID PROJECT



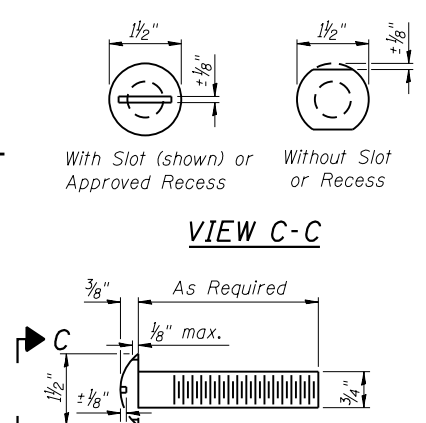
SECTION A-A



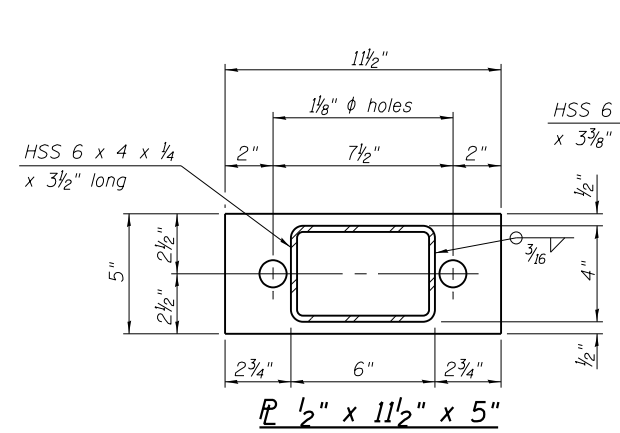
SECTION AT RAIL POST



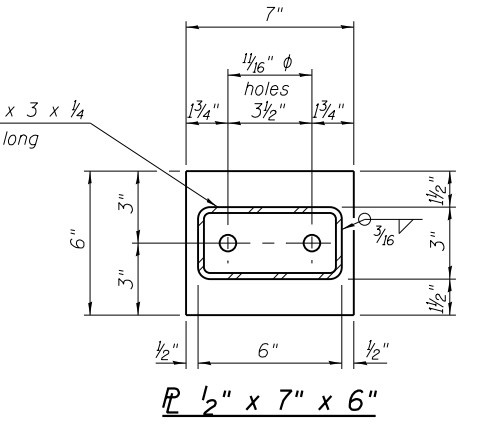
SECTION B-B



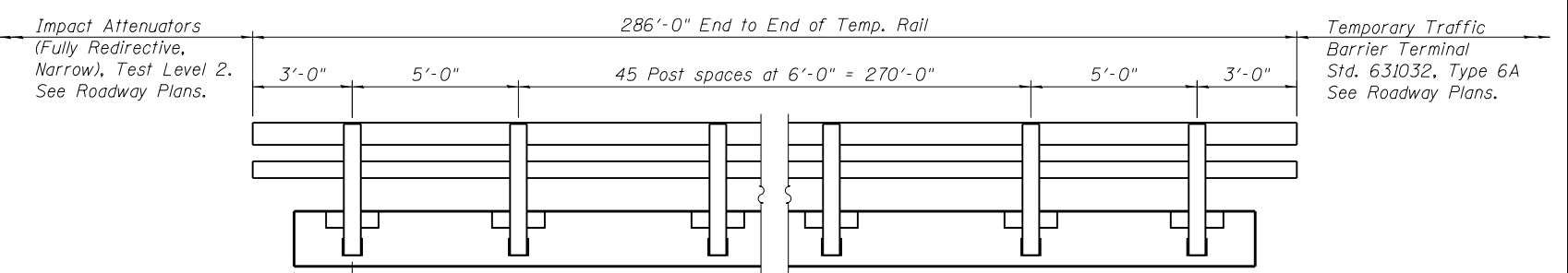
DETAIL OF 3/4" Ø ROUND HEAD BOLT



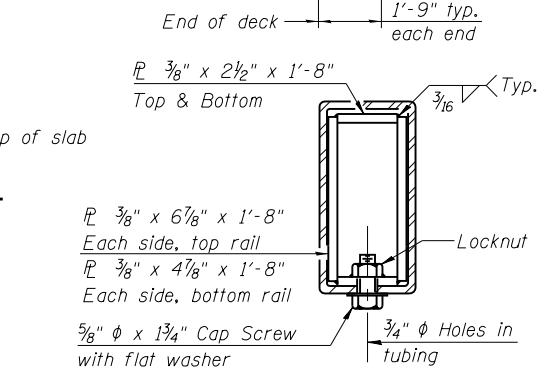
1/2" x 11 1/2" x 5"



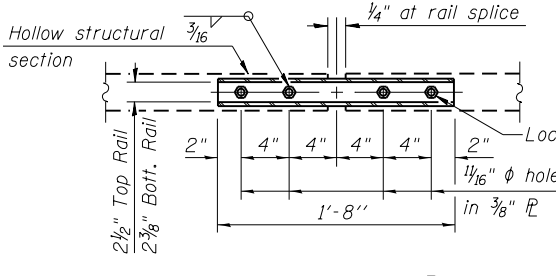
1/2" x 7" x 6"



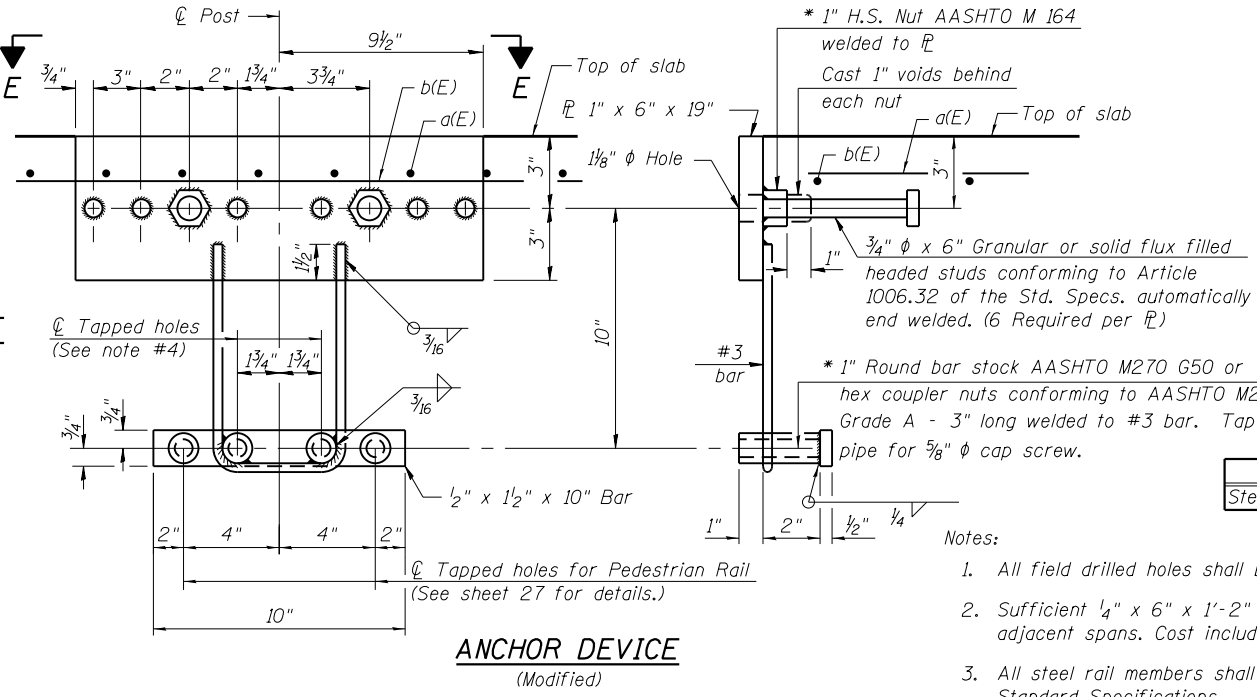
TEMPORARY RAIL POST SPACING



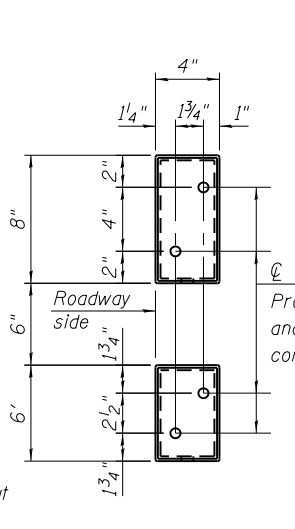
SECTION AT RAIL SPLICE



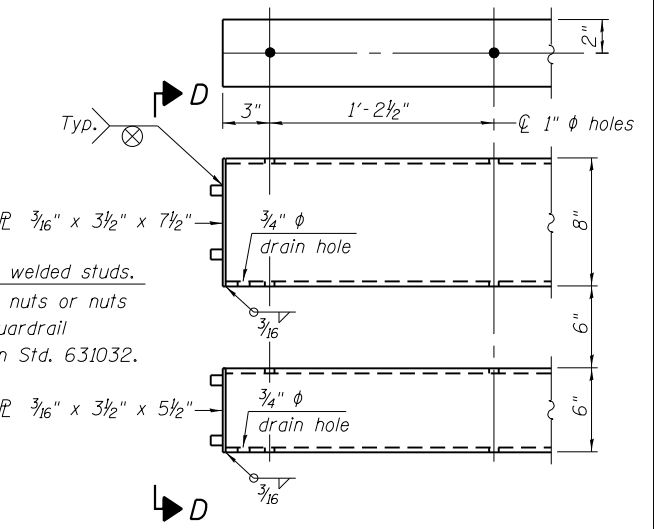
PLAN-BOTT. SPLICE TYPICAL



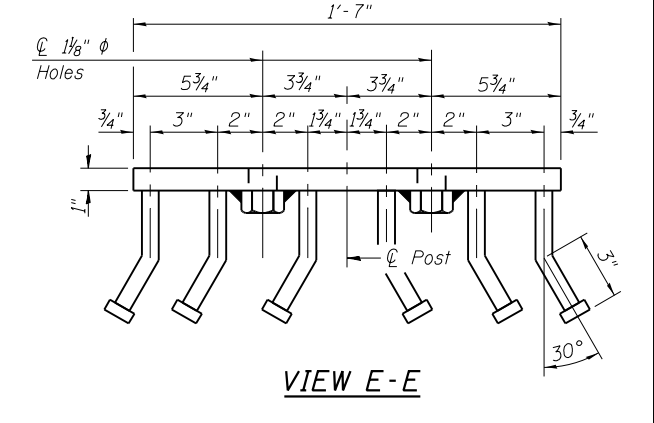
ANCHOR DEVICE (Modified)



VIEW D-D



END OF RAIL DETAILS



VIEW E-E

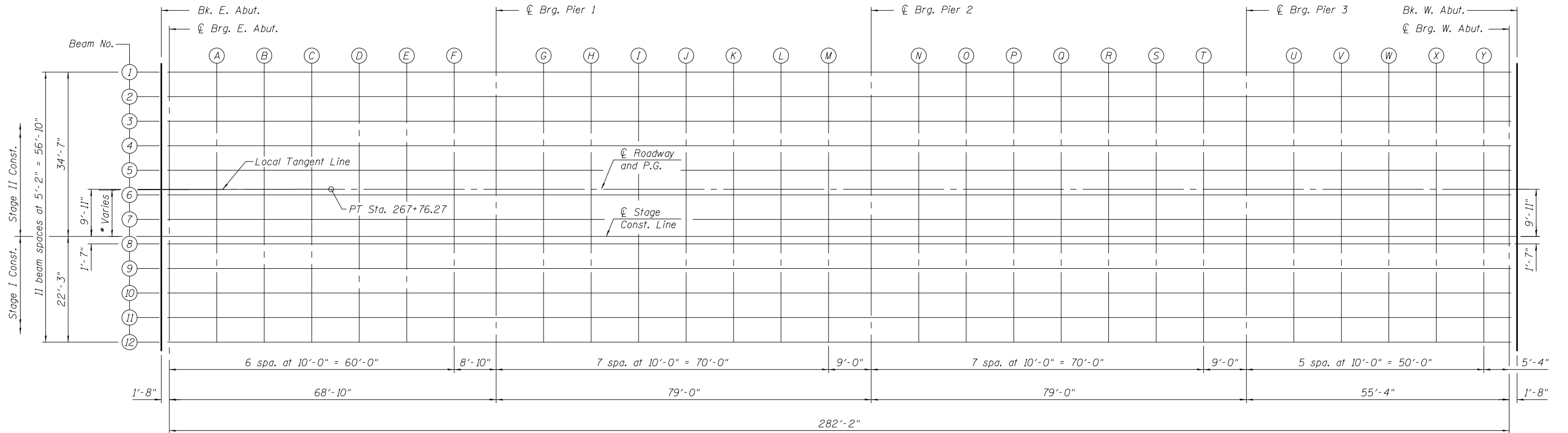
BILL OF MATERIAL

Item	Unit	Quantity
Steel Railing (Special)	Foot	286

Notes:

- All field drilled holes shall be coated with an approved zinc rich paint before erection.
- Sufficient 1/4" x 6" x 1'-2" galvanized steel shims shall be provided to align rail between adjacent spans. Cost included with Steel Railing (Special).
- All steel rail members shall be galvanized according to Article 509.05 of the Standard Specifications.
- Upon removal of Steel Railing (Special), tapped holes shall be filled by replacing the rail cap screws with galvanized bolts of the same diameter to fit flush with the edge of the deck. Cost included with Steel Railing (Special).

* Galvanized after fabrication. Threaded areas shall be plugged or blocked off during casting of slab and when not in use.
 ** The studs of the anchor devices shall be placed below the top reinforcement bar and the outermost longitudinal bar shall be placed directly above the studs of the rail anchor device.



PLAN

* Dimension varies from Sta. 267+40.50 to Sta. 267+76.27.
See Offset Sketch on sheet 2.

Note:
Work this sheet with sheets 10 thru 14.



USER NAME = *OPERATOR*	DESIGNED - TJZ	REVISED
	CHECKED - SBC	REVISED
PLOT SCALE = 0.2" = 1' / 1/4"	DRAWN - DLH	REVISED
PLOT DATE = 8/16/2018	CHECKED - SBC	REVISED

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

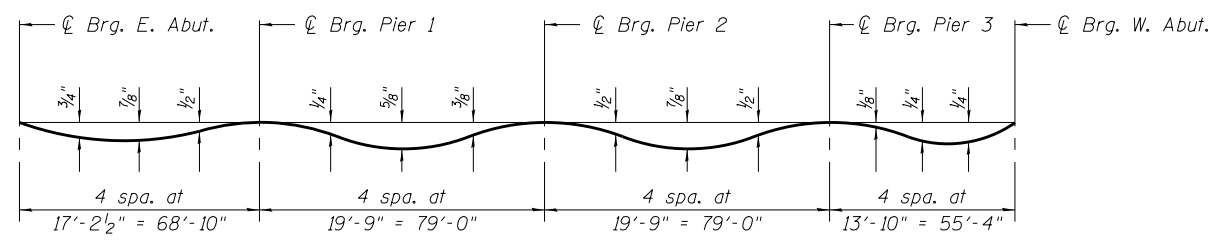
**TOP OF SLAB ELEVATIONS LOCATION PLAN
STRUCTURE NO. 072-0245**

SHEET NO. 9 OF 52 SHEETS

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N,BR-1,RS-4,W-1)	PEORIA	577	423
CONTRACT NO. 68185				

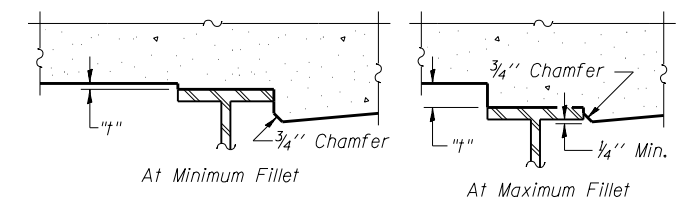
ILLINOIS FED. AID PROJECT

LAST SAVED DATE: 8/15/2018



DEAD LOAD DEFLECTION DIAGRAM
(Includes weight of concrete only.)

Note:
1. The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections as shown below and on sheets 11 thru 14.



To determine "t": After all structural steel has been erected, elevations of the top flanges of the beams shall be taken at intervals shown on sheet 9. These elevations subtracted from the "Theoretical Grade Elevations Adjusted For Dead Load Deflection" shown below and on sheets 11 thru 14, minus slab thickness, equals the fillet heights "t" above top flange of beams.

FILLET HEIGHTS

BEAM 1

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. E. Abut.	267+40.50	-24.76	480.96	480.96
☉ Brg. E. Abut.	267+42.17	-24.76	480.97	480.97
A	267+52.17	-24.71	481.08	481.11
B	267+62.17	-24.68	481.18	481.24
C	267+72.17	-24.67	481.28	481.35
D	267+82.17	-24.67	481.38	481.44
E	267+92.17	-24.67	481.48	481.52
F	268+02.17	-24.67	481.58	481.60
☉ Brg. Pier 1	268+11.00	-24.67	481.67	481.67
G	268+21.00	-24.67	481.77	481.78
H	268+31.00	-24.67	481.87	481.90
I	268+41.00	-24.67	481.97	482.02
J	268+51.00	-24.67	482.07	482.12
K	268+61.00	-24.67	482.17	482.22
L	268+71.00	-24.67	482.28	482.30
M	268+81.00	-24.67	482.38	482.38
☉ Brg. Pier 2	268+90.00	-24.67	482.47	482.47
N	269+00.00	-24.67	482.57	482.58
O	269+10.00	-24.67	482.67	482.71
P	269+20.00	-24.67	482.77	482.83
Q	269+30.00	-24.67	482.87	482.94
R	269+40.00	-24.67	482.97	483.03
S	269+50.00	-24.67	483.07	483.11
T	269+60.00	-24.67	483.17	483.19
☉ Brg. Pier 3	269+69.00	-24.67	483.27	483.27
U	269+79.00	-24.67	483.37	483.37
V	269+89.00	-24.67	483.47	483.48
W	269+99.00	-24.67	483.57	483.59
X	270+09.00	-24.67	483.67	483.69
Y	270+19.00	-24.67	483.77	483.78
☉ Brg. W. Abut.	270+24.33	-24.67	483.82	483.82
Bk. W. Abut.	270+26.00	-24.67	483.84	483.84

BEAM 2

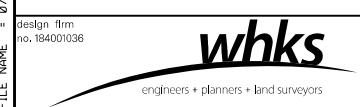
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. E. Abut.	267+40.50	-19.60	481.07	481.07
☉ Brg. E. Abut.	267+42.17	-19.59	481.08	481.08
A	267+52.17	-19.54	481.18	481.22
B	267+62.17	-19.52	481.28	481.35
C	267+72.17	-19.50	481.39	481.46
D	267+82.17	-19.50	481.49	481.55
E	267+92.17	-19.50	481.59	481.63
F	268+02.17	-19.50	481.69	481.70
☉ Brg. Pier 1	268+11.00	-19.50	481.78	481.78
G	268+21.00	-19.50	481.88	481.89
H	268+31.00	-19.50	481.98	482.01
I	268+41.00	-19.50	482.08	482.12
J	268+51.00	-19.50	482.18	482.23
K	268+61.00	-19.50	482.28	482.32
L	268+71.00	-19.50	482.38	482.41
M	268+81.00	-19.50	482.48	482.49
☉ Brg. Pier 2	268+90.00	-19.50	482.58	482.58
N	269+00.00	-19.50	482.68	482.69
O	269+10.00	-19.50	482.78	482.82
P	269+20.00	-19.50	482.88	482.94
Q	269+30.00	-19.50	482.98	483.05
R	269+40.00	-19.50	483.08	483.14
S	269+50.00	-19.50	483.18	483.22
T	269+60.00	-19.50	483.28	483.30
☉ Brg. Pier 3	269+69.00	-19.50	483.37	483.37
U	269+79.00	-19.50	483.47	483.48
V	269+89.00	-19.50	483.58	483.59
W	269+99.00	-19.50	483.68	483.70
X	270+09.00	-19.50	483.78	483.80
Y	270+19.00	-19.50	483.88	483.89
☉ Brg. W. Abut.	270+24.33	-19.50	483.93	483.93
Bk. W. Abut.	270+26.00	-19.50	483.95	483.95

BEAM 3

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. E. Abut.	267+40.50	-14.43	481.15	481.15
☉ Brg. E. Abut.	267+42.17	-14.42	481.17	481.17
A	267+52.17	-14.38	481.27	481.31
B	267+62.17	-14.35	481.37	481.43
C	267+72.17	-14.33	481.47	481.54
D	267+82.17	-14.33	481.57	481.64
E	267+92.17	-14.33	481.67	481.71
F	268+02.17	-14.33	481.77	481.79
☉ Brg. Pier 1	268+11.00	-14.33	481.86	481.86
G	268+21.00	-14.33	481.96	481.97
H	268+31.00	-14.33	482.06	482.09
I	268+41.00	-14.33	482.16	482.21
J	268+51.00	-14.33	482.26	482.31
K	268+61.00	-14.33	482.37	482.41
L	268+71.00	-14.33	482.47	482.49
M	268+81.00	-14.33	482.57	482.57
☉ Brg. Pier 2	268+90.00	-14.33	482.66	482.66
N	269+00.00	-14.33	482.76	482.77
O	269+10.00	-14.33	482.86	482.90
P	269+20.00	-14.33	482.96	483.02
Q	269+30.00	-14.33	483.06	483.13
R	269+40.00	-14.33	483.16	483.22
S	269+50.00	-14.33	483.26	483.31
T	269+60.00	-14.33	483.37	483.38
☉ Brg. Pier 3	269+69.00	-14.33	483.46	483.46
U	269+79.00	-14.33	483.56	483.56
V	269+89.00	-14.33	483.66	483.67
W	269+99.00	-14.33	483.76	483.78
X	270+09.00	-14.33	483.86	483.88
Y	270+19.00	-14.33	483.96	483.97
☉ Brg. W. Abut.	270+24.33	-14.33	484.01	484.01
Bk. W. Abut.	270+26.00	-14.33	484.03	484.03

Note:
Work this sheet with sheets 9 and 11 thru 14.

FILE NAME = 0720245-68185.dgn



USER NAME = *OPERATOR*	DESIGNED - TJZ	REVISED
	CHECKED - SBC	REVISED
PLOT SCALE = 0x2 1/4" = 1"	DRAWN - DLH	REVISED
PLOT DATE = 8/16/2018	CHECKED - SBC	REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TOP OF SLAB ELEVATIONS
STRUCTURE NO. 072-0245

SHEET NO. 10 OF 52 SHEETS

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	424
CONTRACT NO. 68185				

ILLINOIS FED. AID PROJECT

BEAM 4

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. E. Abut.	267+40.50	-9.26	481.23	481.23
☉ Brg. E. Abut.	267+42.17	-9.26	481.25	481.25
A	267+52.17	-9.21	481.35	481.39
B	267+62.17	-9.18	481.45	481.52
C	267+72.17	-9.17	481.55	481.62
D	267+82.17	-9.17	481.65	481.72
E	267+92.17	-9.17	481.75	481.79
F	268+02.17	-9.17	481.85	481.87
☉ Brg. Pier 1	268+11.00	-9.17	481.94	481.94
G	268+21.00	-9.17	482.04	482.05
H	268+31.00	-9.17	482.14	482.17
I	268+41.00	-9.17	482.24	482.29
J	268+51.00	-9.17	482.35	482.39
K	268+61.00	-9.17	482.45	482.49
L	268+71.00	-9.17	482.55	482.57
M	268+81.00	-9.17	482.65	482.66
☉ Brg. Pier 2	268+90.00	-9.17	482.74	482.74
N	269+00.00	-9.17	482.84	482.86
O	269+10.00	-9.17	482.94	482.98
P	269+20.00	-9.17	483.04	483.10
Q	269+30.00	-9.17	483.14	483.21
R	269+40.00	-9.17	483.24	483.31
S	269+50.00	-9.17	483.35	483.39
T	269+60.00	-9.17	483.45	483.46
☉ Brg. Pier 3	269+69.00	-9.17	483.54	483.54
U	269+79.00	-9.17	483.64	483.64
V	269+89.00	-9.17	483.74	483.75
W	269+99.00	-9.17	483.84	483.86
X	270+09.00	-9.17	483.94	483.96
Y	270+19.00	-9.17	484.04	484.05
☉ Brg. W. Abut.	270+24.33	-9.17	484.10	484.10
Bk. W. Abut.	270+26.00	-9.17	484.11	484.11

BEAM 5

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. E. Abut.	267+40.50	-4.10	481.31	481.31
☉ Brg. E. Abut.	267+42.17	-4.09	481.33	481.33
A	267+52.17	-4.04	481.43	481.47
B	267+62.17	-4.02	481.53	481.60
C	267+72.17	-4.00	481.63	481.70
D	267+82.17	-4.00	481.73	481.80
E	267+92.17	-4.00	481.83	481.87
F	268+02.17	-4.00	481.93	481.95
☉ Brg. Pier 1	268+11.00	-4.00	482.02	482.02
G	268+21.00	-4.00	482.12	482.13
H	268+31.00	-4.00	482.22	482.25
I	268+41.00	-4.00	482.32	482.37
J	268+51.00	-4.00	482.43	482.47
K	268+61.00	-4.00	482.53	482.57
L	268+71.00	-4.00	482.63	482.65
M	268+81.00	-4.00	482.73	482.74
☉ Brg. Pier 2	268+90.00	-4.00	482.82	482.82
N	269+00.00	-4.00	482.92	482.94
O	269+10.00	-4.00	483.02	483.06
P	269+20.00	-4.00	483.12	483.18
Q	269+30.00	-4.00	483.22	483.29
R	269+40.00	-4.00	483.32	483.39
S	269+50.00	-4.00	483.43	483.47
T	269+60.00	-4.00	483.53	483.54
☉ Brg. Pier 3	269+69.00	-4.00	483.62	483.62
U	269+79.00	-4.00	483.72	483.72
V	269+89.00	-4.00	483.82	483.83
W	269+99.00	-4.00	483.92	483.94
X	270+09.00	-4.00	484.02	484.04
Y	270+19.00	-4.00	484.12	484.13
☉ Brg. W. Abut.	270+24.33	-4.00	484.18	484.18
Bk. W. Abut.	270+26.00	-4.00	484.19	484.19

☉ ROADWAY AND P.G.

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. E. Abut.	267+40.50	0.00	481.37	481.37
☉ Brg. E. Abut.	267+42.17	0.00	481.39	481.39
A	267+52.17	0.00	481.49	481.53
B	267+62.17	0.00	481.59	481.66
C	267+72.17	0.00	481.69	481.77
D	267+82.17	0.00	481.79	481.86
E	267+92.17	0.00	481.89	481.94
F	268+02.17	0.00	482.00	482.01
☉ Brg. Pier 1	268+11.00	0.00	482.08	482.08
G	268+21.00	0.00	482.19	482.19
H	268+31.00	0.00	482.29	482.31
I	268+41.00	0.00	482.39	482.43
J	268+51.00	0.00	482.49	482.54
K	268+61.00	0.00	482.59	482.63
L	268+71.00	0.00	482.69	482.72
M	268+81.00	0.00	482.79	482.80
☉ Brg. Pier 2	268+90.00	0.00	482.88	482.88
N	269+00.00	0.00	482.98	483.00
O	269+10.00	0.00	483.08	483.12
P	269+20.00	0.00	483.19	483.25
Q	269+30.00	0.00	483.29	483.36
R	269+40.00	0.00	483.39	483.45
S	269+50.00	0.00	483.49	483.53
T	269+60.00	0.00	483.59	483.61
☉ Brg. Pier 3	269+69.00	0.00	483.68	483.68
U	269+79.00	0.00	483.78	483.78
V	269+89.00	0.00	483.88	483.89
W	269+99.00	0.00	483.98	484.00
X	270+09.00	0.00	484.08	484.10
Y	270+19.00	0.00	484.19	484.19
☉ Brg. W. Abut.	270+24.33	0.00	484.24	484.24
Bk. W. Abut.	270+26.00	0.00	484.26	484.26

Note:
Work this sheet with sheets 9, 10, and 12 thru 14.



USER NAME = *OPERATOR*	DESIGNED - TJZ	REVISED
	CHECKED - SBC	REVISED
PLOT SCALE = 0x2" = 1' / 1"	DRAWN - DLH	REVISED
PLOT DATE = 8/16/2018	CHECKED - SBC	REVISED

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**TOP OF SLAB ELEVATIONS
STRUCTURE NO. 072-0245**

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N,BR-1,RS-4,W-1)	PEORIA	577	425
CONTRACT NO. 68185				

BEAM 6

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. E. Abut.	267+40.50	1.07	481.35	481.35
⊕ Brg. E. Abut.	267+42.17	1.08	481.37	481.37
A	267+52.17	1.12	481.47	481.51
B	267+62.17	1.15	481.57	481.64
C	267+72.17	1.17	481.67	481.75
D	267+82.17	1.17	481.78	481.84
E	267+92.17	1.17	481.88	481.92
F	268+02.17	1.17	481.98	481.99
⊕ Brg. Pier 1	268+11.00	1.17	482.07	482.07
G	268+21.00	1.17	482.17	482.17
H	268+31.00	1.17	482.27	482.29
I	268+41.00	1.17	482.37	482.41
J	268+51.00	1.17	482.47	482.52
K	268+61.00	1.17	482.57	482.61
L	268+71.00	1.17	482.67	482.70
M	268+81.00	1.17	482.77	482.78
⊕ Brg. Pier 2	268+90.00	1.17	482.86	482.86
N	269+00.00	1.17	482.97	482.98
O	269+10.00	1.17	483.07	483.11
P	269+20.00	1.17	483.17	483.23
Q	269+30.00	1.17	483.27	483.34
R	269+40.00	1.17	483.37	483.43
S	269+50.00	1.17	483.47	483.51
T	269+60.00	1.17	483.57	483.59
⊕ Brg. Pier 3	269+69.00	1.17	483.66	483.66
U	269+79.00	1.17	483.76	483.77
V	269+89.00	1.17	483.86	483.88
W	269+99.00	1.17	483.97	483.99
X	270+09.00	1.17	484.07	484.09
Y	270+19.00	1.17	484.17	484.18
⊕ Brg. W. Abut.	270+24.33	1.17	484.22	484.22
Bk. W. Abut.	270+26.00	1.17	484.24	484.24

BEAM 7

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. E. Abut.	267+40.50	6.24	481.27	481.27
⊕ Brg. E. Abut.	267+42.17	6.24	481.29	481.29
A	267+52.17	6.29	481.39	481.43
B	267+62.17	6.32	481.49	481.56
C	267+72.17	6.33	481.59	481.67
D	267+82.17	6.33	481.69	481.76
E	267+92.17	6.33	481.80	481.84
F	268+02.17	6.33	481.90	481.91
⊕ Brg. Pier 1	268+11.00	6.33	481.99	481.99
G	268+21.00	6.33	482.09	482.09
H	268+31.00	6.33	482.19	482.21
I	268+41.00	6.33	482.29	482.33
J	268+51.00	6.33	482.39	482.44
K	268+61.00	6.33	482.49	482.53
L	268+71.00	6.33	482.59	482.62
M	268+81.00	6.33	482.69	482.70
⊕ Brg. Pier 2	268+90.00	6.33	482.78	482.78
N	269+00.00	6.33	482.88	482.90
O	269+10.00	6.33	482.99	483.03
P	269+20.00	6.33	483.09	483.15
Q	269+30.00	6.33	483.19	483.26
R	269+40.00	6.33	483.29	483.35
S	269+50.00	6.33	483.39	483.43
T	269+60.00	6.33	483.49	483.51
⊕ Brg. Pier 3	269+69.00	6.33	483.58	483.58
U	269+79.00	6.33	483.68	483.68
V	269+89.00	6.33	483.78	483.80
W	269+99.00	6.33	483.88	483.90
X	270+09.00	6.33	483.99	484.00
Y	270+19.00	6.33	484.09	484.09
⊕ Brg. W. Abut.	270+24.33	6.33	484.14	484.14
Bk. W. Abut.	270+26.00	6.33	484.16	484.16

STAGE CONSTRUCTION LINE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. E. Abut.	267+40.50	9.82	481.22	481.22
⊕ Brg. E. Abut.	267+42.17	9.83	481.23	481.23
A	267+52.17	9.87	481.34	481.37
B	267+62.17	9.90	481.44	481.50
C	267+72.17	9.92	481.54	481.61
D	267+82.17	9.92	481.64	481.70
E	267+92.17	9.92	481.74	481.78
F	268+02.17	9.92	481.84	481.86
⊕ Brg. Pier 1	268+11.00	9.92	481.93	481.93
G	268+21.00	9.92	482.03	482.04
H	268+31.00	9.92	482.13	482.16
I	268+41.00	9.92	482.23	482.28
J	268+51.00	9.92	482.33	482.38
K	268+61.00	9.92	482.43	482.48
L	268+71.00	9.92	482.54	482.56
M	268+81.00	9.92	482.64	482.64
⊕ Brg. Pier 2	268+90.00	9.92	482.73	482.73
N	269+00.00	9.92	482.83	482.84
O	269+10.00	9.92	482.93	482.97
P	269+20.00	9.92	483.03	483.09
Q	269+30.00	9.92	483.13	483.20
R	269+40.00	9.92	483.23	483.29
S	269+50.00	9.92	483.33	483.37
T	269+60.00	9.92	483.43	483.45
⊕ Brg. Pier 3	269+69.00	9.92	483.53	483.53
U	269+79.00	9.92	483.63	483.63
V	269+89.00	9.92	483.73	483.74
W	269+99.00	9.92	483.83	483.85
X	270+09.00	9.92	483.93	483.95
Y	270+19.00	9.92	484.03	484.04
⊕ Brg. W. Abut.	270+24.33	9.92	484.08	484.08
Bk. W. Abut.	270+26.00	9.92	484.10	484.10

Note:
Work this sheet with sheets 9 thru 11, 13, and 14.

BEAM 8

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. E. Abut.	267+40.50	11.40	481.19	481.19
⊕ Brg. E. Abut.	267+42.17	11.41	481.21	481.21
A	267+52.17	11.46	481.31	481.35
B	267+62.17	11.48	481.41	481.48
C	267+72.17	11.50	481.51	481.59
D	267+82.17	11.50	481.61	481.68
E	267+92.17	11.50	481.71	481.76
F	268+02.17	11.50	481.82	481.83
⊕ Brg. Pier 1	268+11.00	11.50	481.90	481.90
G	268+21.00	11.50	482.01	482.01
H	268+31.00	11.50	482.11	482.13
I	268+41.00	11.50	482.21	482.25
J	268+51.00	11.50	482.31	482.36
K	268+61.00	11.50	482.41	482.45
L	268+71.00	11.50	482.51	482.54
M	268+81.00	11.50	482.61	482.62
⊕ Brg. Pier 2	268+90.00	11.50	482.70	482.70
N	269+00.00	11.50	482.80	482.82
O	269+10.00	11.50	482.90	482.94
P	269+20.00	11.50	483.01	483.07
Q	269+30.00	11.50	483.11	483.18
R	269+40.00	11.50	483.21	483.27
S	269+50.00	11.50	483.31	483.35
T	269+60.00	11.50	483.41	483.43
⊕ Brg. Pier 3	269+69.00	11.50	483.50	483.50
U	269+79.00	11.50	483.60	483.60
V	269+89.00	11.50	483.70	483.71
W	269+99.00	11.50	483.80	483.82
X	270+09.00	11.50	483.90	483.92
Y	270+19.00	11.50	484.01	484.01
⊕ Brg. W. Abut.	270+24.33	11.50	484.06	484.06
Bk. W. Abut.	270+26.00	11.50	484.08	484.08

BEAM 9

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. E. Abut.	267+40.50	16.57	481.11	481.11
⊕ Brg. E. Abut.	267+42.17	16.58	481.13	481.13
A	267+52.17	16.62	481.23	481.27
B	267+62.17	16.65	481.33	481.40
C	267+72.17	16.67	481.43	481.51
D	267+82.17	16.67	481.53	481.60
E	267+92.17	16.67	481.63	481.68
F	268+02.17	16.67	481.74	481.75
⊕ Brg. Pier 1	268+11.00	16.67	481.82	481.82
G	268+21.00	16.67	481.93	481.93
H	268+31.00	16.67	482.03	482.05
I	268+41.00	16.67	482.13	482.17
J	268+51.00	16.67	482.23	482.28
K	268+61.00	16.67	482.33	482.37
L	268+71.00	16.67	482.43	482.46
M	268+81.00	16.67	482.53	482.54
⊕ Brg. Pier 2	268+90.00	16.67	482.62	482.62
N	269+00.00	16.67	482.72	482.74
O	269+10.00	16.67	482.82	482.86
P	269+20.00	16.67	482.93	482.99
Q	269+30.00	16.67	483.03	483.10
R	269+40.00	16.67	483.13	483.19
S	269+50.00	16.67	483.23	483.27
T	269+60.00	16.67	483.33	483.35
⊕ Brg. Pier 3	269+69.00	16.67	483.42	483.42
U	269+79.00	16.67	483.52	483.52
V	269+89.00	16.67	483.62	483.63
W	269+99.00	16.67	483.72	483.74
X	270+09.00	16.67	483.82	483.84
Y	270+19.00	16.67	483.93	483.93
⊕ Brg. W. Abut.	270+24.33	16.67	483.98	483.98
Bk. W. Abut.	270+26.00	16.67	484.00	484.00

BEAM 10

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. E. Abut.	267+40.50	21.74	481.02	481.02
⊕ Brg. E. Abut.	267+42.17	21.74	481.03	481.03
A	267+52.17	21.79	481.13	481.17
B	267+62.17	21.82	481.24	481.30
C	267+72.17	21.83	481.34	481.41
D	267+82.17	21.83	481.44	481.50
E	267+92.17	21.83	481.54	481.58
F	268+02.17	21.83	481.64	481.65
⊕ Brg. Pier 1	268+11.00	21.83	481.73	481.73
G	268+21.00	21.83	481.83	481.84
H	268+31.00	21.83	481.93	481.96
I	268+41.00	21.83	482.03	482.07
J	268+51.00	21.83	482.13	482.18
K	268+61.00	21.83	482.23	482.28
L	268+71.00	21.83	482.33	482.36
M	268+81.00	21.83	482.44	482.44
⊕ Brg. Pier 2	268+90.00	21.83	482.53	482.53
N	269+00.00	21.83	482.63	482.64
O	269+10.00	21.83	482.73	482.77
P	269+20.00	21.83	482.83	482.89
Q	269+30.00	21.83	482.93	483.00
R	269+40.00	21.83	483.03	483.09
S	269+50.00	21.83	483.13	483.17
T	269+60.00	21.83	483.23	483.25
⊕ Brg. Pier 3	269+69.00	21.83	483.32	483.32
U	269+79.00	21.83	483.43	483.43
V	269+89.00	21.83	483.53	483.54
W	269+99.00	21.83	483.63	483.65
X	270+09.00	21.83	483.73	483.75
Y	270+19.00	21.83	483.83	483.84
⊕ Brg. W. Abut.	270+24.33	21.83	483.88	483.88
Bk. W. Abut.	270+26.00	21.83	483.90	483.90

Note:
Work this sheet with sheets 9 thru 12 and 14.



USER NAME = *OPERATOR*	DESIGNED - TJZ	REVISED
	CHECKED - SBC	REVISED
PLOT SCALE = 0x2" = 1' / 16"	DRAWN - DLH	REVISED
PLOT DATE = 8/16/2018	CHECKED - SBC	REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TOP OF SLAB ELEVATIONS
STRUCTURE NO. 072-0245

SHEET NO. 13 OF 52 SHEETS

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	427
CONTRACT NO. 68185				

BEAM 11

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. E. Abut.	267+40.50	26.90	480.91	480.91
⊕ Brg. E. Abut.	267+42.17	26.91	480.93	480.93
A	267+52.17	26.96	481.03	481.07
B	267+62.17	26.98	481.13	481.19
C	267+72.17	27.00	481.23	481.30
D	267+82.17	27.00	481.33	481.40
E	267+92.17	27.00	481.43	481.47
F	268+02.17	27.00	481.53	481.55
⊕ Brg. Pier 1	268+11.00	27.00	481.62	481.62
G	268+21.00	27.00	481.72	481.73
H	268+31.00	27.00	481.82	481.85
I	268+41.00	27.00	481.92	481.97
J	268+51.00	27.00	482.02	482.07
K	268+61.00	27.00	482.13	482.17
L	268+71.00	27.00	482.23	482.25
M	268+81.00	27.00	482.33	482.33
⊕ Brg. Pier 2	268+90.00	27.00	482.42	482.42
N	269+00.00	27.00	482.52	482.53
O	269+10.00	27.00	482.62	482.66
P	269+20.00	27.00	482.72	482.78
Q	269+30.00	27.00	482.82	482.89
R	269+40.00	27.00	482.92	482.98
S	269+50.00	27.00	483.02	483.07
T	269+60.00	27.00	483.13	483.14
⊕ Brg. Pier 3	269+69.00	27.00	483.22	483.22
U	269+79.00	27.00	483.32	483.32
V	269+89.00	27.00	483.42	483.43
W	269+99.00	27.00	483.52	483.54
X	270+09.00	27.00	483.62	483.64
Y	270+19.00	27.00	483.72	483.73
⊕ Brg. W. Abut.	270+24.33	27.00	483.77	483.77
Bk. W. Abut.	270+26.00	27.00	483.79	483.79

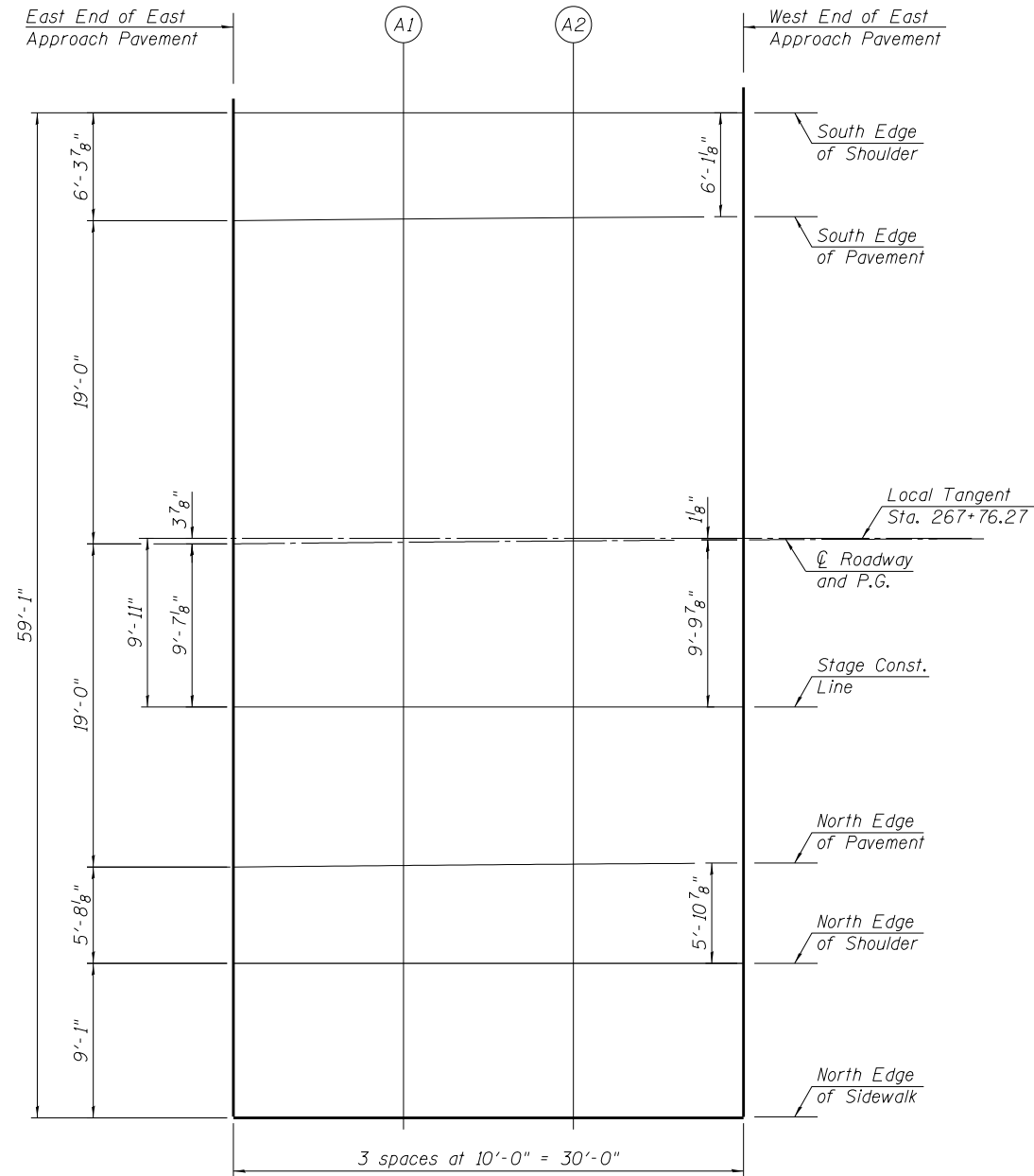
BEAM 12

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. E. Abut.	267+40.50	32.07	480.80	480.80
⊕ Brg. E. Abut.	267+42.17	32.08	480.82	480.82
A	267+52.17	32.12	480.92	480.96
B	267+62.17	32.15	481.02	481.09
C	267+72.17	32.17	481.12	481.20
D	267+82.17	32.17	481.22	481.29
E	267+92.17	32.17	481.32	481.37
F	268+02.17	32.17	481.42	481.44
⊕ Brg. Pier 1	268+11.00	32.17	481.51	481.51
G	268+21.00	32.17	481.61	481.62
H	268+31.00	32.17	481.72	481.74
I	268+41.00	32.17	481.82	481.86
J	268+51.00	32.17	481.92	481.97
K	268+61.00	32.17	482.02	482.06
L	268+71.00	32.17	482.12	482.14
M	268+81.00	32.17	482.22	482.23
⊕ Brg. Pier 2	268+90.00	32.17	482.31	482.31
N	269+00.00	32.17	482.41	482.43
O	269+10.00	32.17	482.51	482.55
P	269+20.00	32.17	482.61	482.68
Q	269+30.00	32.17	482.72	482.79
R	269+40.00	32.17	482.82	482.88
S	269+50.00	32.17	482.92	482.96
T	269+60.00	32.17	483.02	483.03
⊕ Brg. Pier 3	269+69.00	32.17	483.11	483.11
U	269+79.00	32.17	483.21	483.21
V	269+89.00	32.17	483.31	483.32
W	269+99.00	32.17	483.41	483.43
X	270+09.00	32.17	483.51	483.53
Y	270+19.00	32.17	483.61	483.62
⊕ Brg. W. Abut.	270+24.33	32.17	483.67	483.67
Bk. W. Abut.	270+26.00	32.17	483.68	483.68

Note:
Work this sheet with sheets 9 thru 13.

USER NAME = *OPERATOR*	DESIGNED - TJZ	REVISED
	CHECKED - SBC	REVISED
PLOT SCALE = 0x2" = 1/4" = 1/4"	DRAWN - DLH	REVISED
PLOT DATE = 8/16/2018	CHECKED - SBC	REVISED

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	428
CONTRACT NO. 68185				



PLAN

Notes:

1. Stations and offsets shown are located radial to \varnothing Roadway and P.G.
2. Work this sheet with sheet 9 thru 14 and 16.



SOUTH EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations
E. End East Appr. Pav't.	267+11.50	-25.32	480.65
A1	267+21.50	-25.23	480.75
A2	267+31.50	-25.15	480.86
W. End East Appr. Pav't.	267+41.50	-25.09	480.96

SOUTH EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
E. End East Appr. Pav't.	267+11.50	-19.00	480.78
A1	267+21.50	-19.00	480.88
A2	267+31.50	-19.00	480.98
W. End East Appr. Pav't.	267+41.50	-19.00	481.09

\varnothing ROADWAY & PROFILE GRADE

Location	Station	Offset	Theoretical Grade Elevations
E. End East Appr. Pav't.	267+11.50	0.00	481.08
A1	267+21.50	0.00	481.18
A2	267+31.50	0.00	481.28
W. End East Appr. Pav't.	267+41.50	0.00	481.38

STAGE CONSTRUCTION JOINT

Location	Station	Offset	Theoretical Grade Elevations
E. End East Appr. Pav't.	267+11.50	9.59	480.93
A1	267+21.50	9.68	481.08
A2	267+31.50	9.76	481.13
W. End East Appr. Pav't.	267+41.50	9.82	481.23

NORTH EDGE OF PAVEMENT

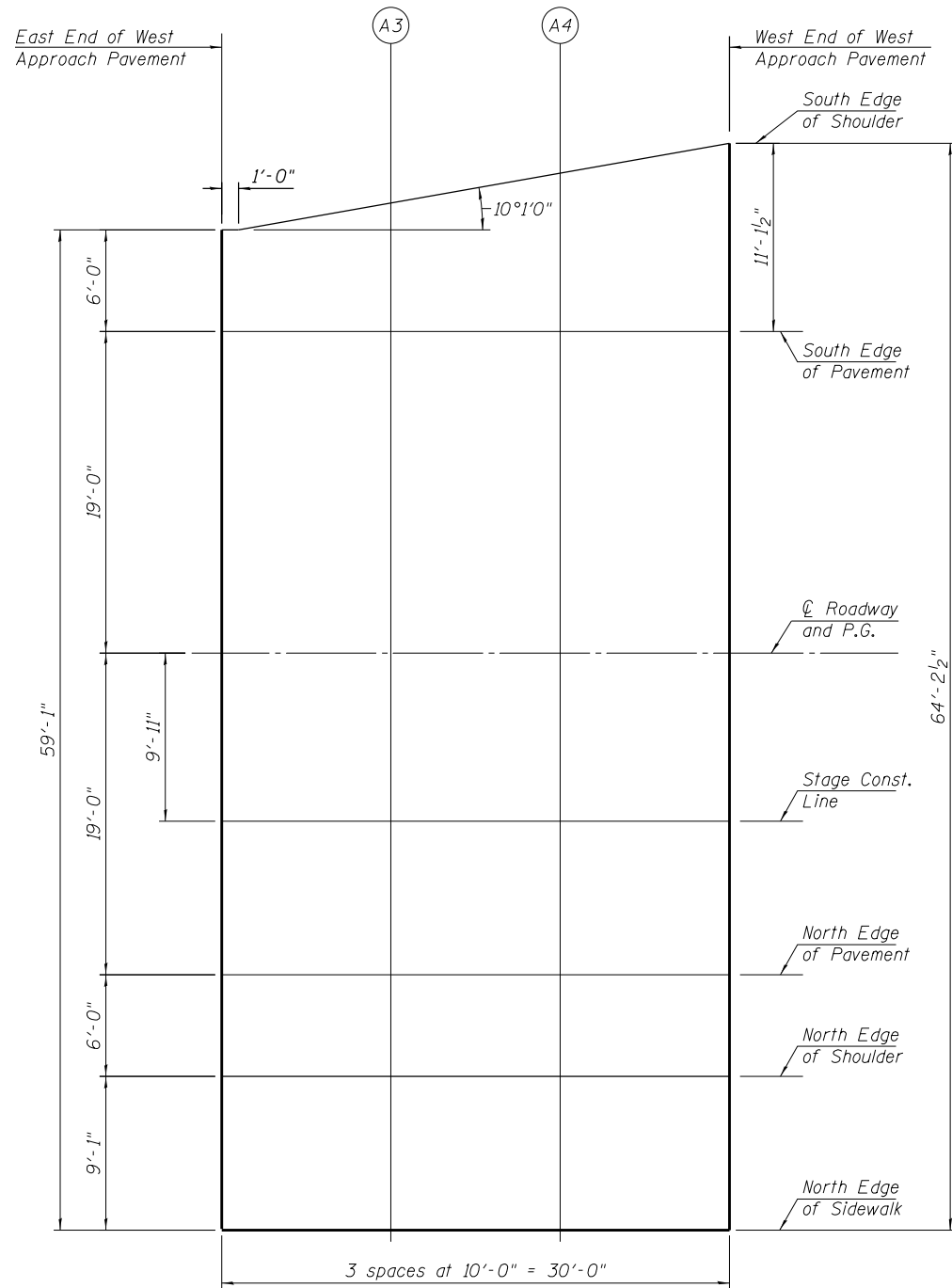
Location	Station	Offset	Theoretical Grade Elevations
E. End East Appr. Pav't.	267+11.50	19.00	480.78
A1	267+21.50	19.00	480.88
A2	267+31.50	19.00	480.98
W. End East Appr. Pav't.	267+41.50	19.00	481.09

NORTH EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations
E. End East Appr. Pav't.	267+11.50	24.68	480.66
A1	267+21.50	24.77	480.76
A2	267+31.50	24.85	480.86
W. End East Appr. Pav't.	267+41.50	24.91	480.96

NORTH EDGE OF SIDEWALK

Location	Station	Offset	Theoretical Grade Elevations
E. End East Appr. Pav't.	267+11.50	33.75	480.48
A1	267+21.50	33.85	480.57
A2	267+31.50	33.92	480.67
W. End East Appr. Pav't.	267+41.50	33.99	480.77



PLAN

Note:
Work this sheet with sheets 9 thru 15.

SOUTH EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations
E. End West Appr. Pav't.	270+25.00	-25.00	483.82
A3	270+35.00	-26.59	483.89
A4	270+45.00	-28.36	483.96
W. End West Appr. Pav't.	270+55.00	-30.12	484.02

SOUTH EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
E. End West Appr. Pav't.	270+25.00	-19.00	483.95
A3	270+35.00	-19.00	484.05
A4	270+45.00	-19.00	484.15
W. End West Appr. Pav't.	270+55.00	-19.00	484.25

ROADWAY & PROFILE GRADE

Location	Station	Offset	Theoretical Grade Elevations
E. End West Appr. Pav't.	270+25.00	0.00	484.25
A3	270+35.00	0.00	484.35
A4	270+45.00	0.00	484.45
W. End West Appr. Pav't.	270+55.00	0.00	484.55

STAGE CONSTRUCTION JOINT

Location	Station	Offset	Theoretical Grade Elevations
E. End West Appr. Pav't.	270+25.00	9.92	484.09
A3	270+35.00	9.92	484.19
A4	270+45.00	9.92	484.29
W. End West Appr. Pav't.	270+55.00	9.92	484.39

NORTH EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
E. End West Appr. Pav't.	270+25.00	19.00	483.95
A3	270+35.00	19.00	484.05
A4	270+45.00	19.00	484.15
W. End West Appr. Pav't.	270+55.00	19.00	484.25

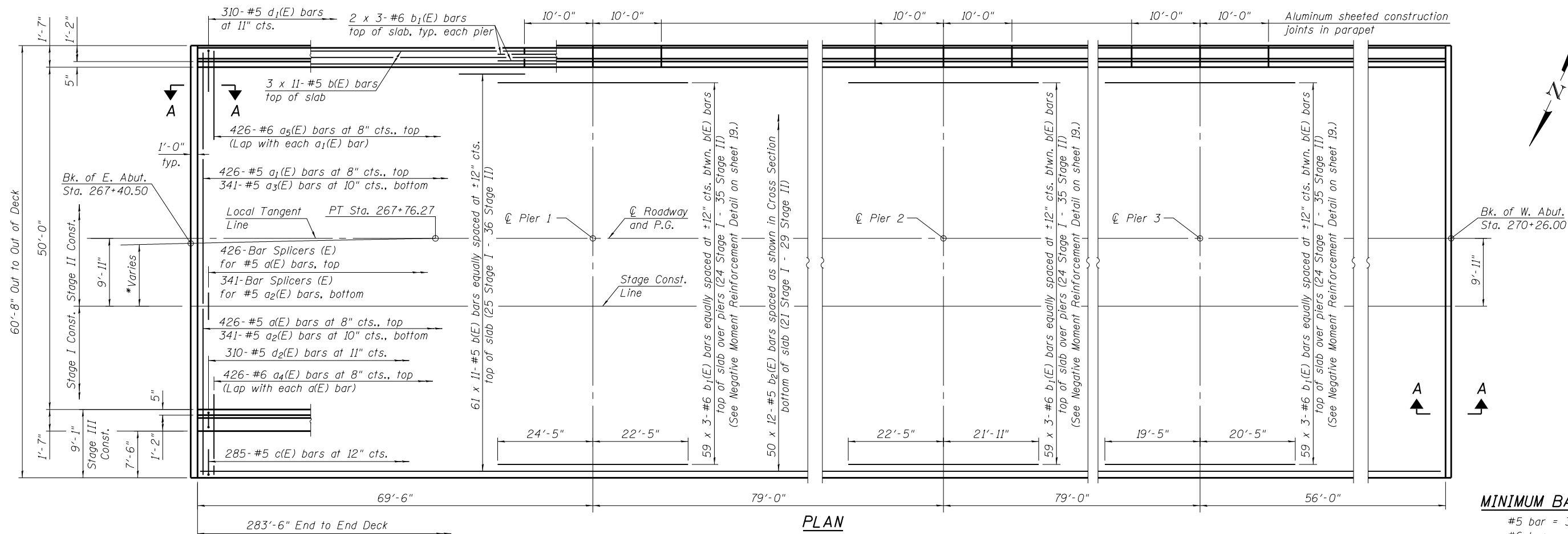
NORTH EDGE OF SHOULDER

Location	Station	Offset	Theoretical Grade Elevations
E. End West Appr. Pav't.	270+25.00	25.00	483.82
A3	270+35.00	25.00	483.92
A4	270+45.00	25.00	484.03
W. End West Appr. Pav't.	270+55.00	25.00	484.13

NORTH EDGE OF SIDEWALK

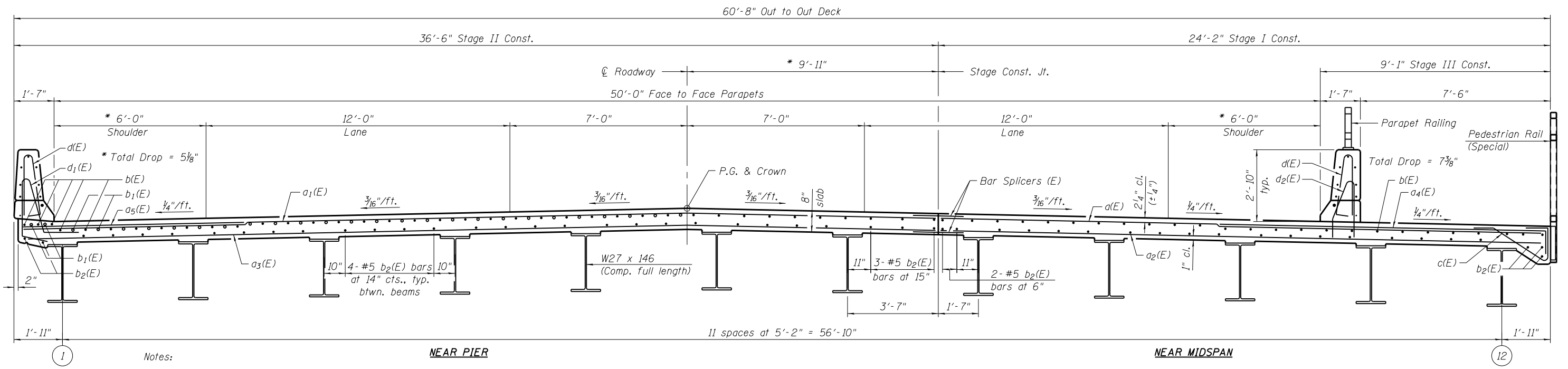
Location	Station	Offset	Theoretical Grade Elevations
E. End West Appr. Pav't.	270+25.00	34.08	483.63
A3	270+35.00	34.08	483.74
A4	270+45.00	34.08	483.84
W. End West Appr. Pav't.	270+55.00	34.08	483.94

LAST SAVED DATE: 8/15/2018



MINIMUM BAR LAP

#5 bar = 3'-9"
#6 bar = 4'-9"



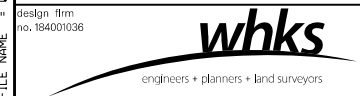
CROSS SECTION

(Looking West)

- Notes:
1. Bridge Deck Grooving, completed in Stage I Construction, shall not be applied directly underneath the proposed north parapet. The grooving shall be completed in accordance with Article 503.16(a)(3) of the Standard Specifications.
 2. See sheet 18 and 19 for superstructure details and Bill of Material.
 3. See sheet 20 for diaphragm details and Section A-A.
 4. Bars indicated thus 59 x 3-#6 etc. indicates 59 lines of bars with 3 lengths per line.
 5. See sheet 18 for parapet reinforcement.
 6. See sheet 42 for bar splicer details.

* Dimension varies from Sta. 267+40.50 to Sta. 267+76.27.
See Offset Sketch on sheet 2.

FILE NAME = 0720245-68185.dgn



USER NAME = *OPERATOR*	DESIGNED - TJZ	REVISED
	CHECKED - SBC	REVISED
PLOT SCALE = 0.2" = 1'-0"	DRAWN - DLH	REVISED
PLOT DATE = 8/16/2018	CHECKED - SBC	REVISED

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

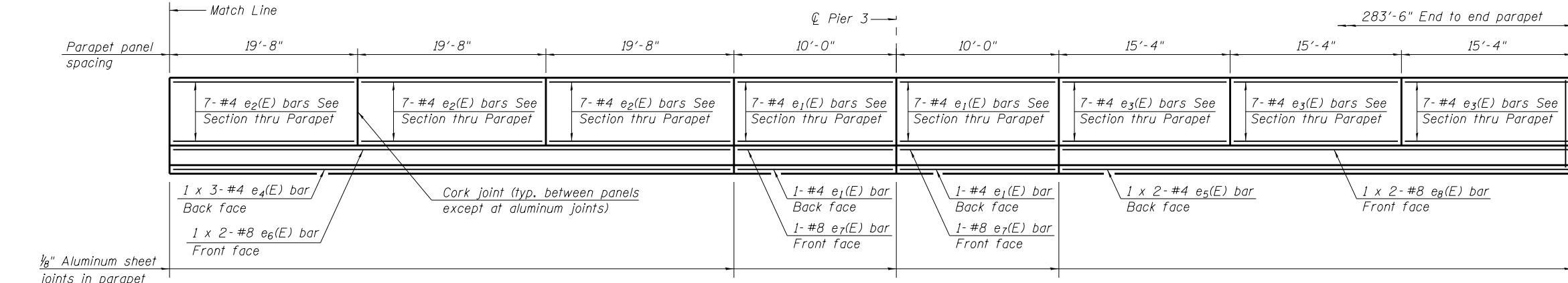
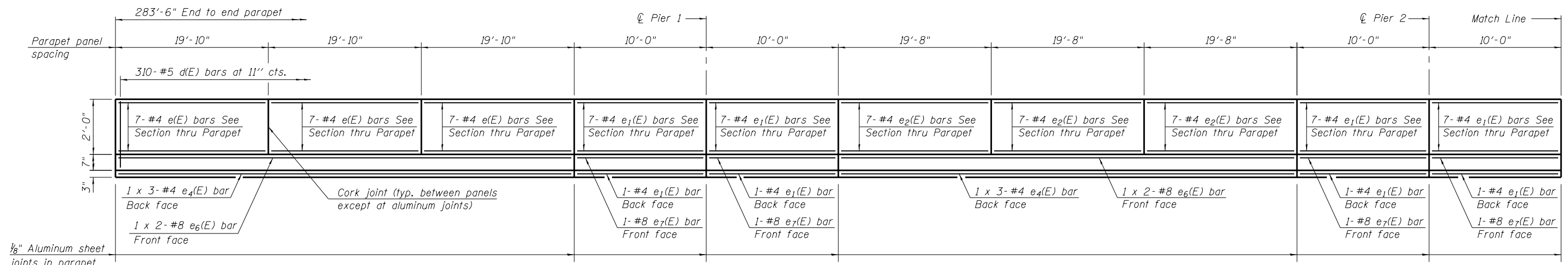
**SUPERSTRUCTURE
STRUCTURE NO. 072-0245**

SHEET NO. 17 OF 52 SHEETS

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	431
CONTRACT NO. 68185				

ILLINOIS FED. AID PROJECT

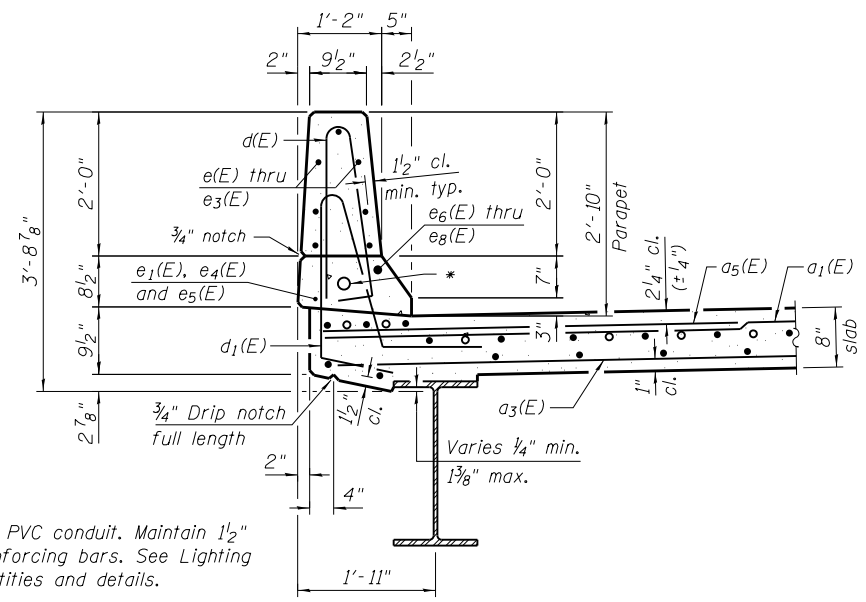
LAST SAVED DATE: 8/15/2018



INSIDE ELEVATION OF SOUTH PARAPET
(North Parapet Similar)

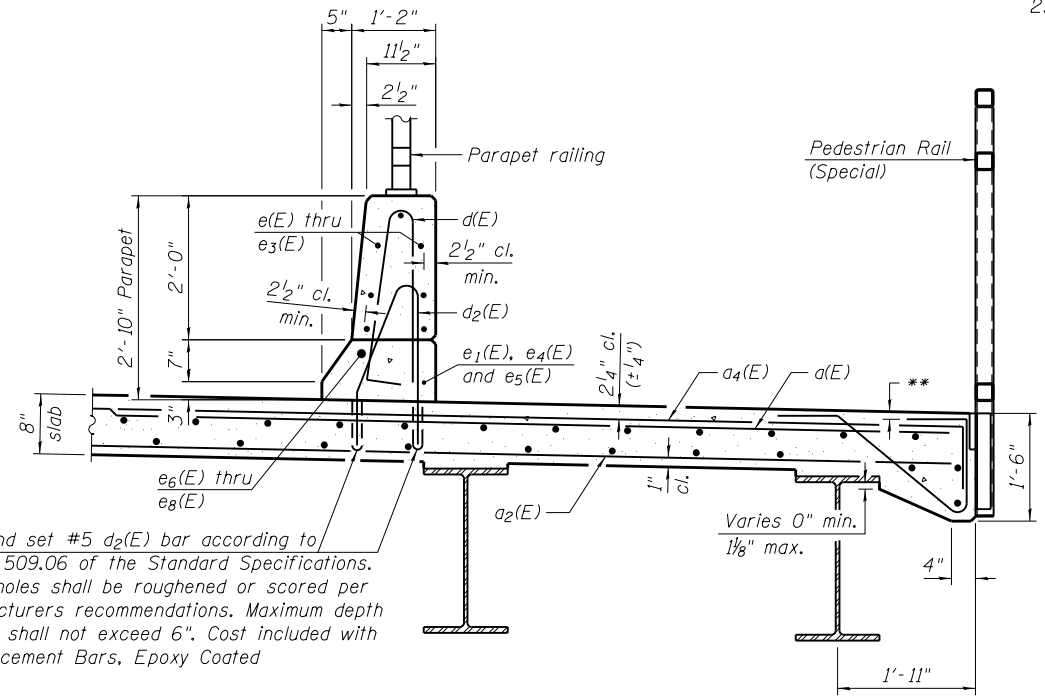
MINIMUM BAR LAP
(Parapet)
#4 bar = 2'-11"
#8 bar = 6'-4"

- Notes:
- See sheet 19 for Bill of Material.
 - Bars indicated thus 1 x 2-#8 etc. indicates 1 lines of bars with 2 lengths per line.



* Location of 2" PVC conduit. Maintain 1 1/2" clear from reinforcing bars. See Lighting Plans for quantities and details.

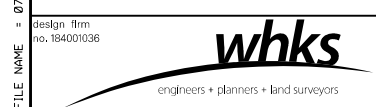
SECTION THRU SOUTH PARAPET
(Looking West)



Core and set #5 d2(E) bar according to Article 509.06 of the Standard Specifications. Cored holes shall be roughened or scored per manufacturers recommendations. Maximum depth of hole shall not exceed 6". Cost included with Reinforcement Bars, Epoxy Coated

** Reinforcement in the top of the deck may be placed with a 1 1/2" minimum clearance in the area of the rail post anchor devices. The studs of the anchor devices shall be placed below the top reinforcement bars and outermost longitudinal reinforcement bar shall be placed directly above the studs of the rail post anchor device.

SECTION THRU NORTH PARAPET AND SIDEWALK
(Looking West)



USER NAME = *OPERATOR*	DESIGNED - TJZ	REVISED
PLOT SCALE = 1/8" = 1'-0"	CHECKED - SBC	REVISED
PLOT DATE = 8/16/2018	DRAWN - DLH	REVISED
	CHECKED - SBC	REVISED

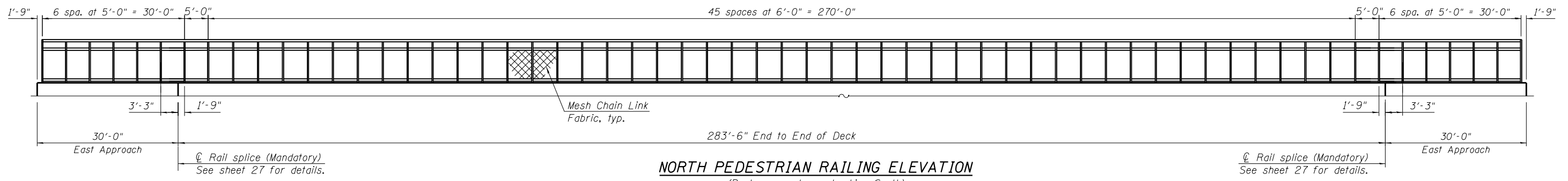
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SUPERSTRUCTURE DETAILS
STRUCTURE NO. 072-0245
SHEET NO. 18 OF 52 SHEETS

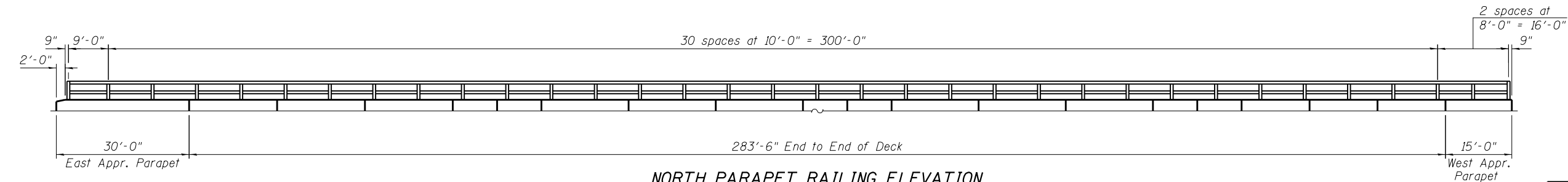
F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	432
CONTRACT NO. 68185				

ILLINOIS FED. AID PROJECT

LAST SAVED DATE: 8/15/2018



NORTH PEDESTRIAN RAILING ELEVATION
(Post space shown, Looking South)

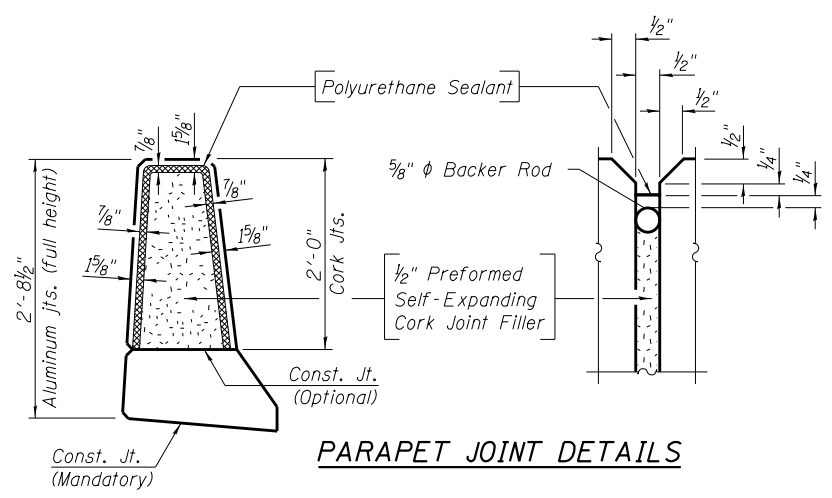


NORTH PARAPET RAILING ELEVATION
(Post spacing shown, Looking South)

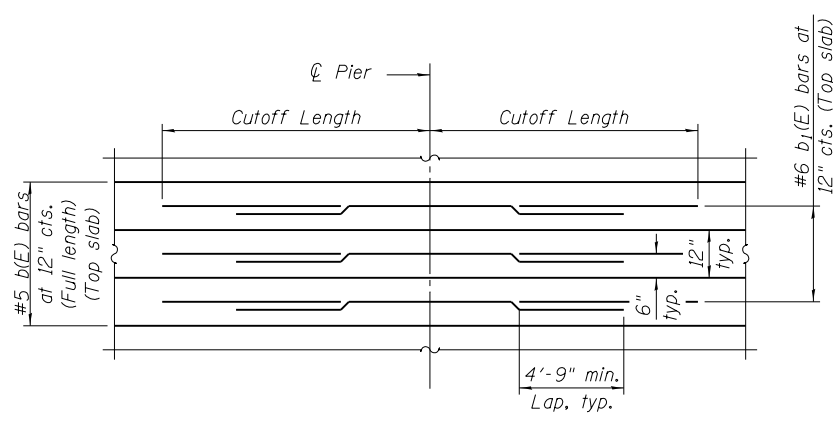
**SUPERSTRUCTURE
BILL OF MATERIAL**

Bar	No.	Size	Length	Shape
a(E)	426	#5	23'-10"	—
a ₁ (E)	426	#5	36'-0"	—
a ₂ (E)	341	#5	23'-10"	—
a ₃ (E)	341	#5	35'-7"	—
a ₄ (E)	426	#6	15'-5"	—
a ₅ (E)	426	#6	6'-6"	—
b(E)	704	#5	29'-2"	—
b ₁ (E)	549	#6	18'-10"	—
b ₂ (E)	600	#5	27'-1"	—
c(E)	285	#5	4'-10"	∩
d(E)	620	#5	5'-7"	∩
d ₁ (E)	310	#5	6'-7"	∩
d ₂ (E)	310	#5	4'-8"	∩
e(E)	42	#4	19'-7"	—
e ₁ (E)	96	#4	9'-9"	—
e ₂ (E)	84	#4	19'-5"	—
e ₃ (E)	42	#4	15'-1"	—
e ₄ (E)	18	#4	21'-10"	—
e ₅ (E)	4	#4	24'-6"	—
e ₆ (E)	12	#8	32'-11"	—
e ₇ (E)	12	#8	9'-9"	—
e ₈ (E)	4	#8	26'-2"	—
m(E)	8	#6	23'-10"	—
m ₁ (E)	8	#6	36'-2"	—
m ₂ (E)	60	#6	4'-10"	—
m ₃ (E)	12	#6	1'-7"	—
m ₄ (E)	6	#6	1'-3"	—
m ₅ (E)	6	#6	3'-3"	—
m ₆ (E)	72	#5	4'-0"	—
s(E)	120	#5	6'-4"	∩
s ₁ (E)	98	#5	8'-8"	∩
Concrete Superstructure		Cu. Yd.	548.3	
Bridge Deck Grooving		Sq. Yd.	1,749	
Protective Coat		Sq. Yd.	2,195	
Reinforcement Bars, Epoxy Coated		Pound	132,190	
Bar Splicers		Each	781	

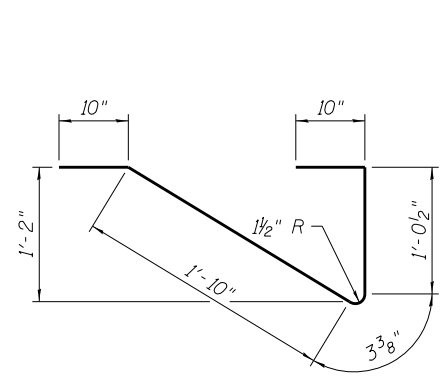
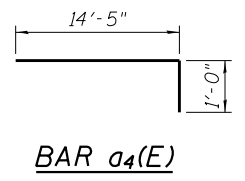
Note:
See sheet 27 and 28 for pedestrian and parapet railing details.



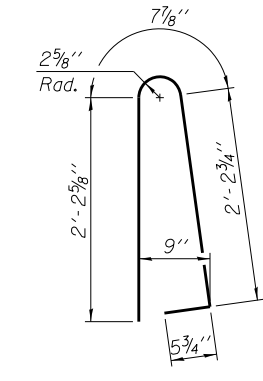
PARAPET JOINT DETAILS



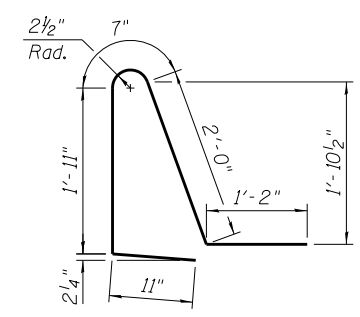
NEGATIVE MOMENT REINFORCEMENT DETAIL
(For b₁(E) bar cutoff lengths at each pier, see sheet 17.)



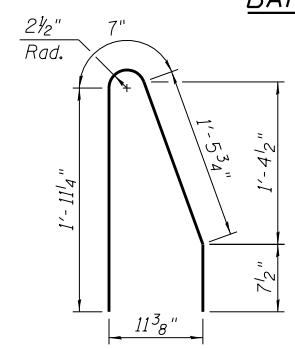
BAR c(E)



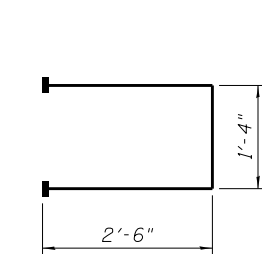
BAR d(E)



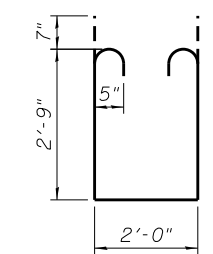
BAR d₁(E)



BAR d₂(E)



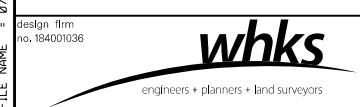
BAR s(E)
(Headed)



BAR s₁(E)

- Notes:
- The 7/8" Aluminum sheet shall be ASTM B 209 alloy 3003-H14 and coated to minimize reaction with wet concrete. Cost included with Concrete Superstructure.
 - The Polyurethane Sealant shall be non-staining gray one component non-sag elastomeric gun grade meeting the requirements of ASTM C-920, Type S, Grade NS, Class 25. Use T with a 5/8" backer rod.
 - The 1/2" Preformed Self-Expanding Cork Joint Filler shall be according to Article 1051.07 of the Std. Spec. Cost included with Concrete Superstructure.
 - Headed bars shall conform to ASTM A970 Class HA. Cost included with Reinforcement Bars, Epoxy Coated.

FILE NAME = 0720245-68185.dgn



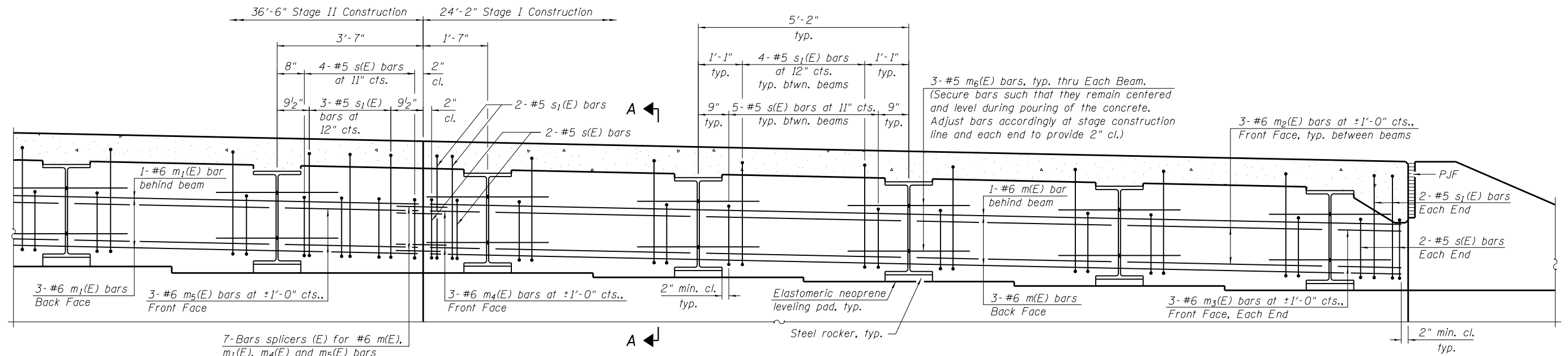
USER NAME = *OPERATOR*	DESIGNED - TJZ	REVISED
PLOT SCALE = 0.2" = 1' / in.	CHECKED - SBC	REVISED
PLOT DATE = 8/16/2018	DRAWN - DLH	REVISED
	CHECKED - SBC	REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SUPERSTRUCTURE DETAILS
STRUCTURE NO. 072-0245

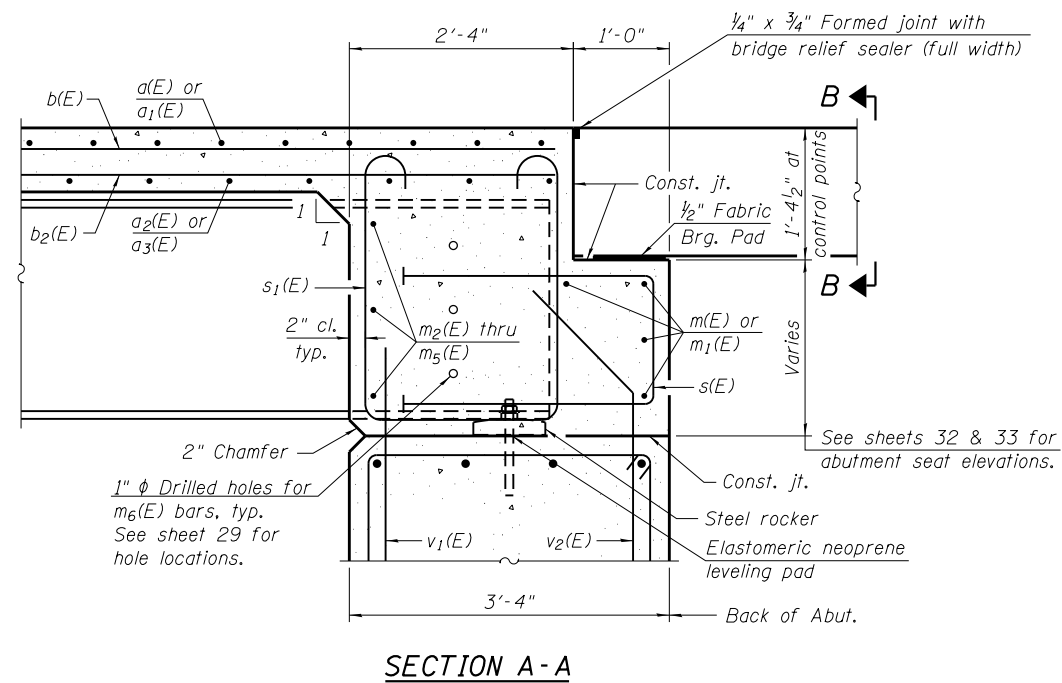
SHEET NO. 19 OF 52 SHEETS

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	433
				CONTRACT NO. 68185
ILLINOIS FED. AID PROJECT				

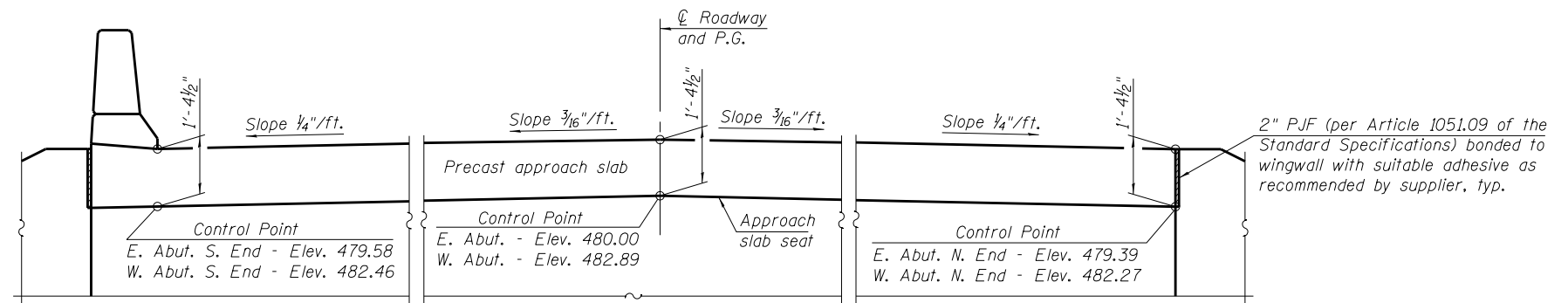


DIAPHRAGM ELEVATION

(West Abutment, looking west, shown. East Abutment, looking east, similar.)
(Parapets & Railing not shown for clarity.)

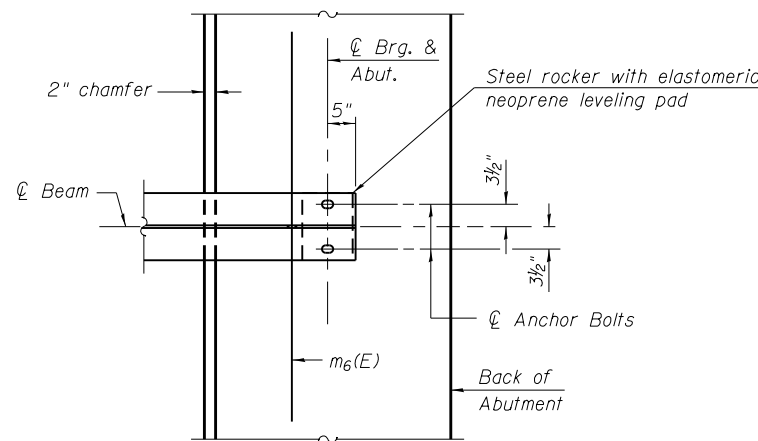


SECTION A-A



SECTION B-B

(North parapet and railing not shown for clarity.)



PARTIAL PLAN AT ABUTMENT
(Showing bottom flange of beam)

Notes:

1. Reinforcement bars in diaphragm are billed with superstructure on sheet 19.
2. Concrete in diaphragm is included with Concrete Superstructure on sheet 19.
3. For details of bars s(E) and s1(E) see sheet 19.
4. The approach slab seat shall have a constant slope determined from the control points shown.
5. For bearing details see sheet 31.

USER NAME = *OPERATOR*	DESIGNED - TJZ	REVISED
	CHECKED - SBC	REVISED
PLOT SCALE = 0.2" = 1' - 0"	DRAWN - DLH	REVISED
PLOT DATE = 8/16/2018	CHECKED - SBC	REVISED

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	434
CONTRACT NO. 68185				

LAST SAVED DATE: 8/15/2018

FILE NAME = 0720245-68185.dgn



DESIGN FIRM no. 184001036	USER NAME = *OPERATOR*	DESIGNED - SBC	REVISED
		CHECKED - TJZ	REVISED
	PLOT SCALE = 0.2" = 1' / 16"	DRAWN - DLH	REVISED
	PLOT DATE = 8/16/2018	CHECKED - TJZ	REVISED

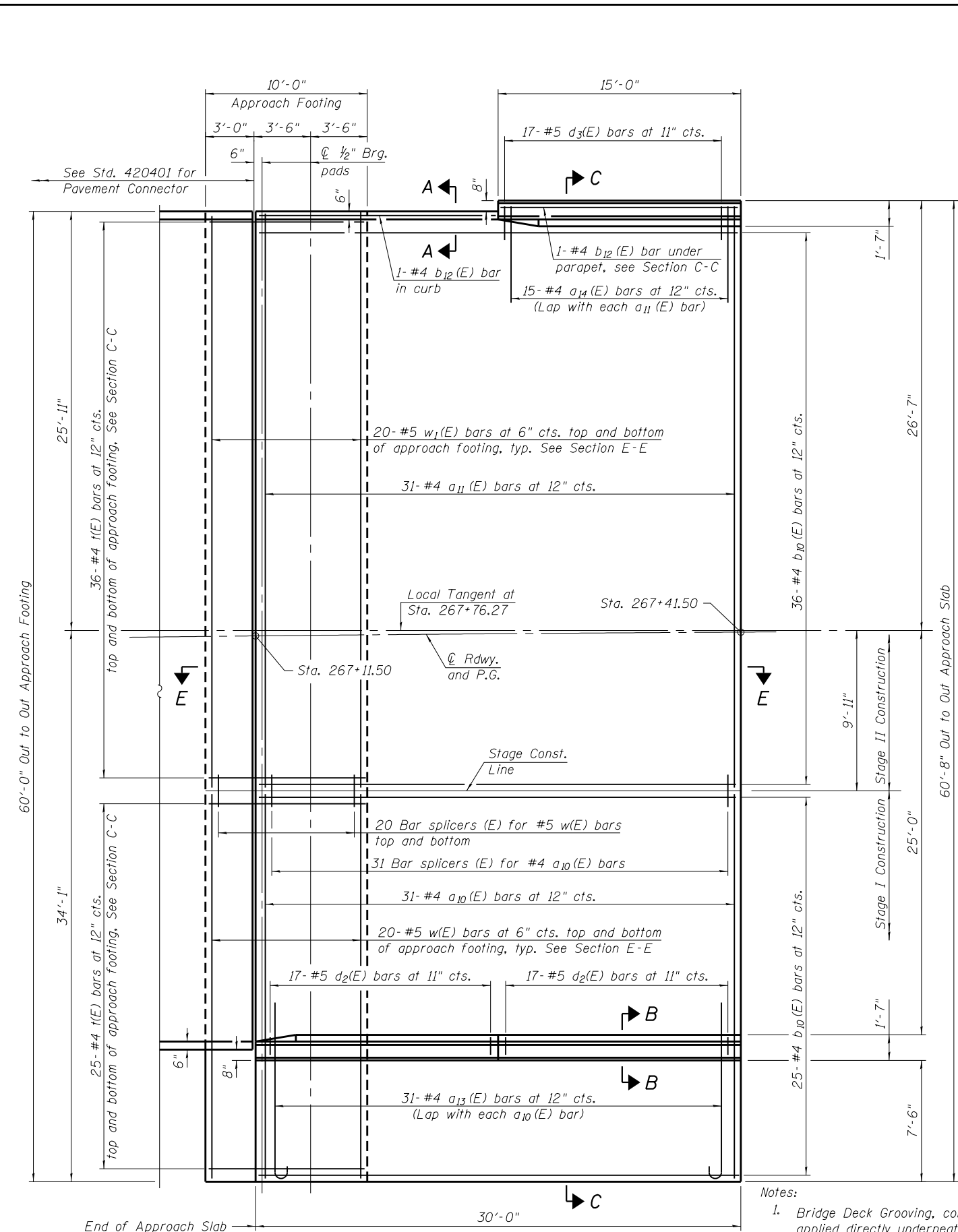
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**PRECAST BRIDGE APPROACH SLAB PLAN
STRUCTURE NO. 072-0245**

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	435
CONTRACT NO. 68185				

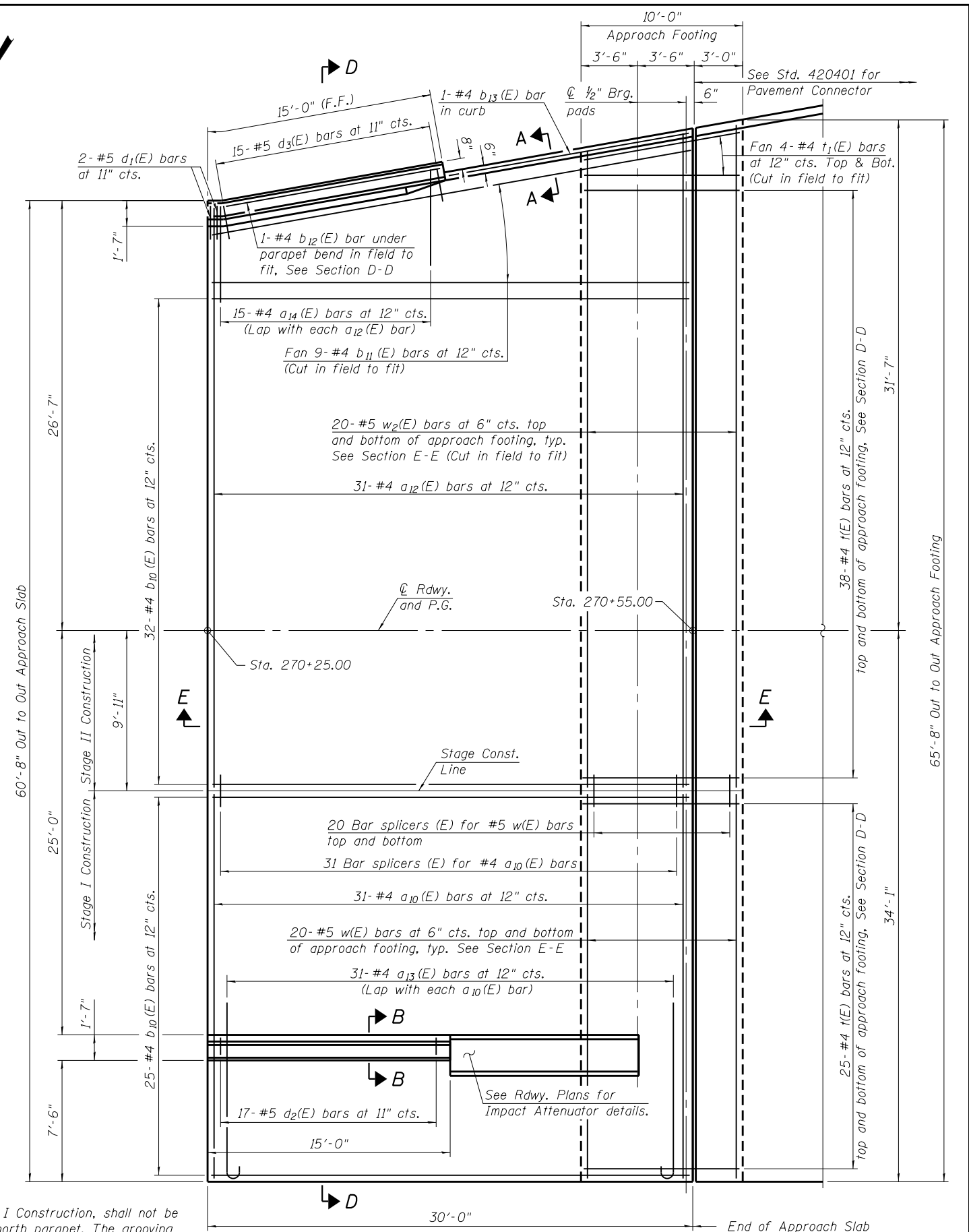
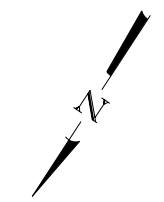
SHEET NO. 21 OF 52 SHEETS

ILLINOIS FED. AID PROJECT



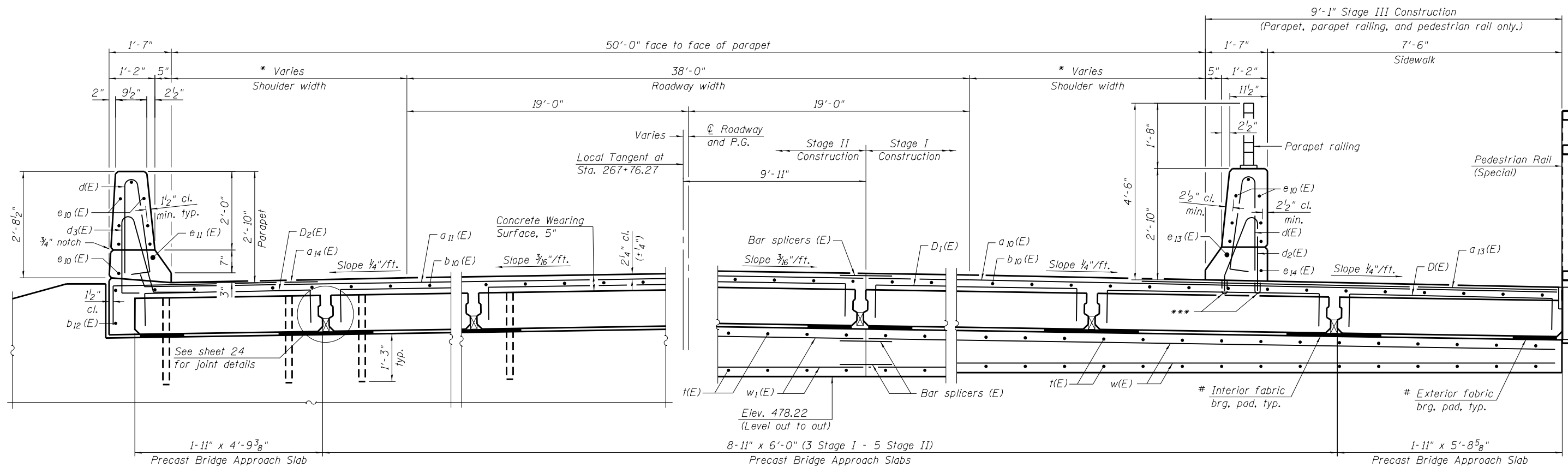
PLAN - EAST APPROACH
(Showing wearing surface)

- Notes:
- Bridge Deck Grooving, completed in Stage I Construction, shall not be applied directly underneath the proposed north parapet. The grooving shall be completed in accordance with Article 503.16(a)(3) of the Standard Specifications.
 - Work this sheet with sheets 22 thru 26.

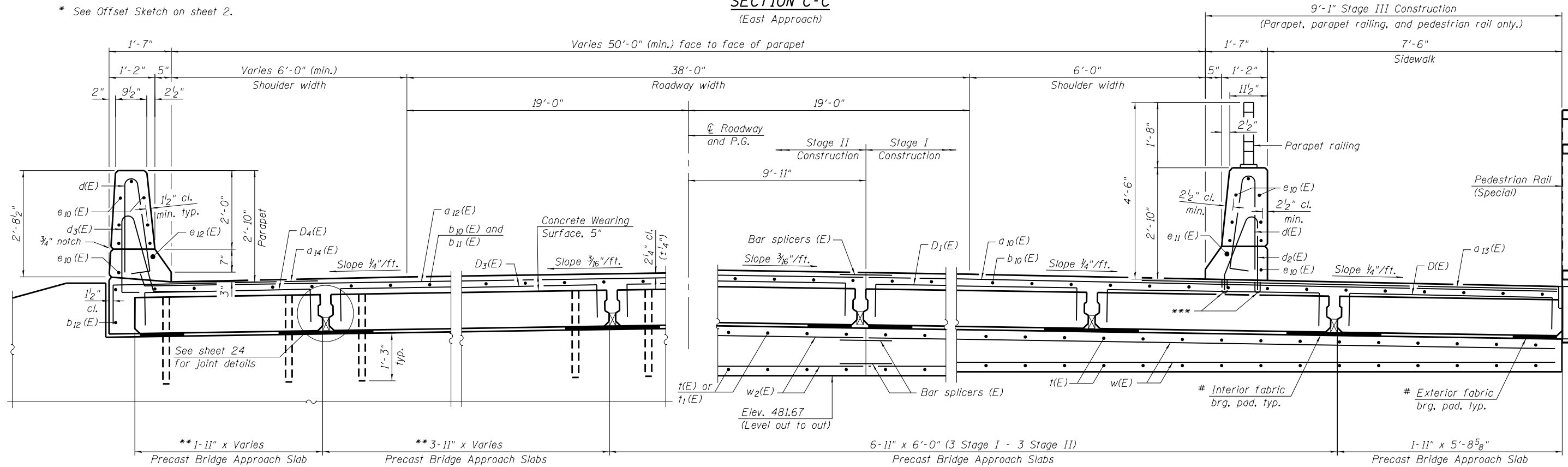


PLAN - WEST APPROACH
(Showing wearing surface)

LAST SAVED DATE: 8/15/2018



SECTION C-C
(East Approach)

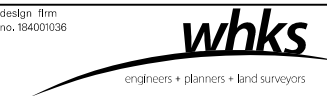


SECTION D-D
(West Approach)

Fabric bearing pads at the expansion end shall be recessed 1/4" into the approach footing and bonded. Adjusting shims, when required, shall be bonded to the top of the fabric bearing pads.

Note: Work this sheet with sheets 21 and 23 thru 26.

FILE NAME = 0720245-68185.dgn



USER NAME = *OPERATOR*	DESIGNED - SBC	REVISED
PLOT SCALE = 0.2" = 1'-0"	CHECKED - TJZ	REVISED
PLOT DATE = 8/16/2018	DRAWN - DLH	REVISED
	CHECKED - TJZ	REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

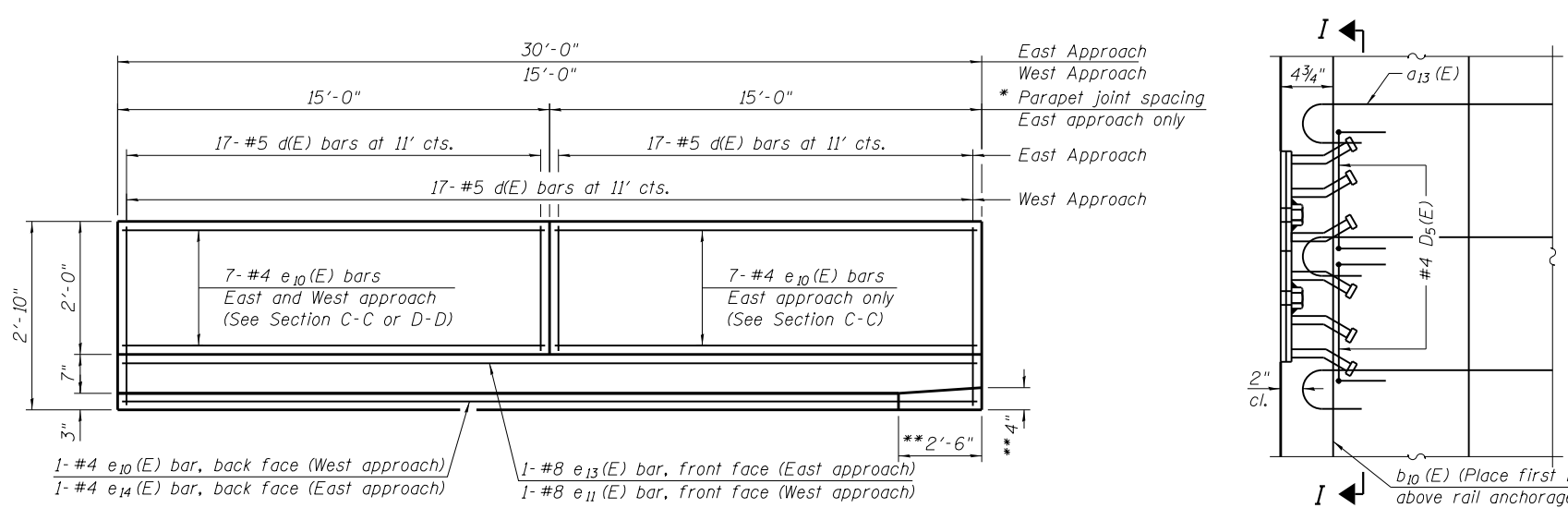
PRECAST BRIDGE APPROACH SLAB DETAILS
STRUCTURE NO. 072-0245

SHEET NO. 22 OF 52 SHEETS

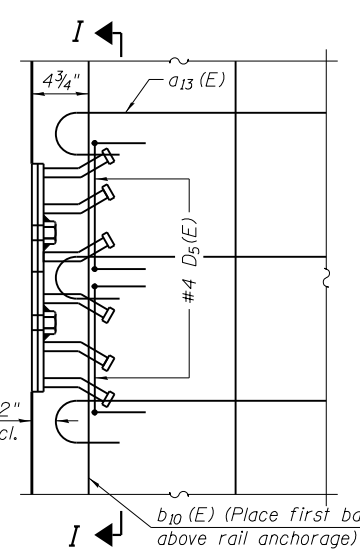
F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	436
CONTRACT NO. 68185				

ILLINOIS FED. AID PROJECT

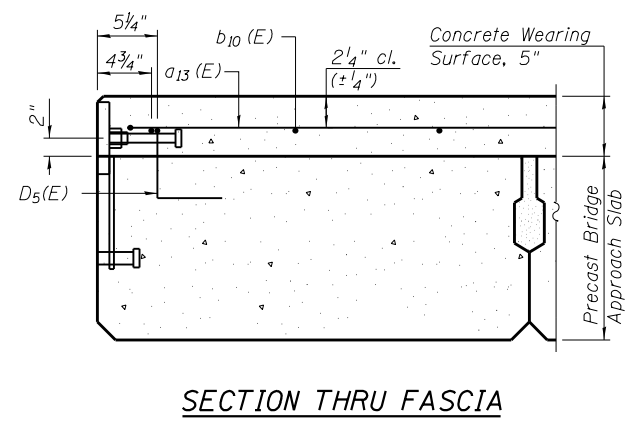
LAST SAVED DATE: 8/15/2018



INSIDE ELEVATION OF NORTH PARAPET
(Parapet rail not shown for clarity.)
* See sheet 19 for parapet joint details.
** East approach only.

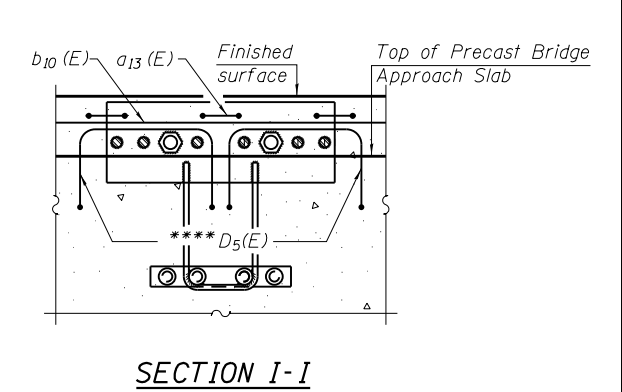


PLAN



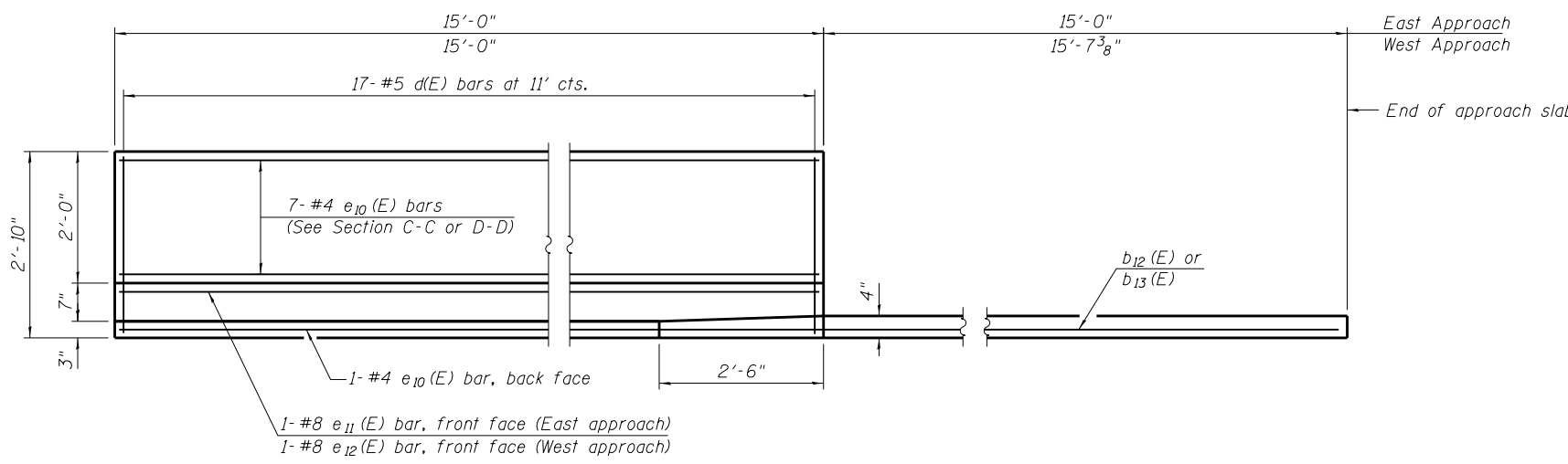
SECTION THRU FASCIA

Note:
Formwork necessary for the wearing surface may be secured utilizing the bottom rail anchorage inserts and/or additional inserts cast into the Precast Bridge Approach Slab. See sheet 8 for rail anchor device notes and details.

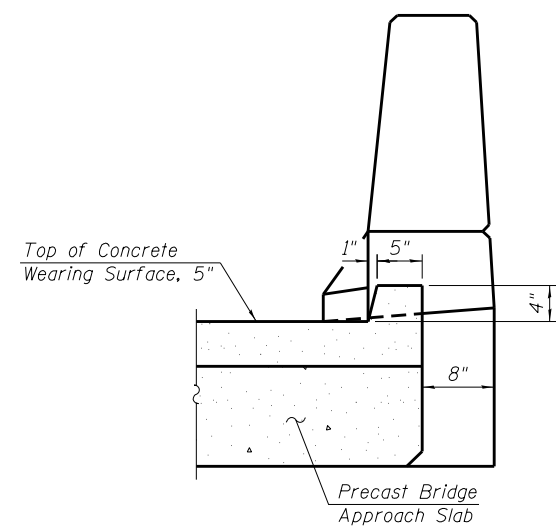


SECTION I-I

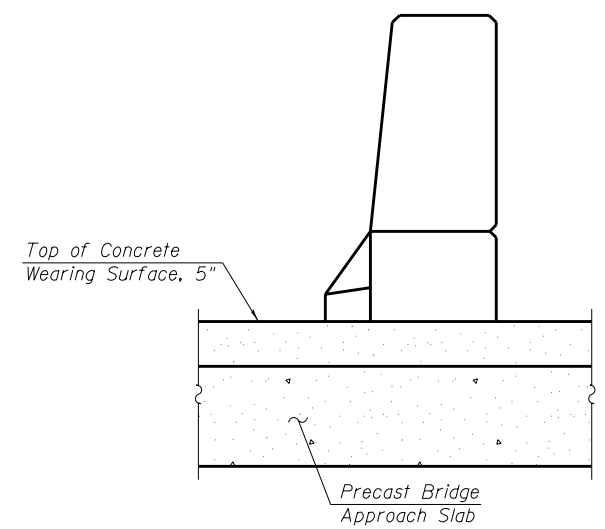
**** Place 2-#4 D5(E) bars in beam at each post location as shown. D5(E) bar included in cost of Precast Bridge Approach Slab.



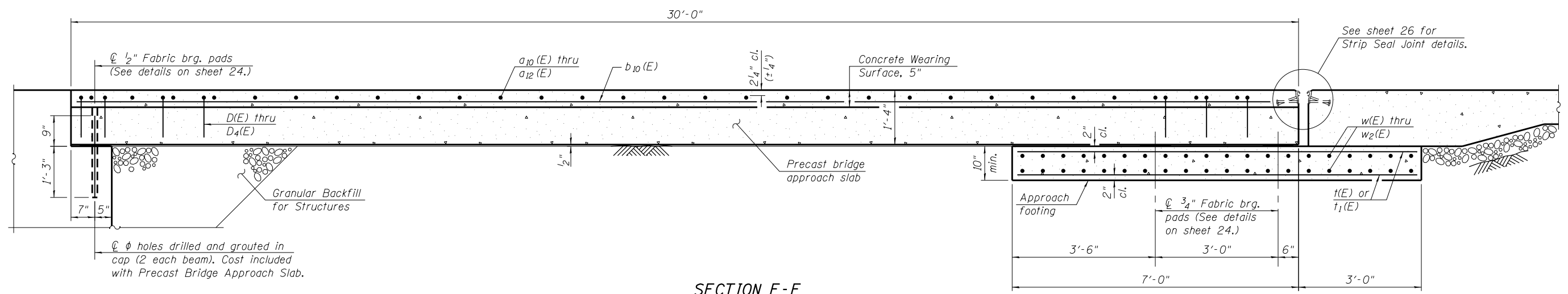
INSIDE ELEVATION OF SOUTH PARAPET AND CURB



SECTION A-A



SECTION B-B



SECTION E-E

Notes:
1. Work this sheet with sheets 21, 22 and 24 thru 26.
2. The approach footing Maximum Applied Service Bearing Pressure (Q_{max}) = 2.0 ksf.

FILE NAME = 0720245-68185.dgn



USER NAME = *OPERATOR*	DESIGNED - SBC	REVISED
PLOT SCALE = 0.2" = 1'-0"	CHECKED - TJZ	REVISED
PLOT DATE = 8/16/2018	DRAWN - DLH	REVISED
	CHECKED - TJZ	REVISED

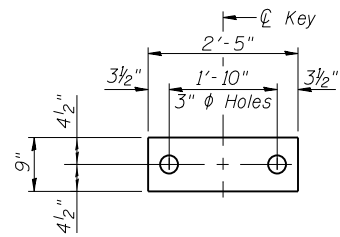
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

PRECAST BRIDGE APPROACH SLAB DETAILS
STRUCTURE NO. 072-0245

SHEET NO. 23 OF 52 SHEETS

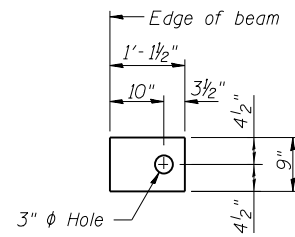
F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	437
CONTRACT NO. 68185				

ILLINOIS FED. AID PROJECT

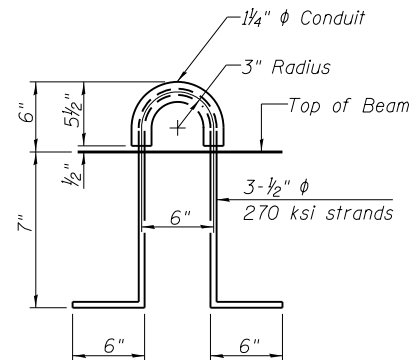


INTERIOR

FABRIC BEARING PAD



EXTERIOR



LIFTING LOOP DETAIL

(An alternate lifting loop with a proof load of 25,000 lbs. and utilized according to the Manufacturer's recommendations may be used.)

BAR LIST
6'-0" INTERIOR BEAM
(For information only)

Bar	No.	Size	Length	Shape
B(E)	7	#5	29'-8"	—
B ₁ (E)	14	#9	29'-8"	—
* D ₁ (E)	22	#4	7'-7"	┌
S ₁ (E)	60	#5	13'-6"	▭

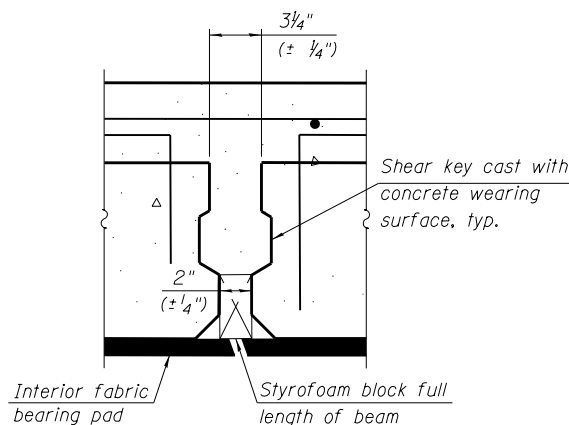
* Provide 41-D₁(E) bars for each north interior beam only.

BAR LIST
5'-8 5/8" EXTERIOR BEAM
(For information only)

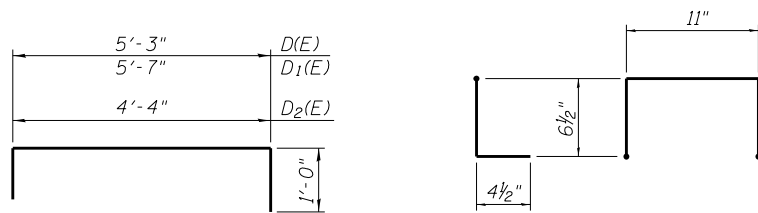
Bar	No.	Size	Length	Shape
B(E)	6	#5	29'-8"	—
B ₁ (E)	14	#9	29'-8"	—
D(E)	22	#4	7'-3"	┌
D ₅ (E)	12	#4	2'-9"	┌
S(E)	60	#5	12'-11"	▭

BAR LIST
4'-9 3/8" EXTERIOR BEAM
(For information only)

Bar	No.	Size	Length	Shape
B(E)	5	#5	29'-8"	—
B ₁ (E)	12	#9	29'-8"	—
D ₂ (E)	41	#4	6'-4"	┌
S ₂ (E)	60	#5	11'-1"	▭

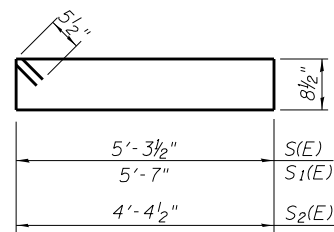


SECTION THRU SHEAR KEY JOINT

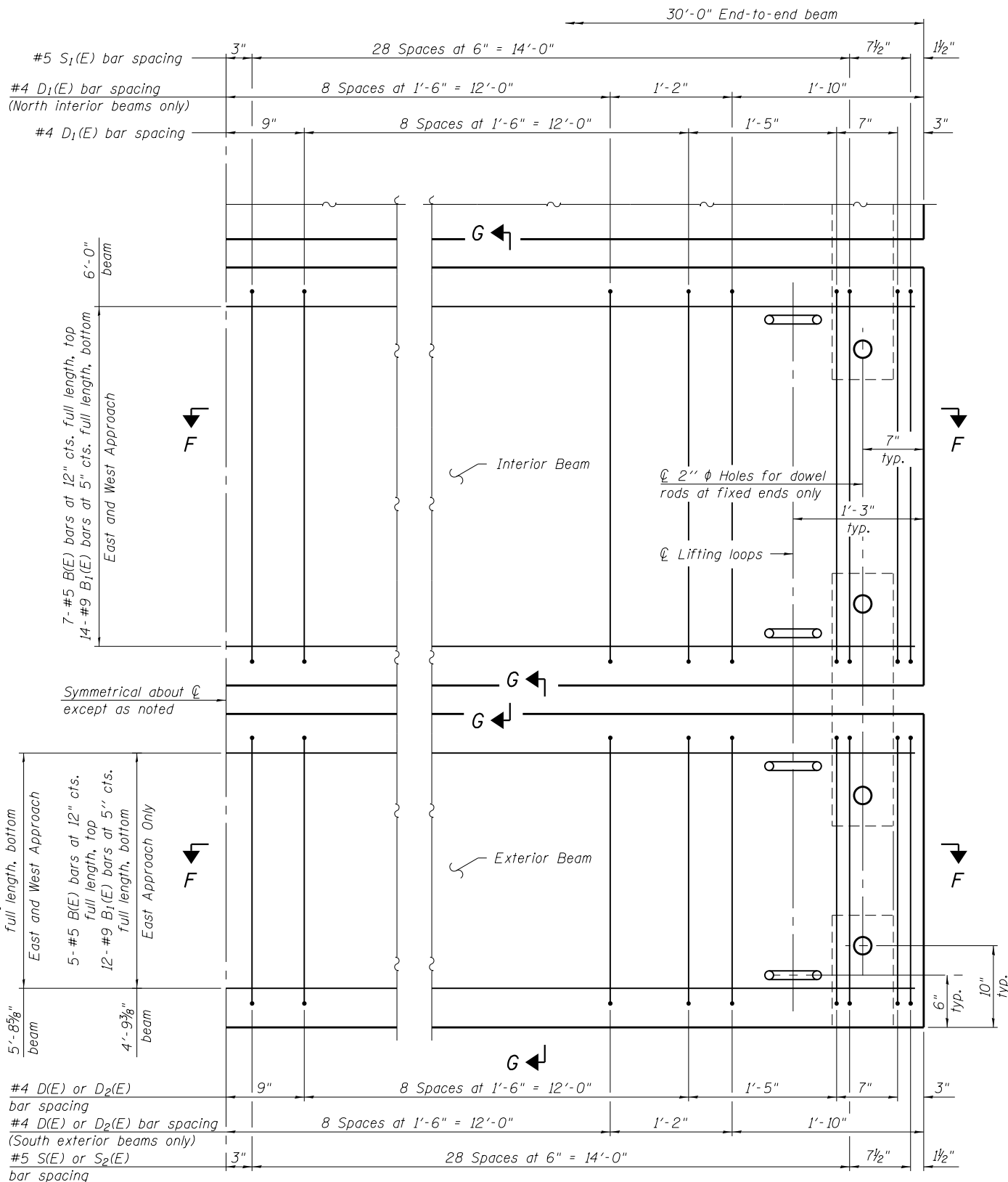


BAR D₅(E)

BARS D(E), D₁(E) & D₂(E)



BARS S(E), S₁(E) & S₂(E)



PLAN

(East approach shown. West approach similar except as noted.)
(See sheet 25 for West approach flared Precast Bridge Approach Slabs not shown.)

- Notes:
- Bearing pads at fixed end shall be 1/2" thick and bearing pads at expansion end shall be 3/4" thick.
 - Omit holes for fabric bearing pads at approach slab footing end of beams.
 - Work this sheet with sheets 21 thru 23, 25, and 26.

BAR LIST
FLARED EXTERIOR BEAM
(For information only)

Bar	No.	Size	Length	Shape
B ₆ (E)	6	#5	30'-1"	—
B ₇ (E)	13	#9	30'-1"	—
D ₄ (E)	82	#4	4'-5"	┌
S ₄ (E)	120	#5	7'-8½"	⊔

BAR LIST
FLARED INTERIOR BEAM A
(For information only)

Bar	No.	Size	Length	Shape
B ₄ (E)	6	#5	29'-11"	—
B ₅ (E)	13	#9	29'-11"	—
D ₃ (E)	44	#4	4'-7"	┌
S ₃ (E)	120	#5	8'-0½"	⊔

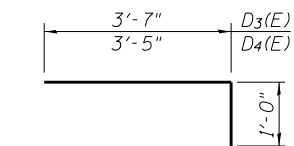
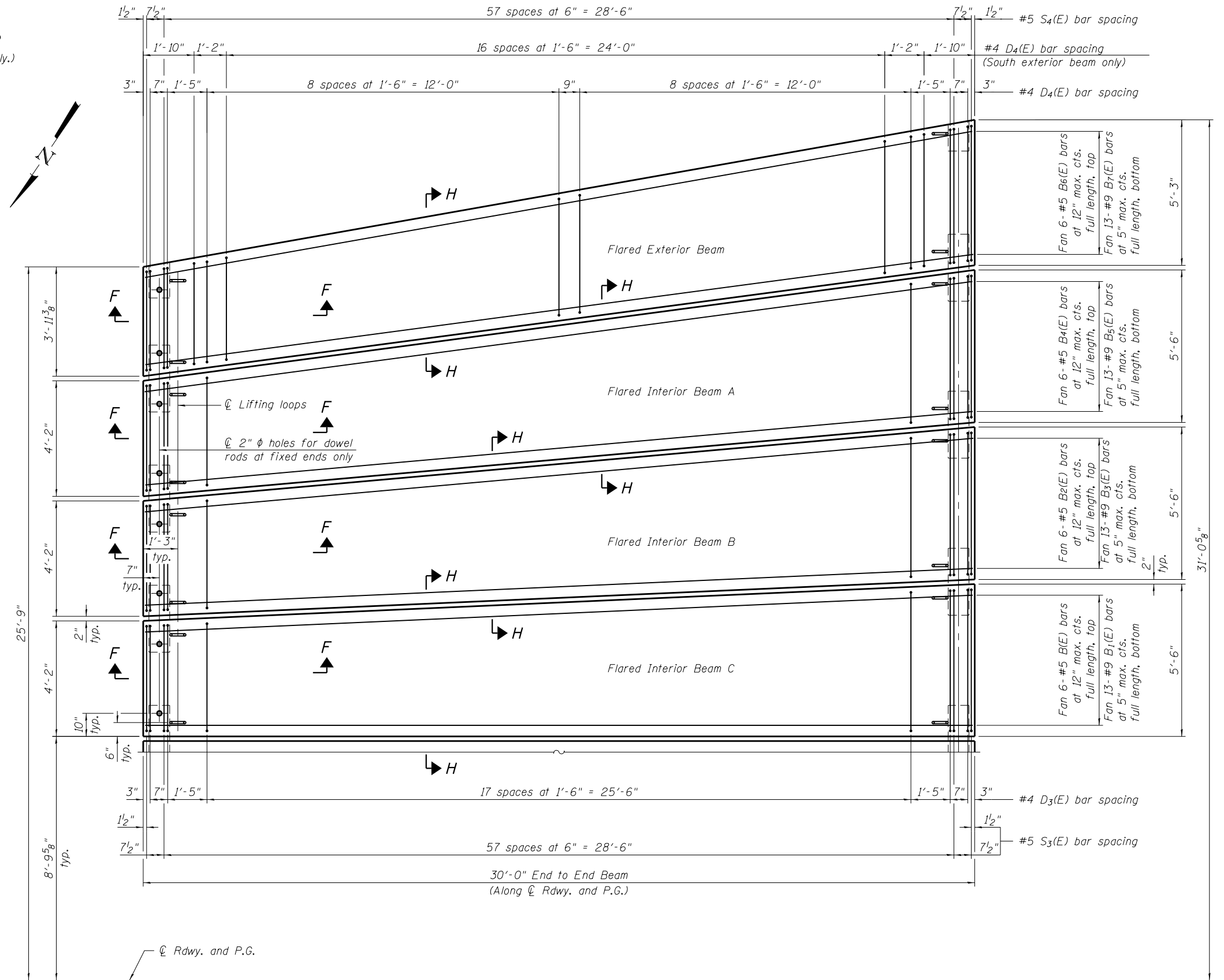
BAR LIST
FLARED INTERIOR BEAM B
(For information only)

Bar	No.	Size	Length	Shape
B ₂ (E)	6	#5	29'-9"	—
B ₃ (E)	13	#9	29'-9"	—
D ₃ (E)	44	#4	4'-7"	┌
S ₃ (E)	120	#5	8'-0½"	⊔

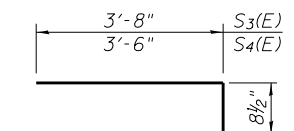
BAR LIST
FLARED INTERIOR BEAM C
(For information only)

Bar	No.	Size	Length	Shape
B(E)	6	#5	29'-8"	—
B ₁ (E)	13	#9	29'-8"	—
D ₃ (E)	44	#4	4'-7"	┌
S ₃ (E)	120	#5	8'-0½"	⊔

MINIMUM BAR LAP
(For D₃, D₄, S₃, S₄ bars only.)
#4 bar = 2'-0"
#5 bar = 2'-3"



BARS D₃(E) & D₄(E)

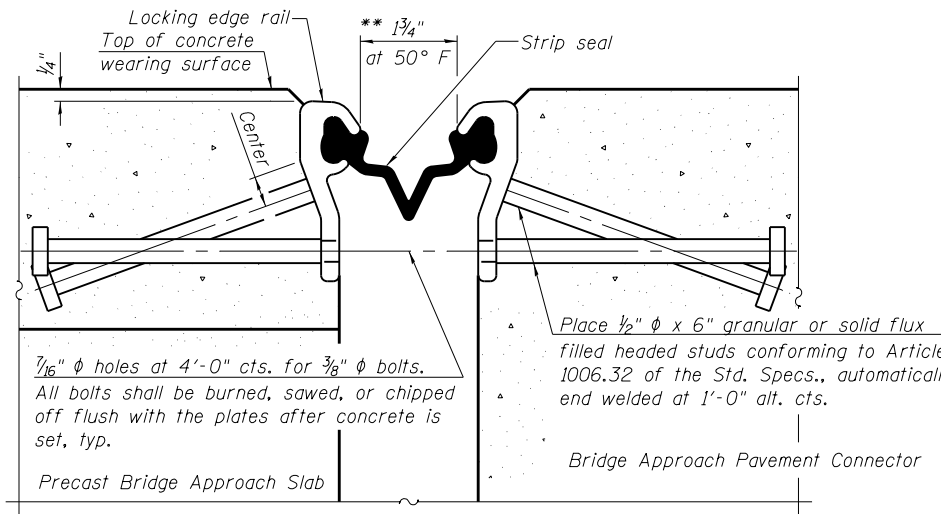


BARS S₃(E) & S₄(E)

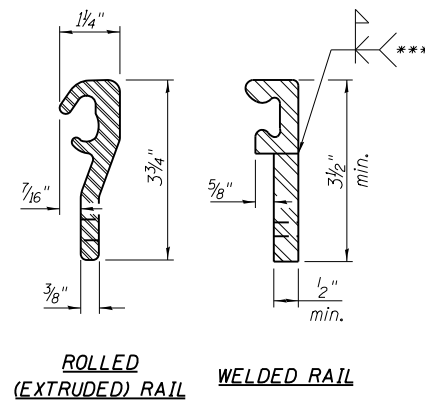
Note:
Work this sheet with sheets 21 thru 24 and 26.

PLAN
(West approach flared bridge approach slabs shown.)

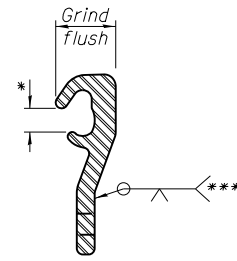
LAST SAVED DATE: 8/15/2018



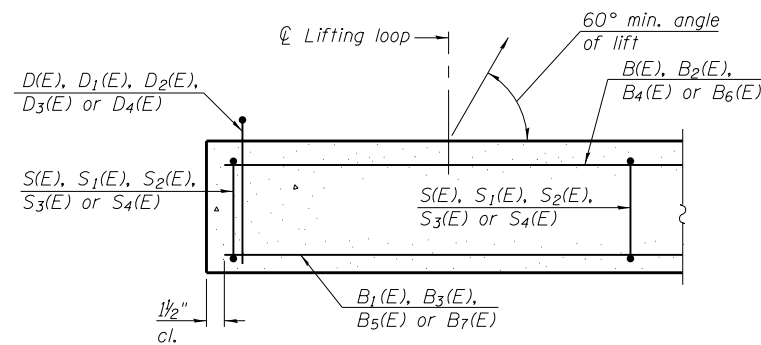
SECTION THRU STRIP SEAL JOINT



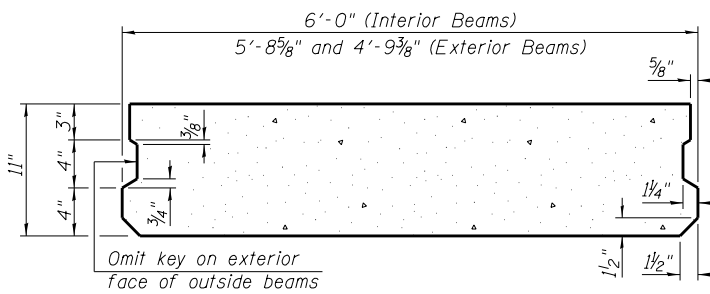
LOCKING EDGE RAIL



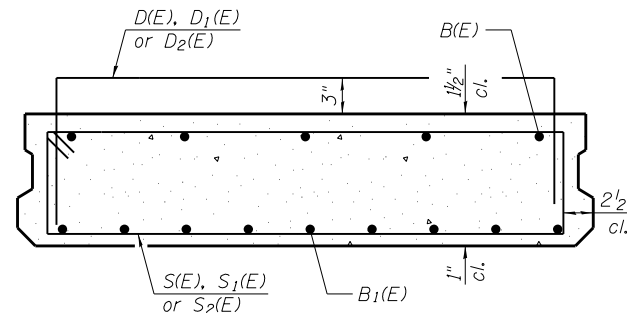
LOCKING EDGE RAIL SPLICE
Rolled rail shown, welded rail similar.



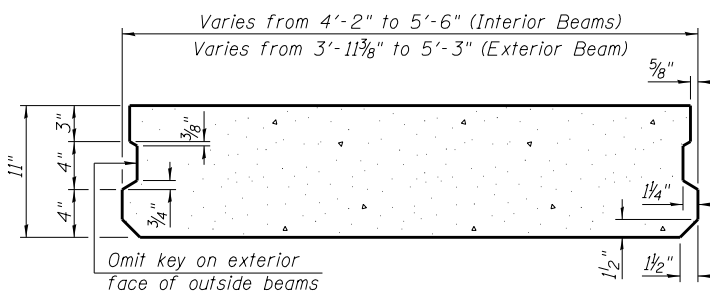
SECTION F-F



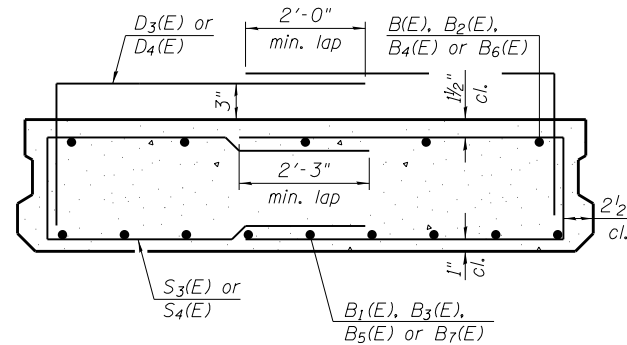
SECTION G-G
(Showing dimensions)



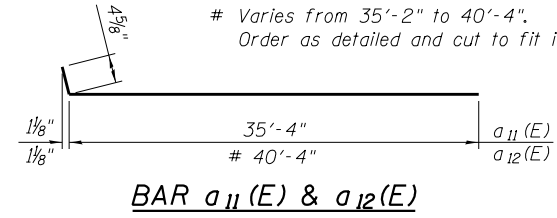
SECTION G-G
(Showing reinforcement)



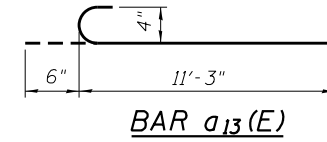
SECTION H-H
(Showing dimensions)



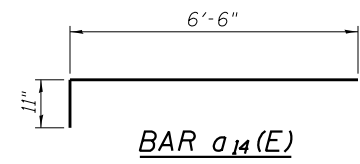
SECTION H-H
(Showing reinforcement)



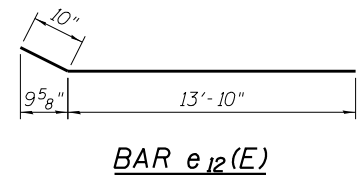
BAR a₁₁(E) & a₁₂(E)



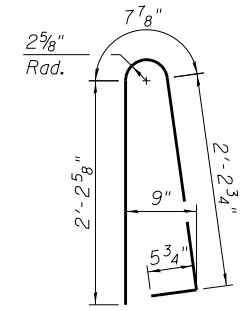
BAR a₁₃(E)



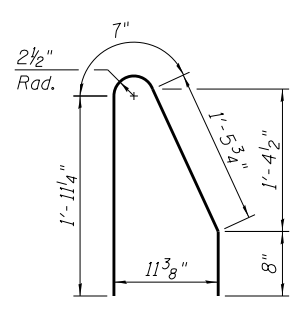
BAR a₁₄(E)



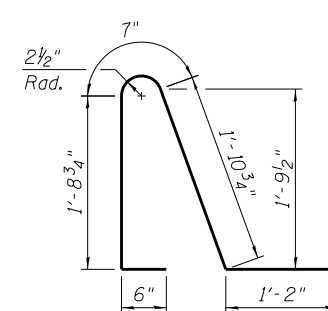
BAR e₁₂(E)



BAR d(E)



BAR d₂(E)



BAR d₃(E)

- * Omit weld at seal opening.
- ** The joint opening shall be adjusted for temperature per Article 520.04 of The Standard Specifications. However, since this detail is for jointless structures, the length of the bridge used to calculate the adjustment shall be equal to half the total bridge length plus the length of the bridge approach slab.
- *** Back gouge not required if complete joint penetration is verified by mock-up.

Notes:

1. The precast bridge approach slab shall be according to Section 504 of the Standard Specifications and shall be paid for at the contract unit price per square foot for Precast Bridge Approach Slab.
2. Cast-in-place substitution of Precast Bridge Approach Slab is not allowed.
3. Parapet concrete shall be paid for as Concrete Superstructure.
4. Approach footing concrete shall be paid for as Concrete Structures.
5. Cost of excavation for approach footing included with Concrete Structures.
6. For Granular Backfill for Structures and Drainage Treatment Details, see sheet 2.
7. The top surface of precast bridge approach slabs shall be finish similar to precast prestressed concrete deck beams with concrete wearing surface as specified in the IDOT "Manual for Fabrication of Precast Prestressed Concrete Products."
8. After precast bridge approach slabs have been erected, holes shall be drilled into abutment and anchor dowels placed. Dowel holes shall be filled with non-shrink grout to top of precast slab and cured according to Article 1020.13(a)(3) or 1020.13(a)(5) of the Standard Specifications for a minimum of 24 hours before casting the shear keys and wearing surface.
9. Two 1/8 inch fabric adjusting shims of the dimensions of the exterior bearing pad shall be provided for each bearing pad location. Cost included with Precast Bridge Approach Slab.
10. A minimum 2 - 1/2 inch diameter lifting pins shall be used to engage the lifting loops during handling.
11. Compressive strength of precast concrete, f'c shall be 6,000 psi.
12. Compressive strength of precast concrete during initial lifting, f'ci shall be 5,000 psi.
13. For additional parapet details, see sheet 17 thru 19.
14. Any concrete poured monolithically with the wearing surface, such as curbs, shall not be paid for separately, but will be included in the cost of Concrete Wearing Surface, 5".
15. The shall extend 6" beyond the edge of the approach slab on each end. The configuration of the strip seal shall match the configuration of the Locking Edge Rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated measurement of 4".
16. Locking Edge Rails may be spliced at slope discontinuities and stage construction joints. The locking edge rails depicted are conceptual only, except for the minimum dimensions shown. The actual configuration of the locking edge rails and matching strip seal may vary from manufacturer to manufacturer. Flange edge rails will not be allowed.
17. The manufacturer's recommended installation methods shall be followed.
18. All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.
19. Maximum Joints in rails within 10' of curbs shall be welded.
20. Work this sheet with sheets 21 thru 25.

**TWO APPROACHES
BILL OF MATERIAL**

Bar	No.	Size	Length	Shape
a ₁₀ (E)	62	#4	23'-10"	—
a ₁₁ (E)	31	#4	35'-9"	—
a ₁₂ (E)	31	#4	40'-9"	—
a ₁₃ (E)	62	#4	11'-9"	U
a ₁₄ (E)	30	#4	7'-5"	—
b ₁₀ (E)	118	#4	29'-8"	—
b ₁₁ (E)	9	#4	30'-2"	—
b ₁₂ (E)	3	#4	14'-8"	—
b ₁₃ (E)	1	#4	15'-3"	—
d(E)	85	#5	5'-7"	U
d ₂ (E)	51	#5	4'-8"	U
d ₃ (E)	34	#5	5'-10 1/2"	U
e ₁₀ (E)	38	#4	14'-8"	—
e ₁₁ (E)	2	#8	14'-8"	—
e ₁₂ (E)	1	#8	14'-8"	—
e ₁₃ (E)	1	#8	29'-8"	—
e ₁₄ (E)	1	#4	29'-8"	—
t(E)	248	#4	9'-8"	—
t ₁ (E)	8	#4	9'-10"	—
w(E)	80	#5	23'-10"	—
w ₁ (E)	40	#5	35'-6"	—
w ₂ (E)	40	#5	41'-2"	—
Concrete Structures		Cu. Yd.	53.3	
Concrete Superstructure		Cu. Yd.	9.4	
Bridge Deck Grooving		Sq. Yd.	383	
Protective Coat		Sq. Yd.	453	
Precast Bridge Approach Slab		Sq. Ft.	3,675	
Concrete Wearing Surface, 5"		Sq. Yd.	410.6	
Reinforcement Bars, Epoxy Coated		Pound	14,150	
Bar Splicers		Each	142	
Preformed Joint Strip Seal		Foot	119	



USER NAME = *OPERATOR*	DESIGNED - SBC	REVISED
PLOT SCALE = 0.2" = 1' / 16"	CHECKED - TJZ	REVISED
PLOT DATE = 8/16/2018	DRAWN - DLH	REVISED
	CHECKED - TJZ	REVISED

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

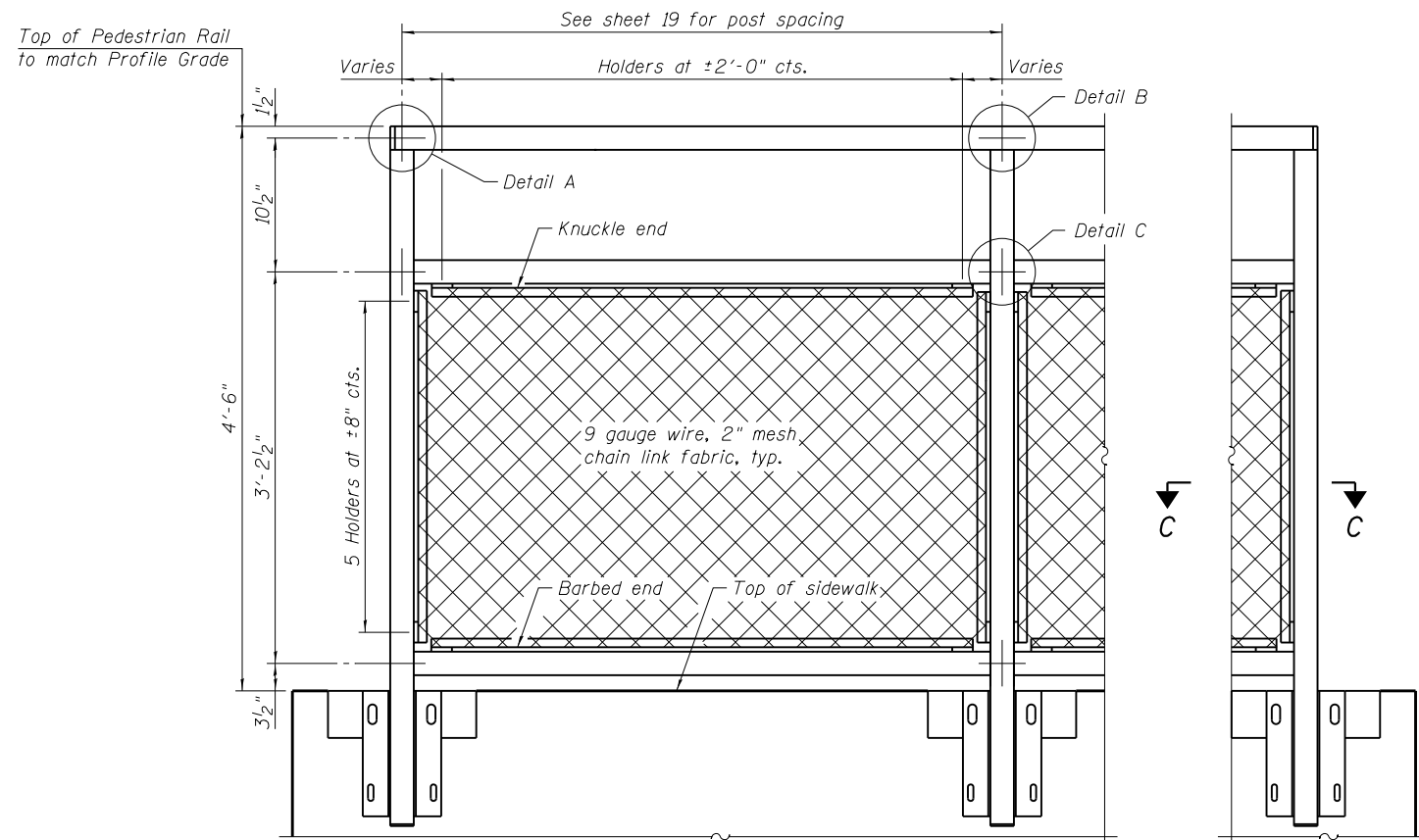
**PRECAST BRIDGE APPROACH SLAB DETAILS
STRUCTURE NO. 072-0245**

F.A.U. RTE. 6659	SECTION 11(N, BR-1, RS-4, W-1)	COUNTY PEORIA	TOTAL SHEETS 577	SHEET NO. 440
				CONTRACT NO. 68185

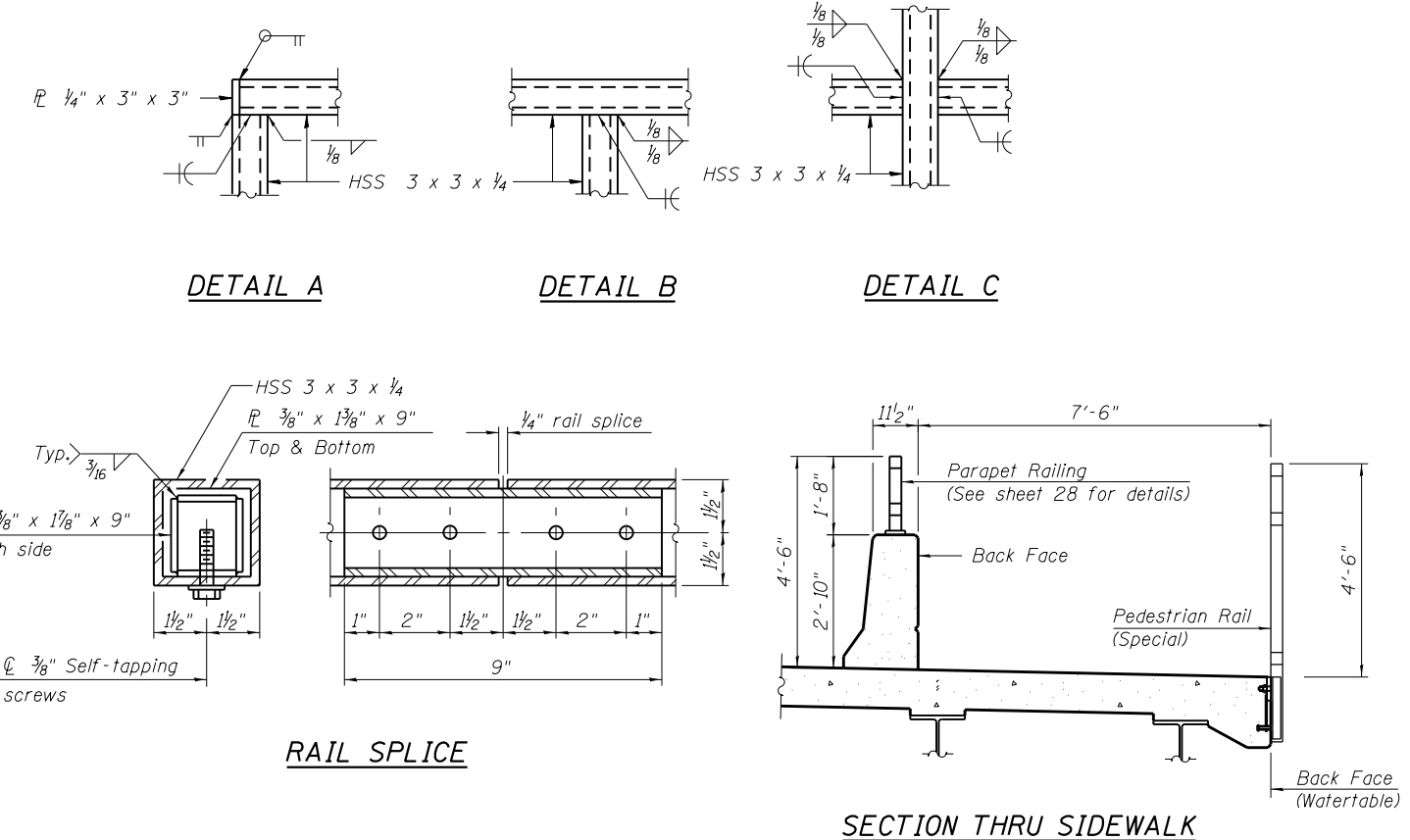
SHEET NO. 26 OF 52 SHEETS

ILLINOIS FED. AID PROJECT

LAST SAVED DATE: 8/15/2018



NORTH PEDESTRIAN RAIL PARTIAL ELEVATION



DETAIL A

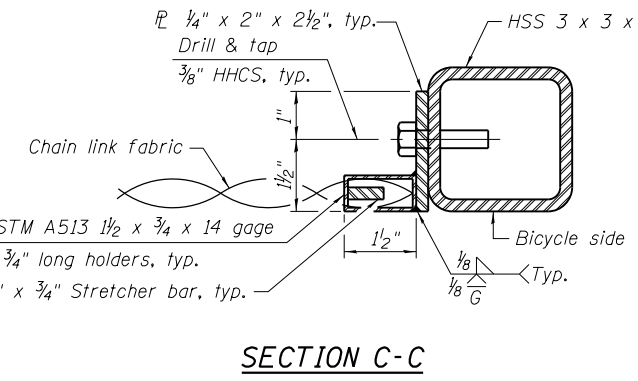
DETAIL B

DETAIL C

RAIL SPLICE

SECTION THRU SIDEWALK

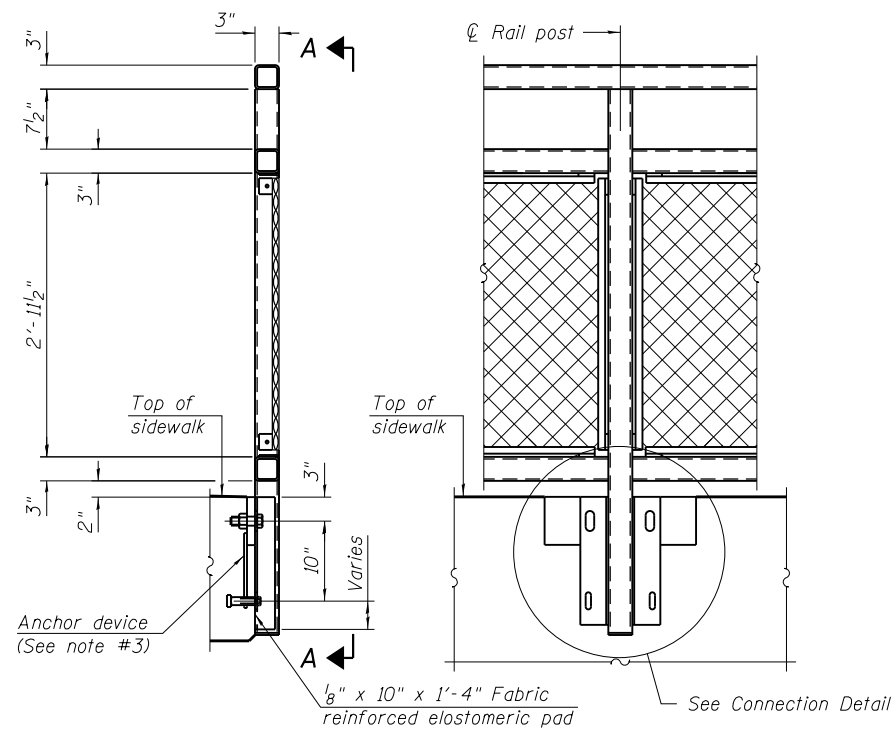
(Bridge deck shown. Bridge approach slab similar.)



SECTION C-C

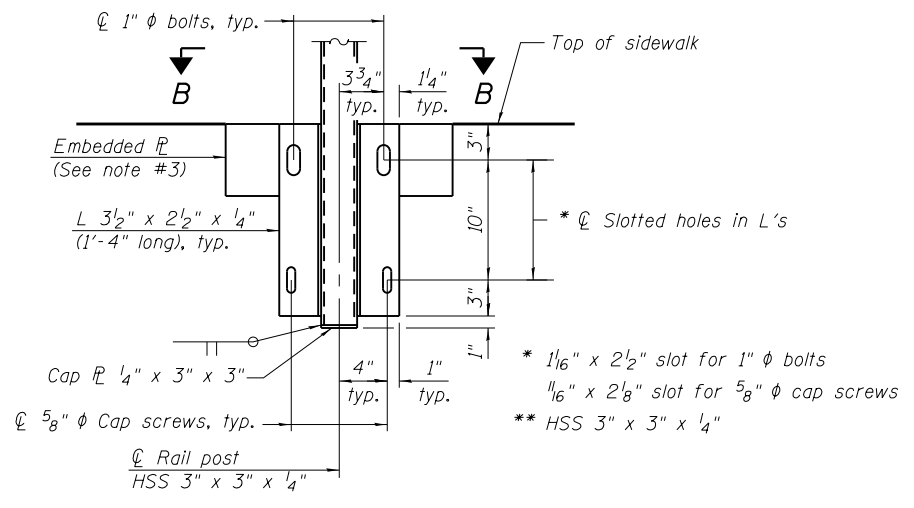
Notes:

1. CVN testing may be omitted for the Pedestrian Railing.
2. Sufficient 1/4" x 6" x 1'-2" galvanized steel shims shall be provided to align rail between adjacent spans. Cost included with Pedestrian Rail (Special).
3. All steel rail elements shall be galvanized according to Article 509.05 of the Standard Specifications.
4. See sheet 8 for anchor device notes and details not shown.

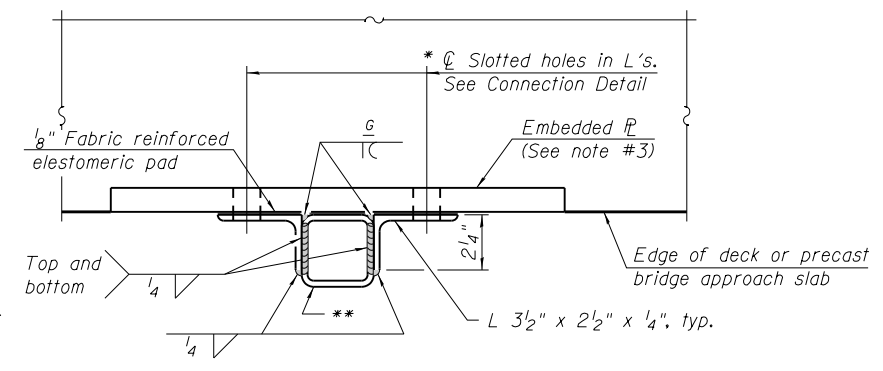


SECTION AT RAIL POST

SECTION A-A



CONNECTION DETAIL

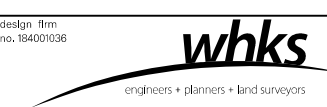


SECTION B-B

BILL OF MATERIAL

Item	Unit	Quantity
Pedestrian Rail (Special)	Foot	340

FILE NAME = 0720245-68185.dgn



USER NAME = *OPERATOR*	DESIGNED - TJZ	REVISED
PLOT SCALE = 0x2' / 1"	CHECKED - SBC	REVISED
PLOT DATE = 8/16/2018	DRAWN - DLH	REVISED
	CHECKED - SBC	REVISED

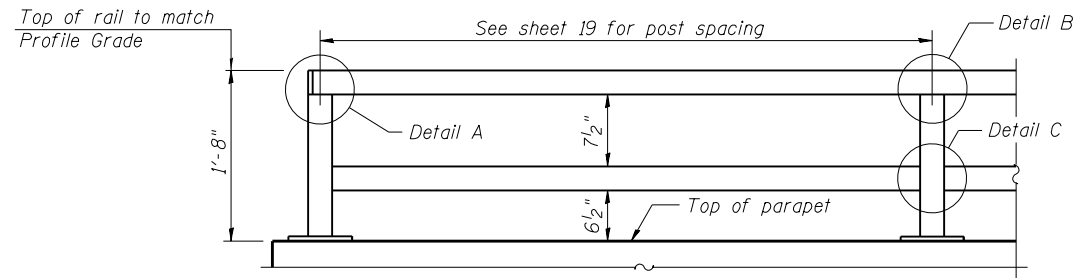
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**PEDESTRIAN RAIL DETAILS I
STRUCTURE NO. 072-0245**

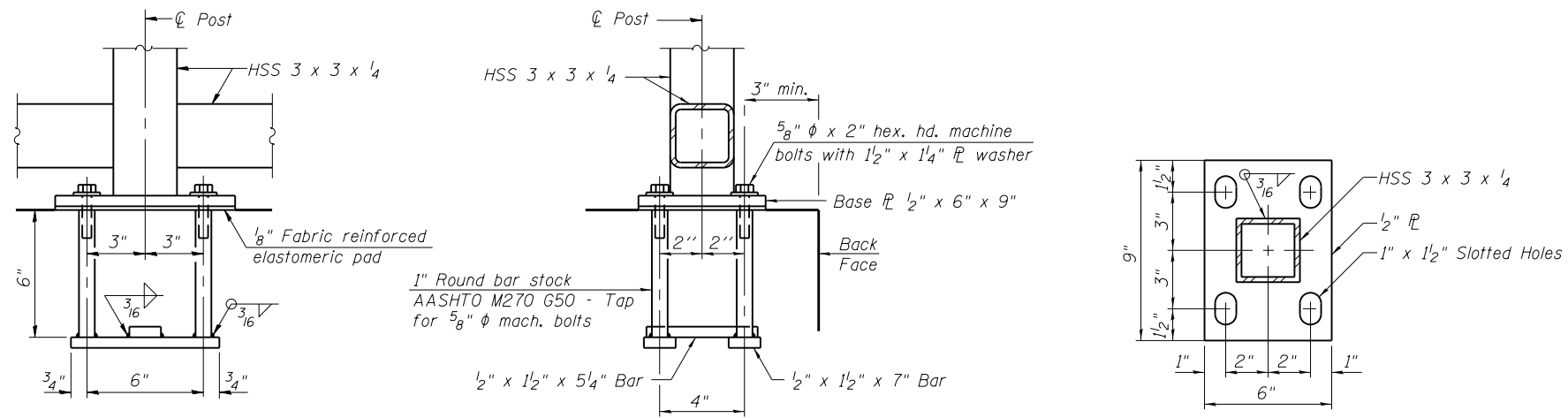
SHEET NO. 27 OF 52 SHEETS

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	441
CONTRACT NO. 68185				

ILLINOIS FED. AID PROJECT



NORTH PARAPET RAILING PARTIAL ELEVATION



ANCHOR BOLT DETAILS

In lieu of the cast-in-place anchor device shown, the Contractor has the option of drilling and setting 5/8" ϕ anchor rods according to Article 509.06 of the Standard Specifications. Embedment shall be according to the manufacturer's specifications.

BASE PL

Notes:

1. All structural steel tubing, post and railing, for parapet railing shall be CVN tested according to 1006.34(b) of the Standard Specifications.
2. All steel rail elements shall be galvanized according to Article 509.05 of the Standard Specifications.
3. See sheet 27 for Details A, B, and C.
4. See sheet 27 for Rail Splice Detail.

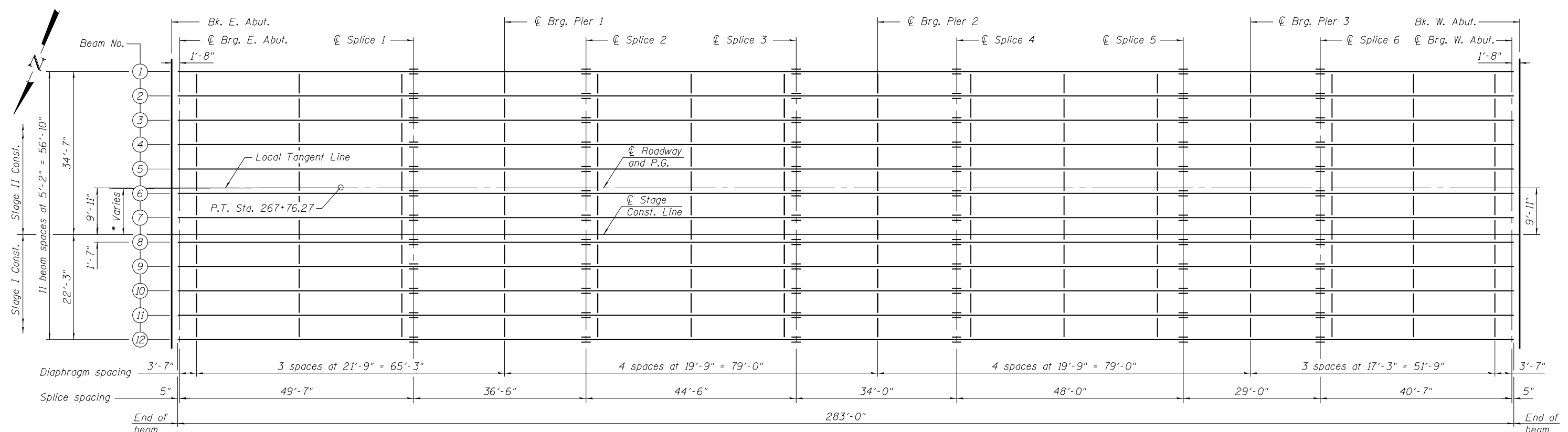
BILL OF MATERIAL

Item	Unit	Quantity
Parapet Railing	Foot	325

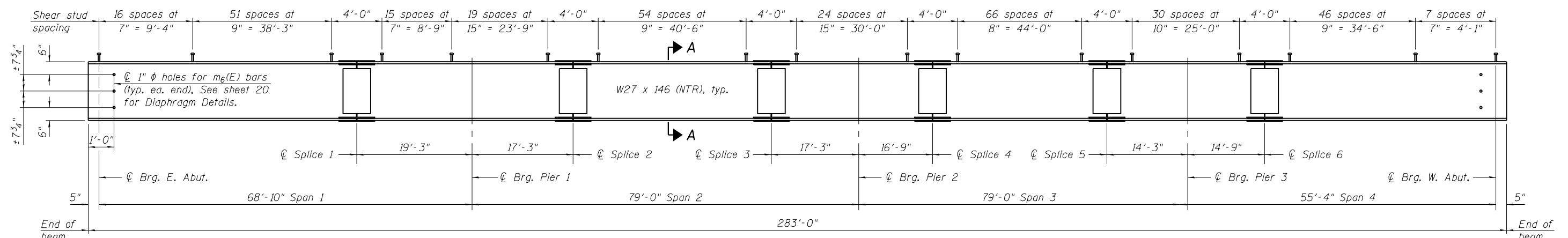
USER NAME = *OPERATOR*	DESIGNED - TJZ	REVISED
	CHECKED - SBC	REVISED
PLOT SCALE = 0:2 1/4" = 1'-0"	DRAWN - DLH	REVISED
PLOT DATE = 8/16/2018	CHECKED - SBC	REVISED

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	442
				CONTRACT NO. 68185

LAST SAVED DATE: 8/15/2018



FRAMING PLAN



BEAM ELEVATION

**** TOP OF BEAM ELEVATIONS**

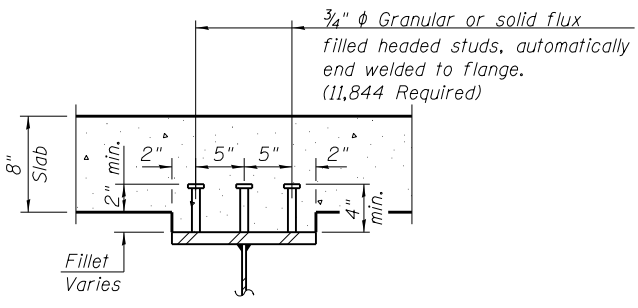
Location	☉ Brg. E. Abut.	☉ Splice 1	☉ Pier 1	☉ Splice 2	☉ Splice 3	☉ Pier 2	☉ Splice 4	☉ Splice 5	☉ Pier 3	☉ Splice 6	☉ Brg. W. Abut.
Beam 1	480.25	480.76	480.94	481.10	481.55	481.73	481.91	482.39	482.52	482.66	483.10
Beam 2	480.36	480.87	481.05	481.21	481.66	481.84	482.02	482.50	482.63	482.77	483.21
Beam 3	480.45	480.95	481.13	481.30	481.75	481.93	482.10	482.58	482.72	482.85	483.30
Beam 4	480.53	481.03	481.22	481.38	481.83	482.01	482.18	482.67	482.80	482.93	483.38
Beam 5	480.61	481.11	481.30	481.46	481.91	482.09	482.26	482.75	482.88	483.01	483.46
Beam 6	480.65	481.16	481.34	481.50	481.95	482.13	482.31	482.79	482.92	483.06	483.50
Beam 7	480.57	481.08	481.26	481.42	481.87	482.05	482.23	482.71	482.84	482.98	483.42
Beam 8	480.49	481.00	481.18	481.34	481.79	481.97	482.15	482.63	482.76	482.90	483.34
Beam 9	480.41	480.92	481.10	481.26	481.71	481.89	482.07	482.55	482.68	482.82	483.26
Beam 10	480.31	480.82	481.00	481.16	481.61	481.79	481.97	482.45	482.58	482.72	483.16
Beam 11	480.20	480.71	480.89	481.05	481.51	481.68	481.86	482.34	482.47	482.61	483.05
Beam 12	480.10	480.60	480.78	480.95	481.40	481.58	481.75	482.23	482.37	482.50	482.95

** For Fabrication Only.

* Dimension varies from Sta. 267+40.50 to Sta. 267+76.27. See Offset Sketch on sheet 2.

Notes:

- All structural steel shall be AASHTO M270, Grade 50, and shall be galvanized. See Special Provisions for "Hot Dip Galvanizing for Structural Steel".
- All diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted. Individual diaphragms at supports may be temporarily disconnected to install bearing anchor rods.
- Load carrying components designated "NTR" shall conform to the Impact Testing Requirement, Zone 2.
- See sheet 30 for Interior Diaphragm and Splice Details.
- See sheet 31 for Anchor Bolt Placement.



SECTION A-A

FILE NAME = 0720245-68185.dgn



USER NAME = *OPERATOR*	DESIGNED - TJZ	REVISED
PLOT SCALE = 0.2" = 1'-0"	CHECKED - SBC	REVISED
PLOT DATE = 8/16/2018	DRAWN - DLH	REVISED
	CHECKED - SBC	REVISED

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**FRAMING PLAN
STRUCTURE NO. 072-0245**

SHEET NO. 29 OF 52 SHEETS

F.A.U. RTE. 6659	SECTION 11(N,BR-1,RS-4,W-1)	COUNTY PEORIA	TOTAL SHEETS 577	SHEET NO. 443
CONTRACT NO. 68185				

ILLINOIS FED. AID PROJECT

INTERIOR BEAM MOMENT TABLE							
	0.4 Sp. 1	Pier 1	0.5 Sp. 2	Pier 2	0.5 Sp. 3	Pier 3	0.6 Sp. 4
I_s	(in ⁴)	5660	5660	5660	5660	5660	5660
$I_c(n)$	(in ⁴)	14411		14411		14411	14411
$I_c(3n)$	(in ⁴)	10262		10262		10262	10262
$I_c(cr)$	(in ⁴)		7276		7276		7276
S_s	(in ³)	414.0	414.0	414.0	414.0	414.0	414.0
$S_c(n)$	(in ³)	595.7		595.7		595.7	595.7
$S_c(3n)$	(in ³)	532.0		532.0		532.0	532.0
$S_c(cr)$	(in ³)		463.6		463.6		463.6
DC1	(k/ft)	0.708	0.708	0.708	0.708	0.708	0.708
M _{DC1}	(k)	248	387	174	369	206	131
DC2	(k/ft)	0.230	0.230	0.230	0.230	0.230	0.230
M _{DC2}	(k)	80	127	55	121	66	42
DW	(k/ft)	0.258	0.258	0.258	0.258	0.258	0.258
M _{DW}	(k)	90	142	62	136	74	47
M _{ℓ + IM}	(k)	611	605	557	604	548	484
M _u (Strength I)	(k)	1614	1914	1354	1874	1410	1134
φ _r M _n	(k)	2698	2326	2698	2326	2698	2698
f _s DC1	(ksi)	7.2	11.2	5.0	10.7	6.0	9.4
f _s DC2	(ksi)	1.8	3.3	1.2	3.1	1.5	2.7
f _s DW	(ksi)	2.0	3.7	1.4	3.5	1.7	1.1
f _s (ℓ + IM)	(ksi)	12.3	15.7	11.2	15.6	11.0	13.7
f _s (Service II)	(ksi)	27.0	38.5	22.3	37.7	23.5	33.0
0.95R _h F _{yf}	(ksi)	47.5	47.5	47.5	47.5	47.5	47.5
f _s (Total) Strength I	(ksi)						
φ _r F _n	(ksi)						
V _r	(k)	21.8	23.3		24.6		21.2

INTERIOR BEAM REACTION TABLE						
	E. Abut.	Pier 1	Pier 2	Pier 3	W. Abut.	
R _{DC1}	(k)	19.4	58.2	56.3	52.8	14.4
R _{DC2}	(k)	6.1	18.9	18.3	17.2	4.4
R _{DW}	(k)	6.8	21.2	20.5	19.3	5.0
R _{ℓ + IM}	(k)	60.8	93.7	93.1	88.5	56.8
R _{Total}	(k)	93.1	192.0	188.2	177.8	80.6

I_s, S_s : Non-composite moment of inertia and section modulus of the steel section used for computing f_s (Total-Strength I, and Service II) due to non-composite dead loads (in⁴ and in³).

$I_c(n), S_c(n)$: Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections due to short-term composite live loads (in⁴ and in³).

$I_c(3n), S_c(3n)$: Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing f_s (Total-Strength I, and Service II) in uncracked sections, due to long-term composite (superimposed) dead loads (in⁴ and in³).

$I_c(cr), S_c(cr)$: Composite moment of inertia and section modulus of the steel and longitudinal deck reinforcement, used for computing f_s (Total-Strength I and Service II) in cracked sections, due to both short-term composite live loads and long-term composite (superimposed) dead loads (in⁴ and in³).

DC1: Un-factored non-composite dead load (kips/ft.).

M_{DC1}: Un-factored moment due to non-composite dead load (kip-ft.).

DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).

M_{DC2}: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).

DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).

M_{DW}: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).

M_{ℓ + IM}: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).

M_u (Strength I): Factored design moment (kip-ft.).

1.25 (M_{DC1} + M_{DC2}) + 1.5 M_{DW} + 1.75 M_{ℓ + IM}

φ_rM_n: Compact composite positive moment capacity computed according to Article 6.10.7.1 or non-slender negative moment capacity according to Article A6.1.1 or A6.1.2 (kip-ft.).

f_s DC1: Un-factored stress at edge of flange for controlling steel flange due to vertical non-composite dead loads as calculated below (ksi).

M_{DC1} / S_{nc}

f_s DC2: Un-factored stress at edge of flange for controlling steel flange due to vertical composite dead loads as calculated below (ksi).

M_{DC2} / S_{c(3n)} or M_{DC2} / S_{c(cr)} as applicable.

f_s DW: Un-factored stress at edge of flange for controlling steel flange due to vertical composite future wearing surface loads as calculated below (ksi).

M_{DW} / S_{c(3n)} or M_{DW} / S_{c(cr)} as applicable.

f_s (ℓ + IM): Un-factored stress at edge of flange for controlling steel flange due to vertical composite live load plus impact loads as calculated below (ksi).

M_{ℓ + IM} / S_{c(n)} or M_{DW} / S_{c(cr)} as applicable.

f_s (Service II): Sum of stresses as computed below (ksi).

f_s DC1 + f_s DC2 + f_s DW + 1.3 f_s (ℓ + IM)

0.95R_hF_{yf}: Composite stress capacity for Service II loading according to Article 6.10.4.2 (ksi).

f_s (Total) Strength I: Sum of stresses as computed below on non-compact section (ksi).

1.25 (f_s DC1 + f_s DC2) + 1.5 f_s DW + 1.75 f_s (ℓ + IM)

φ_rF_n: Non-Compact composite positive or negative stress capacity for Strength I loading according to Article 6.10.7 or 6.10.8 (ksi).

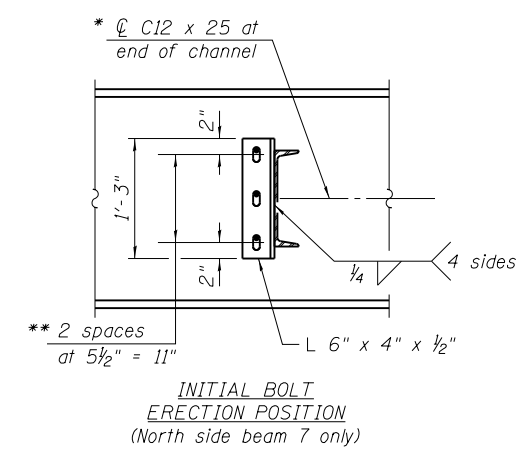
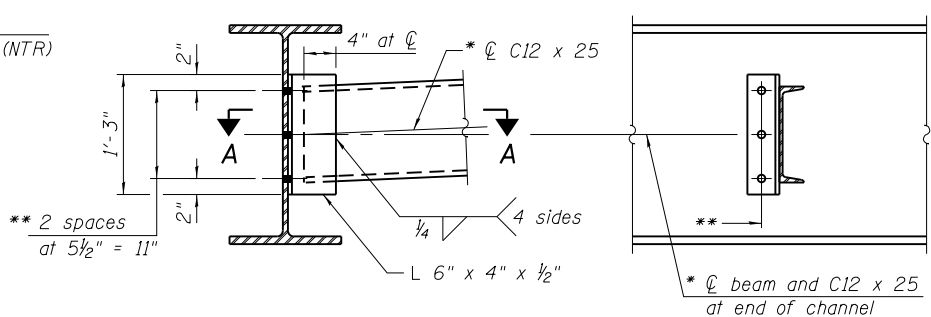
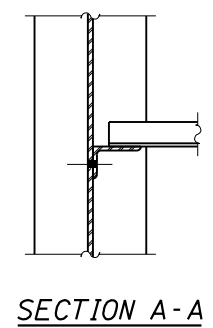
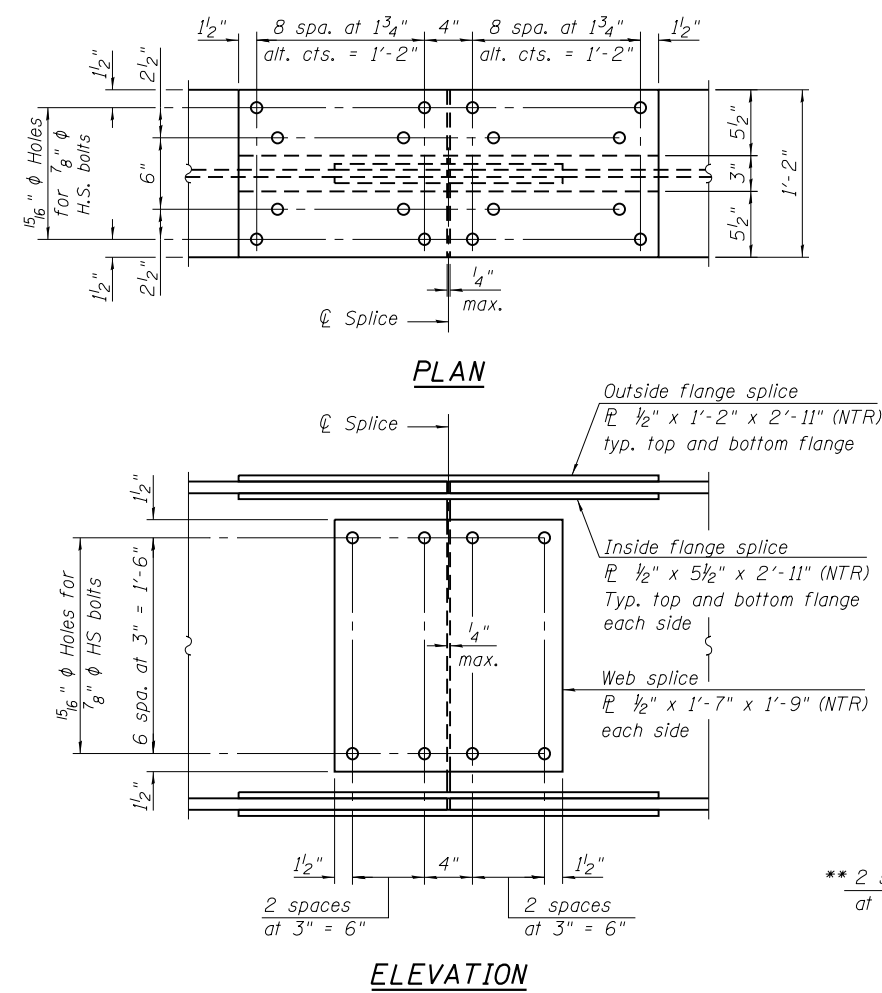
V_r: Maximum factored shear range in span computed according to Article 6.10.10.

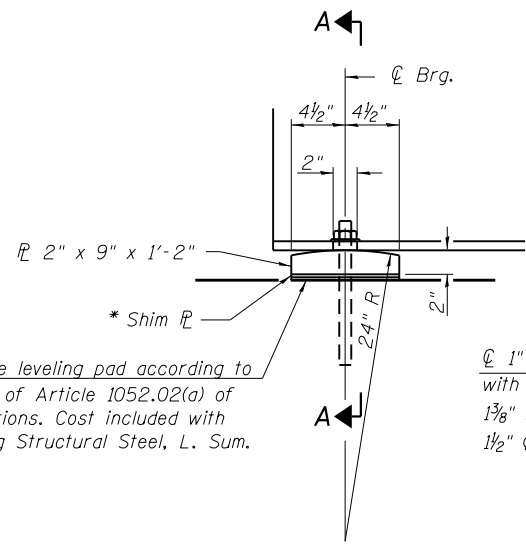
Splice Notes:

- All splices are symmetrical about ℄ splice.
- H.S. bolts shall be 3/8" φ ASTM A325, Type 1.
- Load carrying components designated "NTR" shall conform to the Impact Testing Requirement, Zone 2.
- All plates composing the splices shall be AASHTO M270, Grade 50, and shall be galvanized. See Special Provisions for "Hot Dip Galvanizing for Structural Steel".

Interior Diaphragm Notes:

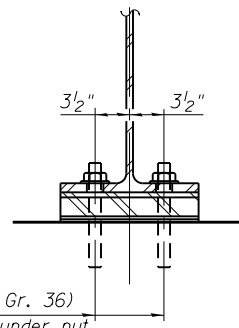
- Two hardened washers required for each set of oversized or slotted holes.
 - All shapes composing the diaphragms shall be AASHTO M270, Grade 50.
- * Alternate C 12 x 30 channels are permitted to facilitate material acquisition. Calculated weight of structural steel is based on the lighter section. The alternate, if utilized, shall be provided at no additional cost to the Department.
- ** 3/4" φ H.S. bolts, 15/16" φ holes typ., except 3/4" φ H.S. bolts, 13/16" x 1/8" slots provided on north side of Beam 7 to accommodate differential displacement between Beams 7 and 8 for Stage Construction. Bolts in slots shall be finger tight until Stage II Construction is complete. Position slots so bolts start at one end with no concrete load and finish near the opposite end under deck load, allowing maximum displacement without laterally stressing main members.





1/8" elastomeric neoprene leveling pad according to the material properties of Article 1052.02(a) of the Standard Specifications. Cost included with Furnishing and Erecting Structural Steel, L. Sum.

1" ϕ x 12" anchor bolts (F1554 Gr. 36) with 2 1/4" x 2 1/4" x 5/16" PL washer under nut. 1 3/8" x 2" slotted hole in flange. 1/2" ϕ holes in bearing plate.



SECTION A-A

FIXED BEARING AT ABUTMENTS

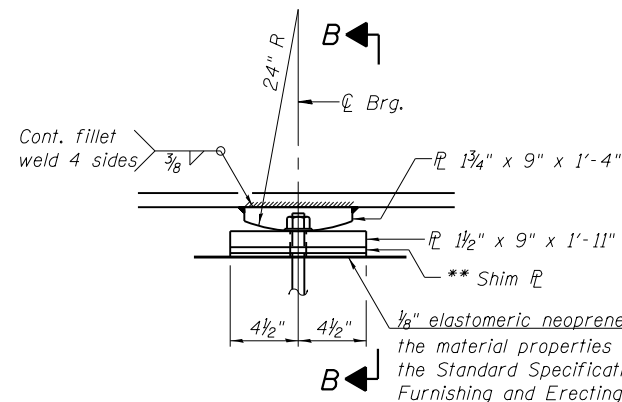
(24 required)

Notes:

1. Anchor bolts shall be ASTM F1554 All-Thread (or an Engineer approved alternate material) of the Grade(s) and Diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.
2. Beams shall be braced for stability during erection and remain braced until deck is poured and cured.
3. Anchor bolts at all supports shall be installed as each member is erected unless an equivalent temporary means of lateral restraint is used.
4. Two 1/8" adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on Bearing Details.
5. All structural steel plates and pintles for the fixed bearings shall conform to the requirements of AASHTO M270 Grade 50.
6. All (embedded or separate) bearing plates, anchor bolts, nuts, washers and pintles shall be galvanized according to AASHTO M111 or M232 as applicable.

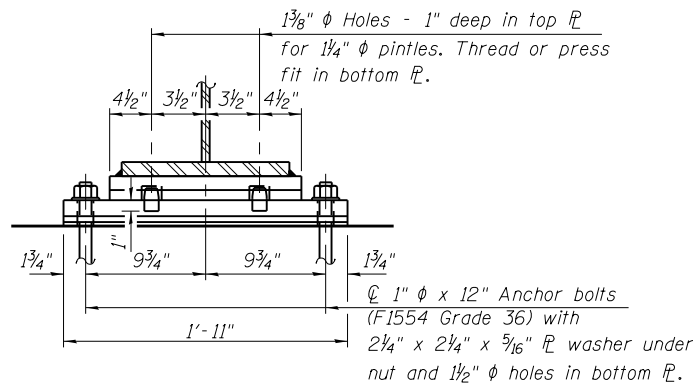
* Provide an additional 1/2" x 9" x 1'-2" Fill PL at Beam 6 (typ. at abutments)
Provide an additional 1/2" x 10" x 1'-11" Fill PL at Beam 6 (typ. at piers)

** Provide an additional 1" x 9" x 1'-2" Fill PL at Beam 7 (typ. at abutments)
Provide an additional 1" x 10' x 1'-11" Fill PL at Beam 7 (typ. at piers)



ELEVATION

1/8" elastomeric neoprene leveling pad according to the material properties of Article 1052.02(a) of the Standard Specifications. Cost included with Furnishing and Erecting Structural Steel, L. Sum.



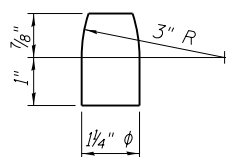
SECTION B-B

FIXED BEARING AT PIERS

(36 required)

BILL OF MATERIAL

ITEM	UNIT	TOTAL
Anchor Bolts, 1"	Each	120

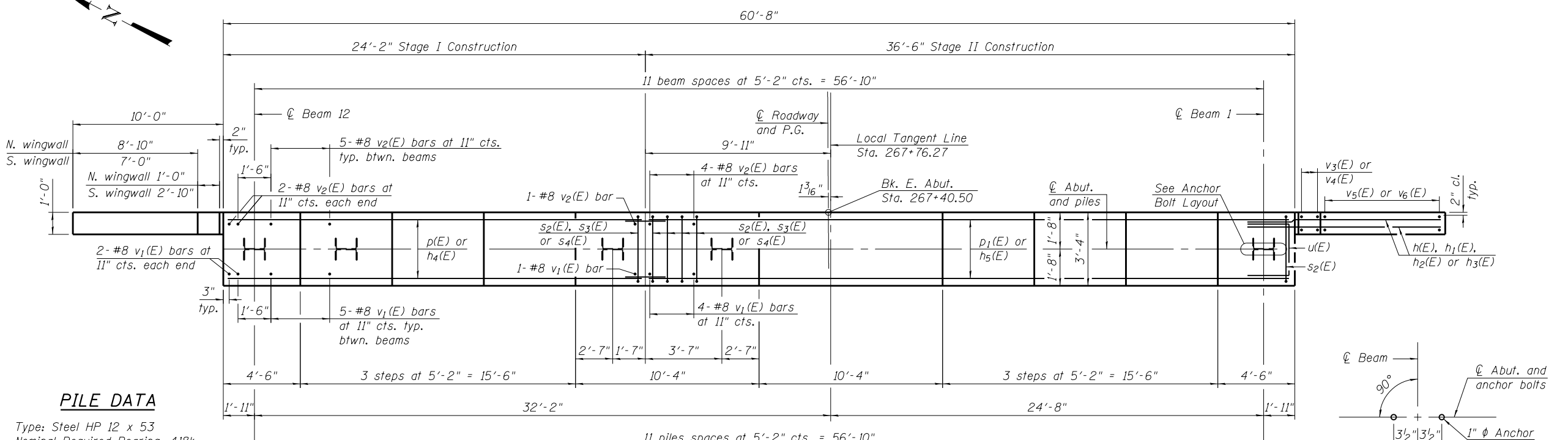
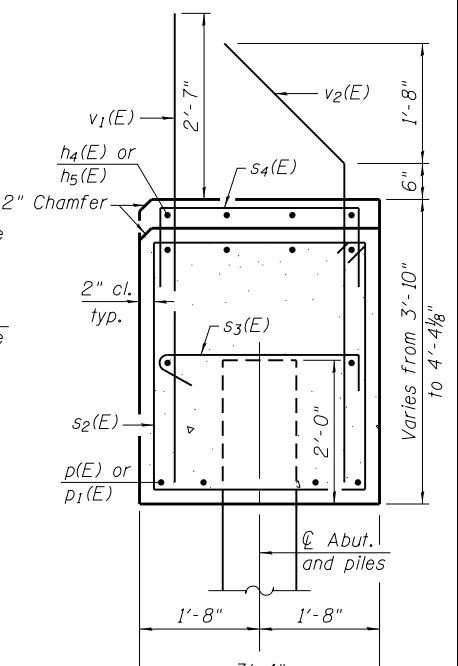
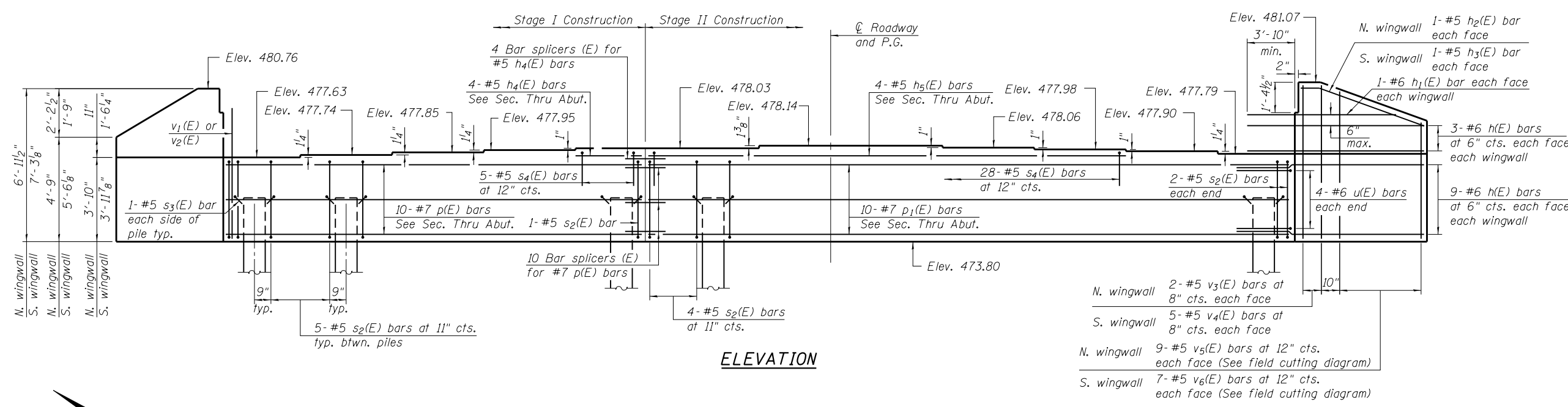


PINTLE

USER NAME = *OPERATOR*	DESIGNED - TJZ	REVISED
	CHECKED - SBC	REVISED
PLOT SCALE = 0:2' = 1" / 1/4"	DRAWN - DLH	REVISED
PLOT DATE = 8/16/2018	CHECKED - SBC	REVISED

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N,BR-1,RS-4,W-1)	PEORIA	577	445
CONTRACT NO. 68185				

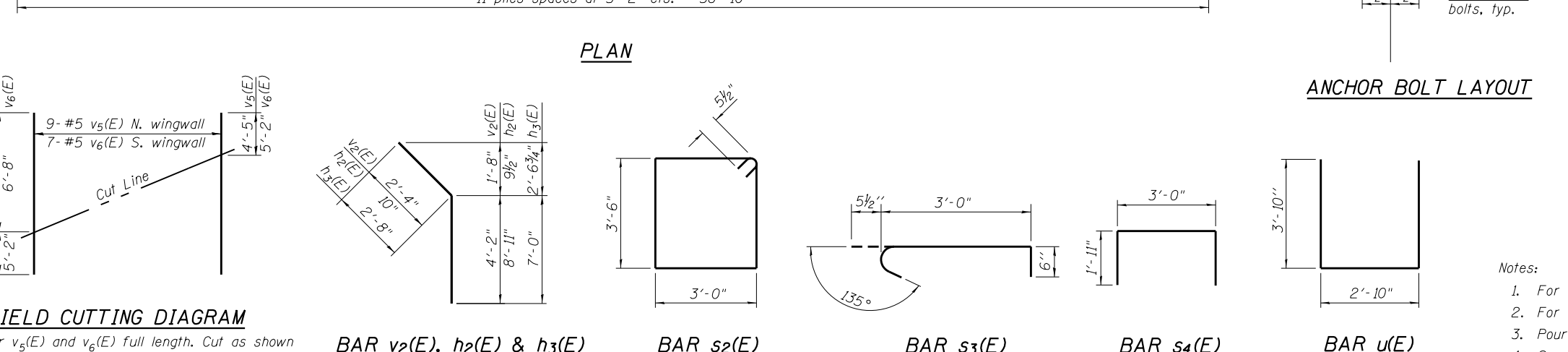
LAST SAVED DATE: 8/15/2018



BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h(E)	48	#6	13'-10"	—
h1(E)	4	#6	11'-4"	—
h2(E)	2	#5	9'-9"	—
h3(E)	2	#5	9'-8"	—
h4(E)	4	#5	3'-10"	—
h5(E)	4	#5	26'-6"	—
p(E)	10	#7	23'-10"	—
p1(E)	10	#7	36'-2"	—
s2(E)	59	#5	13'-11"	□
s3(E)	24	#5	4'-0"	—
s4(E)	33	#5	6'-10"	—
u(E)	8	#6	10'-6"	—
v1(E)	59	#8	6'-3"	—
v2(E)	59	#8	6'-6"	—
v3(E)	4	#5	6'-7"	—
v4(E)	10	#5	6'-11"	—
v5(E)	9	#5	10'-10"	—
v6(E)	7	#5	11'-10"	—
Structure Excavation		Cu. Yd.	42	
Concrete Structures		Cu. Yd.	35.9	
Reinforcement Bars, Epoxy Coated		Pound	6,080	
Bar Splicers		Each	14	
Furnishing Steel		Foot	517	
Piles HP 12 x 53		Foot	517	
Driving Piles		Foot	517	
Test Pile Steel HP 12 x 53		Each	1	
Pile Shoes		Each	12	

PILE DATA
 Type: Steel HP 12 x 53
 Nominal Required Bearing: 418k
 Factored Resistance Available: 230k
 Est. Length: 47 ft.
 No. Production Piles: 11
 No. Test Piles: 1



Notes:

- For details of piles see sheet 41.
- For details of bar splicers see sheet 42.
- Pour steps monolithically with cap.
- Space cap reinforcement to miss anchor bolts.



USER NAME = *OPERATOR*	DESIGNED - SBC	REVISED
PLOT SCALE = 0.2" = 1'-0"	CHECKED - TJZ	REVISED
PLOT DATE = 8/16/2018	DRAWN - DLH	REVISED
	CHECKED - TJZ	REVISED

STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

EAST ABUTMENT
 STRUCTURE NO. 072-0245
 SHEET NO. 32 OF 52 SHEETS

F.A.U. RT. 6659	SECTION 11(N, BR-1, RS-4, W-1)	COUNTY PEORIA	TOTAL SHEETS 577	SHEET NO. 446
CONTRACT NO. 68185				ILLINOIS FED. AID PROJECT

LAST SAVED DATE: 8/15/2018

FILE NAME = 0720245-68185.dgn

Design firm
no. 184001036

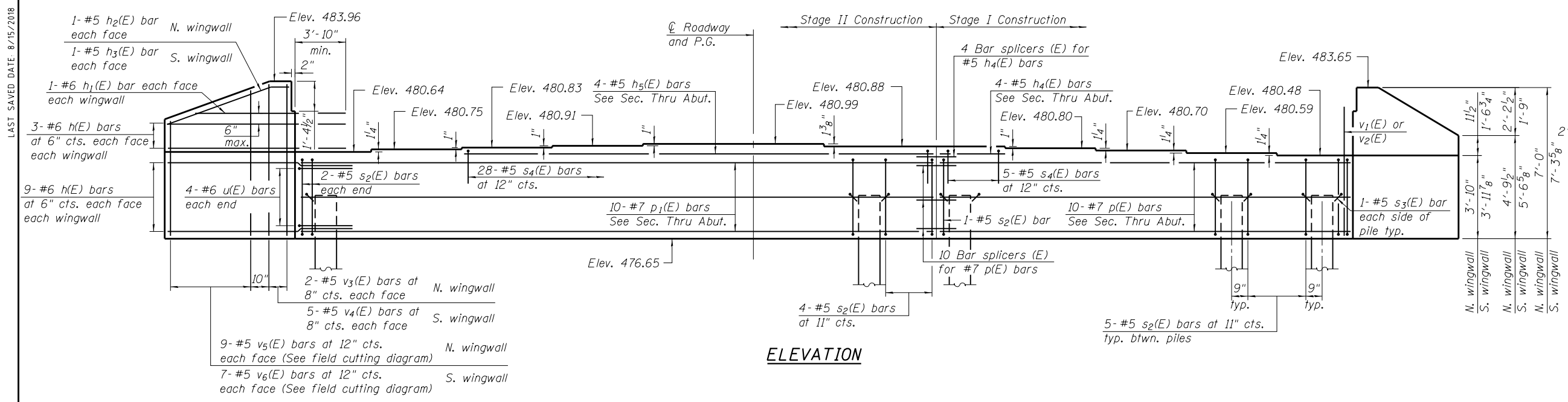


USER NAME = *OPERATOR*	DESIGNED - SBC	REVISED
	CHECKED - TJZ	REVISED
PLOT SCALE = 0.2" = 1' - 0"	DRAWN - DLH	REVISED
PLOT DATE = 8/16/2018	CHECKED - TJZ	REVISED

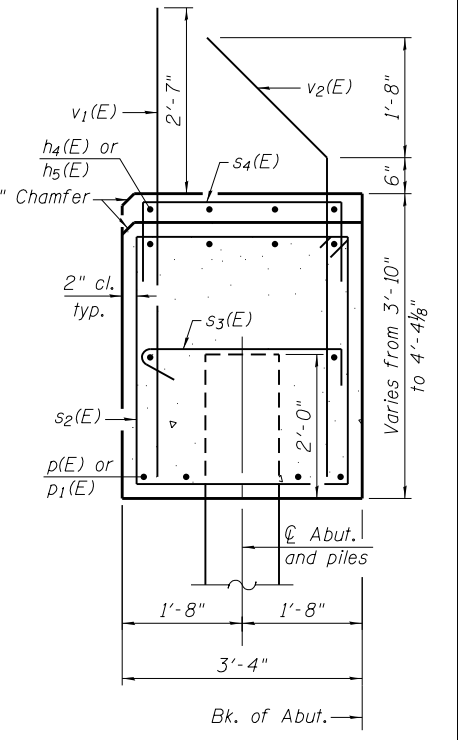
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

WEST ABUTMENT
STRUCTURE NO. 072-0245

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	447
			CONTRACT NO. 68185	
ILLINOIS FED. AID PROJECT				



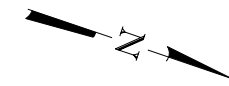
ELEVATION



SEC. THRU ABUT.

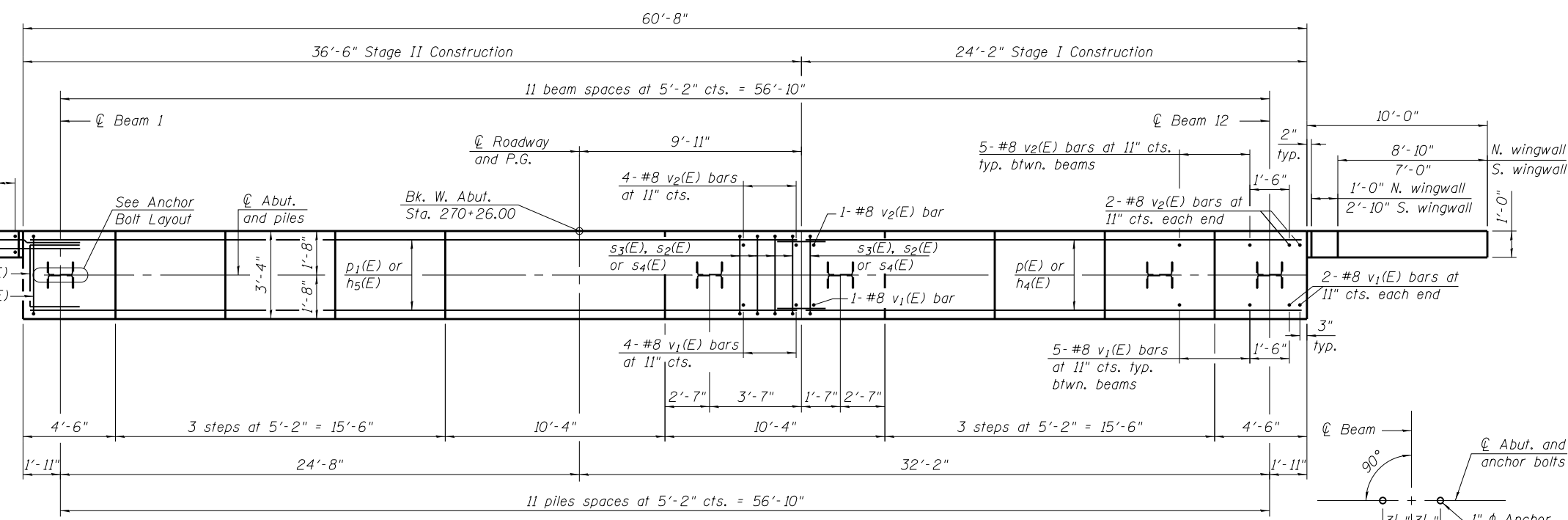
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h(E)	48	#6	13'-10"	—
h1(E)	4	#6	11'-4"	—
h2(E)	2	#5	9'-9"	—
h3(E)	2	#5	9'-8"	—
h4(E)	4	#5	3'-10"	—
h5(E)	4	#5	26'-6"	—
p(E)	10	#7	23'-10"	—
p1(E)	10	#7	36'-2"	—
s2(E)	59	#5	13'-11"	□
s3(E)	24	#5	4'-0"	□
s4(E)	33	#5	6'-10"	□
u(E)	8	#6	10'-6"	□
v1(E)	59	#8	6'-3"	—
v2(E)	59	#8	6'-6"	—
v3(E)	4	#5	6'-7"	—
v4(E)	10	#5	6'-11"	—
v5(E)	9	#5	10'-10"	—
v6(E)	7	#5	11'-10"	—
Structure Excavation		Cu. Yd.	146	
Concrete Structures		Cu. Yd.	35.9	
Reinforcement Bars, Epoxy Coated		Pound	6,080	
Bar Splicers		Each	14	
Furnishing Steel Piles HP 12 x 53		Foot	308	
Driving Piles		Foot	308	
Test Pile Steel HP 12 x 53		Each	1	
Pile Shoes		Each	12	



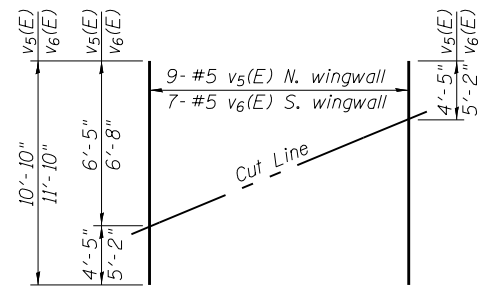
PILE DATA

Type: Steel HP 12 x 53
Nominal Required Bearing: 418k
Factored Resistance Available: 219k
Est. Length: 28 ft.
No. Production Piles: 11
No. Test Piles: 1



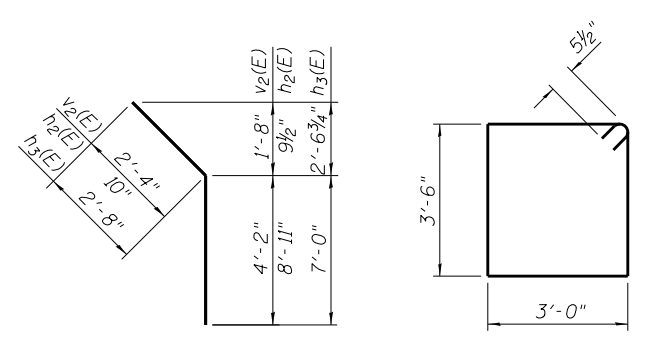
PLAN

ANCHOR BOLT LAYOUT



FIELD CUTTING DIAGRAM

Order v5(E) and v6(E) full length. Cut as shown and use remainder of bars in opposite face.



BAR v2(E), h2(E) & h3(E)

BAR s2(E)

BAR s3(E)

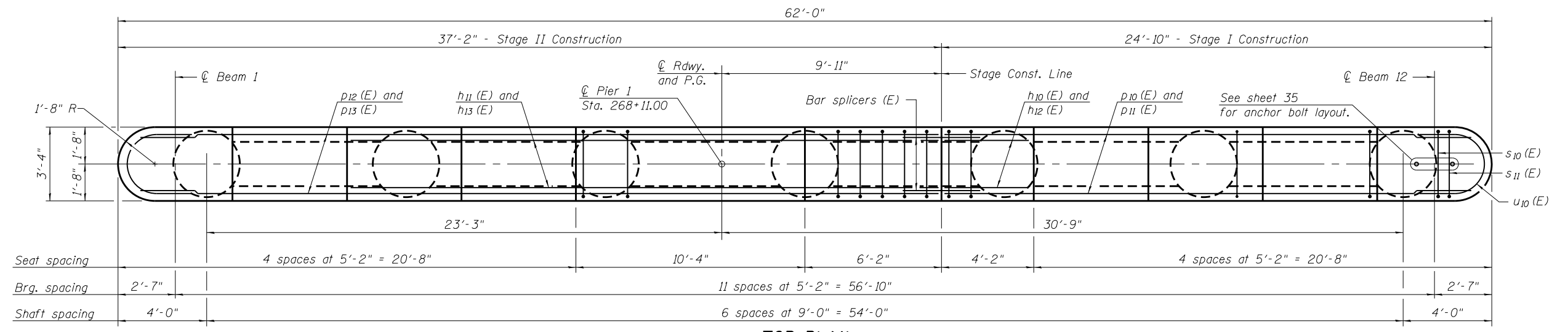
BAR s4(E)

BAR u(E)

Notes:

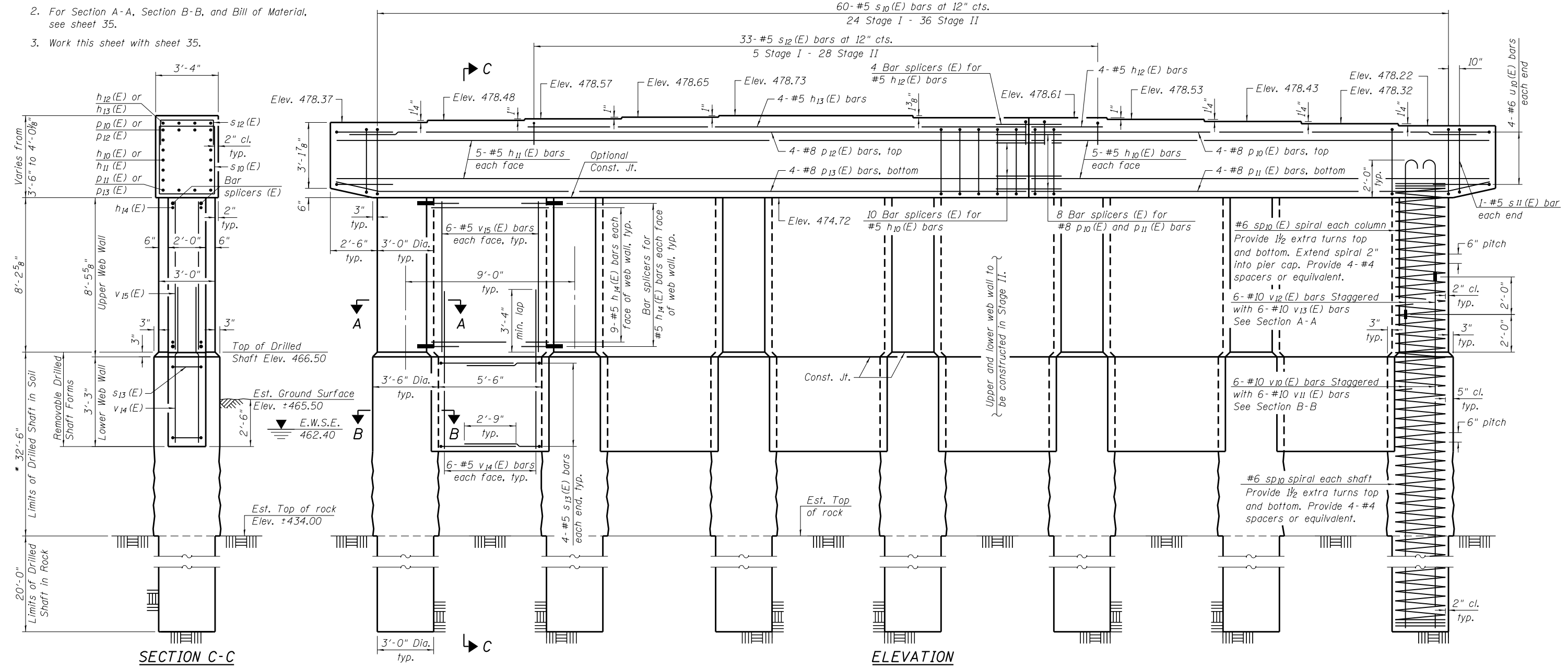
- For details of piles see sheet 41.
- For details of bar splicers see sheet 42.
- Pour steps monolithically with cap.
- Space cap reinforcement to miss anchor bolts.

LAST SAVED DATE: 8/15/2018



TOP PLAN

- Notes:
1. See sheet 42 for bar splicers and mechanical splicers.
 2. For Section A-A, Section B-B, and Bill of Material, see sheet 35.
 3. Work this sheet with sheet 35.



SECTION C-C

ELEVATION
(Looking West)

FILE NAME = 0720245-68185.dgn



USER NAME = *OPERATOR*	DESIGNED - TJZ	REVISED
PLOT SCALE = 0.2" = 1'-0"	CHECKED - SBC	REVISED
PLOT DATE = 8/16/2018	DRAWN - DLH	REVISED
	CHECKED - SBC	REVISED

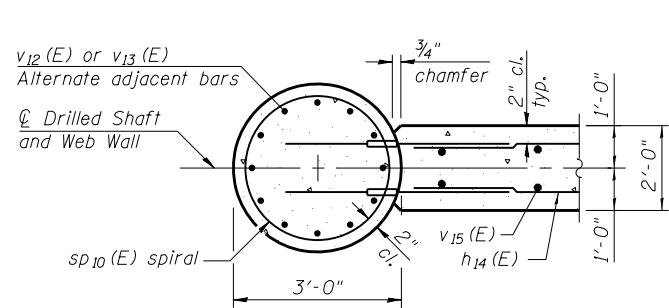
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

PIER 1
STRUCTURE NO. 072-0245

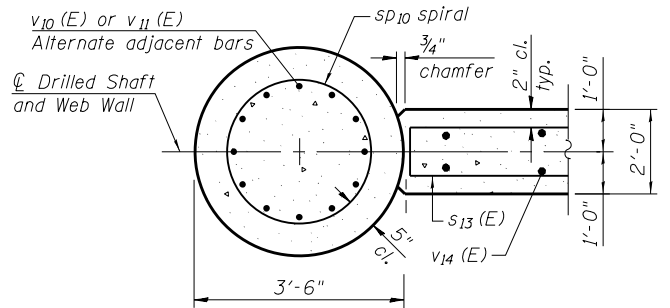
SHEET NO. 34 OF 52 SHEETS

F.A.U. RTE. 6659	SECTION 11(N, BR-1, RS-4, W-1)	COUNTY PEORIA	TOTAL SHEETS 577	SHEET NO. 448
CONTRACT NO. 68185				

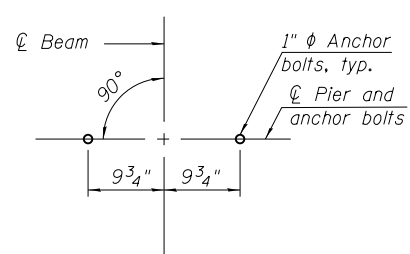
ILLINOIS FED. AID PROJECT



SECTION A-A



SECTION B-B



ANCHOR BOLT LAYOUT

Notes:

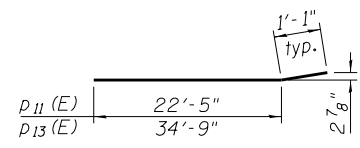
1. Shafts must be poured the same day as the rock socket excavation.
2. Pour steps monolithically with cap.
3. Space cap reinforcement to miss anchor bolts.
4. If a portion of the drilled shaft web walls or concrete encasement is under water, reinforcement may be placed underwater into forms. Concrete shall be tremied according to Article 503.08 of the Standard Specifications to an elevation of 1'-0" above the water line at the time of construction.

Construction Sequence for Web Wall:

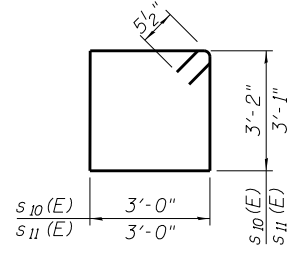
1. Excavate between shafts to elevation of web wall base and set lower web wall forms through water to bear on the circular edge of drilled shafts. Secure in place with fill, struts or tie forms together as required.
2. Place the lower web wall reinforcement cage into the forms using spacers to maintain proper clearances.
3. If the forms can be sealed against the shafts and streambed to allow dewatering, the reinforcement and the concrete placement may be completed in the dry. Alternatively, the rebar cage can be lowered into position through water and the concrete discharged at the base of the excavation through a tremie pipe or pump hose, displacing water, sediment, and tainted concrete out the top of the forms.
4. Construct columns.
5. Construct upper web walls.

* If the prevailing water surface elevation during construction is consistently different than estimated on the plans, the contractor may propose an adjustment to the top of the drilled shaft elevation as part of their installation procedure. The top of all drilled shafts within a substructure unit shall be constructed to the same elevation and extend above the prevailing water surface. The quantities and reinforcement detailing are based on the top of shaft and the estimated elevations shown and may change based on the actual elevations encountered at each shaft and the final top of shaft elevation.

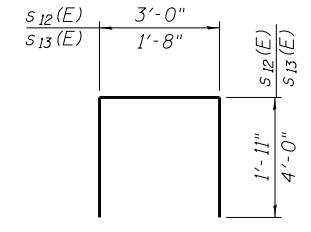
** Length is height of spiral. Minimum lap for spirals = 3'-0".



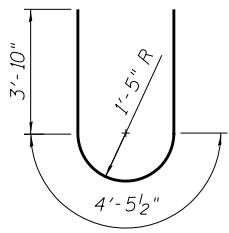
BARS p₁₁(E) and p₁₃(E)



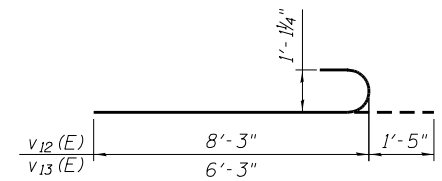
BARS s₁₀(E) and s₁₁(E)



BARS s₁₂(E) and s₁₃(E)



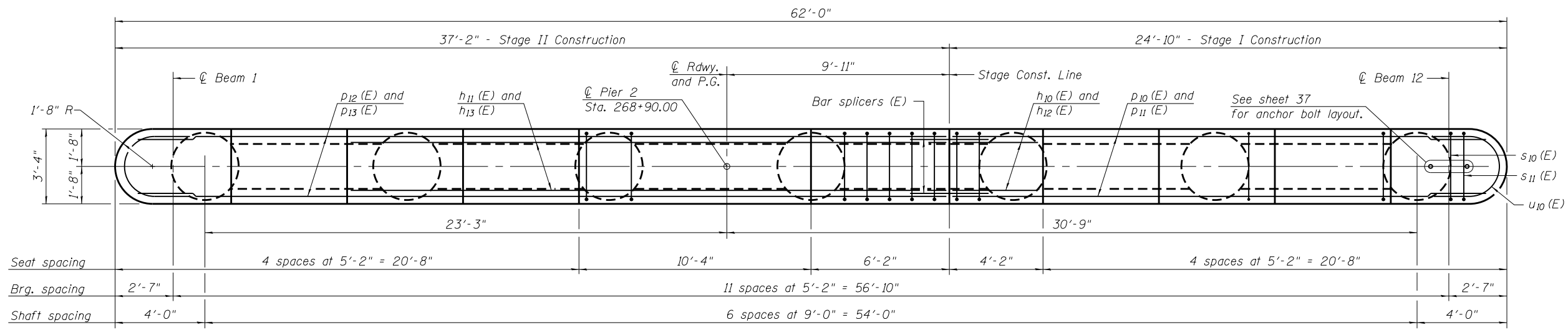
BAR u₁₀(E)



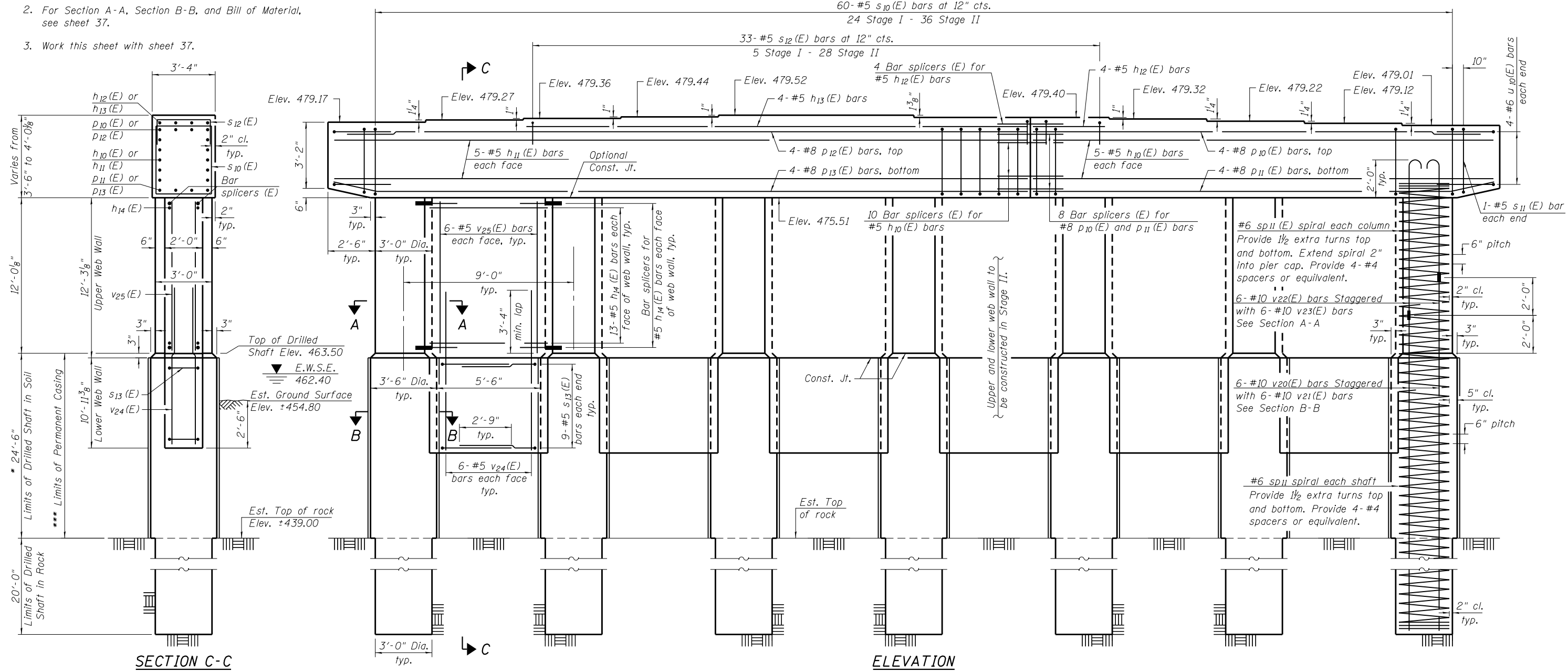
BARS v₁₂(E) & v₁₃(E)

BILL OF MATERIAL
PIER 1

Bar	No.	Size	Length	Shape
h ₁₀ (E)	10	#5	23'-0"	—
h ₁₁ (E)	10	#5	35'-4"	—
h ₁₂ (E)	4	#5	3'-10"	—
h ₁₃ (E)	4	#5	26'-6"	—
h ₁₄ (E)	108	#5	5'-8"	—
p ₁₀ (E)	4	#8	23'-0"	—
p ₁₁ (E)	4	#8	23'-6"	—
p ₁₂ (E)	4	#8	35'-4"	—
p ₁₃ (E)	4	#8	35'-10"	—
sp ₁₀	7	#6	52'-6"	⌘
sp ₁₀ (E)	7	#6	8'-5"	⌘
s ₁₀ (E)	60	#5	13'-3"	□
s ₁₁ (E)	2	#5	13'-1"	□
s ₁₂ (E)	33	#5	6'-10"	U
s ₁₃ (E)	48	#5	9'-8"	U
u ₁₀ (E)	8	#6	12'-2"	U
v ₁₀ (E)	42	#10	54'-6"	—
v ₁₁ (E)	42	#10	56'-6"	—
v ₁₂ (E)	42	#10	9'-8"	—
v ₁₃ (E)	42	#10	7'-8"	—
v ₁₄ (E)	72	#5	6'-7"	—
v ₁₅ (E)	72	#5	8'-5"	—
Structure Excavation		Cu. Yd.	30	
Concrete Structures		Cu. Yd.	74.6	
Reinforcement Bars		Pound	9,310	
Reinforcement Bars, Epoxy Coated		Pound	30,390	
Bar Splicers		Each	238	
Mechanical Splicers		Each	84	
Drilled Shaft in Soil		Cu. Yd.	81.1	
Drilled Shaft in Rock		Cu. Yd.	36.7	

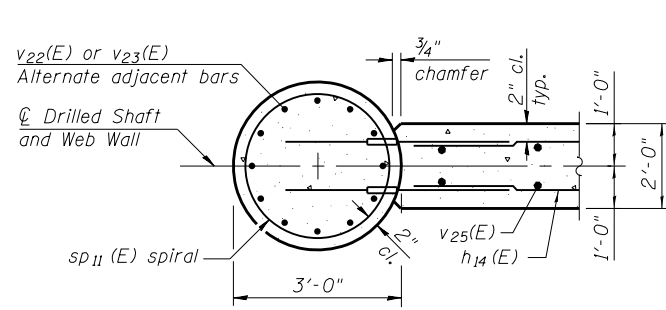


- Notes:
- See sheet 42 for bar splicers and mechanical splicers.
 - For Section A-A, Section B-B, and Bill of Material, see sheet 37.
 - Work this sheet with sheet 37.

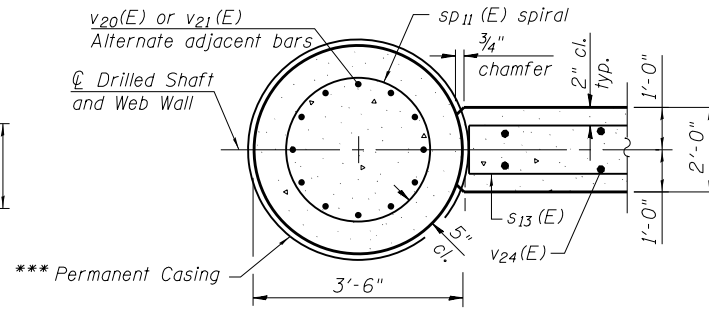


USER NAME = *OPERATOR*	DESIGNED - TJZ	REVISED
PLOT SCALE = 0/2" = 1'	CHECKED - SBC	REVISED
PLOT DATE = 8/16/2018	DRAWN - DLH	REVISED
	CHECKED - SBC	REVISED

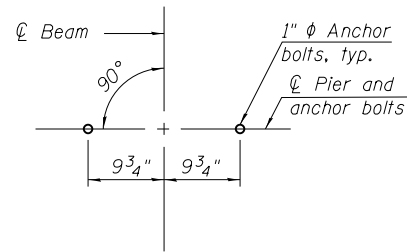
F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	450
CONTRACT NO. 68185				



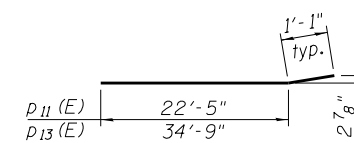
SECTION A-A



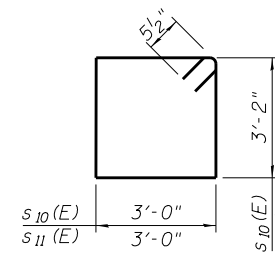
SECTION B-B



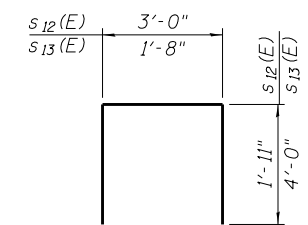
ANCHOR BOLT LAYOUT



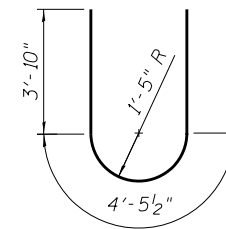
BARS p₁₁(E) and p₁₃(E)



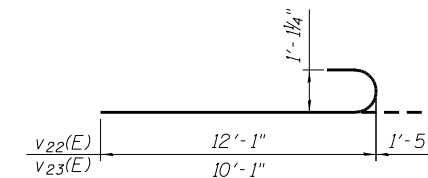
BARS s₁₀(E) and s₁₁(E)



BARS s₁₂(E) and s₁₃(E)



BAR u₁₀(E)



BARS v₂₂(E) & v₂₃(E)

Notes:

1. Shafts must be poured the same day as the rock socket excavation.
2. Pour steps monolithically with cap.
3. Space cap reinforcement to miss anchor bolts.
4. If a portion of the drilled shaft web walls or concrete encasement is under water, reinforcement may be placed underwater into forms. Concrete shall be tremied according to Article 503.08 of the Standard Specifications to an elevation of 1'-0" above the water line at the time of construction.

Construction Sequence for Web Wall:

1. Excavate between shafts to elevation of web wall base and set lower web wall forms through water to bear on the circular edge of drilled shafts. Secure in place with fill, struts or tie forms together as required.
2. Place the lower web wall reinforcement cage into the forms using spacers to maintain proper clearances.
3. If the forms can be sealed against the shafts and streambed to allow dewatering, the reinforcement and the concrete placement may be completed in the dry. Alternatively, the rebar cage can be lowered into position through water and the concrete discharged at the base of the excavation through a tremie pipe or pump hose, displacing water, sediment, and tainted concrete out the top of the forms.
4. Construct columns.
5. Construct upper web walls.

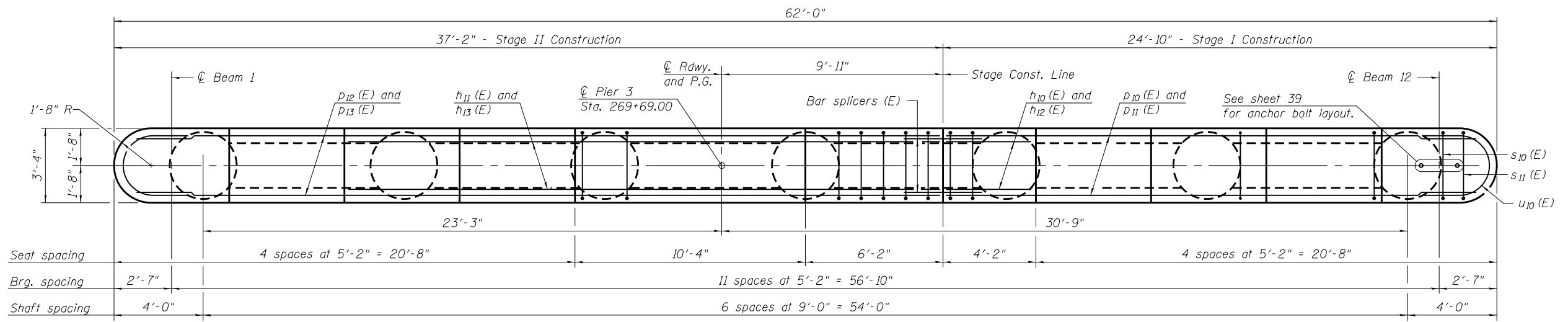
* If the prevailing water surface elevation during construction is consistently different than estimated on the plans, the contractor may propose an adjustment to the top of the drilled shaft elevation as part of their installation procedure. The top of all drilled shafts within a substructure unit shall be constructed to the same elevation and extend above the prevailing water surface. The quantities and reinforcement detailing are based on the top of shaft and the estimated elevations shown and may change based on the actual elevations encountered at each shaft and the final top of shaft elevation.

** Length is height of spiral. Minimum lap for spirals = 3'-0".

*** The Contractor is responsible for determining the casing thickness and the actual bottom of casing elevation, top of encountered rock, to be used at each drilled shaft. See Article 516.06(d) of the Standard Specifications. The limits of casing shall be adjusted as necessary, and as approved, such that the actual installed casing length extends to the as-encountered top of rock at each shaft. Pay limits for the Permanent Casing are based on the minimum length shown.

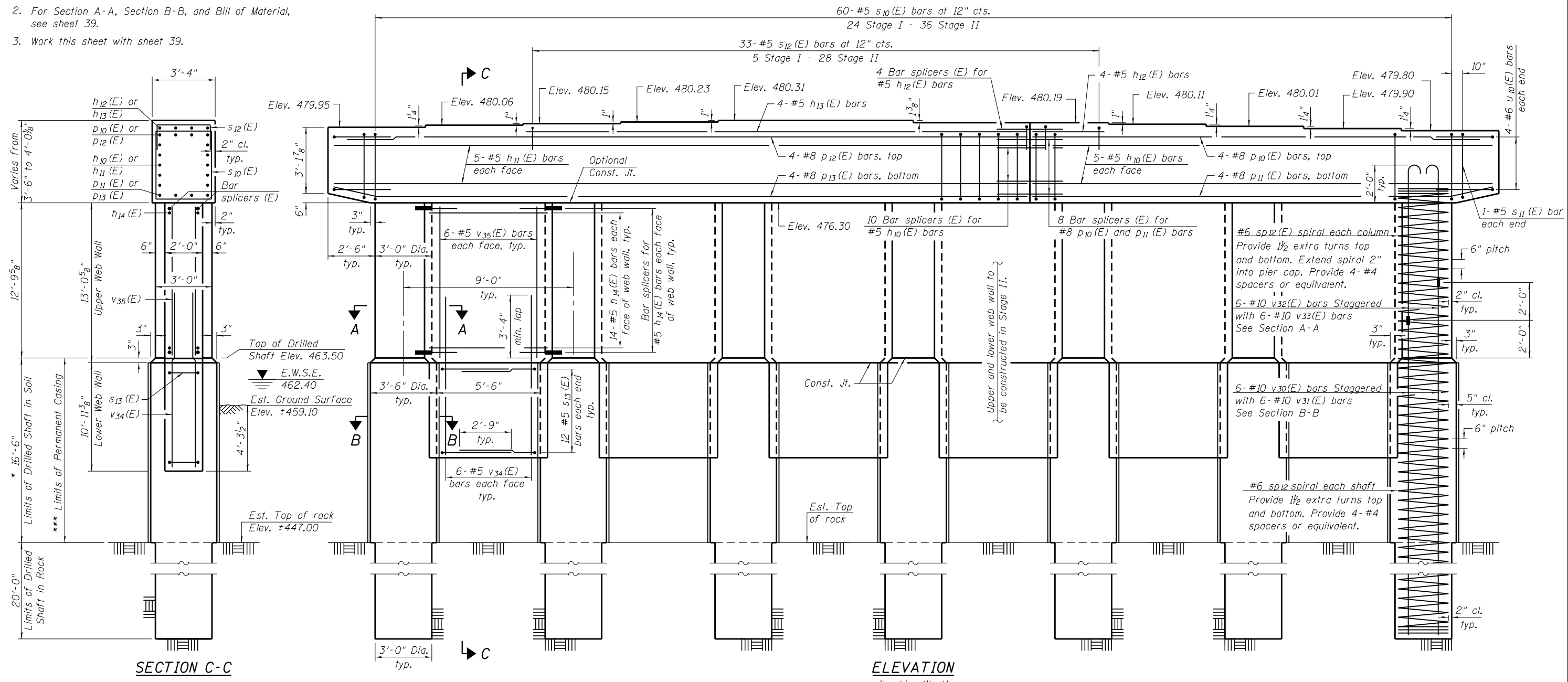
BILL OF MATERIAL
PIER 2

Bar	No.	Size	Length	Shape
h ₁₀ (E)	10	#5	23'-0"	—
h ₁₁ (E)	10	#5	35'-4"	—
h ₁₂ (E)	4	#5	3'-10"	—
h ₁₃ (E)	4	#5	26'-6"	—
h ₁₄ (E)	156	#5	5'-8"	—
p ₁₀ (E)	4	#8	23'-0"	—
p ₁₁ (E)	4	#8	23'-6"	—
p ₁₂ (E)	4	#8	35'-4"	—
p ₁₃ (E)	4	#8	35'-10"	—
sp ₁₁	7	#6	44'-6"	⋈
sp ₁₁ (E)	7	#6	12'-3"	⋈
s ₁₀ (E)	60	#5	13'-3"	□
s ₁₁ (E)	2	#5	13'-1"	□
s ₁₂ (E)	33	#5	6'-10"	U
s ₁₃ (E)	144	#5	9'-8"	U
u ₁₀ (E)	8	#6	12'-2"	U
v ₂₀ (E)	42	#10	46'-6"	—
v ₂₁ (E)	42	#10	48'-6"	—
v ₂₂ (E)	42	#10	13'-6"	—
v ₂₃ (E)	42	#10	11'-6"	—
v ₂₄ (E)	72	#5	14'-4"	—
v ₂₅ (E)	72	#5	12'-3"	—
Structure Excavation		Cu. Yd.	30	
Concrete Structures		Cu. Yd.	110.4	
Reinforcement Bars		Pound	7,930	
Reinforcement Bars, Epoxy Coated		Pound	31,660	
Bar Splicers		Each	334	
Mechanical Splicers		Each	84	
Permanent Casing		Foot	170	
Drilled Shaft in Soil		Cu. Yd.	61.2	
Drilled Shaft in Rock		Cu. Yd.	36.7	



- Notes:
1. See sheet 42 for bar splicers and mechanical splicers.
 2. For Section A-A, Section B-B, and Bill of Material, see sheet 39.
 3. Work this sheet with sheet 39.

TOP PLAN

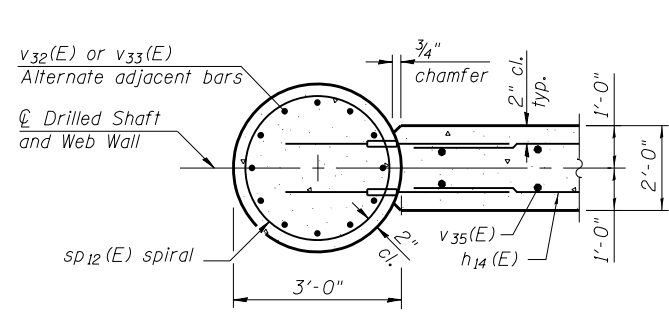


SECTION C-C

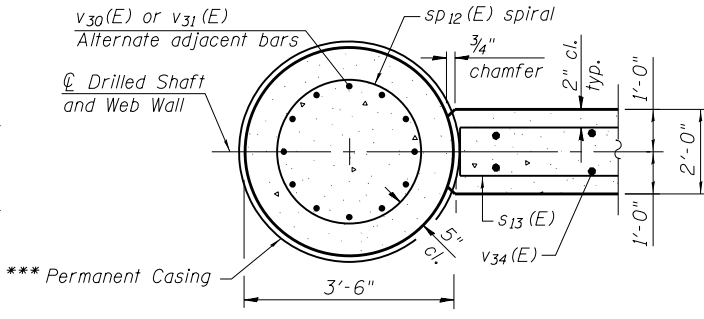
ELEVATION
(Looking West)

USER NAME = *OPERATOR*	DESIGNED - TJZ	REVISED
PLOT SCALE = 0.2" = 1'-0"	CHECKED - SBC	REVISED
PLOT DATE = 8/16/2018	DRAWN - DLH	REVISED
	CHECKED - SBC	REVISED

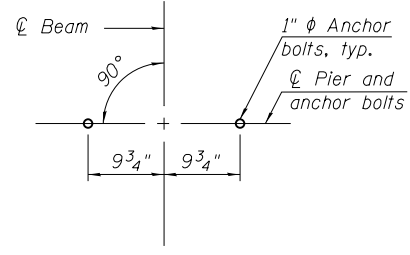
F.A.U. RT.E.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	452
CONTRACT NO. 68185				



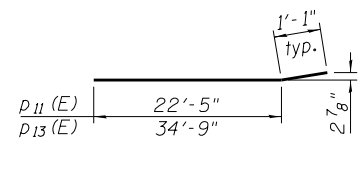
SECTION A-A



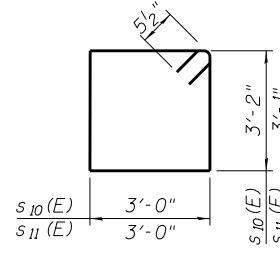
SECTION B-B



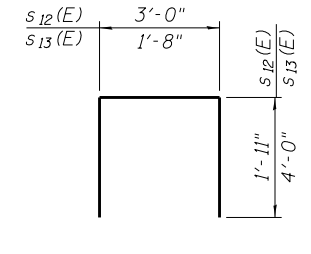
ANCHOR BOLT LAYOUT



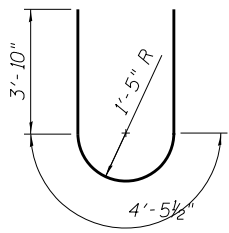
BARS p₁₁(E) and p₁₃(E)



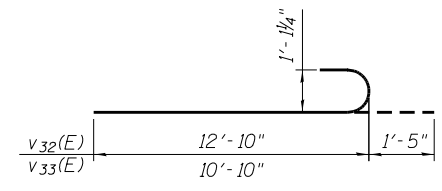
BARS s₁₀(E) and s₁₁(E)



BARS s₁₂(E) and s₁₃(E)



BAR u₁₀(E)



BARS v₃₂(E) & v₃₃(E)

**BILL OF MATERIAL
PIER 3**

Bar	No.	Size	Length	Shape
h ₁₀ (E)	10	#5	23'-0"	—
h ₁₁ (E)	10	#5	35'-4"	—
h ₁₂ (E)	4	#5	3'-10"	—
h ₁₃ (E)	4	#5	26'-6"	—
h ₁₄ (E)	168	#5	5'-8"	—
p ₁₀ (E)	4	#8	23'-0"	—
p ₁₁ (E)	4	#8	23'-6"	—
p ₁₂ (E)	4	#8	35'-4"	—
p ₁₃ (E)	4	#8	35'-10"	—
sp ₁₂	7	#6	36'-6"	⋈
sp ₁₂ (E)	7	#6	13'-0"	⋈
s ₁₀ (E)	60	#5	13'-3"	□
s ₁₁ (E)	2	#5	13'-1"	□
s ₁₂ (E)	33	#5	6'-10"	U
s ₁₃ (E)	144	#5	9'-8"	U
u ₁₀ (E)	8	#6	12'-2"	U
v ₃₀ (E)	42	#10	38'-6"	—
v ₃₁ (E)	42	#10	40'-6"	—
v ₃₂ (E)	42	#10	14'-3"	U
v ₃₃ (E)	42	#10	12'-3"	U
v ₃₄ (E)	72	#5	14'-4"	—
v ₃₅ (E)	72	#5	13'-0"	—
Structure Excavation		Cu. Yd.	30	
Concrete Structures		Cu. Yd.	114.0	
Reinforcement Bars		Pound	6,550	
Reinforcement Bars, Epoxy Coated		Pound	29,300	
Bar Splicers		Each	358	
Mechanical Splicers		Each	84	
Permanent Casing		Foot	114	
Drilled Shaft in Soil		Cu. Yd.	41.2	
Drilled Shaft in Rock		Cu. Yd.	36.7	

Notes:

- Shafts must be poured the same day as the rock socket excavation.
- Pour steps monolithically with cap.
- Space cap reinforcement to miss anchor bolts.
- If a portion of the drilled shaft web walls or concrete encasement is under water, reinforcement may be placed underwater into forms. Concrete shall be tremied according to Article 503.08 of the Standard Specifications to an elevation of 1'-0" above the water line at the time of construction.

Construction Sequence for Web Wall:

- Excavate between shafts to elevation of web wall base and set lower web wall forms through water to bear on the circular edge of drilled shafts. Secure in place with fill, struts or tie forms together as required.
- Place the lower web wall reinforcement cage into the forms using spacers to maintain proper clearances.
- If the forms can be sealed against the shafts and streambed to allow dewatering, the reinforcement and the concrete placement may be completed in the dry. Alternatively, the rebar cage can be lowered into position through water and the concrete discharged at the base of the excavation through a tremie pipe or pump hose, displacing water, sediment, and tainted concrete out the top of the forms.
- Construct columns.
- Construct upper web walls.

* If the prevailing water surface elevation during construction is consistently different than estimated on the plans, the contractor may propose an adjustment to the top of the drilled shaft elevation as part of their installation procedure. The top of all drilled shafts within a substructure unit shall be constructed to the same elevation and extend above the prevailing water surface. The quantities and reinforcement detailing are based on the top of shaft and the estimated elevations shown and may change based on the actual elevations encountered at each shaft and the final top of shaft elevation.

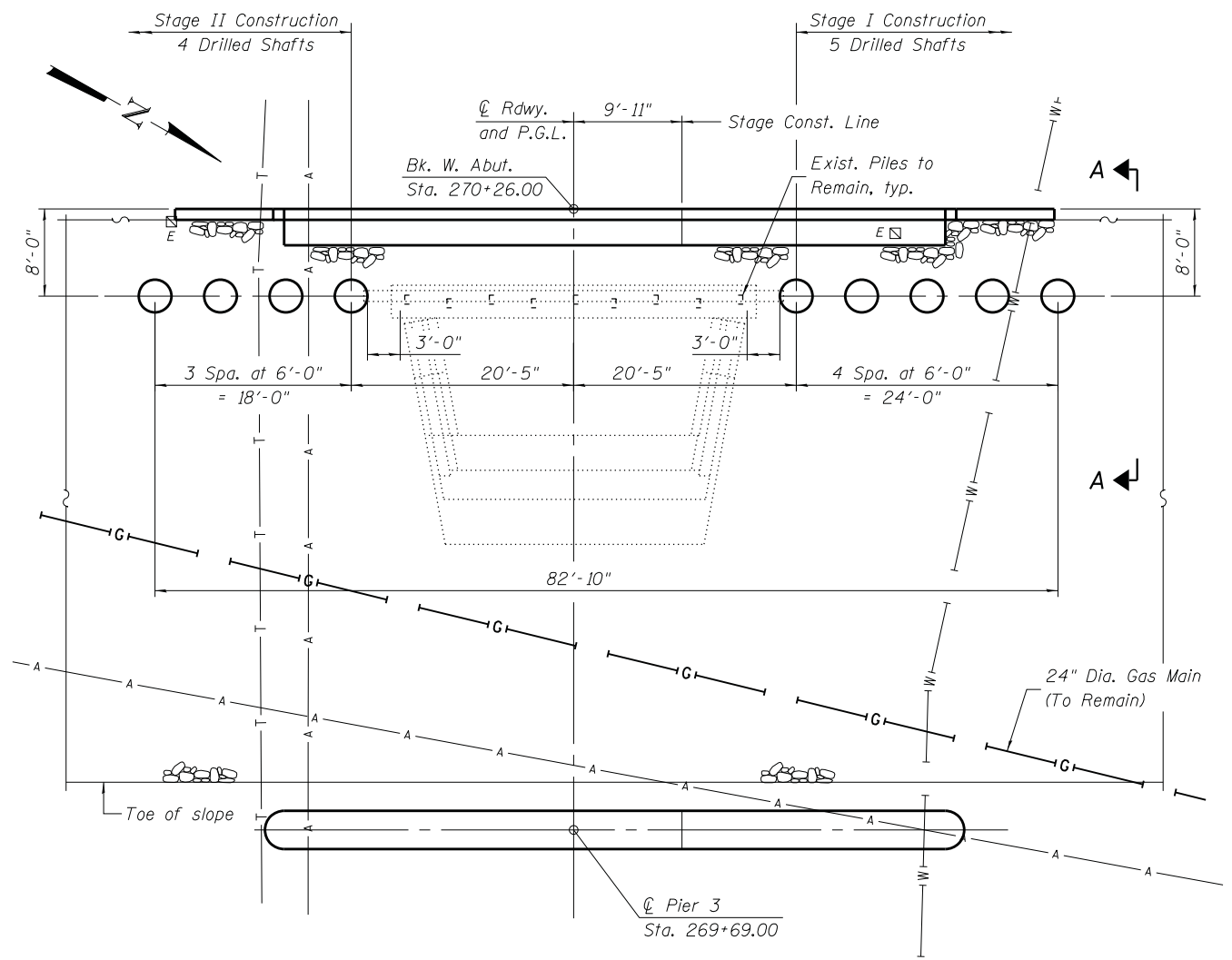
** Length is height of spiral. Minimum lap for spirals = 3'-0".

*** The Contractor is responsible for determining the casing thickness and the actual bottom of casing elevation, top of encountered rock, to be used at each drilled shaft. See Article 516.06(d) of the Standard Specifications. The limits of casing shall be adjusted as necessary, and as approved, such that the actual installed casing length extends to the as-encountered top of rock at each shaft. Pay limits for the Permanent Casing are based on the minimum length shown.

USER NAME = *OPERATOR*	DESIGNED - TJZ	REVISED
	CHECKED - SBC	REVISED
PLOT SCALE = 0.2" = 1' / 16"	DRAWN - DLH	REVISED
PLOT DATE = 8/16/2018	CHECKED - SBC	REVISED

F.A.U. RT.E.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	453
CONTRACT NO. 68185				
ILLINOIS FED. AID PROJECT				

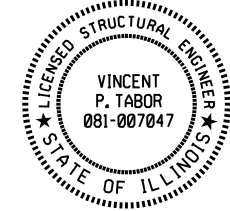
LAST SAVED DATE: 8/13/2018



LEGEND

- Aerial Power Line — A —
- Gas Line — G —
- Telephone Cable — T —
- Water Pipe — W —
- Power Pole — □ —
- Water Meter Valve Box — ○^{WV} —
- Gas Meter Valve Box — ○^{GV} —
- Electrical Junction Box — □^E —

PLAN

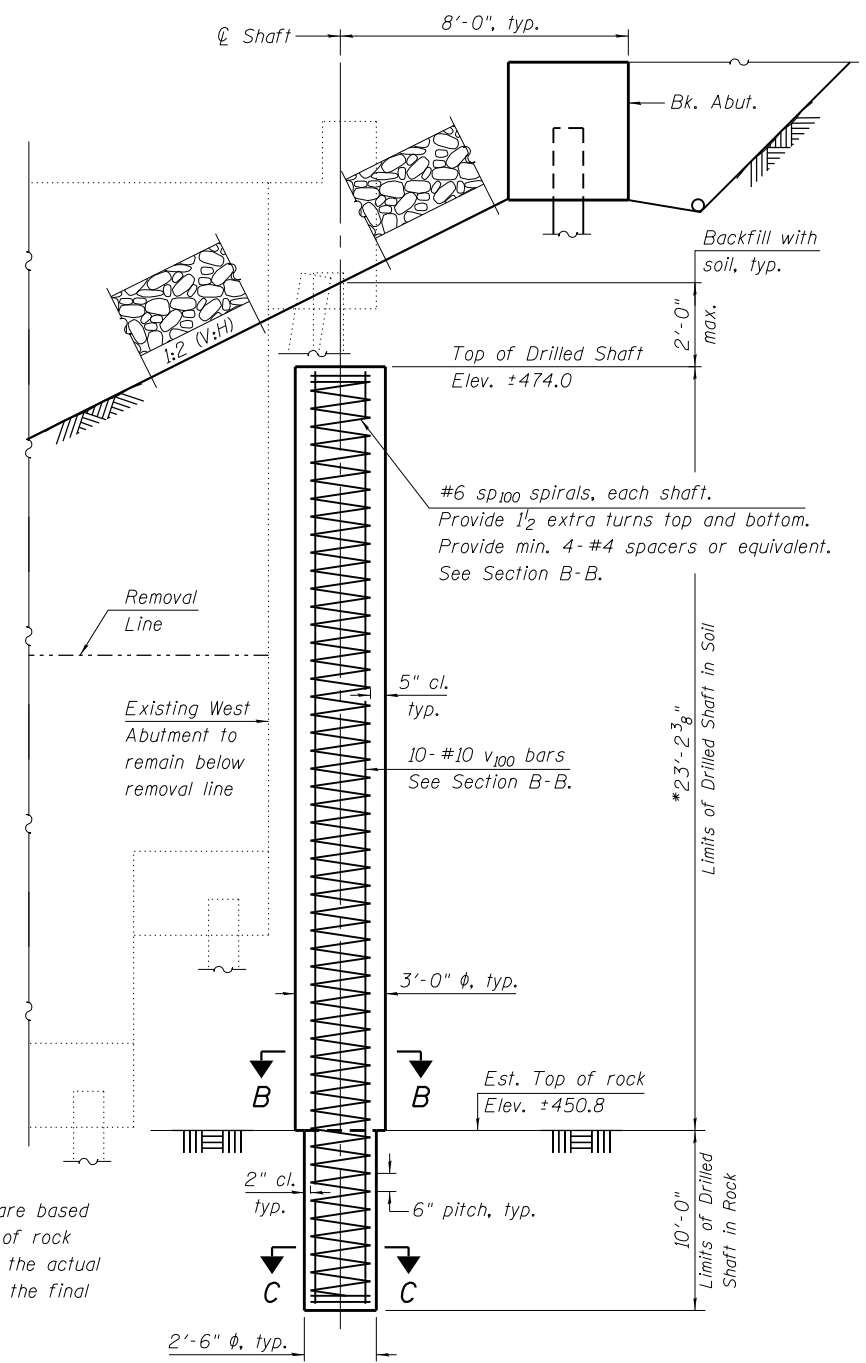
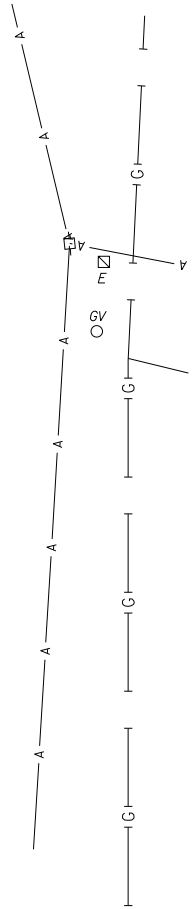


Seal applies to this sheet only.

Vincent P. Tabor 8/13/2018
 Vincent P. Tabor
 Licensed Structural Engineer
 State of Illinois No. 081-007047
 Expires 11/30/2018

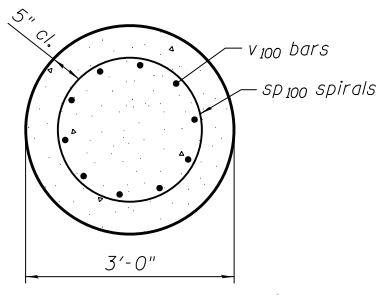
Notes:

1. All utilities shown in plan view shall be relocated by others, unless noted otherwise.
2. The drilled shafts shall be installed and achieve a compressive strength of at least 3,500 psi prior to final grading of the slope, placement of riprap on the slope, and driving of piles for the West Abutment within the same Stage of Construction.

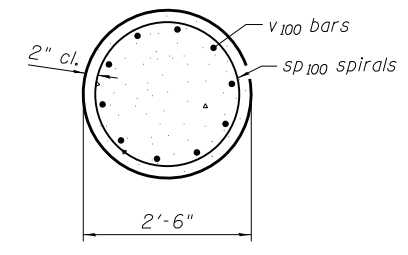


VIEW A-A

*The quantities and reinforcement detailing are based on the top of shaft and the estimated top of rock elevations shown and may change based on the actual top of rock encountered at each shaft and the final top of shaft elevation.



SECTION B-B



SECTION C-C

BILL OF MATERIAL
SLOPE STABILITY DRILLED SHAFTS

Bar	No.	Size	Length	Shape	
** SP 100	9	#6	32'-9"	⋈	
V 100	90	#10	32'-9"	—	
Reinforcement Bars				Pound	19,330
Drilled Shaft in Soil				Cu. Yd.	54.7
Drilled Shaft in Rock				Cu. Yd.	16.4

** Length is height of spiral.
 Minimum lap for spirals = 3'-0".



USER NAME = Lin_31	DESIGNED - VPT	REVISED
PLLOT SCALE = 0.2" = 1'-0"	CHECKED - MTH	REVISED
PLLOT DATE = 8/13/2018	DRAWN - VPT	REVISED
	CHECKED - MTH	REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

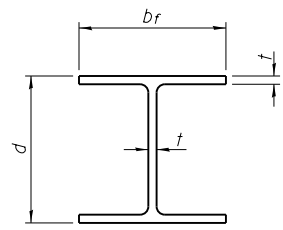
SLOPE STABILITY DRILLED SHAFTS
STRUCTURE NO. 072-0245

SHEET NO. 40 OF 52 SHEETS

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	454
CONTRACT NO. 68185				

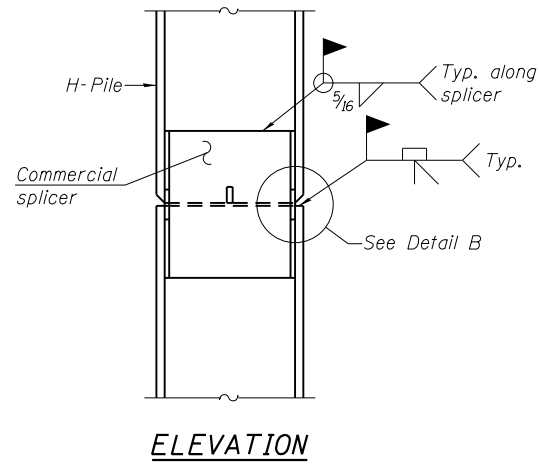
ILLINOIS FED. AID PROJECT

FILE NAME = 0720245-68185_Slope Stability Drilled Shafts.dgn

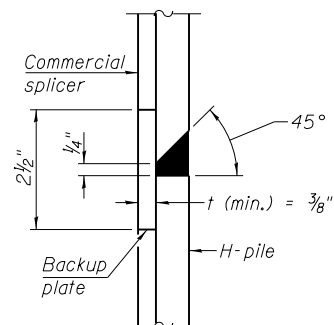


STEEL PILE TABLE

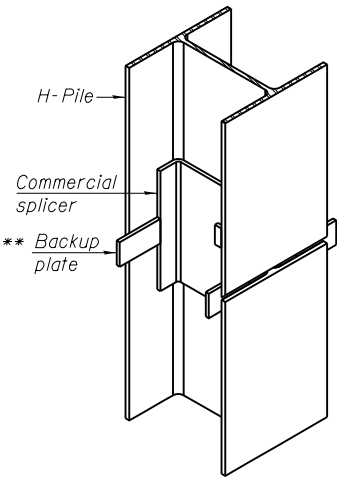
Designation	Depth d	Flange width br	Web and Flange thickness t	Encasement diameter A
HP 14x117	14 1/4"	14 7/8"	1 3/16"	30"
x102	14"	14 3/4"	1/16"	30"
x89	13 7/8"	14 3/4"	5/8"	30"
x73	13 5/8"	14 5/8"	1/2"	30"
HP 12x84	12 1/4"	12 1/4"	1/16"	24"
x74	12 1/8"	12 1/4"	5/8"	24"
x63	12"	12 1/8"	1/2"	24"
x53	11 3/4"	12"	7/16"	24"
HP 10x57	10"	10 1/4"	9/16"	24"
x42	9 3/4"	10 1/8"	7/16"	24"
HP 8x36	8"	8 1/8"	7/16"	18"



ELEVATION

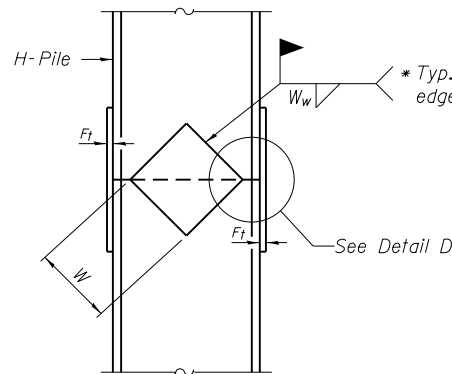


DETAIL "B"

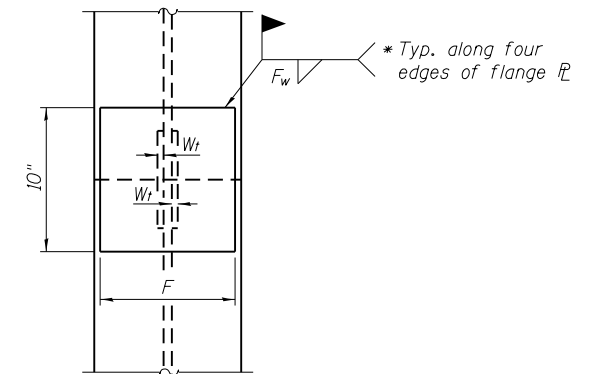


ISOMETRIC VIEW

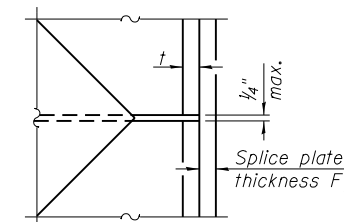
WELDED COMMERCIAL SPLICE



ELEVATION



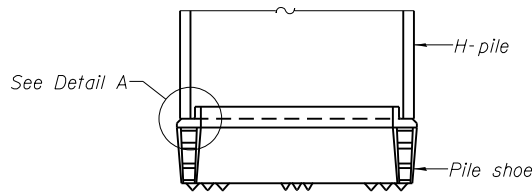
END VIEW



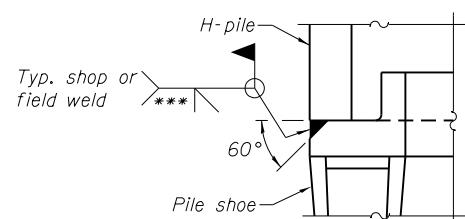
DETAIL D

WELDED PLATE FIELD SPLICE

Designation	F	F _t	F _w	W	W _t	W _w
HP 14x117	12 1/2"	1"	7/8"	7 3/4"	5/8"	1/2"
x102	12 1/2"	7/8"	3/4"	7 3/4"	5/8"	1/2"
x89	12 1/2"	3/4"	1/16"	7 3/4"	5/8"	1/2"
x73	12 1/2"	5/8"	9/16"	7 3/4"	5/8"	1/2"
HP 12x84	10"	7/8"	1/16"	6 1/2"	5/8"	1/2"
x74	10"	7/8"	1/16"	6 1/2"	5/8"	1/2"
x63	10"	5/8"	1/2"	6 1/2"	1/2"	3/8"
x53	10"	5/8"	1/2"	6 1/2"	1/2"	3/8"
HP 10x57	8"	3/4"	9/16"	5 1/4"	1/2"	3/8"
x42	8"	5/8"	9/16"	5 1/4"	1/2"	3/8"
HP 8x36	7"	5/8"	7/16"	4 1/4"	1/2"	3/8"

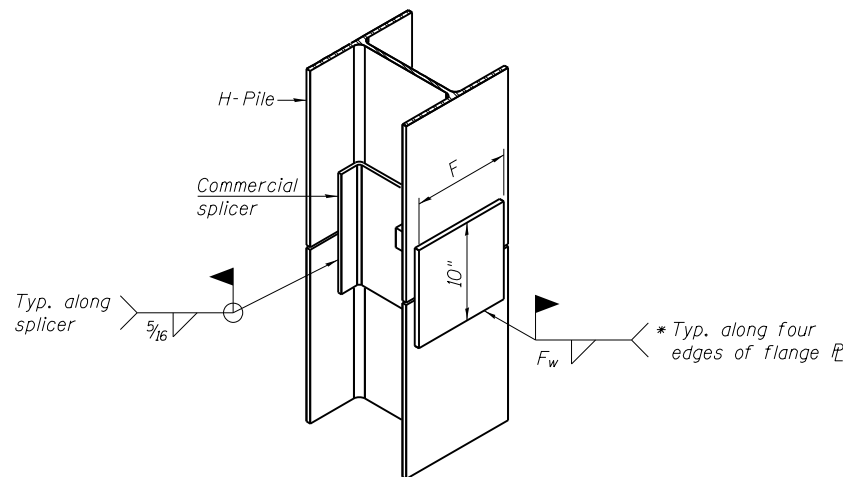


ELEVATION



DETAIL A

H-PILE SHOE ATTACHMENT

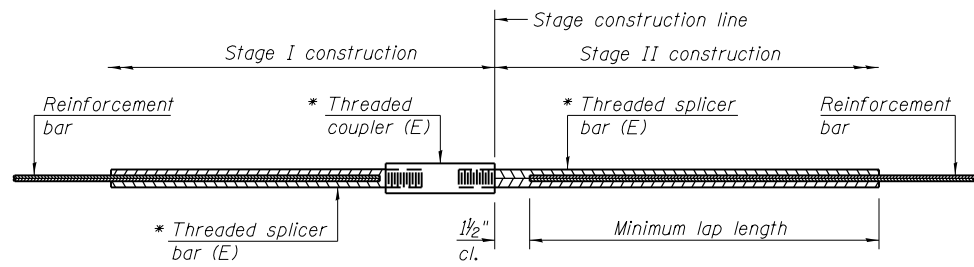


ISOMETRIC VIEW

WELDED COMMERCIAL SPLICE ALTERNATE

- * Interrupt welds 1/4" from end of web and/or each flange.
- ** Remove portions of backup plates that extend outside the flanges.
- *** Weld size per pile shoe manufacturer (5/16" min.).

Note:
The steel H-piles shall be according to AASHTO M270 Grade 50.

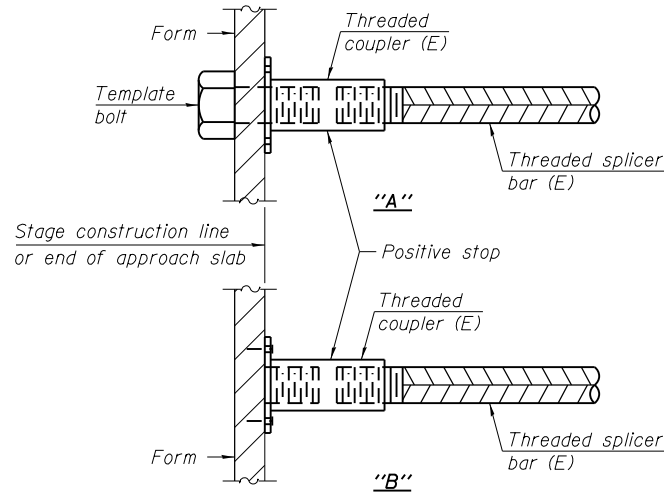


STANDARD BAR SPLICER ASSEMBLY

Threaded splicer bar length = min. lap length + 1/2" + thread length

* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

Location	Bar size	No. assemblies required	Minimum lap length
Deck	#5	767	3'-2"
Abutment Diaphragm	#6	14	4'-4"
Approach CWS	#4	62	2'-11"
Approach Footing	#5	80	3'-2"
Abutment	#5	8	3'-2"
Abutment	#7	20	5'-6"
Pier Cap	#5	42	3'-2"
Pier Cap	#8	24	7'-2"
Pier Web Wall	#5	864	3'-2"

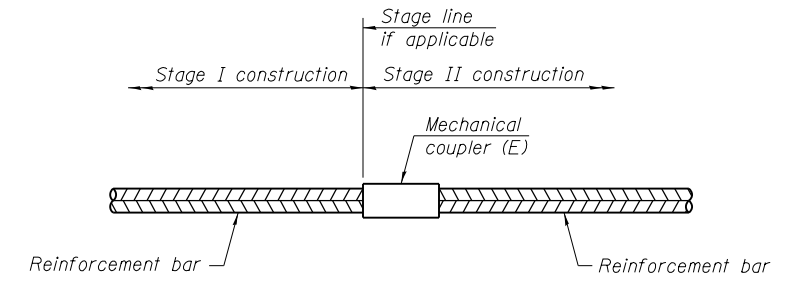


INSTALLATION AND SETTING METHODS

"A" : Set bar splicer assembly by means of a template bolt.

"B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E) : Indicates epoxy coating.



STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required
Pier	#10	252

NOTES

1. Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.
2. All reinforcement shall be lapped and tied to the splicer bars.
3. Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications.
4. See approved list of bar splicer assemblies and mechanical splicers for alternatives.

USER NAME = *OPERATOR*	DESIGNED - TJZ	REVISED
	CHECKED - SBC	REVISED
PLOT SCALE = 0.2" = 1'-0"	DRAWN - DLH	REVISED
PLOT DATE = 8/16/2018	CHECKED - SBC	REVISED

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N,BR-1,RS-4,W-1)	PEORIA	577	456
CONTRACT NO. 68185				



Illinois Department of Transportation
Division of Highways
SCI Engineering

SOIL BORING LOG

Date 12/07/10

ROUTE FAU 6659 DESCRIPTION Structure Replacement - Farmington Road over Kickapoo Creek LOGGED BY KEG

SECTION 111, 11BR-1 LOCATION Limestone Township; SW1/4, SEC. 1, TWP. 8N, RNG. 7E

COUNTY Peoria DRILLING METHOD CME 55LC w/HSA HAMMER TYPE Automatic

STRUCT. NO. 072-0063 (ex.); 072-00XX (prop.)
Station _____
BORING NO. B-1 (E. Abut)
Station 267+50
Offset 36 ft Lt
Ground Surface Elev. 474.53 ft

DEPTH (ft)	SOIL TYPE	UCS (tsf)	MOISTURE (%)	DEPTH (ft)	SOIL TYPE	UCS (tsf)	MOISTURE (%)
471.5	FILL: Brown, sandy loam, fine grain (A-2)			471.5	LOAM: Brown (A-4) (continued)		
5				1			
3				4		<0.25	21
3				2		P	
469.0	FILL: Brown, clay loam, trace fine gravel (A-6)			469.0	SAND: Brown, fine to coarse, trace fine gravel (A-1)		
3				2			
2		2.5	13	1			
1		P		2			
469.0	FILL: Gray, silty clay (A-6)			469.0	Mud rotary drilling started at 23 feet. Hollow stem augers advanced after coarse gravel encountered.		
2				3			
2		0.7	24	3			
3		S/10		6			
464.0	LL-40, PL-24, PI-16			6			
2				10			
2		0.8	26	10			
4		P		30			
464.0	FILL: Brown, sandy clay loam (A-6)			464.0			
5							
4		2.8	16				
5		P					
461.5	SILTY CLAY LOAM: Brown (A-6)			461.5			
2				27			
2		0.3	25	27			
4		S/10		25			
456.5	LOAM: Brown (A-4)			456.5	SAND: Brown, fine to medium (A-3)		
WH				10			
WH				12			
1			19	15			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)



Illinois Department of Transportation
Division of Highways
SCI Engineering

SOIL BORING LOG

Date 12/07/10

ROUTE FAU 6659 DESCRIPTION Structure Replacement - Farmington Road over Kickapoo Creek LOGGED BY KEG

SECTION 111, 11BR-1 LOCATION Limestone Township; SW1/4, SEC. 1, TWP. 8N, RNG. 7E

COUNTY Peoria DRILLING METHOD CME 55LC w/HSA HAMMER TYPE Automatic

STRUCT. NO. 072-0063 (ex.); 072-00XX (prop.)
Station _____
BORING NO. B-1 (E. Abut)
Station 267+50
Offset 36 ft Lt
Ground Surface Elev. 474.53 ft

DEPTH (ft)	SOIL TYPE	UCS (tsf)	MOISTURE (%)	DEPTH (ft)	SOIL TYPE	UCS (tsf)	MOISTURE (%)
433.0	SAND: Brown, fine to medium (A-3) (continued)			433.0	Becomes organish brown		
4				4			
22				22			
50	CLAYEY SHALE: Gray			50			
428.3	Auger refusal at 46.2 feet.			428.3			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)

Design firm no. 184001036



USER NAME = *OPERATOR*	DESIGNED - TJZ	REVISED
	CHECKED - SBC	REVISED
PLOT SCALE = 0.2" = 1'	DRAWN - DLH	REVISED
PLOT DATE = 8/16/2018	CHECKED - SBC	REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SOIL BORING LOGS
STRUCTURE NO. 072-0245

SHEET NO. 43 OF 52 SHEETS

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	457
			CONTRACT NO. 68185	

ILLINOIS FED. AID PROJECT



Illinois Department of Transportation
 Division of Highways
 SCI Engineering

SOIL BORING LOG

Date 12/7, 8/2010

ROUTE FAU 6659 DESCRIPTION Structure Replacement - Farmington Road over Kikapoo Creek LOGGED BY KEG

SECTION 111, 11BR-1 LOCATION Limestone Township; SW1/4, SEC. 1, TWP. 8N, RNG. 7E

COUNTY Peoria DRILLING METHOD CME 55LC w/HSA HAMMER TYPE Automatic

STRUCT. NO. <u>072-0063 (ex.);</u>		D E P T H	B L O S	U C S	M O I S T	Surface Water Elev. _____ ft	
Station <u>072-00XX (prop.)</u>						Stream Bed Elev. _____ ft	
BORING NO. <u>B-3 (Pier 2)</u>						Groundwater Elev.:	
Station <u>268+65</u>						First Encounter <u>454.1 ft</u> ▼	
Offset <u>39 ft Lt</u>		Upon Completion <u>454.1 ft</u> ▼		After - Hrs. _____		(ft) (/6") (tsf) (%)	
Ground Surface Elev. <u>469.56 ft</u>							

Soil Description	Depth (ft)	Classification	Moisture (%)	Penetration (tsf)
SILTY LOAM: Brown (A-4)	0 - 2	LL-30, PL-21, PI-9		
	2 - 3	P	1.5	23
	465.6 - 464.1			
SANDY LOAM: Brown, trace fine gravel (A-4)	465.6 - 464.1		2.0	13
	464.1 - 461.6			
SILTY CLAY: Brown (A-6)	461.6 - 461.6		2.5	25
	461.6 - 461.6			
SILTY LOAM: Gray (A-4)	461.6 - 449.1		1.0	28
	449.1 - 441.6			
SAND: Brown, fine (A-3)	441.6 - 439.1			
CLAYEY SHALE: Gray	439.1 - 456.6		0.3	34
	456.6 - 454.1			
SILTY CLAY LOAM: Gray (A-6)	454.1 - 454.1		0.4	33
	454.1 - 451.6			
SAND: Dark gray, medium to coarse, trace fine gravel (A-1)	451.6 - 451.6			
	451.6 - 451.6		0.3	35
SILTY LOAM: Gray (A-4)	451.6 - 451.6			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)



Illinois Department of Transportation
 Division of Highways
 SCI Engineering

SOIL BORING LOG

Date 12/7, 8/2010

ROUTE FAU 6659 DESCRIPTION Structure Replacement - Farmington Road over Kikapoo Creek LOGGED BY KEG

SECTION 111, 11BR-1 LOCATION Limestone Township; SW1/4, SEC. 1, TWP. 8N, RNG. 7E

COUNTY Peoria DRILLING METHOD CME 55LC w/HSA HAMMER TYPE Automatic

STRUCT. NO. <u>072-0063 (ex.);</u>		D E P T H	B L O S	U C S	M O I S T	Surface Water Elev. _____ ft	
Station <u>072-00XX (prop.)</u>						Stream Bed Elev. _____ ft	
BORING NO. <u>B-3 (Pier 2)</u>						Groundwater Elev.:	
Station <u>268+65</u>						First Encounter <u>454.1 ft</u> ▼	
Offset <u>39 ft Lt</u>		Upon Completion <u>454.1 ft</u> ▼		After - Hrs. _____		(ft) (/6") (tsf) (%)	
Ground Surface Elev. <u>469.56 ft</u>							

Soil Description	Depth (ft)	Classification	Moisture (%)	Penetration (tsf)
CLAYEY SHALE: Gray (continued)	0 - 38			
	38 - 50/4"		0.5	21
	50 - 50/2"			
	50/2" - 50/5"			
Boring terminated at 46.6 ft.	50/2" - 50/2"			17

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)



USER NAME = *OPERATOR*	DESIGNED - TJZ	REVISED
CHECKED - SBC	REVISED	
PLOT SCALE = 0x2" = 1' / 10"	DRAWN - DLH	REVISED
PLOT DATE = 8/16/2018	CHECKED - SBC	REVISED

**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**SOIL BORING LOGS
 STRUCTURE NO. 072-0245**

SHEET NO. 45 OF 52 SHEET

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	459
CONTRACT NO. 68185				
ILLINOIS FED. AID PROJECT				



Illinois Department of Transportation
 Division of Highways
 SCI Engineering

SOIL BORING LOG

Date 12/08/10

ROUTE FAU 6659 DESCRIPTION Structure Replacment - Farmington Road over Kickapoo Creek LOGGED BY KEG

SECTION 11I, 11BR-1 LOCATION Limestone Township; SW1/4, SEC. 1, TWP. 8N, RNG. 7E

COUNTY Peoria DRILLING METHOD CME 55LC w/HSA HAMMER TYPE Automatic

STRUCT. NO. 072-0063 (ex.); 072-00XX (prop.)
 Station _____
 BORING NO. B-4 (W. Abut)
 Station 270+19
 Offset 26 ft Rt
 Ground Surface Elev. 479.30 ft

DEPTH (ft)	SOIL DESCRIPTION	UNCONSOLIDATED SOIL TESTS	MOISTURE (%)	WATER CONTENT (%)	DEPTH (ft)	SOIL DESCRIPTION	UNCONSOLIDATED SOIL TESTS	MOISTURE (%)	WATER CONTENT (%)
458.8	FILL: Brown, fine to medium sand with silt lumps (A-3)			8	458.8	CLAY LOAM: Gray (A-7)	WH		21
3					2		0.3		
3					5		B		
475.3	FILL: Brown, sandy loam, trace fine gravel (A-2)			10	456.3	CLAYEY SHALE: Greenish gray			
4					7				
4					19				19
-5					-25				
	Trace coarse gravel			12					
4					26				
3		1.0			43				16
4		P			50/3"				
471.3	FILL: Brown, clay loam, trace fine gravel (A-6)			17					
2		0.1			50/2"				
2		S/10			50				16
-10					-30				
468.8	SANDY LOAM: Brown (A-4)			18					
5		1.3							
4		P							
5									
466.3	CLAY LOAM: Brown (A-7)			20					
2		0.2			50				
1		B			50/3"				18
-15					-35				
463.8	SILTY CLAY: Dark brown (A-7)			30					
	LL-49, PL-29, PI-20								
		0.5							
		P							
	Becomes brown			27					
3		0.5			50				
3		B			50/1"				13
4									
-20					-40				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 AASHTO Classifications are based on visual classifications unless otherwise noted BBS, form 137 (Rev. 8-99)



USER NAME = *OPERATOR*	DESIGNED - TJZ	REVISED
CHECKED - SBC	REVISED	
PLOT SCALE = 0.2" / 1 ft.	DRAWN - DLH	REVISED
PLOT DATE = 8/16/2018	CHECKED - SBC	REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SOIL BORING LOGS
STRUCTURE NO. 072-0245
 SHEET NO. 46 OF 52 SHEETS

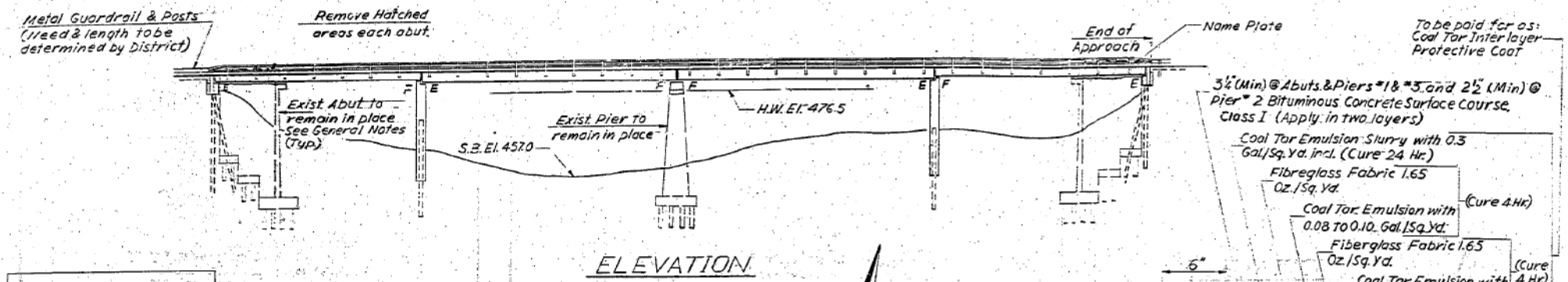
F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	460
CONTRACT NO. 68185				

LAST SAVED DATE: 8/15/2018

STATE OF ILLINOIS
DEPARTMENT OF PUBLIC WORKS & BUILDINGS
DIVISION OF HIGHWAYS

PROJECT NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
0720245	11R	PEORIA	10	3

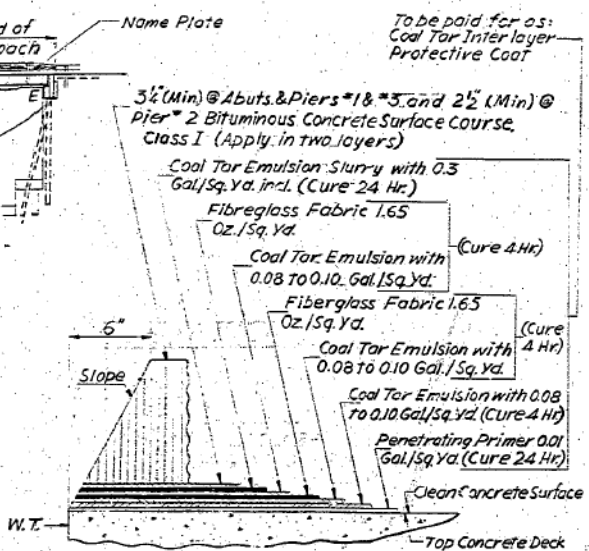
Left Sta. 270+15, Chiseled Square, S.W. Corner of S.W. Wing of W. abutment Elev. 481.64
S.W. Structure built in 1922 as S.B.I. Rte 8, Sec. 11B & 11C
Exist. Structure - 2-100' span trusses at sta. 269+00, Superstructure - 2-100' span trusses on conc. closed abuts. and solid pier.
Superstructure and abut. parapet walls to be removed by Bridge Contractor. No salvage. Top of existing pier and abutment to be removed by Bridge Contractor.



ELEVATION

STATION 269+00
REBUILT 197 BY
STATE OF ILLINOIS
S.B.I. RT. 8 SEC. 11 BR
LOADING HS 20

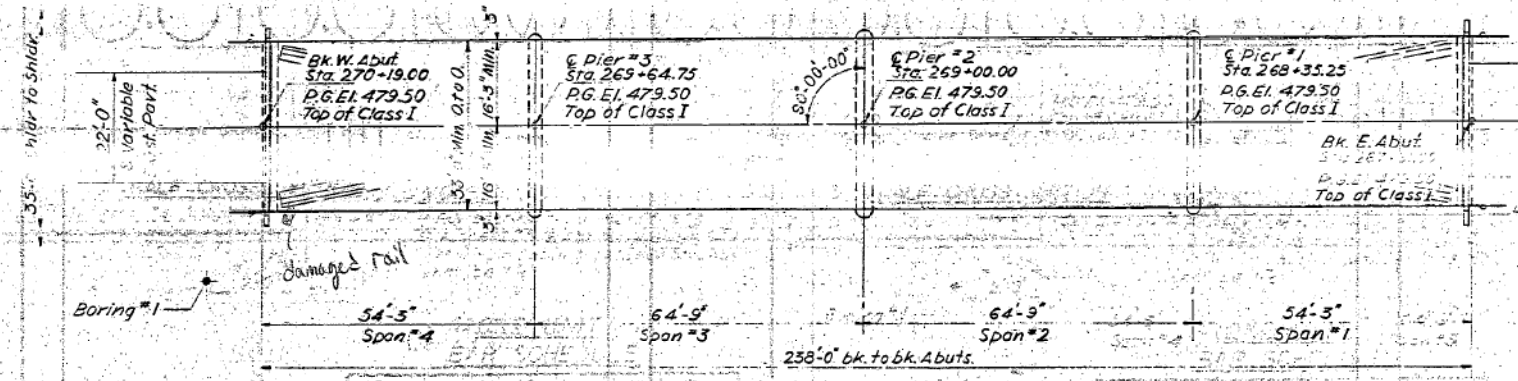
NAME PLATE
(See Standard 2113-1)



DETAIL OF DECK SURFACING

GENERAL NOTES

All reinforcement bars shall be lapped 24 diameters unless otherwise shown.
It shall be the responsibility of the contractor to verify all dimensions and conditions existing in the field prior to construction and ordering of materials.
The contractor shall drive 1 steel Test Pile each of new abutments and piers, in a permanent location as directed by the Engineer before ordering the remainder of piles.
Limits of Coal Tar Intlayer Protective Coat shall be one foot beyond end of deck beams and out to out of deck.
Existing abutments partially removed to provide minimum 5" clearance between underside of 21" Precast section and top of abutments. Cost shall be considered incidental to the Contract Unit price for class X concrete.



PLAN

TOTAL BILL OF MATERIAL

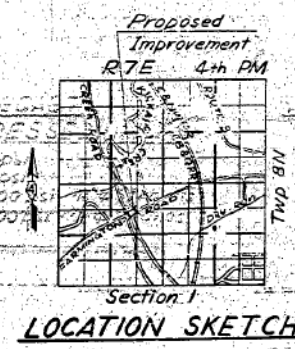
ITEM	UNIT	SUPER	SUB	TOTAL
Bituminous Concrete Surface Course, Class I	Top	146	-	146
Removal of Existing Superstructures	Each	1	-	1
Concrete Removal	Cu. Yd.	-	12	12
Precast Prestressed Concrete Deck Beam 27" Depth	Sq. Ft.	4273	-	4273
Precast Prestressed Concrete Deck Beam 21" Depth	Sq. Ft.	3471	-	3471
Reinforcement Bars	Pound	-	9777	9777
Name Plates	Each	1	-	1
Coal Tar Interlayer Protective Coat	Sq. Yd.	921	-	921
Pavement Removal & P.C.C. Replacement, Type II (10')	Sq. Yd.	10	-	10
Steel Railing, Type N	Lin. Ft.	470	-	470
Preformed Joint Sealer	Lin. Ft.	152	-	152
Steel Piles (HP 10X42)	Lin. Ft.	-	856	856
Test Piles Steel (HP 10X42)	Each	-	4	4

DESIGN STRESSES
FIELD UNITS
SUPERSTRUCTURE
fc = 1400 psi
fs = 20000 psi (reinf.)
V = 75 psi
n = 10
25" for future wearing surface allowed

DESIGN SPECIFICATIONS
1969 AASHTO as applicable
LOADING HS 20-44

PRECAST PRESTRESSED UNITS
fc = 5000 psi
fs = 4000 psi
fs = 248,000 psi - 7/8" strands
fs = 173,600 psi - 7/8" strands

WATERWAY INFORMATION
Drainage Area = 290 Sq. Miles
Character - Hilly
Present Opening = 2710 Sq. Ft.
Required Opening = 2710 Sq. Ft.
Proposed Opening = 2710 Sq. Ft.
Q(50) = 20740 cfs



LOCATION SKETCH

GENERAL PLAN & ELEVATION
S.B.I. RTE 8 OVER KICKAPOO CREEK
SBI ROUTE 8 SEC. 11 BR
PEORIA COUNTY
STATION 269+00

DESIGNED	M.A.J.
CHECKED	May
IN CHARGE	M.A.J.
APPROVED	May

EXAMINED	
PASSED	
APPROVED	

FILE NAME = 0720245-68185.dgn



USER NAME = *OPERATOR*	DESIGNED - TJZ	REVISED
PLOT SCALE = 0.2" = 1'	CHECKED - SBC	REVISED
PLOT DATE = 8/16/2018	DRAWN - DLH	REVISED
	CHECKED - SBC	REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

EXISTING PLANS (FOR INFORMATION ONLY)
STRUCTURE NO. 072-0245
SHEET NO. 47 OF 52 SHEETS

F.A.U. R.T.E.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	461
			CONTRACT NO. 68185	

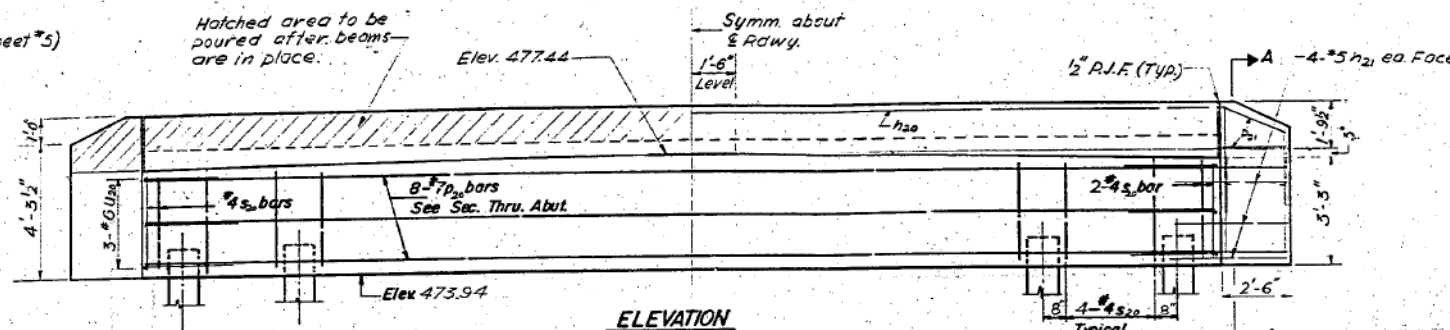
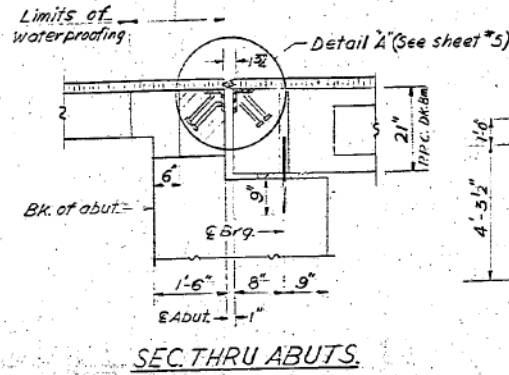
ILLINOIS FED. AID PROJECT

STATE OF ILLINOIS
DEPARTMENT OF PUBLIC WORKS & BUILDINGS
DIVISION OF HIGHWAYS

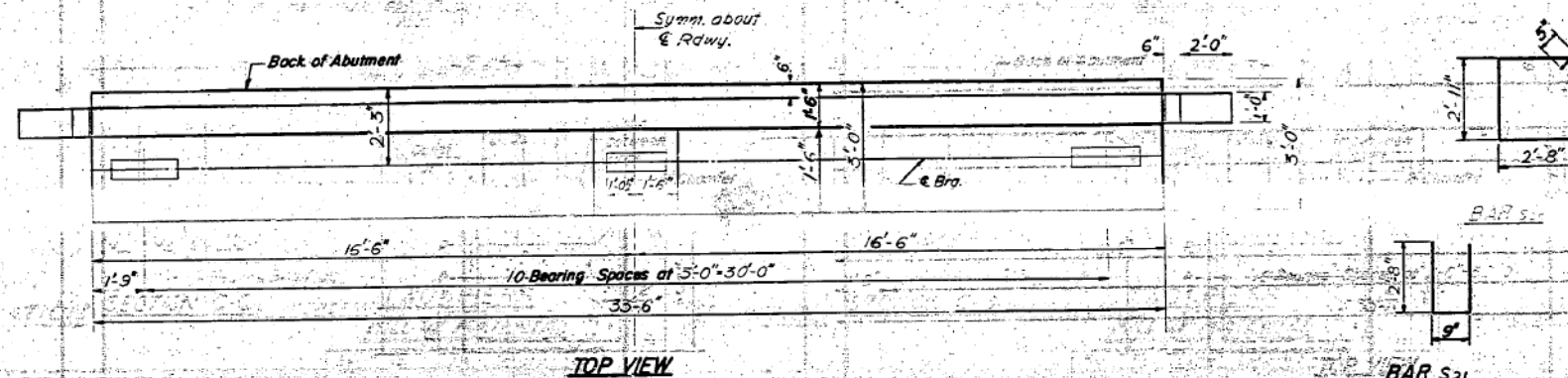
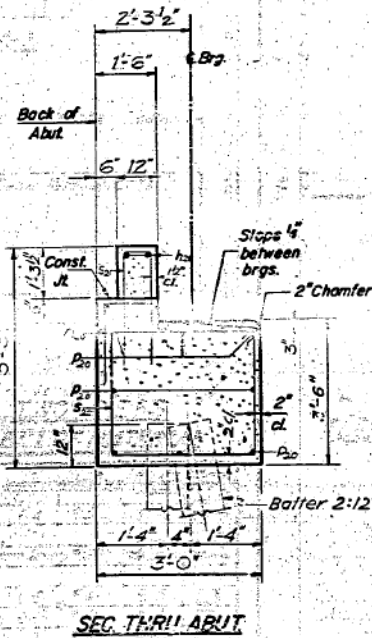
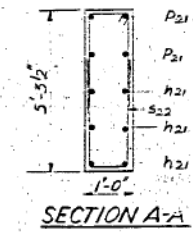
PROJECT NO.	SECTION	COUNTY	SHEET NO.	TOTAL SHEETS
8	11BR	PEORIA	10	6

SHEET NO. 4
8 SHEETS

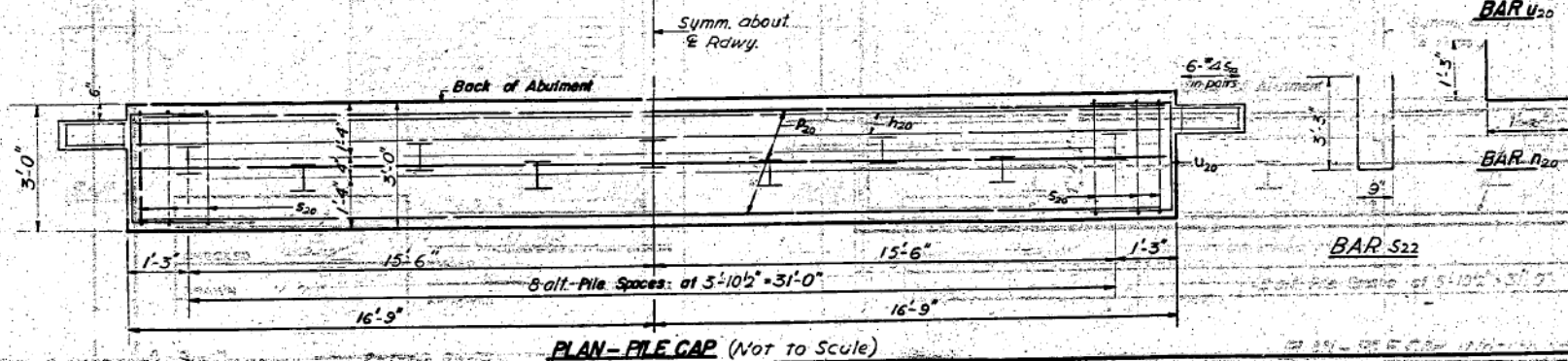
± 2'-0" Pavt. Removal
& R.C.C. Replacemnt



NOTES:
Hatched area to be poured after beams are in place.
All edges shall have standard 3/4" chamfer.
All piles shall be driven to refusal.



PILE DATA
Type: Steel H Pile (HP 10x42)
Capacity: 45 Tons. Drive to refusal.
Est. Length: 40'-0" (E. Abut.) & 25'-0" (W. Abut.)
No. Required: 8 plus 1 test pile (ea. abut.)



* TWO ABUTMENTS
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h20	4	#6	32'-9"	
h21	24	#5	4'-6"	
P21	16	#6	32'-8"	
P21	16	#5	2'-5"	
S20	72	#4	11'-8"	
S21	64	#4	6'-1"	
U20	12	#6	8'-0"	
n20	64	#4	2'-9"	
				Class X Concrete
				Cu Yds. 32
				Reinforcement Bars
				Lbs. 2536
				Steel H - Piles (HP10x42)
				Lin. Ft. 520
				Test Piles Steel (HP10x42)
				Ea. 2

DESIGNED	M.O.J.	EXAMINED	
CHECKED	may	PASSED	
DRAWN	M.O.J.	APPROVED	
CHECKED	may		

EAST & WEST ABUTMENTS
S.B.I. RTE. 8 SEC. 11 BR
PEORIA COUNTY
STATION 269+00

USER NAME = *OPERATOR*	DESIGNED - TJZ	REVISED
PLOT SCALE = 0.2" = 1'-0"	CHECKED - SBC	REVISED
PLOT DATE = 8/16/2018	DRAWN - DLH	REVISED
	CHECKED - SBC	REVISED

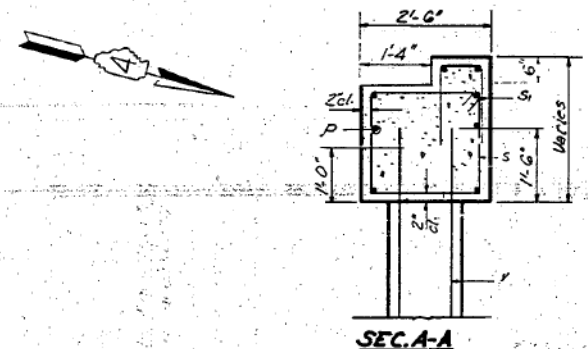
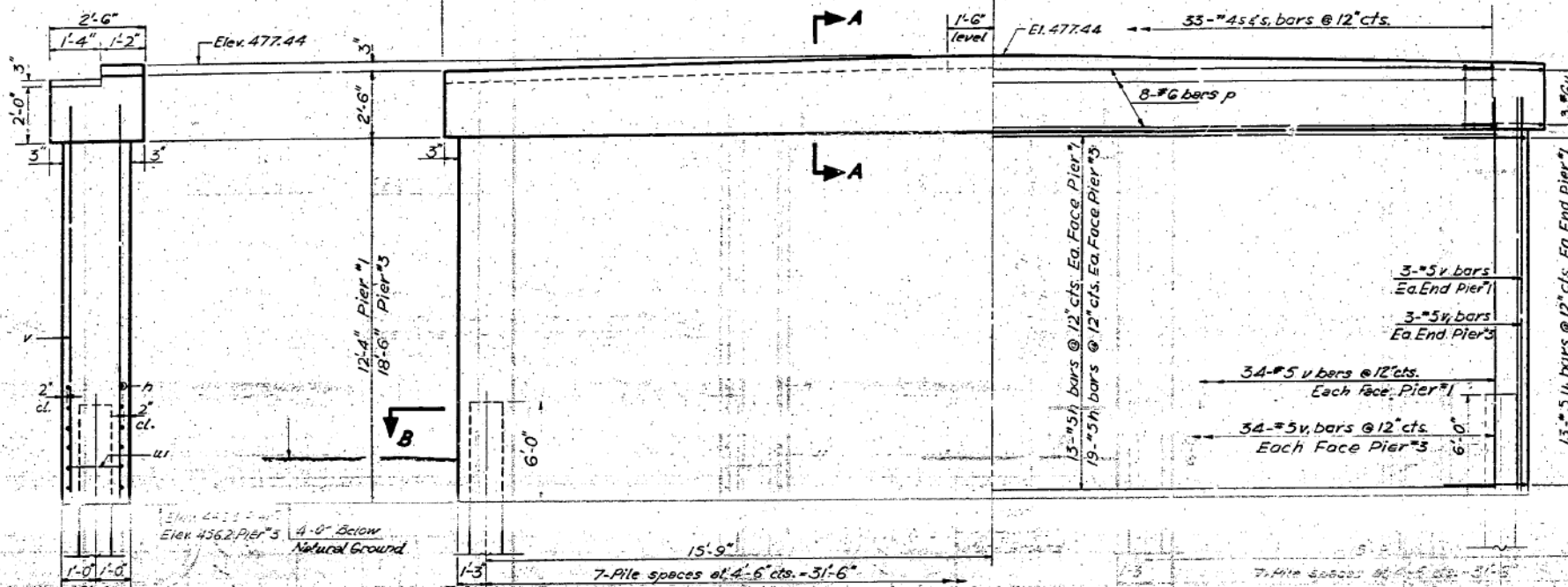
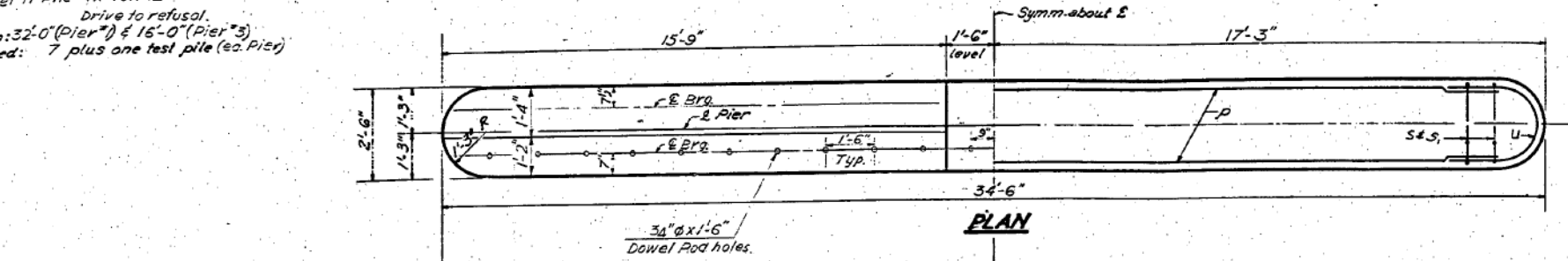
F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	462
				CONTRACT NO. 68185

LAST SAVED DATE: 8/15/2018

STATE OF ILLINOIS
DEPARTMENT OF PUBLIC WORKS & BUILDINGS
DIVISION OF HIGHWAYS

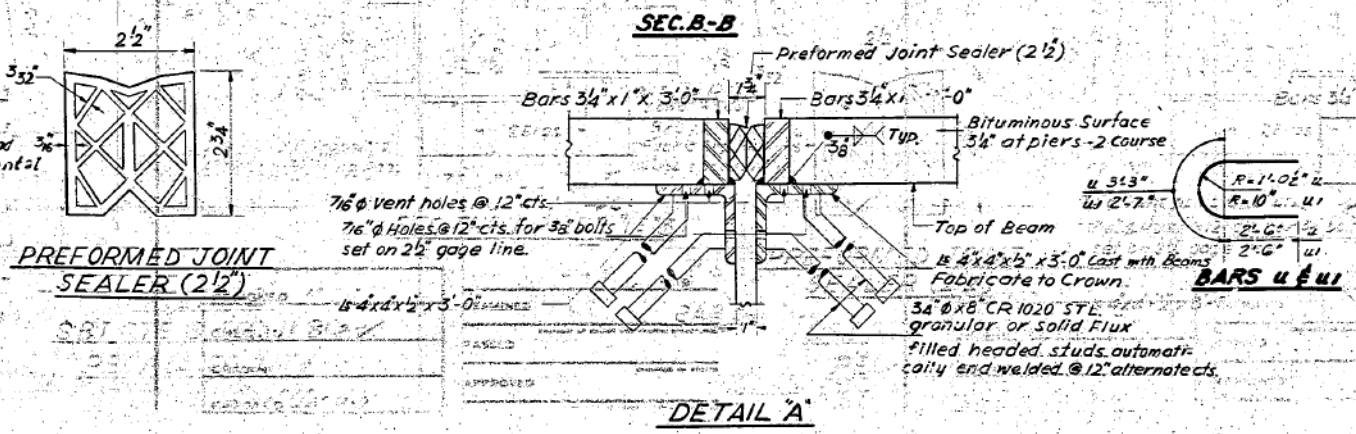
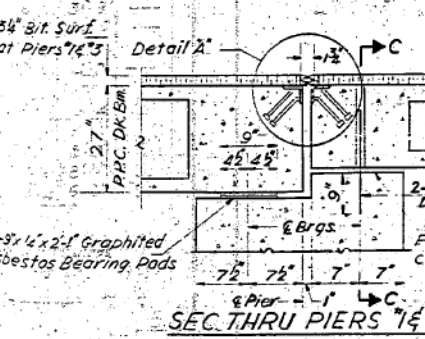
SHEET NO. 5		8 SHEETS	
NO. 8	11BR	PEORIA	10 7

PILE DATA
Type: Steel H Pile - HP10x42
Capacity: Drive to refusal.
Est. length: 32'-0" (Pier #1) & 16'-0" (Pier #3)
No. Required: 7 plus one test pile (ea. Pier)



Notes:
All edges shall have 3/16" chamfers.
All piles shall be driven to refusal.

END VIEW
(End View Pier #3 by rotation)



BILL OF MATERIAL

Part No.	Size	Length	Shape
h	#4	31'-6"	—
p	#6	52'-0"	—
s	#4	6'-5"	□
s	#4	3'-6"	—
u	#6	8'-3"	—
u	#5	7'-7"	—
v	#5	13'-10"	—
w	#5	13'-6"	—
Class X Concrete Cu. Yds. 92			
Reinforcement Bars Lbs. 6625			
Steel Piles (HP10x42) Lin. Ft. 336			
Test Piles (HP10x42) Each - 2			

DESIGNED M.O.T.	EXAMINED
CHECKED <i>May</i>	PASSED
DRAWN H.O.J.	APPROVED
CHECKED <i>May</i>	

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

EXISTING PLANS (FOR INFORMATION ONLY)
STRUCTURE NO. 072-0245

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	463
CONTRACT NO. 68185				

SHEET NO. 49 OF 52 SHEETS

ILLINOIS FED. AID PROJECT

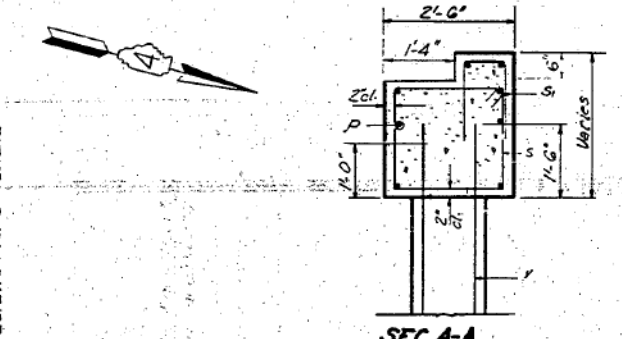
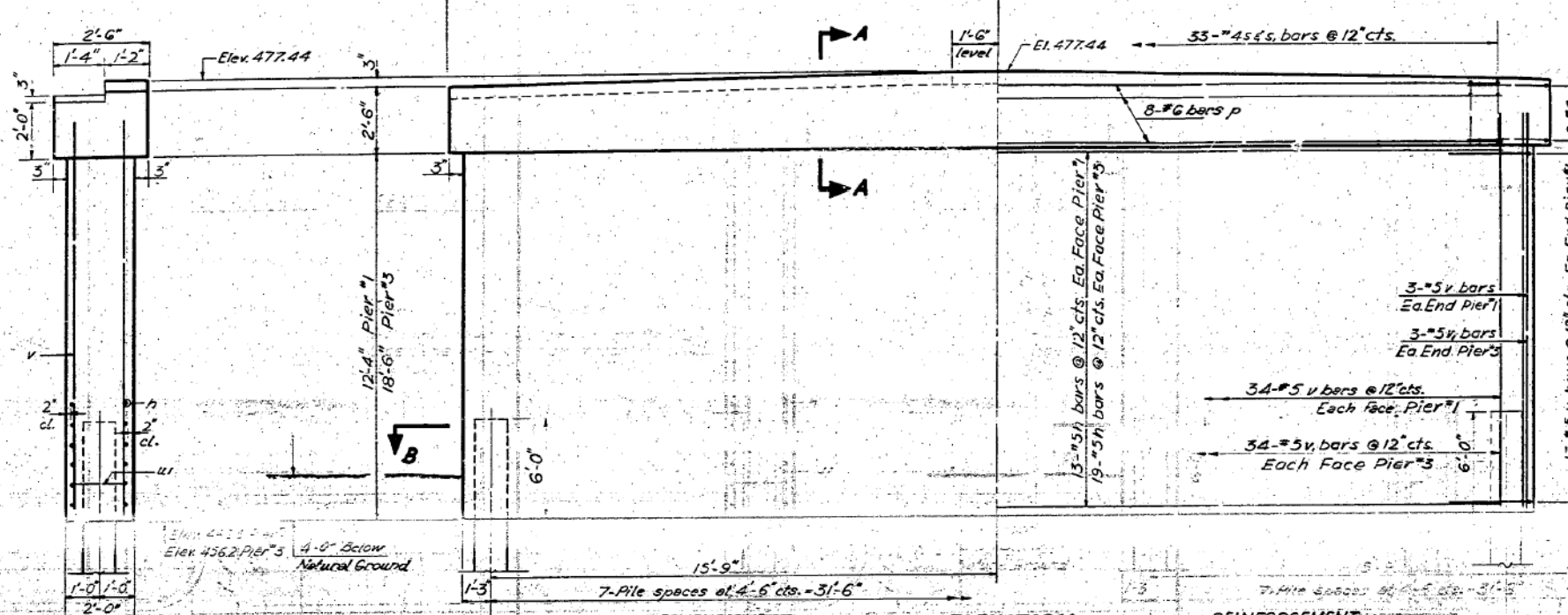
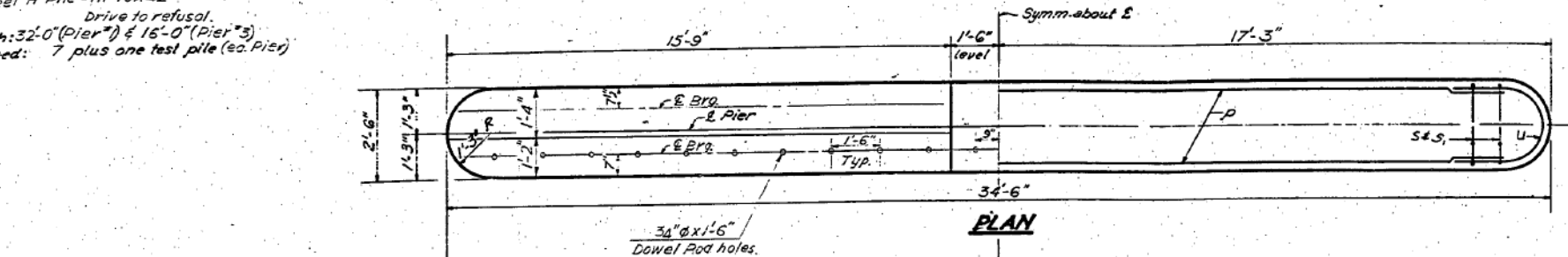
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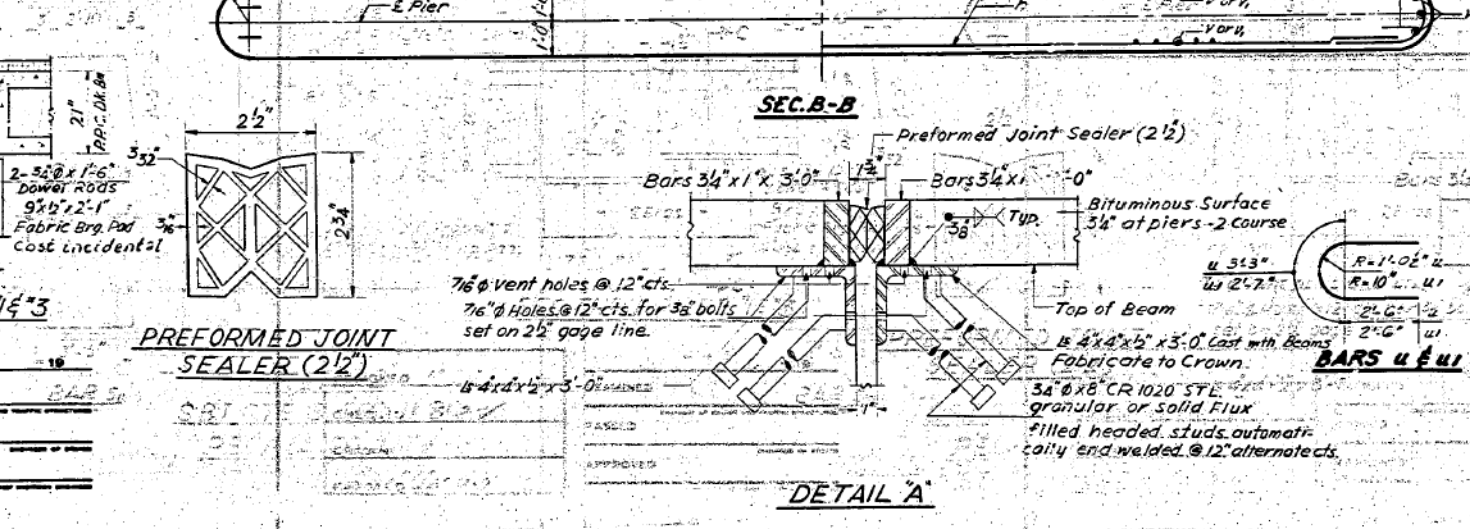
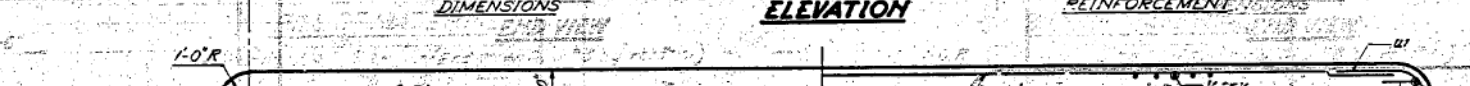
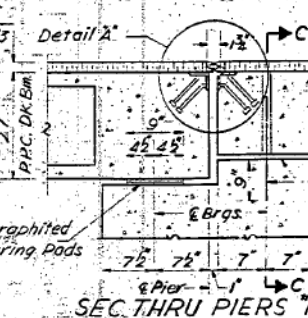
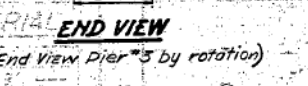
USER NAME = *OPERATOR*	DESIGNED - TJZ	REVISED
PLOT SCALE = 0.2" = 1'	CHECKED - SBC	REVISED
PLOT DATE = 8/16/2018	DRAWN - DLH	REVISED
	CHECKED - SBC	REVISED

FILE DATA

Type: Steel H Pile - HP10X42
Capacity: Drive to refusal.
Est. length: 32'-0" (Pier #1) & 16'-0" (Pier #3)
No. Required: 7 plus one test pile (ea. Pier)



Notes:
All edges shall have std. 3/8" chamfers.
All piles shall be driven to refusal.



BILL OF MATERIAL

Item	No.	Size	Length	Shape
a	64	#5	31'-6"	—
b	16	#6	32'-0"	—
s	66	#4	8'-5"	□
5	66	#4	3'-6"	□
u	12	#6	8'-3"	□
u	64	#5	7'-7"	□
v	74	#5	13'-10"	—
v	74	#5	19'-6"	—
Class X Concrete Cu Yds 92				
Reinforcement Bars Lbs. 6625				
Steel Piles (HP10X42) Linft. 336				
Test Piles (HP10X42) Each 2				

DESIGNED	M.O.J.	EXAMINED	
CHECKED	May	PASSED	
DRAWN	M.O.J.	APPROVED	
CHECKED	May		



USER NAME = \$OPERATOR\$	DESIGNED - TJZ	REVISED
PLOT SCALE = 0.2" = 1'	CHECKED - SBC	REVISED
PLOT DATE = 8/16/2018	DRAWN - DLH	REVISED
	CHECKED - SBC	REVISED

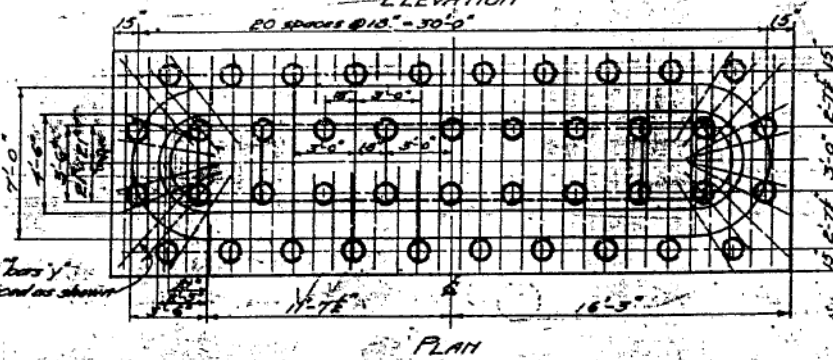
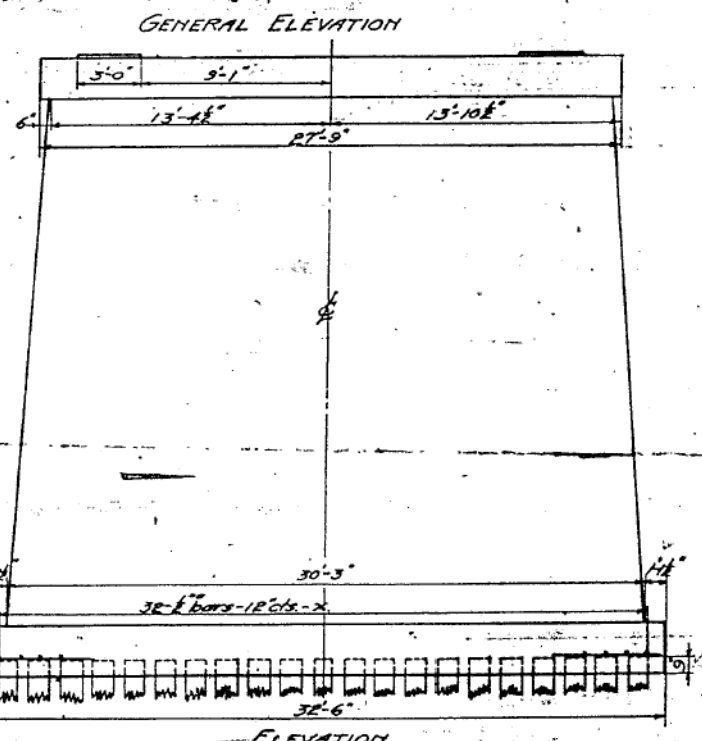
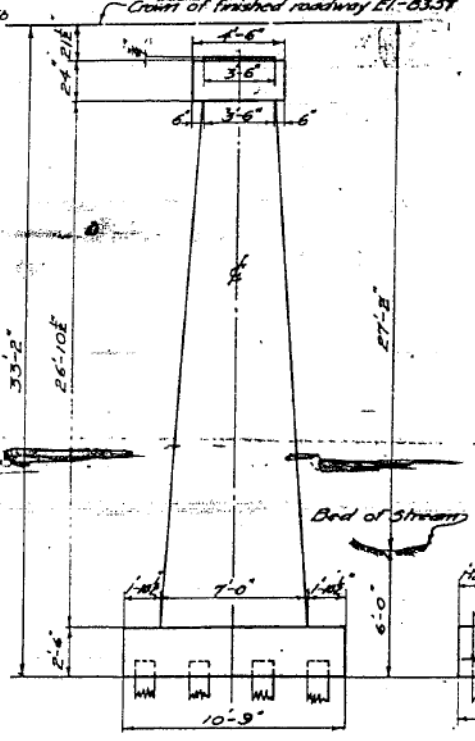
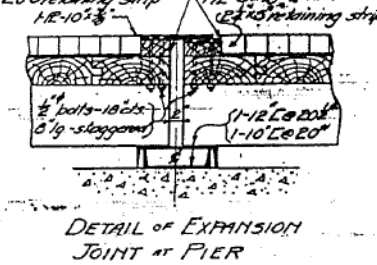
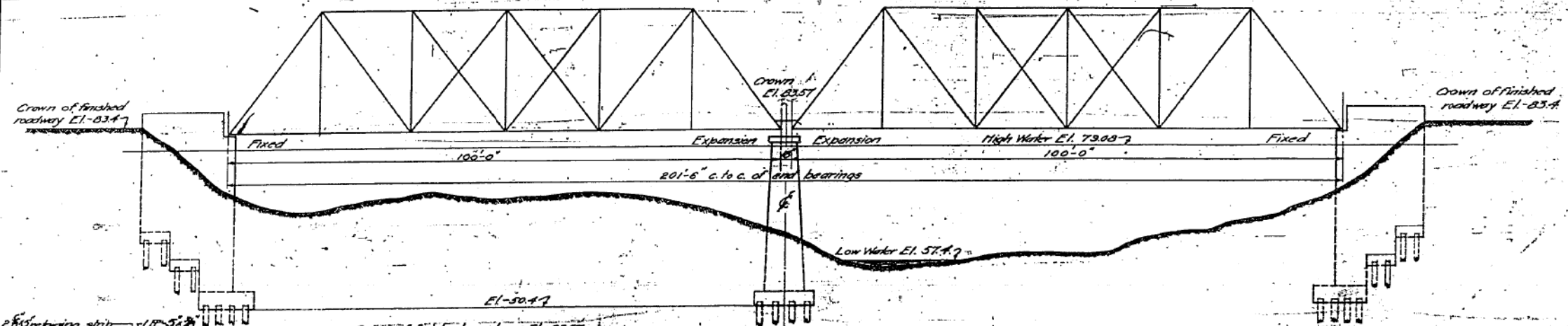
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

EXISTING PLANS (FOR INFORMATION ONLY)
STRUCTURE NO. 072-0245
SHEET NO. 50 OF 52 SHEETS

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	464
CONTRACT NO. 68185				

STATE OF ILLINOIS
DEPARTMENT OF PUBLIC WORKS & BUILDINGS
DIVISION OF HIGHWAYS

PROJECT NO. 072-0245 COUNTY PEORIA SHEET NO. 51 OF 52 SHEETS



BILL OF MATERIAL - PIER

X	32 1/2	10'-6"
Y	20 1/2	5'-0"
Reinforcing Steel lbs.	570	
Concrete cu yds.	185.5	
Piles - 12T untreated	42	

All piles under pier shall be driven to a bearing capacity of 12 tons
Minimum dimensions 12 ton piles
Dutt-12, Tip-10
B.M. #10 on S end pier. Ward
bridge - 2 1/2 ft. N of SW corner of
pier. El. - 80.00
Class B concrete to be used
throughout. Proportions 1:3:5

COMPUTED	- H. K. G. G.
CHECKED	- H. F. D.
DRAWN	- H. F. D.
CHECKED	- H. F. D.
ASSEMBLED	
CHECKED	

DESIGNED: *W. J. B. Smith*
DRAWN: *W. J. B. Smith*
CHECKED: *W. J. B. Smith*
APPROVED: *W. J. B. Smith*

TOTAL BILL OF MATERIAL

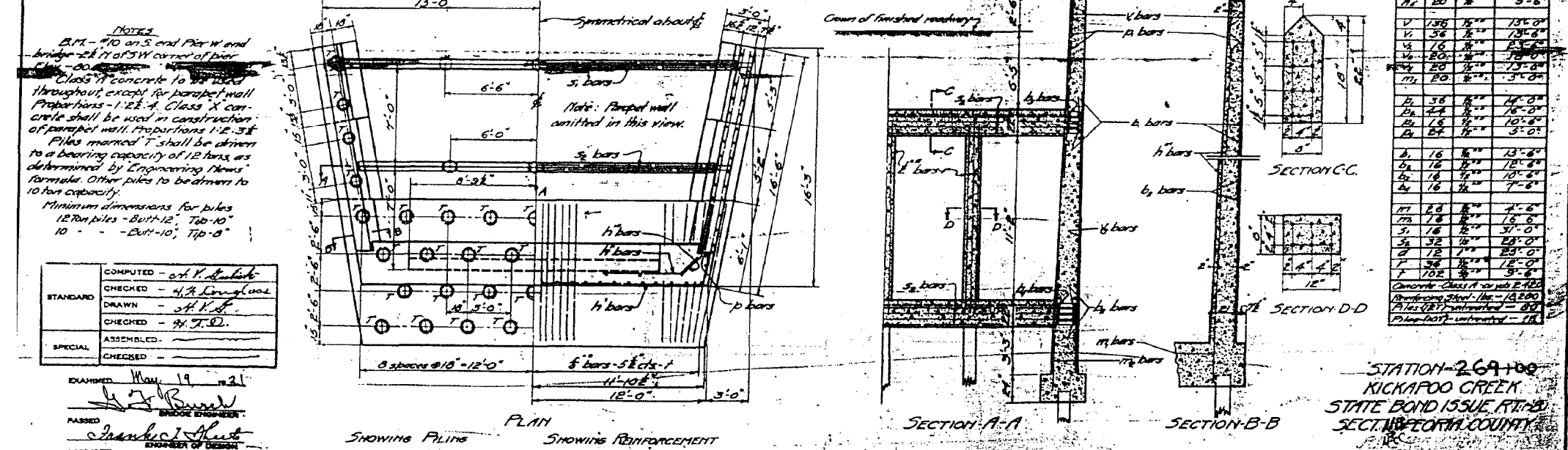
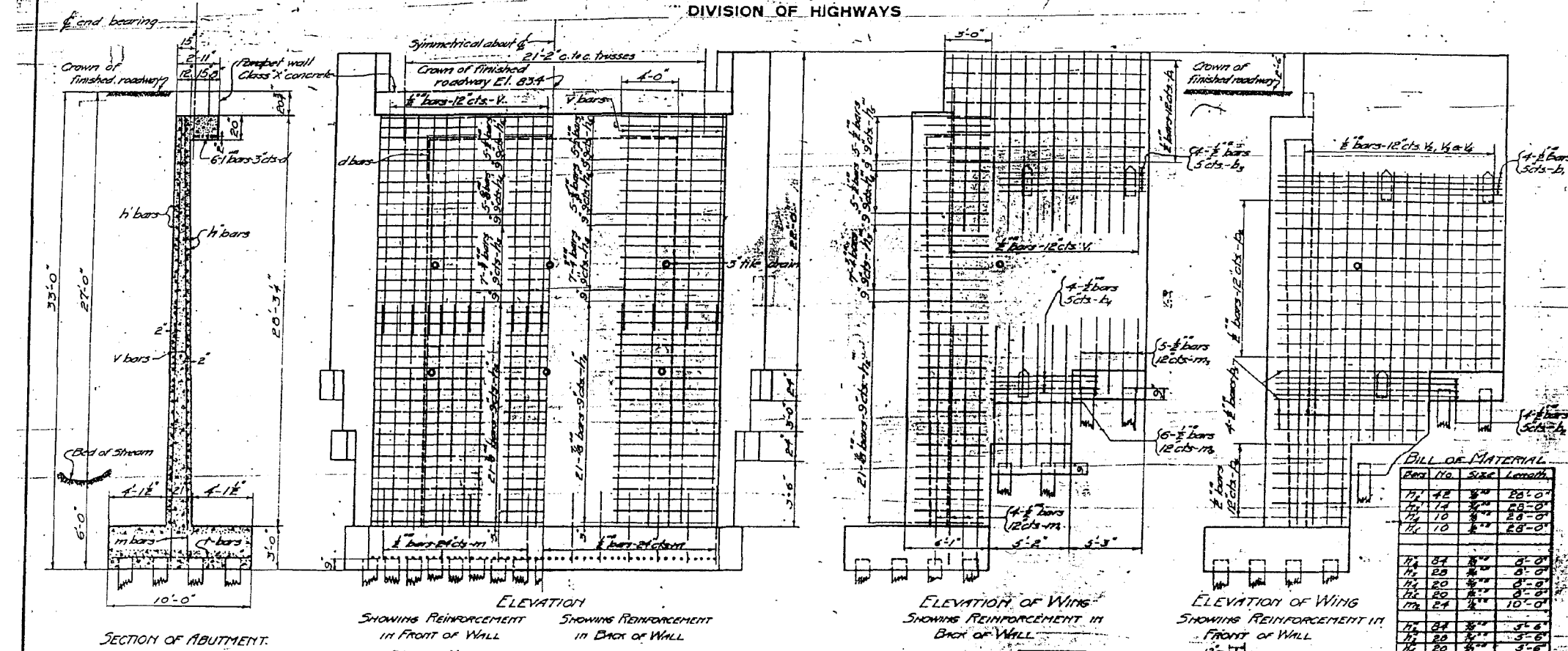
Superstructure:	
Structural Steel lbs.	157,300
Concrete Class X cu yds.	1.9
Crossed Wood Block sq yds	430
Substructure:	
Abutments:	
Reinforcing Steel lbs.	18,580
Concrete Class A cu yds.	2,420
Piles - 12T untreated	80
Piles - 10T untreated	12
Pier:	
Concrete Class B cu yds.	185.5
Reinforcing Steel lbs.	570
Piles - 12T untreated	42
Piles - 10T untreated	12
Totals:	
Concrete Class A cu yds.	2,420
Concrete Class B cu yds.	185.5
Concrete Class X cu yds.	1.9
Total Concrete cu yds.	2,607.4
Reinforcing Steel lbs.	18,580
Structural Steel lbs.	157,300
Piles - 12T untreated	122
Piles - 10T untreated	12
Crossed Wood Block sq yds	430

STATION: 269+00
KICKAPOO CREEK
STATE BOND ISSUE RT-8
SECT. PEORIA COUNTY

USER NAME = \$OPERATOR\$	DESIGNED - TJZ	REVISED
PLOT SCALE = 0.2" = 1'	CHECKED - SBC	REVISED
PLOT DATE = 8/16/2018	DRAWN - DLH	REVISED
	CHECKED - SBC	REVISED

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	465
CONTRACT NO. 68185				

STATE OF ILLINOIS
DEPARTMENT OF PUBLIC WORKS & BUILDINGS
DIVISION OF HIGHWAYS



BILL OF MATERIAL

Item No.	Size	Length
17	4#	23'-0"
18	14#	20'-0"
19	10#	20'-0"
20	10#	20'-0"
21	10#	20'-0"
22	8#	0'-0"
23	8#	0'-0"
24	8#	0'-0"
25	8#	0'-0"
26	8#	0'-0"
27	8#	0'-0"
28	8#	0'-0"
29	8#	0'-0"
30	8#	0'-0"
31	8#	0'-0"
32	8#	0'-0"
33	8#	0'-0"
34	8#	0'-0"
35	8#	0'-0"
36	8#	0'-0"
37	8#	0'-0"
38	8#	0'-0"
39	8#	0'-0"
40	8#	0'-0"
41	8#	0'-0"
42	8#	0'-0"
43	8#	0'-0"
44	8#	0'-0"
45	8#	0'-0"
46	8#	0'-0"
47	8#	0'-0"
48	8#	0'-0"
49	8#	0'-0"
50	8#	0'-0"
51	8#	0'-0"
52	8#	0'-0"
53	8#	0'-0"
54	8#	0'-0"
55	8#	0'-0"
56	8#	0'-0"
57	8#	0'-0"
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59	8#	0'-0"
60	8#	0'-0"
61	8#	0'-0"
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100	8#	0'-0"

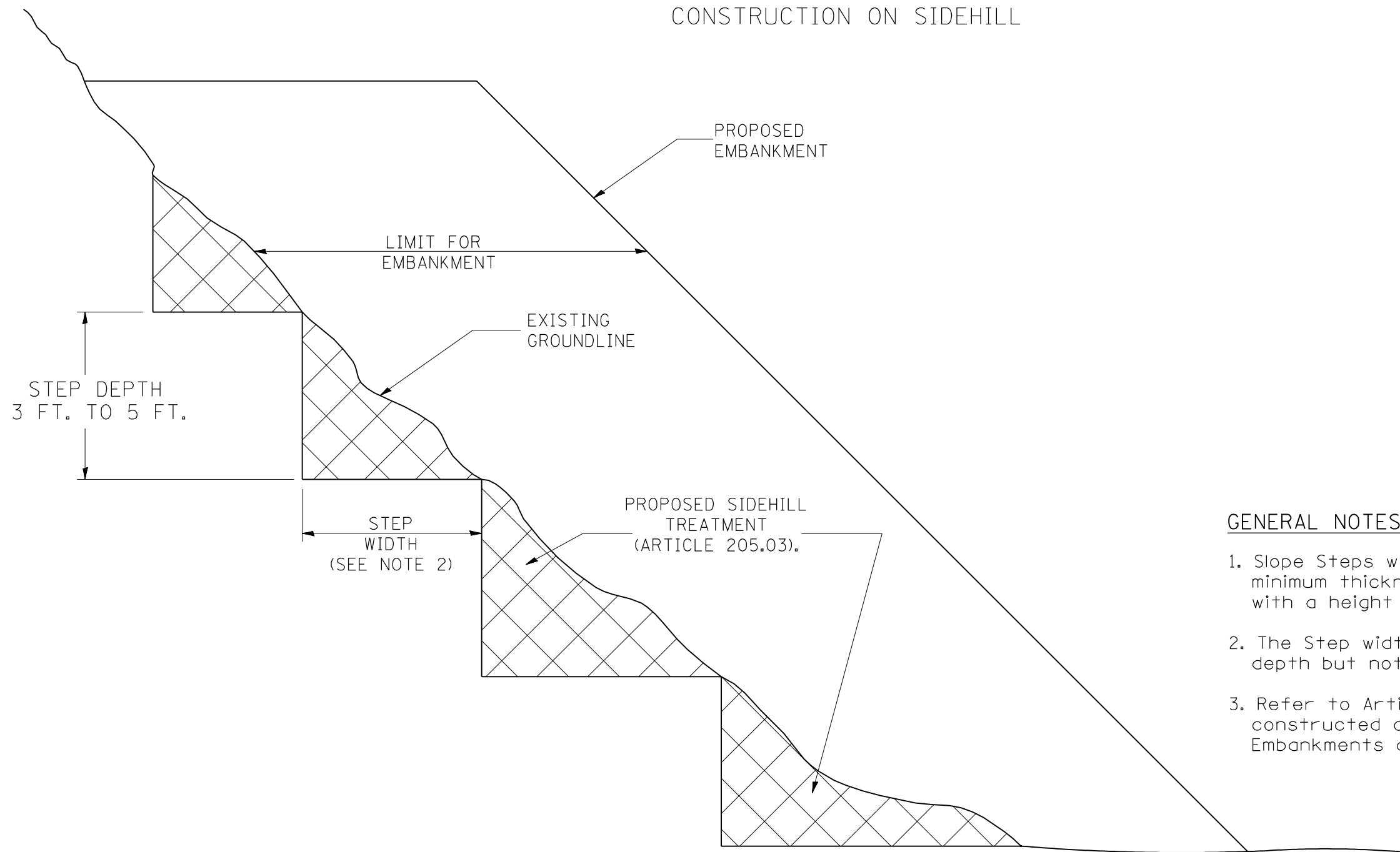
NOTES
 B.M. - #10 on S. end Pier W and bridge #21 of SW corner of pier
 Class A concrete to be used throughout, except for parapet wall. Proportions - 1:2:4. Class X concrete shall be used in construction of parapet wall. Proportions 1:2:3:8.
 Piles marked T shall be driven to a bearing capacity of 12 tons, as determined by Engineering News Formula. Other piles to be driven to 10 ton capacity.
 Minimum dimensions for piles:
 12 Ton piles - Butt-12", Top-10"
 10 - - - Butt-10", Top-8"

STANDARD	COMPUTED - <i>of V. Amick</i>
	CHECKED - <i>of A. Langlois</i>
	DRAWN - <i>A. V. L.</i>
	CHECKED - <i>H. J. D.</i>
SPECIAL	ASSEMBLED -
	CHECKED -

DRAWN: *W. J. B.* 19 21
 PASSED: *Franklin Sturt*
 APPROVED: *Charles R. ...*

SLOPE STEPS DETAIL

TYPICAL CROSS-SECTION EMBANKMENT CONSTRUCTION ON SIDEHILL



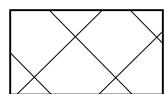
GENERAL NOTES:

1. Slope Steps will be required for all 12(300) minimum thickness "sliver fills" and on all fills with a height of 10 feet or greater.
2. The Step width shall be twice the Step depth but not less than 6 feet.
3. Refer to Article 205.03 for Embankment to be constructed on Hillside or Slopes, or if existing Embankments are to be widened.

DESIGNER NOTE:

1. EACH PROJECT SHOULD BE REVIEWED INDEPENDENTLY FOR TREATMENT REQUIRED.
2. REFER TO THIS DETAIL WITH NOTE ON APPLICABLE TYPICAL SECTIONS.

REPLACEMENT MATERIAL:



STANDARD EMBANKMENT
(IN ACCORDANCE WITH
205 OF THE STANDARD SPECIFICATION).

All dimensions are in inches (millimeters) unless otherwise noted.

1-1-97	RENUM. L-5.03, NEW REVISION BOX, REVISED TITLE BOX, REVISED GENERAL NOTES.	T.P.			
10-16-06	REVISED TO 2007 SPEC.	M.A.			
5-30-18	MINOR CORRECTION	R.D.			

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

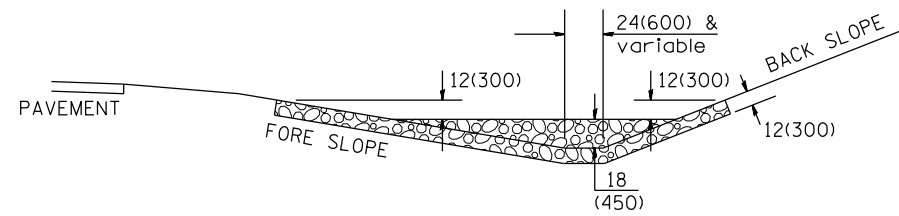
SLOPE STEPS DETAIL

NOT TO SCALE

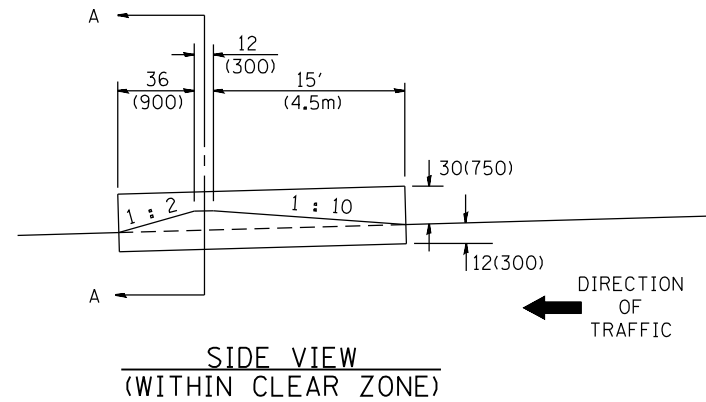
CADD STD. 205001-D4

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N,BR-1,RS-4,W-1)	PEORIA	577	467
FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT			CONTRACT NO. 68185	

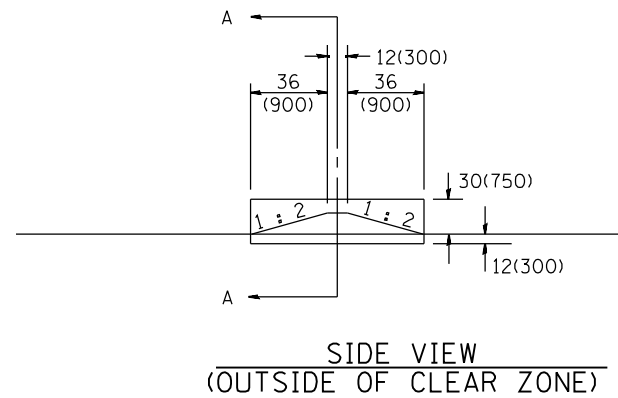
DESIGNER NOTES:
 1. Designer to modify this detail Special Detail Sheet, as needed, for inclusion in plans.
 2. Determine the required clear zone in order to select the berm slopes.
 3. Include State Standard 280001.



SECTION A - A

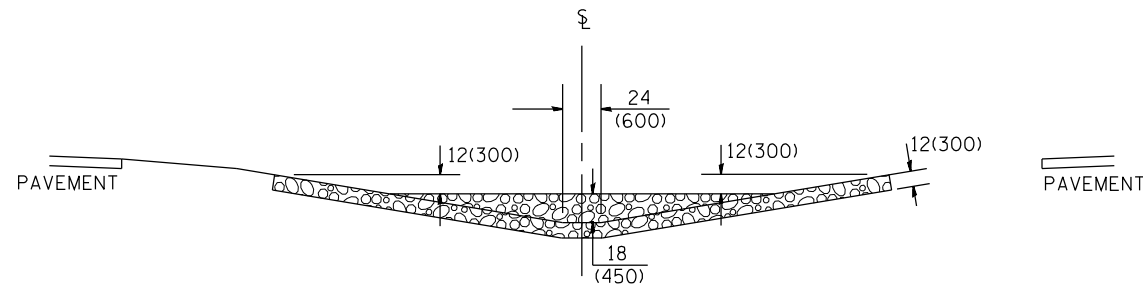


SIDE VIEW (WITHIN CLEAR ZONE)

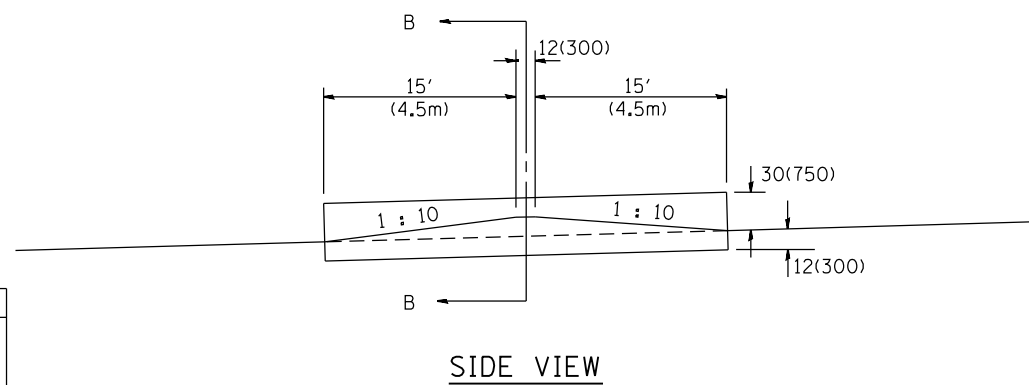


SIDE VIEW (OUTSIDE OF CLEAR ZONE)

SIDE DITCH AGGREGATE DITCH CHECK



SECTION B - B



SIDE VIEW

MEDIAN AGGREGATE DITCH CHECK

NOTES:

- FOR DITCH BOTTOM PROTECTED BY EROSION CONTROL BLANKET, USE 400'(120m) SPACING. FOR SEEDED DITCH BOTTOM, USE 200'(60m) SPACING.
- THIS WORK CONSISTS OF THE COMPLETE INSTALLTION OF EROSION CONTROL DITCH CHECK AT LOCATIONS AS SHOWN ON THE PLANS, OR AS DIRECTED BY THE ENGINEER. THE AGGREGATE GRADATION SHALL BE RR3 WITH A MINIMUM QUALITY OF CLASS B.

STATION	LOCATION		NUMBER OF DITCH CHECKS	FORE SLOPE	DITCH BOTTOM WIDTH	BACK SLOPE	BERM SLOPE
	MEDIAN	SIDE DITCH LEFT RIGHT					

ESTIMATE QUANTITIES

	FORE SLOPE	DITCH BOTTOM	BACK SLOPE	BERM SLOPE	AGGREGATE DITCH CHECK EROSION CONTROL TON (METRIC TON)
MEDIAN DITCH	1 : 6	24(600)	—	1 : 10	95(86)
SIDE DITCH	1 : 6	24(600)	1 : 4	1 : 10 & 1 : 2	50(45)
SIDE DITCH	1 : 6	24(600)	1 : 4	1 : 2 & 1 : 2	19(17)
SIDE DITCH	1 : 4	24(600)	1 : 3	1 : 10 & 1 : 2	18(16)
SIDE DITCH	1 : 4	24(600)	1 : 3	1 : 2 & 1 : 2	14(13)

All slope ratios are expressed as units of vertical displacement to units of horizontal displacement (V:H).
 All dimensions are in inches (millimeters) unless otherwise noted.

QUANTITIES
 CALC. BY: _____ DATE: _____
 CHECKED BY: _____ DATE: _____

QUANTITY CALCULATIONS ARE ON FILE AT THE DISTRICT 4 OFFICE; BUREAU OF PROJECT IMPLEMENTATION; DOCUMENTATION SECTION

1-1-97	RENUM. A-12.04, NEW REVISION BOX, REVISED TITLE BOX, ADDED QUANTITY CALCULATION BOX	T.P.	03-15-12	CHANGED NOTE 1.	R.D.
9-15-05	REVISED DESIGNER NOTE	M.A.			
10-16-06	REVISED RR3 QUALITY & TO 2007 SPEC.	M.A.			

STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

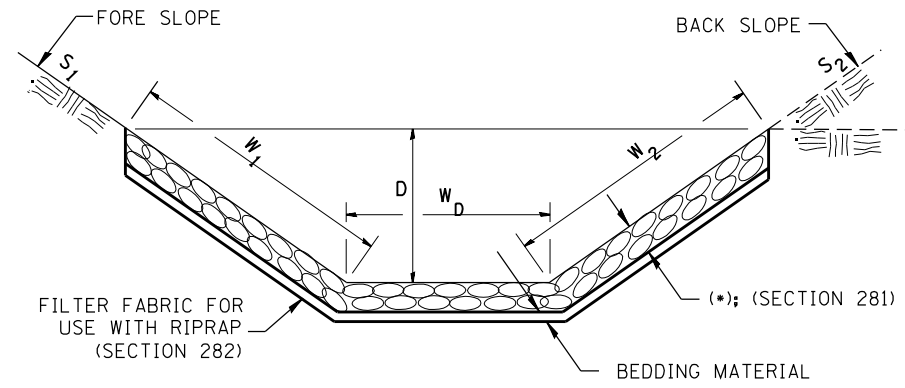
EROSION CONTROL AGGREGATE DITCH CHECK
 NOT TO SCALE

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	468
CONTRACT NO. 68185				
FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT				

CADD STD. 280101-D4

Designer NOTES:
 1. Designer to modify this Special Detail Sheet, as needed for inclusion in plans.
 2. (*) Designer to specify pay item including material, quality, and gradation.
 3. Include District Special Provision if needed.

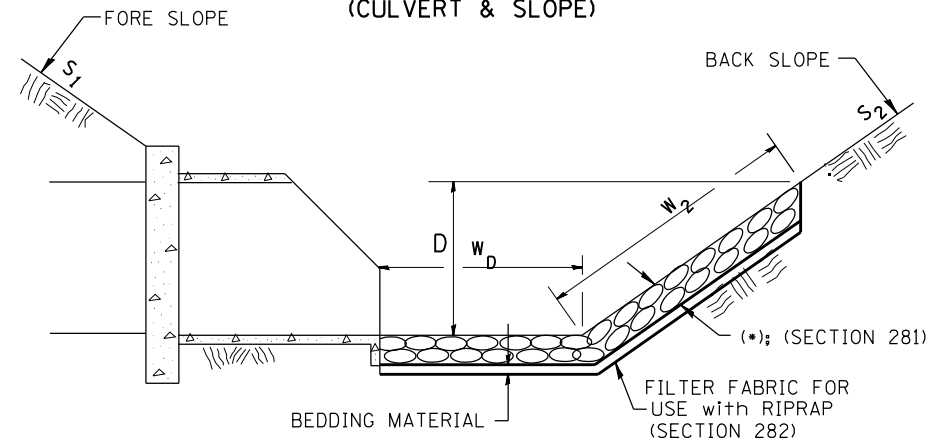
**CASE 1
(DITCH)**



(*)				
LOCATION	WIDTH (1)	LENGTH	RIPRAP	FABRIC
STA TO STA	lin ft (m)	lin ft (m)	tons (m tons)	sq yds (m ²)
TOTAL				

(1) WIDTH = $W_1 + W_2 + W_D$

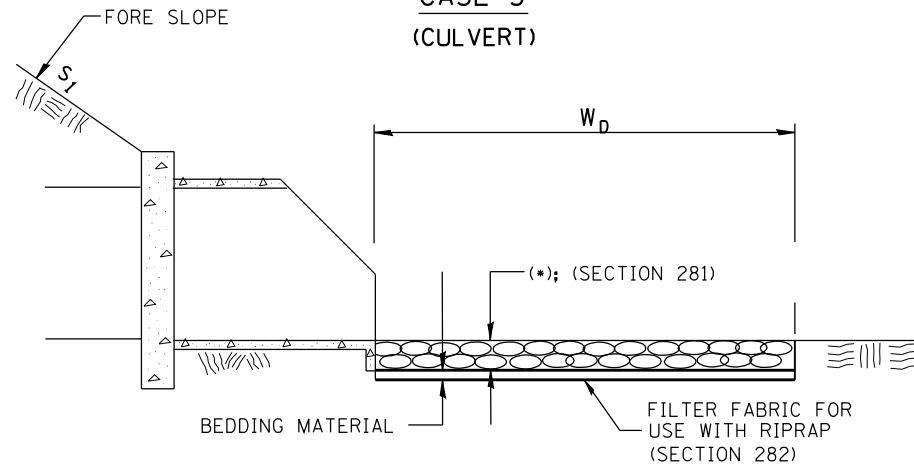
**CASE 2
(CULVERT & SLOPE)**



(*)				
LOCATION	WIDTH (1)	LENGTH	RIPRAP	FABRIC
STA TO STA	lin ft (m)	lin ft (m)	tons (m tons)	sq yds (m ²)
TOTAL				

(1) WIDTH = $W_2 + W_D$

**CASE 3
(CULVERT)**



(*)				
LOCATION	WIDTH (1)	LENGTH	RIPRAP	FABRIC
STA TO STA	lin ft (m)	lin ft (m)	tons (m tons)	sq yds (m ²)
TOTAL				

(1) WIDTH = W_D

All slope ratios are expressed as units of vertical displacement to units of horizontal displacement (V:H).

All dimensions are in inches (millimeters) unless otherwise noted.

1-1-97	RENUM. A-12.02, NEW REVISION BOX	T.P.
12-1-97	CORRECT FILTER FABRIC LEADER ARROW	J.A.
10-16-06	REVISED TO 2007 SPEC.	M.A.
9-6-12	REMOVED A DESIGNER NOTE AND MADE MINOR CHANGES	R.D.

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

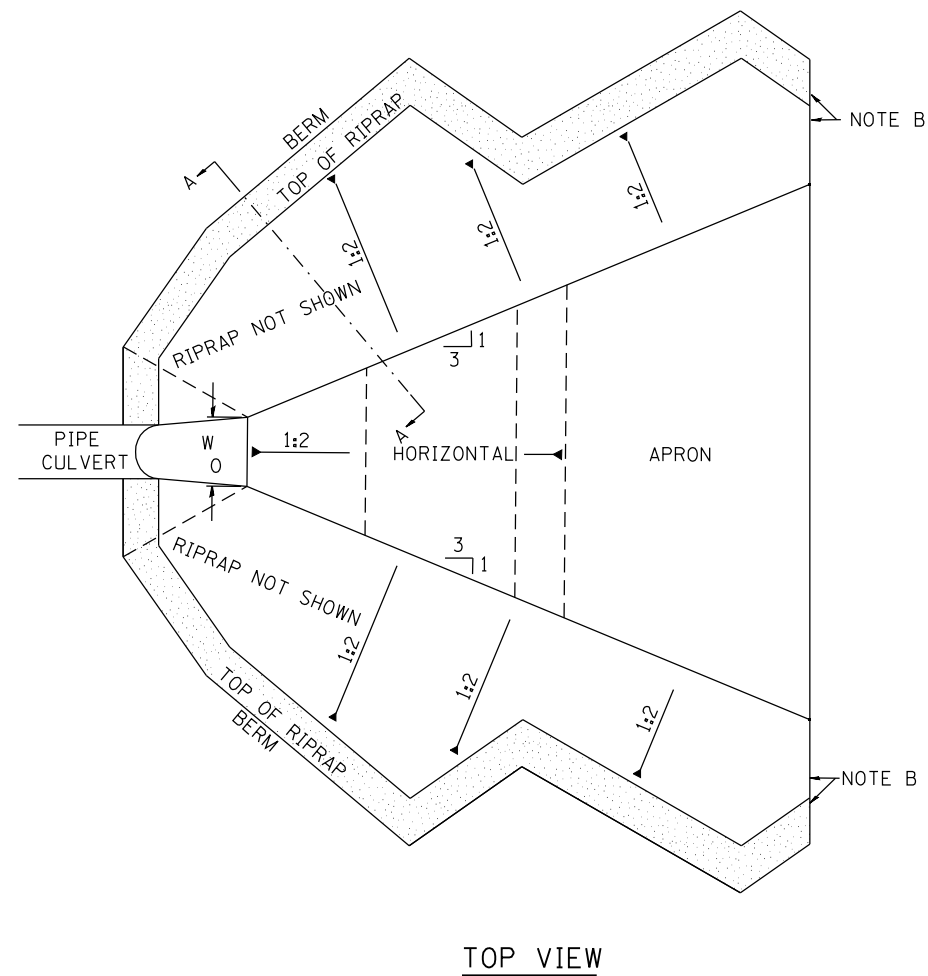
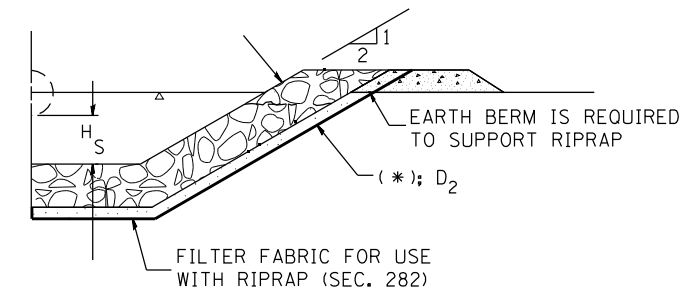
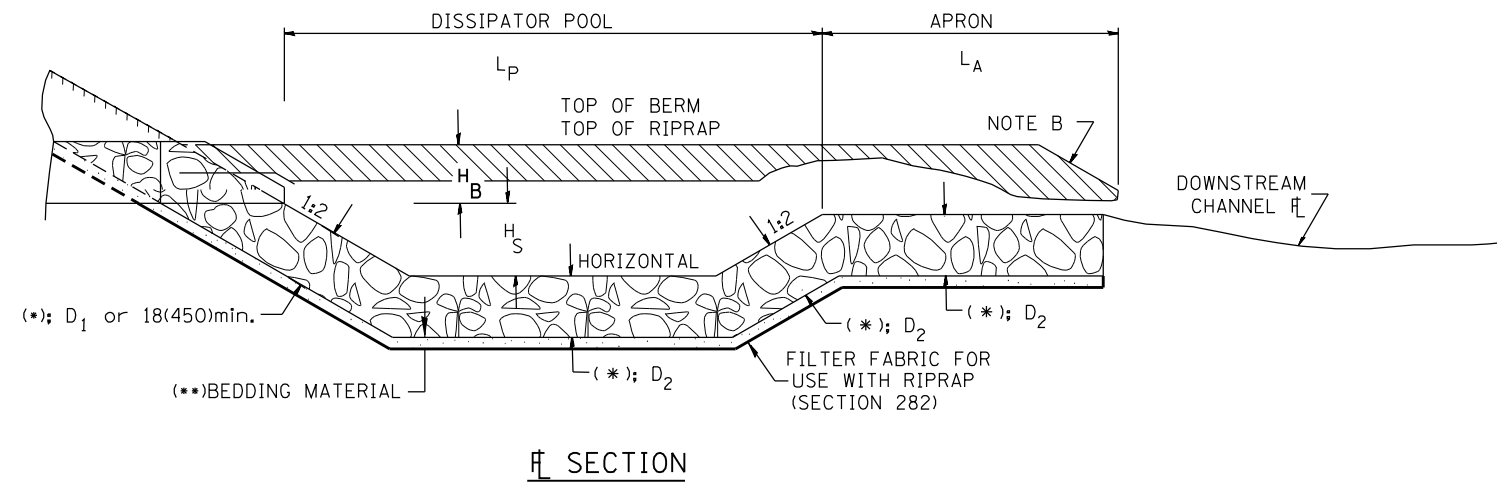
RIPRAP DITCH FOR EROSION PROTECTION

NOT TO SCALE

CADD STD. 281001-D4

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	469
CONTRACT NO. 68185				
FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT				

Designer NOTES:
 1. Designer to modify this Special Detail Sheet, as needed for inclusion in plans.
 2. (*) Designer to specify pay item including material, quality, and gradation.
 3. (**) Designer to specify thickness of bedding material.
 4. Include District Special Provision if needed.

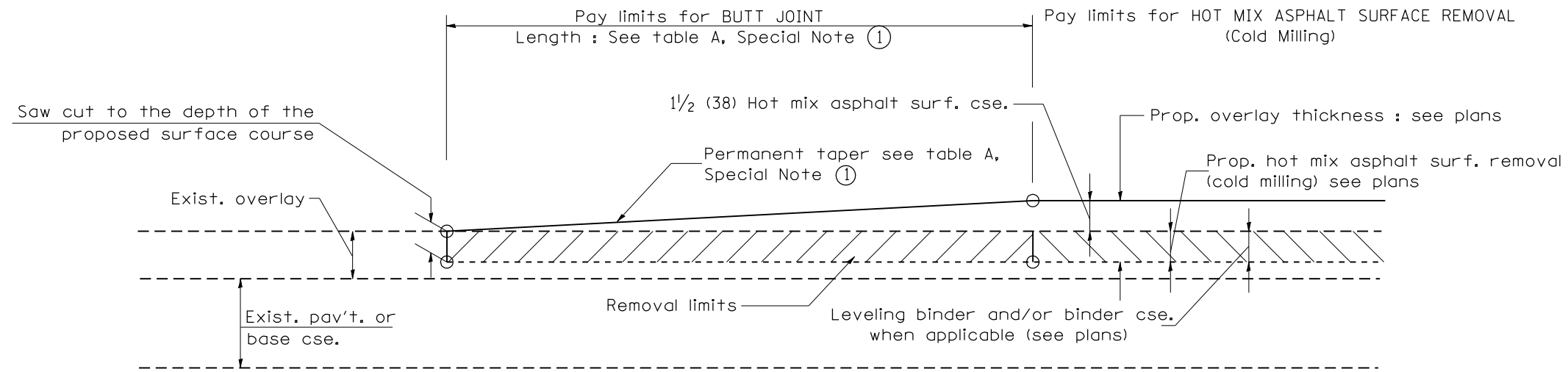


STATION	W ₀	L _P	L _A	H _S	H _B	(*) (**) (***)		
						D ₁	D ₂	D ₃
275+10	10	15	2	2	3	1.33	1.33	0.5

DIMENSIONS IN FEET.

NOTE B: WARP BASIN TO CONFORM TO NATURAL STREAM CHANNEL

All slope ratios are expressed as units of vertical displacement to units of horizontal displacement (V:H).
 All dimensions are in inches (millimeters) unless otherwise noted.



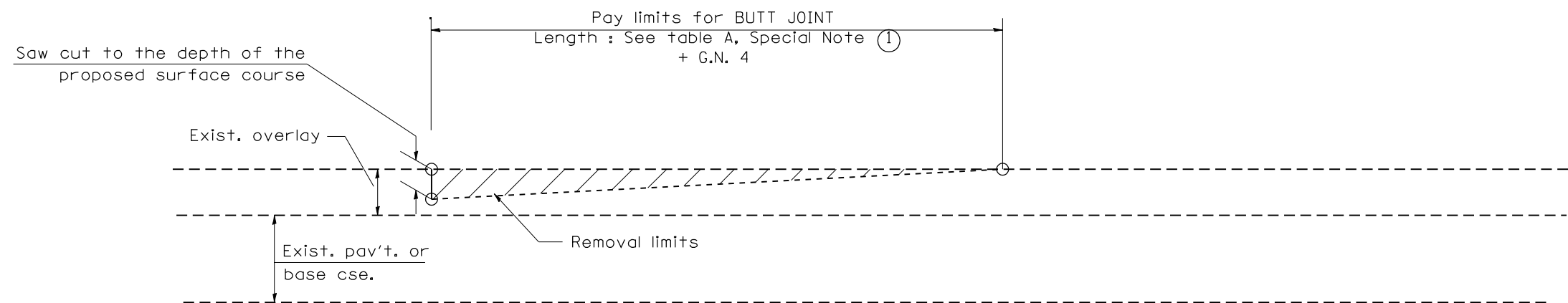
CASE 1 : WITH HOT MIX ASPHALT SURFACE REMOVAL (COLD MILLING)

**TABLE A
TAPER RATES**

SPECIAL NOTE NUMBER	ELEMENT	MAINLINE INTERSTATES & 4-LANE EXPRESSWAYS	ALL OTHERS
①	BUTT JOINT TAPER RATE	1:480	1:240
②	TEMPORARY RAMP TAPER RATE	1:80	1:40

GENERAL NOTES

1. The work shall be done in accordance with Article 406.08 and the Special Provision for Butt Joints.
2. The pavement surface to be removed may be either bituminous or P.C. concrete. The work shall be performed in accordance with Article 440.04 and the Special Provisions for Butt Joints.
3. The saw cut joints shall be primed just prior to the placing of bituminous material. The work will be in accordance with the applicable portions of Article 406.05.
4. The length of butt joint is based on the taper rate times change in cold milling depth within the butt joint pay limits, unless otherwise indicated.
5. Temporary ramps are paid for separately and not included in the cost of the butt joints.



CASE 2 : NO HOT MIX ASPHALT SURFACE REMOVAL (COLD MILLING)

All dimensions are in inches (millimeters) unless otherwise noted.

01-01-97	RENUM. C-23.01, NEW REVISION BOX	T.P.	08-21-13	MAJOR MODIFICATIONS	R.D.
04-01-97	CORRECTION TO DEPTH	J.A.	02-29-16	MINOR CORRECTIONS	R.D.
09-15-05	REVISED DESIGNER NOTE	M.M.A.	04-12-16	MINOR CORRECTIONS	R.D.
10-16-06	REVISED TO 2007 SPEC.	M.A.	02-14-17	ADDED NOTE 5	R.D.

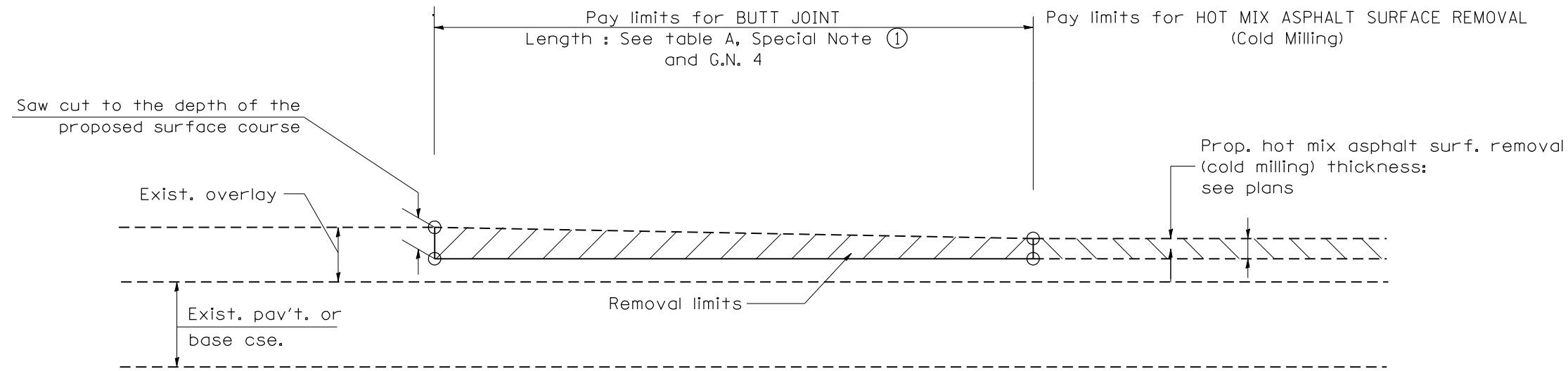
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

BUTT JOINTS

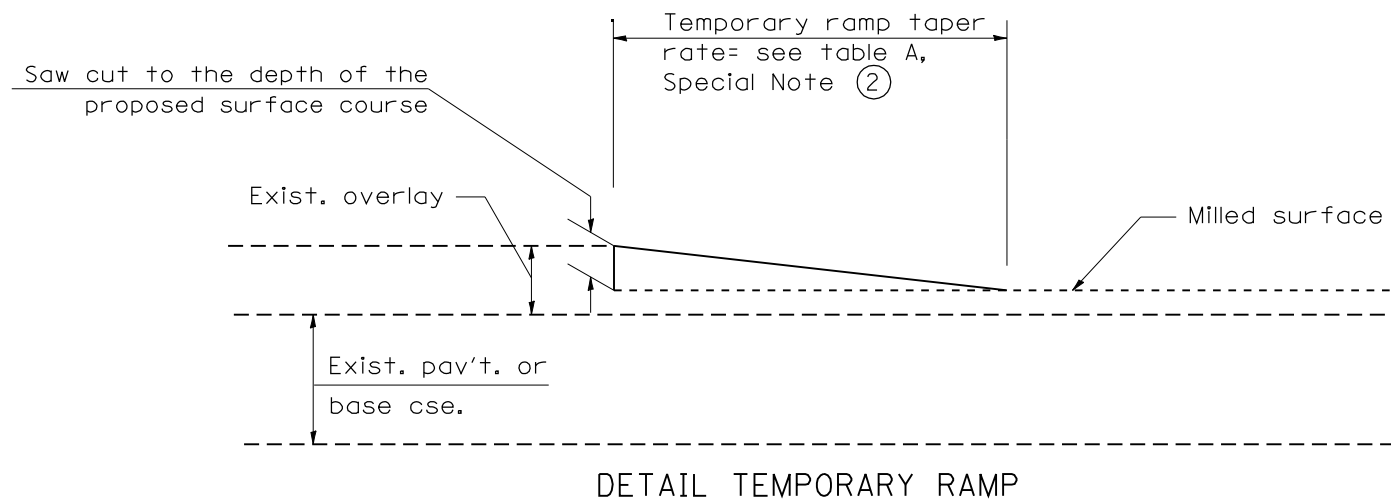
NOT TO SCALE

SHT. 1 OF 3
CADD STD. 406101-D4

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N,BR-1,RS-4,W-1)	PEORIA	577	471
FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT			CONTRACT NO. 68185	

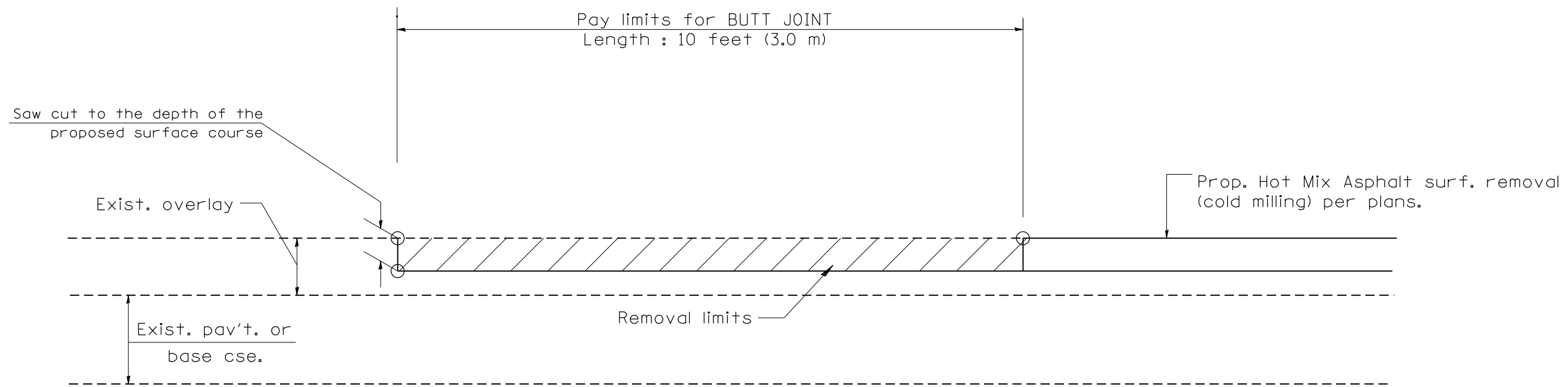


CASE 3 : HOT MIX ASPHALT SURFACE REMOVAL (COLD MILLING)
TIE-IN TO EXISTING BITUMINOUS TAPER



All dimensions are in inches (millimeters) unless otherwise noted.

				STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION		BUTT JOINTS		SHT. 2 OF 3 CADD STD. 406101-D4	
				NOT TO SCALE				CONTRACT NO. 68185	
F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.					
6659	11(N,BR-1,RS-4,W-1)	PEORIA	577	472					
FED. ROAD DIST. NO.		ILLINOIS FED. AID PROJECT							

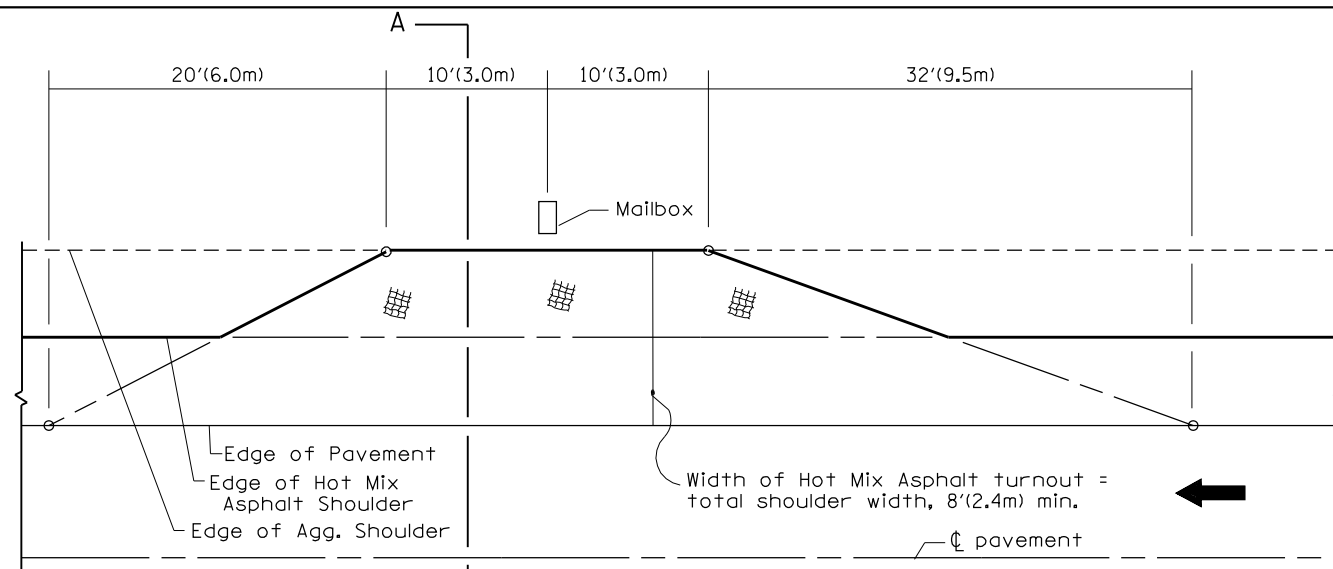


CASE 4 : SINGLE LIFT OVERLAY WITH EQUIVALENT DEPTH
HOT MIX ASPHALT SURFACE REMOVAL (COLD MILLING)
TIE-IN TO EXISTING BITUMINOUS TAPER

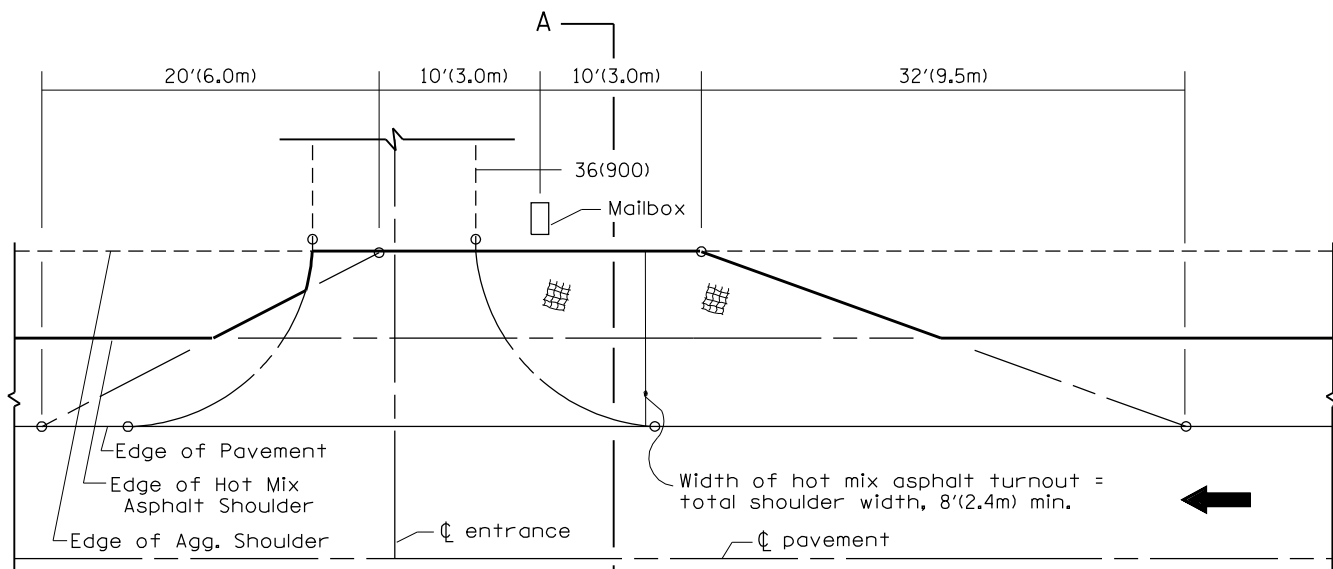
All dimensions are in inches (millimeters) unless otherwise noted.

				STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION		BUTT JOINTS		SHT. 3 OF 3 CADD STD. 406101-D4		NOT TO SCALE		FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT	
F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.									
6659	11(N,BR-1,RS-4,W-1)	PEORIA	577	473	CONTRACT NO. 68185								

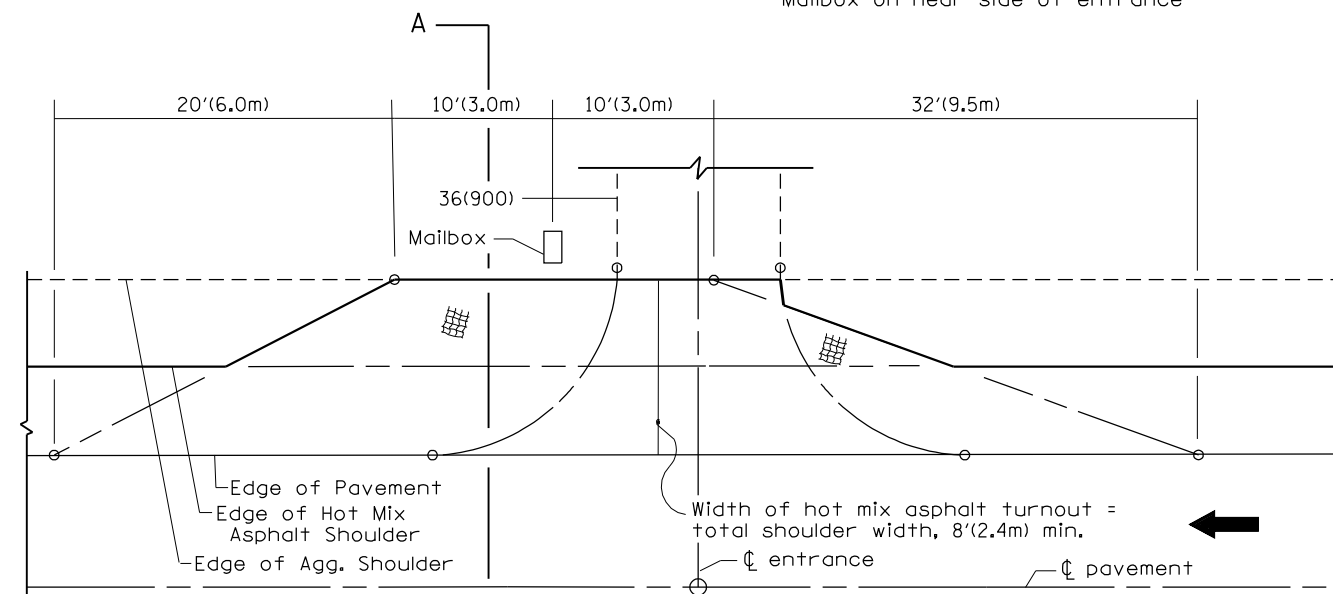
DESIGNER NOTES:
 1. THIS DRAWING REPLACES STATE STANDARD 406201
 2. DESIGNER SHOULD CONSULT CHAPTER 49 OF THE BDE MANUAL



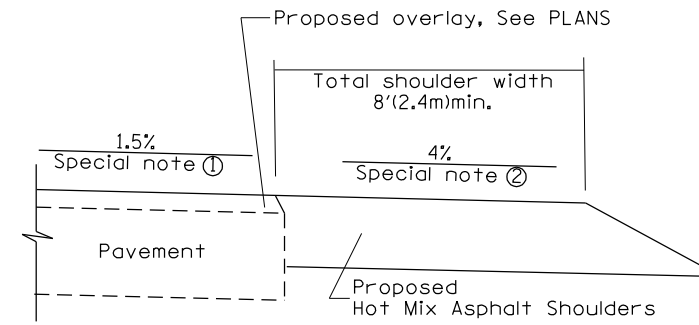
METHOD "T"
 Typical Application



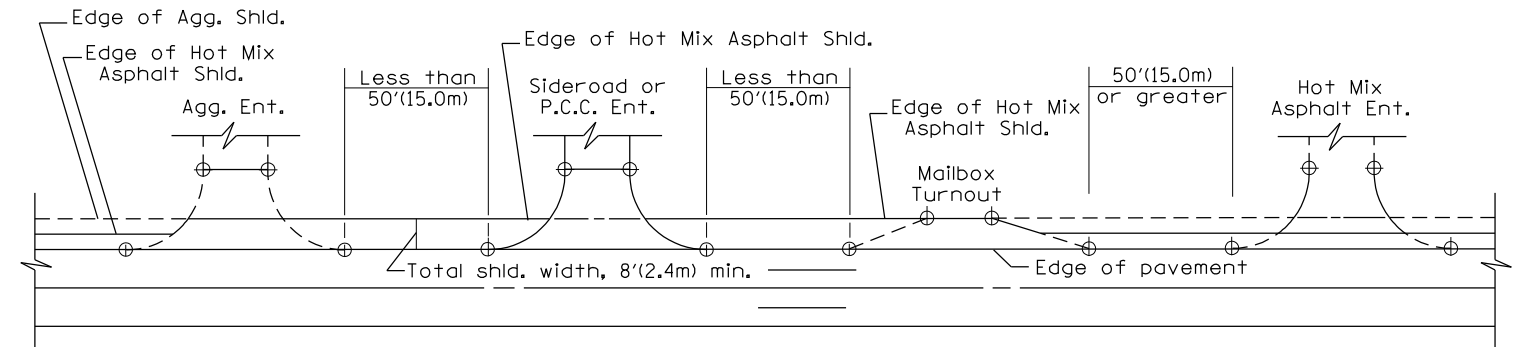
METHOD "N"
 Mailbox on near side of entrance



METHOD "F"
 Mailbox on far side of entrance



SECTION A-A



DETAIL A

SHOULDER TREATMENT FOR CLOSELY SPACED SIDEROADS, ENTRANCES, AND/OR MAILBOX TURNOUTS

GENERAL NOTES

- Mailbox turnouts shall slope away from the pavement edge at a rate equal to the shoulder slope. See SECTION A-A.
- The total shoulder width, 8'(2.4m) minimum, shall be paved between sideroads entrances and/or mailbox turnouts at locations where the distance between radius or taper control points is less than 50'(15.0m). See DETAIL A.
- Mailboxes shall be mounted such that the face of the mailbox is 6(150) to 12(300) and the post a minimum of 24(600) from the edge of the turnout surfacing.

SPECIAL NOTES

- The mainline pavement cross-slope is 1.5% for tangent alignment. See PLANS for cross-slope on superelevated horizontal curves.
- The shoulder slope shall control the turnout slope. The standard cross-slope is 4% for tangent alignment. Through superelevated curves, the maximum pavement-shoulder breakover should not be greater than 10% for shoulders 6'(1.8m) and wider and 12% for shoulders 4'(1.2m) and less. Where 12(300) paved shoulders are provided, the breakover should be at the edge of the paved shoulder rather than at the pavement edge.

All dimensions are in inches (millimeters) unless otherwise noted.

01-01-97	RENUM. C-90.01, NEW REVISION BOX	T.P.	
07-01-97	REVISE DESIGNER NOTES	J.A.	
09-15-05	REVISED DESIGNER NOTE	M.M.A.	
10-16-06	REVISED TO 2007 SPEC.	M.A.	

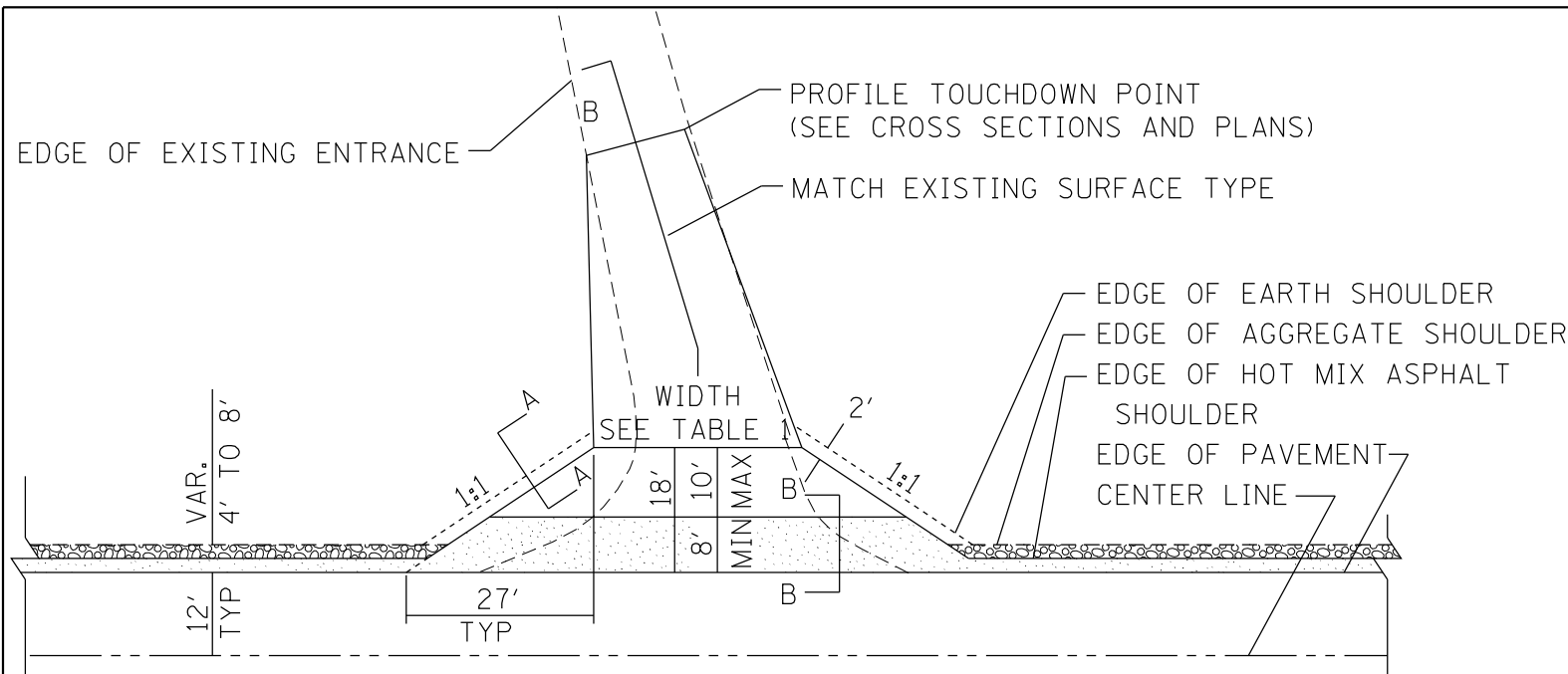
**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

MAILBOX TURNOUTS FOR "3R" PROJECTS

NOT TO SCALE

CADD STD. 406201-D4

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	474
FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT			CONTRACT NO. 68185	



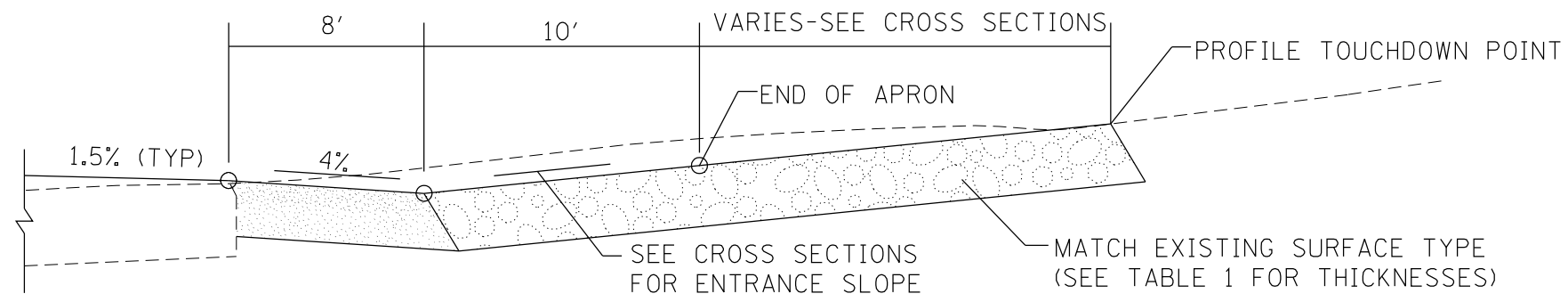
HOT MIX ASPHALT SHOULDER, 8"
 AGGREGATE SHOULDER, TYPE B, 6"

PLAN

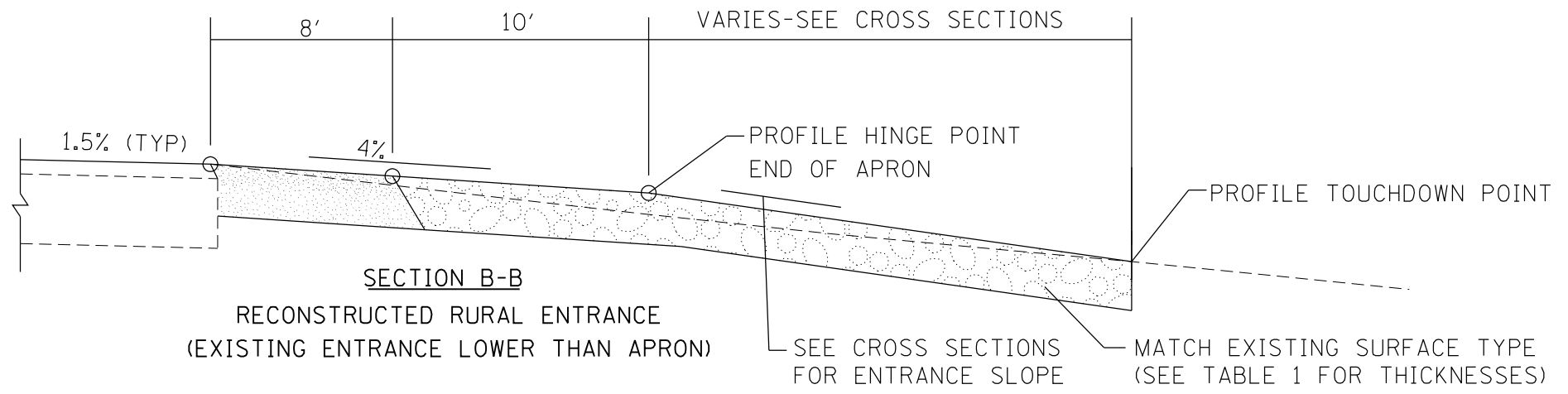
COMMERCIAL / FARM-RELATED ENTRANCE

TABLE 1					
RURAL ENTRANCE DESIGN					
ELEMENT	NON-COMMERCIAL		NON-COMMERCIAL W/ LARGE FARM EQUIPMENT	COMMERCIAL	
				1-WAY OPERATION	2-WAY OPERATION
WIDTH (W)	12'(3.6m) Min.	24'(7.2m) Max.	20' (6.1m)Min.	14'(4.3m) Min.	24'(7.2m) Max.
FLARE	1:1.5				24'(7.2m) Min.
MAX. GRADE (G)	12%		12%		10%

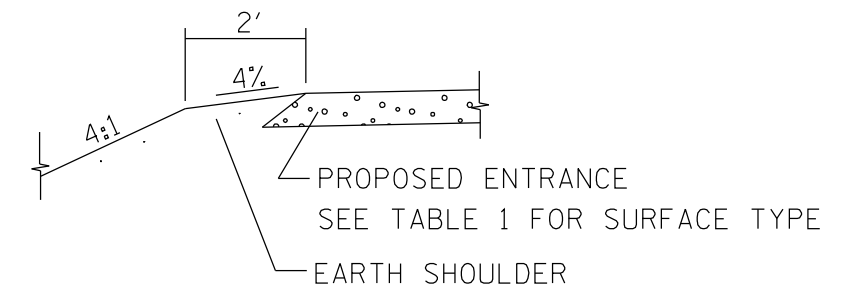
SURFACE TYPE				
INCIDENTAL HOT MIX ASPHALT SURFACING	6"	—	—	8"
AGGREGATE SURFACE COURSE	6"	8"	—	—
PCC DRIVEWAY PAVEMENT	6"	—	—	7"



SECTION B-B
RECONSTRUCTED RURAL ENTRANCE
(EXISTING ENTRANCE HIGHER THAN APRON)



SECTION B-B
RECONSTRUCTED RURAL ENTRANCE
(EXISTING ENTRANCE LOWER THAN APRON)



SECTION A-A
SHOULDER TREATMENT FOR RURAL ENTRANCES

GENERAL NOTES

- ENTRANCES SHALL SLOPE AWAY FROM THE PAVEMENT AT A RATE EQUAL TO THE SHOULDER SLOPE FOR A MINIMUM DISTANCE OF 8'.
- A MINIMUM 8' PAVED SHOULDER SHALL BE CONSTRUCTED BETWEEN LOCATIONS WHERE THE RURAL ENTRANCE IS LESS THAN 50' FROM AN ADJACENT SIDEROAD, ENTRANCE OR MAILBOX TURNOUT.
- A TAPER RATE OF 5:1 IS DESIRABLE WHEN TRANSITING FROM THE RURAL ENTRANCE WIDTH SHOWN IN TABLE 1, TO THE EXISTING ENTRANCE WIDTH.

All dimensions are in inches (millimeters) unless otherwise noted.

01-01-97	RENUM. C-103.06, NEW REVISION BOX	T.P.	10-16-06	REVISED TO 2007 SPEC.	M.A.
07-01-97	REVISE DESIGNER NOTES	J.A.	9-15-15	UPDATED TABLE 1	R.D.
01-17-03	ADJUST DESIGN, CHANGE ENTRANCE	JATR	2-29-16	MINOR CORRECTIONS	R.D.
09-15-05	RADIUS FOR FLARE	M.M.A.	5-9-17	CHANGED TAPER RATE	R.D.

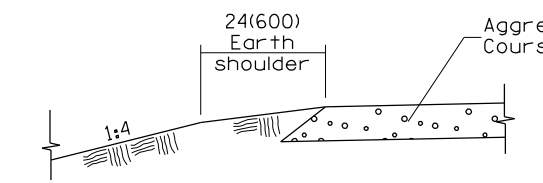
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

RURAL ENTRANCES FOR "3R" PROJECTS

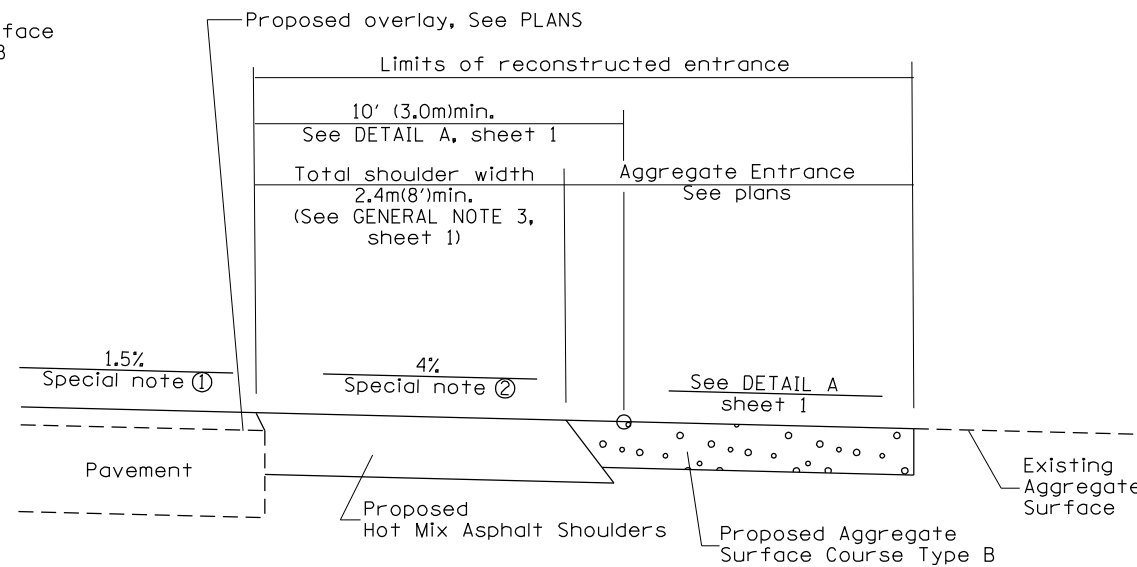
NOT TO SCALE

SHT. 1 OF 2
CADD STD. 406301-D4

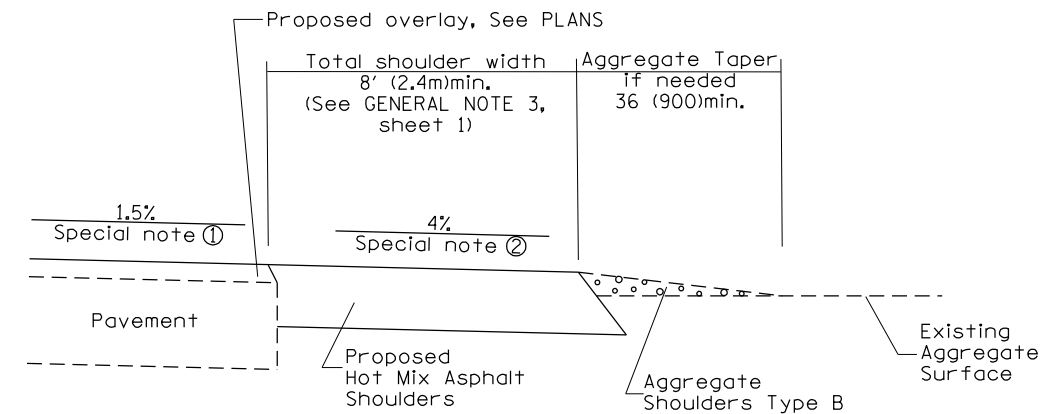
F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N,BR-1,RS-4,W-1)	PEORIA	577	475
FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT			CONTRACT NO. 68185	



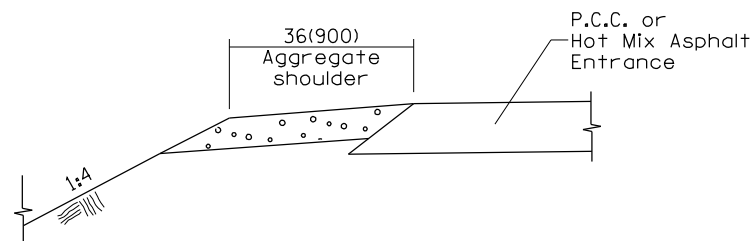
SECTION A-A
SHOULDER TREATMENT FOR AGGREGATE ENTRANCES



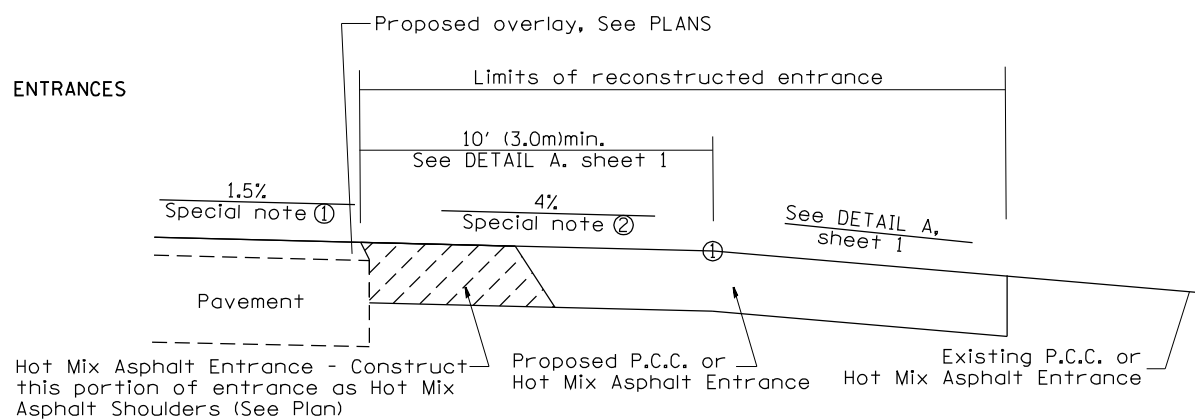
SECTION B-B
RECONSTRUCTED AGGREGATE ENTRANCE



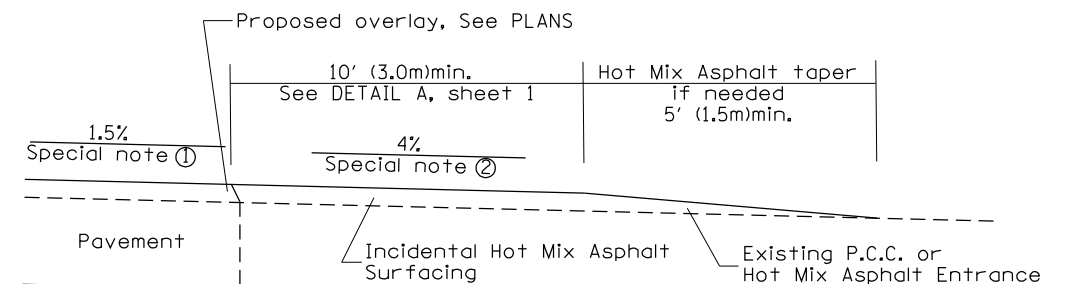
SECTION B-B
EXISTING AGGREGATE ENTRANCE



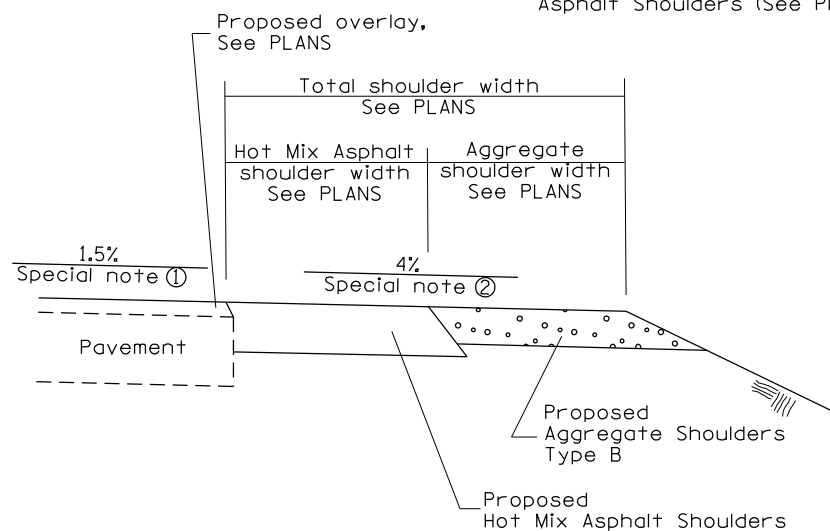
SECTION C-C
SHOULDER TREATMENT FOR P.C.C. OR HOT MIX ASPHALT ENTRANCES



SECTION D-D
RECONSTRUCTED P.C.C. OR HOT MIX ASPHALT ENTRANCE



SECTION D-D
EXISTING P.C.C. OR HOT MIX ASPHALT ENTRANCE



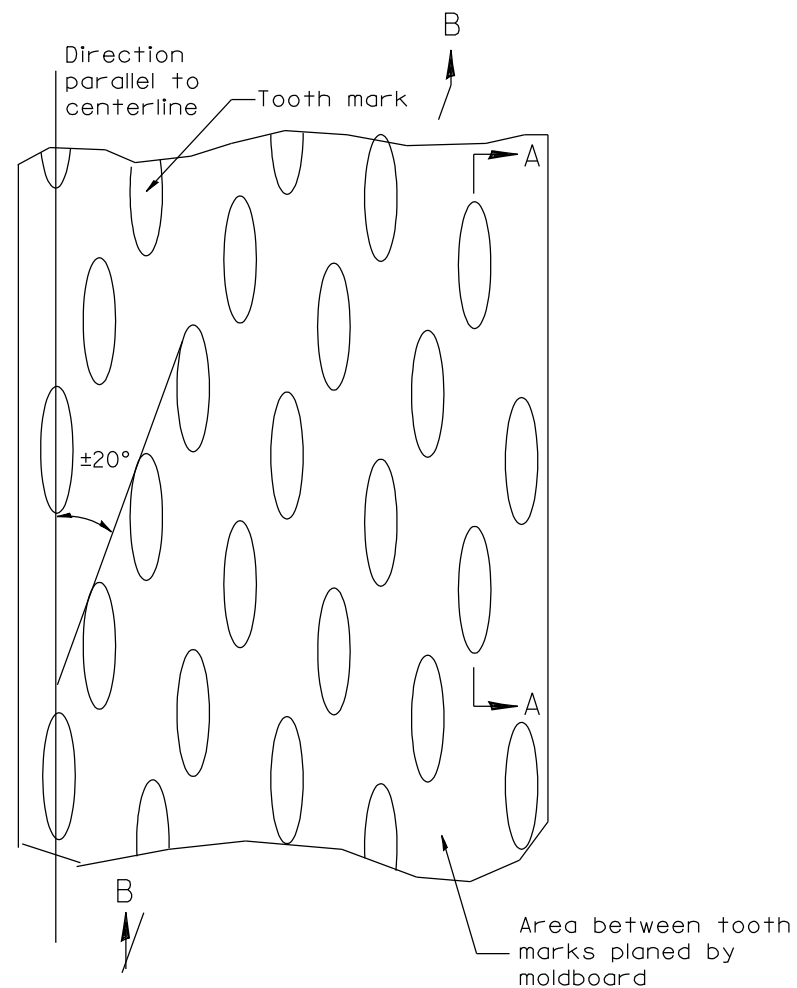
SECTION E-E
MAINLINE SHOULDER TREATMENT

SPECIAL NOTES

- ① The mainline pavement cross-slope is 1.5% for tangent alignment. See PLANS for cross-slope on super-elevated horizontal curves.
- ② The shoulder slope shall control the entrance profile for a distance of 10' (3.0m) minimum from the pavement edge. The shoulder cross-slope is 4% for tangent alignment. Through super-elevated curves, the maximum pavement-shoulder breakover should not be greater than 10% for shoulders 6' (1.8m) and wider and 12% for shoulders 4' (1.2m) and less. Where 12' (366cm) paved shoulders are provided, the breakover should be at the edge of the paved shoulder rather than at the pavement edge.

All dimensions are in inches (millimeters) unless otherwise noted.

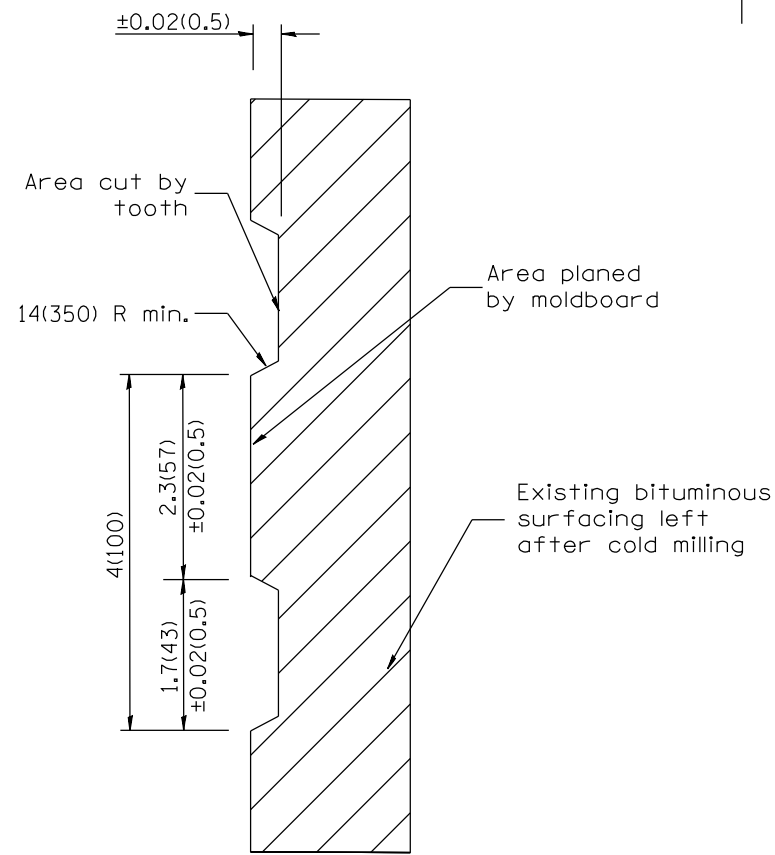
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION				RURAL ENTRANCES FOR "3R" PROJECTS				F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
								6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	476
NOT TO SCALE				SHT. 2 OF 2 CADD STD. 406301-D4				CONTRACT NO. 68185				
								FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT				



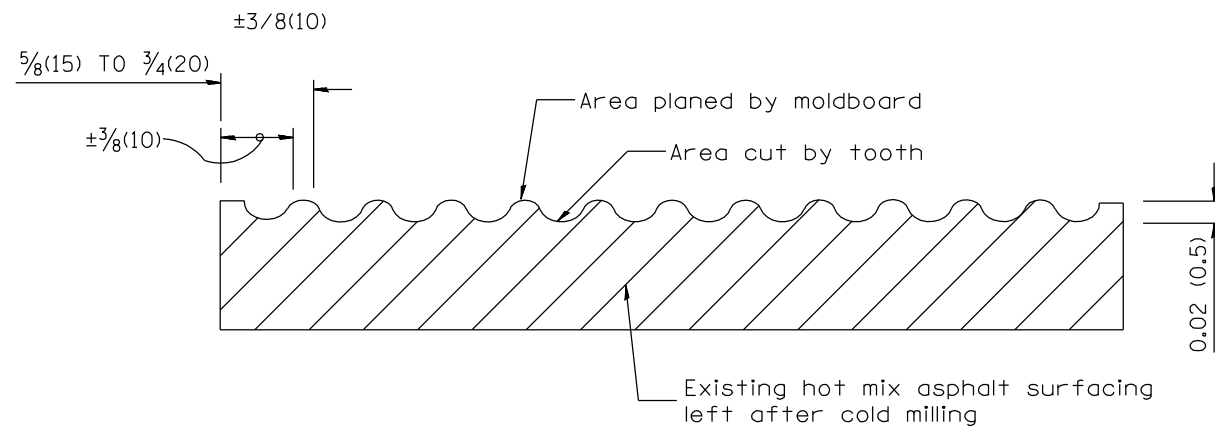
PLAN

General notes:

1. Coldmilling shall consist of two processes: Cutting with carbide teeth mounted on a rotating drum, and planing with a moldboard mounted immediately behind the cutting drum.
2. Other similar patterns will be acceptable if they consist of a smooth, flat, planed surface interspersed with a pattern of discontinuous longitudinal striations.



SECTION A-A



SECTION B-B PROJECTED
PERPENDICULAR TO CENTERLINE

DESIGNER NOTES:
1. INCLUDE DISTRICT SPECIAL PROVISION, IF APPLICABLE.

All dimensions are in inches (millimeters) unless otherwise noted.

01-01-97	RENUM. C-104.01, NEW REVISION BOX	T.P.
04-20-98	REMOVED MILLING DETAIL FROM STANDARD	J.A.
09-08-98	CORRECT NOTE LEADER PLACEMENT	R.W.
10-16-06	REVISED TO 2007 SPEC.	M.A.

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

HOT MIX ASPHALT SURFACE REMOVAL (COLD MILLING)

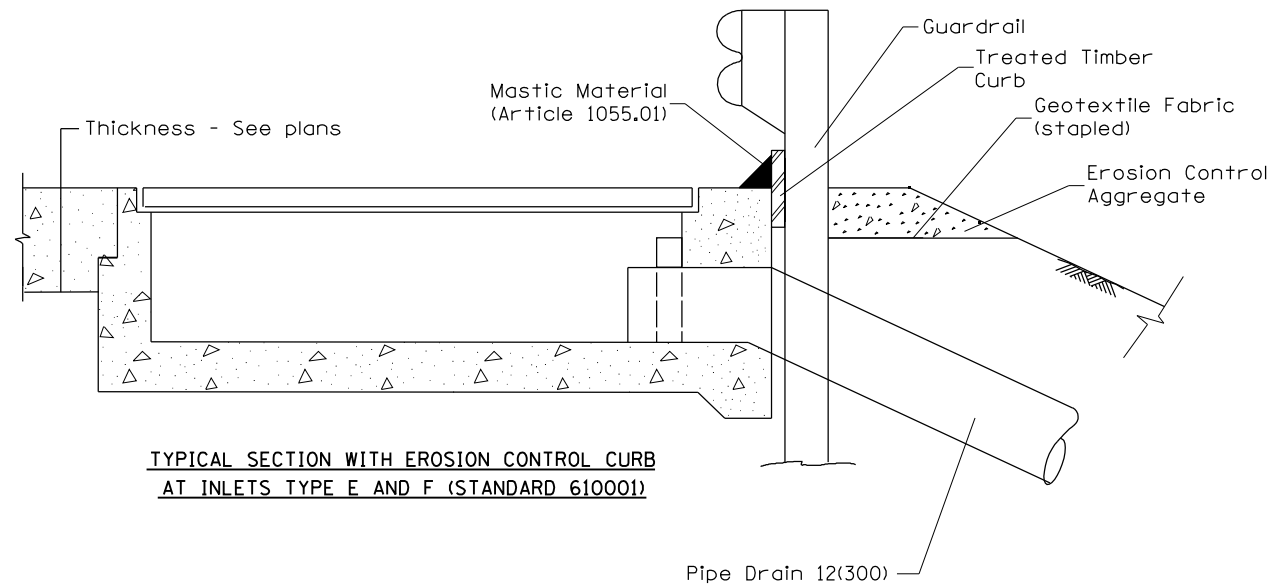
NOT TO SCALE

CADD STD. 440001-D4

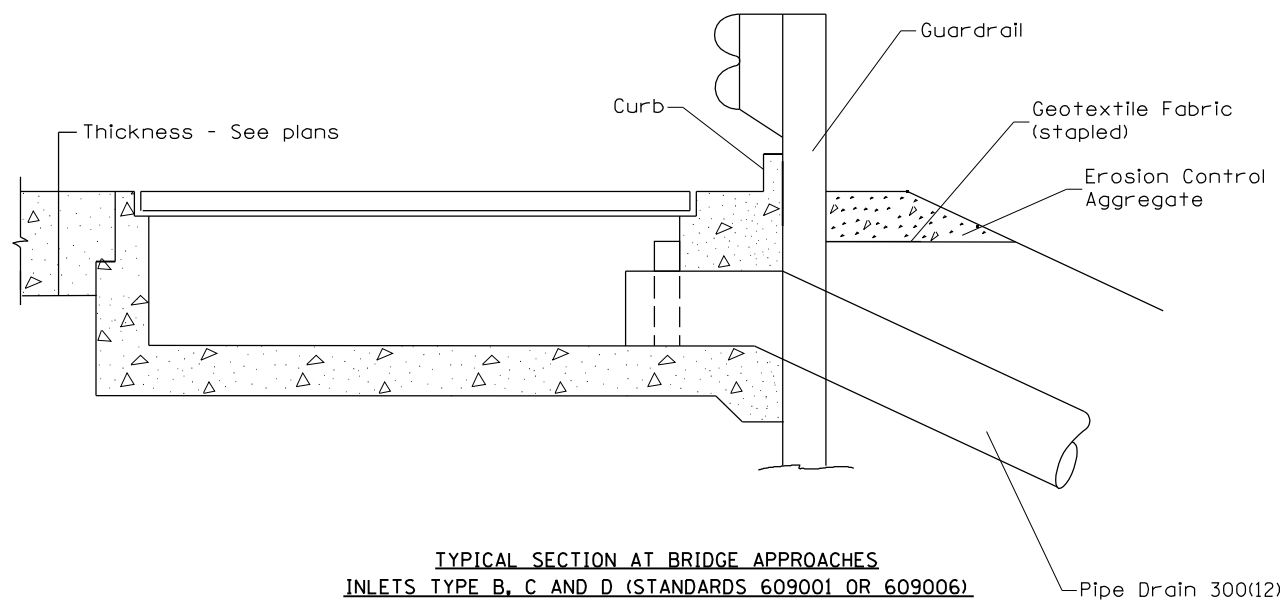
F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N,BR-1,RS-4,W-1)	PEORIA	577	477
CONTRACT NO. 68185				
FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT				

DESIGNER NOTES:

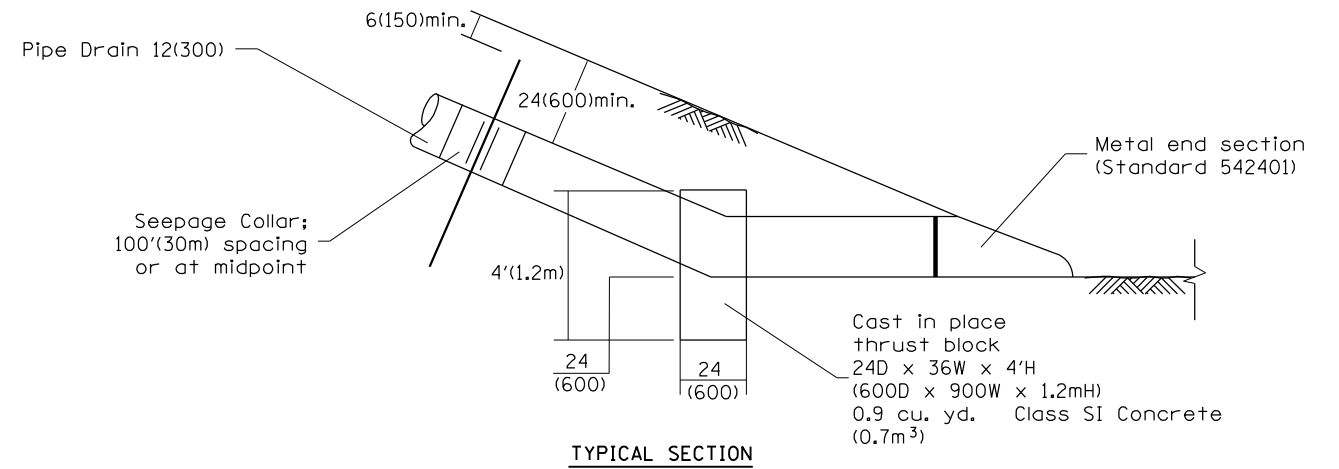
- 2. INCLUDE STATE STANDARDS 609001, 609006 OR 610001, IF APPLICABLE
- 3. INCLUDE DISTRICT CADD STANDARDS FOR "GUARDRAIL EROSION CONTROL TREATMENTS", "SEEPAGE COLLARS FOR BURIED PIPES", AND "PIPE ELBOW"



TYPICAL SECTION WITH EROSION CONTROL CURB AT INLETS TYPE E AND F (STANDARD 610001)



TYPICAL SECTION AT BRIDGE APPROACHES INLETS TYPE B, C AND D (STANDARDS 609001 OR 609006)



TYPICAL SECTION

GENERAL NOTES

1. The material for Pipe Drains shall be bituminous coated galvanized corrugated steel culvert pipe or bituminous coated corrugated aluminum alloy pipe in accordance with Article 601.02(f) or 601.02(i).
2. An approved mastic material (Article 1055.01) shall be applied to the inside of the connecting bands.

All dimensions are in inches (millimeters) unless otherwise noted.

QUANTITIES	
CALC. BY: _____	DATE: _____
CHECKED BY: _____	DATE: _____
QUANTITY CALCULATIONS ARE ON FILE AT THE DISTRICT 4 OFFICE; BUREAU OF PROJECT IMPLEMENTATION; DOCUMENTATION SECTION	

01-01-97	RENUM. H-1.04, NEW REVISION BOX, REVISED TITLE BOX, REVISED DESIGNER NOTES, ADDED QUANTITY	T.P.			
	CALCULATION BOX				
10-16-06	REVISED TO 2007 SPEC.	M.A.			

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SLOPE DRAIN DETAILS FOR BURIED PIPES

NOT TO SCALE

CADD STD. 601101-D4

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N,BR-1,RS-4,W-1)	PEORIA	577	478
CONTRACT NO. 68185				
FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT				

DESIGNER NOTES: 1. Use with District CADD Standards: Slope Drains, Buried Pipe and/or Slope Drains - Exposed Pipe

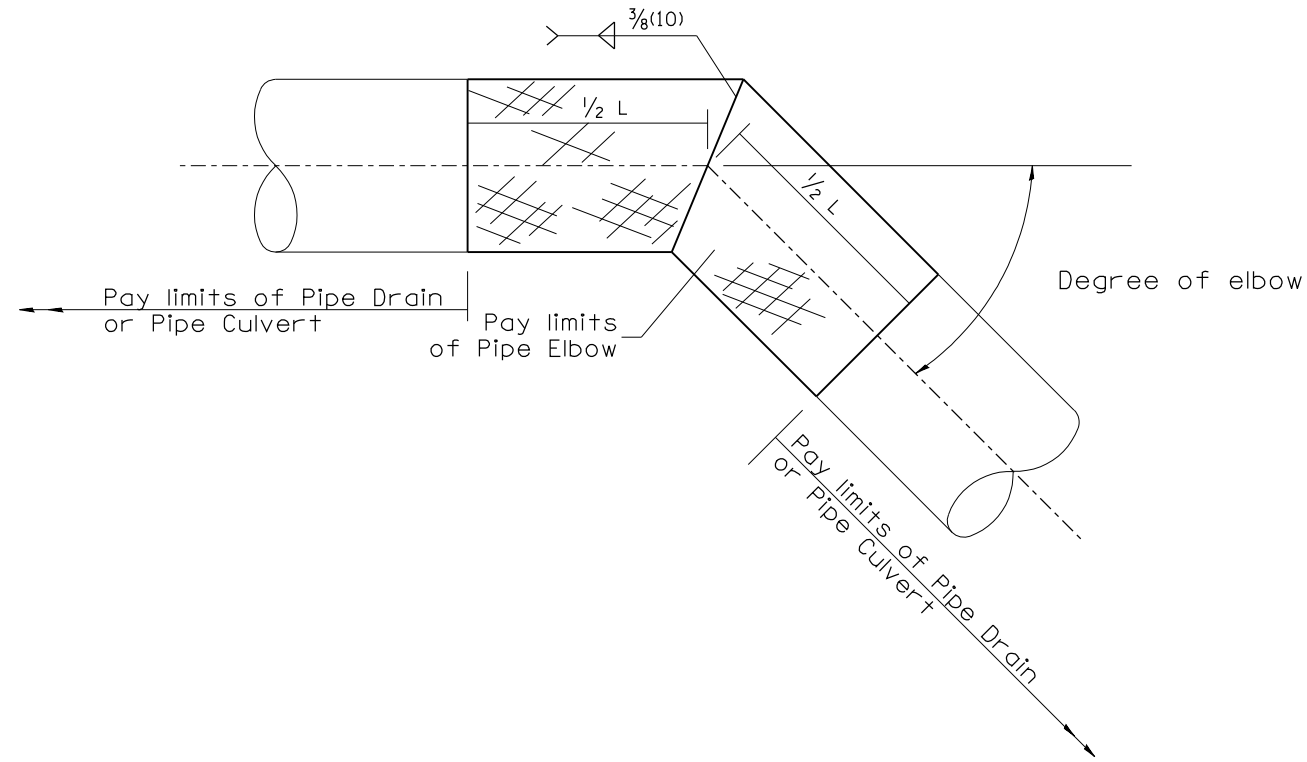
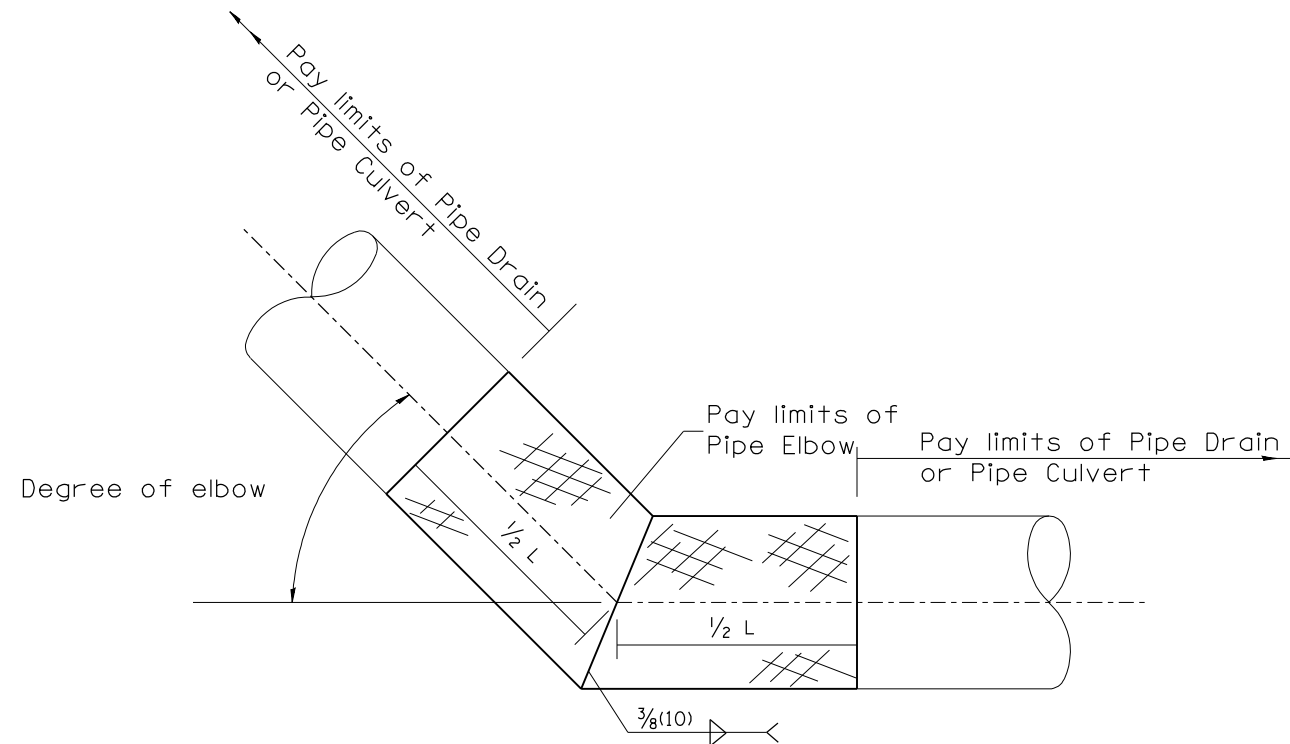


TABLE A		
ELBOW DESIGN CONTROLS		
PIPE DIAMETER	L = Pay limits of Pipe Elbow and minimum length of pipe required for fabrication	
	DEGREE OF ELBOW ≤ 45°	DEGREE OF ELBOW ≥ 46°
12(300)	24(600)	4'(1.22M)
15(375)	24(600)	4'(1.22M)
18(450)	24(600)	4'(1.22M)
21(525)	24(600)	4'(1.22M)
24(600)	4'(1.22M)	4'(1.22M)
30(750)	4'(1.22M)	6'(1.83M)
36(900)	4'(1.22M)	6'(1.83M)

TABLE B	
ELBOW DESIGN CONTROLS	
EARTH SLOPE (V:H)	DEGREE OF ELBOW *
1:6	9°
1:4	14°
1:3	18°
1:2	26°
1:1½	33°

* Approximate - based upon 0.5% inlet and outlet flowlines.



All dimensions are in inches (millimeters) unless otherwise noted.

01-01-97	RENUM. J-11.05, NEW REVISION BOX,	T.P.
	REVISED TITLE BOX	
10-16-06	REVISED TO 2007 SPEC.	M.A.

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

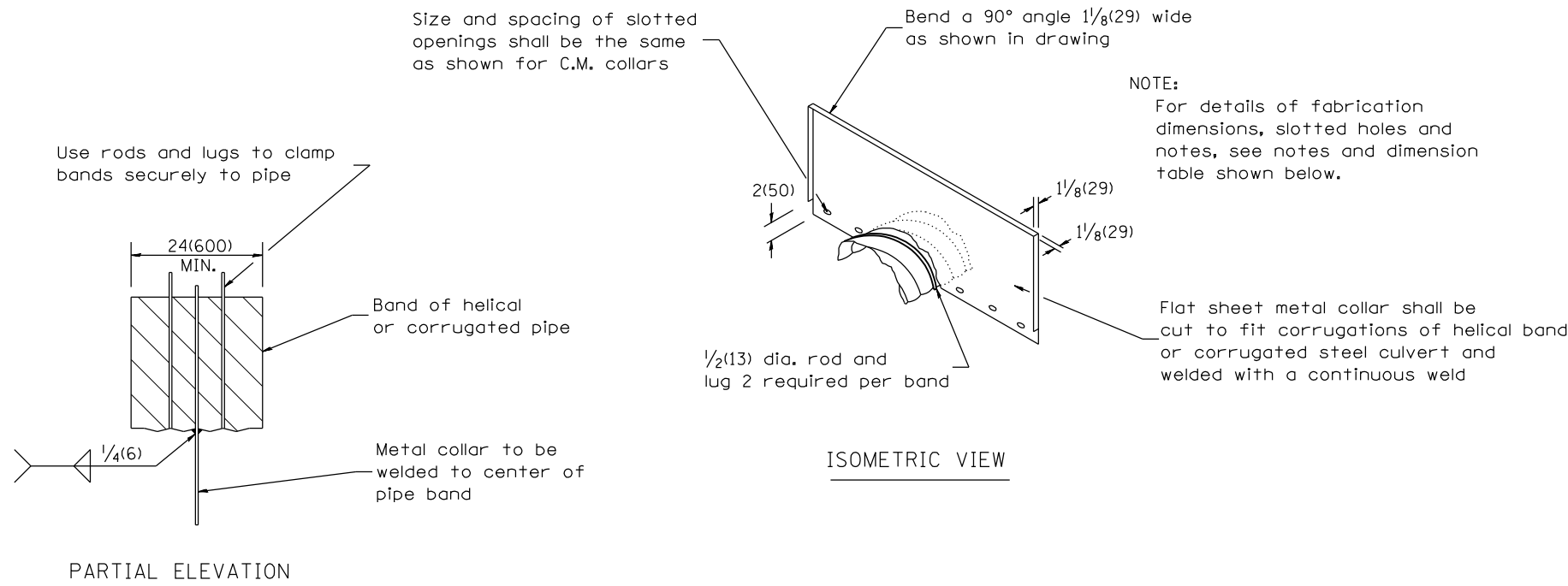
PIPE ELBOW

NOT TO SCALE

CADD STD. 601301-D4

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N,BR-1,RS-4,W-1)	PEORIA	577	479
FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT			CONTRACT NO. 68185	

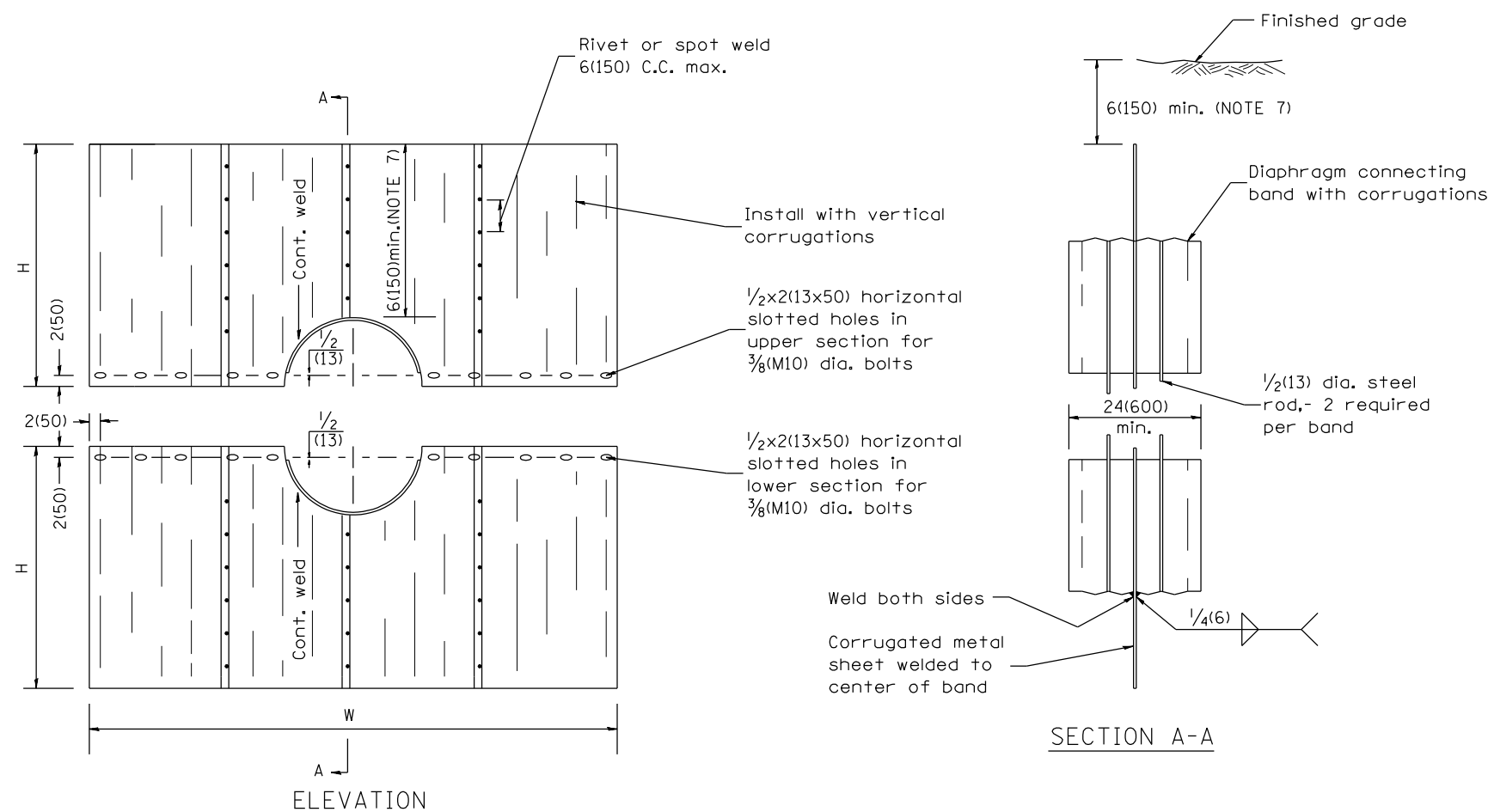
DESIGNER NOTES:
 1. USE WITH DISTRICT CADD STANDARD: "SLOPE DRAIN FOR BURIED PIPES"
 2. ADD DISTRICT SPECIAL PROVISION.



DETAILS OF CORRUGATED PIPE COLLAR

NOTES FOR COLLARS:

1. Materials and coatings for all collars shall be the same as that specified for the pipe, except that bituminous coated steel or aluminum collars may be used with PVC pipe.
2. Collars shall be shop fabricated, assembled and marked by painting to identify matching half sections of each collar.
3. The laps between the half sections and between the pipe and connecting bands shall be caulked with fiberized asphalt mastic at the time of installation.
4. All tank lugs, rods, and nuts shall be galvanized steel. Where aluminum collars are used, The rods and lugs shall be separated from the aluminum bands. By at least two (2) layers of 2(50) wide plastic tape with a total thickness of 2 1/4 mils or more.
5. The collars shall be welded to the connecting bands as shown on the drawings, all welds shall be treated as specified for class I, II, and III welds, miscellaneous. (Refer to AWS Standard Specifications)
6. Bands shall be fabricated from material having the same class of corrugations as the pipe to which it is to be attached.
7. Upper half of sheet may be cut shorter to provide 6(150) min. earth cover.



DETAILS OF SEEPAGE COLLAR

SEEPAGE COLLAR DIMENSION TABLE

PIPE DIAMETER	NOMINAL COLLAR SIZE	FABRICATIONS DIMENSIONS	
		W(WIDTH)	H(HEIGHT)
12(300) 15(375), 18(450) 21(525), 24(600)	8'x6' (2.4m x 1.8m)	8'-0" (2.44m)	38(966)
27(675) 30(750)	8'x7' (2.4m x 2.1m)	8'-0" (2.44m)	3'-8" (1.12m)
36(900), 42(1050) 48(1200)	10'x7' (3.0m x 2.1m)	10'-0" (3.05m)	3'-8" (1.12m)

Collar dimensions shown may be increased to allow fabrication from standard size sheets.

SEEPAGE COLLAR SPACING
Less than 24(600) pipe: 100' (30m) spacing or midpoint
Equal to or greater than 24(600) pipe: 80' (24m) spacing or midpoint

All dimensions are in inches (millimeters) unless otherwise noted.

01-01-97	RENUM. J-10.02, NEW REVISION BOX, REVISED	T.P.
	TITLE BOX, REVISED DESIGNER NOTES	
10-16-06	REVISED TO 2007 SPEC.	M.A.
7-15-15	REVISED NOTE 1 FOR PVC PIPE	R.D.

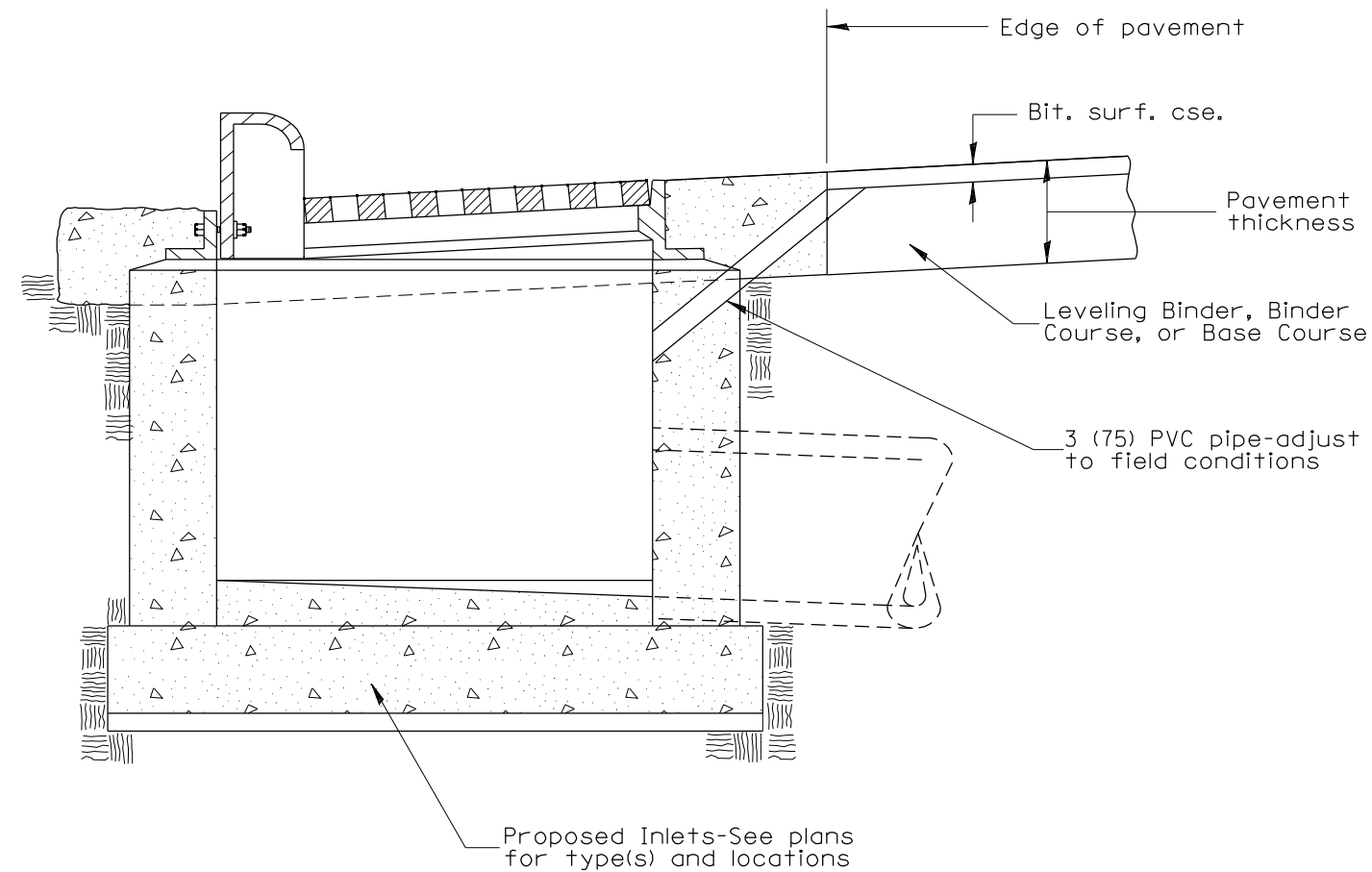
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

NOT TO SCALE

DETAILS OF SEEPAGE COLLARS FOR BURIED PIPES

CADD STD. 601401-D4

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N,BR-1,RS-4,W-1)	PEORIA	577	480
CONTRACT NO. 68185				
FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT				



GENERAL NOTES:

1. Temporary Drainage - This work consists of furnishing and installing a 3 (75) PVC pipe in the proposed inlet to facilitate drainage off the pavement prior to the placement of bituminous surface course.
2. Adequate pavement drainage shall be maintained during all phases of construction.
3. The PVC pipe is to be filled with mortar prior to surface course placement.
4. This detail applies to all inlet types.

All dimensions are in inches (millimeters) unless otherwise noted.

01-01-97	RENUM. B-4.11, NEW REVISION BOX	T.P.
12-01-98	CORRECT GENERAL NOTE	J.A.
10-16-06	REVISED TO 2007 SPEC.	M.A.

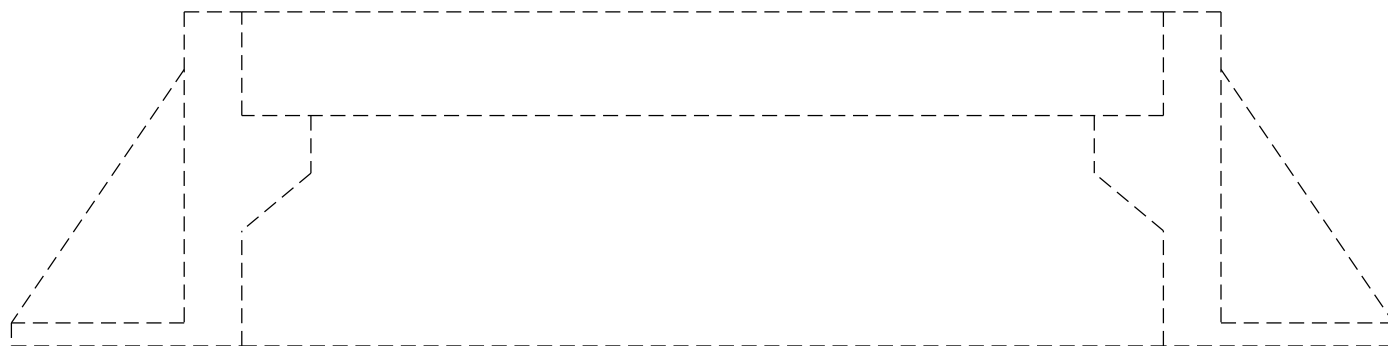
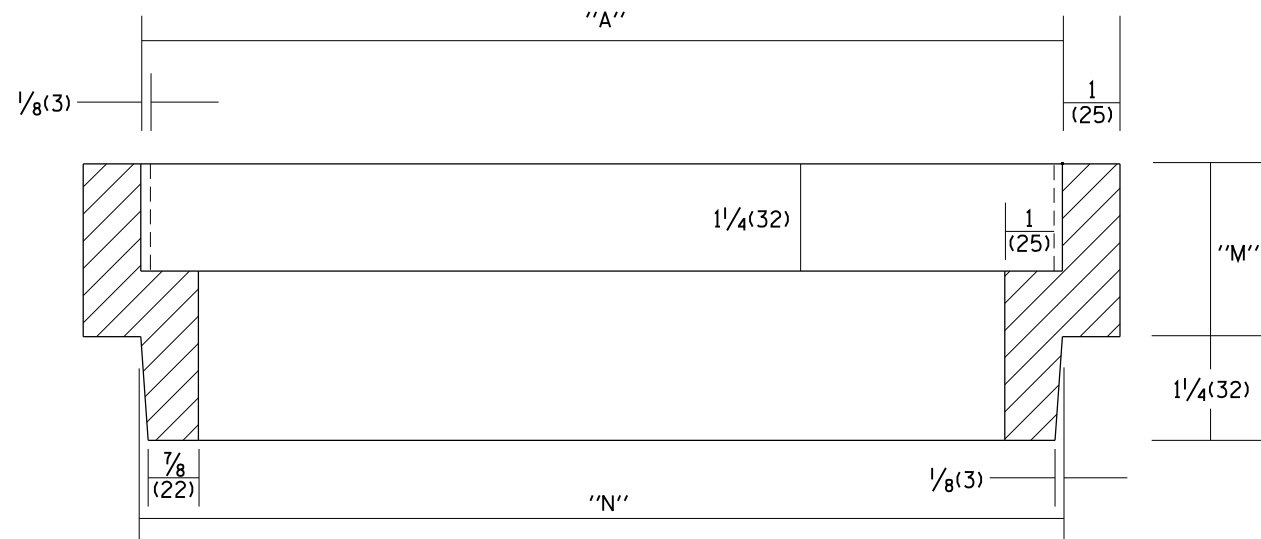
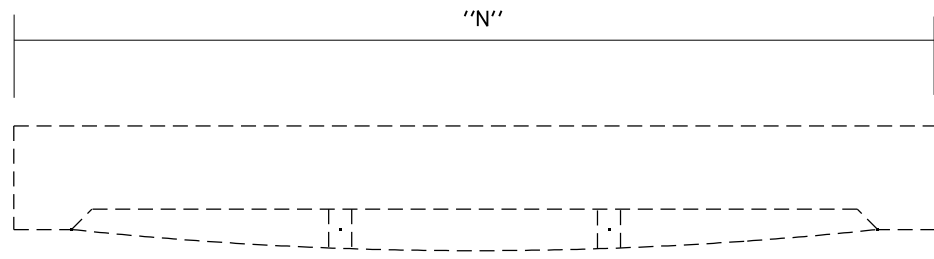
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

TEMPORARY INLET DRAINAGE TREATMENT

NOT TO SCALE

CADD STD. 602401-D4

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N,BR-1,RS-4,W-1)	PEORIA	577	481
CONTRACT NO. 68185				
FED. ROAD DIST. NO.		ILLINOIS FED. AID PROJECT		



Station	Dimension inches (millimeters)			Quantity
	"A"	"N"	"M"	
Total				

General Notes:
 1. Article 603.08 shall apply

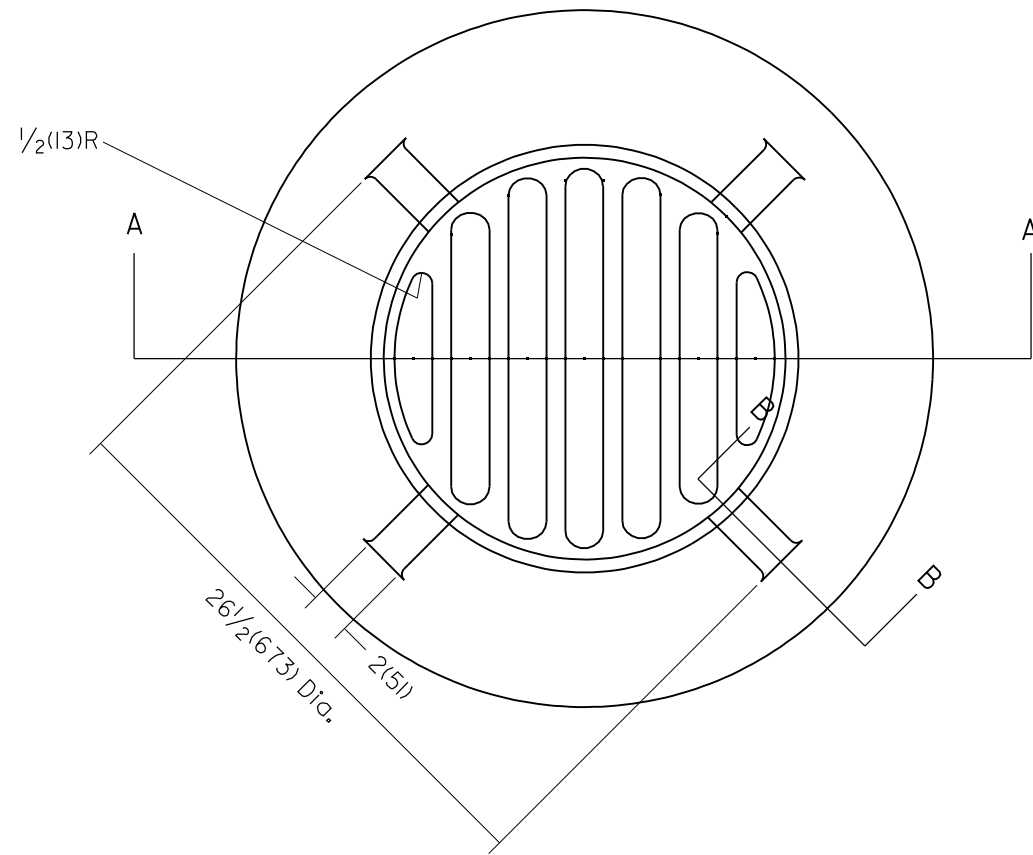
All dimensions are in inches (millimeters) unless otherwise noted.

01-01-97	RENUM. B-6.01, NEW REVISION BOX	T.P.		STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	FRAME & GRATE ADJUSTING RING	F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
10-16-06	REVISED TO 2007 SPEC.	M.A.				6659	11(N,BR-1,RS-4,W-1)	PEORIA	577	482
						CONTRACT NO. 68185				

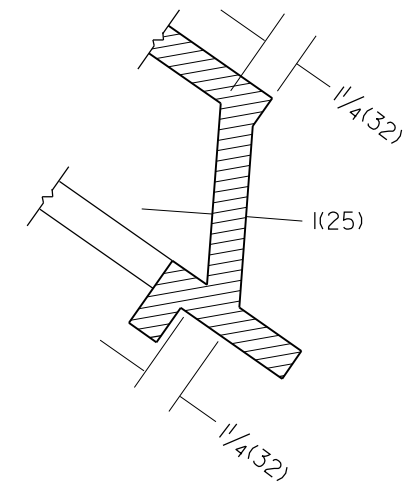
NOT TO SCALE

CADD STD. 604201-D4

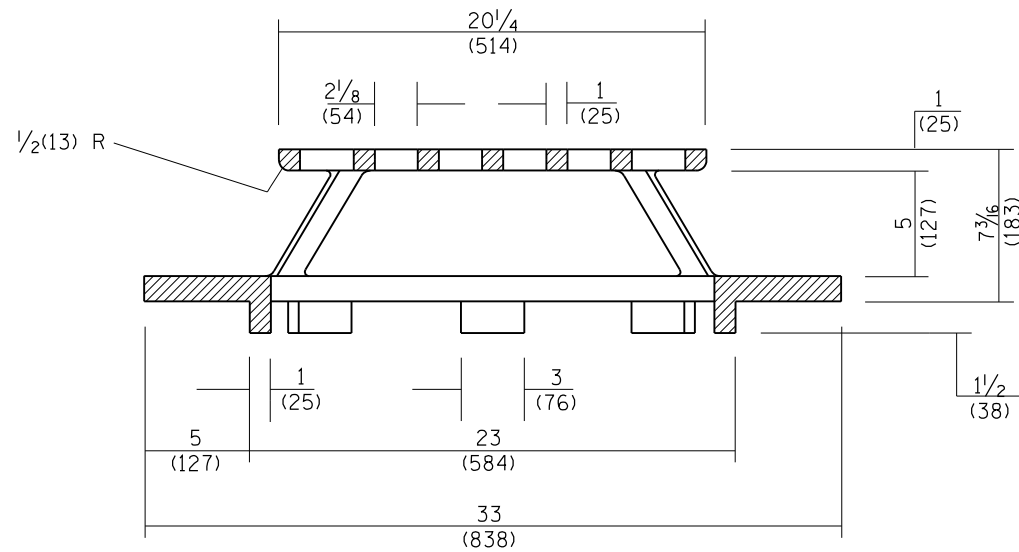
FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT



PLAN



SECTION B - B



SECTION A - A

GENERAL NOTES

- MATERIAL - Cast Gray Iron
Weight 209 lbs (95 kg)

DESIGNER NOTE:
1. INCLUDE DISTRICT SPECIAL PROVISION.

All dimensions are in inches (millimeters) unless otherwise noted.

01-01-97	RENUM. B-10.02, NEW REVISION BOX	T.P.
10-16-06	REVISED TO 2007 SPEC.	M.A.

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

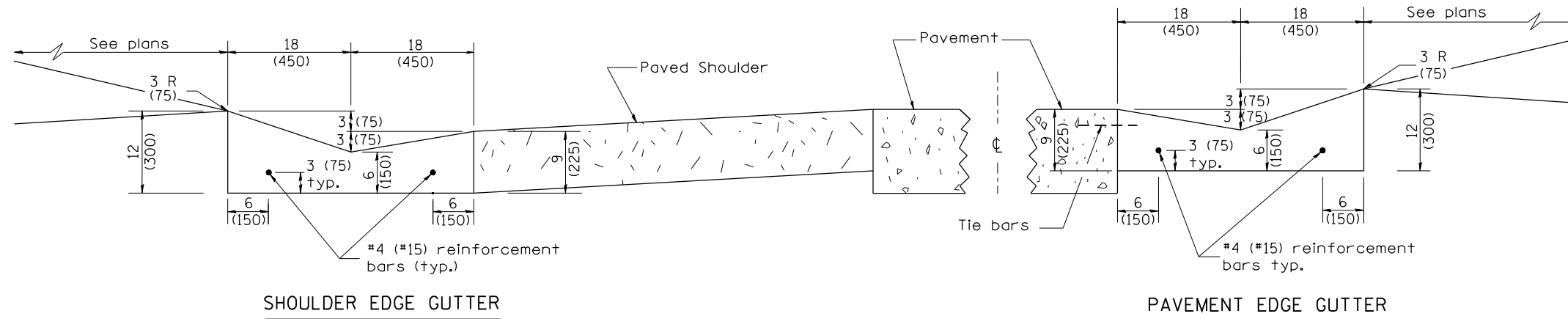
TYPE 37 GRATE

NOT TO SCALE

CADD STD. 604301-D4

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N,BR-1,RS-4,W-1)	PEORIA	577	483
CONTRACT NO. 68185				
FED. ROAD DIST. NO.	ILLINOIS	FED. AID PROJECT		

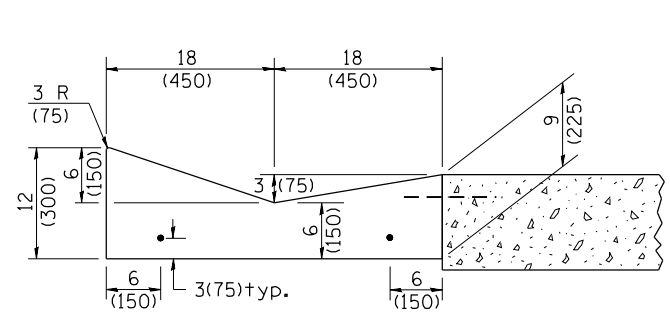
DESIGNER NOTES:
 1. INCLUDE STATE STANDARD 420001.
 2. PAY ITEM FOR INLETS, OUTLETS, AND ENTRANCES IS X6060097 CLASS SI CONCRETE (OUTLET), SPECIAL



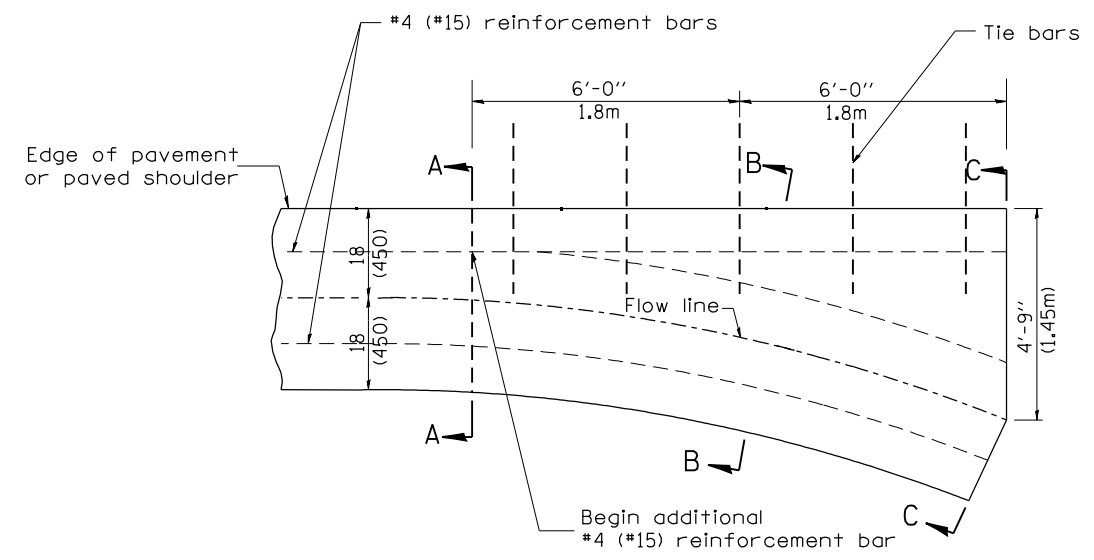
SHOULDER EDGE GUTTER

PAVEMENT EDGE GUTTER

CONCRETE GUTTER, TYPE A, (SPECIAL)

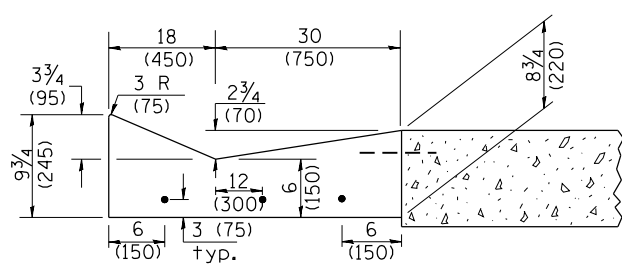


SECTION A-A

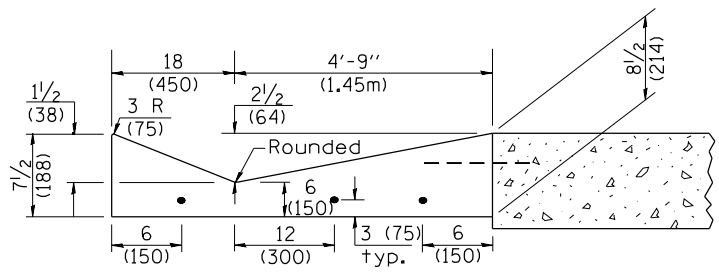


PLAN

QUANTITY
 Section C-C to A-A= 1.2 cu. yd.
 (0.92 m³) concrete.



SECTION B-B



SECTION C-C

INLET

GENERAL NOTES:

1. CONCRETE GUTTER, TYPE A, (SPECIAL) shall conform to the applicable portions of Section 606.
2. Tie bars shall be No. 6x24 (No. 19x600) at 36" (900mm) centers unless otherwise shown.
3. Gutter, gutter inlets, gutter outlets, and gutter entrances shall be tied to rigid pavement in accordance with details shown on Standard 420001.
4. Joints shall be constructed in accordance with Article 606.06.
5. Welded wire fabric shall conform to Article 1006.10(c)(1), and shall not be less than 58 lbs/100 sq.ft. (2.83 kg/m²).

QUANTITIES	
CALC. BY:	DATE:
CHECKED BY:	DATE:
QUANTITY CALCULATIONS ARE ON FILE AT THE DISTRICT 4 OFFICE; BUREAU OF PROJECT IMPLEMENTATION; DOCUMENTATION SECTION	

DATE	DESCRIPTION	BY	REVISION
01-01-97	RENUM. A-1.02, NEW REVISION BOX, ELIMINATED	T.P.	
11-16-07	EXPANSION ANCHOR TIES	M.A.	
02-28-02	ENTRANCE TYPICALS REVISED	M.A.	
10-16-06	REVISED TO 2007 SPEC.	M.A.	
01-10-07	REVISED QUANTITY	M.A.	
11-16-07	REVISED QUANTITY	M.A.	
02-15-11	CHANGED MODIFIED TO SPECIAL	R.D.	
01-31-18	REVISED TIE BAR SIZE & SPACING	R.D.	

**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**CONCRETE GUTTER, TYPE A, (SPECIAL)
 (INLET, OUTLET & ENTRANCE)**

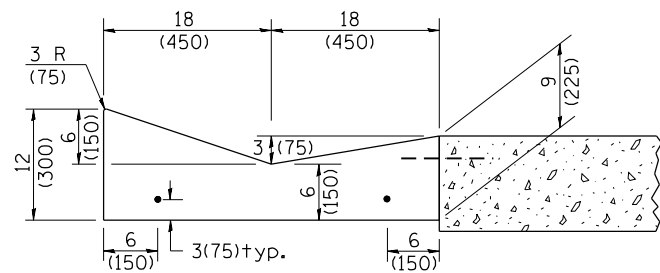
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SHT. 1 OF 3
 CADD STD. 606101-D4

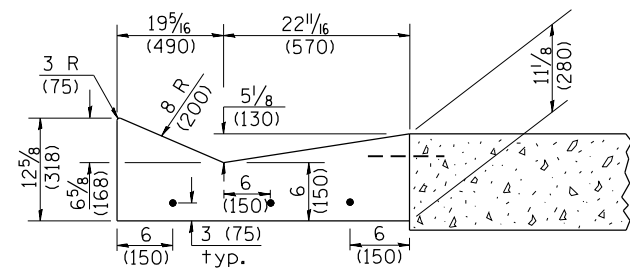
All dimensions are in inches (millimeters) unless otherwise noted.

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	484
FED. ROAD DIST. NO.			ILLINOIS FED. AID PROJECT	

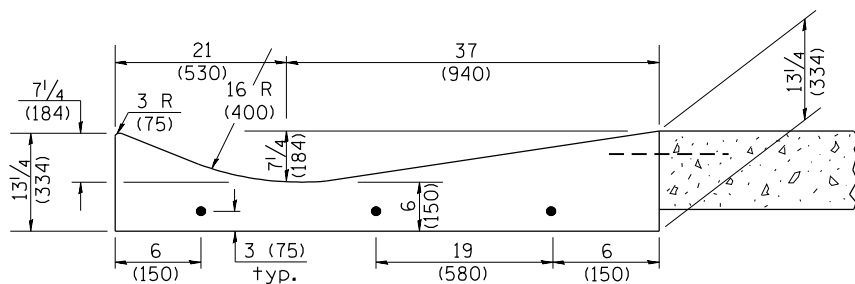
CONTRACT NO. 68185



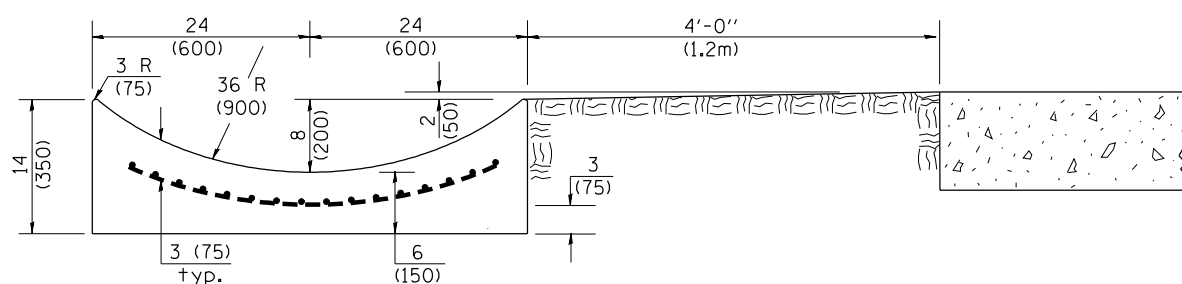
SECTION A-A



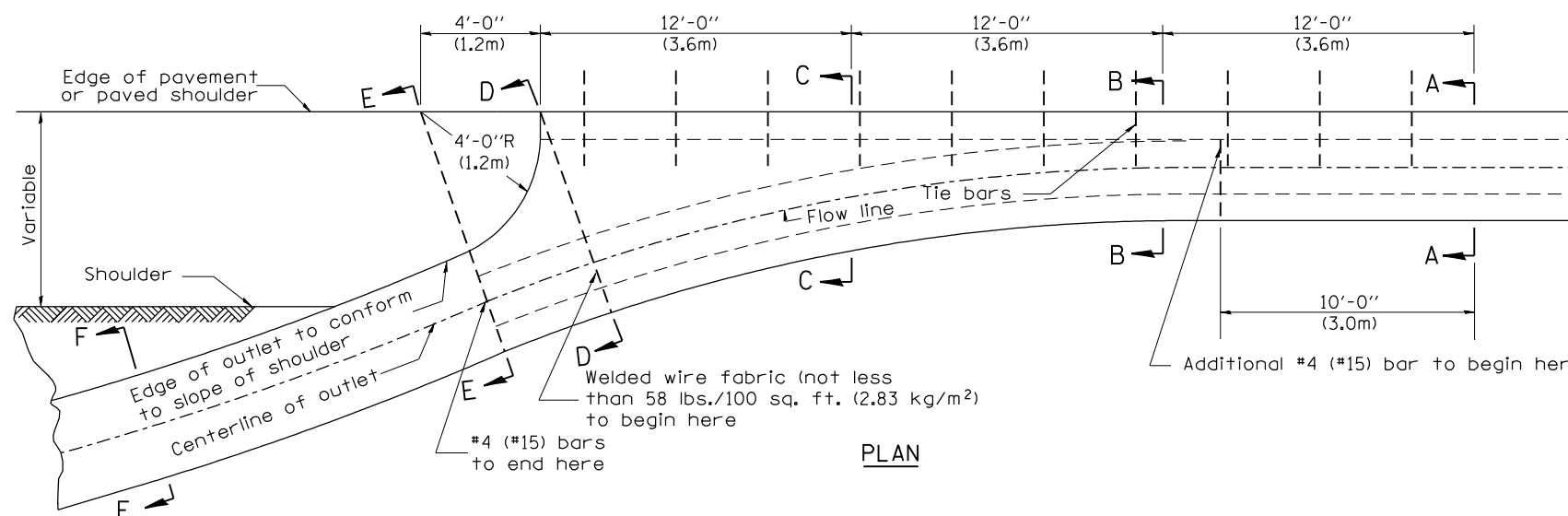
SECTION B-B



SECTION C-C



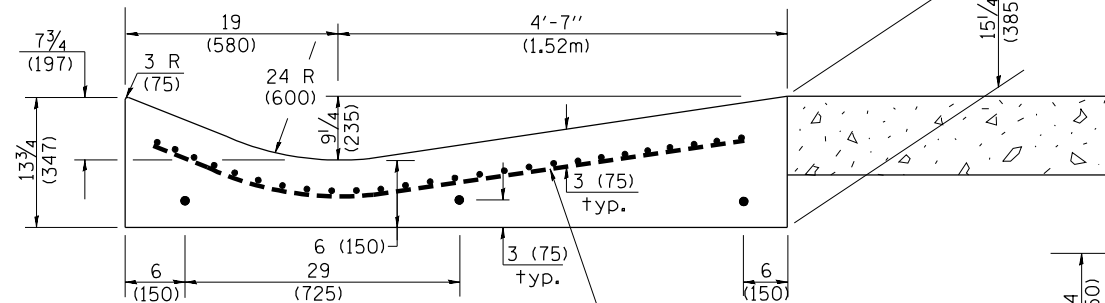
SECTION E-E



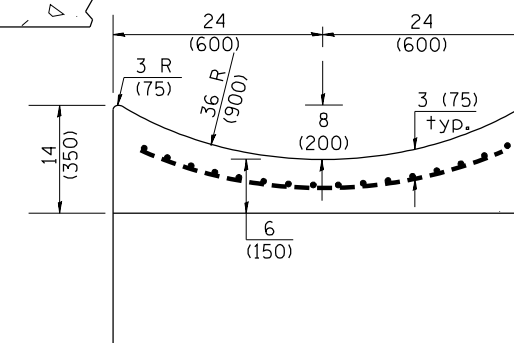
PLAN

QUANTITY
 Section A-A to E-E = 4.5 cu. yd. (3.36 m³) concrete.
 Section E-E to F-F = 0.10 cu. yd./ft. (0.26 m³/m) concrete.

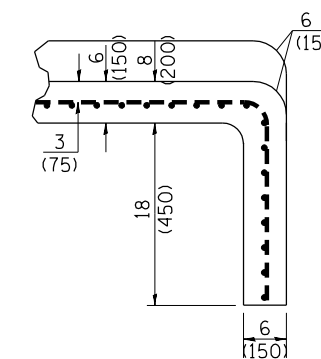
If the average grade of pavement for the distance from section A-A to section D-D exceeds 2%, this distance shall be increased 6 ft. (1.8 m) for each 1% increase in grade. A quantity adjustment is required.



SECTION D-D



SECTION F-F



SECTIONS AT END OF OUTLET
(CURTAIN WALL)

QUANTITY
 Curtain Wall = 0.1 cu. yd. (0.08 m³) concrete.

QUANTITIES
 CALC. BY: _____ DATE: _____
 CHECKED BY: _____ DATE: _____
 QUANTITY CALCULATIONS ARE ON FILE AT THE DISTRICT 4 OFFICE; BUREAU OF PROJECT IMPLEMENTATION; DOCUMENTATION SECTION

OUTLET

STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

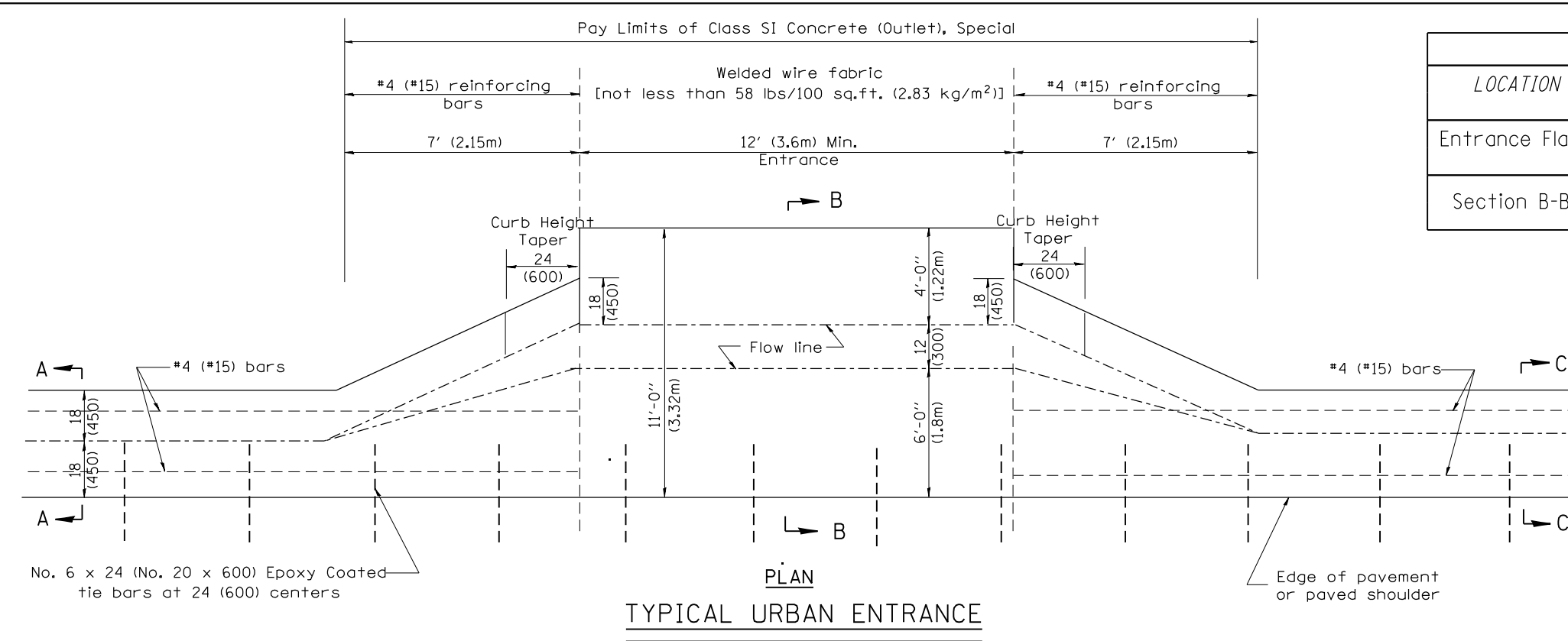
CONCRETE GUTTER, TYPE A, (SPECIAL)
 (INLET, OUTLET & ENTRANCE)

NOT TO SCALE

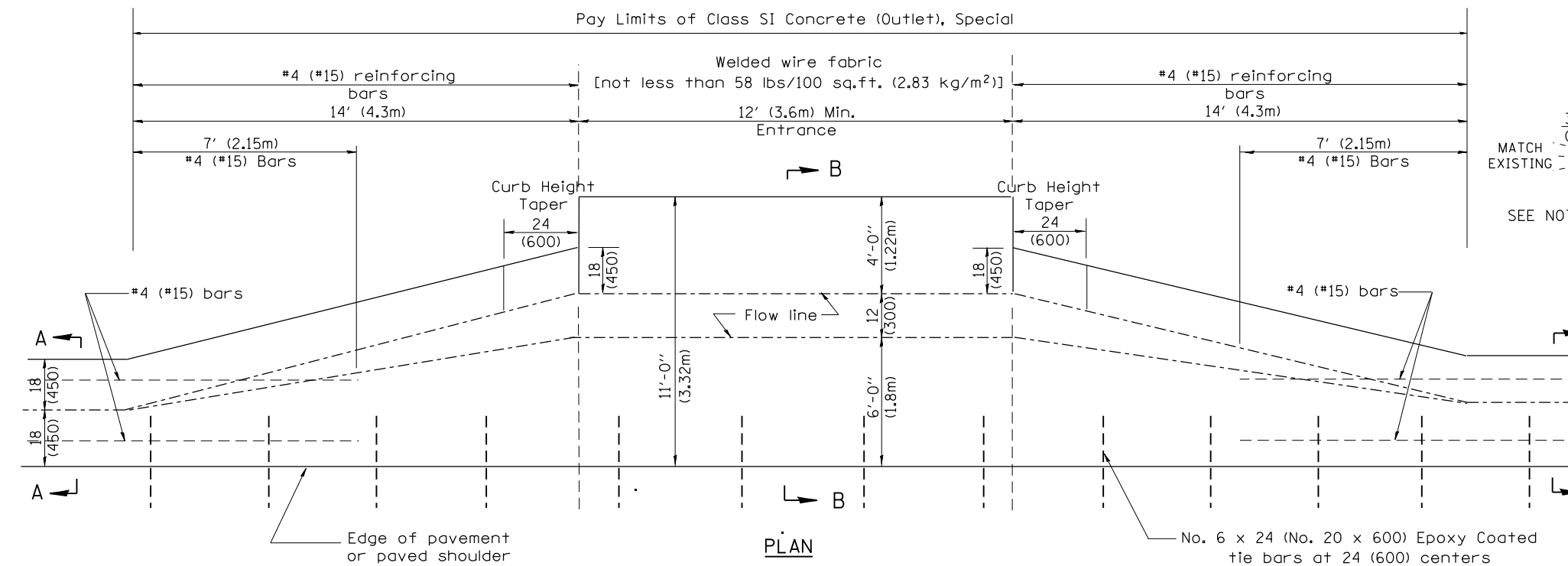
SHT. 2 OF 3
 CADD STD. 606101-D4

All dimensions are in inches (millimeters) unless otherwise noted.

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	485
FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT			CONTRACT NO. 68185	



TYPICAL URBAN ENTRANCE

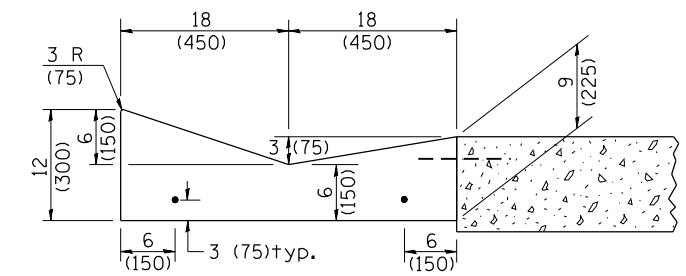


TYPICAL RURAL ENTRANCE

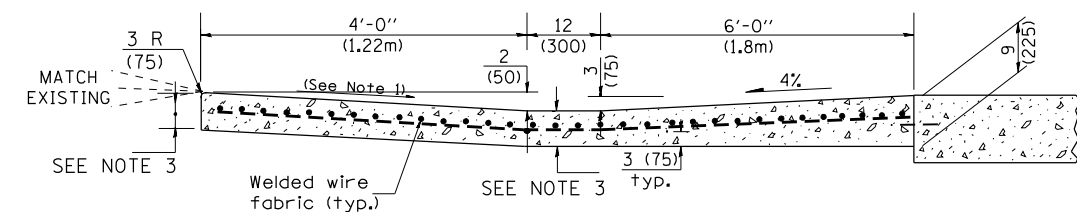
QUANTITIES	
CALC. BY: _____	DATE: _____
CHECKED BY: _____	DATE: _____

QUANTITY CALCULATIONS ARE ON FILE AT THE DISTRICT 4 OFFICE; BUREAU OF PROJECT IMPLEMENTATION; DOCUMENTATION SECTION

QUANTITY CALCULATION			
LOCATION	LENGTH	NON-COMMERCIAL 6 (150)	COMMERCIAL ENTRANCE 8 (200)
Entrance Flare	7 Ft (2.15 m) Urban 14 Ft (4.30 m) Rural	0.15 Cu Yd / Ft (0.37 Cu M / M)	0.18 Cu Yd / Ft (0.45 Cu M / M)
Section B-B	See Plans	0.23 Cu Yd / Ft (0.57 Cu M / M)	0.28 Cu Yd / Ft (0.70 Cu M / M)



SECTION A-A & C-C



SECTION B-B

- GENERAL NOTES
- Slope may be increased from 4% (min.) to 6% (max.) in order to match the existing.
 - The cross-slope is to be constructed as given in the plans from back turnout to where driveway matches existing.
 - For Non-Commercial Entrances the driveway thickness shall be 6 (150). For Commercial Entrances the driveway thickness shall be 8 (200).

All dimensions are in inches (millimeters) unless otherwise noted.

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

CONCRETE GUTTER, TYPE A, (SPECIAL)
(INLET, OUTLET & ENTRANCE)

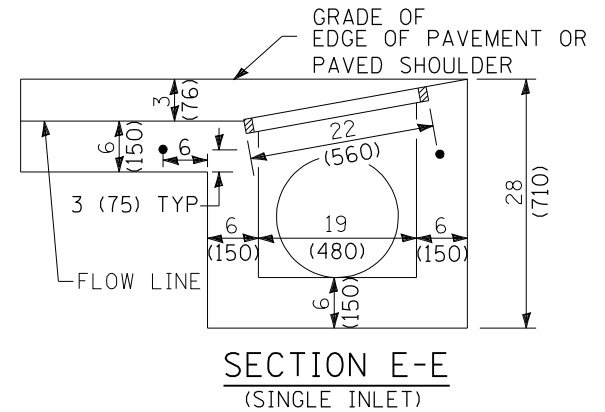
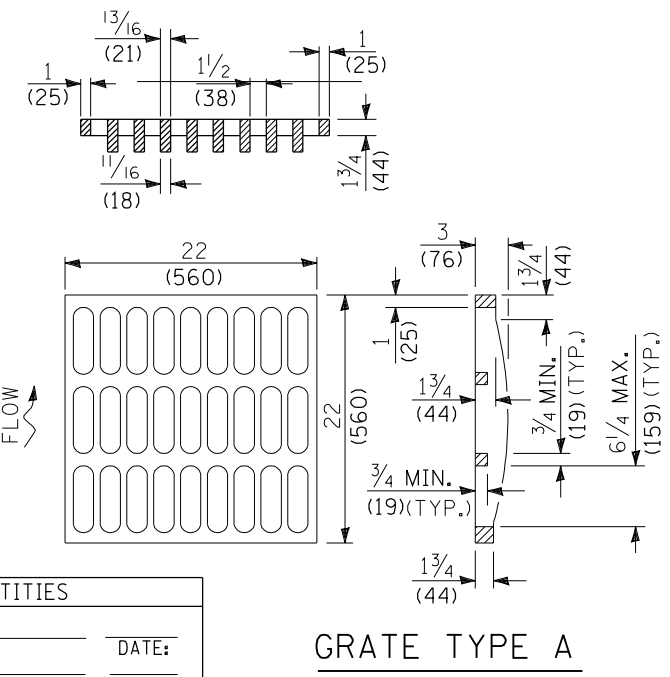
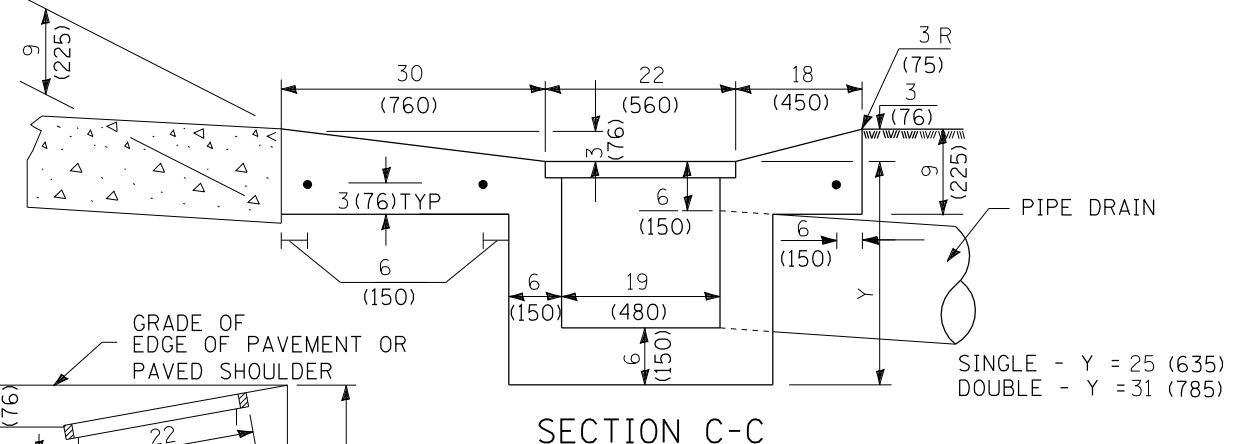
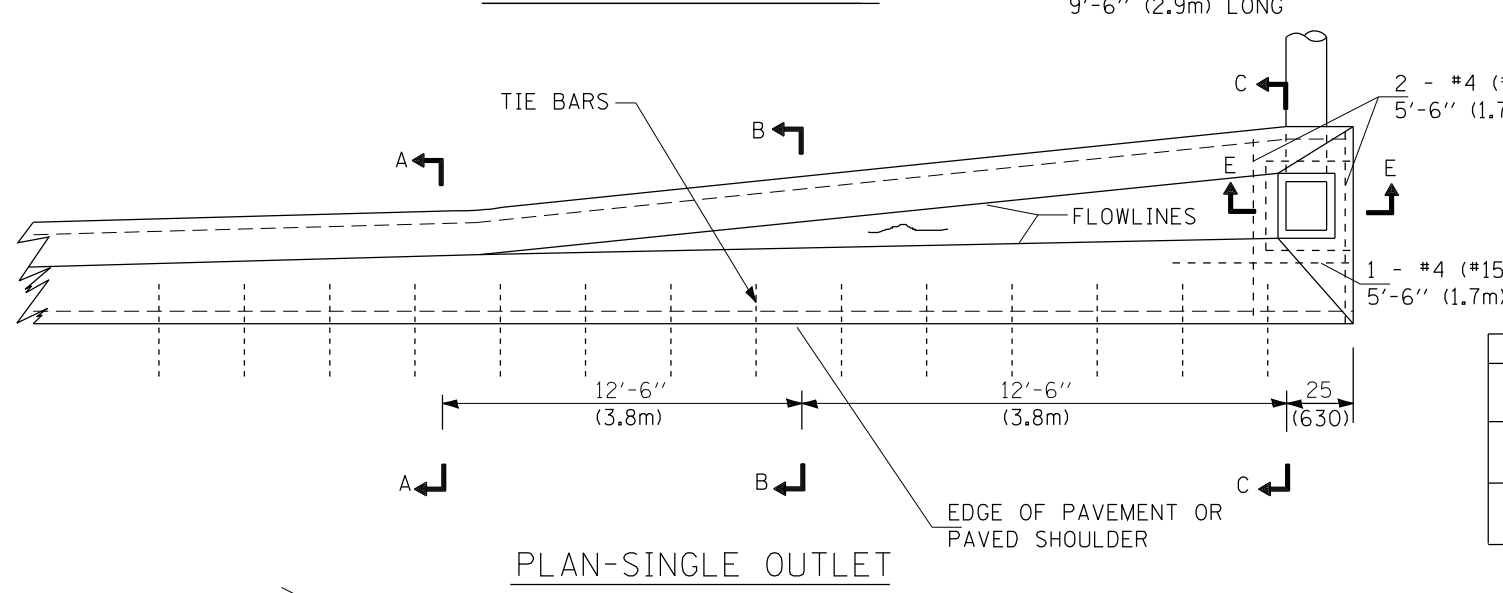
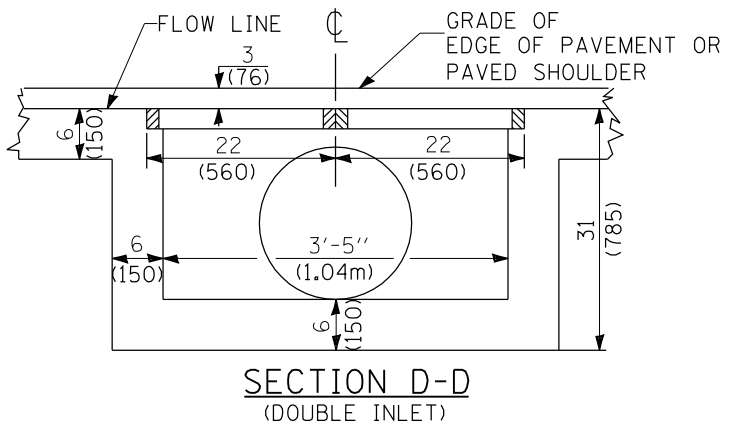
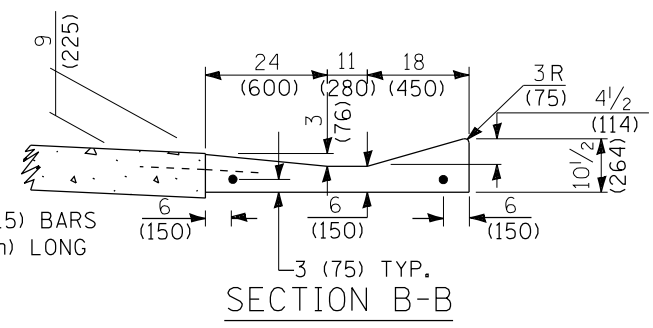
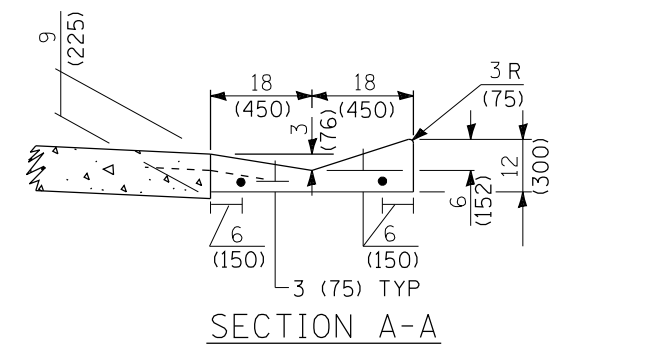
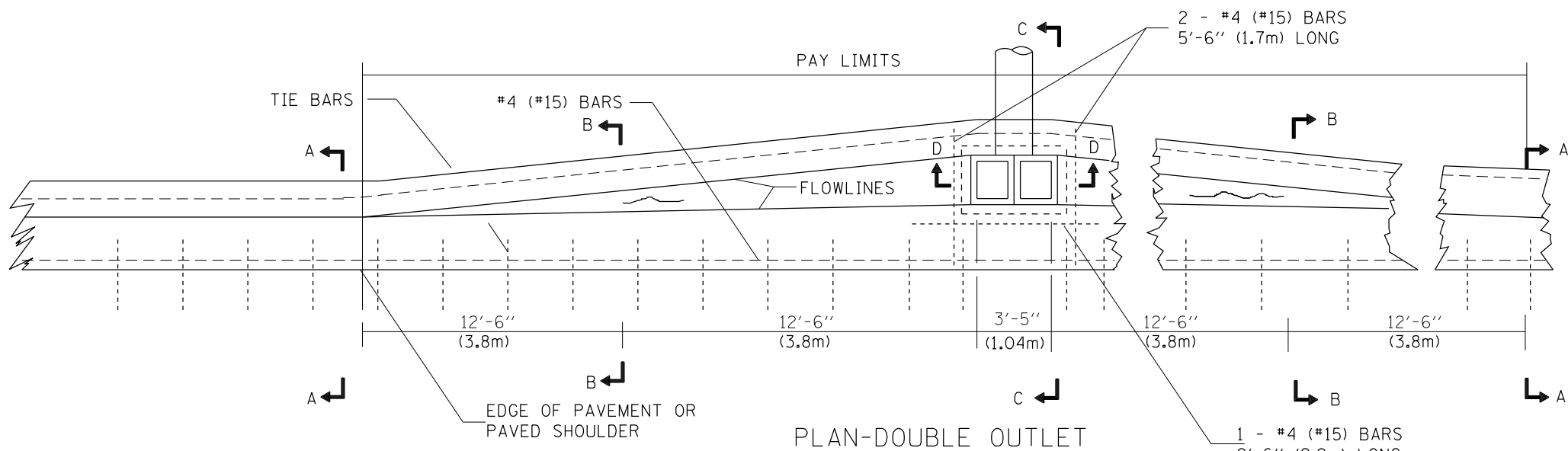
SHT. 3 OF 3
CADD STD. 606101-D4

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	486

CONTRACT NO. 68185

FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT

NOT TO SCALE



QUANTITIES

MATERIAL	SINGLE	DOUBLE
CONCRETE - (cu.yd.) m ³	()	()
CAST IRON GRATE - Ea. 125 lbs. (57 kg)	1	2
PIPE DRAIN - DIA. (in.) mm	15 (400)	18 (450)

GENERAL NOTES

The gutter outlet shall be tied to the pavement in accordance with details for longitudinal construction joint shown on Standard 420001.

Tie bars shall be No. 6x24 (No. 19x600) at 36" (900mm) centers unless otherwise shown.

If the average grade of pavement for the distance A-D exceeds 2%, this distance shall be increased 6' (1.8m) for each 1% increase in grade.

All dimensions are in inches (millimeters) unless otherwise noted.

DESIGNER NOTES:
1. INCLUDE STATE STANDARD 420001.
2. PAY ITEM IS CLASS SI CONCRETE (OUTLET), SPECIAL.

QUANTITIES

CALC. BY: _____ DATE: _____
 CHECKED BY: _____ DATE: _____

QUANTITY CALCULATIONS ARE ON FILE AT THE DISTRICT 4 OFFICE; BUREAU OF PROJECT IMPLEMENTATION; DOCUMENTATION SECTION

01-01-97	RENUM. A-1.07, NEW REVISION BOX, ADDED	T.P.	01-31-18	REVISED TIE BAR SIZE & SPACING	R.D.
	DESIGNER NOTES				
10-16-06	REVISED TO 2007 SPEC.	M.A.			
03-15-12	CHANGED MODIFIED TO SPECIAL	R.D.			

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**OUTLET TYPE 1 (SPECIAL) FOR
TYPE A GUTTER (SPECIAL)**

NOT TO SCALE

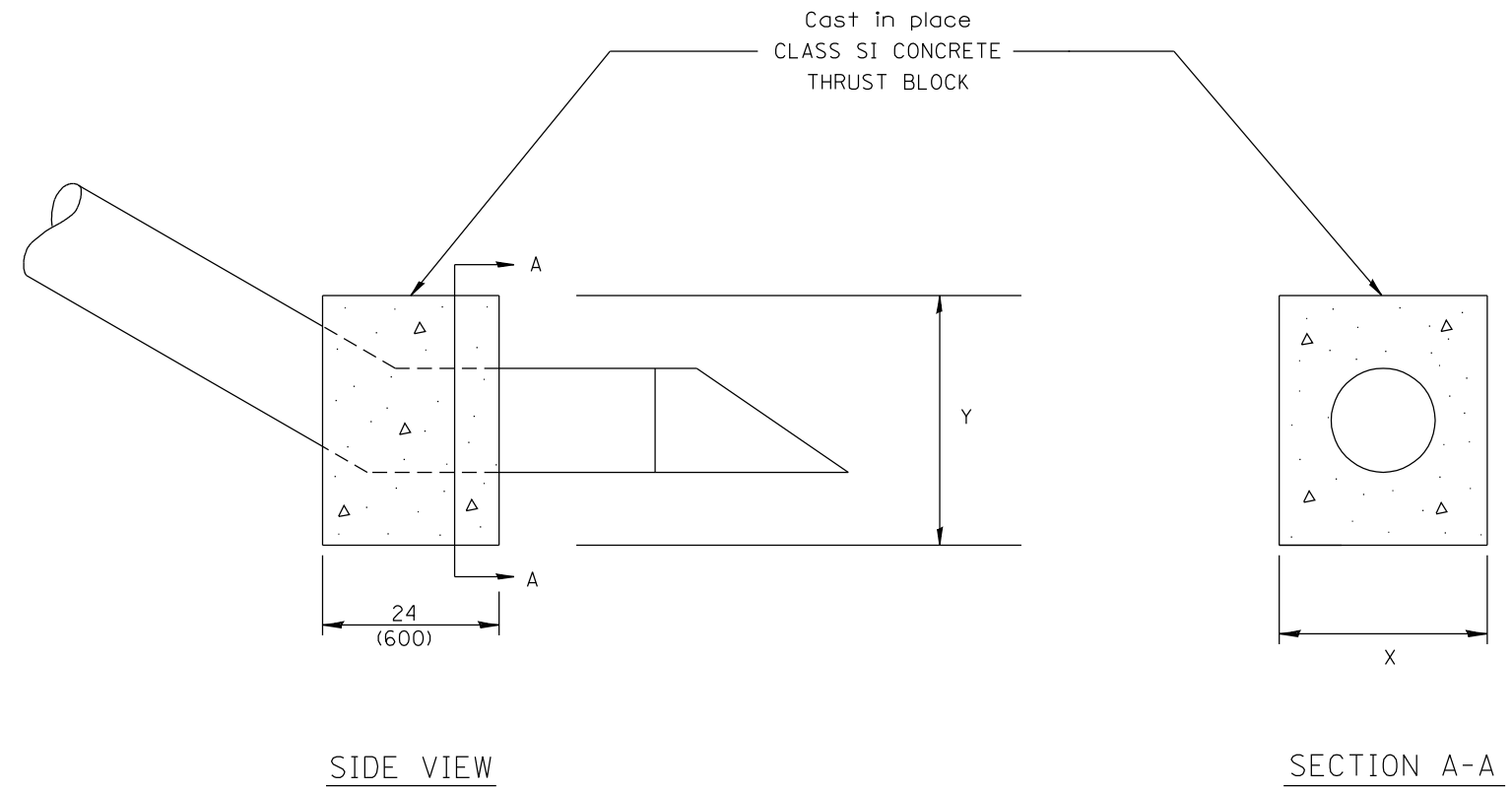
CADD STD. 606106-D4

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	487
CONTRACT NO. 68185				
FED. ROAD DIST. NO.	ILLINOIS	FED. AID PROJECT		

DESIGNER NOTES:
 1. A THRUST BLOCK SHALL BE INSTALLED WHEN THE DIFFERENCE BETWEEN THE UPSTREAM AND DOWNSTREAM INVERT ELEVATIONS EXCEEDS 36(900) AND GRADE EXCEEDS 10%.
 2. USE WITH DISTRICT CADD STANDARDS: "DITCH CHECK, IF NEEDED. DO NOT USE WITH DISTRICT CADD STANDARDS: "SLOPE DRAIN DETAILS FOR BURIED PIPE" "SLOPE DRAIN DETAILS FOR EXPOSED PIPE".

CONCRETE THRUST BLOCK BILL OF MATERIALS

PIPE SIZE	X	Y	CLASS SI CONCRETE cu. yd. (m ³)
12(300)	24(600)	24(600)	0.2(0.2)
15(375)	27(675)	27(675)	0.3(0.2)
18(450)	30(750)	30(750)	0.3(0.2)
24(600)	36(900)	36(900)	0.4(0.3)
30(750)	3'-6" (1.07m)	3'-6" (1.07m)	0.8(0.6)



The contract unit price each for CONCRETE THRUST BLOCK shall include the cost of excavation, CLASS SI CONCRETE and compacted backfill.

QUANTITIES	
CALC. BY: _____	DATE: _____
CHECKED BY: _____	DATE: _____
QUANTITY CALCULATIONS ARE ON FILE AT THE DISTRICT 4 OFFICE; BUREAU OF PROJECT IMPLEMENTATION; DOCUMENTATION SECTION	

01-01-97	RENUM. J-10.04, NEW REVISION BOX, ADDED QUANTITY	T.P.	
	CALCULATION BOX		
10-16-06	REVISED TO 2007 SPEC.	M.A.	

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

CONCRETE THRUST BLOCKS

NOT TO SCALE

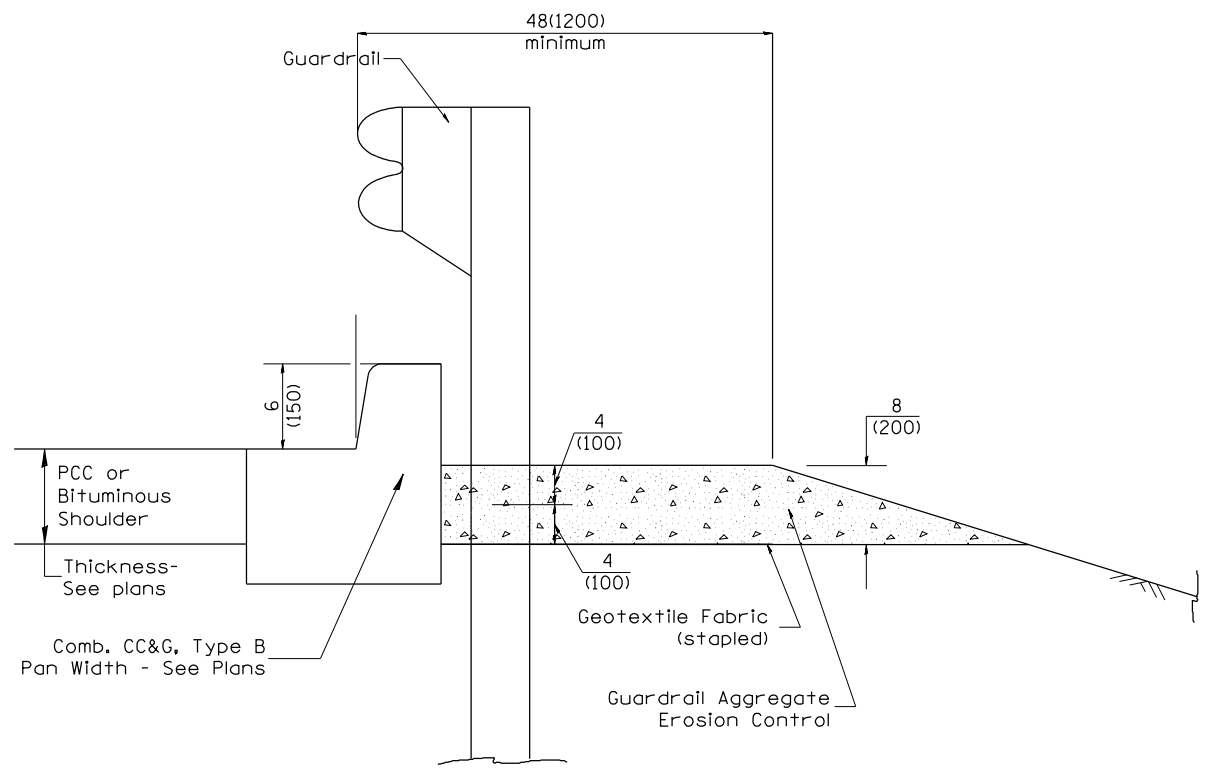
CADD STD. 609001-D4

All dimensions are in inches (millimeters) unless otherwise noted.

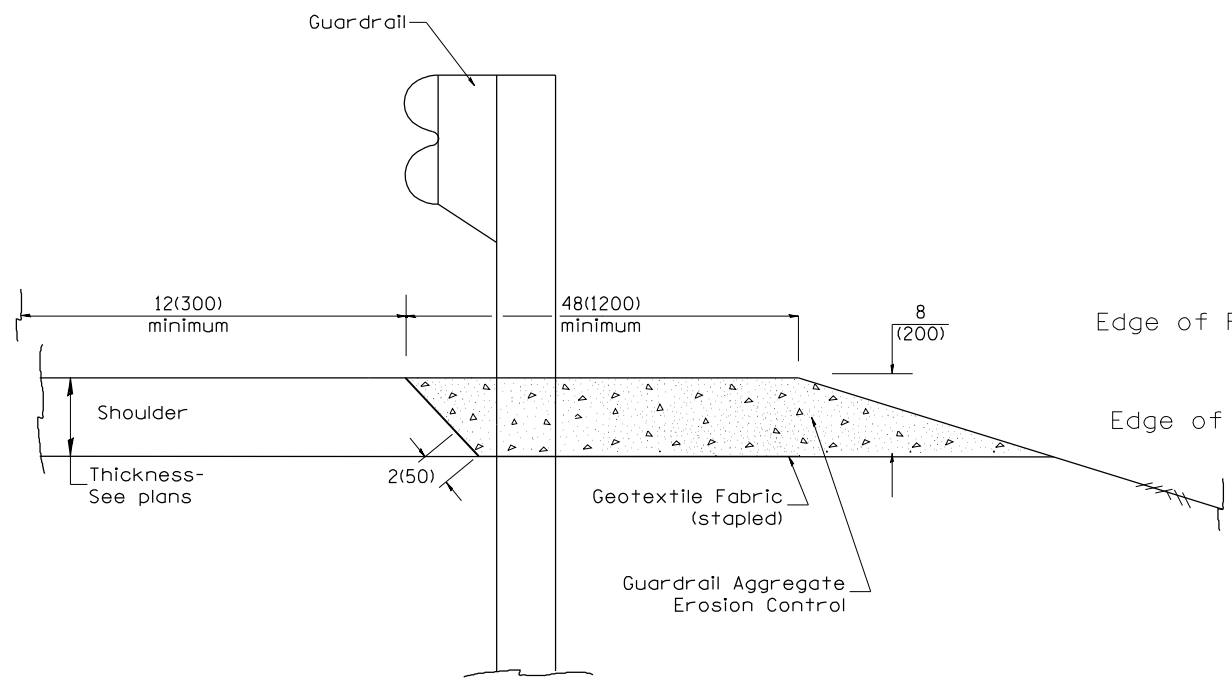
F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N,BR-1,RS-4,W-1)	PEORIA	577	488
CONTRACT NO. 68185				
FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT				

DESIGNER NOTES:

1. CONSIDER USING A "B" CURB PAY ITEM AT GUARDRAIL INSTALLATIONS WHERE GRADES ARE EQUAL TO OR GREATER THAN 1% AND AT INLETS. (INCLUDE DISTRICT SPECIAL PROVISION)
2. USE "GUARDRAIL AGGREGATE EROSION CONTROL" AT GUARDRAIL INSTALLATIONS WHERE GRADES ARE LESS THAN 1% (INCLUDE DISTRICT SPECIAL PROVISION).
3. INCLUDE STATE STANDARD 610001, IF APPLICABLE.
4. INCLUDE THE FOLLOWING DISTRICT CADD STANDARDS AS NEEDED: SLOPE DRAINS FOR EXPOSED PIPES; SLOPE DRAINS FOR BURIED PIPES; SEE PAGE COLLARS FOR BURIED PIPES
5. SEE PAGE COLLARS FOR EXPOSED PIPES; CONCRETE THRUST BLOCKS AND PIPE ELBOW.
6. INCLUDE DISTRICT SPECIAL PROVISION - "AGGREGATE QUALITY" FOR PROJECTS LOCATED IN THE WESTERN AREA OF THE DISTRICT - APPROX. DIVIDING LINE IS IL 97.
7. DELETE DESIGNER NOTES WHEN INSERTING INTO PLAN FILES.
8. OPERATIONS PREFERS USE OF PIPE OUTLETTING ONTO FORESLOPE WITH RIPRAP. USE NON-METALLIC PIPE WHEN POSSIBLE BECAUSE OF FUTURE CORROSION ISSUES. IF NO OTHER SEEDING IS PAID FOR ON THE CONTRACT, USE DISTRICT SPECIAL PROVISION FOR SEEDING, MINOR AREAS.



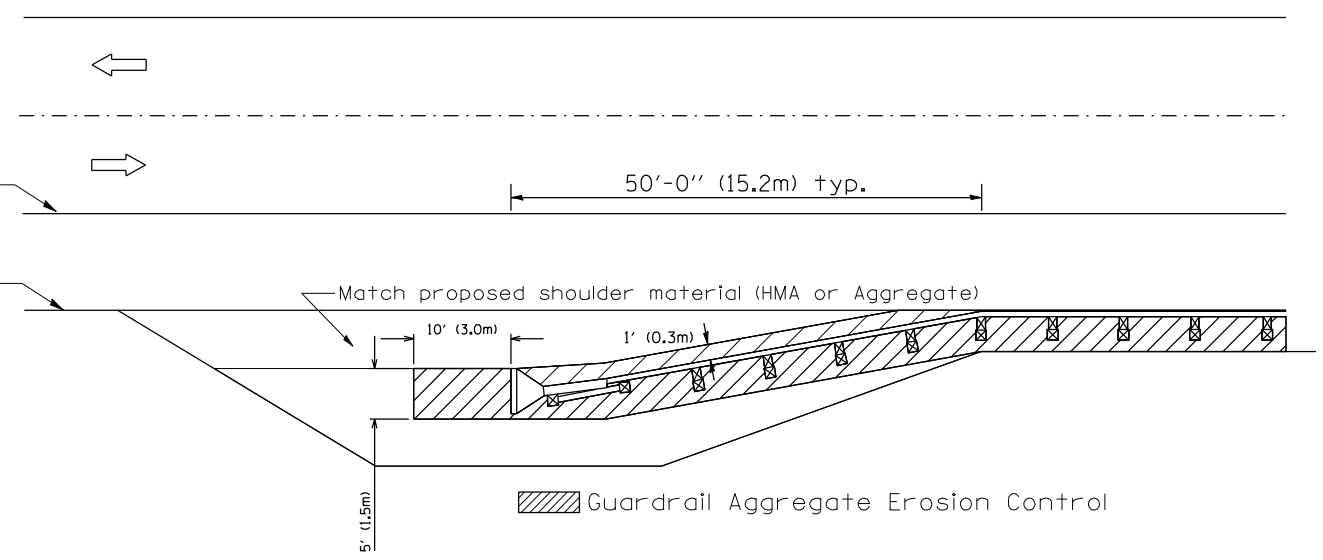
TYPICAL SECTION WITH COMBINATION CONCRETE CURB & GUTTER



TYPICAL SECTION WITHOUT EROSION CONTROL CURB

GENERAL NOTES: GUARDRAIL AGGREGATE EROSION CONTROL

1. This work shall consist of grading as needed, furnishing and installing geotextile fabric and staples, and furnishing, placing and shaping crushed aggregate around and behind Steel Plate Beam Guardrail posts in accordance with Plan Details.
2. Before placing the aggregate and the Geotextile Fabric, weeds and grass shall be removed from the area to be covered.
3. After the area has been prepared, and in a dry condition, the Geotextile fabric shall be placed with a 12(300) minimum overlap. A knife cut for guardrail post installation is necessary.
4. The aggregate shall be deposited, compacted and shaped by either mechanical or hand methods, in a manner reasonably true to line and grade.
5. The Contractor shall have the option of placing the guardrail before or after the Geotextile Fabric and Aggregate are in place. If the guardrail is placed after the Geotextile Fabric and Aggregate, then any voids must be filled and the aggregate returned to line and grade.
6. Materials shall meet the following requirements:
 - A. The crushed aggregate shall be CA1 gradation in accordance with Article 1004.01(c) of the Standard Specifications.
 - B. The Geotextile Fabric shall be nonwoven fabric in accordance with Article 1080.02 of the Standard Specifications.



All dimensions are in inches (millimeters) unless otherwise noted.

03-07-11	ADDED DETAIL SHOWING PLAN VIEW	R.D.	5-30-18	CHANGE B CURB TO CC&G	R.D.
08-10-12	REVISED CURB "B" AND AGGREGATE	R.D.			
07-15-15	ADDRESSED SHOULDER INLET CURB	R.D.			
01-26-17	REVISED	R.D.			

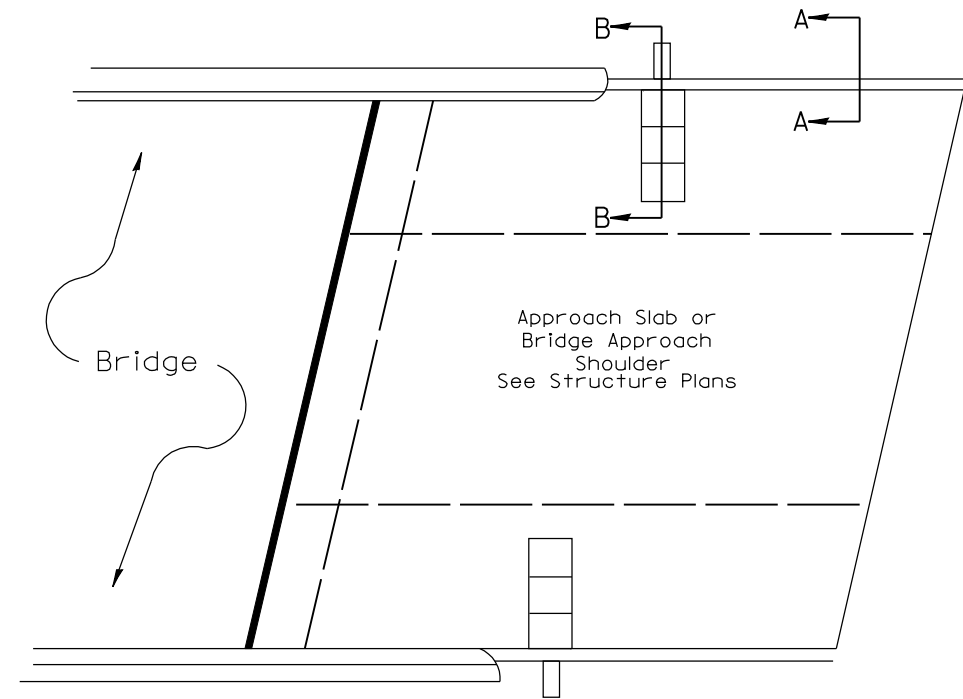
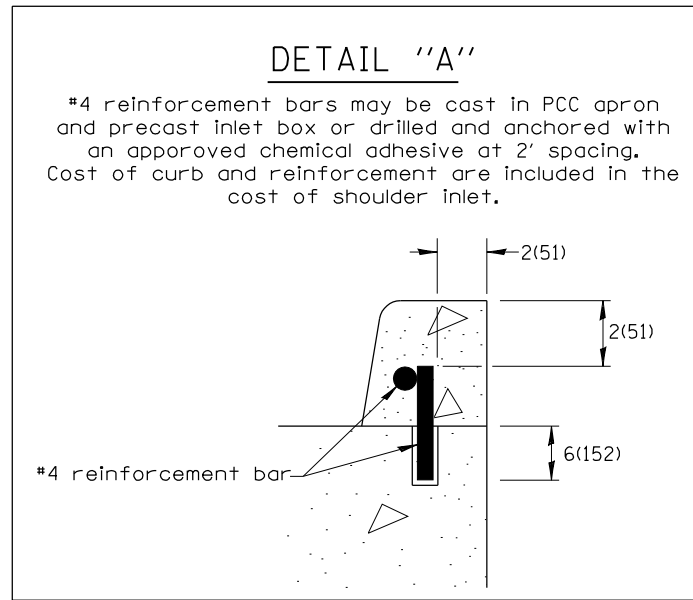
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

GUARDRAIL EROSION CONTROL TREATMENTS

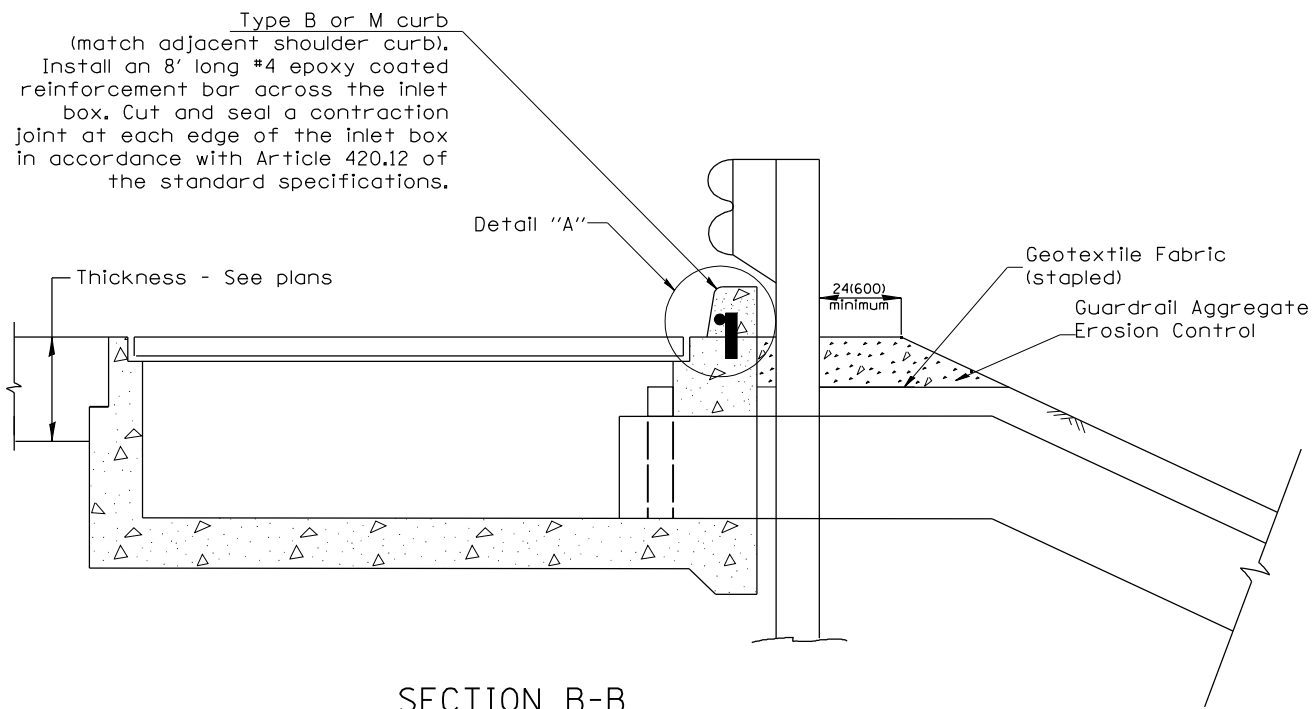
NOT TO SCALE

SHT. 1 OF 2
CADD STD. 630101-D4

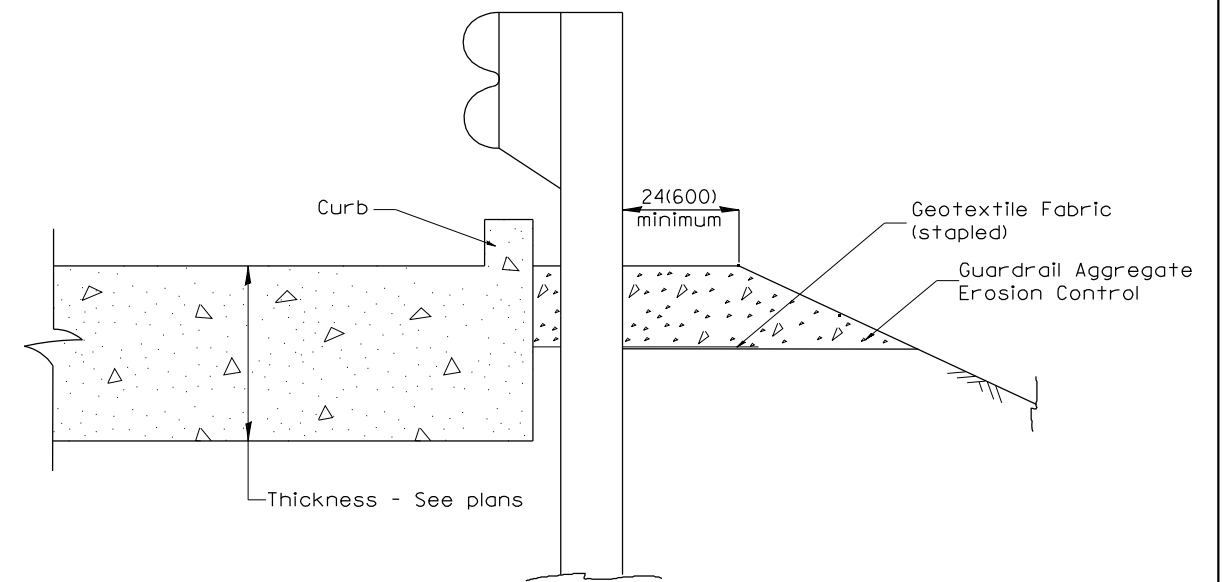
F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N,BR-1,RS-4,W-1)	PEORIA	577	489
CONTRACT NO. 68185				
FED. ROAD DIST. NO.		ILLINOIS FED. AID PROJECT		



PLAN VIEW
APPROACH SLAB OR SHOULDER PLACEMENT



SECTION B-B
TYPICAL SECTION AT INLETS
TYPE E, F & G (HIGHWAY STANDARD 610001)



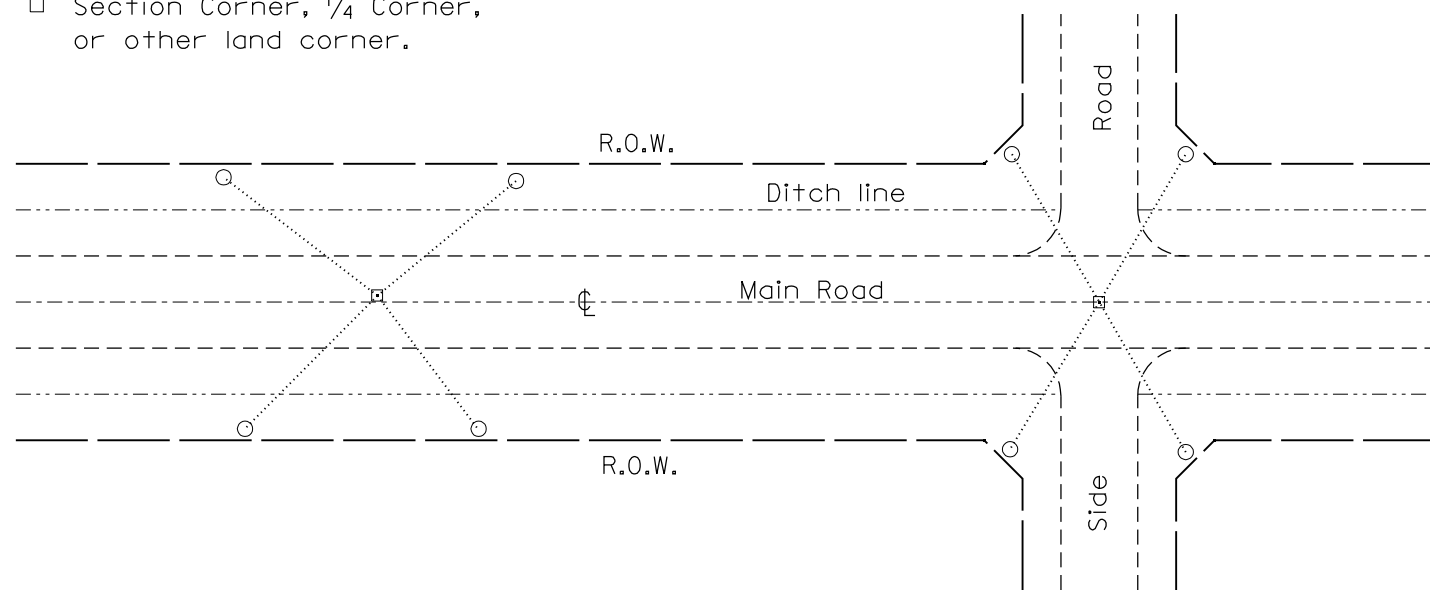
SECTION A-A
TYPICAL SECTION WITH BRIDGE APPROACH CURB

All dimensions are in inches (millimeters) unless otherwise noted.

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION				GUARDRAIL EROSION CONTROL TREATMENTS				F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
				NOT TO SCALE				6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	490
				SHT. 2 OF 2 CADD STD. 630101-D4				CONTRACT NO. 68185				
								FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT				

PERMANENT SURVEY TIES

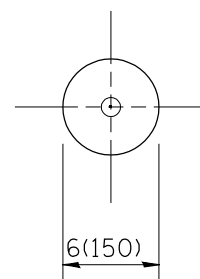
- Permanent Survey Tie
- Section Corner, 1/4 Corner, or other land corner.



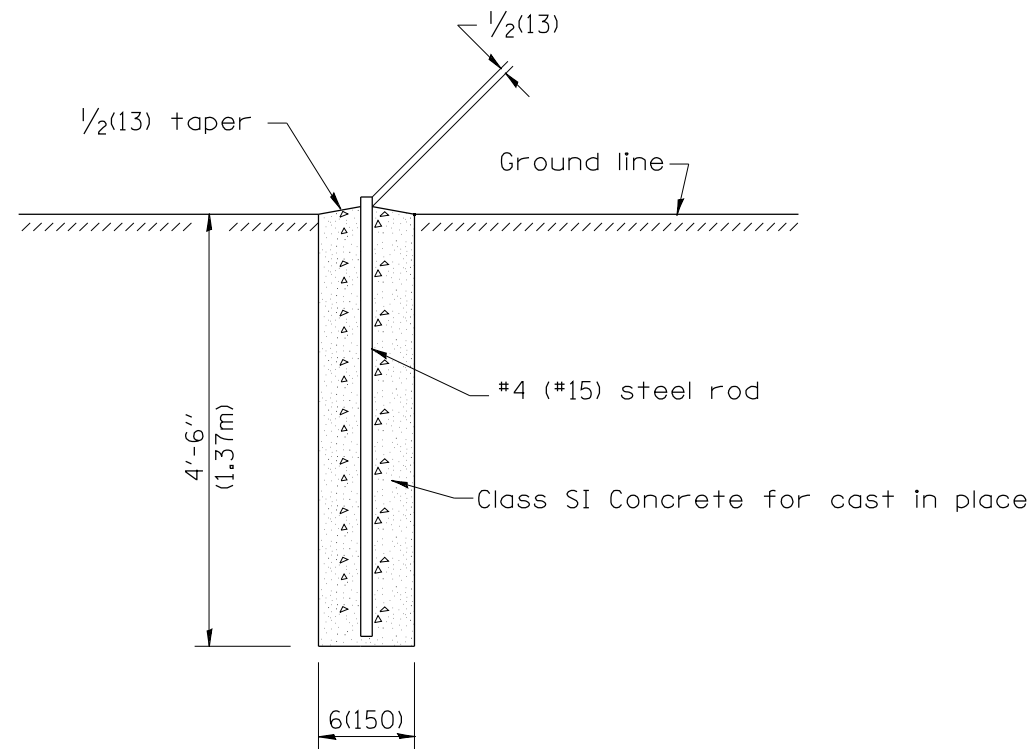
TYPICAL APPLICATION

GENERAL NOTES

1. The marker shall be cast in place of Class SI Concrete.
2. Tie marker shall be installed after the final seeding has been completed unless otherwise specified by the Engineer.
3. The tie distances to the section corner shall be measured and recorded by the surveyor setting the PSM. All ties shall be turned over to the IDOT Chief of Surveys or Chief of Plats for recordation.
4. All documentation shall be performed by a PLS

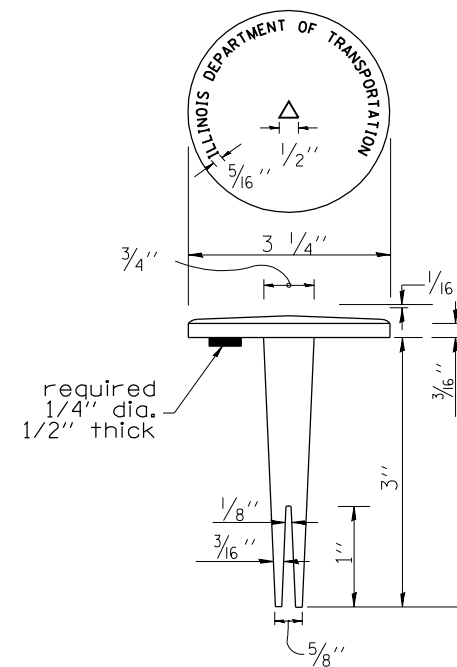


PLAN

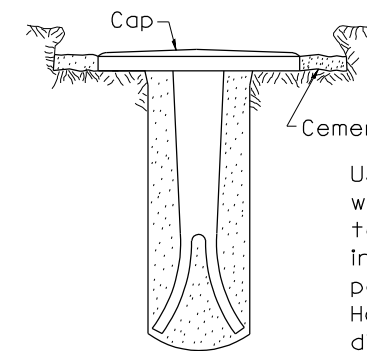


SECTION

PERMANENT SURVEY MARKERS



BRASS TABLET

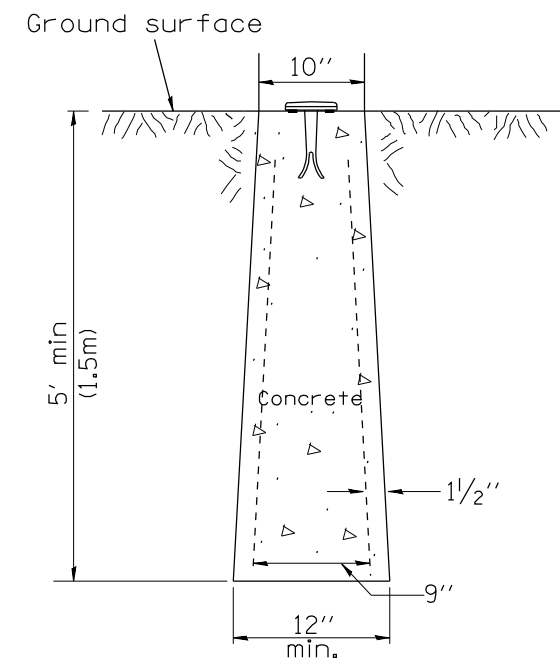


Tablet constructed in rock ledge or concrete.

TYPE I

GENERAL NOTES

1. All type II markers shall be cast in place, and precast markers will not be allowed.
2. Two permanent magnets, each having a diameter of 3/4 (19) and a thickness of 1/4 (6), or equivalent, shall be attached to the underside of the tablet with an approved epoxy bonding agent.
3. The location of the markers shall be in accordance with the plans in general, the markers will be placed at the P.T.'s, P.C.'s, and P.I.'s located within the R.O.W. of horizontal curves and spaces along the tangents in a way that a minimum of two markers are always inter-visible, and not to exceed 1000' (300m).
4. The markers shall be placed under the direction of the Engineer and shall be installed in a workmanlike manner in order that there will be no further settlement or horizontal shifting. The monuments shall be placed in a way that the survey point will fall within the portion of the plaque provided for that purpose.
5. The project designation, the centerline station, the survey point, and the elevation shall be permanently marked by the use of metal dies after marker has been installed.



**TYPE II
CAST-IN-PLACE MARKER**

All dimensions are in inches (millimeters) unless otherwise noted.

DESIGNER NOTES:
 1. ADD DISTRICT SPECIAL PROVISION IF PLACING A TYPE I MARKER ON A STRUCTURE.
 2. MODIFIES STATE STD 667101. DON'T USE STATE STD IF USING CADD STANDARD
 3. PERMANENT SURVEY MARKERS SHALL BE PLACED TO PERPETUATE THE SURVEY LINES OF DIVIDED HIGHWAYS AND THE CENTERLINE OF ALL OTHERS WHERE THESE LINES HAVE BEEN ESTABLISHED BY SURVEY.
 4. PERMANENT SURVEY MARKERS SHALL BE PLACED AT ALL LAND SECTION CORNERS WITHIN THE STATE R.O.W. WHERE THE MONUMENTS HAVE BEEN FOUND OR RELOCATED BY SURVEY.

01-01-97	RENUM. D-3.01, NEW REVISION BOX, REVISED	T.P.	10-16-06	REVISED TO 2007 SPEC.	M.A.
	TITLE BOX, ADD DESIGNER NOTE		01-04-11	REVISED FOR CORRECTIONS	R.D.
07-07-98	ADD DESIGNER NOTE	J.A.	08-21-13	CHANGED MIN. DIAMETER	R.D.
05-24-06	REMOVED GEN. NOTE UNDER TIES	M.A.	08-25-15	REVISED MATERIAL	R.D.

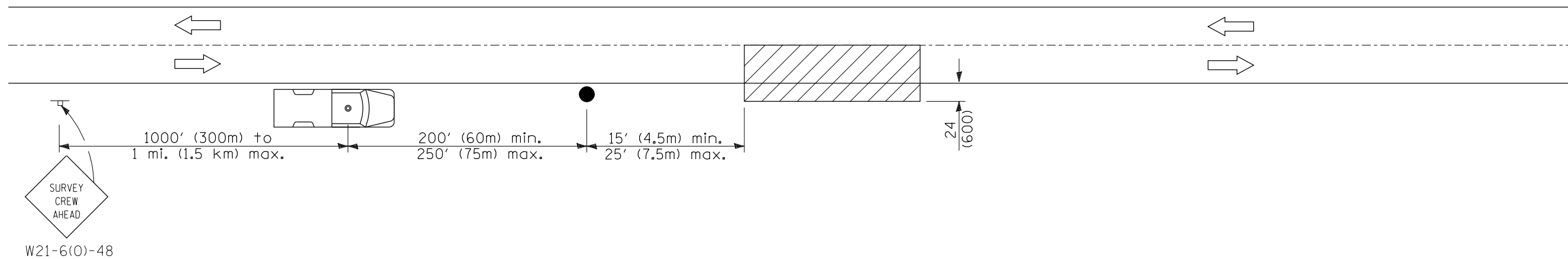
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**PERMANENT SURVEY TIE &
PERMANENT SURVEY MARKERS TY.I - TY.II**

NOT TO SCALE


CADD STD. 667101-D4

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	491
CONTRACT NO. 68185				
FED. ROAD DIST. NO.		ILLINOIS FED. AID PROJECT		



SYMBOLS

 Work area

 Sign on portable or permanent support

 Truck with flashing amber light and dual emergency flashers

 Flagger with traffic control sign

TYPICAL APPLICATIONS
Utility operations

All dimensions are in inches (millimeters) unless otherwise noted.

01-01-97	RENUM. E-3.04, METRICS, NEW REVISION BOX, REVISED	T.P.
	TITLE BOX	
10-16-06	REVISED TO 2007 SPEC.	M.A.

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

NIGHTTIME LIGHTING INSPECTION

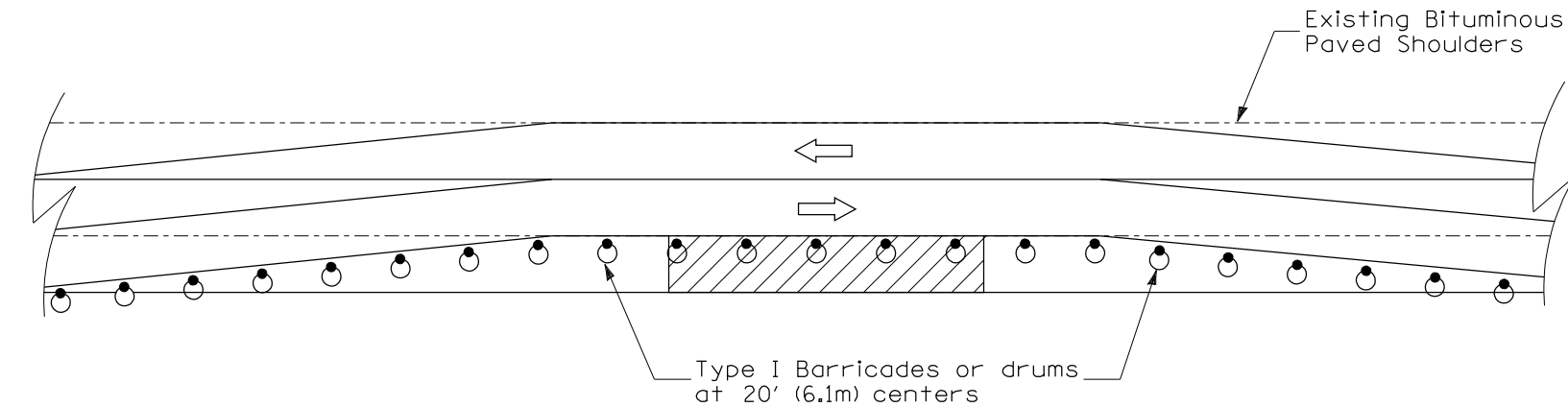
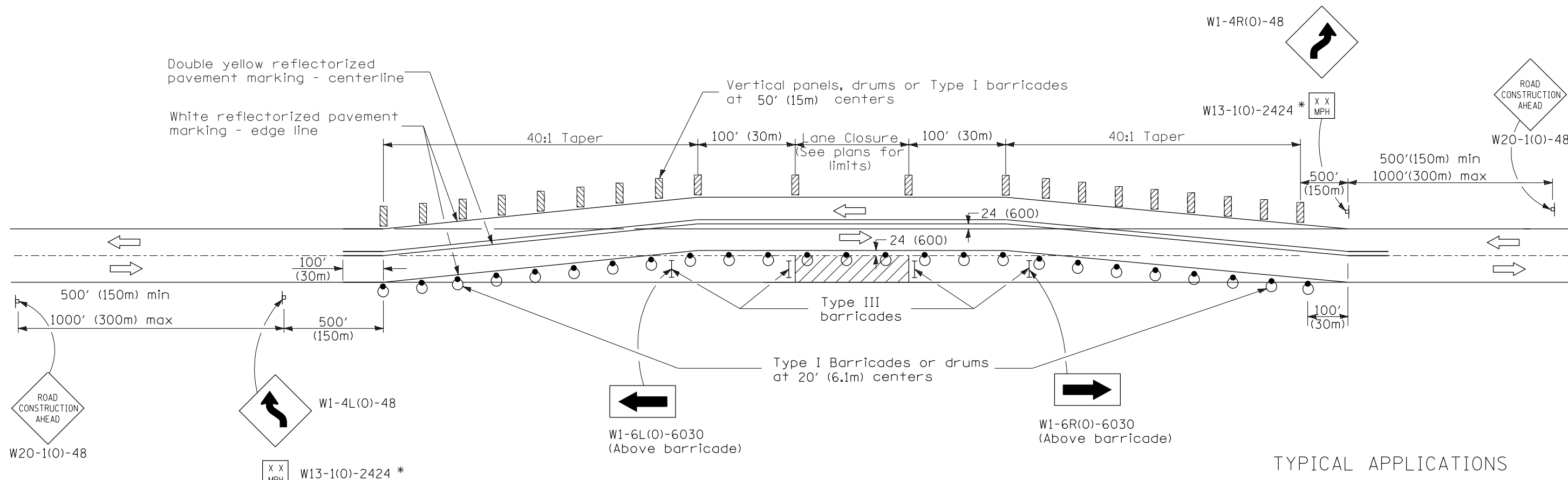
NOT TO SCALE

CADD STD. 701301-D4

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N,BR-1,RS-4,W-1)	PEORIA	577	492
FED. ROAD DIST. NO.			ILLINOIS FED. AID PROJECT	
			CONTRACT NO. 68185	

DESIGNER NOTES:

1. Include State Standard 701331.
2. Review treatments with District Traffic Control Engineer (Technician).
3. Include District Special Provision.



SYMBOLS

- Work area
- Sign
- Barricade or drum with monodirectional steady burning light
- Vertical panel, drum or Type I barricade
- Type III barricade

TYPICAL APPLICATIONS

1. Connection of relocated pavements to existing pavements.
2. Emergency pavement repairs.

GENERAL NOTES

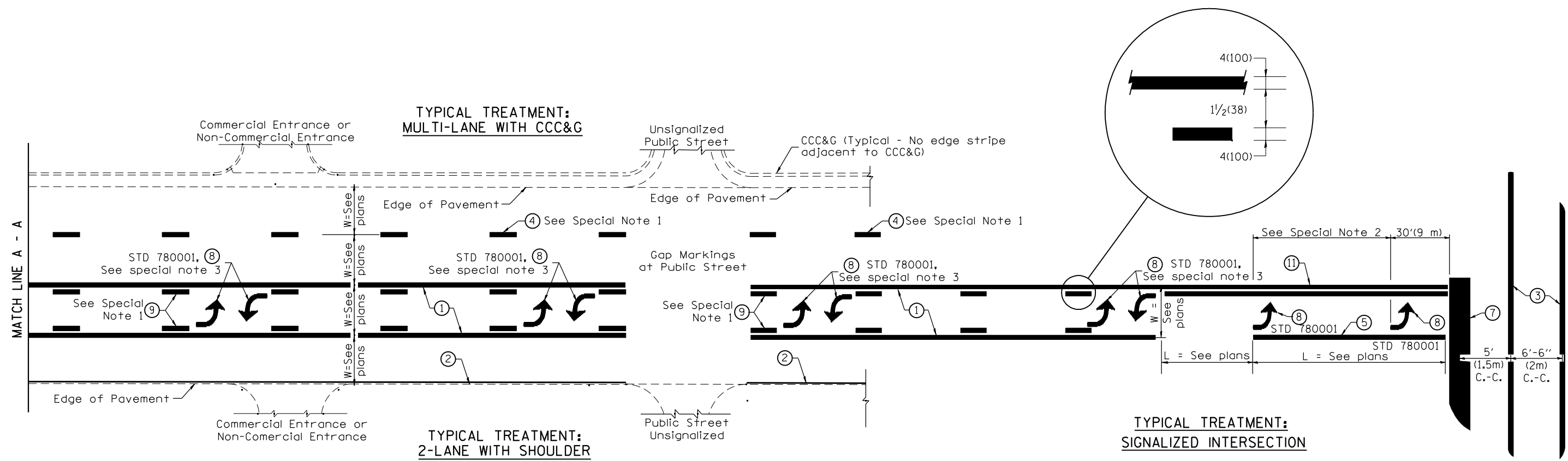
1. This Standard is used where at any time, any vehicle, equipment, workers or their activities require the closure of a single lane and a temporary run-around is constructed adjacent to the existing pavement.
- *2. The advisory speed to be shown below the reverse curve (turn) signs shall be determined at the site and approved by the Engineer.
3. Type III reflectorized pavement marking tape shall be used for marking the edge lines and centerline on the existing pavement. Type III reflectorized pavement marking tape shall also be used for markings on the paved run-arounds. Type III reflectorized pavement marking tape will be paid for in accordance with Article 703.07 of the Standard Specifications

All dimensions are in inches (millimeters) unless otherwise noted.

01-01-97	RENUM. F-6.01, NEW REVISION BOX, REVISED	T.P.				STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	LANE CLOSURE, 2L, 2W WITH RUNAROUND ADJACENT TO EXISTING PAVEMENT FOR SPEEDS ≥ 45MPH (STANDARD 701331, SPECIAL)	F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
10-16-06	REVISED TO 2007 SPEC.	M.A.						6659	11(N,BR-1,RS-4,W-1)	PEORIA	577	493	
6-20-12	Minor Updates	R.D.											CONTRACT NO. 68185
											FED. ROAD DIST. NO.	ILLINOIS FED. AID PROJECT	

NOT TO SCALE

CADD STD. 701331-D4



TYPICAL PAVEMENT MARKING LEGEND

(Note: This is a District Standard Legend. Some elements may not apply to specific project.)

- ① 4(100) Solid (Yellow)
- ② 4(100) Solid (White)
- ③ 2-6(150) Crosswalk @ 6'-6" (2m)min C.-C. (White)
2-8(200) Crosswalk @ 6'-6" (2m)min C.-C. (White) (When traffic signals are present.)
- ④ 6(150) Skip-Dash (White) (See Special Note 1)
- ⑤ 8(200) Solid (White)
- ⑥ 12(300) Diagonal (White) (Item ⑥ is shown on Std. 780001)
- ⑦ 24(600) Stop Bar (White)
- ⑧ Letters & Arrows (See Std. 780001 and Special Notes 2 & 3)
- ⑨ 4(100) Skip-Dash (Yellow) (See Special Note 1)
- ⑩ 12(300) Diagonal (Yellow) (See Table A) (See Table A)
- ⑪ 4(100) Double Solid (Yellow) (See Table A)

SPECIAL NOTES

1. Skip-Dash markings will be centered between both ends of city blocks and shall be placed in alignment transversely across the pavement.
2. The following shall apply to arrows located in one-way left turn lanes:
 - A. A minimum of two (2) arrows is required.
 - B. The maximum spacing between arrows is 80' (24 m).
 - C. Arrows shall be evenly spaced if three (3) or more are required.
3. The following shall apply to arrow pairs located in two-way left turn lanes:
 - A. A minimum of two (2) arrow pairs is required.
 - B. The maximum spacing between arrow pairs is 200' (61 m).
 - C. Arrow pairs shall be evenly spaced if three (3) or more are required.
 - D. The spacing between Bi Directional Left Turn Arrows is 33' (10 m).

GENERAL NOTES

1. Refer to State Standard 780001 for additional Pavement Markings including letters & arrows.
2. See Plans for Pavement Markings adjacent to curbed islands and medians, and through lane reductions.
3. Refer to Article 780.13 for letter, number and symbol areas (sq. ft.)
4. Areas are grooved 1" beyond each edge for the following symbols:
 - Through Arrow= 14.8 sq. ft.
 - Large Left or Right Arrow= 21.9 sq. ft.
 - 2 Arrow Combination Left (or Right) and Through= 34.9 sq. ft.
 - Wrong Way Arrow= 29.5 sq. ft.
 - Railroad Crossing Symbol= 69.8 sq. ft.
 (For further information, refer to BDE Special Provision: Grooving for Recessed Pavement Markings)

DESIGNER NOTES:
1. Include State Standard 780001 (Typical Pavement Markings)

01-01-97	RENUM. F-8.03, NEW REVISION BOX	T.P.	10-16-06	REVISED TO 2007 SPEC.	
02-07-97	ADD BI DIRECTIONAL DIMENSION	J.A.	2/29/16	ADDED GROOVING AREAS	R.D.
10-97	CORRECT BI DIRECTIONAL DIMENSION	J.A.			
08-02	ADD CROSSWALK DMNS. WITH T.S.	M.A.			

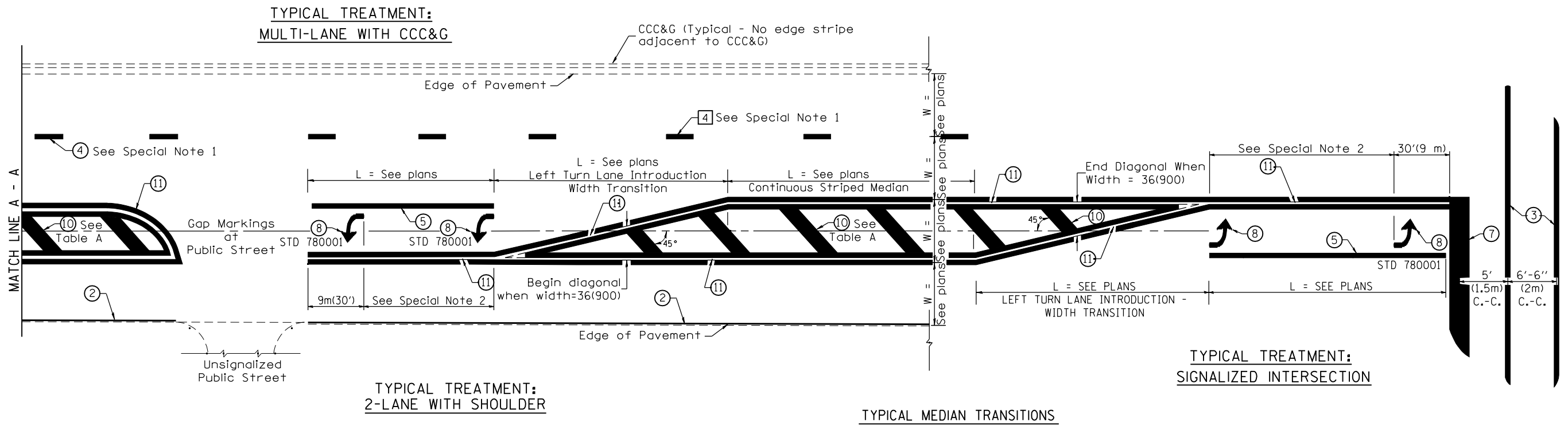
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

NOT TO SCALE

TYPICAL PAVEMENT MARKINGS

SHT. 1 OF 2
CADD STD. 780001-D4

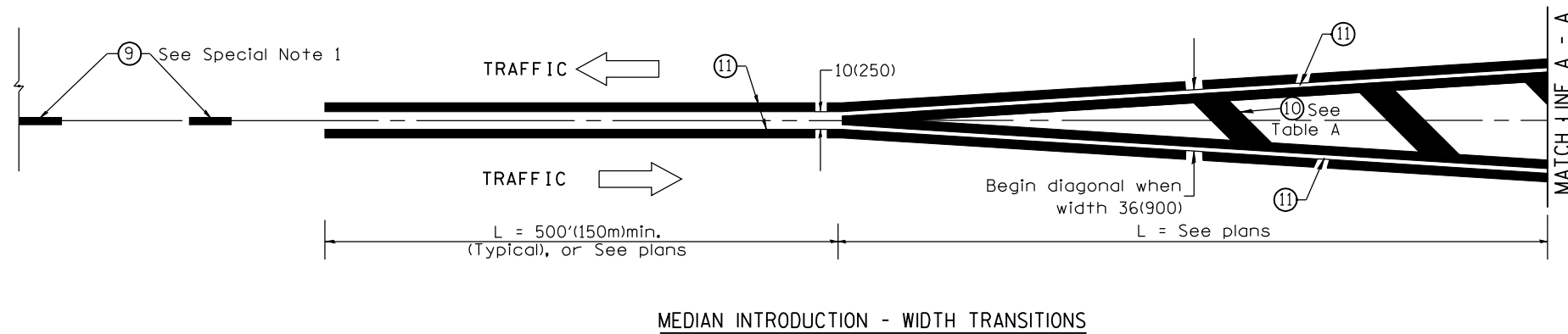
F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	494
FED. ROAD DIST. NO. ILLINOIS FED. AID PROJECT			CONTRACT NO. 68185	



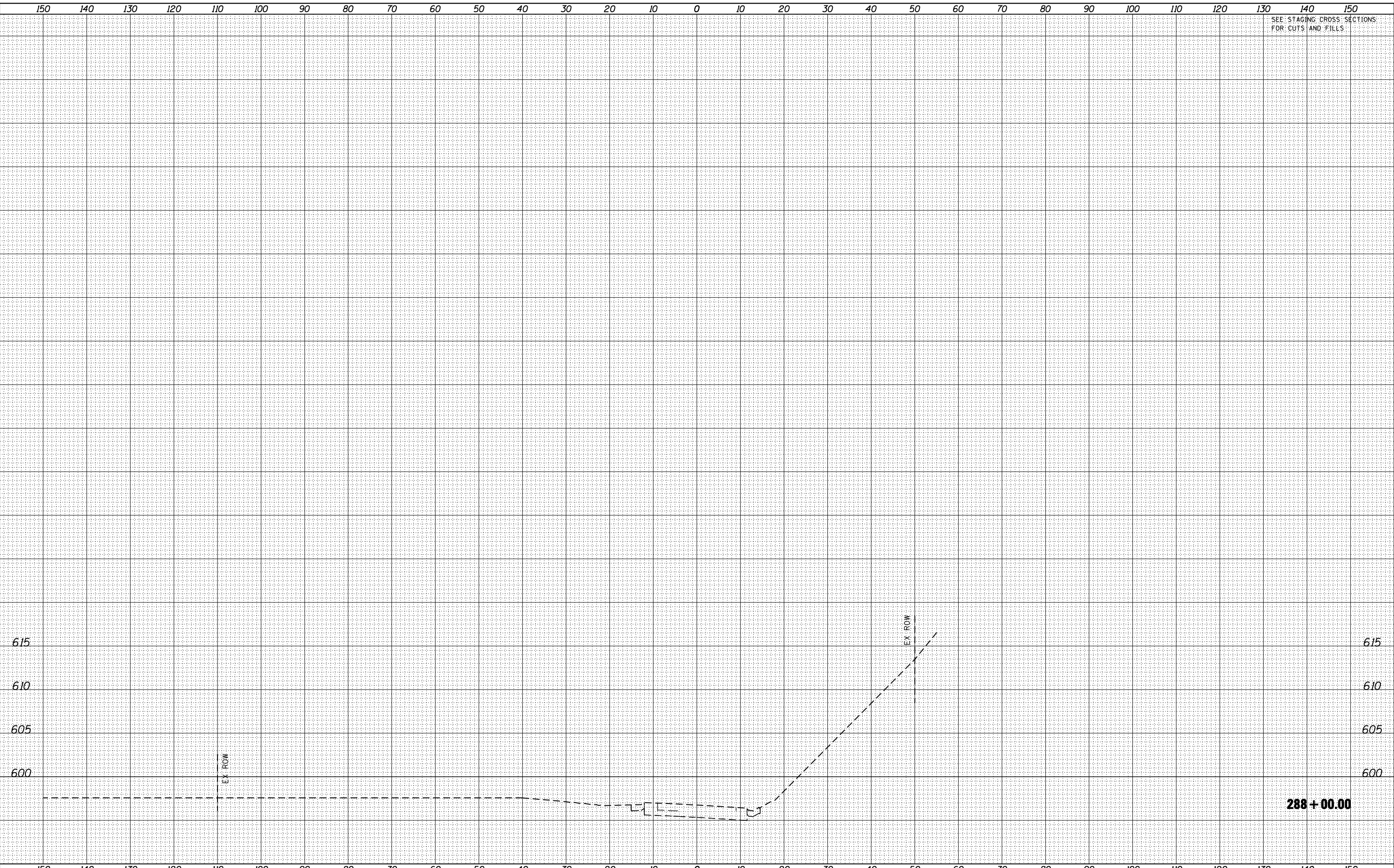
FLUSH PAVED MEDIAN: RESTRICTED LEFT TURN LANE

TABLE A
RECOMMENDED SPACING BETWEEN DIAGONAL LINES

SPEED LIMIT RANGE	INTERSECTION CHANNELIZATION (Includes Width Transitions for Median and Left Turn Lane Introductions)	
	CONTINUOUS	
Less Than 30 mph (50 km/h)	50' (15m)	15' (5m)
30 - 45 mph (50 - 70 km/h)	75' (23m)	20' (6m)
Over 45 mph (70 km/h)	150' (46m)	30' (9m)



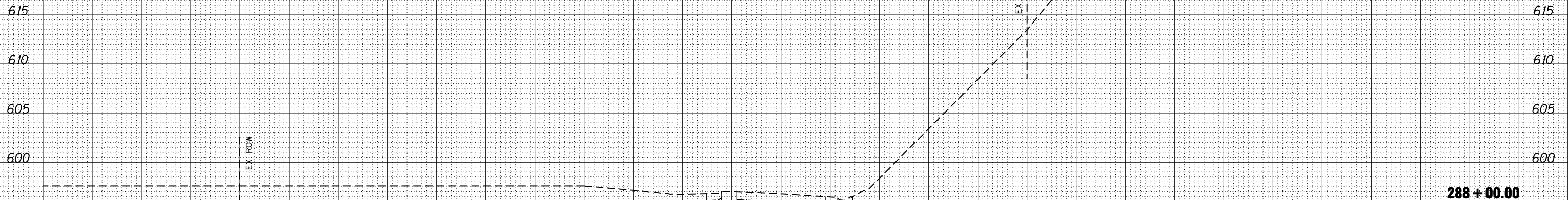
All dimensions are in inches (millimeters) unless otherwise noted.



DATE		
BY		
FINL SURVEY NO.	SURVEYED PLOTTED AREAS CHECKED	TEMPLATE AREAS CHECKED
NOTE BOOK AREAS CHECKED		

DATE		
BY		
ORIGI SURVEY NO.	SURVEYED PLOTTED AREAS CHECKED	TEMPLATE AREAS CHECKED
NOTE BOOK AREAS CHECKED		

SEE STAGING CROSS SECTIONS FOR CUTS AND FILLS



design firm
no. 184001036



USER NAME = g_jameson	DESIGNED - JAC	REVISED
PLOT SCALE = 20.0000' / IN.	CHECKED - RKA/CWC	REVISED
PLOT DATE = 8/14/2018	DRAWN - GSJ	REVISED
	CHECKED - CWC/RKA	REVISED

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**PROPOSED CROSS SECTIONS - FARMINGTON ROAD
FARMINGTON ROAD IMPROVEMENT**

SCALE: SHEET NO. 1 OF 46 SHEETS STA. 288+00.00 TO STA. 288+00.00

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	496
CONTRACT NO. 68185				

ILLINOIS FED. AID PROJECT

FINAL SURVEY NO.	SURVEYED AREAS CHECKED
SURVEY NO.	PLOTTED AREAS CHECKED
DATE	

ORIGINAL SURVEY NO.	SURVEYED AREAS CHECKED
SURVEY NO.	PLOTTED AREAS CHECKED
DATE	

FILE NAME = D:\68185-SHT-5SEC-1\FARM1.dgn



USER NAME = gjameson
PLOT SCALE = 20.0000' / IN.
PLOT DATE = 8/14/2018

DESIGNED - JAC	REVISED
CHECKED - RKA/CWC	REVISED
DRAWN - GSJ	REVISED
CHECKED - CWC/RKA	REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

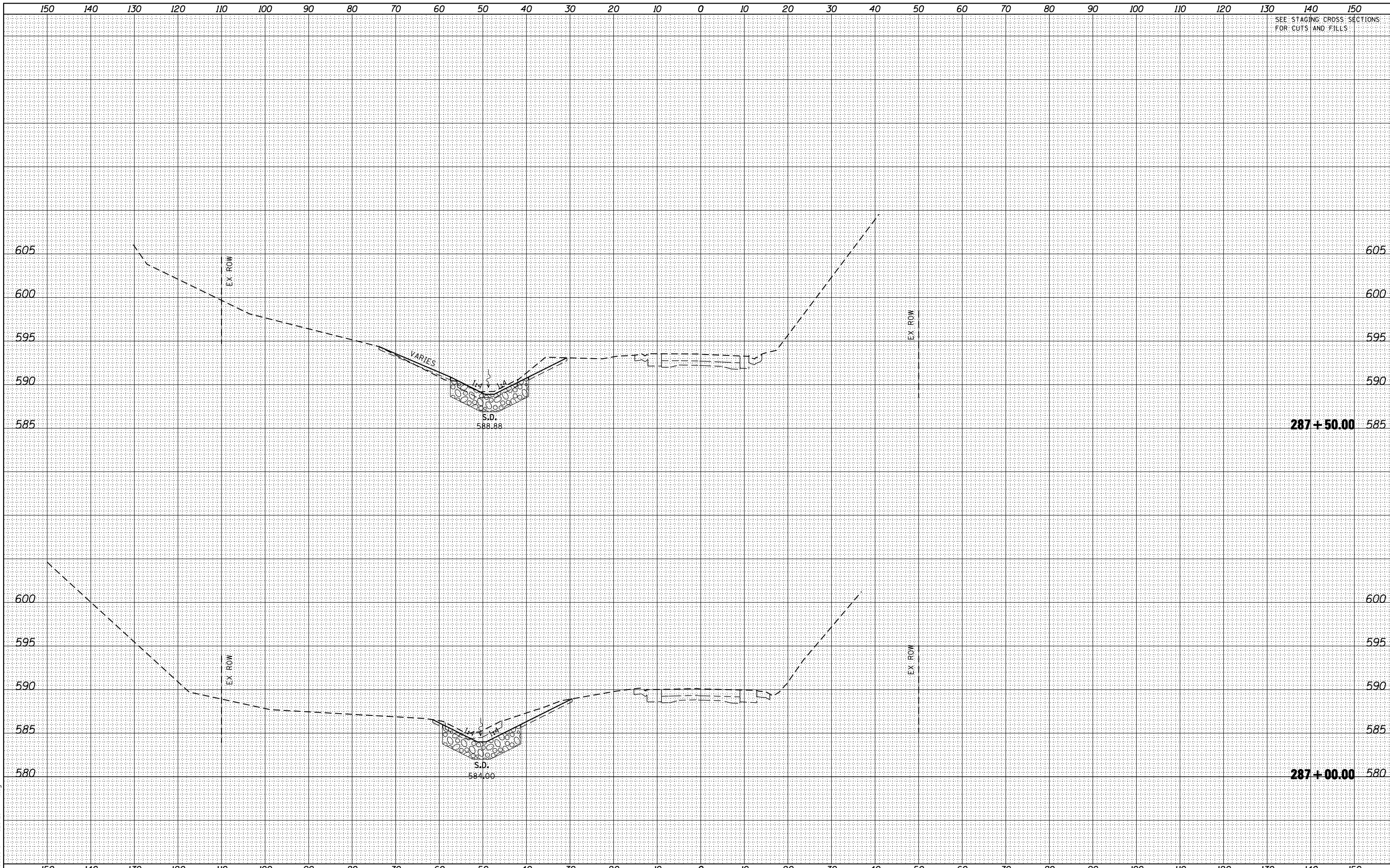
PROPOSED CROSS SECTIONS - FARMINGTON ROAD
FARMINGTON ROAD IMPROVEMENT

SCALE: SHEET NO. 2 OF 46 SHEETS STA. 287+00.00 TO STA. 287+50.00

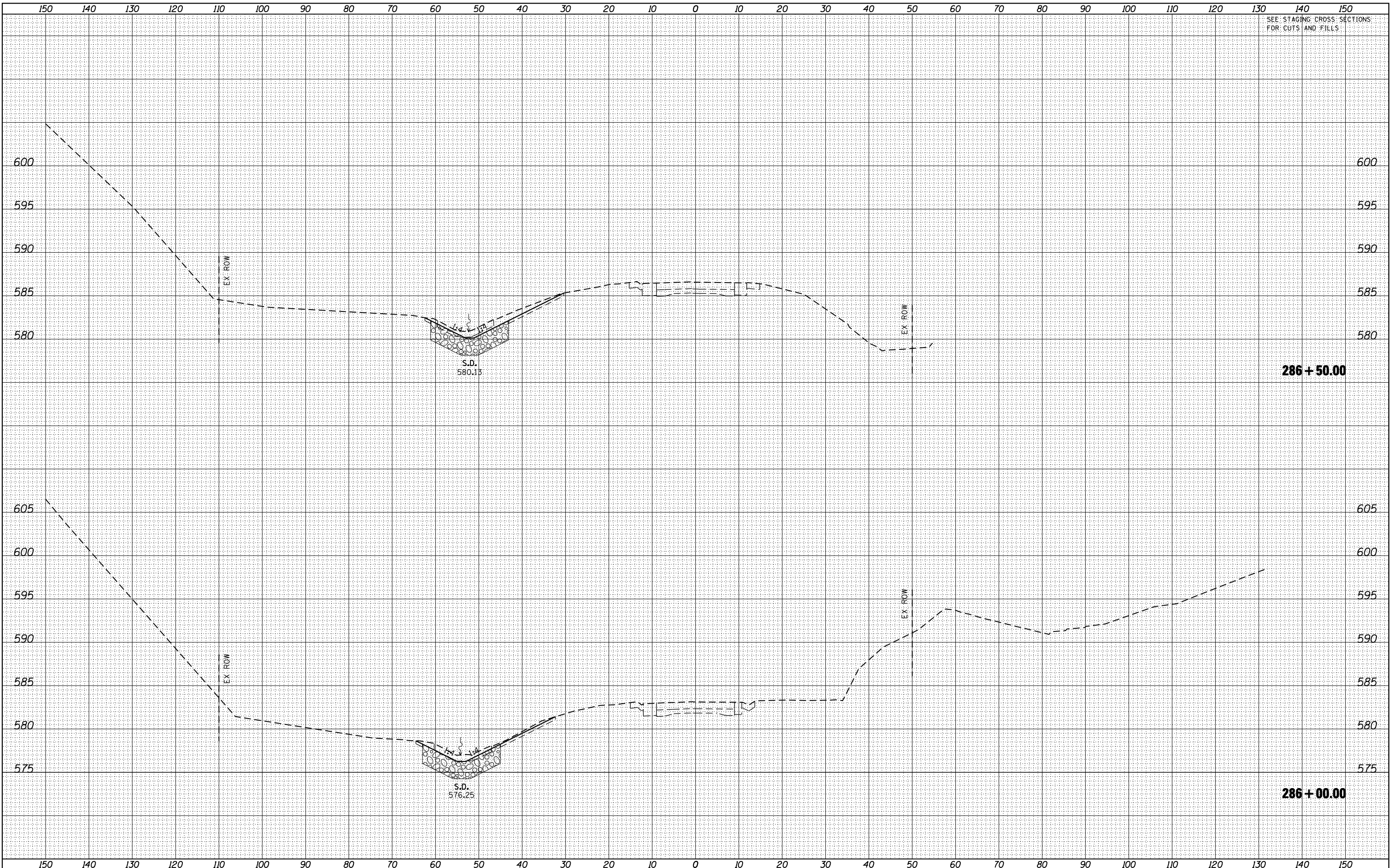
F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N, BR-1, RS-4, W-1)	PEORIA	577	497

CONTRACT NO. 68185

ILLINOIS FED. AID PROJECT



SEE STAGING CROSS SECTIONS FOR CUTS AND FILLS



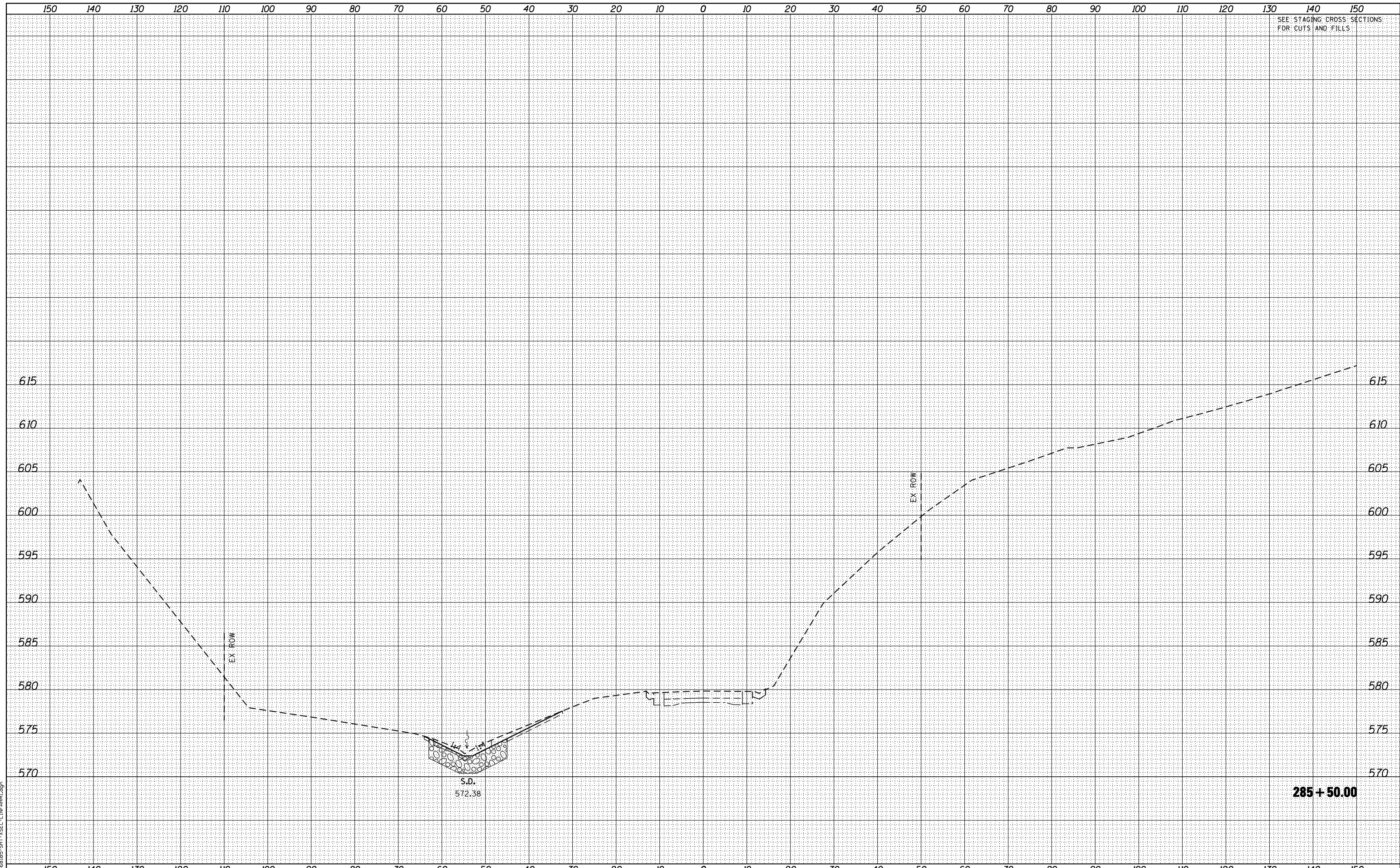
SEE STAGING CROSS SECTIONS FOR CUTS AND FILLS

DATE	
BY	
SURVEYED	
PLOTTED	
TEMPLATE	
AREAS CHECKED	
FINL SURVEY	
NOTE BOOK	
NO.	

DATE	
BY	
SURVEYED	
PLOTTED	
TEMPLATE	
AREAS CHECKED	
ORIGINAL SURVEY	
NOTE BOOK	
NO.	

DATE	
BY	
SURVEYED	
PLOTTED	
TEMPLATE	
AREAS	
CHECKED	
FINAL SURVEY	
NOTE BOOK	
NO.	

DATE	
BY	
SURVEYED	
PLOTTED	
TEMPLATE	
AREAS	
CHECKED	
ORIGINAL SURVEY	
NOTE BOOK	
NO.	



SEE STAGING CROSS SECTIONS FOR CUTS AND FILLS

285 + 50.00

design firm
no. 184001036
whks
engineers • planners • land surveyors

USER NAME = g.jameson	DESIGNED - JAC	REVISED
	CHECKED - RKA/CWC	REVISED
PLOT SCALE = 20.0000' / IN.	DRAWN - GSJ	REVISED
PLOT DATE = 8/14/2018	CHECKED - CWC/RKA	REVISED

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**PROPOSED CROSS SECTIONS - FARMINGTON ROAD
FARMINGTON ROAD IMPROVEMENT**

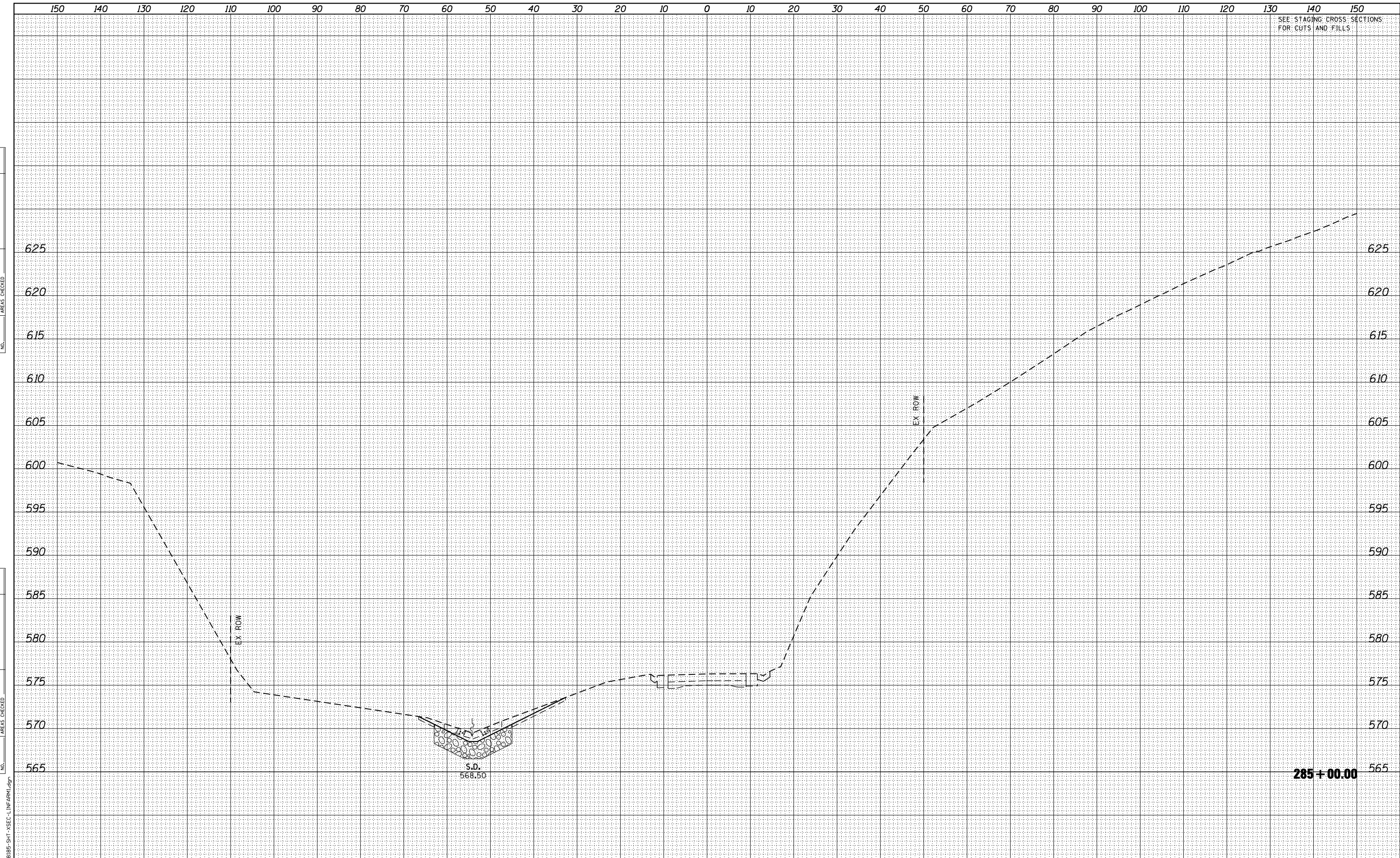
SCALE: SHEET NO. 4 OF 46 SHEETS STA. 285+50.00 TO STA. 285+50.00

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N,BR-1,RS-4,W-1)	PEORIA	577	499
			CONTRACT NO. 68185	

ILLINOIS FED. AID PROJECT

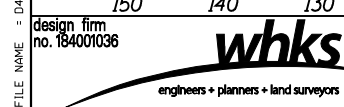
BY	DATE
FINAL SURVEY NO.	SURVEYED PLOTTED AREAS CHECKED

BY	DATE
ORIGINAL SURVEY NO.	SURVEYED PLOTTED AREAS CHECKED



SEE STAGING CROSS SECTIONS FOR CUTS AND FILLS

285+00.00



USER NAME = gjameson	DESIGNED - JAC	REVISED
	CHECKED - RKA/CWC	REVISED
PLOT SCALE = 20.0000' / IN.	DRAWN - GSJ	REVISED
PLOT DATE = 8/14/2018	CHECKED - CWC/RKA	REVISED

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

PROPOSED CROSS SECTIONS - FARMINGTON ROAD		
FARMINGTON ROAD IMPROVEMENT		
SCALE:	SHEET NO. 5 OF 46 SHEETS	STA. 285+00.00 TO STA. 285+00.00

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
6659	11(N,BR-1,RS-4,W-1)	PEORIA	577	500
CONTRACT NO. 68185				

ILLINOIS FED. AID PROJECT